DATE 03/24/2010

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000028448

APPLICANT GARY JOHNSON PHONE 752-3444

ADDRESS P.O. BOX 1016 LAKE CITY FL 32056

OWNER CHRISTOPHER DICKS PHONE 752-2057

ADDRESS 4037 SE CR 252 LAKE CITY FL 32025

CONTRACTOR GARY JOHNSON PHONE 752-3444

LOCATION OF PROPERTY 441S, TL CR 252, PAST OLD COUNTRY CLUB ROAD, 1.4 MILES FROM OLD COUNTRY CLUB RD, ON LEFT

TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 276750.00

HEATED FLOOR AREA 3835.00 TOTAL AREA 5535.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB

LAND USE & ZONING A-3 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 23-4S-17-08714-003 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 20.00

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
		RG0024685	Gary Johnson

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	10-107	BK	HD	Y

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE

Check # or Cash 2910

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Insulation _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by Pool _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

Pump pole _____ date/app. by Utility Pole _____ date/app. by M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Reconnection _____ date/app. by RV _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 1385.00 CERTIFICATION FEE $ 27.68 SURCHARGE FEE $ 27.68

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ **TOTAL FEE** 1515.36

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

DATE 03/24/2010

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000028448

APPLICANT GARY JOHNSON PHONE 752-3444

ADDRESS P.O. BOX 1016 LAKE CITY FL 32056

OWNER CHRISTOPHER DICKS PHONE 752-2057

ADDRESS 4037 SE CR 252 LAKE CITY FL 32025

CONTRACTOR GARY JOHNSON PHONE 752-3444

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TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 276750.00

HEATED FLOOR AREA 3835.00 TOTAL AREA 5535.00 HEIGHT STORIES 1

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LAND USE & ZONING A-3 MAX. HEIGHT

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 23-4S-17-08714-003 SUBDIVISION

LOT BLOCK PHASE UNIT TOTAL ACRES 20.00

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
18"X32'MITERED	10-107	BK	HD
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance

RG0024685

Y — New Resident

COMMENTS: ONE FOOT ABOVE THE ROAD, NOC ON FILE. HOMEOWNER CHANGED MIND TO PURCHASE CULVERT PERMIT. 03.30.2010(JLW)

Check # or Cash 2910

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power date/app. by — Foundation date/app. by — Monolithic date/app. by

Under slab rough-in plumbing date/app. by — Slab date/app. by — Sheathing/Nailing date/app. by

Framing date/app. by — Insulation date/app. by

Rough-in plumbing above slab and below wood floor date/app. by — Electrical rough-in date/app. by

Heat & Air Duct date/app. by — Peri. beam (Lintel) date/app. by — Pool date/app. by

Permanent power date/app. by — C.O. Final date/app. by — Culvert date/app. by

Pump pole date/app. by — Utility Pole date/app. by — M/H tie downs, blocking, electricity and plumbing date/app. by

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MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

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INSPECTORS OFFICE CLERKS OFFICE

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"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED NOT SUSPENDED, ABANDONED OR INVALID WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS OT THE PREVIOUS INSPECTION.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Prepared by:
Elaine R. Davis / Megan M. Harrell
American Title Services of Lake City, Inc.
321 SW Main Blvd, Suite 105
Lake City, Florida 32025

File Number: 08-271

Inst:200812015362 Date:8/19/2008 Time:10:26 AM
Doc Stamp-Deed:770.00
DC,P.DeWitt Cason,Columbia County Page 1 of 2 B:1156 P:2067

Warranty Deed

Made this August 15, 2008 A.D. By

CAMILLE PEACOCK RUSSELL PEARCE, f/k/a Camille Peacock Russell, An Unmarried Woman 271 SE Peacock Terrace, Lake City, Florida 32025, hereinafter called the grantor, to

CHRISTOPHER QUILLIAN DICKS and AMANDA DEE DICKS, husband and wife, whose post office address is: 4035 SE CR 252, Lake City, Florida 32025, hereinafter called the grantee:

(Whenever used herein the term "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations)

Witnesseth, that the grantor, for and in consideration of the sum of Ten Dollars, ($10.00) and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz:

See Attached Schedule "A"

Parcel ID Number: **08714-000 Parent**

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances except taxes accruing subsequent to December 31, 2007.

In Witness Whereof, the said grantor has signed and sealed these presents the day and year first above written.

Signed, sealed and delivered in our presence:

Megan M. Harrell
Witness Printed Name Megan M. Harrell

Elaine R. Davis
Witness Printed Name Elaine R. Davis

(Seal)
CAMILLE PEACOCK RUSSELL PEARCE, f/k/a Camille Peacock Russell
Address: 271 SE Peacock Terrace, Lake City, Florida 32025

_____(Seal)
Address:

State of Florida
County of Columbia

The foregoing instrument was acknowledged before me this 15th day of August, 2008, by CAMILLE PEACOCK RUSSELL PEARCE, f/k/a Camille Peacock Russell, who is/are personally known to me or who has produced Drivers License as identification.

Megan M. Harrell
Notary Public
Print Name:_____

My Commission Expires:_____

Megan M Harrell
MY COMMISSION # DD780566
EXPIRES March 30, 2009
(407) 398-0153 FloridaNotaryService.com

Prepared by:
Elaine R. Davis / Megan M. Harrell
American Title Services of Lake City, Inc.
321 SW Main Blvd, Suite 105
Lake City, Florida 32025

File Number: 08-271

"Schedule A"

TOWNSHIP 4 SOUTH, RANGE 17 EAST

SECTION 23: Commence at the SE corner of the NE 1/4 of Section 23, Township 4 South, Range 17 East, Columbia County, Florida, Run Thence South 87° 28' 49" West along the South line of said NE 1/4, a distance of 1326.73 feet to the SE corner of the SW 1/4 of the NE 1/4, said Point also being the Point of Beginning, Thence South 87° 20' 35" West along the South line of the said SW 1/4 of the NE 1/4 a distance of 524.34 feet to the centerline of a branch, Thence along the centerline of said branch the following three courses: (1.) South 31° 10' 52" East, 125.34 feet, (2) South 07° 02' 50" East, 56.82 feet, (3) South 64° 04'18" West, 291.88 feet to the Northerly right of way line of county road No. 252 (A 80 foot right of way), Thence North 71° 07' 37" West along said Northerly right of way line a distance of 258.73 feet, Thence North 01° 52' 25" West a distance of 187.17 feet to the South Line of the aforesaid SW 1/4 of the NE 1/4, Thence continue North 01° 52' 25" West along a line 360.00 feet East of and parallel with the West line of the NE 1/4 a distance of 455.00 feet, Thence North 46° 49' 37" East a distance of 692.73 feet, Thence North 87° 20' 35" East a distance of 448.50 feet to the East line of said SW 1/4 of the NE 1/4, Thence South 01° 42' 36" East along said East line of SW 1/4 of the NE 1/4 a distance of 905.12 feet to the Point of Beginning. **IN COLUMBIA COUNTY, FLORIDA.**

(2)

Permit Number:_____

Tax Folio Number: **08714-003**

State of: **Florida**
County of: **Columbia**

File Number: **10-064**

Ins: 201012003022 Date 3/1/2010 Time 9:50 AM
DC,P DeWitt Cason,Columbia County Page 1 of 1 B 1189 P 2236

NOTICE OF COMMENCEMENT

The undersigned hereby gives notice that improvement will be made to certain real property, and, in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of Property:

TOWNSHIP 4 SOUTH, RANGE 17 EAST

SECTION 23: Commence at the SE corner of the NE 1/4 of Section 23, Township 4 South, Range 17 East, Columbia County, Florida, Run Thence South 87° 28' 49" West along the South line of the SE 1/4 of said NE 1/4, of said Section 23 a distance of 1326.73 feet to the SE corner of the SW 1/4 of the NE 1/4, said Point also being the Point of Beginning, Thence South 87° 20' 35" West along the South line of the said SW 1/4 of the NE 1/4 a distance of 524.34 feet to the centerline of a branch, Thence along the centerline of said branch the following three courses: (1.) South 31° 10' 52" East, 125.34 feet, (2) South 07° 02' 50" East, 56.82 feet, (3) South 64° 04'18" West, 291.88 feet to the Northerly right of way line of county road No. 252 (A 80 foot right of way), Thence North 71° 07' 37" West along said Northerly right of way line a distance of 258.73 feet, Thence North 01° 52' 25" West a distance of 187.17 feet to the South Line of the aforesaid SW 1/4 of the NE 1/4, Thence continue North 01° 52' 25" West along a line 360.00 feet East of and parallel with the West line of the NE 1/4 a distance of 455.00 feet, Thence North 46° 49' 37" East a distance of 692.73 feet, Thence North 87° 20' 35" East a distance of 448.50 feet to the East line of said SW 1/4 of the NE 1/4, Thence South 01° 42' 36" East along said East line of SW 1/4 of the NE 1/4 a distance of 905.12 feet to the Point of Beginning. **IN COLUMBIA COUNTY, FLORIDA.**

2. General Description of Improvements: RESIDENTIAL

3. Owner Information:
   a. Name and Address: Christopher Quillian Dicks and Amanda Dee Dicks
   4032 SE County Road 252, Lake City, Florida 32025

   b. Interest in property: Fee Simple

   c. Names and address of fee simple title holder (if other than owner):

4. Contractor: Gary Johnson of GARY JOHNSON CONSTRUCTION, INC.
   Post Office Box 1016, Lake City, Florida 32056

5. Surety: N/A

6. Lender: Columbia Bank, 173 NW Hillsboro Street, Lake City, Florida 32055

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1) (a)7., Florida Statutes.

8. In addition to himself, Owner designates the following persons to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of Notice of Commencement (the expiration date is 1 year from date of recording unless a different date is specified): FEBRUARY 25, 2011.

Christopher Q. Dicks
CHRISTOPHER QUILLIAN DICKS

Amanda Dee Dicks
AMANDA DEE DICKS

Sworn to and subscribed before me February 25, 2010 by CHRISTOPHER QUILLIAN DICKS AND AMANDA DEE DICKS who is personally known to me or who did provide Drivers Licenses as identification.

Elaine R. Davis
Notary Public
My Commission Expires:_____

ELAINE R. DAVIS
MY COMMISSION # DD 709588
EXPIRES: October 14, 2011
Bonded Thru Notary Public Underwriters

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001799

DATE 03/30/2010 PARCEL ID # 23-4S-17-08714-003

APPLICANT CHRISTOPHER DICKS PHONE 386.752.3444

ADDRESS 4037 SE CR 252 LAKE CITY FL 32025

OWNER CHRISTOPHER DICKS PHONE 386.752.2057

ADDRESS 4037 SE CR 252 LAKE CITY FL 32025

CONTRACTOR GARY JOHNSON PHONE 386.752.3444

LOCATION OF PROPERTY 441-S TO C-252,TL PAST COUNTRY CLUB ROAD & 1.4 MILES FROM OLD COUNTRY CLUB ROAD ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT

SIGNATURE /Christopher W Dicks

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 23-4S-17-08714-003 **Building permit No.** 000028448

Use Classification SFD,UTILITY **Fire:** 138.31

Permit Holder GARY JOHNSON **Waste:** 112.63

Owner of Building CHRISTOPHER DICKS **Total:** 250.94

Location: 4037 SE COUNTY RD 252, LAKE CITY, FL 32025

Date: 03/04/2014

Troy Craven

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

RC

Columbia County Building Permit Application

For Office Use Only Application # 1003-20 Date Received 3/15/10 By G Permit # 28448

Zoning Official BLK Date 24.03.10 Flood Zone Structure in X Land Use A-3 Zoning A-3

FEMA Map # N/A Elevation N/A MFE ~~Adam Rd~~ River N/A Plans Examiner ND Date 3-23-10

Comments

☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan ☐ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter

IMPACT FEES: EMS Fire Corr Road/Code

School = TOTAL N/A Suspended

Septic Permit No. Fax

Name Authorized Person Signing Permit GARY JOHNSON Phone (386) 752-3444

Address PO BOX 1016 LAKE CITY, FL 32056

Owners Name CHRISTOPHER Q. DICKS Phone (386) 752-2057

911 Address 4037 SE CR 252 LAKE CITY, FL 32025

Contractors Name GARY JOHNSON Phone (386) 752-3444

Address PO BOX 1016 LAKE CITY, FL 32056

Fee Simple Owner Name & Address

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address MARTY J. HUMPHRIES 7932 240th ST. O'BRIEN, FL 32071

Mortgage Lenders Name & Address COLUMBIA BANK P.O. BOX 1609 LAKE CITY, FL 32056

Circle the correct power company – FL Power & Light – (Clay Elec.) – Suwannee Valley Elec. – Progress Energy

Property ID Number 23-4S-17-08714-003 Estimated Cost of Construction $350,000

Subdivision Name N/A Lot Block Unit Phase

Driving Directions FROM US90 AND SR 441, TRAVEL SOUTH ON SR 441. TURN LEFT(MERGE) ONTO SR 41/441. TURN LEFT ONTO CR 252, TRAVEL EAST ON CR 252 UNTIL CR 133 (OLD COUNTRY CLUB ROAD). TRAVEL EAST ON CR 252 1.4 MILES FROM CR 133. PROPERTY IS ON LEFT. Peacock horse Number of Existing Dwellings on Property ZERO

Construction of SINGLE FAMILY DWELLING Total Acreage 20 ac. Lot Size N/A

Do you need a - Culvert Permit or Culvert Waiver or (Have an Existing Drive) Total Building Height 23'-3¾"

Actual Distance of Structure from Property Lines - Front 75' Side 680' Side 180' Rear 770'

Number of Stories 1 Heated Floor Area 3835 SF Total Floor Area 5535 SF Roof Pitch 6:12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Columbia County Building Permit Application

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full.
This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. It may be to your advantage to check and see if your property is encumbered by any restrictions.

(Owners Must Sign All Applications Before Permit Issuance.)

Christopher A Dibbs
Owners Signature ****OWNER BUILDERS MUST PERSONALLY APPEAR AND SIGN THE BUILDING PERMIT.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Tony Johnson
Contractor's Signature (Permitee)

Contractor's License Number RG0024685
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 15 **day of** March **20**10.

Personally known ____ **or Produced Identification** J525-295-49-346-0 FDL

Lawanda Y. Rentz
State of Florida Notary Signature (For the Contractor)

SEAL:

LAWANDA Y. RENTZ
MY COMMISSION # DD 710646
EXPIRES: October 29, 2011
Bonded Thru Notary Public Underwriters

SUBCONTRACTOR VERIFICATION FORM

APPLICATION NUMBER _____ CONTRACTOR _____ PHONE _____

THIS FORM MUST BE SUBMITTED PRIOR TO THE ISSUANCE OF A PERMIT

In Columbia County one permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the permit. Per Florida Statute 440 and Ordinance 89-6, a contractor shall require all subcontractors to provide evidence of workers' compensation or exemption, general liability insurance and a valid Certificate of Competency license in Columbia County.

Any changes, the permitted contractor is responsible for the corrected form being submitted to this office prior to the start of that subcontractor beginning any work. Violations will result in stop work orders and/or fines.

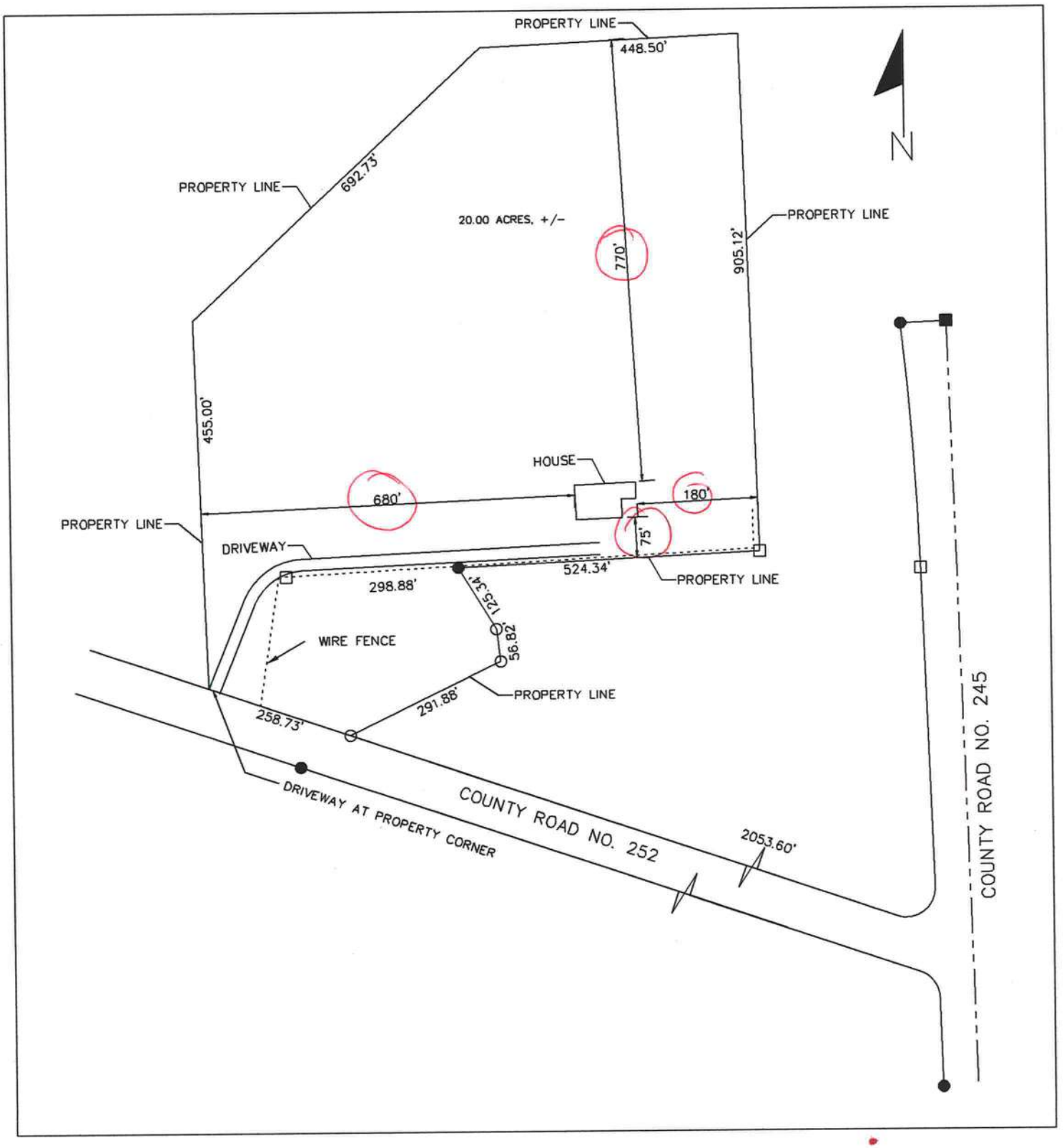
ELECTRICAL Good 37	Print Name Donald R. Hollingsworth License #: 13012377 Holly Electric	Signature Phone #: 386-755-5944
MECHANICAL/ A/C Good 327	Print Name HARRY'S HEATING AND AIR License #: RA0030316	Signature Harry Moseley Phone #: (386) 752-2308
PLUMBING/ GAS Good 563	Print Name CURTIS GRADDY PLUMBING LLC License #: CFC043064	Signature Curtis Graddy Phone #: (386) 755-4456
ROOFING	Print Name GARY JOHNSON License #: RC0026693	Signature Gary Johnson Phone #: 386-752-3444
SHEET METAL	Print Name N/A License #:	Signature Phone #:
FIRE SYSTEM/ SPRINKLER	Print Name N/A License#:	Signature Phone #:
SOLAR	Print Name N/A License #:	Signature Phone #:

Specialty License	License Number	Sub-Contractors Printed Name	Sub-Contractors Signature
MASON Good	000620	Brant Stevens	Brant Stevens
CONCRETE FINISHER Good	000218	TONY JORDAN	Tony E Jordan
FRAMING	RG0024685	GARY JOHNSON	Gary Johnson
INSULATION	RG0024685	GARY JOHNSON	Gary Johnson
STUCCO		N/A	
DRYWALL Good	000627	BOBBY JACKSON	Bobby D Jackson
PLASTER		N/A	
CABINET INSTALLER	RG0024685	GARY JOHNSON	Gary Johnson
PAINTING	RG0024685	GARY JOHNSON	Gary Johnson
ACOUSTICAL CEILING		N/A	
GLASS		N/A	
CERAMIC TILE	RG0024685	GARY JOHNSON	Gary Johnson
FLOOR COVERING Good	710	Marc A Vann	M A V
ALUM/VINYL SIDING	RG0024685	GARY JOHNSON	Gary Johnson
GARAGE DOOR Good	000619 (exp 19)	LAKE CITY GLASS	Carl Ballard Jr
METAL BLDG ERECTOR		N/A	

F. S. 440.103 Building permits; identification of minimum premium policy.--Every employer shall, as a condition to applying for and receiving a building permit, show proof and certify to the permit issuer that it has secured compensation for its employees under this chapter as provided in ss. 440.10 and 440.38, and shall be presented each time the employer applies for a building permit.

```
@ CAM110M01    S  CamaUSA Appraisal System                      Columbia County
 1/26/2010 17:15  Property Maintenance                             Land  000
Year T Property                              Sel                2860  AG    001
2010 R 23-4S-17-08714-003_____________ ... _   *                     Bldg  000
 Owner DICKS CHRISTOPHER QUILLIAN &   + Conf                        Xfea  000
  Addr AMANDA DEE DICKS                                           2860  TOTAL     B*
       4032 SE CR 252                                -Cap?-       20.000 Total Acres
                                                     SOH 10% ApYr ERnwl ARnwl   Notc
City,St LAKE CITY            FL Zip 32025             N   Y
Country                         (PUD1)                (PUD2)          (PUD3) MKTA02
Splt/Co          JVChgCd        pud4                  pud5            pud6
Appr By DF   Date 10/17/2005 AppCode     UseCd 005600 TIMBERLAND 70-79
      TxDist Nbhd        MktA    ExCode  Exemption/%     TxCode   Units    Tp
        003   23417.00   02
             DIST 3
 House#               Street                                MD      Dir    #
       -                City                                 Zip
   Subd       N/A                    Condo      .00 N/A
   Sect     23 Twn      4S Rnge     17 Subd        Blk          Lot
 Legals COMM AT SE COR OF NE1/4 OF SEC   RUN W 1326.73 FT FOR POB, CONT
        W 524.34 FT TO C/L OF BRANCH,    RUN ALONG BRANCH 474.04 FT TO   +
   Map# 153                                        Mnt  1/25/2010 JEFF
 F1=Task  F2=ExTx  F3=Exit  F4=Prompt  F11=Docs  F10=GoTo  PgUp/PgDn  F24=More
```

SITE PLAN:


PROPERTY LINE
448.50'
N
692.73'
PROPERTY LINE
20.00 ACRES, +/-
770'
905.12'
PROPERTY LINE
455.00'
HOUSE
680'
180'
75'
PROPERTY LINE
DRIVEWAY
298.88'
524.34'
PROPERTY LINE
125.34'
56.82'
WIRE FENCE
291.86'
PROPERTY LINE
258.73'
DRIVEWAY AT PROPERTY CORNER
COUNTY ROAD NO. 252
2053.60'
COUNTY ROAD NO. 245

1003-20

10-0107

STATE OF FLORIDA
DEPARTMENT OF HEALTH AND REHABILITATIVE SERVICES
ONSITE SEWAGE DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT
Authority: Chapter 381, FS & Chapter 10D-6, FAC

PERMIT # 954554
DATE PAID 3/1/10
FEE PAID $ 310.00
RECEIPT # 1240242
CR # 09-4822

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Temporary/Experimental System
[ ] Repair [ ] Abandonment [ ] Other(Specify) _____

APPLICANT: CHRISTOPHER & AMANDA DICKS TELEPHONE: 752-2057

AGENT: PAUL LLOYD

MAILING ADDRESS: 4035 SE CR 252 CITY: LAKE CITY STATE: FL ZIP: 32025

==========

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. ATTACH BUILDING PLAN AND TO-SCALE SITE PLAN SHOWING PERTINENT FEATURES REQUIRED BY CHAPTER 10D-6, FLORIDA ADMINISTRATIVE CODE.

==========

PROPERTY INFORMATION [IF LOT IS NOT IN A RECORDED SUBDIVISION, ATTACH LEGAL DESCRIPTION OR DEED]

LOT: _____ BLOCK: _____ SUBDIVISION: MEETS & BOUNDS DATESUBD: _____

PROPERTY ID #: 23-4S-17-08714-003 [Section/Township/Range/Parcel] ZONING: AG

PROPERTY SIZE: 20.0 ACRES [Sqft/43560] PROPERTY WATER SUPPLY: [X] PRIVATE [ ] PUBLIC

PROPERTY STREET ADDRESS: 4035 SE CR 252

DIRECTIONS TO PROPERTY: 441 SOUTH TURN LEFT ON CR 252 , CROSS COUNTRY CLUB, SITGE ON LEFT JUST BEFORE PRICE CREEK RD.

