

GENERAL NOTES

1. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO , THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS OR TIE-DOWNS MAY BE NECESSARY.

CODES AND SPECIFICATIONS

- A. GENERAL BUILDING CODE;
1. FLORIDA BUILDING CODE 2020.

- B. CONCRETE:
1. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-14)
2. SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301-16).

DESIGN LOADS

1. DESIGN LIVE LOADS
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| ROOF | 20 PSF |
| PRIVATE ROOMS | 40 PSF |
| BALCONY | 60 PSF |
| CORRIDORS/LANDINGS | 80 PSF |
| STAIRS | 100 PSF |
| MECH ROOMS | 100 PSF |
| PUBLIC ROOMS | 100 PSF |
2. DESIGN WIND LOAD SHALL BE BASED ON THE FLORIDA BUILDING CODE 2020.
- a) BASIC WIND SPEED = 140 MPH
b) BUILDING CATAGORY II
c) EXPOSURE 'C'
d) INTERNAL PRESSURE COEFFICIENT ±0.18 FULLY ENCLOSED STRUCTURE
e) COMPONENTS & CLADDING PRESSURES (ASD): ±34.4 & -44.9 PSF

FOUNDATIONS

1. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 5000 PSF AS RECOMMENDED IN THE FOUNDATION INVESTIGATION PREPARED BY UNIVERSAL ENGINEERING SCIENCES (PROJECT NO. 1563044v2) APRIL 25, 2019.
2. PLACE FOOTINGS/SLAB ON COMPACTED SOIL. FOLLOW RECOMMENDATIONS OF SOIL'S REPORT.

CAST IN PLACE CONCRETE

1. ALL CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:
- | | |
|-------------------------|----------|
| SLAB ON GRADE, FOOTINGS | 3000 PSI |
| REMAINING CONCRETE | 4000 PSI |
2. ALL CONCRETE SHALL HAVE A SLUMP OF 4" PLUS OR MINUS 1", AND HAVE 2 TO 4% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.58.
3. CONCRETE MIX DESIGN SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 CHAPTER 3, METHOD 1 OR METHOD 2. SUBMIT BACKUP DATA AS REQUIRED BY CHAPTER 5 SECTION 5.3. OF THE LATEST EDITION OF ACI 318.
4. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60.
5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-105. WWF SHALL BE LAPPED AT LEAST 8' AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 8'.
6. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH 'THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE' ACI 318 LATEST EDITION, AND 'SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS,' ACI 301.

1. ALL REINFORCING DETAILS SHALL CONFORM TO 'MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES' ACI 315 LATEST EDITION, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.
8. CONTRACTOR SHALL REVIEW ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF EMBEDDED ITEMS, SLEEVES, SLAB DEPRESSIONS, SLOPES, ETC. REQUIRED BY OTHER TRADES. THESE ITEMS SHALL BE FURNISHED AND INSTALLED PRIOR TO PLACEMENT OF CONCRETE.
9. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, ANCHOR BOLTS, INSERTS, ETC., AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED.

10. WHERE BAR LENGTHS ARE GIVEN ON THE DRAWINGS, THE LENGTH OF ANY HOOK, IF REQUIRED, IS NOT INCLUDED. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS AND AT SLABS EDGES.
11. CONTRACTOR SHALL PROVIDE SPACERS, CHAIRS, BOLSTERS, ETC. NECESSARY TO SUPPORT REINFORCING STEEL. SUPPORT ITEMS WHICH BEAR ON EXPOSED CONCRETE SURFACES SHALL HAVE ENDS WHICH ARE PLASTIC TIPPED OR STAINLESS STEEL.

12. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

- | | |
|--------|---|
| 3" | CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. |
| 2" | CONCRETE EXPOSED TO EARTH OR WEATHER, #6 THROUGH #18 BARS. |
| 1 1/2" | CONCRETE EXPOSED TO EARTH OR WEATHER, #5 BAR AND SMALLER. |
| 1 1/2" | CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH EARTH FOR THE PRIMARY REINFORCEMENT, TIES, STIRRUPS, AND SPIRALS IN BEAMS AND COLUMNS. |
| 3/4" | CONCRETE NOT EXPOSED TO WEATHER NOR IN CONTACT WITH EARTH FOR SLABS, WALLS, AND JOISTS, #1 BAR AND SMALLER. |
13. HORIZONTAL WALL AND FOOTING BARS SHALL BE BENT 1'-0" AROUND CORNERS OR CORNER BARS WITH 2'-0" LAP SHALL BE PROVIDED.
14. HORIZONTAL KEYWAYS IN CONSTRUCTION JOINTS SHALL BE PROVIDED IN BEAMS, SUPPORTED SLABS, AND WALL FOOTINGS WITH A DEPTH OF 1-1/2" AND HEIGHT EQUAL TO ONE-THIRD OF THE MEMBER'S DEPTH. REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS UNLESS OTHERWISE NOTED ON THE DRAWINGS. CONSTRUCTION JOINTS MAY BE USED ONLY AT LOCATIONS SHOWN ON THE DRAWINGS OR AT OTHER LOCATIONS APPROVED BY THE ARCHITECT.
15. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 48 BAR DIAMETERS TYP. EXCEPT WHERE OTHERWISE NOTED ON THE DRAWINGS. FOR BEAMS AND ELEVATED SLABS, LAP BOTTOM STEEL AT THE SUPPORT AND TOP STEEL OVER THE MIDSPAN, UNLESS OTHERWISE NOTED.
16. TESTING LABORATORY SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE ENGINEER.

17. REINFORCING STEEL TO BE WELDED: ASTM A 106.

STRUCTURAL STEEL

- A. STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATION:
- W-SHAPES (UNO.), A572(GR50)
 - STRUCTURAL STEEL PIPE.....A53 (FY=35)
 - STRUCTURAL STEEL TUBE.....A500 (FY=46)
 - BASE PLATES AND CONNECTION PLATES(UNO.).....A36 (FY=36)
 - ALL OTHER STEEL.....A36 (FY=36)
- B. BOLTED CONNECTIONS:
- LOAD-INDICATOR WASHERS MUST BE USED FOR FIELD CONNECTIONS.
 - ALL BEAM TO COLUMN CONNECTIONS, BRACE CONNECTIONS AND MOMENT CONNECTED MEMBERS SHALL HAVE A325-N
 - ALL OTHER CONNECTIONS (UNLESS NOTED OTHERWISE) 3/4" DIAMETER, A325 N
 - OVERSIZED AND LONG-SLOTTED HOLES NOT ALLOWED UNLESS SHOWN ON STRUCTURAL DRAWINGS.
 - THE SHOP DRAWINGS SHALL CLEARLY INDICATE THE TYPE OF BOLT USED IN EACH CONNECTION AND THE ALLOWABLE VALUES USED FOR THE VARIOUS BOLT TYPES.
 - THE FOLLOWING MINIMUM STANDARDS APPLY:
a) MINIMUM PLATE THICKNESS:....3/8"
b) MINIMUM BOLT DIAMETER:....3/4"
c) MINIMUM WELD:.....3/16" THICK THROAT
d) MINIMUM DESIGN LOAD ON ANY CONNECTION:10 KIPS
- C. WELDED CONNECTIONS:
- ALL SHOP AND FIELD WELDING SHALL CONFORM TO A5W STRUCTURAL WELDING CODE-STEEL, ANSI/AWS D11
 - MINIMUM WELD: 3/16" THICK THROAT.
 - ALL STEEL TO STEEL CONNECTIONS NOT SHOWN BOLTED SHALL BE WELDED TO DEVELOP FULL SHEAR CAPACITY OF CONNECTING MEMBERS AS PER AISC SPECIFICATIONS. MINIMUM SIZE OF FILLET WELD (UNLESS NOTED OTHERWISE ON DRAWINGS):
MATERIAL THICKNESS OF MINIMUM SIZE OF THICKER PART JOINED OF FILLET WELD
1/4" TO 1/2" 3/16" ALL AROUND
OVER 1/2" TO 3/4" 1/4" ALL AROUND
OVER 3/4" TO 1 1/2" 5/16" ALL AROUND
- D. SPLICING OF STRUCTURAL STEEL WHERE NOT DETAILED IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER.

E. BEAM CONNECTIONS:

- DESIGN CONNECTIONS FOR BEAMS TO SUPPORT HALF OF THE UNIFORM CAPACITY SHOWN IN AISC 'TABLES FOR ALLOWABLE LOADS ON BEAMS' FOR THE GIVEN SECTION AND SPAN UNLESS OTHERWISE SHOWN.
 - EXAMINE DRAWINGS FOR OTHER ITEMS TO BE GALVANIZED.
- G. GALVANIZING:
- HOT-DIP GALVANIZE AFTER FABRICATION ALL STRUCTURAL STEEL ITEMS AND THEIR CONNECTIONS PERMANENTLY EXPOSED TO THE OUTSIDE.
 - EXAMINE DRAWINGS FOR OTHER ITEMS TO BE GALVANIZED.
- G. ANCHOR BOLTS EXCEPT FOR STEEL TRUSSES:
- ALL ANCHOR BOLTS SHALL BE MADE FROM THREADED ROUND STOCK, ASTM A36. ALL ANCHOR BOLTS, NUTS AND WASHERS USED WITH GALVANIZED BASE PLATES SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.
 - NUTS SHALL BE HEX HEAD, ASTM A563.
 - WASHERS FOR ALL BASE PLATES SHALL BE 1/4" THICK PLATES EXTENDING MINIMUM 1" FROM EDGE OF BASE PLATE HOLES ON EACH SIDE WITH HOLES 1/16" LARGER THAN THE NOMINAL BOLT DIAMETER AND CONFORM TO ASTM A36.
 - ALL ANCHOR BOLTS SET IN CONCRETE SHALL UTILIZE TEMPLATES. TEMPLATES SHALL BE DETAILED ON THE SHOP DRAWINGS.
- H. NON-SHRINK GROUT FOR BASE PLATES AND BEARING PLATES
- NON-METALLIC, SHRINKAGE RESISTANT, PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING PORTLAND CEMENT, SILICA SANDS, SHRINKAGE COMPENSATING AGENTS, AND FLUIDITY IMPROVING COMPOUNDS, AND SHALL CONFORM TO CORPS OF ENGINEERS SPECIFICATION FOR NON-SHRINK GROUT, CRD -C621-83.
 - TWENTY- EIGHT DAY COMPRESSIVE STRENGTH SHALL BE A MINIMUM OF 6,000 PSI.