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	# Persons Served	Business Activity For Commercial Only
1	HOUSE	5	3835	6	
2					
3					
4					

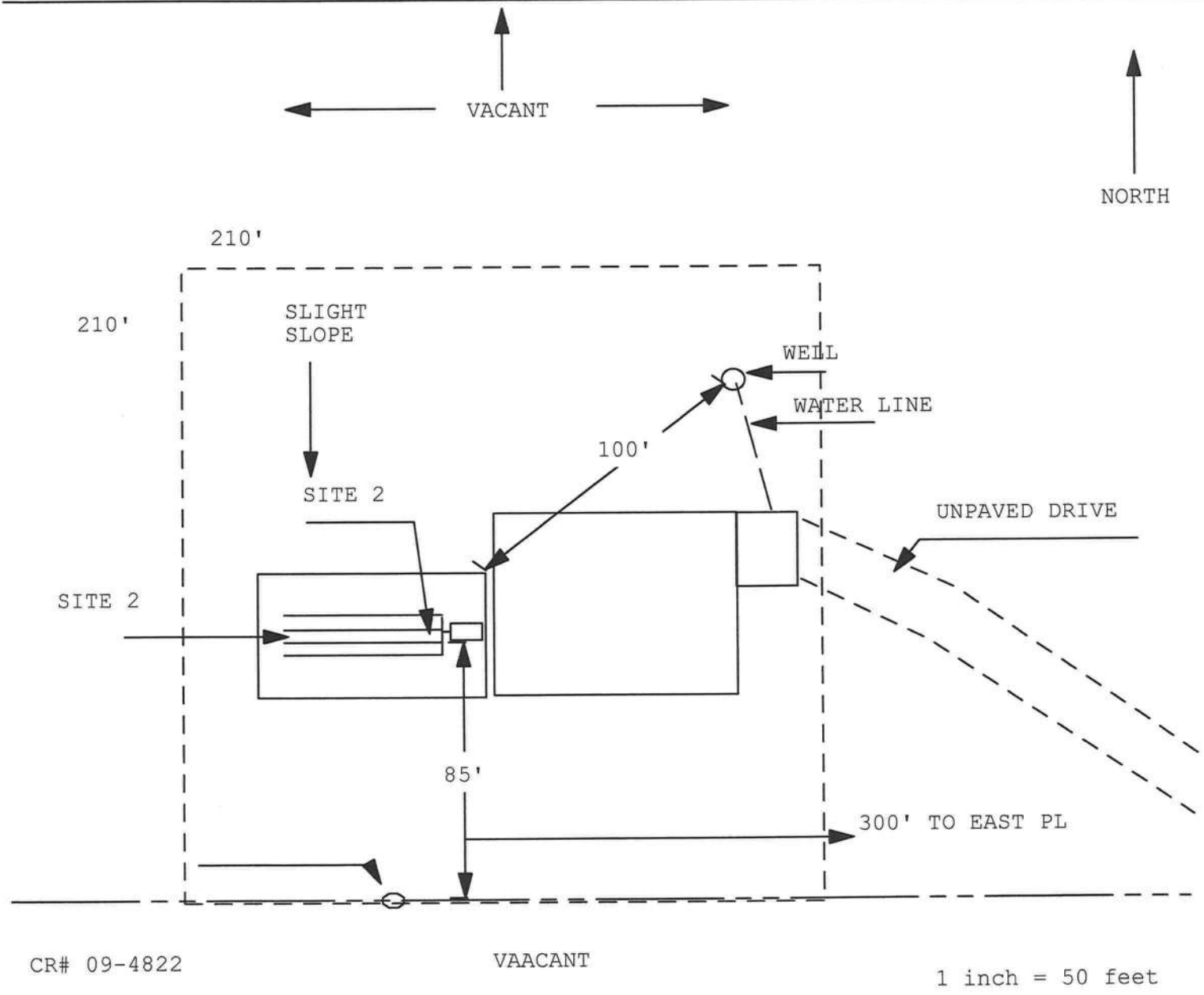
[N] Garbage Grinders/Disposals [N] Spas/Hot Tubs [N] Floor/Equipment Drains
[N] Ultra-low Volume Flush Toilets [N] Other (Specify) _____

APPLICANT'S SIGNATURE: Paul Lloyd DATE: 3/1/10

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 10-107-N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Site Plan Submitted By Paul Lloyd **Date** 2/1/10

Plan Approved ✓ **Not Approved** ____ **Date** 3/8/10

By Sabi Ford. Columbia ~~CHD~~ **CPHU**

Notes: See attached for full property view

COLUMBIA COUNTY 9-1-1 ADDRESSING

P. O. Box 1787, Lake City, FL 32056-1787
PHONE: (386) 758-1125 * FAX: (386) 758-1365 * Email: ron_croft@columbiacountyfla.com

Addressing Maintenance

To maintain the Countywide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for assigning and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Service Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County.

DATE REQUESTED: 2/19/2010 **DATE ISSUED:** 2/24/2010

ENHANCED 9-1-1 ADDRESS:

4037 SE COUNTY ROAD 252

LAKE CITY FL 32025

PROPERTY APPRAISER PARCEL NUMBER:

23-4S-17-08714-003

Remarks:

Address Issued By: _____

Columbia County 9-1-1 Addressing / GIS Department

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION INFORMATION BE FOUND TO BE IN ERROR, THIS ADDRESS IS SUBJECT TO CHANGE.

1647

Effective March 1, 2009

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

FORM 1100B-08 — **Residential Component Prescriptive Method B** — **ALL CLIMATE ZONES**

Compliance with Method B of Chapter 11 of the *Florida Building Code, Residential*, or Subchapter 13-6 of the *Florida Building Code, Building*, may be demonstrated by the use of Form 1100B for single-and multiple-family residences of three stories or less in height, additions to existing residential buildings, renovations to existing residential buildings, new heating, cooling, and water heating systems in existing buildings, and site-added components of manufactured homes and manufactured buildings. To comply, a building must meet or exceed all of the energy efficiency requirements on Table 11B-1 and all applicable mandatory requirements summarized in Table 11B-2 of this form. If a building does not comply with this method, it may still comply under Method A of Chapter 11 or Subchapter 13-6 of the applicable code.

PROJECT NAME AND ADDRESS:	DICKS 4037 SE CO RD 252 32025	**BUILDER:**	GARY JOHNSON CONST INC
		PERMITTING OFFICE:	COLUMBIA
OWNER:	CHRISTOPHER & AMANDA DICKS	**PERMIT NO.:**	28448 — **JURISDICTION NO.:** 221000

1. New construction including additions which incorporate any of the following features cannot comply using this method: skylights or other nonvertical roof glass, glass areas in excess of 16 percent of conditioned floor area, and electric resistance heat (See Notes to Table 11B-1 on page 2).
2. Fill in all the applicable spaces of the "To Be Installed" column on "Table 11B-1 with the information requested. All "To Be Installed" values must be equal to or more efficient than the required levels.
3. Complete page 1 based on the "To Be Installed" column information.
4. Read "Minimum Requirements for All Packages", Table 11B-2 and check each box to indicate your intent to comply with all applicable items.
5. Read, sign and date the "Prepared By" certification statement at the bottom of page 1. The owner or owner's agent must also sign and date the form.

		Please Print	CK
1.	**New construction, addition, or existing building**	1. New	
2.	**Single-family detached or multiple-family attached**	2. S/F	
3.	**If multiple-family–No. of units covered by this submission**	3.	
4.	**Is this a worst case? (yes/no)**	4.	
5.	**Conditioned floor area (sq. ft.)**	5. 3835	
6.	**Glass type and area:**		
	a. U-factor	6a.	
	b. SHGC	6b.	
	c. Glass area	6c. 287 sq. ft.	
7.	**Percentage of glass to floor area**	7. .08 %	
8.	**Floor type, area or perimeter, and insulation:**		
	a. Slab-on-grade (*R*-value)	8a. R= ____ ____ lin. ft.	
	b. Wood, raised (*R*-value)	8b. R= ____ ____ sq. ft.	
	c. Wood, common (*R*-value)	8c. R= ____ ____ sq. ft.	
	d. Concrete, raised (*R*-value)	8d. R= ____ ____ sq. ft.	
	e. Concrete, common (*R*-value)	8e. R= ____ ____ sq. ft.	
9.	**Wall type, area and insulation:**		
	a. **Exterior:** 1. Masonry (Insulation R-value)	9a-1. R= ____ ____ sq. ft.	
	2. Wood frame (Insulation *R*-value)	9a-2. R= ____ ____ sq. ft.	
	b. **Adjacent:** 1. Masonry (Insulation *R*-value)	9b-1. R= ____ ____ sq. ft.	
	2. Wood frame (Insulation *R*-value)	9b-2. R= ____ ____ sq. ft.	
10.	**Ceiling type, area and insulation:**		
	a. Under attic (Insulation *R*-value)	10a. R= 30 sq. ft. ____	
	b. Single assembly (Insulation *R*-value)	10b. R= ____ ____ sq. ft.	
11.	**Air distribution system: Duct insulation, location**	11a. R= ____ ____	
	Test report required if duct in unconditioned space	11b. **Test report attached?** Yes No	
12.	**Cooling system:**	12a. Type: Central	
	(Types: central, room unit, package terminal A.C., gas, none)	12b. SEER/EER: 14	
		12c. Capacity: ____	
13.	**Heating system:**	13a. Type: Heat Pump	
	(Types: heat pump, elec. strip, nat. gas, LP-Gas, gas h.p., room or PTAC, none)	13b. HSPF/COP/AFUE: ____	
		13c. Capacity: ____	
14.	**Programmable thermostat installed on HVAC systems:**	14. Yes No	
15.	**Hot water system:**	15a. Type: Electric	
	(Types: elec., nat. gas, LP-gas, solar, heat rec., ded. heat pump, other, none)	15b. EF: .90	

I hereby certify that the plans and specifications covered by the calculation are in compliance with the Florida Energy Code.

PREPARED BY: _____ DATE: _____

I hereby certify that this building is in compliance with the Florida Energy Code:
OWNER AGENT: _____ DATE: _____

Review of plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed, this building will be inspected for compliance in accordance with Section 553.908, F.S.

BUILDING OFFICIAL: _____

DATE: _____

* **TABLE 11B-1** **MINIMUM REQUIREMENTS (See Note 1)** **All Climate Zones**

BUILDING COMPONENT	PERFORMANCE CRITERIA	INSTALLED VALUES:
Windows (see Note 2):	U-Factor = 0.65 SHGC = 0.35 % of CFA < = 16%	U-Factor = .30 SHGC = .23 % of CFA = .53
Exterior door type	Wood or insulated	Type: INSULATED
Walls – Ext. and Adj. (see Note 3): Frame Mass (see Note 3) Interior of wall: Exterior of wall:	R-13 R-6 R-4	R-Value = 13 R-Value = R-Value =
Electric resistance heat (See Note 10)	Not allowed	
Ceilings (see Notes 3 & 4)	R=30	R-Value = 30
Floors: Slab-on-grade Over unconditioned spaces (see Note 3)	No requirement R-13	R-Value =
Hot water systems (storage type) Electric (see Note 5): Gas fired (see Note 6):	40 gal: EF = 0.92 50 gal: EF = 0.90 40 gal: EF = 0.59 50 gal: EF = 0.58	Gallons = 80 EF = .90 Gallons = EF = N/A
Air conditioning systems (see Note 7)	SEER = 13.0	SEER = 14
Heat pump systems (see Note 8)	SEER = 13.0 HSPF = 7.7	SEER = HSPF =
Gas furnaces	AFUE = 78%	AFUE = N/A
Oil furnaces	AFUE = 78%	AFUE = N/A
Programmable thermostat (see Note 10)	Must be installed on all HVAC systems.	Installed? Yes No
Ductwork: (see Note 9) Unconditioned space[a] Conditioned space Unvented attic assembly per R806.4 with insulation at the roof plane	R-6, TESTED NA R-4.2	Location: Unconditioned space R-Value = 6 Test report: Conditioned space R-Value = (No test report required)
Air Handler location: Unconditioned attic[a] or garage Conditioned space or Unvented attic assembly per R806.4 with insulation at the roof plane	Requires test report No duct test required	Location: Test report:

(1) Each component present in the As-Built home must meet or exceed each of the applicable performance criteria in order to comply with this code using this method; otherwise Method A compliance must be used.

** (2) Windows and doors qualifying as glazed fenestration areas must comply with both the maximum *U*-Factor and the maximum SHGC (Solar Heat Gain Coefficient) criteria and have a maximum total window area equal to or less than 16% of the conditioned floor area (CFA), otherwise Method A must be used for compliance. **Exceptions:** 1. Additions of 600 square feet (56 m^2) or less may have maximum glass to CFA of 50 percent. 2. Renovations with new windows under ≥ 2 foot overhang whose lower edge does not extend further than 8 feet from the overhang may have tinted glazing or double-pane clear glazing. Replacement skylights installed in renovations shall be doublepaned or single paned with a diffuser.

(3) *R*-Values are for insulation material only as applied in accordance with manufacturers' installation instructions. For mass walls, the "interior of wall" requirement (R-6) must be met except if at least 50% of the R-4 insulation value required for the "exterior of wall" is installed exterior of, or integral to, the wall.

(4) Attic knee walls shall be insulated to same level as ceilings and shall have a positive means of maintaining insulation in place. Such means may include rigid insulation board or air barrier sheet materials adequately fastened to the attic sides of knee wall framing materials.

(5) For other electric storage volumes, minimum EF = 0.97 - (0.00132 * volume).

(6) For other natural gas storage volumes, minimum EF = 0.67 - (0.0019 * volume).

(7) For all conventional units with capacities greater than 30,000 Btu/hr. For Small-Duct, High-Velocity units, Space Constrained units, and units with capacities less than 30,000 Btu/hr see Table 13-607.AB.3.2A of the *Florida Building Code, Building*, or Table N1107.AB.3.2A of the *Florida Building Code, Residential*.

(8) For all conventional units with capacities greater than 30,000 Btu/hr. For Small-Duct, High-Velocity units, Space Constrained units, and units with capacities less than 30,000 Btu/hr see Table 13-607.AB.3.2B of the *Florida Building Code, Building*, or Table N1107.AB.3.2B of the *Florida Building Code, Residential*.

(9) All ducts and air handlers shall be either located in conditioned space or tested by a Class 1 BERS rater to be "substantially" leak free. "Substantially leak free" shall mean distribution system air leakage to outdoors no greater than 3 cfm per 100 square feet of conditioned floor area at a pressure differential of 25 Pascal (0.10 in. w.c.) across the

** entire air distribution system, including the manufacturer's air handler enclosure. **Exception:** New or replacement ducts installed onto an existing air distribution system as part of an addition or renovation. Such ducts shall either be insulated to R-6 or be installed in conditioned space.

** 10) The prohibition on electric resistance heat and the requirement for programmable thermostats do not apply to additions, renovations, and new heating systems installed in existing buildings.

TABLE 11B-2 MINIMUM REQUIREMENTS FOR ALL PACKAGES

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Exterior Joints & Cracks	N1106.AB.1.2	To be caulked, gasketed, weather-stripped or otherwise sealed.	✓
Exterior Windows & Doors	N1106.AB.1.1	Max .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Sole & Top Plates	N1106.AB.1.2.1	Sole plates and penetrations through top plates of exterior walls must be sealed.	✓
Recessed Lighting	N1106.AB.1.2.4	Type IC rated with no penetrations (two alternatives allowed).	✓
Multistory Houses	N1106.AB.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Exhaust Fans	N1106.AB.1.3	Exhaust fans vented to unconditioned space shall have dampers, except for combustion devices with integral exhaust ductwork.	✓
Water Heaters	N1112.AB.3	Comply with efficiency requirements in Table N1112.AB.3. Switch or clearly marked circuit breaker electric or cutoff (gas) must be provided. External or built-in heat trap required for vertical pipe risers.	✓
Swimming Pools & Spas	N1112.AB.2.3.4	Spas & heated pools must have covers (except solar heated). Noncommercial pools must have a pump timer. Gas spa & pool heaters must have minimum thermal efficiency of 78%. Heat pump pool heaters shall have a minimum COP of 4.0.	N/A
Hot Water Pipes	N1112.AB.5	Insulation is required for hot water circulating systems (including heat recovery units).	✓
Shower Heads	N1112.AB.2.4	Water flow must be restricted to no more than 2.5 gallons per minute at 80 psig.	✓
HVAC Duct Construction, Insulation & Installation	N1110.AB	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section N1110.AB. Ducts in attics must be insulated to a minimum of R-6.	✓
HVAC Controls	N1107.AB.2	Separate readily accessible manual or automatic thermostat for each system.	✓

Prepared By: _____

TOTAL HEATING AND COOLING REQUIREMENTS

For:

Name: Christopher Dicks

Address: 4035 SE CR 252

City: Lake City FL 32025

(✓) Check Constr. Type	ITEM	AREA SQUARE FEET	DESIGN TEMPERATURE DIFFERENCE 30°	35°	40°	45°	50°	HEATING (BTUH LOSS)	DESIGN TEMP 90°	95°	COOLING (BTUH GAIN)
	Gross Wall Area	2598									
	Glass Area (From page 1)	344						12925			12279
	Partitions, Frame										
	Finished 1 side, No Insulation		17	19	22	25	28		6.5	10.0	
	Finished 2 sides, No Insulation		9	11	12	14	16		4.5	6.0	
	Finished 2 sides, R-5		4	5	5.5	6	7		2.5	3.5	
	Finished 2 sides, R-11	1972	2	3	(3)	4	4	5916	2.0	(2.5)	4930
	Other										
	Doors (Excluding glass)										
	No weatherstripping		135	160	180	200	225		10.0	13.0	
	Weatherstripped		70	85	95	110	120		10.0	13.0	
	R-5 Insulation, No weatherstripping		123	144	164	185	205		4.3	5.5	
	R-5 Insulation, weatherstripping	20	68	79	(90)	101	113	1800	4.0	5.0	100
	Other										
	Net Exterior Walls										
	CBS Furred, No Insulation		9	10	12	13	14		4.5	6.0	
	CBS Furred, R-3 Insulation		5	6	7	8	8		3.0	4.2	
	CBS Furred, R-4 Insulation		4	5	6	6	7		2.7	3.8	
	CBS Furred, R-5 Insulation		4	5	5	6	6		2.5	3.5	
	Frame, No Insulation		8	9	10	11	13		5.5	7.0	
	Frame, R-11 Insulation	2254	2	2	(3)	3	4	6762	2.5	3.0	6762
	Frame, R-14 Insulation		1.5	1.7	2	2.5	3		2	2.8	
	Other										
	Ceiling under attic — Roof										
	No Insulation DK / LT		18	21	24	27	30		9 / 7	10 / 8.5	
	R-11 Insulation DK / LT		2.4	2.8	3.2	3.5	3.9		2.5 / 2	3 / 2.5	
	R-19 Insulation DK / LT		1.5	1.7	1.9	2.2	2.4		1.5 / 1.5	2 / 1.5	
	R-22 Insulation DK / LT		1.2	1.5	1.7	1.9	2.1		1.5 / 1.0	1.5 / 1.5	
	R-26 Insulation DK / LT		1.1	1.3	1.4	1.6	1.8		1.3 / 1	1.5 / 1.2	
	R-30 Insulation DK / LT	5765	1	1.1	(1.3)	1.4	1.6	7495	1.1 / .9	(1.3) / 1.0	7495
	Other										
	Floor, Concrete Slab	Perimeter Ft.									
	No Edge Insulation	288	35	40	(40)	45	45	11520	0	0	
	Other										
	Subtotal							46418			31586
	People @ 300 & Appl. @ 1200										7600
	Sensible BTUH Gain										
	Duct BTUH Loss & Gain							46418			39386
	2 In. Flex. or 1 In. Rigid				.10			4642	.10		3939
	1½ In. Rigid				.075				.075		
	Total BTUH Loss							51050			
	Subtotal BTUH Gain										43325
	× 1.3 = Total BTUH Gain										

Calculated Heating Requirements 51050 BTUH

Size of Unit Chosen ..... 60,000 BTUH

% Oversized .....

% Undersized .....

Calculated Cooling Requirements 56323 BTUH

Size of Unit Chosen ..... 60,000 BTUH

% Oversized .....

% Undersized .....

RESIDENTIAL HEATING AND COOLING REQUIREMENTS*

HVAC WORKSHEET FOR WATT-WISE LIVING

HEATING AND COOLING REQUIREMENTS DUE TO GLASS AREA

DESIGN TEMPERATURE DIFFERENCE

WINDOWS & GLASS DOORS	AREA SQUARE FEET	HEATING MULTIPLIER (CIRCLE ONE) 30°	35°	40°	45°	50°	HEATING (BTUH LOSS)
Glass Doors, Infiltration less than 1.0 CFM/FT							
Single Glass		50	60	70	75	85	
Double Glass	60	40	45	(50)	55	60	3000
Other Sliding Glass Doors							
Single Glass		75	85	100	115	125	
Double Glass		60	70	80	90	100	
Windows, Infiltration less than 0.50 CFM/FT							
Single Glass		40	50	55	60	70	
Double Glass	251	25	30	(35)	40	45	8765
Windows, Infiltration less than 0.75 CFM/FT							
Single Glass		45	50	60	65	75	
Double Glass		30	35	40	45	50	
Other Windows							
Single Glass		75	90	105	115	130	
Double Glass		60	70	80	90	105	
Fixed or Picture Windows							
Single Glass		40	50	55	60	70	
Double Glass	32.58	25	30	(35)	40	45	1140
Other							
Total BTUH Loss (Enter on Line 2, Page 2)							12925

WINDOWS & GLASS DOORS	AREA SQUARE FEET	COOLING MULTIPLIER (CIRCLE) SINGLE GLASS 90° C	T	R	95° C	T	R	DOUBLE GLASS 90° C	T	R	95° C	T	R	COOLING (BTUH GAIN)
No Shading														
N	201.29	30	22	20	30	26	25	20	14	13	(25)	17	16	5032
NE & NW		60	41	36	65	45	41	50	29	24	50	32	27	
E & W	45	85	60	53	90	64	57	70	44	36	(75)	47	39	3375
SE & SW		75	51	45	80	55	50	60	37	30	65	40	33	
S	97.29	45	31	28	50	35	33	35	21	18	(40)	24	21	3892
Draperies or Blinds														
N		20	17	16	25	21	20	15	11	11	20	14	14	
NE & NW		35	33	30	40	37	34	30	22	21	35	25	24	
E & W		55	48	43	55	52	47	45	32	30	50	35	33	
SE & SW		45	39	35	50	43	39	40	26	25	40	29	28	
S		30	26	24	30	30	28	25	17	16	25	20	19	
Roller Shades														
N		25	19	17	25	23	22	20	12	11	20	15	14	
NE & NW		45	36	32	50	40	37	40	26	22	45	29	25	
E & W		65	53	47	70	57	51	55	37	32	60	40	35	
SE & SW		55	44	39	60	48	44	50	32	27	50	35	30	
S		35	28	25	40	32	30	30	20	16	35	23	19	
Awnings, Porches, Etc.														
All Directions		25	22	20	30	26	25	15	14	13	20	17	16	
Other														
Total BTUH Gain (Line 2, Page 2)														12299

*REFERENCE A.C.C.A. MANUAL "J"

(C - Clear T - Tinted R - Reflective)

Hall's Pump & Well Service, Inc.
904 NW Main Blvd
Lake City, FL. 32055

March 15, 2010

Notice to All Contractors:
Attn: Gary Johnson

Please be advised that due to the new building codes we will use a large capacity diaphragm tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphragm tank is used then we will install a cycle stop valve which will produce the same results. All wells will have a pump & tank combination that will be sufficient enough for each situation.

If you have any questions please feel free to call our office.

Thank You,

Russell Davis

COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL CHECK LIST REQUIRMENTS

MINIMUM PLAN REQUIREMENTS FOR THE FLORIDA BUILDING CODE RESIDENTIAL 2007 ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE with the Current 2007 FLORIDA BUILDING CODES RESIDENTIAL. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FIGURE R301.2(4) of the FLORIDA BUILDING CODES RESIDENTIAL (Florida Wind speed map) **SHALL BE USED.**

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE -------- 100 MPH
ALL BUILDINGS CONSTRUCTED WEST OF SAID LINE SHALL BE --------110 MPH
NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL			Items to Include- Each Box shall be Circled as Applicable		
				Yes	No	N/A
1	Two (2) complete sets of plans containing the following:			✓		
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void			✓		
3	Condition space (Sq. Ft.)	3835	Total (Sq. Ft.) under roof 5535	IIIIIII	IIIIIII	IIIII

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

Site Plan information including:

4	Dimensions of lot or parcel of land	✓		
5	Dimensions of all building set backs	✓		
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	✓		
7	Provide a full legal description of property.	✓		

Wind-load Engineering Summary, calculations and any details required

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
8	Plans or specifications must show compliance with FBCR Chapter 3	IIIIII YES	IIIII NO	IIIIII N/A
9	Basic wind speed (3-second gust), miles per hour	✓		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	✓		
11	Wind importance factor and nature of occupancy	✓		
12	The applicable internal pressure coefficient, Components and Cladding	✓		
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.	✓		

Elevations Drawing including:

14	All side views of the structure	✓		
15	Roof pitch	✓		
16	Overhang dimensions and detail with attic ventilation	✓		
17	Location, size and height above roof of chimneys			✓
18	Location and size of skylights with Florida Product Approval			✓
18	Number of stories	✓		
20A	Building height from the established grade to the roofs highest peak	✓		

Floor Plan including:

20	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	✓		
21	Raised floor surfaces located more than 30 inches above the floor or grade	✓		
22	All exterior and interior shear walls indicated	✓		
23	Shear wall opening shown (Windows, Doors and Garage doors)	✓		
24	Emergency escape and rescue opening shown in each bedroom (net clear opening shown)	✓		
25	Safety glazing of glass where needed	✓		
26	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 of FBCR)			✓
27	Stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails (see FBCR SECTION 311)	✓		
28	Identify accessibility of bathroom (see FBCR SECTION 322)	✓		

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plan (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable

FBCR 403: Foundation Plans

		YES	NO	N/A
29	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	✓		
30	All posts and/or column footing including size and reinforcing	✓		
31	Any special support required by soil analysis such as piling.			✓
32	Assumed load-bearing valve of soil_____ Pound Per Square Foot	✓		
33	Location of horizontal and vertical steel, for foundation or walls (include # size and type)	✓		

FBCR 506: CONCRETE SLAB ON GRADE

34	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)	✓		
35	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports	✓		

FBCR 320: PROTECTION AGAINST TERMITES

36	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or submit other approved termite protection methods. **Protection shall be provided by registered termiticides**	✓		

FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

37	Show all materials making up walls, wall height, and Block size, mortar type	✓		
38	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	✓		

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

39	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer			✓
40	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers			✓
41	Girder type, size and spacing to load bearing walls, stem wall and/or priers			✓
42	Attachment of joist to girder			✓
43	Wind load requirements where applicable			✓
44	Show required under-floor crawl space			✓
45	Show required amount of ventilation opening for under-floor spaces			✓
46	Show required covering of ventilation opening			✓
47	Show the required access opening to access to under-floor spaces			✓
	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &			✓

48	intermediate of the areas structural panel sheathing			✓
49	Show Draftstopping, Fire caulking and Fire blocking	✓		
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 309	✓		
51	Provide live and dead load rating of floor framing systems (psf).			✓

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
	YES	NO	N/A
52 Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	✓		
53 Fastener schedule for structural members per table FBCR 602.3 are to be shown	✓		
54 Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	✓		
55 Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	✓		
56 Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per FBCR Table 502.5 (1)	✓		
57 Indicate where pressure treated wood will be placed	✓		
58 Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	✓		
59 A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	✓		

FBCR :ROOF SYSTEMS:

60	Truss design drawing shall meet section FBCR 802.10 Wood trusses	✓		
61	Include a layout and truss details, signed and sealed by Florida Professional Engineer	✓		
62	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	✓		
63	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	✓		
64	Provide dead load rating of trusses	✓		

FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing			✓
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating			✓
67	Valley framing and support details			✓
68	Provide dead load rating of rafter system			✓

FBCR Table 602,3(2) & FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	✓		
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	✓		

FBCR ROOF ASSEMBLIES FRC Chapter 9

71	Include all materials which will make up the roof assembles covering	✓		
72	Submit Florida Product Approval numbers for each component of the roof assembles covering	✓		

FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter11 Residential buildings compliance methods. *Two of the required forms are to be submitted, showing dimensions condition area equal to the total condition living space area*

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable		
		YES	NO	N/A
73	Show the insulation R value for the following areas of the structure	✓		
74	Attic space	✓		
75	Exterior wall cavity	✓		
76	Crawl space			✓

HVAC information

77	Submit two copies of a Manual J sizing equipment or equivalent computation study	✓		
78	Exhaust fans locations in bathrooms	✓		
79	Show clothes dryer route and total run of exhaust duct	✓		

Plumbing Fixture layout shown

80	All fixtures waste water lines shall be shown on the foundation plan	✓		
81	Show the location of water heater	✓		

Private Potable Water

82	Pump motor horse power	✓		
83	Reservoir pressure tank gallon capacity	✓		
84	Rating of cycle stop valve if used			✓

Electrical layout shown including

85	Switches, outlets/receptacles, lighting and all required GFCI outlets identified	✓		
86	Ceiling fans			
87	Smoke detectors & Carbon dioxide detectors	✓		
88	Service panel, sub-panel, location(s) and total ampere ratings	✓		
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type.	✓		

90	Appliances and HVAC equipment and disconnects	✓		
91	Arc Fault Circuits (AFCI) in bedrooms	✓		

Disclosure Statement for Owner Builders *If you as the applicant will be acting as an owner builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.*

Notice Of Commencement

A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

		YES	NO	N/A
92	**Building Permit Application** A current Building Permit Application form is to be completed and submitted for all residential projects	✓		
93	**Parcel Number** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested	✓		
94	**Environmental Health Permit or Sewer Tap Approval** A copy of a approved Columbia County Environmental Health (386) 758-1058	✓		
95	**City of Lake City** A permit showing an approved waste water sewer tap			✓
96	**Toilet facilities shall be provided for all construction sites**	✓		
97	**Town of Fort White** (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.			✓
98	**Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations	✓		
99	CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the base flood elevation (100 year flood) has been established			✓
100	A development permit will also be required. Development permit cost is **$50.00**	✓		
101	**Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.			✓
102	**911 Address:** If the project is located in an area where a 911 address has not been issued, then application for a 911address must be applied for and **received** through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125	✓		

Section R101.2.1 of the Florida Building Code Residential:

The provisions of Chapter 1, Florida Building Code, Building shall govern the administration and enforcement of the Florida Building Code, Residential.

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the applicati or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance c a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became nu and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date if issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

When the submitted application is approved for permitting the applican will be notified by phone as to the date and time a building permit will b prepared and issued by the Columbia County Building & Zoning Department

PRODUCT APPROVAL SPECIFICATION SHEET

Location:_____ **Project Name:**_____

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and the product approval number(s) on the building components listed below if they will be utilized on the construction project for which you are **applying for a building permit on or after April 1, 2004**. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. More information about statewide product approval can be obtained at www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
A. EXTERIOR DOORS			
1. Swinging	MASONITE	EXTERIOR DOORS	FL 4334-R4
2. Sliding	N/A		
3. Sectional	N/A		
4. Roll up	N/A		
5. Automatic	N/A		
6. Other	N/A		
B. WINDOWS			
1. Single hung	ATRIUM	INSULATED WINDOWS	FL 6752.2
2. Horizontal Slider	N/A		
3. Casement	N/A		
4. Double Hung	N/A		
5. Fixed	N/A		
6. Awning	N/A		
7. Pass -through	N/A		
8. Projected	N/A		
9. Mullion	N/A		
10. Wind Breaker	N/A		
11 Dual Action	N/A		
12. Other	N/A		
C. PANEL WALL			
1. Siding	CertainTeed		FL 12483
2. Soffits	CertainTeed		FL 13389
3. EIFS	N/A		
4. Storefronts	N/A		
5. Curtain walls	N/A		
6. Wall louver	N/A		
7. Glass block	N/A		
8. Membrane	N/A		
9. Greenhouse	N/A		
10. Other	N/A		
D. ROOFING PRODUCTS			
1. Asphalt Shingles	CertainTeed	ARCH SHINGLES	FL 5444-R2
2. Underlayments	WOODLAND		
3. Roofing Fasteners	N/A		
4. Non-structural Metal Rf	N/A		
5. Built-Up Roofing	N/A		
6. Modified Bitumen	N/A		
7. Single Ply Roofing Sys	N/A		
8. Roofing Tiles	N/A		
9. Roofing Insulation	N/A		
10. Waterproofing	N/A		
11. Wood shingles /shakes	N/A		
12. Roofing Slate	N/A		

Category/Subcategory (cont.)	Manufacturer	Product Description	Approval Number(s)
13. Liquid Applied Roof Sys	N/A		
14. Cements-Adhesives – Coatings	CertainTeed	Adhesive (Bxx)	FL 490-R2
15. Roof Tile Adhesive	N/A		
16. Spray Applied Polyurethane Roof	N/A		
17. Other			
E. SHUTTERS	N/A		
1. Accordion	N/A		
2. Bahama	N/A		
3. Storm Panels	N/A		
4. Colonial	N/A		
5. Roll-up	N/A		
6. Equipment	N/A		
7. Others	N/A		
F. SKYLIGHTS			
1. Skylight	N/A		
2. Other	N/A		
G. STRUCTURAL COMPONENTS			
1. Wood connector/anchor	Simpson	Anchors	FL 2355-R3
2. Truss plates	Simpson		FL 10655
3. Engineered lumber	Weyerhaeuser		FL 1630-R5
4. Railing	N/A		
5. Coolers-freezers	N/A		
6. Concrete Admixtures	N/A		
7. Material			
8. Insulation Forms	N/A		
9. Plastics	N/A		
10. Deck-Roof		APA Approved	
11. Wall		APA Approved	
12. Sheds	N/A		
13. Other	N/A		
H. NEW EXTERIOR ENVELOPE PRODUCTS			
1.			
2.			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) the performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

I understand these products may have to be removed if approval cannot be demonstrated during inspectic

Contractor or Contractor's Authorized Agent Signature

Print Name

Date

Permit # (FOR STAFF USE ONLY)

Dicks Residence, Columbia County FL

Wind Load Analysis Requirements

(In Compliance with the 2007 Florida Building Code and 2009 Amendments)

Prepared By: Marty J. Humphries, P.E. # 51976
7932 240th St., O'Brien, FL 32071 (386)935-2406

Description of New Residence:

Footprint: 80' wide x 60' deep rectangular section with a 26'wide x 28'3 1/2" deep stub out on right side for garage. Front porch is inset on left front and is 10' deep x 36'2" and rear porch is inset on rear left and is 55'3" wide by varying depth(up to 12' deep) (see plans by Christopher Dicks)

Walls: 9' walls - 2x6-16" O.C. with 7/16" OSB sheathing and brick veneer and ½"gypsum-wallboard interior. (Note: wall between garage and heated area of home shall be sheathed with 7/16" OSB and 1/2" gypsum wallboard on the garage side)

Roof Structure: Pre-engineered roof trusses and 7/16" OSB sheathing (min.)

Roof Type: gable roof (analyzed for 2' eave overhangs and porch areas)

Foundation: footer & stemwall with slab and special footers as shown

Windload Data and Exposure:

Basic Wind Speed = 110 mph

Importance Factor = 1.0

Exposure category = B

Height and Exposure Adjustment Coefficient = 1.0

Residential Occupancy = Group R3

Analysis Method = ASCE 7-05

Component and Cladding Pressures: Roof – Zone 1=19.9,-21.8, Zone 2=19.9 -25.5, Zone 3=19.9,-25.5, Wall – Zone 4=21.8,-23.6, Zone 5 =21.8, -29.1

Mean roof height = 18'

Roof Cross Slope = 6:12 primarily with 3:12 portion along front

Eave Overhang= (Analyzed for 2' and porch areas)

Wall Height = 9' plate ht

Shear Wall locations = exterior walls (>3' in length)(all exterior walls shall be sheathed), wall between garage and heated area of home shall be sheathed and strapped as required for exterior walls.

Nailing Pattern Requirements:

Wall sheathing: (exterior walls & (garage wall)	Shall be 7/16" Oriented Strand Board(OSB) minimum nailed with 8d common nails 3" on center around edges(including around doors and windows) and 6" on center interior. Long dimension of sheathing shall be installed vertical and full depth blocking shall be installed at horizontal joints in sheathing.

Marty J. Humphries
3-11-10

Roof sheathing: Shall be 7/16" Oriented Strand Board(OSB) min. nailed with 8d ring shank nails 6" on center at panel ends and overhangs and 6" on center elsewhere.

Top wall plate: Nail with 1-16d common nail 12" O.C.(average)

Strapping and Anchor Requirements:

truss to bearing plate locations: At trusses to exterior wall locations install Simpson H10 anchors with the exception of trusses A2, A4, A5, A6, A9 & A10, M1, M2, MGE & MGE2. Connect M1, M2, MGE & MGE2 to front porch beam and front wall with one Simpson H2.5A anchor each location. Connect A4 to exterior walls with Simpson H14 anchor. Connect A2 to front exterior wall with Simpson H14 anchor and to rear exterior wall with Simpson H10 anchor. Connect A9 & A10 to exterior walls with Simpson H10-2 anchors. Connect all trusses to interior load bearing walls with Simpson H2.5A anchor each location with the exception of trusses A5, A6. Connect trusses to rear porch beam with Simpson H2.5A anchors with the exception of A5 & A6 with will require 2-H2.5A anchors at rear porch beam.

Strapping for A5 shall be as follows: at front and rear wall at each side of door install 5/8" all thread connected to 5/8"x10" anchor bolt with Simpson CNW5/8 and extended to top of wall, install 5/8" nut with 3"x3" square washer. Install Simpson VGTR(1) with 5/8" all-thread thru header with 3"x 3"square washer & 5/8" nut.

Strapping for A6 shall be as follows: at rear wall and front bearing wall at bedroom 3 - at each side of window/door install 5/8" all thread connected to 5/8" x 10" anchor bolt with Simpson CNW5/8 and extended to top of wall, install 5/8" nut with 3"x 3" square washer. Install Simpson VGTR(1) with 5/8" all-thread thru header with 3"x 3"square washer & 5/8" nut. At wall between bathroom and kitchen install 5/8" all thread connected to 5/8" anchor bolt with Simpson CNW5/8 and extended to Simpson VGTR(1) at top of wall connected to A6.

wall strap tie requirements: (exterior walls and wall between garage and heated area of home, and interior load bearing walls) At top and bottom of wall install one Simpson model SP6 at each side of each door and window under 4' in width. At top and bottom of wall for windows and doors larger than 4' in width install two Simpson model SP6's each side of each opening. All other wall locations install SP6's top and bottom of wall 4' on center. At each side of garage door openings at top and bottom of the wall install 2-SPH4's. For interior load bearing walls install SP6's or SP4's depending on the thickness of the wall 32" on center at the top and bottom of the wall and each side of doors.

Marty J. Hay
3-11-10

Porch Columns: ABU44 & AC4Max each column (AC4EMax may be installed at end columns)

Lookouts: Install one Simpson model H5 where lookouts connect to end gable truss.

Gable end: Install one LSTA18 - 4' on center connecting gable end truss to wall framing.

Gable End Bracing Requirements:
At each gable end install one 2x4 SPF 8' stud spaced 6' on center horizontal along top of bottom chord of trusses, nail with 2-12d nails at each truss including end truss. In addition, install a 2x4 brace extending from this stud at the gable end truss approx. 45 degrees to truss at roof sheathing, nail with 2 -12d nails where it crosses truss members and at ends. Gable end trusses shall be built to receive sheathing with vertical members 2' on center. Vertical members of gable end truss greater than 5' in height shall be stiffened with one 2x4 SPF nailed with 12d nails 8" on center to back of vertical member. (See attached detail) Bracing not required where 3/4" T&G subfloor installed in attic areas.

Dormers:
Dormer walls shall be 2x4 stud walls, bottom plate connected to roof trusses with Simpson 1/4"x 3 1/2" SDS screw 2' on center. Sheathing for dormers shall be 7/16" OSB nailed as required for wall sheathing. No additional strapping is required. At joints in sheathing full depth blocking is required.

Foundation Requirements:
(see attached details for footer requirements)

Header Requirements:

Windows/Doors: Minimum header shall be 2 - #2 SYP 2x12's w 1/2" OSB or plywood between nailed w 12d nails 10" on center top & bottom.

Front Porch Header: Minimum header shall be 2 - #2 SYP 2x10's w 1/2" OSB or plywood between nailed w 12d nails 10" on center top & bottom. Install Simpson HUC410 where beam connects to wall.

Rear Porch Header: Minimum header shall be 2 - #2 SYP 2x12's w 1/2" OSB or plywood between nailed w 12d nails 10" on center top & bottom. Install Simpson HUC410 where beam connects to wall.

At front door & rear French drs: (Truss A5) Minimum header shall be 1-LVL beam 5.25" x 16" Fb=2650 and E =1.8 million. psi(4-2x6 #2 SYP cripples each end of header)

3-11-10

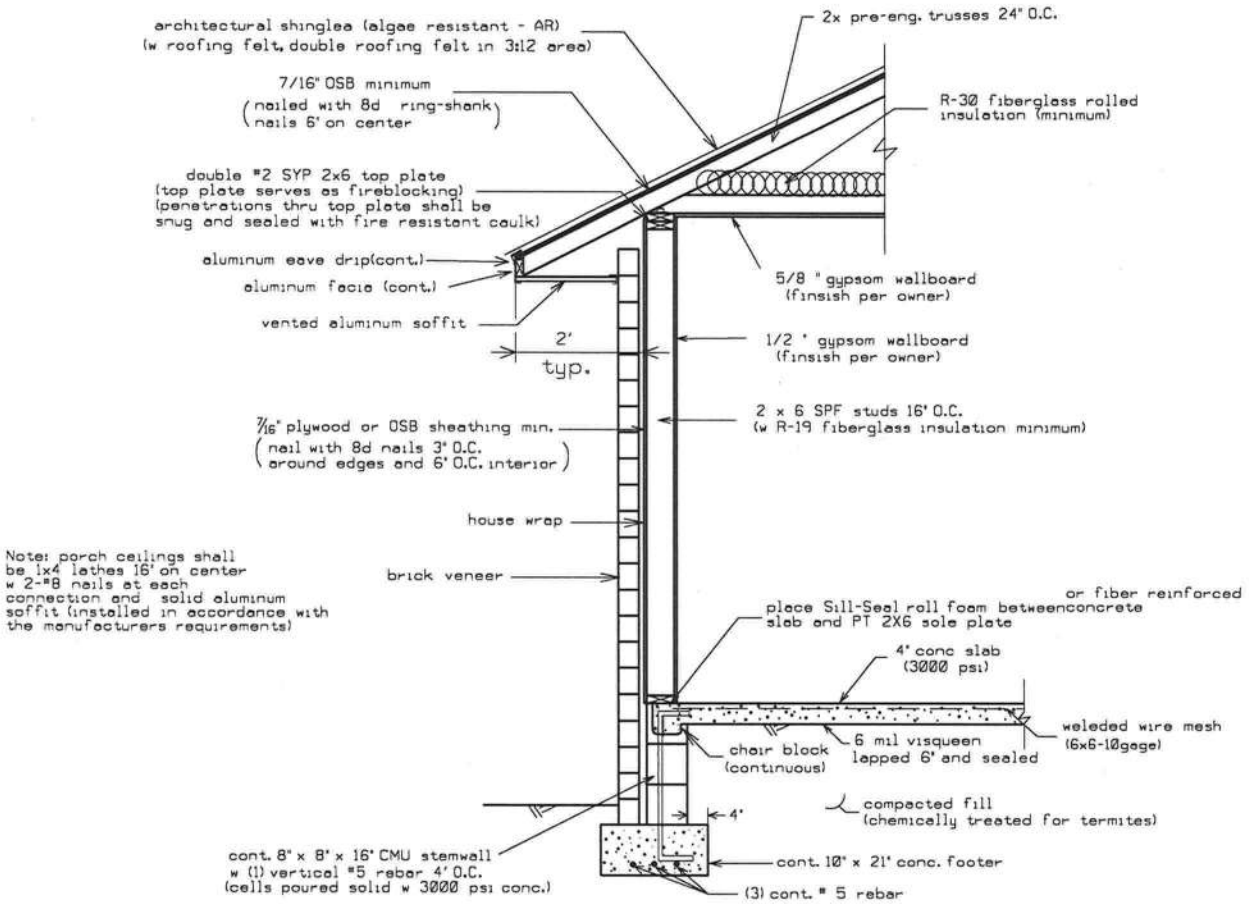
At Window under Truss A6:	Minimum header shall be 1-LVL beam 5.25" x 16" Fb=2650 and E =1.8 million. Psi(4-#2 SYP 2x6 cripples each end of header)
At Bedroom 3 door: Truss A6:	Minimum header shall be 3-#2 SYP 2x12's with 1/2" OSB or plywood between nailed w 12d nails 10" on center top & bottom(2-#2 SYP 2x6 cripples each end of header)

Note: where truss A6 bears on wall between bathroom and dining area 4-#2 SYP 2x6 studs shall be installed in wall.

Garage Door Hdrs:	Minimum header shall be 2- #2 SYP 2x10's with 2x6 top and bottom Nailed with 12 nails 10" on center (2#2 SYP 2x6 cripples each end of header)

Equivalent capacity anchors may be substituted, installed in accordance with the manufacturers requirements.

3-11-10

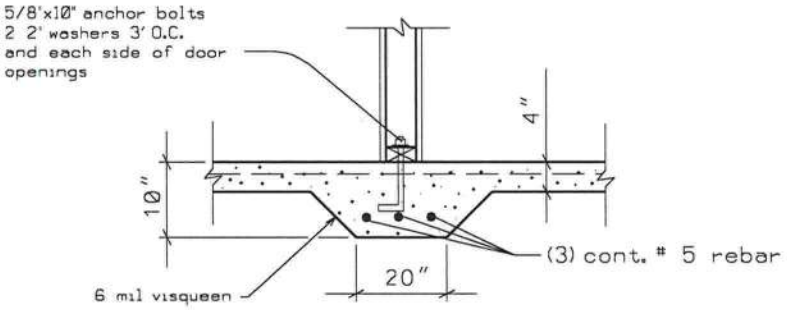


TYPICAL WALL SECTION(N.T.S.)

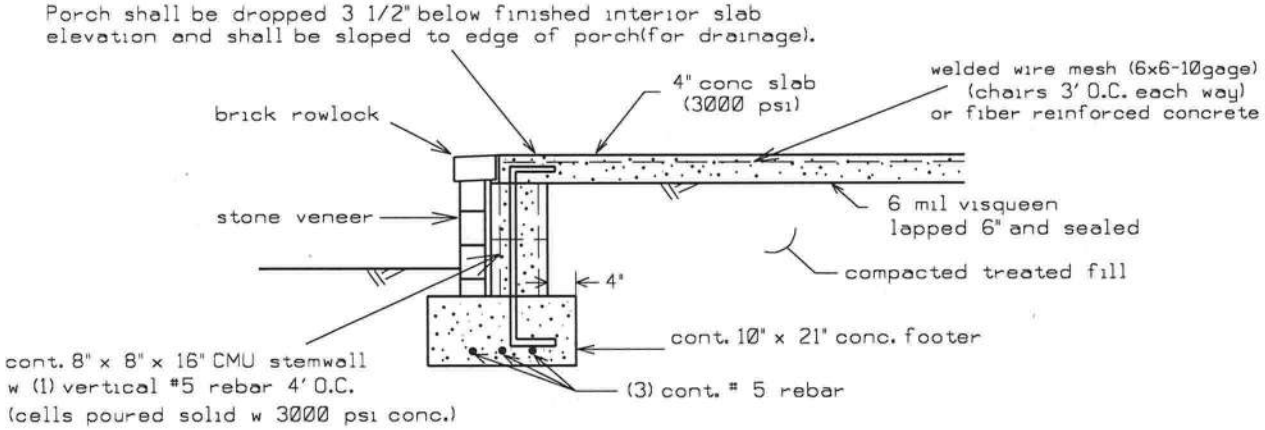
Marty J. Humphries

3-11-10

Dicks Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071



INTERIOR MONOLITHIC FOOTER DETAIL(N.T.S.)

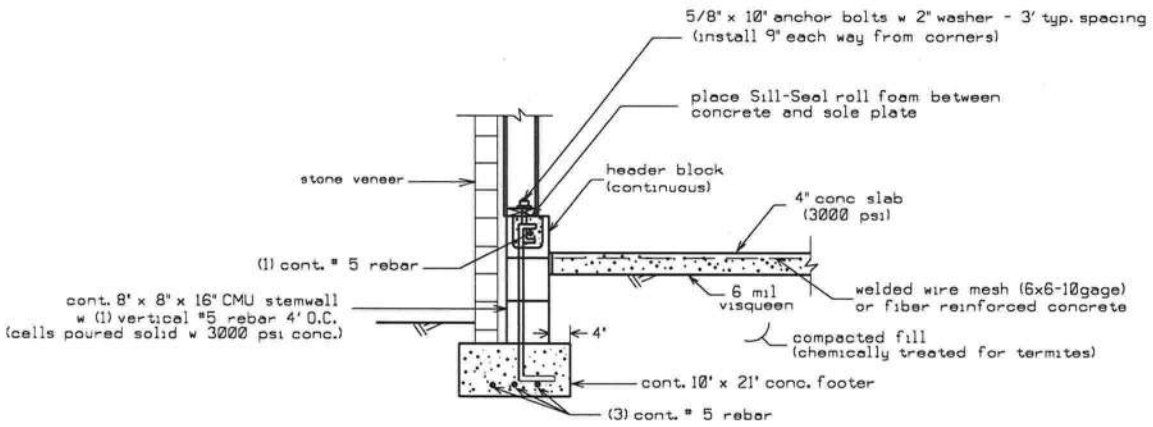


PORCH FOUNDATION(N.T.S.)

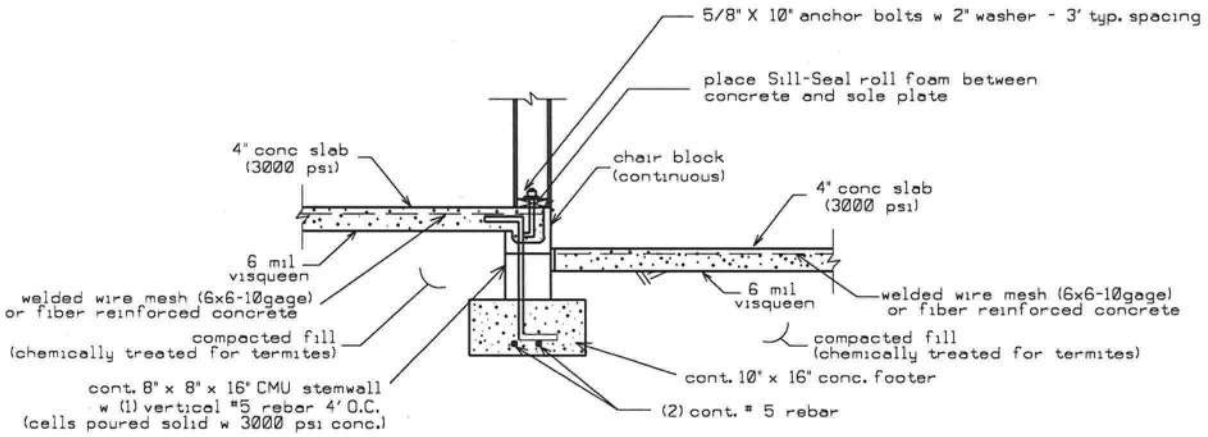
Marty J. Humphries

3-11-10

Dicks Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071



GARAGE EXTERIOR WALL FOUNDATION(N.T.S.)

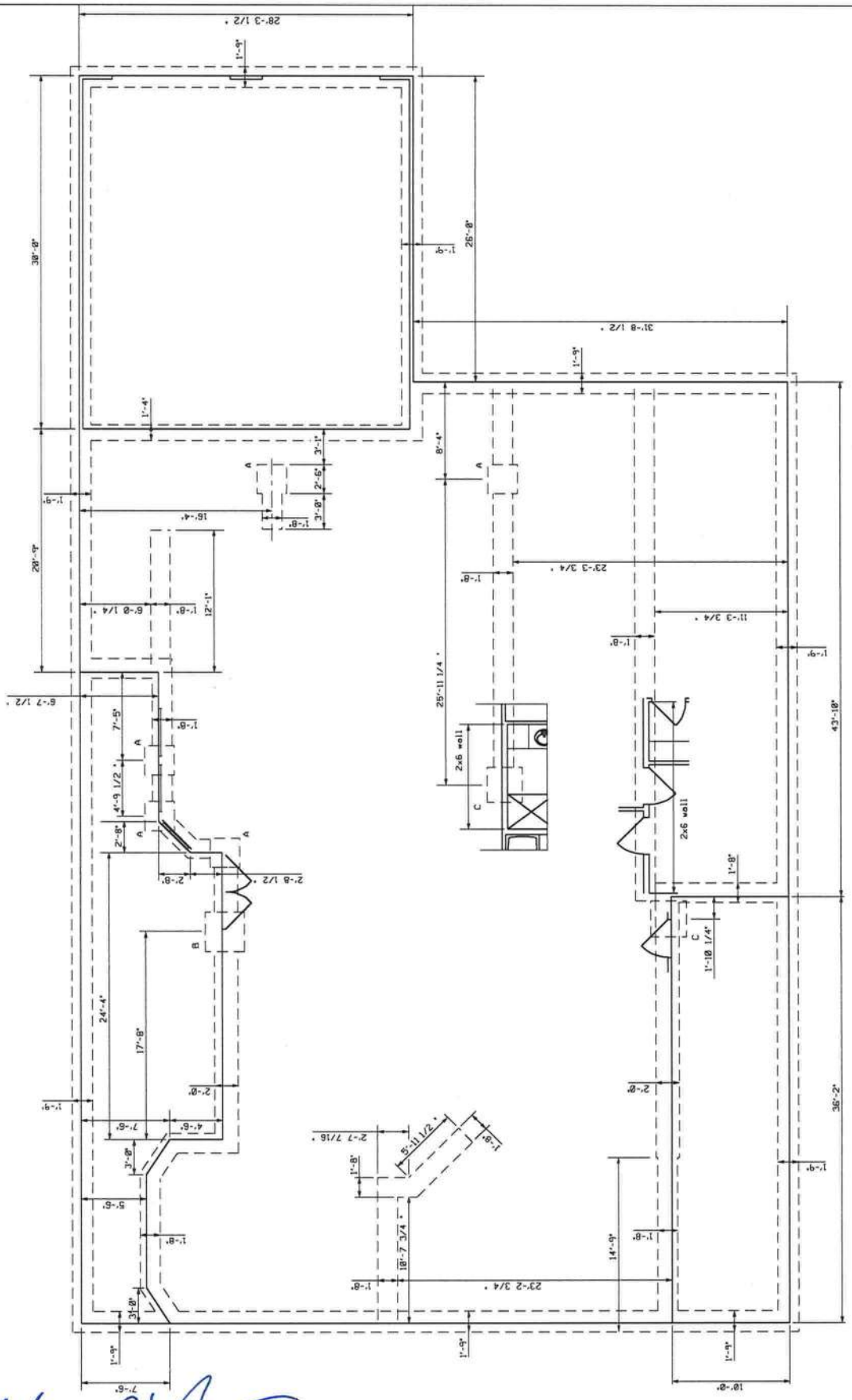


GARAGE ADJACENT WALL FOUNDATION(N.T.S.)