- J. SHEAR CONNECTORS(HEADED STUDS):
- MECHANICAL REQUIREMENTS STUD SHALL BE MADE FROM COLD DRAW BAR STOCK CONFORMING TO ASTM A 108, ' SPECIFICATION FOR STEEL BARS, CARBON COLD FINISHED, STANDARD QUALITY' GRADES 1010 THROUGH 1020 SEMI-KILLED OR FULLY KILLED. TENSILE STRENGTH SHALL BE 60,000 PSI MINIMUM WITH 20% MINIMUM ELONGATION IN 2 INCHES AND 50% MINIMUM AREA REDUCTION WHEN TESTED IN ACCORDANCE WITH APPLICABLE SECTIONS OF ASTM A310, 'MECHANICAL TESTING OF STEEL PRODUCTS'.

- DESIGN. THE DESIGN OF THE STUDS SHALL BE SUITABLE FOR ARC WELDING TO STEEL MEMBERS WITH AUTOMATICALLY TINED STUD WELDING EQUIPMENT.
- FERRULE. AN ARC SHIELD (FERRULE) OF HEAT RESISTANT CERAMIC SHALL BE FURNISHED WITH EACH STUD. AFTER WELDING, ALL ARC SHIELDS SHALL BE REMOVED FROM EACH SHEAR CONNECTOR.
- DIMENSIONS. STUDS SHALL BE OF UNIFORM DIAMETER, HEADS SHALL BE CONCENTRIC AND NORMAL TO THE SHAFT, AND THE WELD END SHALL BE CHAMFERED. STANDARD DIMENSIONS AND TOLERANCES SHALL BE AS SPECIFIED IN AWS D11.
- SHEAR CONNECTORS MUST BE INSTALLED AFTER METAL DECK INSTALLATION.
- EPOXY ANCHOR FOR MISC. STEEL ATTACHMENT SHALL BE 3/4"DIAx6" HILTY HY-150.

STEEL ROOF DECK

- ROOF DECK SHALL BE AS FOLLOWS:
1). ALL METAL ROOF DECK EXCEPT WHERE SHOWN ON PLAN SHALL BE 1 1/2" DEEP 22 GAGE WIDE RIB, MIN PROPERTIES: 9p-0.186 IN3 , 9n-1.92 IN3 , 1-0.163 IN4, GALVANIZED G90.
 - ROOF DECK SHALL BE PLACED IN AT LEAST TWO SPAN SEGMENTS. NO SINGLE SPAN CONDITIONS SHALL BE USED.
 - STEEL DECK SHALL CONFORM TO ASTM A446 GRADES A, B, C, D, E OR F FOR GALVANIZED DECK, MINIMUM YIELD STRENGTH OF 33,000 PSI.
 - STEEL DECK SHALL BE GALVANIZED WITH A PROTECTIVE ZINC COATING CONFORMING TO ASTM A525 G90 CLASS.
5. ATTACHMENT:
- 1) WELDING:
a) ROOF DECK UNITS SHALL BE WELDED TO EACH STRUCTURAL SUPPORT MEMBER USING 5/8" DIAMETER PUDDLE WELDS AT ALL RIBS (3/6" FASTENER LAYOUT). WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND SIDE JOINTS AND SHALL BE COMPLETELY FUSED TO THE SUPPORTING MEMBERS.
b) SIDE LAPS OF ADJACENT UNITS SHALL BE FASTENED BY WELDING (ON 18 GAUGE OR HEAVIER DECK ONLY) OR SHEET METAL SCREWS, SO THAT SPACING BETWEEN FASTENERS AND BETWEEN THE FIRST FASTENER AND SUPPORT DOES NOT EXCEED 18 INCHES.
c) AT ALL ROOF OPENINGS AND PERIMETER ROOF EDGE CONDITIONS, ROOF DECK UNITS SHALL BE WELDED TO EDGE STEEL USING 5/8"DIAMETER PUDDLE WELDS AT 6' O.C.
- 2) PROVIDE A MINIMUM END BEARING OF 2" OVER SUPPORTS.
- 3) END LAPS OF SHEETS SHALL BE A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS. ROOFS SHALL BE ERECTED BEGINNING AT THE LOW SIDE TO INSURE THAT END LAPS ARE SINGLE FASHION.
- 4) ATTACH DECK TO EDGE SUPPORT WITH 5/8" DIA PUDDLE WELD @ 6" O.C.

PRE-ENGINEERED LIGHT GAGE METAL STUD

- A. THE FABRICATOR SHALL FURNISH A STRUCTURAL SUBMITTAL BEARING THE SEAL AND SIGNATURE OF A STRUCTURAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. THIS SUBMITTAL SHALL BE CHECKED BY THE CONTRACTOR FOR COMPLETENESS AND CONTENT PRIOR TO SUBMITTAL TO THE STRUCTURAL ENGINEER OF RECORD FOR REVIEW. THE SUBMITTAL SHALL INCLUDE COMPONENT DETAILS AND SYSTEM LAYOUT DRAWINGS, IT SHALL IDENTIFY THE PROJECT LIST LOADING AND OTHER CRITERIA. THE DRAWINGS SHALL IDENTIFY AND LOCATE COMPONENTS AND SHALL SPECIFY MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS (SUCH AS STUDS TO CONC SLAB, TO CMU WALL, TO STL MEMBER, ETC.) & ALL OTHER NECESSARY FABRICATION AND ERECTION INFORMATION. THE SUBMITTAL SHALL INCLUDE CALCULATIONS VERIFYING ITS ADEQUACY TO RESIST THE LOADS INDICATED ON THE CONSTRUCTION DOCUMENTS. FABRICATION SHALL NOT COMMENCE UNTIL THIS REVIEW IS COMPLETED.
- B. DESIGN OF LIGHTGAUGE METAL FRAMING SHALL CONFORM TO THE LATEST EDITION OF 'SPECIFICATION FOR THE DESIGN OF COLD-FORMED STRUCTURAL STEEL MEMBERS'(AISI).
- C. MATERIALS: STUDS, RUNNERS AND ANGLES SHALL MEET THE REQUIREMENTS OF ASTM 1446 WITH MINIMUM YIELD STRENGTH AS FOLLOWS:

- | | |
|---------------------------|--------|
| 16, 14, AND 12 GAGE STUDS | 50 KSI |
| 22, 20, AND 18 GAGE STUDS | 33 KSI |
| RUNNERS | 33 KSI |
- D. METAL STUDS SHALL BE 'C' SHAPED STUDS
- E. PROVIDE MANUFACTURER'S STANDARD STEEL RUNNERS, BLOCKING, LITELS, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS AND ACCESSORIES AS RECOMMENDED BY MANUFACTURER AND/OR SHOWN ON DRAWINGS FOR APPLICATIONS INDICATED, AS NEEDED TO PROVIDE A COMPLETE STEEL FRAMING SYSTEM.
- F. GALVANIZED FINISH SHALL COMPLY WITH ASTM A525 WITH A G90 COATING. ALL WELDS SHALL BE TOUCHED UP WITH A ZINC-RICH PROTECTIVE PAINT FOR CORROSION RESISTANCE.
- G. ALL STUDS SHALL BE FULL LENGTH. NO SPLICING PERMITTED UNLESS SPECIFICALLY DETAILED.
- H. ALL METAL TO METAL CONNECTIONS TO BE WELD OR SCREW ATTACHMENTS AS SHOWN ON DRAWINGS OR AS REQUIRED BY MANUFACTURER.

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SHOP DRAWING SUBMITTALS

- A. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL.
- B. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS:
- STRUCTURAL STEEL
 - REINFORCING STEEL
 - STEEL STAIRS (*)
 - CONCRETE MIX DESIGNS
 - LIGHT GAUGE METAL STUD AND CONNECTION WITH CALCULATIONS (+).
 - H.C. PLANK (+)
- ITEMS MARKED (+) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. ITEMS MARKED (*) SHALL BE SUBMITTED FOR ENGINEER'S RECORD ONLY.
- C. MANUFACTURER'S LITERATURE: SUBMIT TWO COPIES OF MANUFACTURER'S LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.
- D. DESIGN CALCULATIONS: THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW TWO SET OF DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA, FOR THE FOLLOWING ITEM:
- STRUCTURAL STEEL CONNECTIONS.
 - STRUCTURAL STAIRS