Marty J. Humphries

3-11-10

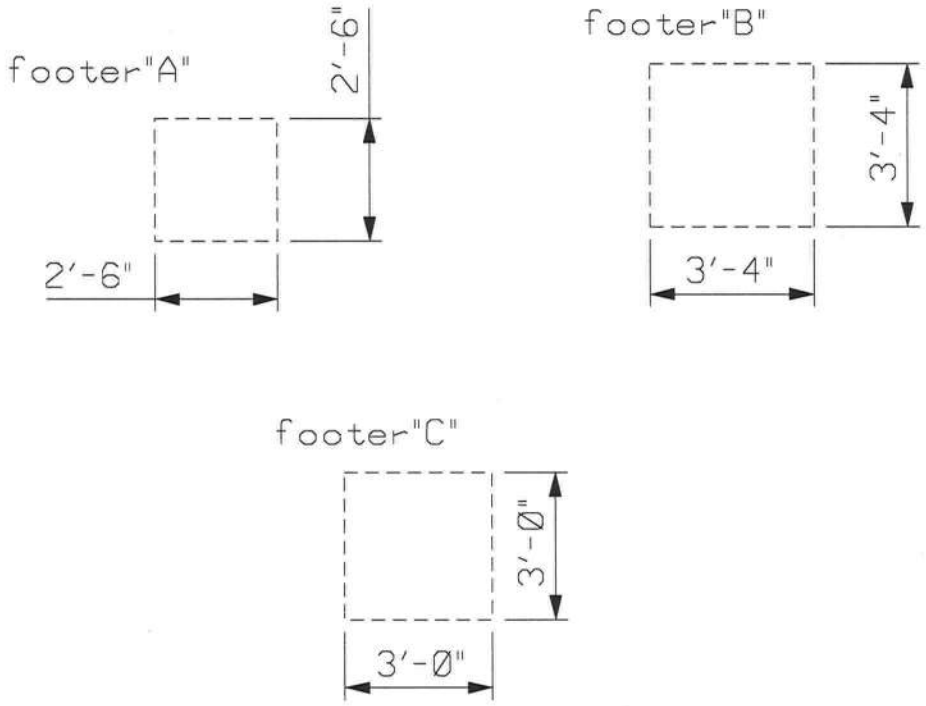
Dicks Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071



FOUNDATION PLAN

Marty O. Humphries
3-11-10

Dicks Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071



special footers

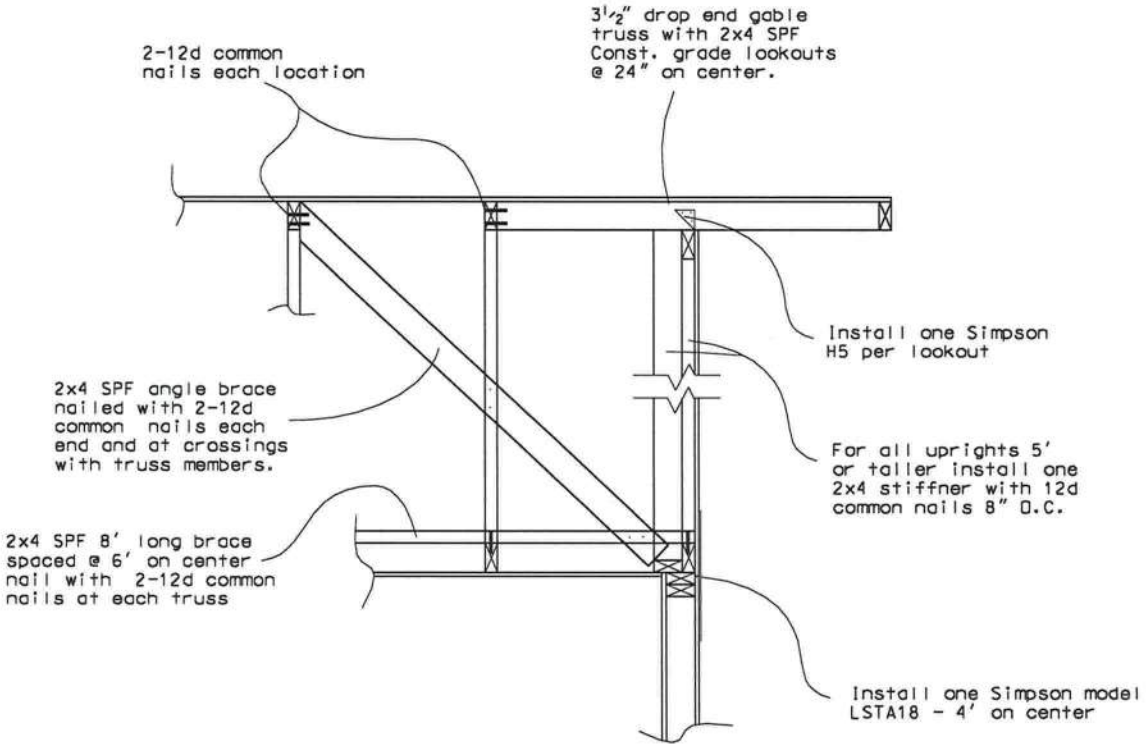
1.) Footer "A" shall be 14" thick with one reinf. mat of 5-#5 rebars each way. (3000 psi conc.)

2.) Footers "B" & "C" shall be 14" thick with one reinf. mat of 6-#5 rebars each way. (3000 psi conc.)

3.) Longitudinal wall footer reinforcement shall run thru these footers.

Marty J. Humphries
3-11-10

Dicks Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071



GABLE END BRACING DETAIL (N.T.S.)

Note: Bracing is not required where 3/4" T&G plywood is installed in upstairs attic area.

Marty J. Humphries
3-11-10

Dicks Residence Columbia County, FL	DETAIL PREPARED BY: MARTY J. HUMPHRIES P.E. # 51976 7932 240TH ST., O'BRIEN, FL 32071

NEW! The H2.5A is symetrically designed for easy installation, with higher uplift loads to meet new code requirements. A placement mark allows easy installation on double top plates.

NEW! The H5A has an installed cost benefit, as it only requires 6 nails, to meet lower uplift requirements.

The H connector series provides wind and seismic ties for trusses and rafters.

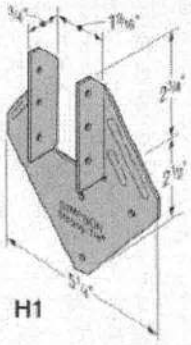
Allowable loads for more than one direction for a single connection cannot be added together. A design load which can be divided into components in the directions given must be evaluated as follows:
Design Shear/Allowable Shear + Design Tension/Allowable Tension < 1.0.

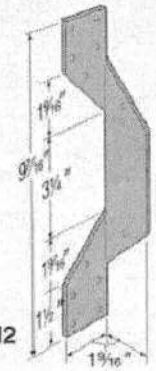
MATERIAL: See table

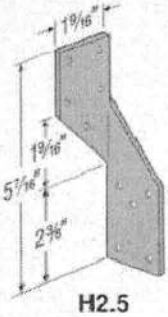
FINISH: Galvanized; H10-2, H11Z–Z-MAX. Other models available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners. See General Notes.
- H1 can be installed with flanges facing outwards (reverse of drawing number 1). When installed inside a wall, a birdsmouth cut is required.
- H2.5, H3, H4, H5 and H6 ties are shipped in equal quantities of rights and lefts.
- Bend the H7 over the top of the truss. Install a minimum of four 8d nails into the truss, including two into the truss side.
- Hurricane Ties do not replace solid blocking.

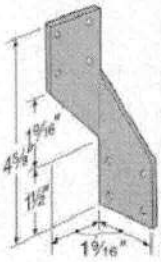
CODES: BOCA, ICBO, SBCCI NER-422, NER-393, NER-432; NER-499; City of L.A. RR 24818; Dade Co, FL 00-0512.11 (H10); 00-0926.01 (H1, H2.5, H3, H4, H5).

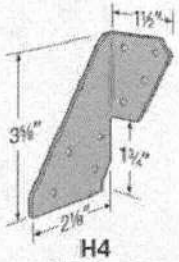

H1

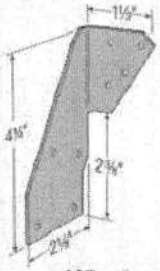

H2

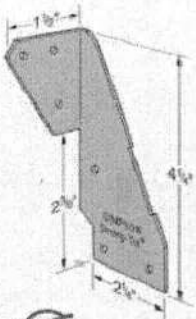

H2.5


H2.5A

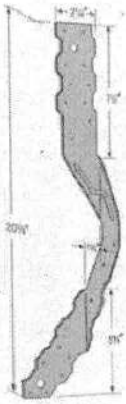

H3

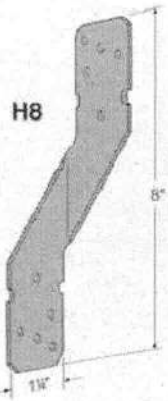

H4
U.S. Patent 4,714,372

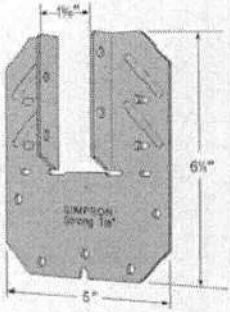

H5
U.S. Patent 4,714,372

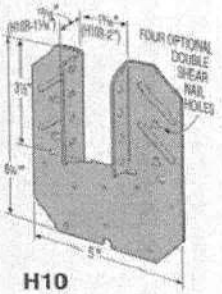

H5A
U.S. Patent 4,714,372


H6

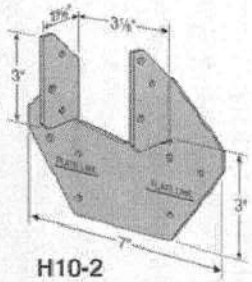

H7

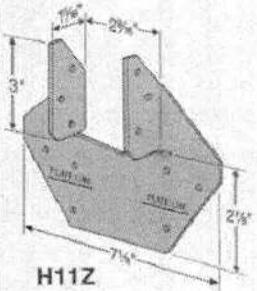

H8


H9


H10
(H10R similar)
U.S. Patents 4,480,941; Canada Patent 1,193,418

Model No.	Ga	Fasteners: To Rafters/ Truss	Fasteners: To Plates	Fasteners: To Studs	Uplift Avg Ult	Doug-Fir Larch/So. Pine Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½ Nails (133 & 160)	Spruce-Pine-Fir Allowable Loads[1,2]: Uplift (133)	Uplift (160)	Lateral (133/160) F_1	Lateral (133/160) F_2	Uplift Load with 8dx1½ Nails (133 & 160)
H1	18	6-8dx1½	4-8d	—	1958	490	585	485	165	455	400	400	415	140	370
H2	18	5-8d	—	5-8d	1040	335	335	—	—	335	230	230	—	—	230
H2.5	18	5-8d	5-8d	—	1300	415	415	150	150	415	365	365	130	130	365
H2.5A	18	5-8d	5-8d	—	1793	600	600	110	110	480	520	535	110	110	480
H3	18	4-8d	4-8d	—	1433	455	455	125	160	415	320	320	105	140	290
H4	20	4-8d	4-8d	—	1144	360	360	165	160	360	235	235	140	135	235
H5	18	4-8d	4-8d	—	1485	455	465	115	200	455	265	265	100	170	265
H5A	18	3-8d	3-8d	—	1500	350	420	115	180	290	245	245	100	120	170
H6	16	—	8-8d	8-8d	3983	915	950	650	—	—	785	820	560	—	—
H7	16	4-8d	2-8d	8-8d	2991	930	985	400	—	—	800	845	345	—	—
H8	18	5-10dx1½	5-10dx1½	—	2422	620	745	—	—	—	530	565	—	—	—
H9KT	18	4-SDS¼x1½	5-SDS¼x1½	—	2812	875	875	680	125	—	755	755	680	125	—
H10	18	8-8dx1½	8-8dx1½	—	3136	905	990	585	525	—	780	850	505	450	—
H10R	18	8-8dx1½	8-8dx1½	—	3135	905	990	585	525	—	780	850	505	450	—
H10-2	18	6-10d	6-10d	—	2447	760	760	455	395	—	655	655	390	340	—
H11Z	18	6-16dx2½	6-16dx2½	—	5097	830	830	525	760	—	715	715	450	655	—

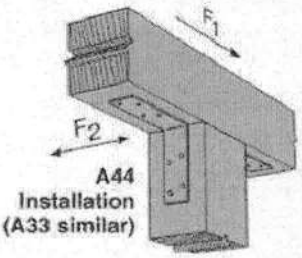

H10-2


H11Z

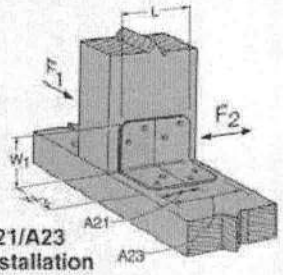
1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed.
2. Allowable loads are for one anchor. A minimum rafter thickness of 2½" must be used when framing anchors are installed on each side of the joist and on the same side of the plate.
3. Allowable uplift load for stud to bottom plate installation is 400 lbs (H2.5); 390 lbs (H2.5A); 360 lbs (H4) and 310 lbs (H8).
4. The H9KT is sold in 20 piece packs with screws.
5. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.
6. Hurricane Ties are shown installed on the outside of the wall for clarity. Installation on the inside of the wall is acceptable. For a Continuous Load Path, connections must be on same side of the wall.

Z2 clips secure 2x4 flat blocking between joists or trusses to support sheathing.
MATERIAL: Z clips–see table. A21 and A23–18 ga.; all other A angles–12 ga.
FINISH: Galvanized
INSTALLATION: • Use all specified fasteners. See General Notes.
• Z clips do not provide lateral stability. Do not walk on stiffeners or apply load until diaphragm is installed and nailed to stiffeners.
CODES: BOCA, ICBO, SBCCI NER-421 (except A33, A44); City of L.A. RR 25076 (except A33, A44); Dade Co. FL 99-0623.04 (A21 and A23).

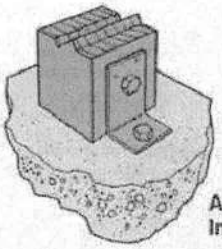
Model No.	Dimensions			Fasteners				Avg Ult F_2	Allowable Loads[2] DF/SP			
				Base		Post			(133)		(160)	
	W_1	W_2	L	Bolts	Nails	Bolts	Nails		F_1	F_2	F_1	F_2
A21	2	1½	1⅜	—	2-10dx1½	—	2-10dx1½	540	245	175	290	175
A23	2	1½	2¾	—	4-10dx1½	—	4-10dx1½	1767	485	485	585	565
A33	3	3	1½	—	4-10d	—	4-10d	2635	625	330	750	330
A44	4⅜	4⅜	1½	—	4-10d	—	4-10d	2490	625	295	750	295
A66	5⅞	5⅞	1½	2-⅞	—	2-⅞	—	N/A	N/A	N/A	N/A	N/A
A88	8	8	2	3-⅞	—	3-⅞	—	N/A	N/A	N/A	N/A	N/A
A24	3⅝	2	2½	1-½	—	1-½	2-10d	N/A	N/A	N/A	N/A	N/A
A311	11	3⅝	2	1-½	—	1-½	4-10d	N/A	N/A	N/A	N/A	N/A

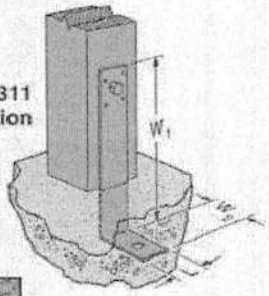


A44 Installation (A33 similar)



A21/A23 Installation

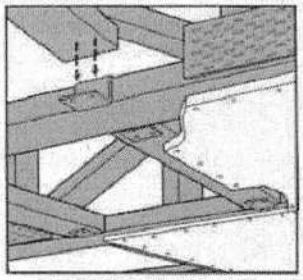

A24 Installation

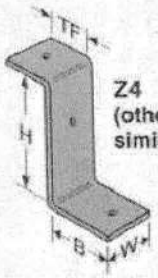


A311 Installation

Model No.	Ga	Dimensions				Fasteners[1] (Total)	Avg Ult	Allowable[2] Download (125)
		W	H	B	TF			
Z2	20	2⅜	1½	1⅜	1⅜	4-10dx1½	1507	465
Z4	12	1½	3½	2⅛	1¼	2-16d	1450	465
Z6	12	1½	5½	2	1⅜	2-16d	1517	485
Z28	28	2⅜	1½	1⅜	1⅜	10dx1½[3]	—	—
Z38	28	2⅜	2½	1⅜	1⅜	10dx1½[3]	—	—
Z44	12	2⅜	3½	2	1⅜	4-16d	2800	865

1. Z28 and Z38 do not have nail holes. Fastener quantities are as required.
2. Allowable loads have been increased 25% for roof loading (Z clips), 33% and 60% for earthquake or wind loading (A angles); no further increase allowed; reduce for other load durations according to the code.
3. Z4 and Z6 loads apply with a nail into the top and a nail into the seat.


Typical Z2 Installation

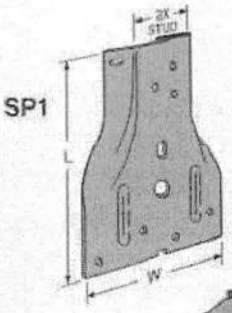


Z4 (others similar)

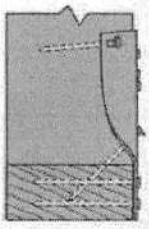
SP/SPH/RSP4 STUD PLATE TIES

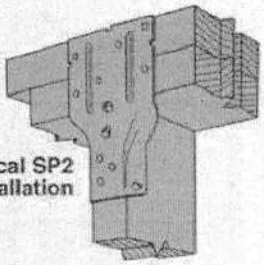
The RSP4 is a reversible stud plate tie with locating tabs, which aid placement on double top plates or a single bottom plate.
MATERIAL: SPH–18 gauge, all others–20 gauge **FINISH:** Galvanized
INSTALLATION: • Use all specified fasteners; see General Notes.
• SP–one of the 10d common stud nails is driven at a 45° angle through the stud into the plate.
CODES: BOCA, ICBO, SBCCI NER-432, NER-443, NER-499; SBCCI 9603A; City of LA RR 25318 (RSP4); Dade Co. FL 99-0623.04 (SP1, SP2, SP4, SP6, SP8).

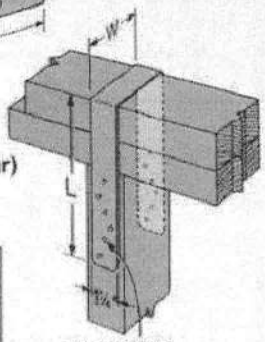
Model No.	Dimensions		Fasteners		Avg Ult	Allowable Uplift Loads DF/SP	
	W	L	Stud[1]	Plate		(133)[2]	(160)[2]
SP1	3½	5⅛	6-10d	4-10d	1950	585	585
SP2	3½	6⅜	6-10d	6-10d	3300	890	1065
SP3	4½	6⅜	6-10d	6-10d	3467	890	1065
SP4	3⅜	7¼	6-10dx1½	—	2917	735	885
SP5	4½	5⅛	6-10d	4-10d	1950	585	585
SP6	5⅜	7¼	6-10dx1½	—	2917	735	885
SP8	7⅜	8⅝	6-10dx1½	—	2917	735	885
SPH4	3⅜	8¾	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH6	5⅜	9¼	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
SPH8	7⅜	8¾	10-10dx1½	—	3993	1240	1240
			12-10dx1½	—	4470	1360	1360
RSP4 (1)	2⅜	4⅛	4-8dx1½	4-8dx1½	1032	315	315
RSP4 (2)	2⅜	4⅛	4-8dx1½	4-8dx1½	1445	450	450



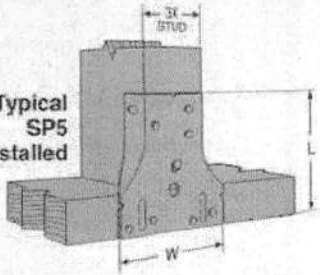
SP1


SP1 Nailing Profile

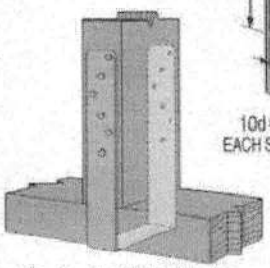

Typical SP2 Installation

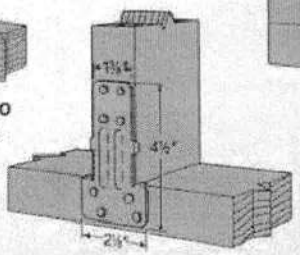


Typical SPH Installation (SP4, 6, 8 similar)

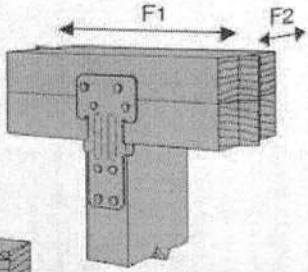


Typical SP5 Installed


Typical SPH4 Stud to Single Bottom Plate



(1) Typical RSP4 Stud to Single Bottom Plate



(2) Typical RSP4 Stud to Double Top Plate (see footnote 4)

1. SP1, 2, 3 and SP5: drive one stud nail at an angle through the stud into the plate to achieve the table load (see illustration).
2. Allowable loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
3. RSP4–see Installation details (1) and (2) for reference.
4. RSP4 F2 is 280 lbs (installation 1) and 305 lbs (installation 2). F1 load is 210 lbs for both installations.
5. Maximum load for SPH in Southern Yellow Pine is 1490 lbs.
6. When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement

U.S. Patent 5,697,725

The MSTC series has countersunk nail slots for a lower nailing profile. Coined edges ensure safer handling. The RPS meets UBC and City of Los Angeles code requirements for notching plates where plumbing, heating or other pipes are placed in partitions.

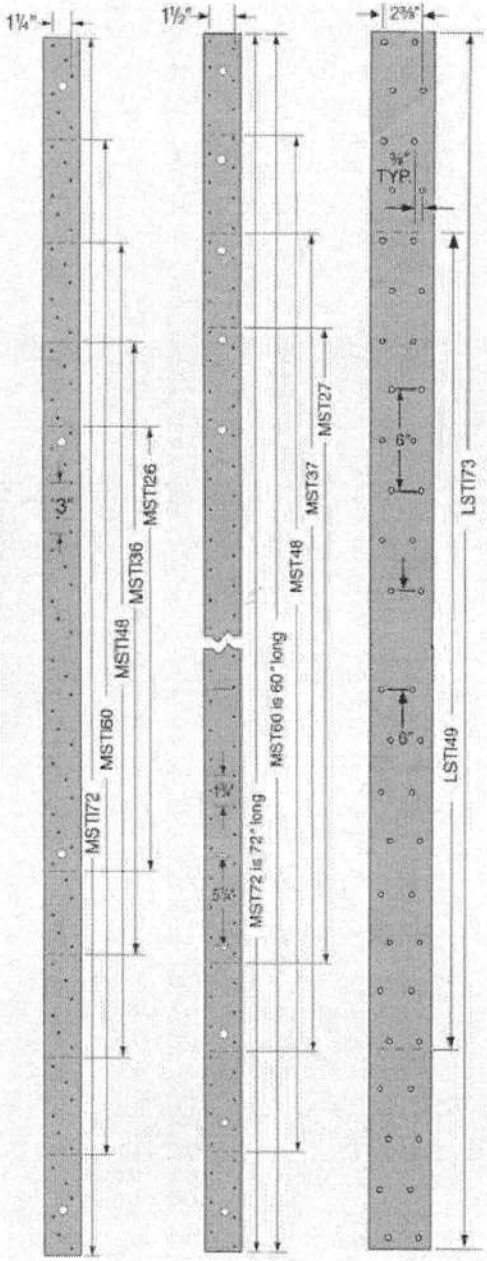
Install Strap Ties where plates or soles are cut, at wall intersections, and as ridge ties. LSTA and MSTA straps are engineered for use on 1½" members. The 3" center-to-center nail spacing reduces the possibility of splitting. For the MST, this may be a problem on lumber narrower than 3½"; either fill every nail hole with 10dx1½" nails or fill every other nail hole with 16d commons. Reduce the allowable load based on the size and quantity of fasteners used. The LSTI light strap ties are suitable where gun-nailing is necessary through diaphragm decking and wood chord open web trusses.

FINISH: HST–Simpson gray paint; PS–HDG; all others–galvanized. Some products are available in stainless steel or Z-MAX; see Corrosion-Resistance, page 5.

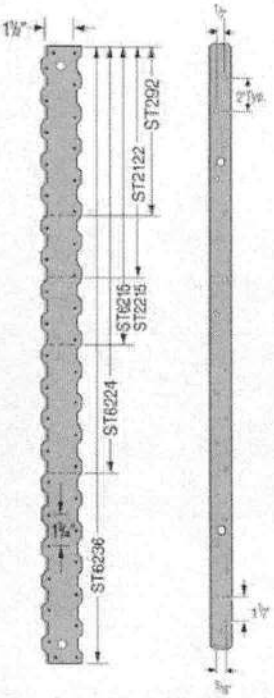
INSTALLATION: Use all specified fasteners. See General Notes.

OPTIONS: Special sizes can be made to order. See also HCST.

CODES: BOCA, ICBO, SBCCI NER-413, NER-443; ICBO 4935, 5357; Dade County, FL. 00-1023.05 (MSTA30, MSTA36, ST12, ST18, ST22); City of L.A. RR 25119, RR 25149, RR 25281.