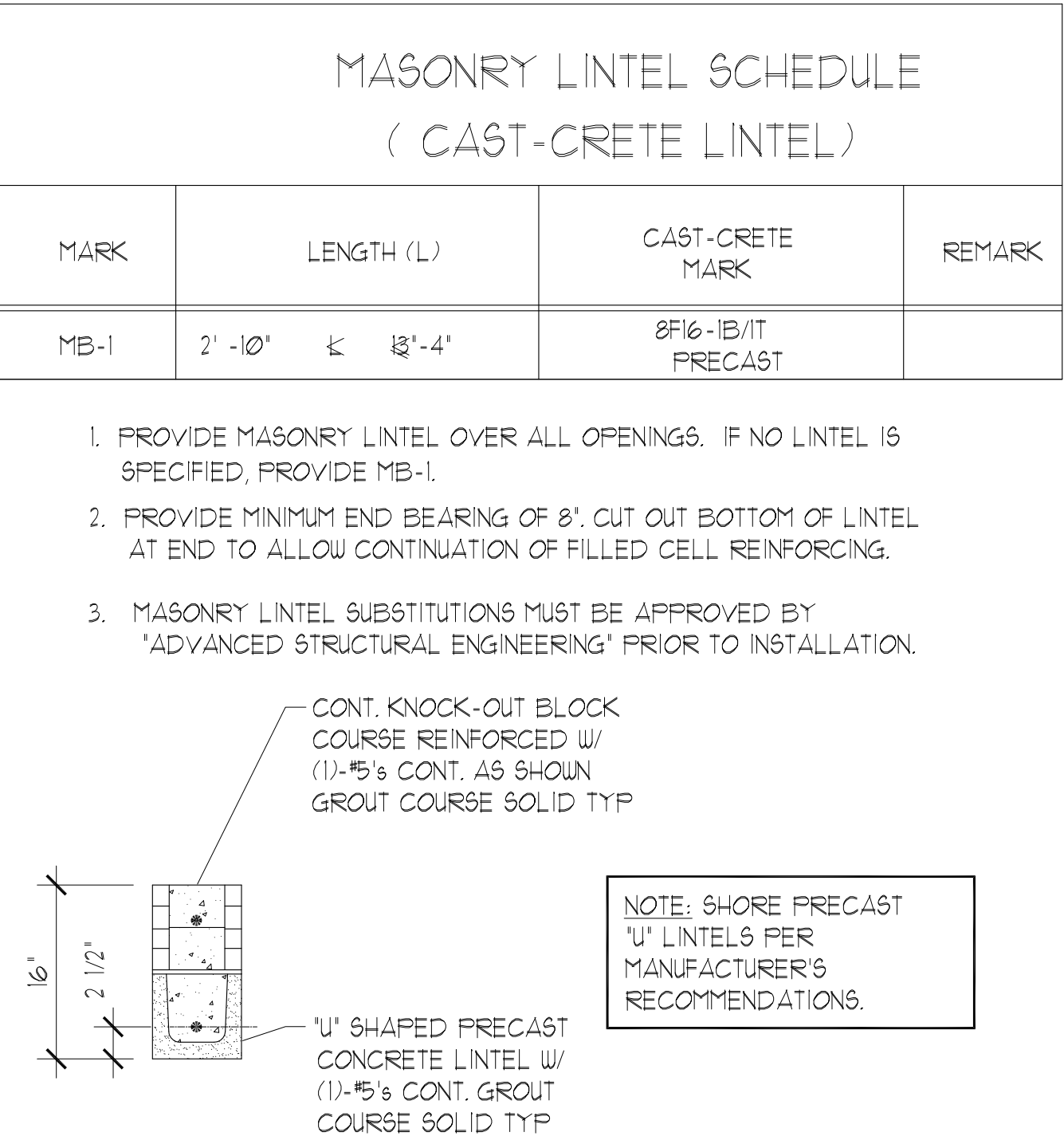
MASONRY WALL CONSTRUCTION

- A. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM COMPRESSIVE STRENGTH OF (F'm = 1500 PSI). FOR SECOND FLOOR ABOVE.
- A MINIMUM COMPRESSIVE STRENGTH OF F'm = 2500 PSI FOR FIRST FLOOR (FOOTING TO SECOND FLOOR).
- B. COURSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI FOR SECOND FLOOR ABOVE.
- A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI FOR FIRST FLOOR (FOOTING TO SECOND FLOOR).
- C. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.
- D. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH CELLS FILLED WITH COARSE GROUT.
- E. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 4'-0". REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL WITH DURO-WAL BAR POSITIONER D/A 811 TYPICAL UNLESS OTHERWISE NOTED. SEE TYPICAL GROUTING DETAILS FOR ADDITIONAL INFORMATION.
- F. REINFORCING STEEL SHALL BE LAPPED MINIMUM 48 BAR DIAMETERS WHERE SPLICED UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- G. HORIZONTAL WALL REINFORCEMENT SHALL BE STANDARD TRUSS TYPE DUR-O-WAL (9 GA. SIDE X 9 GA. CROSS) @ 16" O.C., UNLESS SHOWN OTHERWISE ON DRAWINGS.
- H. SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6' AND CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6'. LAP WITH STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.

- I. FOUNDATION DOWELS SHALL BE POSITIONED SUCH THAT IT IS ALIGNED WITH THE CENTER OF THE MASONRY UNITS VERTICAL CORE AND GROUTED SOLID. SHOULD THE FOUNDATION DOWEL NOT ALIGN WITH THE MASONRY UNITS VERTICAL CORE - NOTIFY THE ARCHITECT / STRUCTURAL ENGINEER FOR DIRECTION ON HOW TO CORRECT THE MIS ALIGNMENT.
- J. PROVIDE PRECAST CONCRETE LITELS OVER ALL OPENINGS UNLESS NOTED OTHERWISE ON DRAWINGS. LITELS SHALL BE OF SUFFICIENT SIZE AND REINFORCEMENT FOR THE GIVEN SPANS AND LOADING CONDITIONS. SUBMIT SHOP DRAWINGS WITH RATED LOAD CAPACITIES TO THE ARCHITECT FOR REVIEW.
- K. PROVIDE A KNOCK OUT BLOCK OR U-BLOCK REINFORCED WITH (1)-#5 CONTINUOUS AT THE GILL OF ALL WINDOW OPENINGS. EXTEND 8" BEYOND EACH SIDE OF THE OPENING TYPICALLY.

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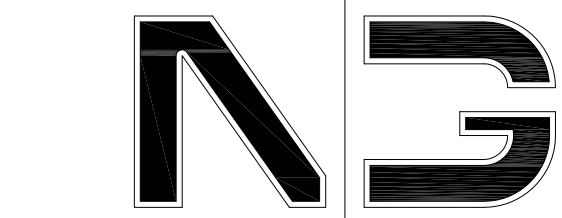
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MB-1

MASONRY LITEL SCHEDULE

ASE ENGINEERING SERVICES, INC.
STRUCTURAL DESIGN GROUP
"TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODES, THE RATING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS."
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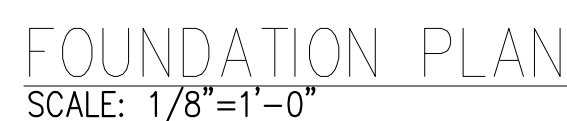
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GENERAL NOTES

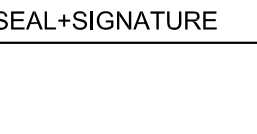
PROJECT NAME		
COURTYARD INN®, Lake City, Florida		
SEAL+SIGNATURE	DATE	07 DEC 2021
	PROJECT NUMBER	2K2101
	DRAWING NUMBER	
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 TO THE BEST OF ITS KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS
 COMPATIBLE WITH BUILDING STRUCTURE DESIGN AND REALIZATION OF THE PLANS
 AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED
 BY WIND, LIVE AND GRAVITY LOADS.
 10244 East Colonial Drive, Suite 202
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 Certificate of Authorization No. 25873
 Minsheng Xie P.E.
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PROJECT NAME	
COURTYARD INN[®] Lake City, Florida	
SEAL+SIGNATURE	DATE 07 DEC 2021
	PROJECT NUMBER 2K2101
	DRAWING NUMBER
	DECOR: XH
	PAGE NUMBER



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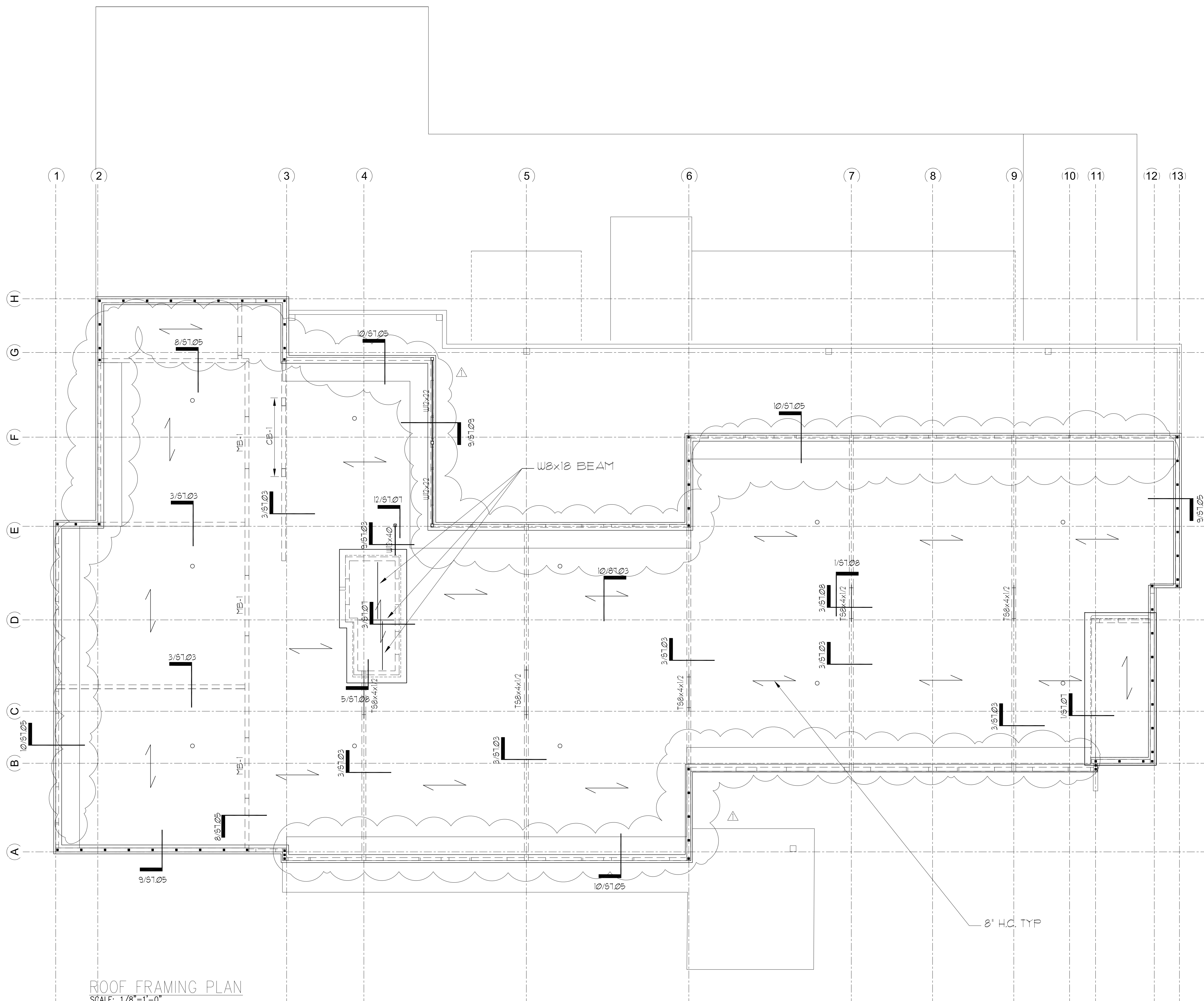
COURTYARD[®]
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ROOF FRAMING PLAN

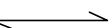
COURTYARD INN,[®]
Lake City, Florida

SEAL+SIGNATURE	DATE	07 DEC 2021
S6.01	PROJECT NUMBER	2K2101
	DRAWING NUMBER	
	DECOR:	XH
	PAGE NUMBER	
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ROOF FRAMING PLAN
SCALE: 1/8"=1'-0"

PLAN NOTES:

- [] SEE GENERAL NOTES ON SHEET S101.
- [2] DO NOT SCALE DRAWINGS. SEE ARCH'L. DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN. VERIFY ALL DIMENSIONS WITH ARCH'L. DRAWINGS PRIOR TO START OF CONSTRUCTION. IF DISCREPANCIES SHOULD OCCUR - CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
- [3]  INDICATES 8" MASONRY PARAPET WALLS REINFORCED WITH (1) #5 BAR (VERT.) AT 48" O.C. MAX. GROUT SOLID CONC. ALL THE FILLED CELLS. PROVIDE ADDITIONAL BARS AS SHOWN ON PLAN AT ALL CORNERS, INTERSECTIONS, ADJACENT MASONRY OPENINGS AND ENDS OF WALLS.
- [4] 8" PRESTRESSED HOLLOW CORE SLAB UNO.
- [5]  HOLLOW CORE SPAN.
- [6] SEE ARCH'L. DRAWINGS FOR LOCATIONS / LIMITS AND CONSTRUCTION INFORMATION OF INTERIOR NON-BEARING PARTITION WALLS NOT SHOWN ON PLAN. SEE GENERAL NOTES.
- [7] COORDINATE ALL SLAB (TOPPING) SLOPES AND DEPRESSIONS WITH ARCH'L. DRAWINGS. (FOR ACTUAL TOP OF SLAB ELEVATIONS, SEE ARCH'L. DRAWINGS)
- [8] PROVIDE MASONRY LINTEL OVER ALL OPENINGS. IF NO LINTEL TYPE IS INDICATED, PROVIDE TYPE MB-1 FOR 8" CMU WALL. SEE MASONRY LINTEL SCHEDULE ON SHEET S101. PROVIDE SHORING DURING CONSTRUCTION, IF LINTEL SPAN MORE THAN SIX FEET.
- [9] PLACE THE PLUMBING SLEEVES INTO H.C. PLANKS, CONCRETE BEAMS AND COLUMNS. SEE PLUMBING DRAWINGS FOR THE LOCATION OF SLEEVES.
- [10]  INDICATES MASONRY CONTROL JOINT. SEE DETAIL.
- [11] PROVIDE DOUBLE K.O. BOND BEAM AT ROOF LEVEL WITH (1) #5 AT EACH COURSE.
- [12] CB-1: 2x16 W/ (2) #1 TOP AND BOTTOM AND #3 TIES AT 6" O.C.

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Technical drawing of a column base plate. The drawing shows a central square column block with a diamond-shaped reinforcement pattern. Dimensions include 2" for the central block, 6" for the base plate, and 8" for the overall width. A 1/4" dimension is shown for a small circular feature. The drawing is labeled with dimensions and a note about the column block.

1/4"

2"

6"

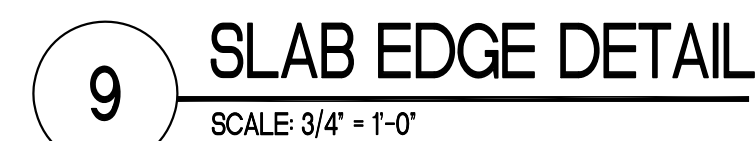
8"

8"

8"

(4)-3/4" DIA A36 ANCHOR BOLTS W/ DOUBLE NUTS & WASHERS.

COLUMN BLOCK-OUT 2'-6"x2'-6" (SHAPED, FILLED W/ CONC. AFTER COLUMN HAS BEEN INSTALLED. (TOP MATCH THAT OF ADJACENT



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DRAWING NAME

SECTIONS AND DETAILS

PROJECT NAME

COURTYARD INN,[®]
Lake City, Florida

	PROJECT NUMBER 2K2101
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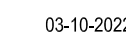
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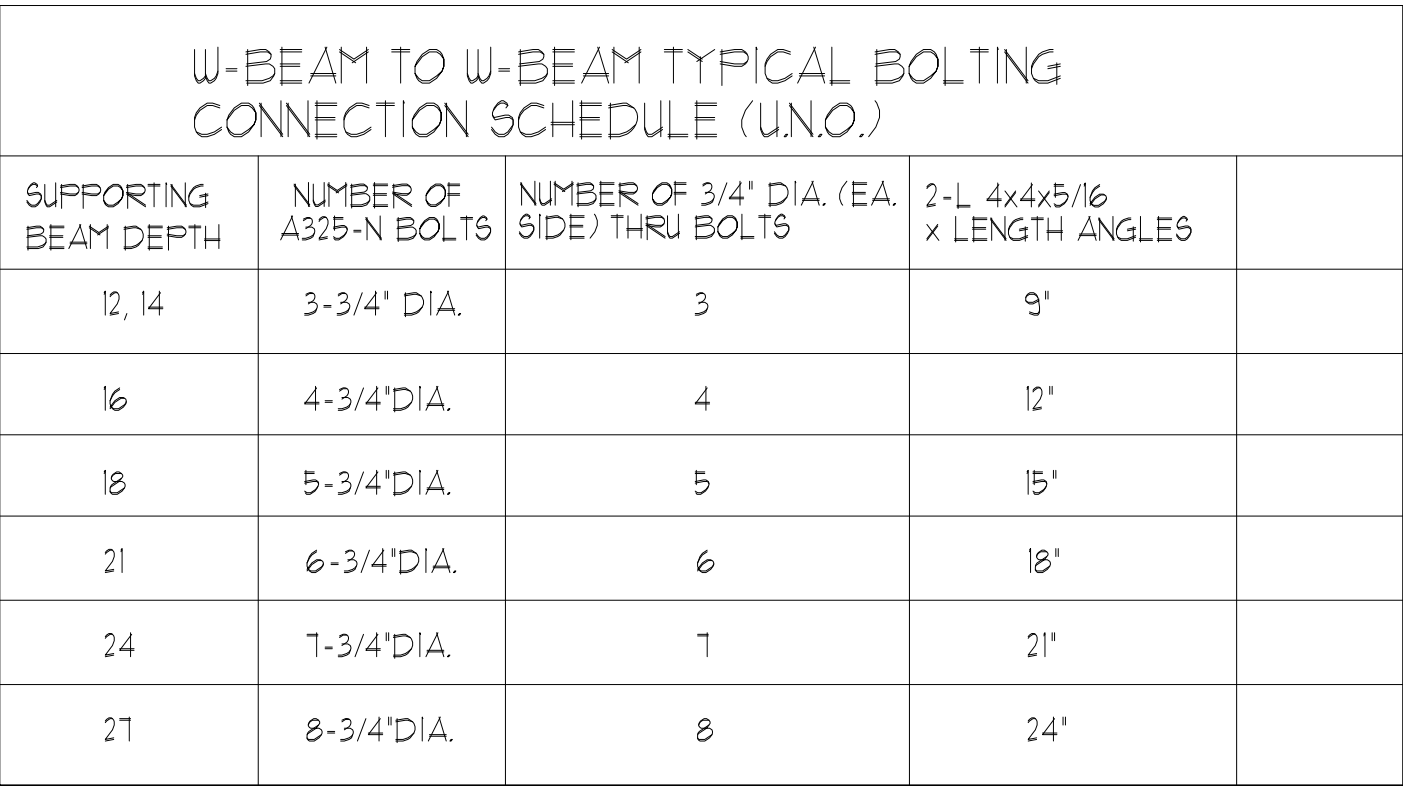
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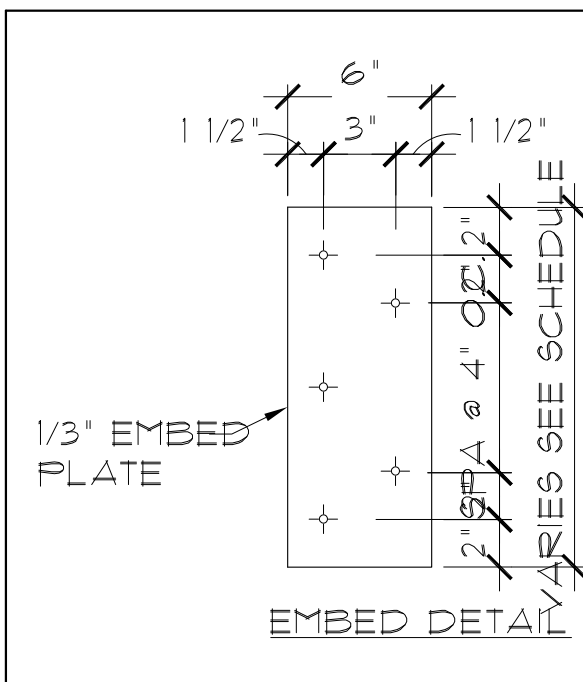


2 TYPICAL BEAM TO BEAM CONNECTION

SCALE: N.T.S.

STL BM TO CONC CONNECTION SCHEDULE				
STEEL BEAM SIZE	STEEL CLIP ANGLE CONNECTION		CONCRETE EMBED CONNECTION	
	2-L 4x3x5/16 ANGLE LENGTH (IN)	BOLTS	EMBED PLATE	HEADED STUDS (6" LG MIN)
W24, W27	18	6	1/2 x 15 x 2'-0"	(12) 3/4" DIA IN (4) ROWS
W18, W21	15	5	1/2 x 15 x 1'-6"	(9) 3/4" DIA IN (3) ROWS
W14, W16	12	4	1/2 x 15 x 1'-3"	(9) 3/4" DIA IN (2) ROWS
W12	9	3	1/2 x 15 x 1'-3"	(6) 3/4" DIA IN (2) ROWS
STL JOIST TO CONC	SEE SECT	N/A	1/2 x 12 x 1'-0"	(4) 3/4" DIA IN (2) ROWS

NOTE: 1 ALL STUDS ARE 6" AND EQUALLY SPACED
2 MINIMUM EDGE DISTANCE FROM CL OF STUDS TO FACE OF CONCRETE SHALL NOT BE LESS THAN 4" WITHOUT APPROVAL BY ENGINEER
3 PROVIDE NELSON TYPE STUDS OR APPROVED EQUAL BY ARCHITECT