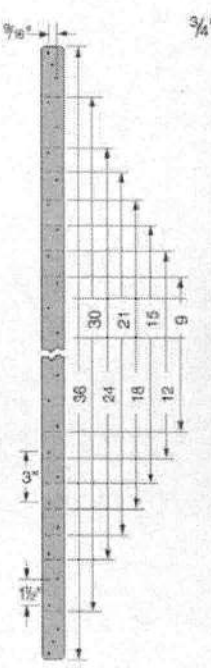


MSTI MST LSTI

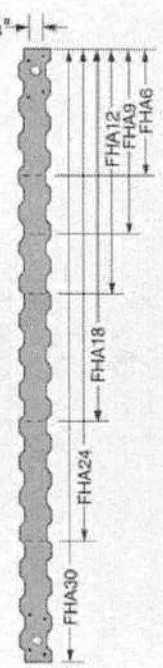


ST
U.S. Patent 4,367,973

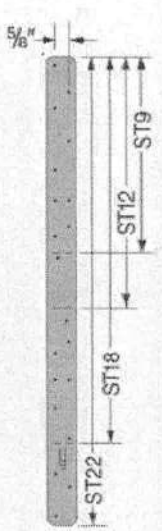
MSTAM36



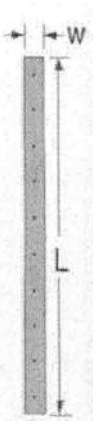
LSTA and MSTA (pilot holes not shown)



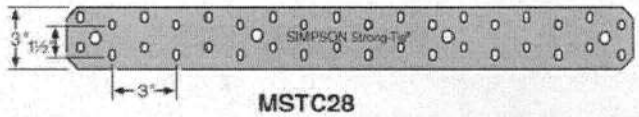
FHA



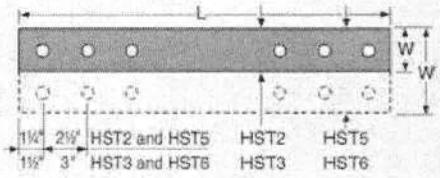
ST9, ST12, ST18, ST22



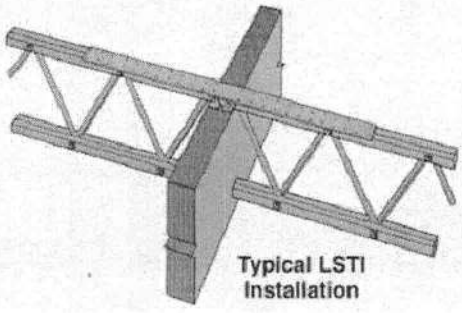
ST2115



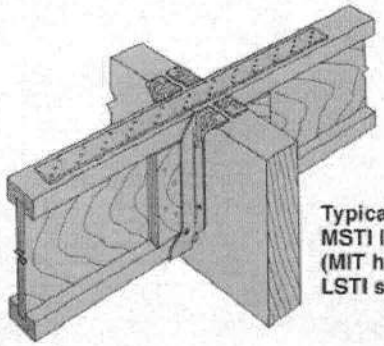
MSTC28



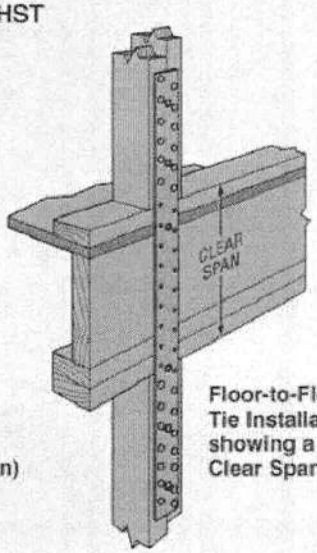
HST



Typical LSTI Installation

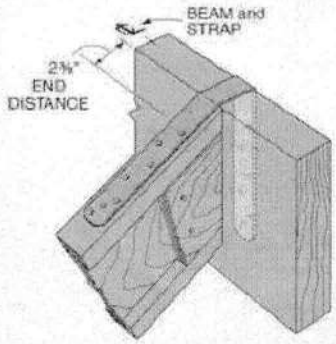


Typical MSTI Installation (MIT hanger shown) LSTI similar

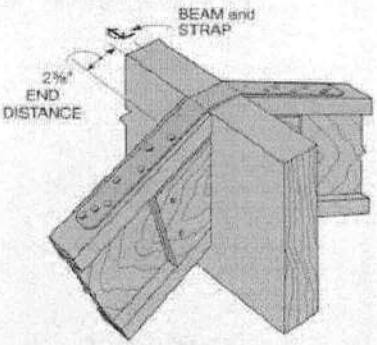


Floor-to-Floor Tie Installation showing a Clear Span

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Allowable Tension Loads Floor (100)	(133)	(160)
RPS18	16	1½	18 5/16	12-16d	810	1080	1295
RPS22		1½	22 5/16	16-10d	905	1205	1445
RPS28		1½	28 5/16	12-16d	810	1080	1295
LSTA9	20	1¼	9	8-10d	450	605	725
LSTA12		1¼	12	10-10d	565	755	905
LSTA15		1¼	15	12-10d	680	905	1085
LSTA18		1¼	18	14-10d	790	1055	1265
LSTA21		1¼	21	16-10d	905	1205	1295
LSTA24		1¼	24	18-10d	1015	1295	1295
ST292		2 5/16	9 5/16	12-16d	790	1055	1130
ST2122		2 5/16	12 5/16	16-16d	1070	1425	1505
ST2115		¾	16 5/16	10-16d	450	600	600
ST2215		2 5/16	16 5/16	20-16d	1270	1695	1695
LSTA30	18	1¼	30	22-10d	1255	1670	1715
LSTA36		1¼	36	26-10d	1480	1715	1715
LSTI49		3¾	49	32-10dx1½	1455	1940	2330
LSTI73		3¾	73	48-10dx1½	2185	2910	3495
MSTA9		1¼	9	8-10d	455	610	730
MSTA12		1¼	12	10-10d	570	760	910
MSTA15		1¼	15	12-10d	685	910	1095
MSTA18		1¼	18	14-10d	800	1065	1275
MSTA21		1¼	21	16-10d	910	1215	1460
MSTA24		1¼	24	18-10d	1025	1370	1640
MSTA30	16	1¼	30	22-10d	1265	1685	2025
MSTA36		1¼	36	26-10d	1495	1995	2135
ST6215		2 5/16	16 5/16	20-16d	1330	1775	2130
ST6224		2 5/16	23 5/16	28-16d	1890	2520	2630
ST9		1¼	9	8-16d	530	705	850
ST12		1¼	11½	10-16d	665	885	1065
ST18		1¼	17¼	14-16d	900	1200	1200
ST22		1¼	21¼	18-16d	1025	1370	1370
MSTC28		3	28¼	36-16d sinkers	2070	2760	3310
MSTC40		3	40¼	52-16d sinkers	2990	3985	4740
MSTC52		3	52¼	62-16d sinkers	3555	4740	4740
MSTC66	14	3	65¾	76-16d sinkers	4390	5855	5855
MSTC78		3	77¾	76-16d sinkers	4390	5855	5855
ST6236		2 5/16	33 5/16	40-16d	2575	3430	3430
FHA6	12	1 5/16	6⅜	8-16d	550	735	885
FHA9		1 5/16	9	8-16d	550	735	885
FHA12		1 5/16	11⅜	8-16d	550	735	885
FHA18		1 5/16	17¼	8-16d	550	735	885
FHA24		1 5/16	23⅜	8-16d	550	735	885
FHA30		1 5/16	30	8-16d	550	735	885
MSTI26		2 5/16	26	26-10dx1½	1130	1510	1810
MSTI36		2 5/16	36	36-10dx1½	1565	2090	2505
MSTI48		2 5/16	48	48-10dx1½	2135	2850	3420
MSTI60		2 5/16	60	60-10dx1½	2760	3680	4415
MSTI72		2 5/16	72	72-10dx1½	3310	4415	4725



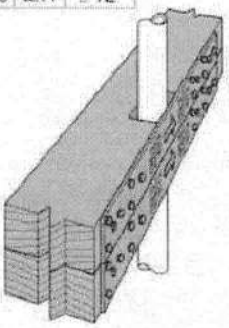
Typical LSTA Installation (hanger not shown)



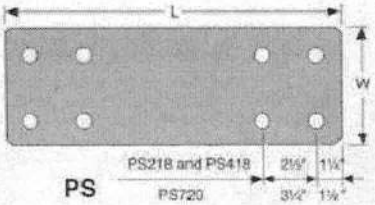
Typical LSTA Installation (hanger not shown)

Model No.	Plate	Notch Width
RPS18	2x4	≤ 5½"
RPS22	2x6	≤ 5½"
RPS28	2x4	≤ 12"

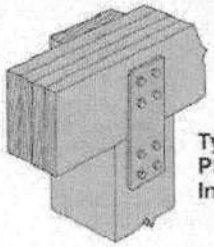
RPS



Typical RPS Installation



PS



Typical PS720 Installation

Model No.	Ga	Dimensions W	Dimensions L	Bolts Qty	Bolts Dia
PS218[6]	7	2	18	4	⅝
PS418[6]		4	18	4	⅝
PS720[6]		6¾	20	8	½

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Load (133)	(160)
MSTC28	18	12-16d sinker	920	1105
	16	16-16d sinker	1225	1470
MSTC40	18	28-16d sinker	2145	2575
	16	36-16d sinker	2455	2945
MSTC52	18	44-16d sinker	3375	4050
	16	48-16d sinker	3680	4415
MSTC66	18	64-16d sinker	5035	5855
	16	68-16d sinker	5350	5855
MSTC78	18	80-16d sinker	5855	5855
	16	80-16d sinker	5855	5855
MST37	18	20-16d	1905	2285
	16	22-16d	2100	2515
MST48	18	32-16d	3135	3765
	16	34-16d	3330	4000
MST60	18	46-16d	4785	5740
	16	48-16d	4990	5800
MST72	18	56-16d	5800	5800
	16	56-16d	5800	5800
MSTI36	18	14-10dx1½	810	975
	16	16-10dx1½	930	1115
MSTI48	18	26-10dx1½	1545	1855
	16	28-10dx1½	1660	1990
MSTI60	18	38-10dx1½	2330	2800
	16	40-10dx1½	2455	2945
MSTI72	18	50-10dx1½	3065	3680
	16	52-10dx1½	3190	3830

Model No.	Ga	Dimensions W	Dimensions L	Fasteners (Total) Nails	Bolts Qty	Bolts Dia	Allowable Tension Loads: Nails Floor (100)	Nails (133)	Nails (160)	Bolts[5] Floor (100)	Bolts[5] (133)	Bolts[5] (160)
MST27	12	2 5/16	27	30-16d	4	½	2070	2760	2790	1295	1725	2070
MST37		2 5/16	37½	42-16d	6	½	2660	3815	3815	1825	2435	2920
MST48		2 5/16	48	46-16d	8	½	3345	4460	4460	2225	2970	3560
MST60	10	2 5/16	60	56-16d	10	½	4350	5800	5800	2670	3565	4275
MST72		2 5/16	72	56-16d	10	½	4350	5800	5800	2670	3565	4275
HST2	7	2½	21¼	—	6	⅝	—	—	—	3130	4175	5005
HST5		5	21¼	—	12	⅝	—	—	—	6385	8510	10210
HST3	3	3	25½	—	6	¾	—	—	—	4645	6195	7435
HST6		6	25½	—	12	¾	—	—	—	9350	12465	14955

1. Loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed. Floor loads may not be increased for other load durations.
2. 10dx1½" nails may be substituted where 16d sinkers are specified at 0.80 of the table loads.
3. 10d commons may be substituted where 16d sinkers are specified at 100% of table loads.
4. 16d sinkers (9 gauge x 3¼") or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
5. Allowable bolt loads are based on parallel-to-grain loading and these minimum member thicknesses: MST–2½"; HST2 and HST5–4", HST3 and HST6–4½".
6. PS strap design loads must be determined by the building designer for each installation. Bolts are installed both perpendicular and parallel-to-grain.
7. Use half of the nails at each member being connected to achieve the listed loads.

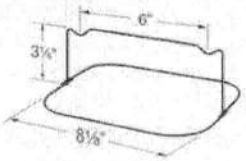
WRC WIRE REBAR CHAIRS

The WRC holds rebar in position during the concrete pour.

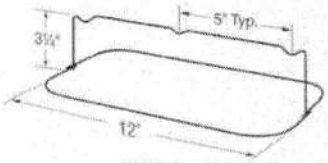
MATERIAL: 11 gauge

FINISH: None

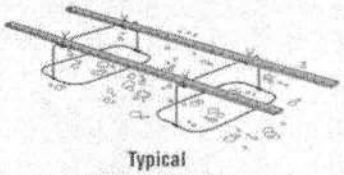
INSTALLATION: • Tie rebar to chair with wire twists prior to the concrete pour.



WRC2



WRC3



Typical WRC2 Installation

CNW COUPLER NUTS

All-thread rod is correctly installed when visible through CNW's "witness" holes. CNW's dimple provides a positive stop to allow even bolt threading top and bottom.

CNW's are tested and load-rated coupler nuts. They can be used for extending anchor bolts, for example, through floor framing. CNW's meet and exceed the capacity of corresponding ASTM A307, A36, SAE1018 and Grade 2 bolts and threaded rod. Contact factory for other coupler nut sizes.

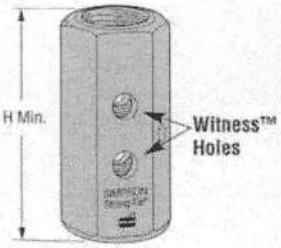
FINISH: Zinc Plated.

INSTALLATION:

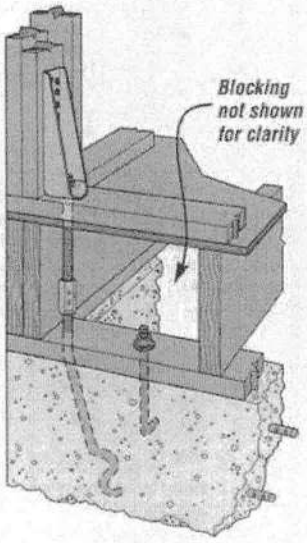
- Each rod must be threaded halfway through CNW.
- Each rod must meet at the center.
- Tighten the two rods against the central stop in the coupler nut.

CODES: See page 10 for Code Listing Key Chart.

Model No.	Rod Dia.	H Min	Avg. Ultimate Tension Capacity	Code Ref.
CNW½	0.50	1½	10750	160
CNW⅝	0.625	1⅞	18071	
CNW¾	0.75	2	32576	
CNW⅞	0.875	2⅞	55588	



CNW allows fast visual check for correct all thread rod installation



Typical CNW Rim Joist Installation

BP/LBP BEARING PLATES

The BP⅝S uses SDS¼ x 1½ screws to provide lateral resistance when sill holes are overdrilled (screws are provided). The shear capacity is 975 lbs. (100%) and 1300 lbs. (133%) for DFL.

Bearing Plates give greater bearing surface than standard cut washers, and help distribute the load at these critical connections.

MATERIAL: See table

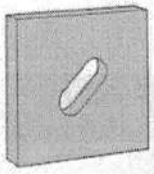
FINISH: LBP, LBPS & BP⅝S—galvanized; **BP—None. May be ordered HDG or ZMAX™; check factory. Refer to page 5 for corrosion information.**

INSTALLATION: See General Notes.

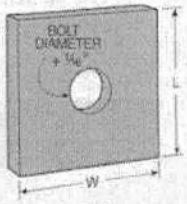
CODES: See page 10 for Code Listing Key Chart.

Model No.	Thickness	Dimensions W	Dimensions L	Bolt Dia.	Code Ref.
LBP½	9/64	2	2	½	180
LBP⅝	9/64	2	2	⅝	
LBPS½	9/64	3	3	½	
LBPS⅝	9/64	3	3	⅝	
BP½	3/16	2	2	½	97
BP⅝-2	3/16	2	2	⅝	190
BP⅝SKT	3 ga	4	2	⅝	170
BP⅝	¼	2½	2½	⅝	97
BP¾	5/16	2¾	2¾	¾	
BP⅞	5/16	3	3	⅞	
BP1	⅜	3½	3½	1	

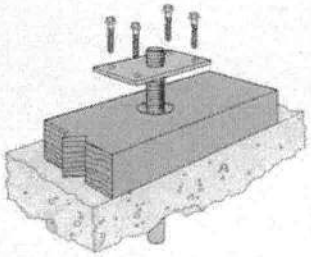
1. BP⅝SKT sold as a kit.



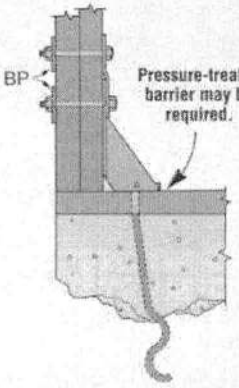
LBPS



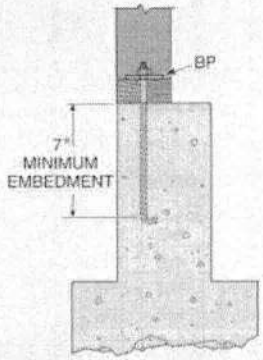
BP *(LBP similar)*



The BP⅝SKT is used when sill bolt holes are overdrilled



Typical BPs Installed with a Holdown and SSTB Anchor Bolt



Typical BP Installed with a Mudsill Anchor Bolt

Locking prongs inserts into concrete. The one-piece design assures maximum strength.

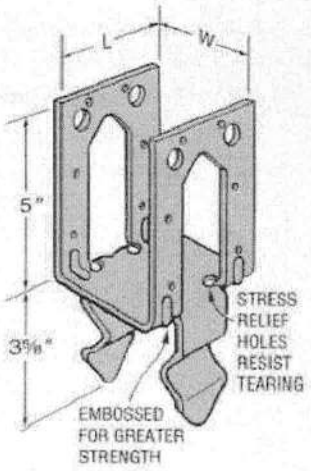
MATERIAL: 12 gauge. **FINISH**: Galvanized

INSTALLATION: • Use all specified fasteners. See General Notes.
- Holes are provided for installation with either 16d commons or ½" bolts for PB66 and PB66R; all other models use 16d commons only.
- A 2" minimum sidecover is required to obtain the full load.
- Not recommended for non-top-supported installations such as fences.

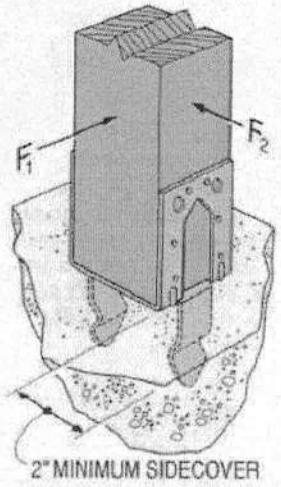
CODES: BOCA, ICBO, SBCCI NER-443; City of LA RR 25149; **Dade Co. 00-0512.11 (PB44).**

Model No.	Dimensions W	Dimensions L	Uplift Avg Ult	Allowable Loads 12-16d Nails (133 & 160) Uplift	F_1	F_2	2-½MB Uplift (133 & 160)
PB44	3⅜	3½	4267	1365	765	1325	—
PB44R	4	3½	4267	1365	765	1325	—
PB46	5½	3½	4267	1365	765	1325	—
PB46R	6	3½	4267	1365	765	1325	—
PB66	5½	5½	5143	1640	765	1325	1640
PB66R	6	5½	5143	1640	765	1325	1640

1. Allowable loads have been increased 33% and 60% for earthquake or wind loading, with no further increase allowed.



PB



Typical PB Installation

AC/LPC/LCE POST CAPS

The LCE4's universal design provides high capacity while eliminating the need for rights and lefts.

The AC MAX design allows for higher load capacity to match comparable post bases.

LPC—Adjustable design allows greater connection versatility.

MATERIAL: LCE4—20 ga; AC, ACE, LPC4—18 ga; LPC6—16 ga

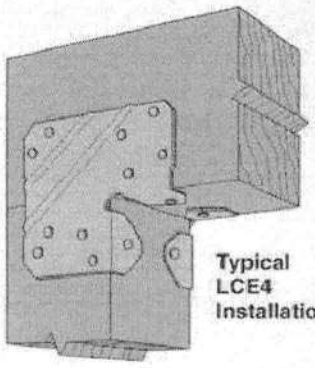
FINISH: Galvanized. Some products available with Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION: • Use all specified fasteners. See General Notes.
- Install all models in pairs. LPC—2½" beams may be used if 10dx1½" nails are substituted for 10d commons.

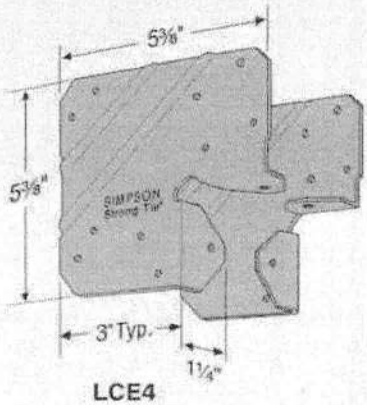
CODES: BOCA, ICBO, SBCCI NER-421, NER-443, NER-469; City of L.A. RR 25076; Dade County, FL 99-0623.04 (LPC) and Dade County, FL 99-0713.05 (AC, ACE).

Model No.	Dimensions W	Dimensions L	Total No. Fasteners Beam	Total No. Fasteners Post	Uplift Avg Ult	Allowable Loads (133 & 160)[1] Uplift	Allowable Loads (133 & 160)[1] Lateral
AC4 MIN	3⅜	6½	12-16d	8-16d	4467	1430	715
AC4 MAX	3⅜	6½	14-16d	14-16d	10000	2500	1070
AC4R MIN	4	7	12-16d	8-16d	4467	1430	715
AC4R MAX	4	7	14-16d	14-16d	10000	2500	1070
ACE4 MIN	—	4½	8-16d	6-16d	4215	1070	715
ACE4 MAX	—	4½	10-16d	10-16d	6238	1785	1070
AC6 MIN	5½	8½	12-16d	8-16d	4467	1430	715
AC6 MAX	5½	8½	14-16d	14-16d	10000	2500	1070
AC6R MIN	6	9	12-16d	8-16d	4467	1430	715
AC6R MAX	6	9	14-16d	14-16d	10000	2500	1070
ACE6 MIN	—	6½	8-16d	6-16d	4537	1070	715
ACE6 MAX	—	6½	10-16d	10-16d	6432	1785	1070
LPC4	3⅜	3½	8-10d	8-10d	2333	760	325
LPC6	5⅜	5½	8-10d	8-10d	2817	915	490
LCE4	—	5⅜	14-16d	10-16d	5518	1800	1425

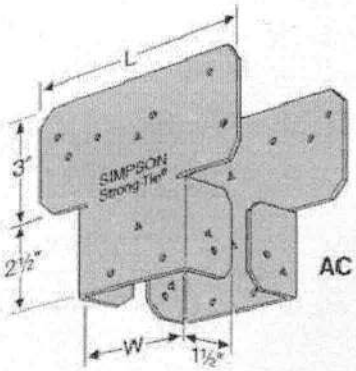
1. Allowable loads have been increased 33% and 60% for earthquake or wind loading with no further increase allowed; reduce for other load durations according to the code.
2. Loads apply only when used in pairs.
3. LPC lateral load is in the direction of the beam's axis.
4. MIN nailing quantity and load values – fill all round holes; MAX nailing quantities and load values – fill round and triangle holes.



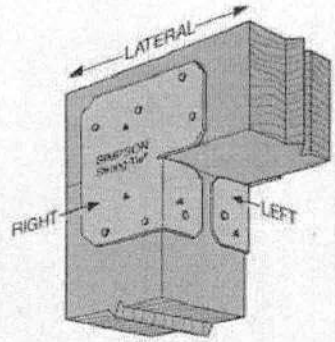
Typical LCE4 Installation



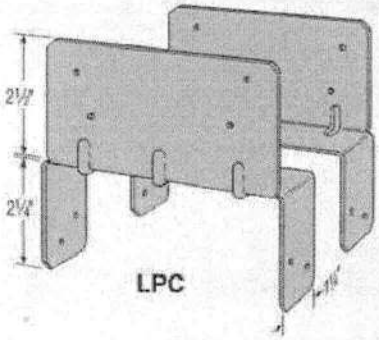
LCE4



AC



Typical ACE Installation



LPC

Caps & Bases

AB/ABA/ABE/ABU/PBS ADJUSTABLE AND STANDOFF POST BASES

SIMPSON Strong-Tie CONNECTORS

The AB is a fully-adjustable post base which offers moisture protection and finished hardware appearance.

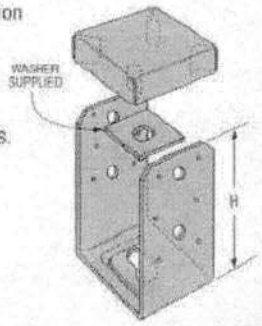
Post Bases provide tested capacity. They feature 1" standoff height above concrete floors, code-required when supporting permanent structures that are exposed to the weather or water splash, or in basements. They reduce the potential for decay at post and column ends.

MATERIAL: AB—12 ga plates; 16 ga base cover; all others—see table.

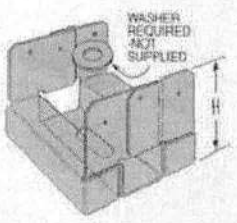
FINISH: Galvanized. Some products available in Z-MAX; see Corrosion-Resistance, page 5.

INSTALLATION:
- Use all specified fasteners.See General Notes.
- Not recommended for non-top-supported installations such as fences.
- PBS embed into wet concrete up to the bottom of the 1" standoff base plate. A 2" minimum side cover is required to obtain the full load for PBS. Holes in the bottom of the PBS straps allow for free concrete flow.
- AB—Post nail holes are sized for 10d commons. Rectangular adjustment plate assumes ½" dia anchorage. Supplied as shown; position the post, secure the easy-access nut, then bend up the fourth side.
- AB, ABA, ABE and ABU—for pre-pour installed anchors. For epoxy or wedge anchors, select and install according to anchor manufacturer's recommendations; anchor diameter shown in table. Install required washer, which is not included for ABAs.
- See Simpson Anchor Systems for tested, load-rated anchors.

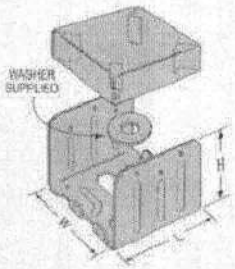
CODES: BOCA, ICBO, SBCCI NER-393, NER-422, NER-432, NER-469, NER-499; ICBO 5670; City of L.A. RR 24818, RR 25064, 25074, 25158; Dade Co FL. 99-0713.05 (ABA, ABE), 00-0512.11 (ABU).



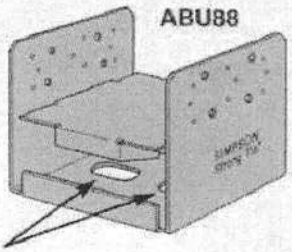
ABU44 (other sizes similar)



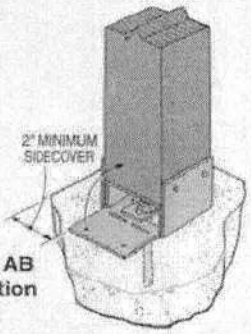
ABA44 (other sizes similar)
U.S. Patent 5,333,435



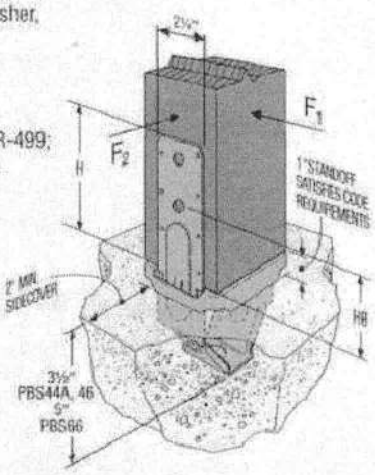
ABE44
ABE46,46R,66 and 66R supplied with rectangular washer.