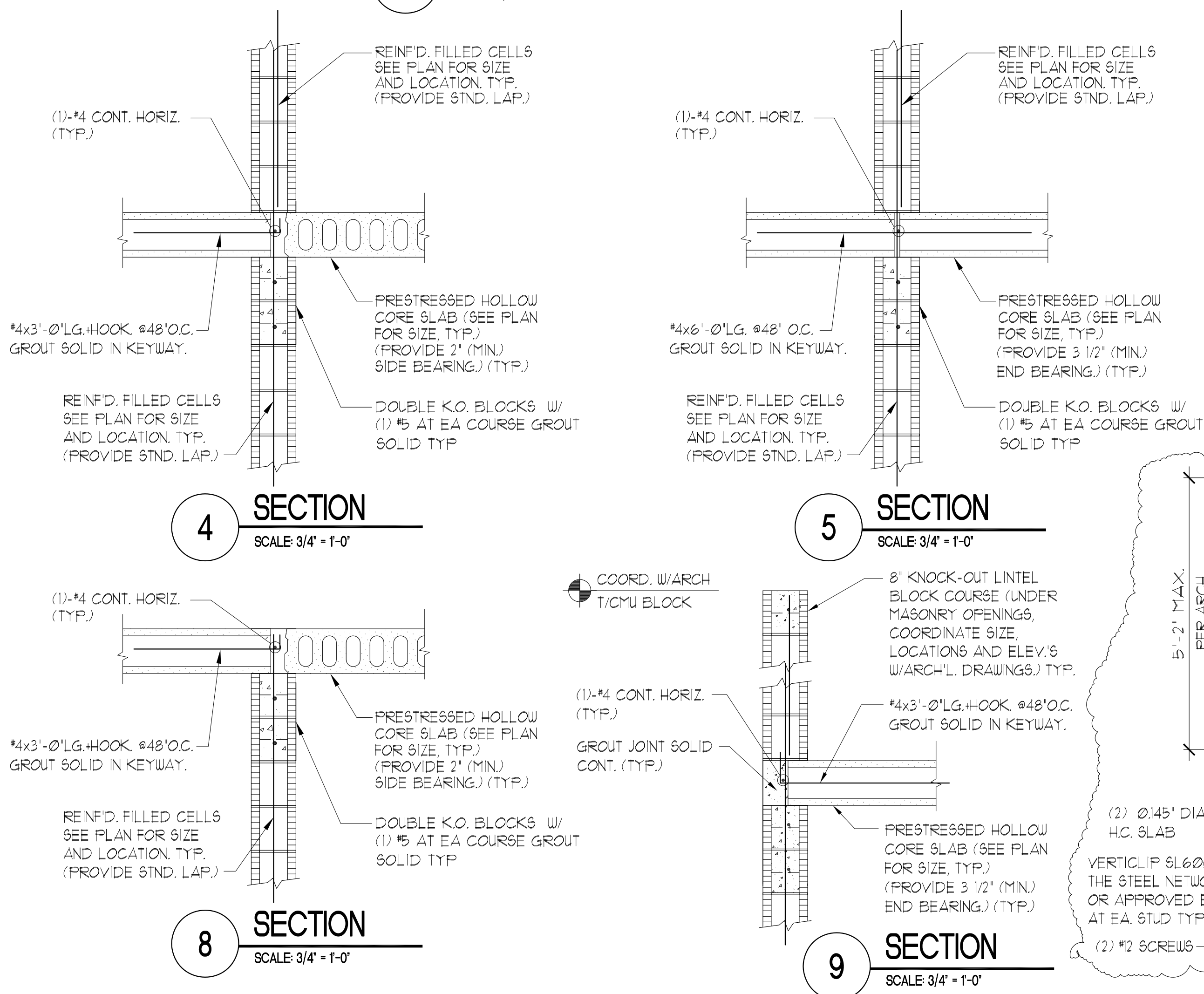


STL BM TO CONCRETE CONNECTION SCHEDULE AT END OF WALLS AND NARROW COLUMN FACE

STEEL BEAM SIZE	# OF BOLTS	SINGLE PLATE SIZE		SINGLE PLATE WELD TO EMBED	# OF STUDS	EMBED PLATE LENGTH
		t	l			
W10, W12	(3)	5/16"	9"	1/4" FILLET	(4)	12"
W14, W16	(4)	5/16"	12"	1/4" FILLET	(5)	16"
W18	(5)	3/8"	15"	5/16" FILLET	(6)	20"
W21	(6)	3/8"	18"	5/16" FILLET	(7)	24"
W24, W27	(7)	3/8"	21"	5/16" FILLET	(8)	28"

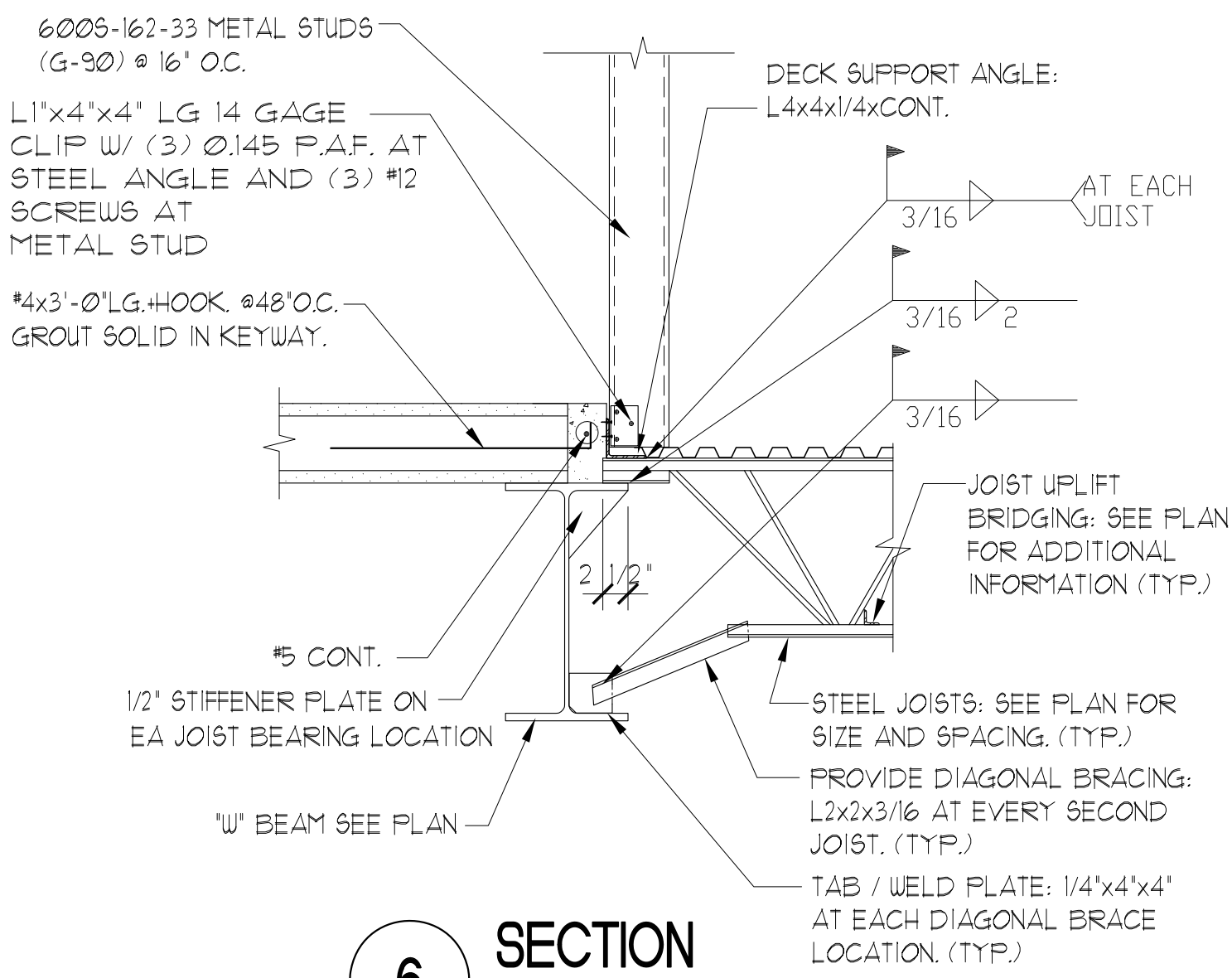
1 STL BM TO CONC CONNECTION SCHEDULE

SCALE: 3/4" = 1'-0"



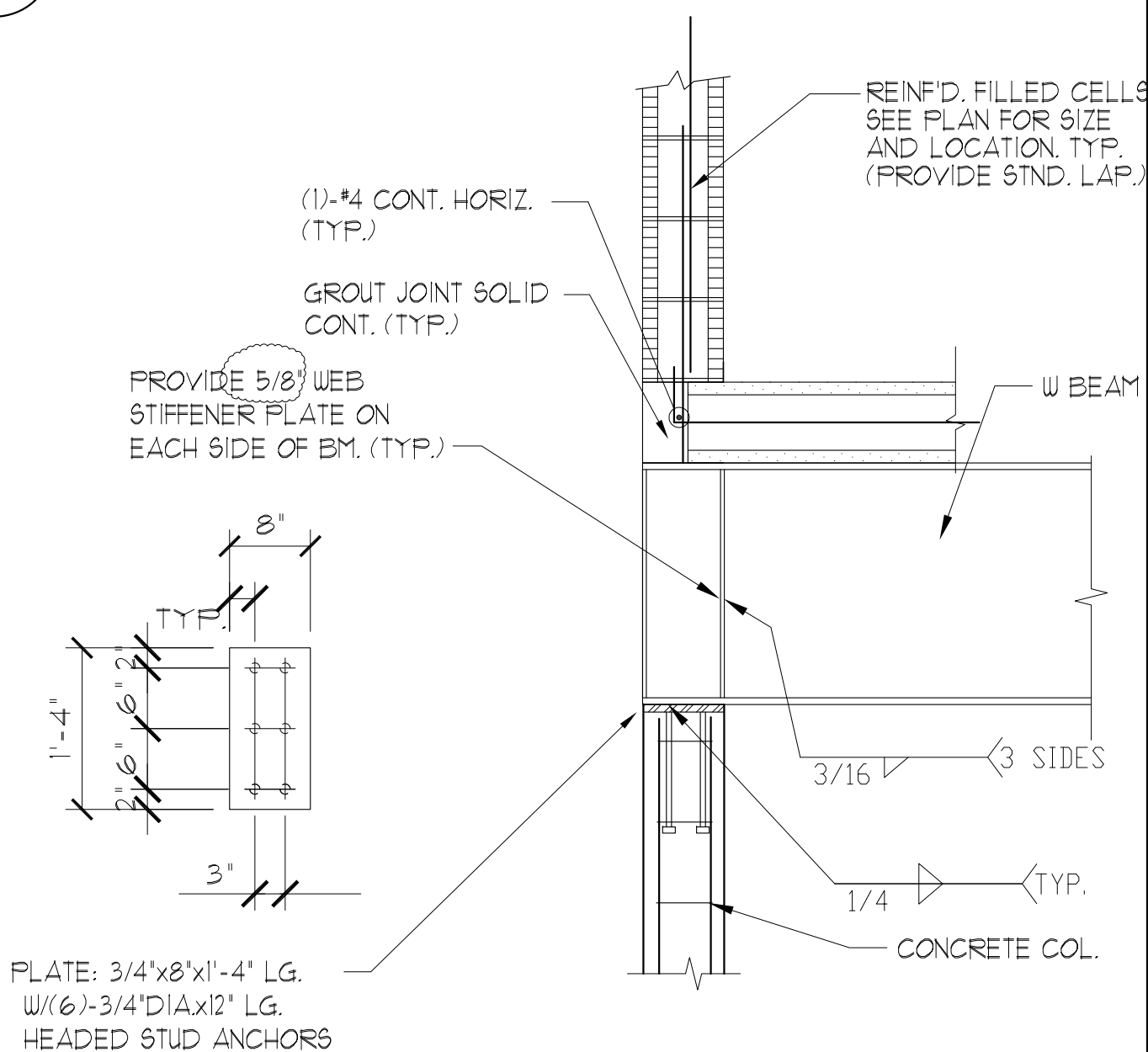
6 SECTION

SCALE: 3/4" = 1'-0"



SECTION

SCALE: 3/4" = 1'-0"



ROOF MECH UNIT FRAMING

SCALE: 3/4" = 1'-0"

ROOF TOP UNIT FRAME SCHEDULE	
UNIT WEIGHT	ANGLE SIZE:
0 - 675 LBS.	L4x3x1/4 (L.L.H.)
676 - 1500 LBS.	L4x4x5/16
1501 - 3000 LBS.	L6x3x3/8 (L.L.H.)
3001 - 6000 LBS.	L6x6x3/8
6001 - 10000 LBS.	L8x6x3/4

NOTE:
SEE SECTION 3/59 FOR
ADDITIONAL JOIST REINFORCING
INFORMATION. (AS REQUIRED)

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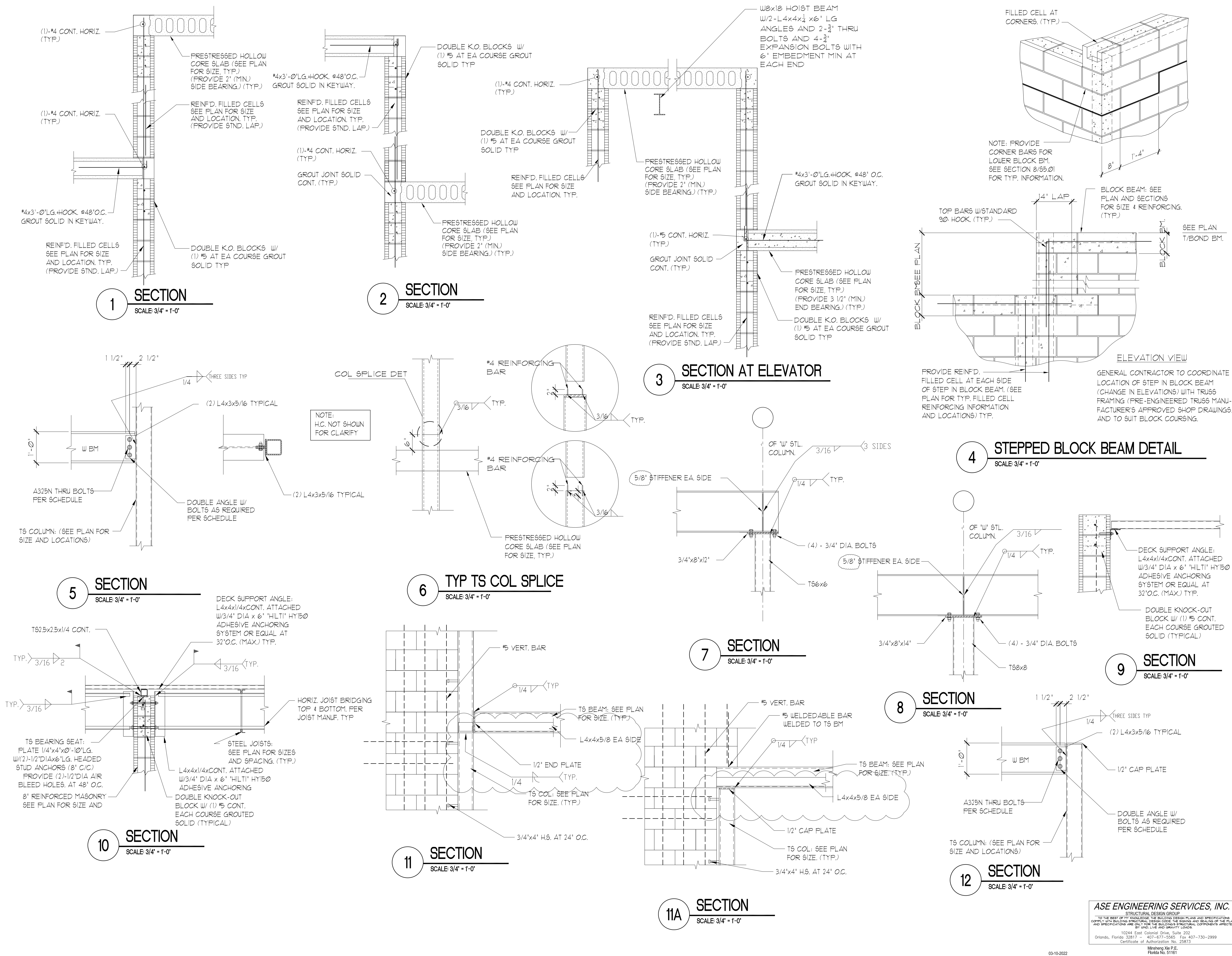
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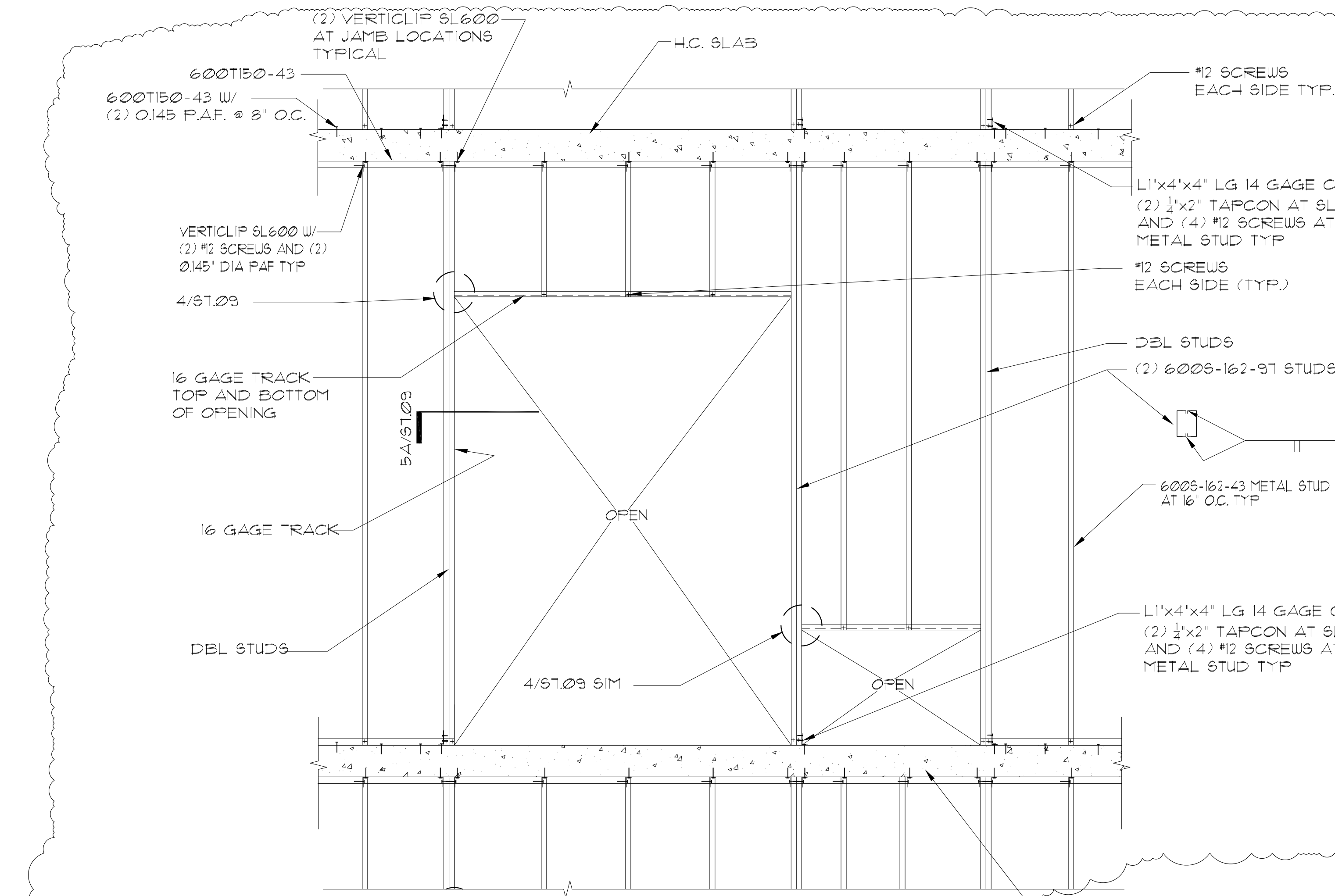
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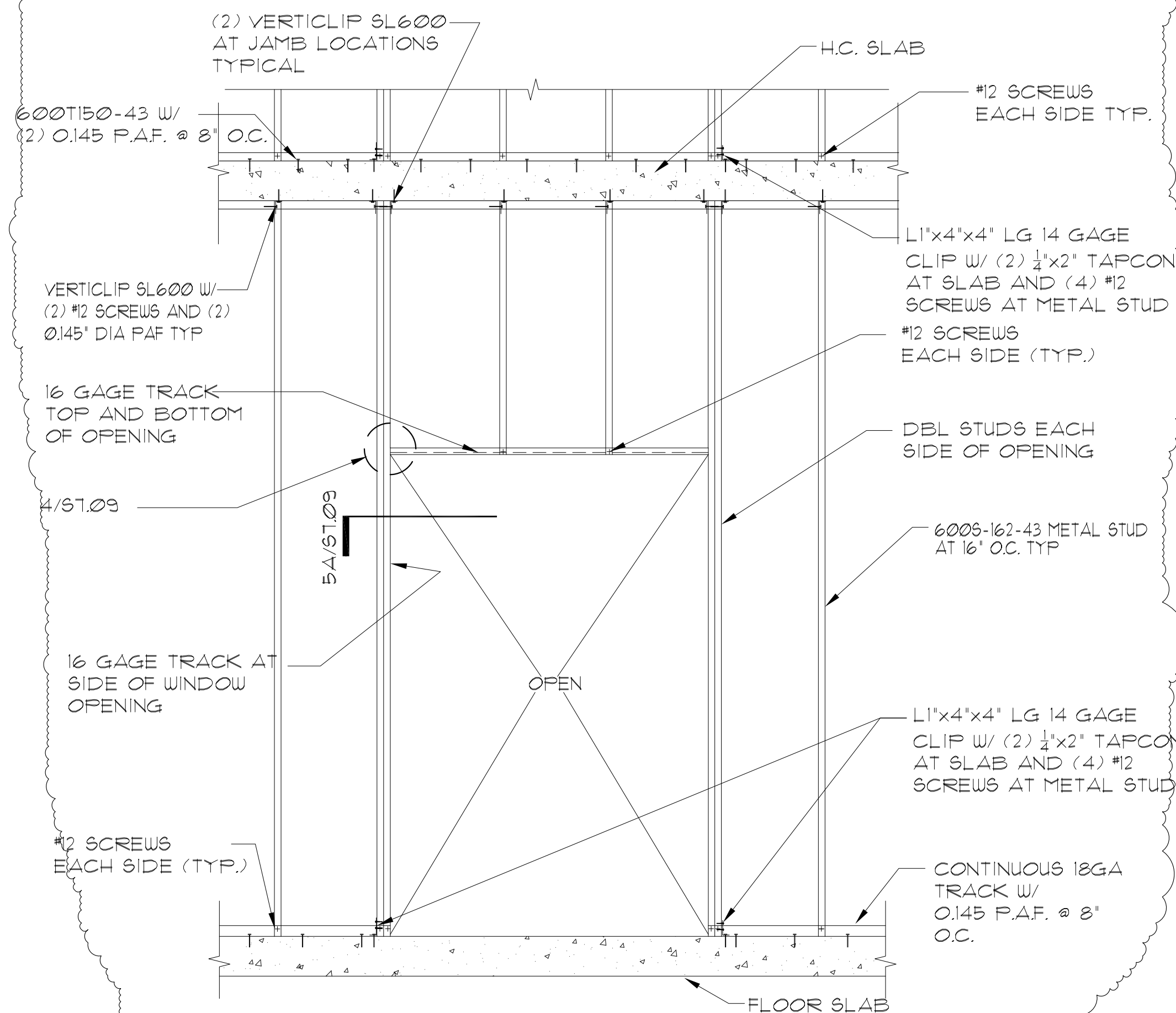
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S7.09