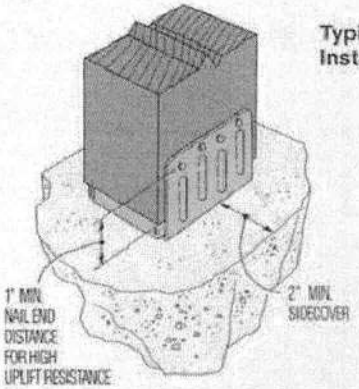
ABU88
2 load transfer plates supplied



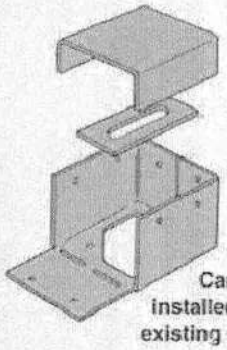
Typical AB Installation



Typical PBS44A Installation



Typical ABE46R Installation for rough lumber (ABE similar)



AB
Can be installed on existing slab

Model No.	Dimensions W	Dimensions L	Allowable Downloads (100)
ABA44	3⅜	3⅜	4065
ABA44R	4	4⅛	4065
ABA46	3⅜	5⅛	4165
ABA46R	4	6	4165
ABA66	5½	5⅜	5335
ABA66R	6	6	5335

1. Loads may not be increased for short-term loading.

Model No.	Nominal Post Size	Material Base (Ga)	Material Strap (Ga)	Dimensions W	L	H	HB	Fasteners Anch. Dia	Post Nails	Post Bolts Qty	Post Bolts Dia	Uplift Avg Ult	Allowable Loads Uplift (133) Nails	Uplift (133) Bolts	Uplift (160) Nails	Uplift (160) Bolts	F_1 (133 & 160) Nails	F_1 (133 & 160) Bolts	F_2 (133 & 160) Nails	F_2 (133 & 160) Bolts	Down (100)
ABA44	4x4	16	16	3⅜	3⅛	3⅞	—	½	6-10d	—	—	2120	555	—	555	—	—	—	—	—	6000
ABE44	4x4	16	16	3⅜	3½	2¾	—	½	6-10d	—	—	1893	520	—	520	—	—	—	—	—	6665
ABU44	4x4	16	12	3⅜	3	5⅛	1¼	⅝	12-16d	2	½	7833	2200	1800	2200	2160	—	—	—	—	6665
PBS44A	4x4	12	14	3⅜	2¼	6¼	3⅜	—	14-16d	2	½	7733	2400	2400	2400	2400	1165	230	885	885	6665
ABA44R	RGH 4x4	16	16	4⅛	3⅛	2⅞	—	½	6-10d	—	—	2120	555	—	555	—	—	—	—	—	8000
ABE44R	RGH 4x4	16	16	4	3½	2⅝	—	½	6-10d	—	—	1893	400	—	400	—	—	—	—	—	6665
ABE46	4x6	12	16	3⅜	5⅞	4⅛	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
PBS46	4x6	12	14	3⅜	2¼	6⅜	3⅜	—	14-16d	2	½	7733	2400	2400	2400	2400	1165	360	885	885	9335
ABA46	4x6	14	14	3⅜	5⅜	3⅛	—	⅝	8-16d	—	—	2967	700	—	700	—	—	—	—	—	9435
ABU46	4x6	12	12	3⅜	5	7	2⅜	⅝	12-16d	2	½	8633	2255	2300	2300	2300	—	—	—	—	10335
ABE46R	RGH 4x6	12	16	4⅛	5⅞	3⅛	—	⅝	8-16d	—	—	5167	810	—	810	—	—	—	—	—	7335
ABA46R	RGH 4x6	14	14	4⅛	5⅜	2⅞	—	⅝	8-16d	—	—	2967	935	—	935	—	—	—	—	—	12000
PBS66	6x6	12	12	5½	2¼	6½	3⅛	—	14-16d	2	½	13100	2630	3560	3160	4000	1865	570	1700	1700	9335
ABA66	6x6	14	14	5½	5¼	3⅛	—	⅝	8-16d	—	—	3050	720	—	720	—	—	—	—	—	10665
ABE66	6x6	12	14	5½	5⅛	3⅛	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU66	6x6	12	10	5½	5	6⅛	1¼	⅝	12-16d	2	½	8900	2300	2300	2300	2300	—	—	—	—	12000
ABA66R	RGH 6x6	14	14	6	5⅜	2⅛	—	⅝	8-16d	—	—	3050	985	—	985	—	—	—	—	—	12665
ABE66R	RGH 6x6	12	14	6⅛	5⅛	2⅞	—	⅝	8-16d	—	—	4833	900	—	900	—	—	—	—	—	12000
ABU88[4]	8x8	12	14	7½	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335
ABU88R[4]	RGH 8x8	12	14	8	7	7	—	2-⅝	18-16d	—	—	12893	2320	—	2320	—	—	—	—	—	24335

1. Uplift and lateral loads have been increased 33% and 60% for earthquake or wind loading; no further increase allowed. Reduce by 33% and 60% for normal loading.
2. Downloads may not be increased for short-term loading.
3. Specifier to design concrete for shear capacity.
4. ABU88 and ABU88R may be installed with 8-SDS¼X3 wood screws for the same table load.

Caps & Bases

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:ITYR8228Z0225105744

Truss Fabricator:	Anderson Truss Company
Job Identification:	10-016--Fill in later dicks -- , **
Truss Count:	30
Model Code:	Florida Building Code 2007 and 2009 Supplement
Truss Criteria:	FBC2007Res/TPI-2002(STD);CUSTOM/TPI-2002(STD)
Engineering Software:	Alpine Software,Version 9.02.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - 55.0 PSF @ 1.00 Duration
	Wind - 110 MPH ASCE 7-05 -Closed

Notes:

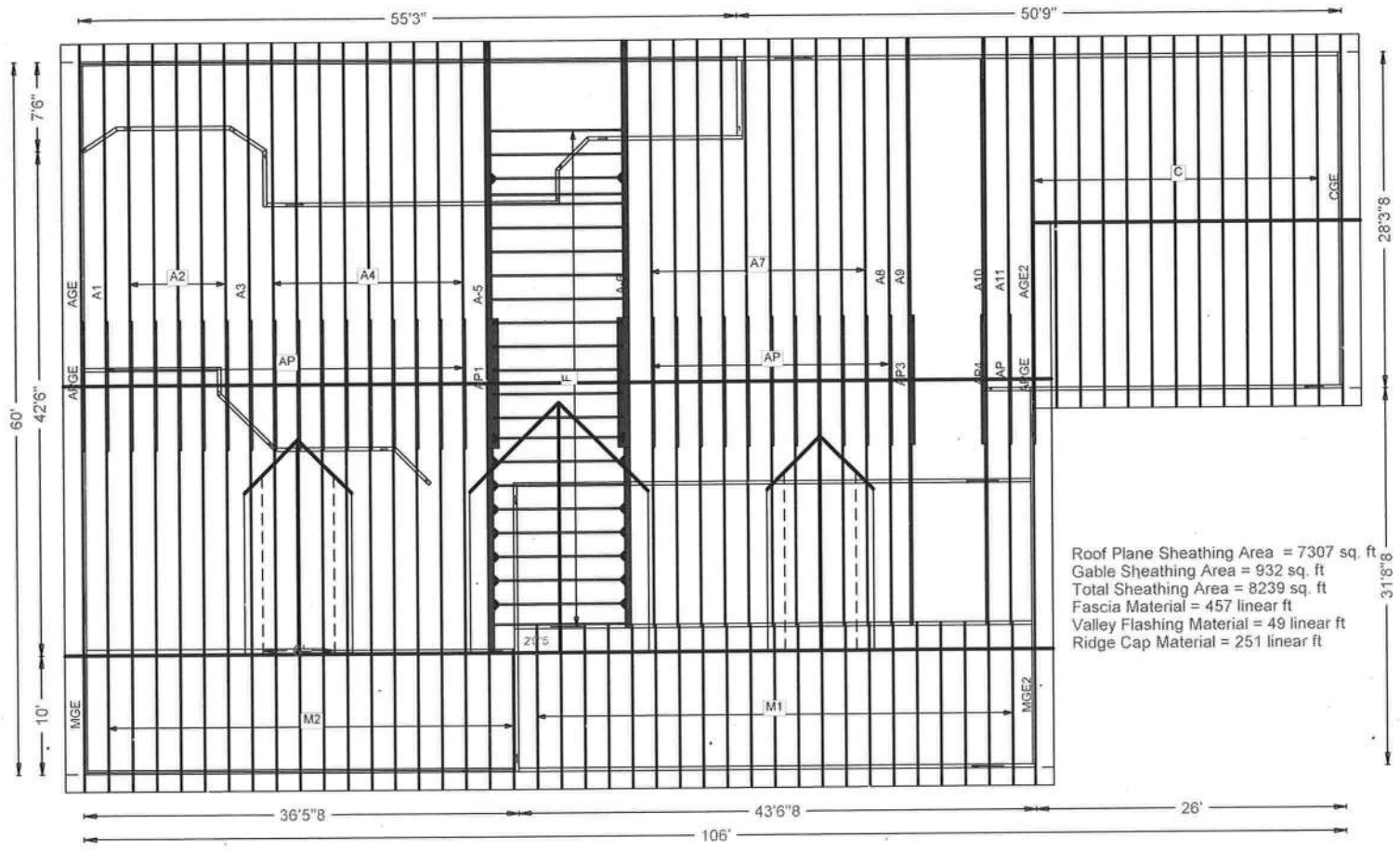
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: CNNAILSP-A1101505-GBLLETIN-A1103005-BRCLBSUB-PB120-

Seal Date: 01/25/2010

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

#	Ref Description	Drawing#	Date
1	44942--A4	10025014	01/25/10
2	44943--A3	10025006	01/25/10
3	44944--A2	10025001	01/25/10
4	44945--A1	10025012	01/25/10
5	44946--A7	10025016	01/25/10
6	44947--A8	10025015	01/25/10
7	44948--M2	10025008	01/25/10
8	44949--M1	10025009	01/25/10
9	44950--C	10025005	01/25/10
10	44951--AP	10025013	01/25/10
11	44952--A9	10025018	01/25/10
12	44953--A10	10025021	01/25/10
13	44954--A11	10025004	01/25/10
14	44955--MGE2	10025011	01/25/10
15	44956--MGE	10025010	01/25/10
16	44957--AGE	10025002	01/25/10
17	44958--CGE	10025003	01/25/10
18	44959--AP3	10025020	01/25/10
19	44960--AP4	10025022	01/25/10
20	44961--A-6	10025009	01/25/10
21	44962--A-5	10025003	01/25/10
22	44963--F	10025007	01/25/10
23	44964--AP2	10025002	01/25/10
24	44965--AP1	10025019	01/25/10
25	44966--APGE	10025017	01/25/10
26	44967--AGE2	10025004	01/25/10
27	44968--DOR	10025005	01/25/10
28	44969--DORGE	10025006	01/25/10
29	44970--DOR1	10025007	01/25/10
30	44971--DOR1GE	10025008	01/25/10



CHRIS DICKS

JOB DESCRIPTION:: Fill in later
/: dicks

JOB NO:
10-016

PAGE NO:
1 OF 1

(10-016--Fill in later dicks -- , ** - A4)

Top chord 2x4 SP #2 Dense :T2 2x6 SP SS:
:T4, T5 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2 2x6 SP #1 Dense:
:B6, B7 2x6 SP SS:
Webs 2x4 SP #3
:W3, W4, W5, W7, W9, W32 2x4 SP #2 Dense:

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC - From	62 plf at	-2.00 to	62 plf at	0.00
TC - From	62 plf at	0.00 to	62 plf at	11.11
TC - From	62 plf at	11.11 to	62 plf at	16.31
TC - From	85 plf at	16.31 to	85 plf at	20.49
TC - From	62 plf at	20.49 to	62 plf at	21.85
TC - From	62 plf at	21.85 to	62 plf at	32.78
TC - From	62 plf at	32.78 to	62 plf at	34.13
TC - From	85 plf at	34.13 to	85 plf at	35.16
TC - From	190 plf at	35.16 to	212 plf at	38.31
TC - From	190 plf at	38.31 to	214 plf at	41.72
TC - From	214 plf at	41.72 to	272 plf at	50.00
PLT- From	20 plf at	21.00 to	20 plf at	33.63
BC - From	4 plf at	-2.00 to	4 plf at	0.00
BC - From	20 plf at	0.00 to	20 plf at	16.31
BC - From	120 plf at	16.31 to	120 plf at	38.31
BC - From	20 plf at	38.31 to	20 plf at	50.00

TC - 105 lb Conc. Load at 50.00
BC - 118 lb Conc. Load at 16.31 , 38.31

Wind reactions based on MWFRS pressures.

(A) 2x8 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Brg blocks:0.131"x3" nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	12.000'	1	12"	6	Rigid Surface
3	49.707'	1	12"	4	Rigid Surface

Brg block to be same size and species as bottom chord.
Refer to drawing CNNAILSP0109 for more information.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.31" due to live load and 0.27" due to dead load.

(B) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(D) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

(C) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

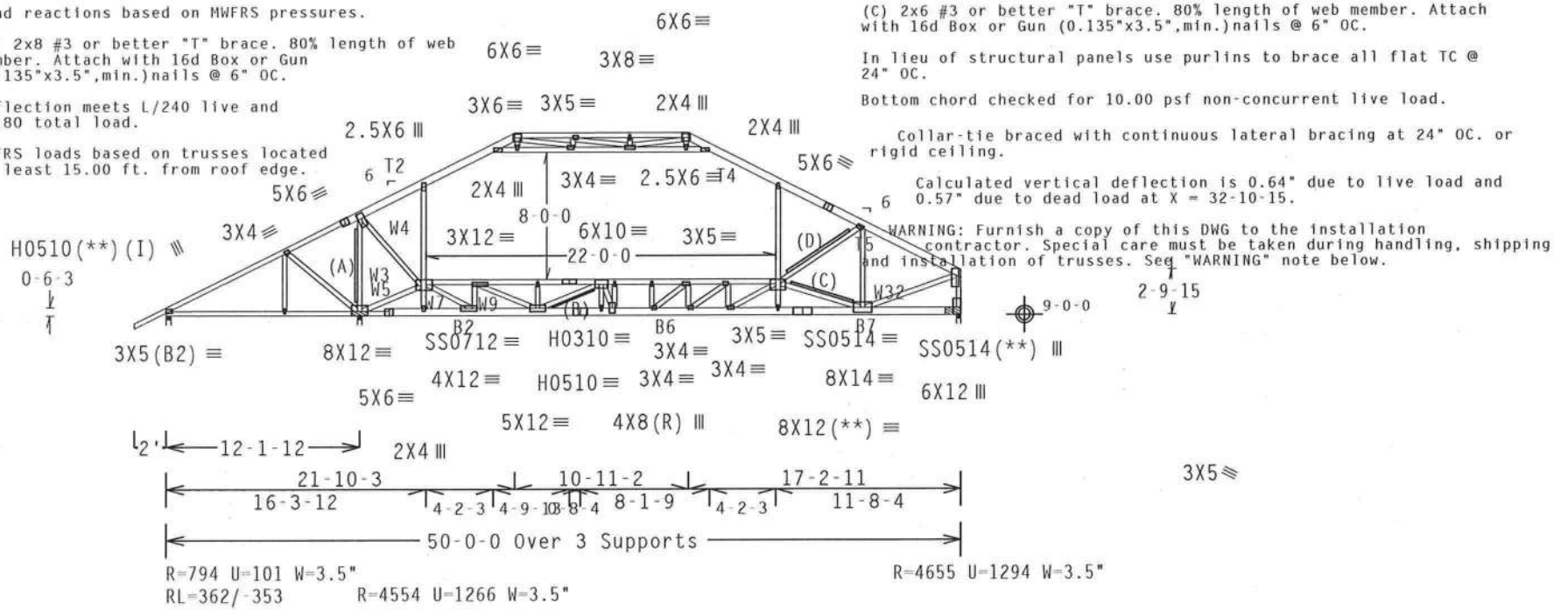
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Calculated vertical deflection is 0.64" due to live load and 0.57" due to dead load at X = 32-10-15.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 1.5X4 Except As Shown.
PLT TYP. 20 Gauge HS,18 Gauge HS, Wave
Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/0(0)
9.02.00
QTY:9
FL/-/4/-/-/R/-
Scale =.09375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES R. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44942
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025014
BC LL	0.0 PSF		HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN-	76142
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A3)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3
:W5, W8, W10, W18 2x4 SP #2 Dense:

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

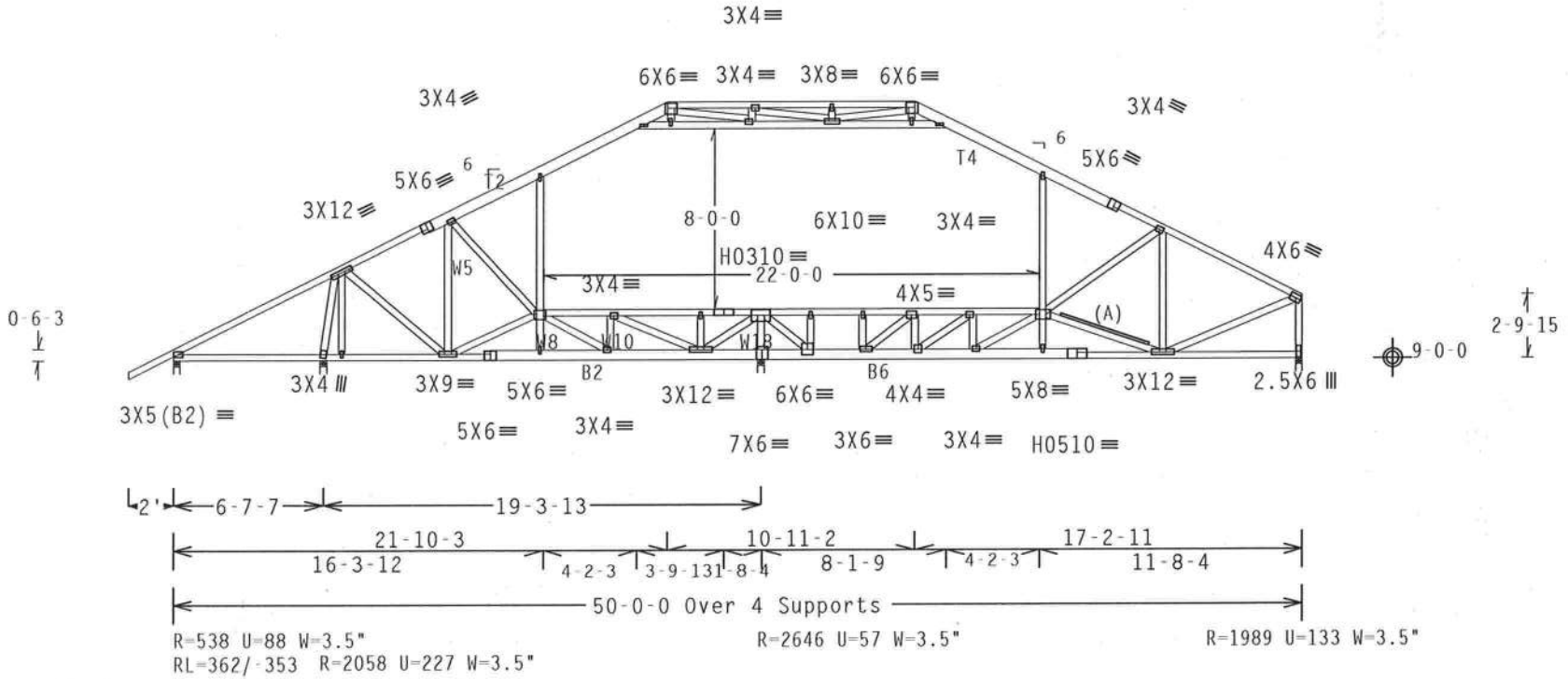
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00.6[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan[illegible]

TC LL	20.0 PSF	REF	R8228- 44943
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025006
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	76151
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A2)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2 2x6 SP #2: :B6 2x6 SP SS:
Webs 2x4 SP #3
:W5, W8, W10, W18, W29, W33 2x4 SP #2 Dense:

Wind reactions based on MWFRS pressures.

Roof overhang supports 2.00 psf soffit load.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	67 PLF	at -2.00	to	67 PLF	at 0.00
TC - From	62 PLF	at 0.00	to	62 PLF	at 16.31
TC - From	85 PLF	at 16.31	to	85 PLF	at 20.49
TC - From	62 PLF	at 20.49	to	62 PLF	at 21.00
TC - From	82 PLF	at 21.00	to	82 PLF	at 33.63
TC - From	62 PLF	at 33.63	to	62 PLF	at 34.13
TC - From	85 PLF	at 34.13	to	85 PLF	at 38.31
TC - From	62 PLF	at 38.31	to	62 PLF	at 50.00
BC - From	20 PLF	at 0.00	to	20 PLF	at 16.31
BC - From	120 PLF	at 16.31	to	120 PLF	at 38.31
BC - From	20 PLF	at 38.31	to	20 PLF	at 50.00

BC - 118 LB Conc. Load at 16.31
BC - 118 LB Conc. Load at 38.31

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

(A) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

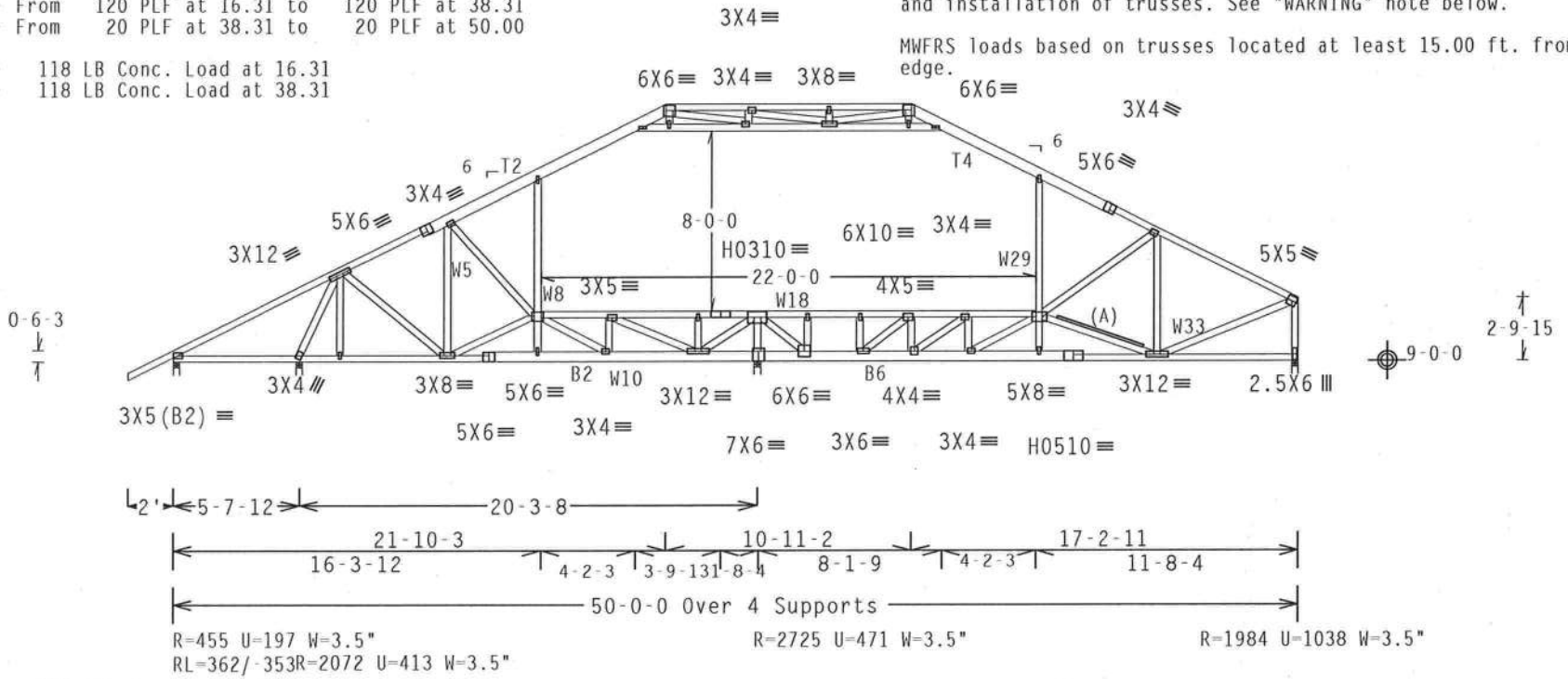
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00.0 QTY:5 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44944
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025001
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	76156
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A1)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3
:W5, W8, W10, W18 2x4 SP #2 Dense:

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

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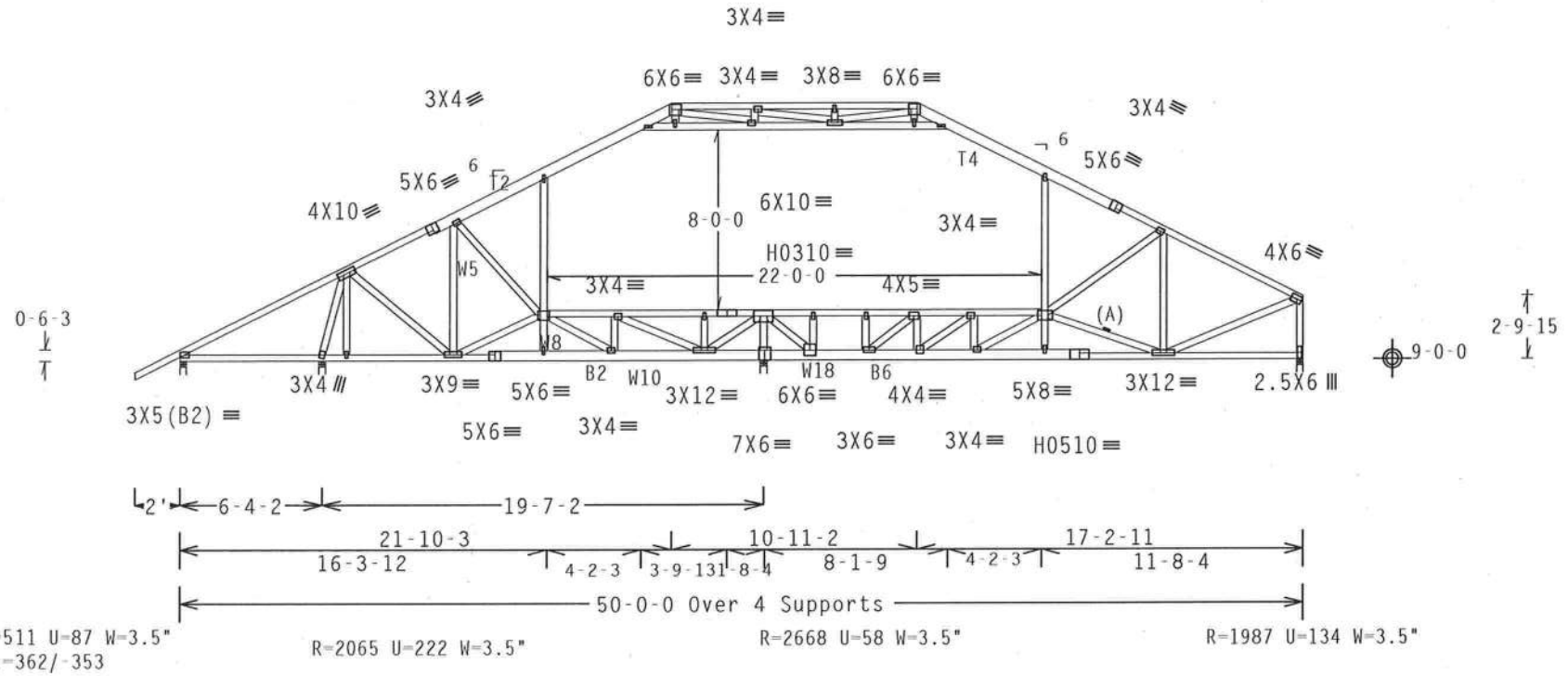