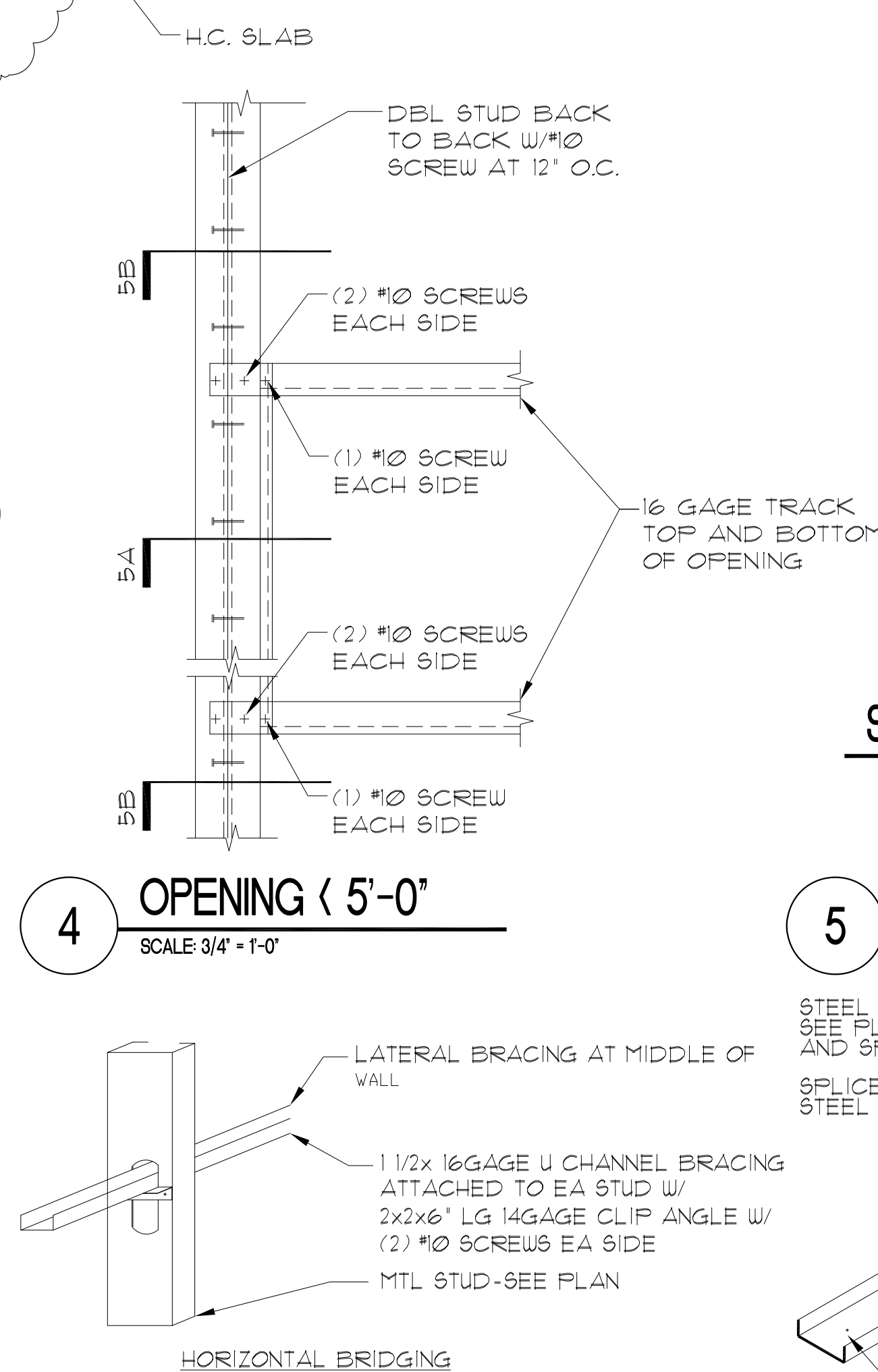
1
TYPICAL EXTER. METAL STUD
WALL WINDOW OPENING W/ AC OPENING

SCALE: 3/4" = 1'-0"



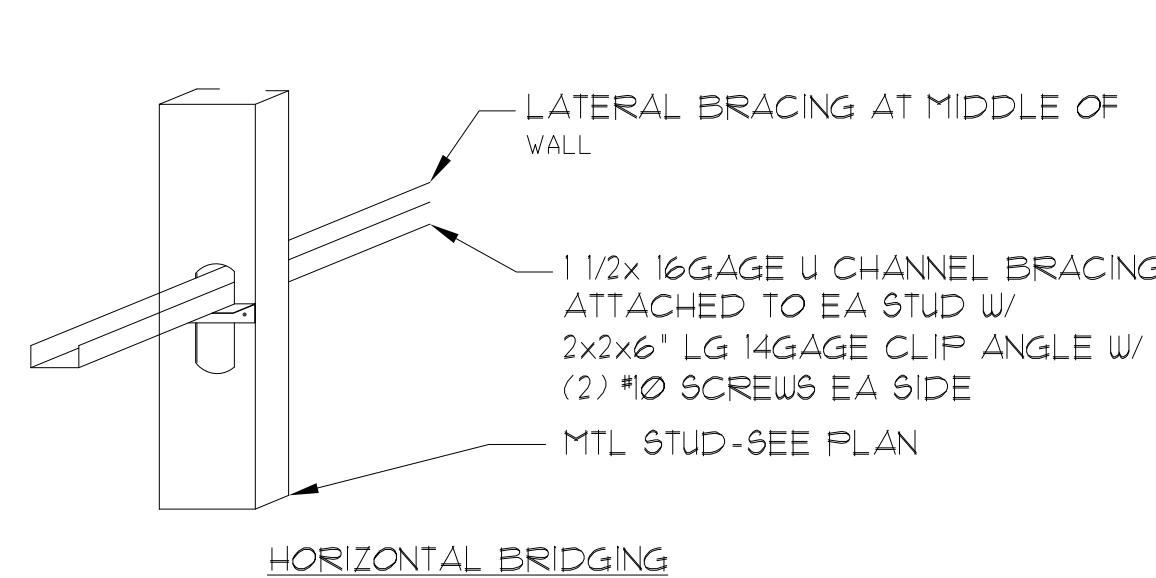
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TYPICAL EXTER. METAL STUD
WALL DOOR OPENING < 6'-0"

SCALE: 3/4" = 1'-0"