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00.[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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JAMES F. COLLINS, JR. VICE PRES. No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 29,10

TC LL	20.0 PSF	REF R8228- 44945
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025012
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76084
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A7)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
:T5 2x6 SP #2:
Bot chord 2x4 SP #2 Dense :B2, B7 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W5, W8, W10 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Negative reaction(s) of -630# MAX. (See below) from a non-wind
** load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

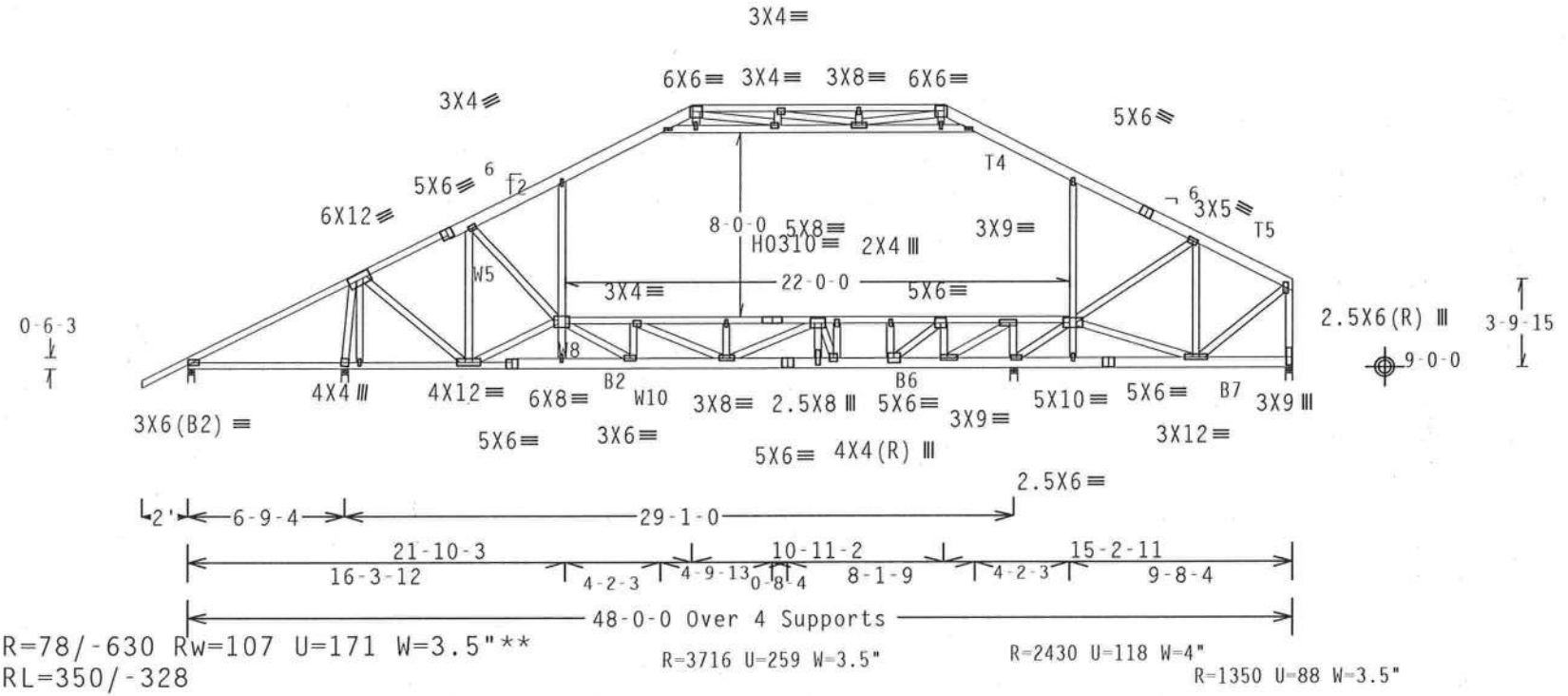
Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=78/-630 Rw=107 U=171 W=3.5"**
RL=350/-328

R=3716 U=259 W=3.5"

R=2430 U=118 W=4"

R=1350 U=88 W=3.5"

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00 QTY:10 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES P. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 29 '10

TC LL	20.0 PSF	REF R8228- 44946
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025016
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76171
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A8)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2, B7 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W4, W7, W9 2x4 SP #2 Dense:

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Wind reactions based on MWFRS pressures.

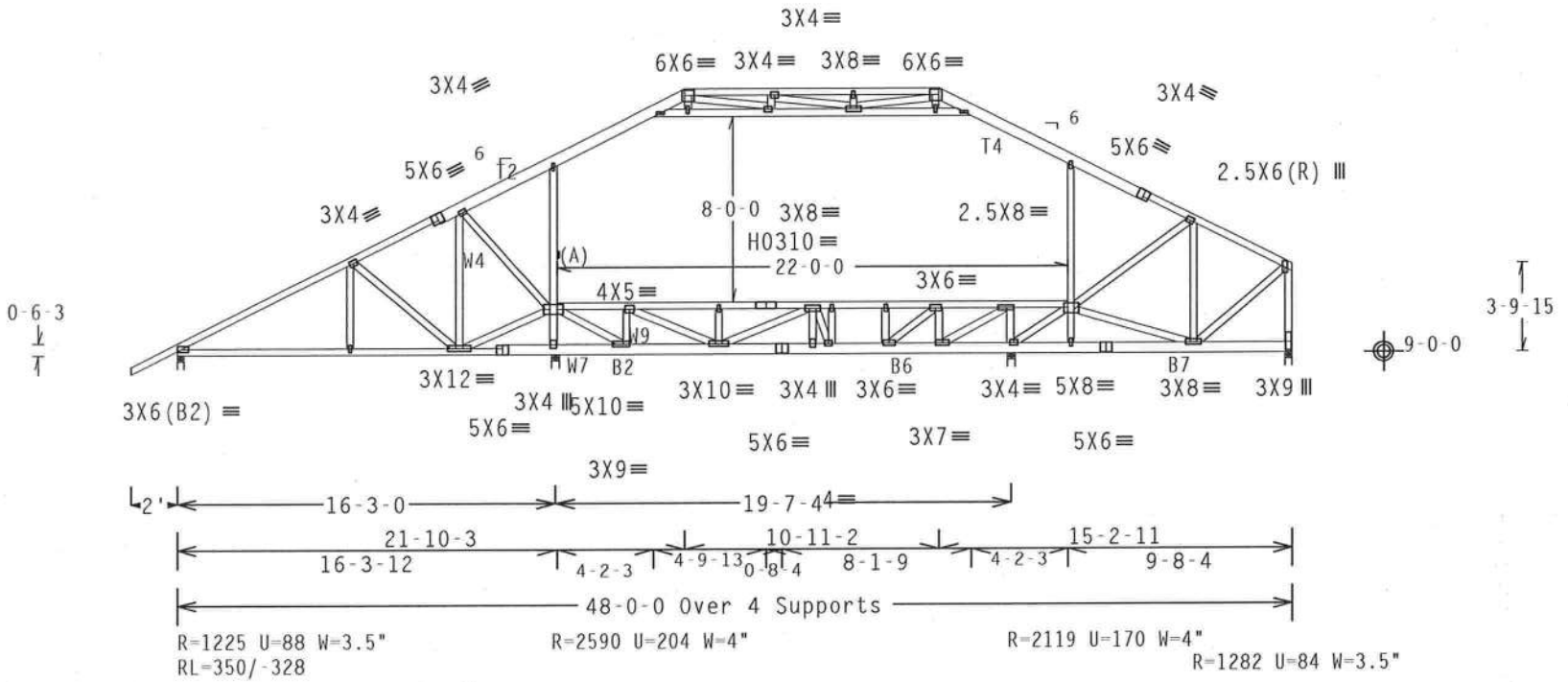
Right end vertical not exposed to wind pressure.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00.06 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan

TC LL	20.0 PSF	REF	R8228- 44947
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025015
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	76184
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - M2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

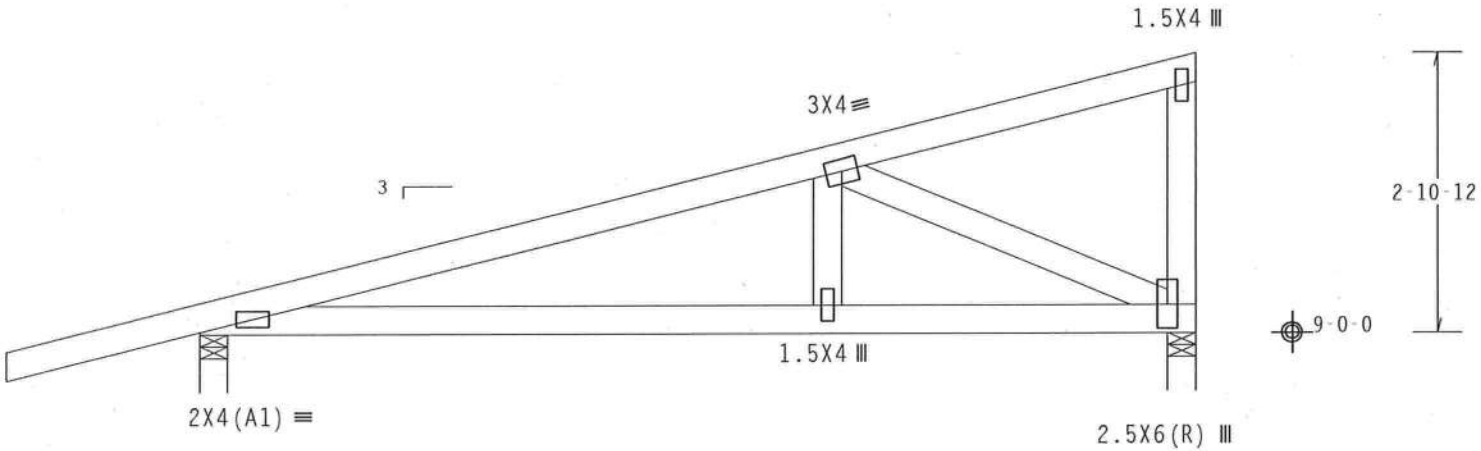
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave | FT/RT=20%(0%)/0(0) | 9.02.00.06 | QTY:18 FL/-/4/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES K. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 29 '10

TC LL	20.0 PSF	REF R8228- 44948
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025008
BC LL	0.0 PSF	HC-ENG JB/AP *
TOT.LD.	40.0 PSF	SEQN- 76088
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - M1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

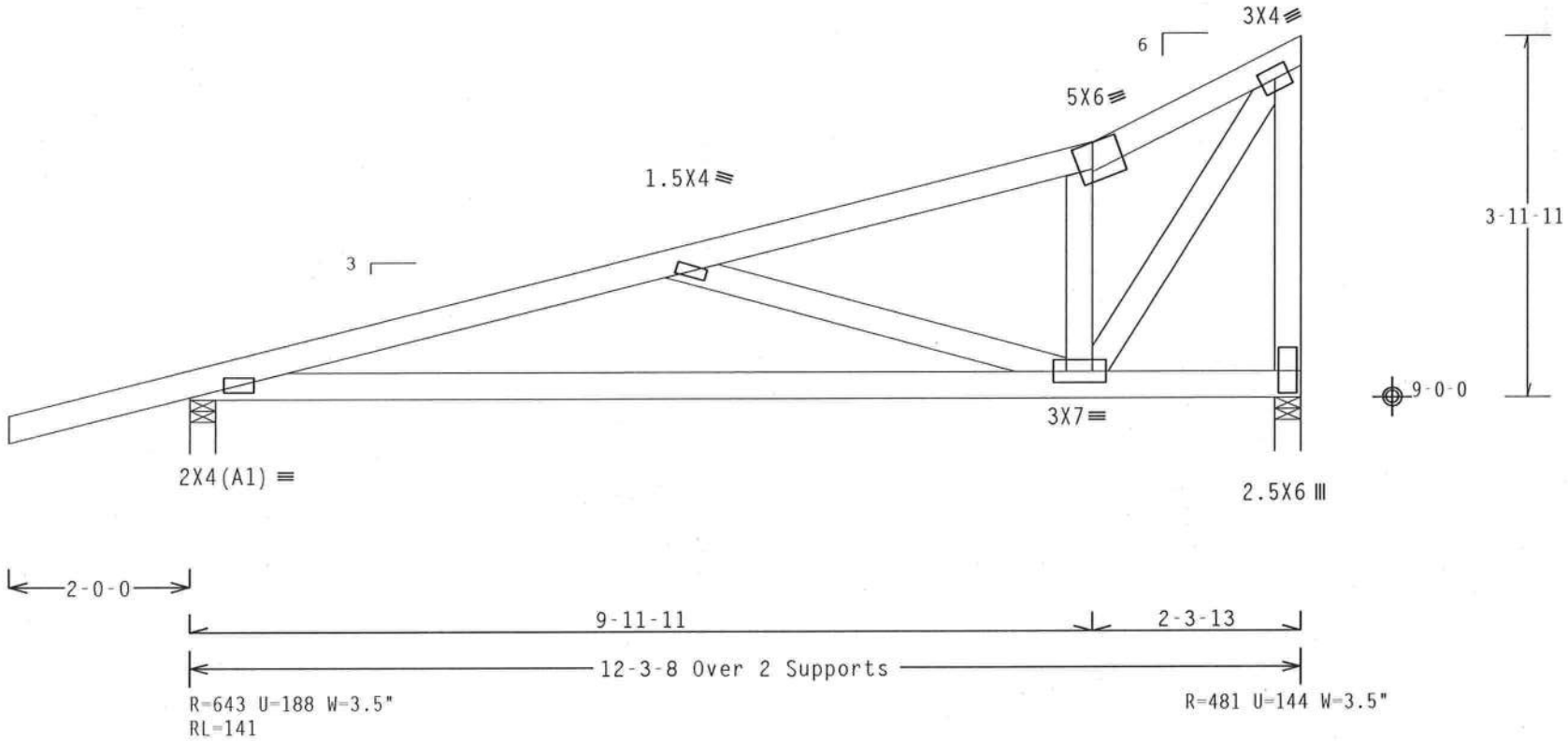
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00 QTY:21 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLLINS JR LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF R8228- 44949
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025009
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76092
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - C)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense :B2 2x6 SP #1 Dense:
Webs 2x4 SP #3

Special loads
(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC - From	62 plf at	-2.00 to	62 plf at	14.15
TC - From	62 plf at	14.15 to	62 plf at	30.29
BC - From	4 plf at	-2.00 to	4 plf at	0.00
BC - From	20 plf at	0.00 to	20 plf at	9.68
BC - From	70 plf at	9.68 to	70 plf at	12.08
BC - From	110 plf at	12.08 to	110 plf at	16.23
BC - From	70 plf at	16.23 to	70 plf at	18.12
BC - From	20 plf at	18.12 to	20 plf at	28.29
BC - From	4 plf at	28.29 to	4 plf at	30.29

Wind reactions based on MWFRS pressures.

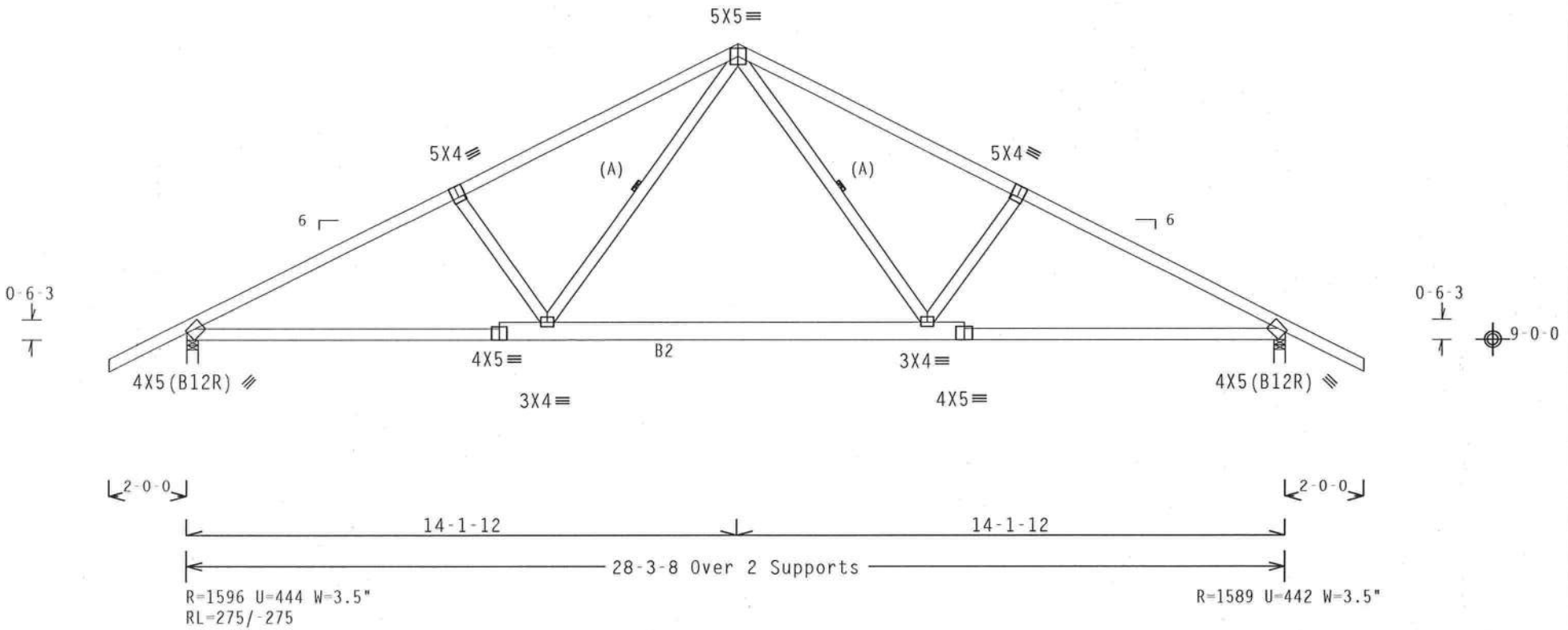
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave | FT/RT=20%(0%)/0(0) | 9.02.00.16 | QTY:13 FL/-/4/-/-/R/- | Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS, JR. LICENSED No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44950
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025005
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	76346
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - AP)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.81 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

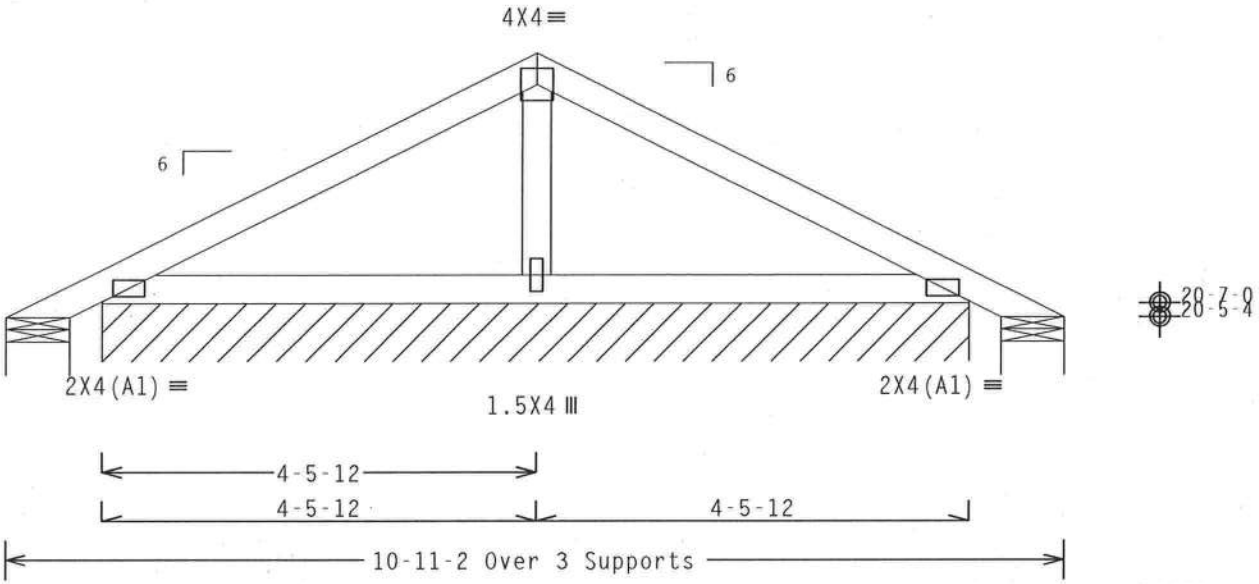
Wind reactions based on MWFRS pressures.

Refer to DWG PB1200109 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 62 plf at 0.00 to 62 plf at 5.46
TC - From 62 plf at 5.46 to 62 plf at 10.93
BC - From 4 plf at 0.00 to 4 plf at 10.93

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load.



R=-48 Rw=69 U=72 W=7.826"
RL=78/-78

R=87 PLF U=54 PLF W=8-11-8

R=-48 Rw=42 U=38 W=7.827"

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/0(0)

PLT TYP. Wave

9.02.00.161 IRW

QTY:28 FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 25 10

TC LL	20.0 PSF	REF R8228- 44951
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025013
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 75993
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2, B7 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W4, W7, W9 2x4 SP #2 Dense:

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

(A) continuous lateral bracing, equally spaced on member.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 60.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 7.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
4" o.c. spacing of nails perpendicular and parallel to grain required in area over bearings greater than 4"

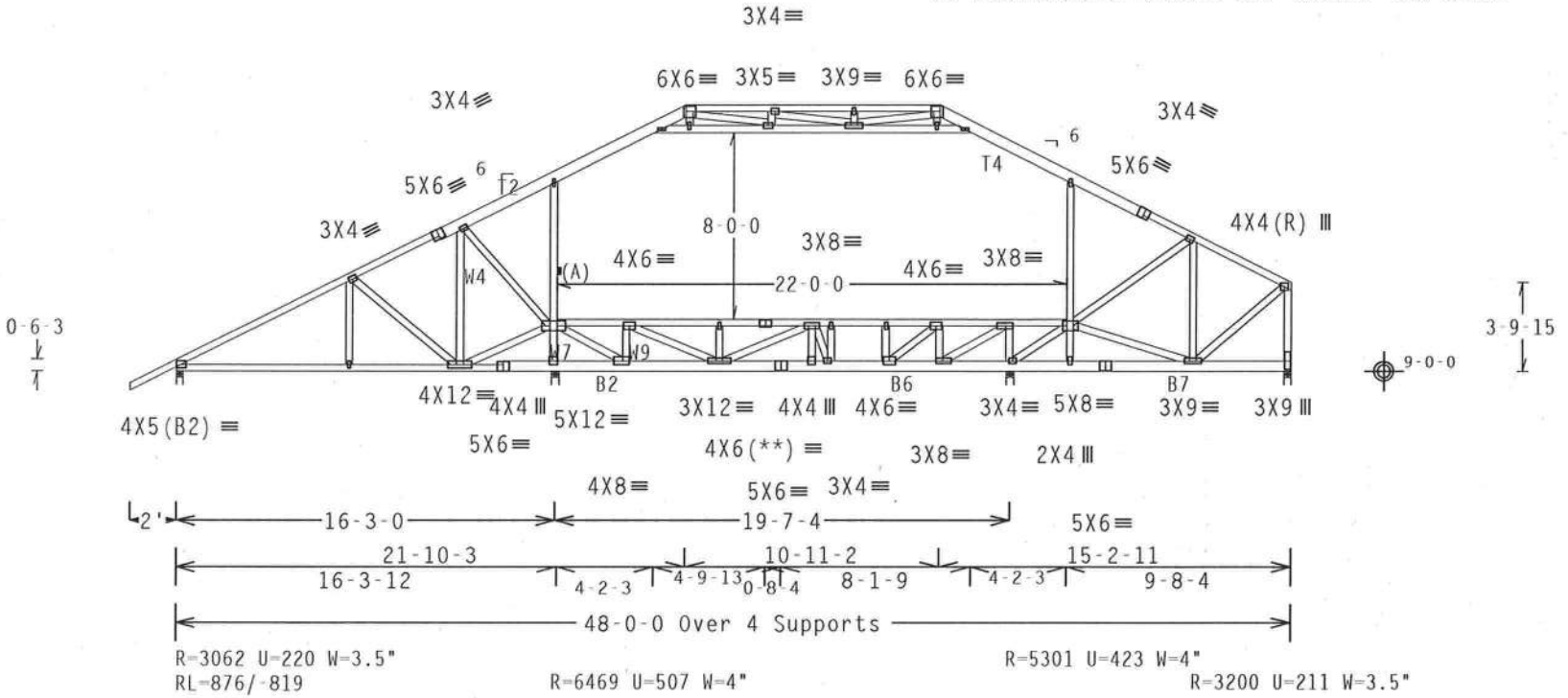
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 5.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF R8228- 44952
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025018
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76192
DUR.FAC.	1.25	
SPACING	60.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A10)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2, B7 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W4, W7, W9 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Trusses to be spaced at 60.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 7.75" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