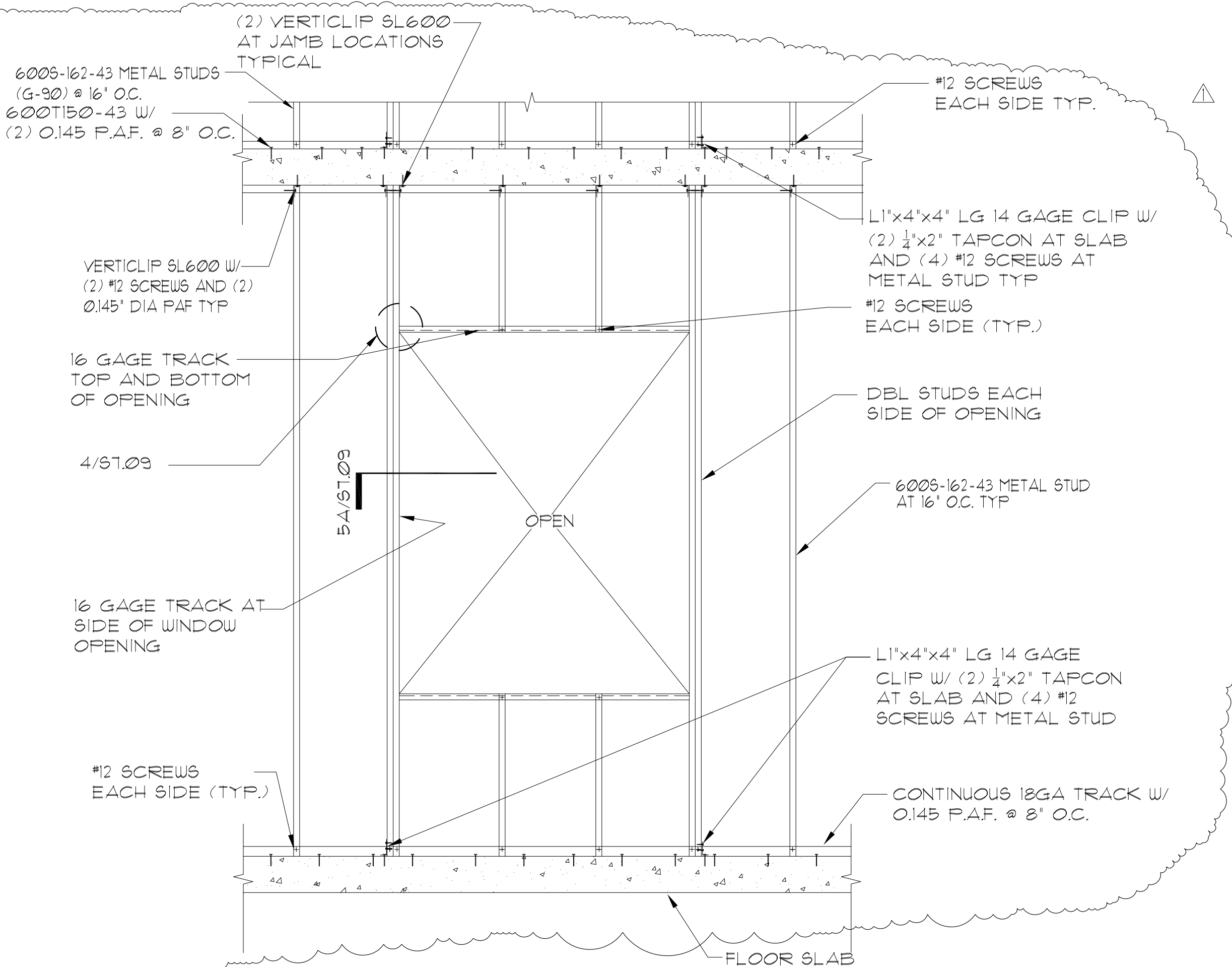
4
OPENING < 5'-0"

SCALE: 3/4" = 1'-0"



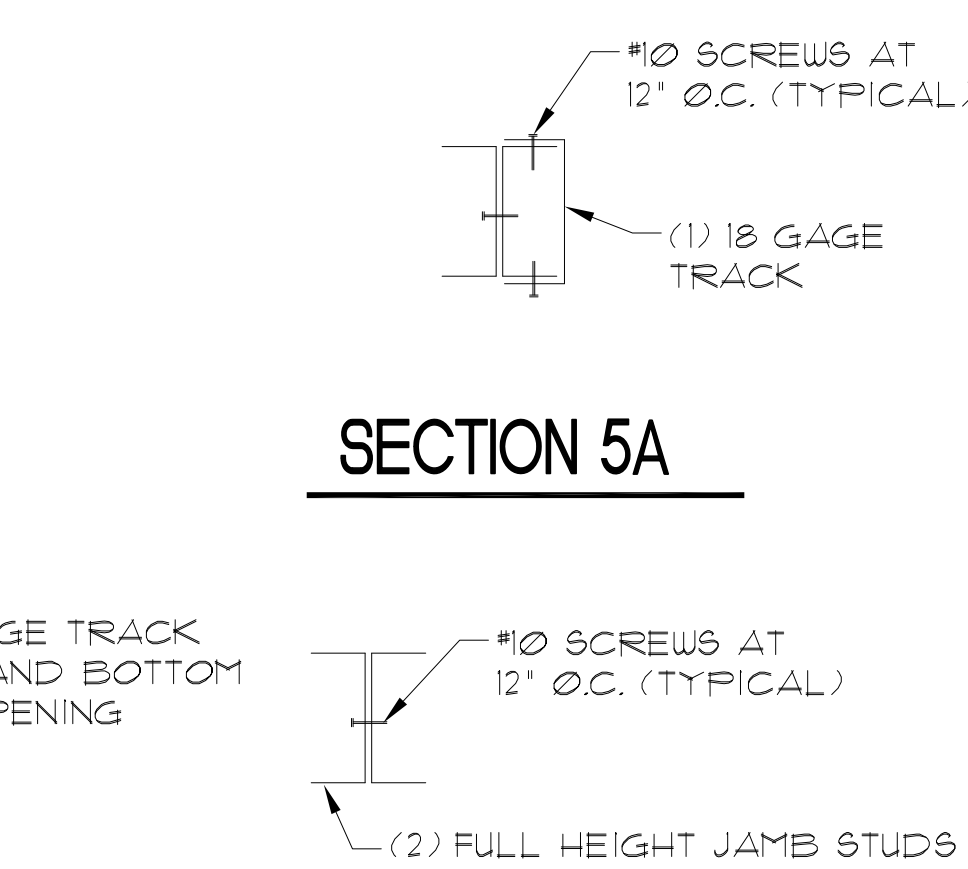
6
TYP. WALL LATERAL BRACING DETAIL

SCALE: 3/4" = 1'-0"



2
TYPICAL EXTER. METAL STUD
WALL WINDOW OPENING < 6'-0"

SCALE: 3/4" = 1'-0"

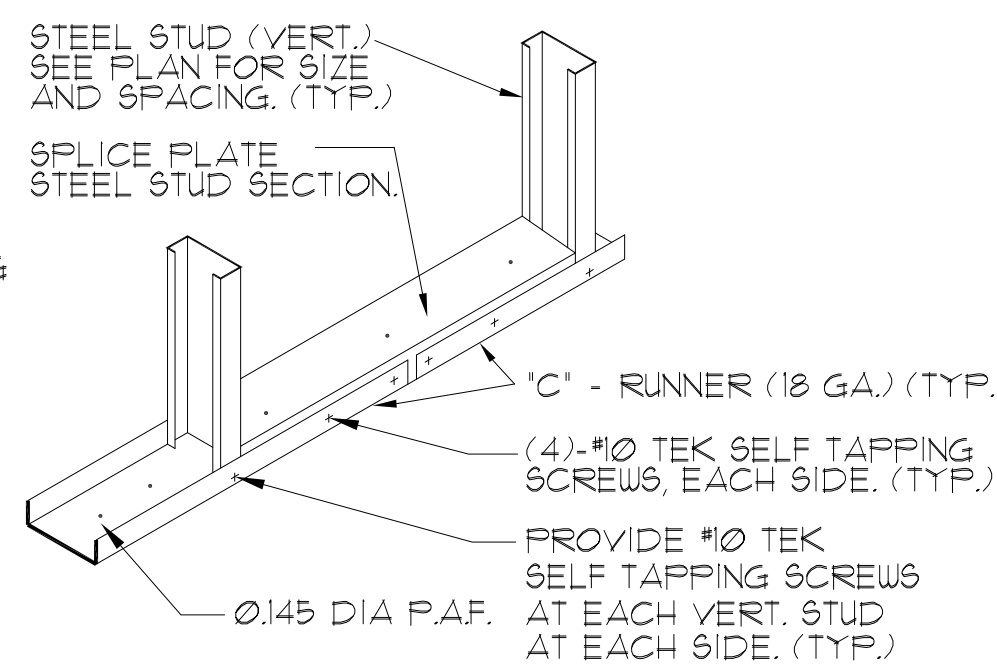


SECTION 5A

SECTION 5B

5
TYPICAL SECTION

SCALE: NTS.



7
TYP. 'C'-RUNNER SPLICE

SCALE: 3/4" = 1'-0"

EXTERIOR METAL STUD FRAMING
SPECIFICATIONS

FRAMING CONTRACTOR:

1. ALL LIGHT STEEL FRAMING SHALL BE COLD-FORMED GALVANIZED FORMED STEEL, WITH G-50 MINIMUM COATING CONFORMING WITH ASTM A446. ALL STEEL THAT IS 18 GAGE OR LIGHTER SHALL BE GRADE A, WITH MINIMUM YIELD STRENGTH OF 33 KSI. ALL STEEL THAT IS 16 GAGE OR HEAVIER SHALL BE GRADE D, WITH A MINIMUM YIELD STRENGTH OF 50 KSI.

2. SCREW FASTENERS SHALL BE OF THE NUMBER AND SPACING AS SHOWN IN THE DETAILS, USING #12-16 SELF-DRILLING SCREWS AT EXTERIOR FRAMING CONNECTIONS. FRAMING SCREWS SHALL BE ZINC OR CAD-PLATED, OR OTHERWISE TREATED WITH A CORROSION INHIBITOR.

3. ALL POWDER-ACTUATED FASTENERS (DENOTED AS P.A.F.'S IN THE DETAIL DRAWINGS) TO STRUCTURAL STEEL SHALL BE 0.145' DIAMETER STEEL PINS. ALL P.A.F.'S TO CONCRETE SHALL BE 0.145' DIA. X 1-1/4' LONG PINS.

4. ALL STUDS SHALL BE FULL LENGTH. NO SPLICING PERMITTED UNLESS SPECIFICALLY DETAILED.

5. MATERIALS: STUDS, RUNNERS AND ANGLES SHALL MEET THE REQUIREMENTS OF ASTM 1446 WITH MINIMUM YIELD STRENGTH AS FOLLOWS:

16, 14, AND 12 GAGE STUDS	50 KSI
22, 20, AND 18 GAGE STUDS	33 KSI
RUNNERS	33 KSI

6. CONTRACTOR SHALL VERIFY THE LENGTH OF METAL STUDS IN FIELD PRIOR TO FABRICATE THEM.

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