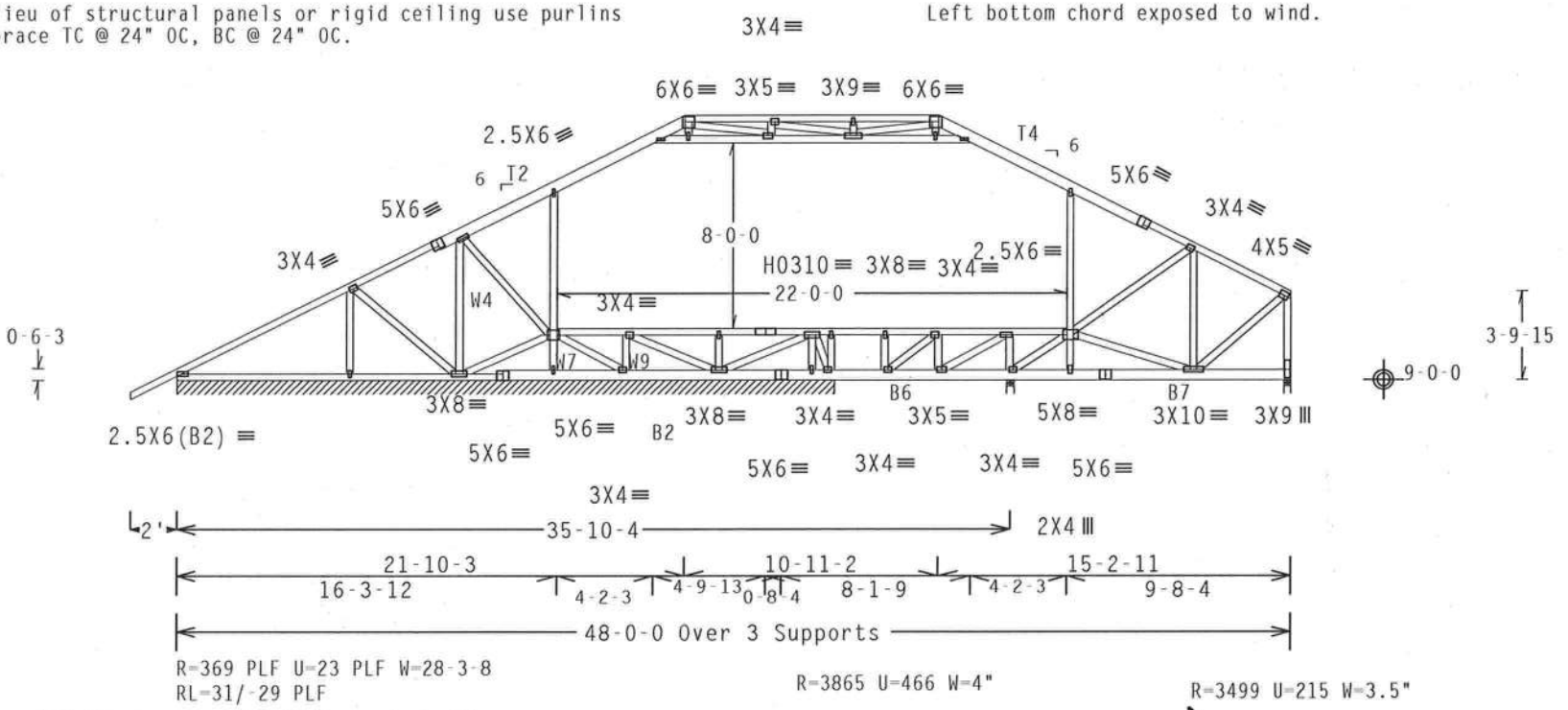
110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 5.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Left bottom chord exposed to wind.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44953
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025021
BC LL	0.0 PSF		HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN-	76200
DUR.FAC.	1.25		
SPACING	60.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A11)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2, B7 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W4, W7, W9, W13 2x4 SP #2 Dense:
:Lt Wedge 2x4 SP #3:

Negative reaction(s) of -229# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Roof overhang supports 2.00 psf soffit load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

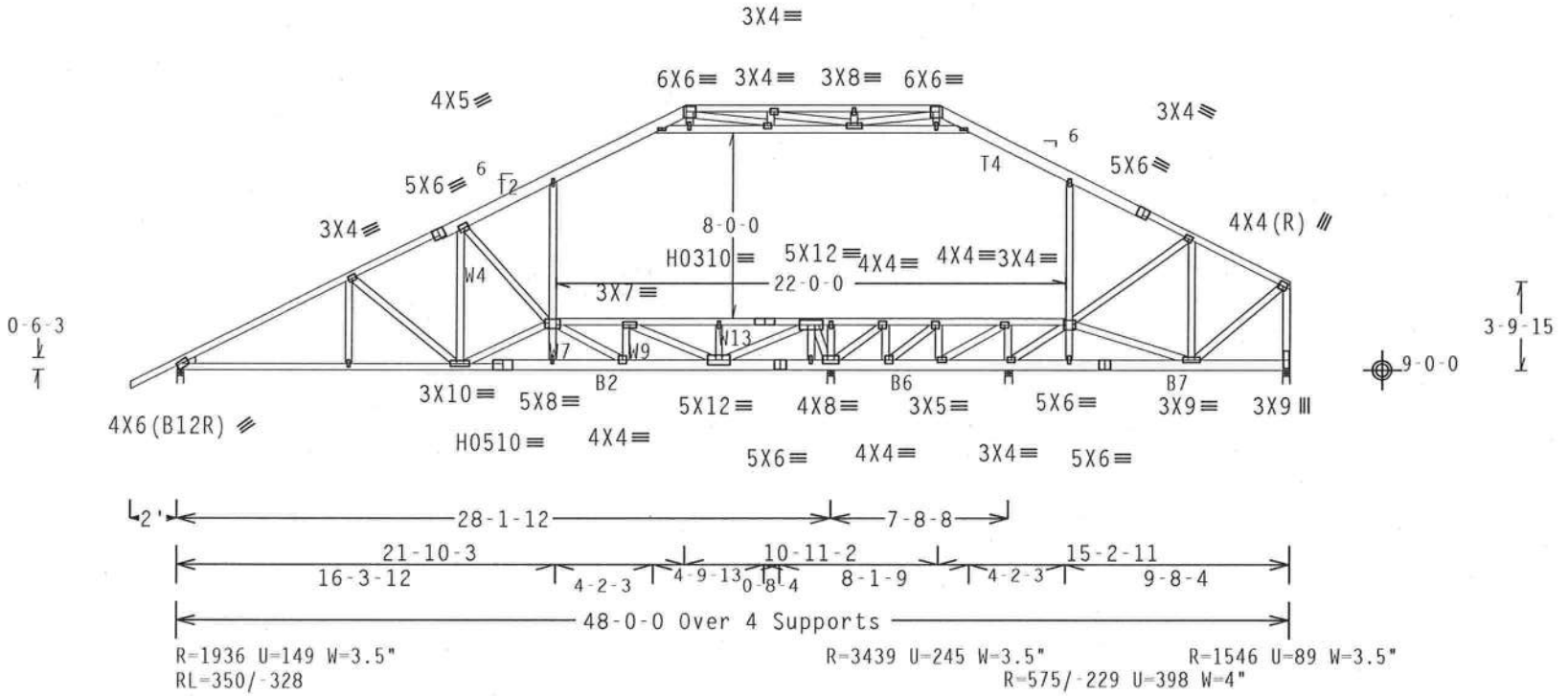
Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

Deflection meets L/240 live and L/180 total load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44954
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025004
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	76205
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - MGE2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

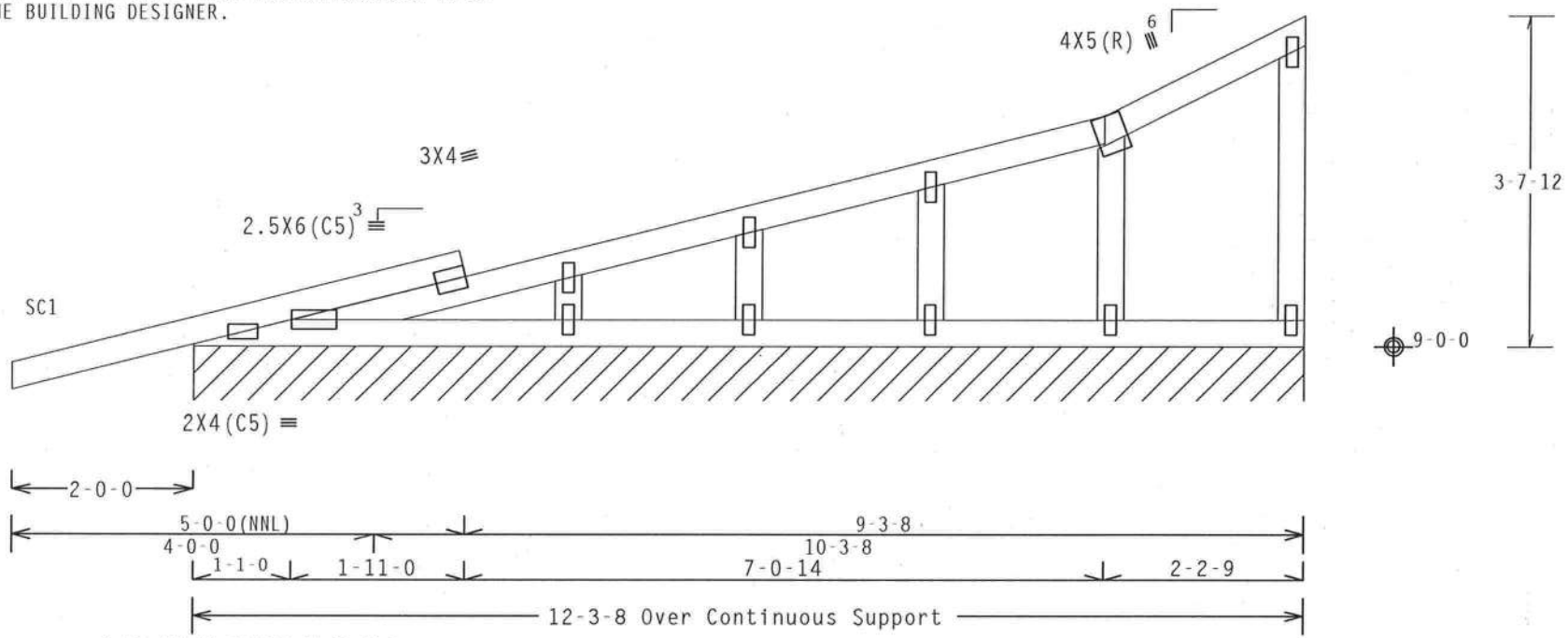
Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Left and right bottom chords exposed to wind.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00.0010 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES P. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 2

TC LL	20.0 PSF	REF R8228- 44955
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025011
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76043
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - MGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

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110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

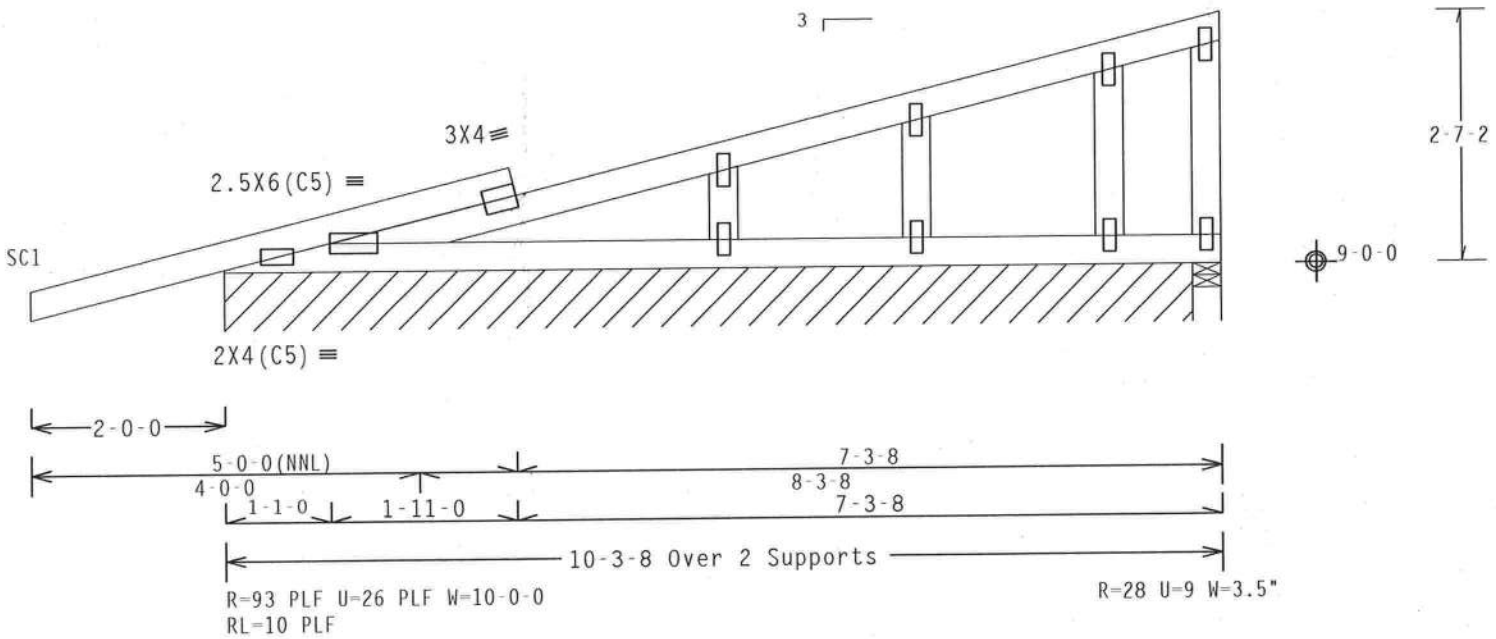
Right end vertical not exposed to wind pressure.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave

FT/RT=20%(0%)/0(0)

9.02.00

QTY:1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES E. COLLINS, JR. LICENSE
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jan 25 '10

TC LL	20.0 PSF	REF R8228- 44956
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025010
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76052
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - AGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense :B2 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W4, W7, W9, W17 2x4 SP #2 Dense:
:Stack Chord SC1 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Gable end supports 8" max rake overhang.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

(**) 8 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

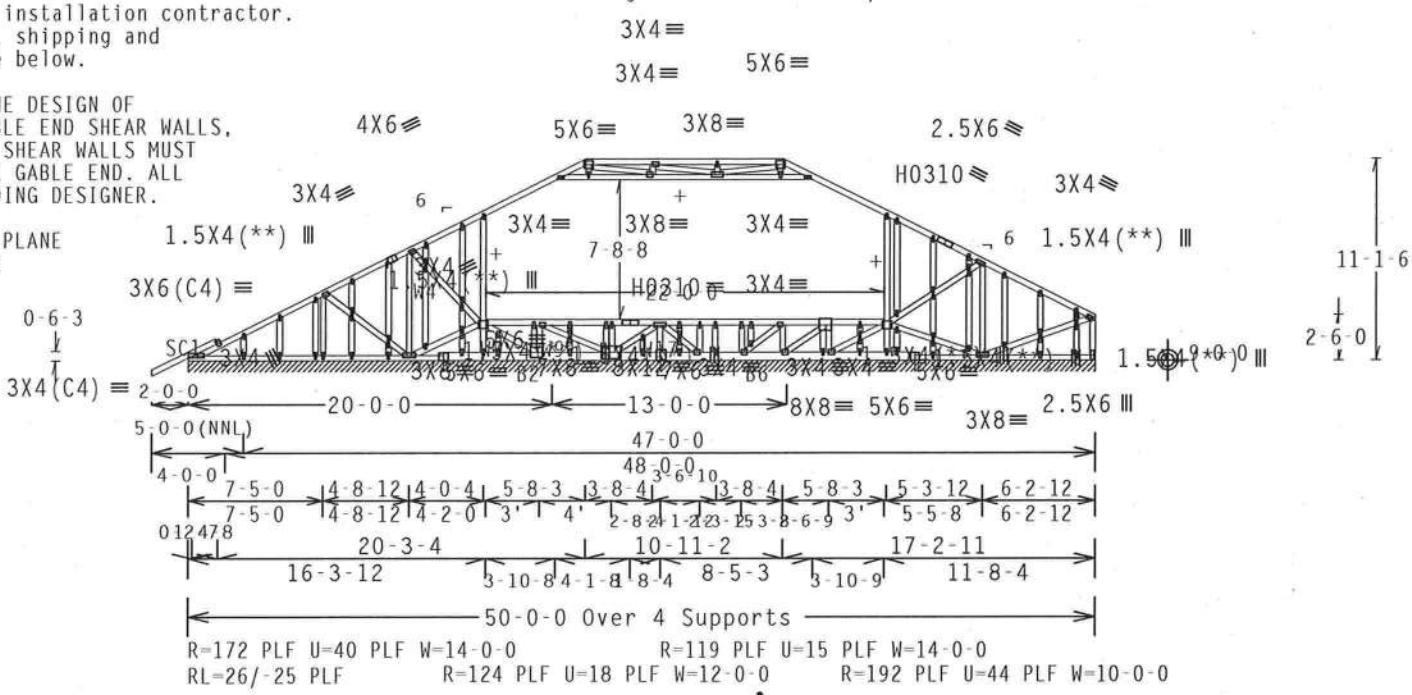
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.

Left and right bottom chords exposed to wind.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00.06 QTY:1 FL/-/4/-/-/R/- Scale =.09375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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JAMES T. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 29,10

TC LL	20.0 PSF	REF	R8228- 44957
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025002
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	77041
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - CGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

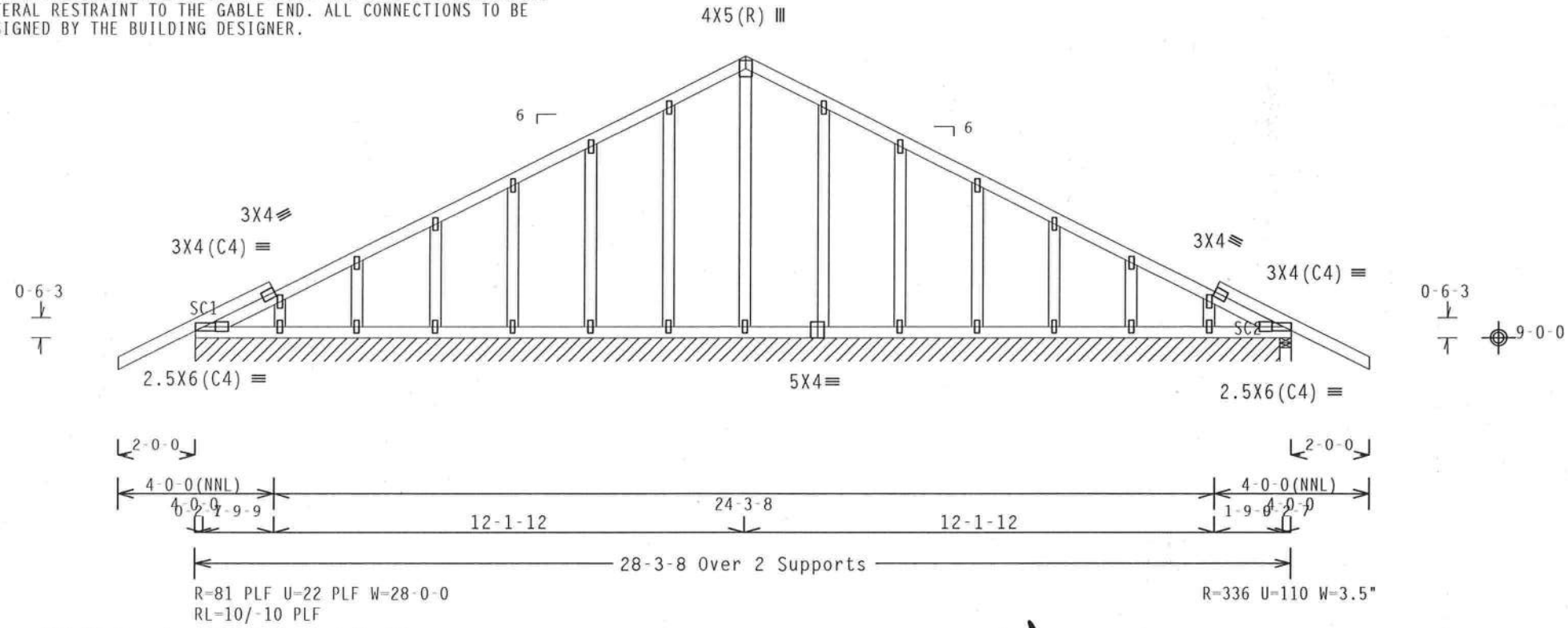
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00.0610 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES R. FOLKNER, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 29

TC LL	20.0 PSF	REF R8228- 44958
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025003
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76211
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - AP3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 62 plf at 0.00 to 62 plf at 5.46
TC - From 62 plf at 5.46 to 62 plf at 10.93
BC - From 4 plf at 0.00 to 4 plf at 10.93

Wind reactions based on MWFRS pressures.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

MWFRS loads based on trusses located at least 21.81 ft. from roof edge.

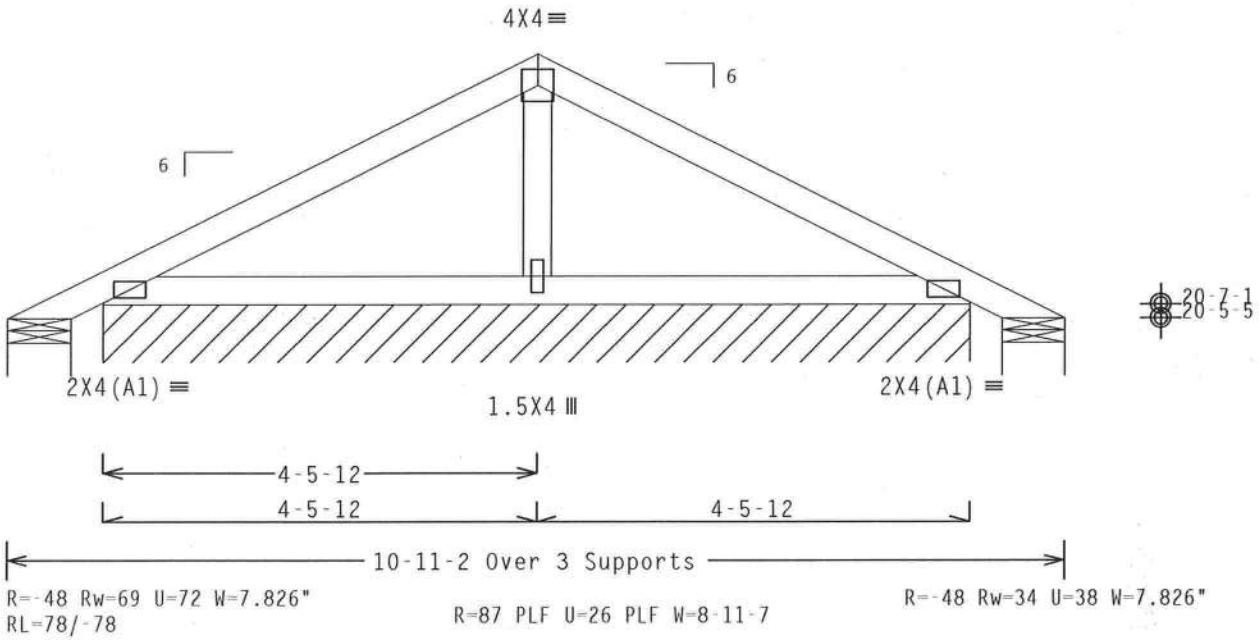
Refer to DWG PB1200109 for piggyback details.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 21.81 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00 76 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0278

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JAMES P. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 2

TC LL	20.0 PSF	REF R8228- 44959
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025020
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76228
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - AP4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 62 plf at 0.00 to 62 plf at 5.46
TC - From 62 plf at 5.46 to 62 plf at 10.93
BC - From 4 plf at 0.00 to 4 plf at 10.93

Wind reactions based on MWFRS pressures.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

MWFRS loads based on trusses located at least 21.81 ft. from roof edge.

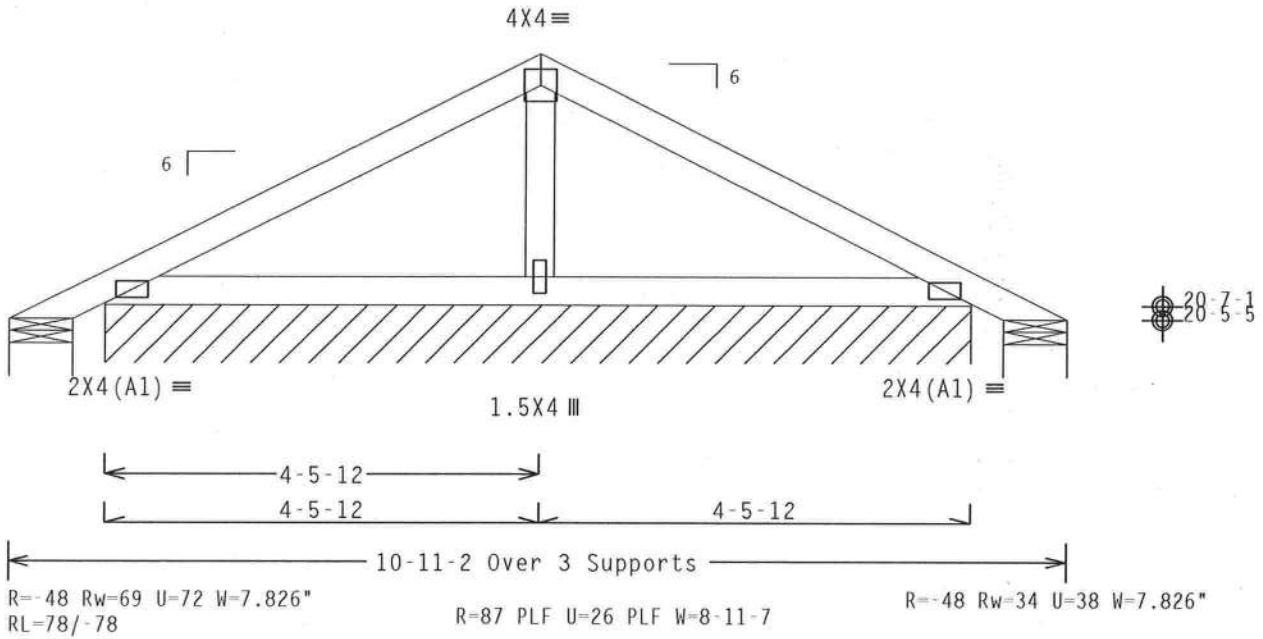
Refer to DWG PB1200109 for piggyback details.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 21.81 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF R8228- 44960
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025022
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76228
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A-6)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
:T5 2x6 SP #2:
Bot chord 2x6 SP #2 :B3, B4, B5 2x4 SP #2 Dense:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3 :W5, W8, W10 2x4 SP #2 Dense:

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC - From	218 plf at	-2.00 to	218 plf at	0.00
TC - From	218 plf at	0.00 to	218 plf at	10.00
TC - From	548 plf at	10.00 to	548 plf at	11.11
TC - From	548 plf at	11.11 to	548 plf at	16.31
TC - From	627 plf at	16.31 to	627 plf at	19.47
TC - From	297 plf at	19.47 to	297 plf at	20.49
TC - From	218 plf at	20.49 to	218 plf at	21.85
TC - From	218 plf at	21.85 to	218 plf at	32.78
TC - From	218 plf at	32.78 to	218 plf at	34.13
TC - From	297 plf at	34.13 to	297 plf at	35.16
TC - From	522 plf at	35.16 to	547 plf at	38.31
TC - From	469 plf at	38.31 to	497 plf at	41.72
TC - From	497 plf at	41.72 to	548 plf at	48.00
PLT- From	70 plf at	21.00 to	70 plf at	33.63
BC - From	16 plf at	-2.00 to	16 plf at	0.00
BC - From	20 plf at	0.00 to	20 plf at	16.31
BC - From	120 plf at	16.31 to	120 plf at	38.31
BC - From	20 plf at	38.31 to	20 plf at	48.00

BC - 605 lb Conc. Load at 6.00 , 8.00, 10.00, 11.25
12.33, 14.33, 18.33, 20.33, 22.33, 24.33, 26.33, 28.33, 30.33
31.71, 33.71, 35.71, 37.71, 39.79, 41.71, 43.71, 45.71
BC - 716 lb Conc. Load at 16.31

Calculated horizontal deflection is 0.21" due to live load and 0.16" due to dead load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Trusses to be spaced at 84.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. 20 Gauge HS,Wave

4 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @ 3.50" o.c.
Bot Chord: 1 Row @ 4.25" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Brg blocks:0.131"x3" nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
1	6.625'	2	12"	13	Rigid Surface

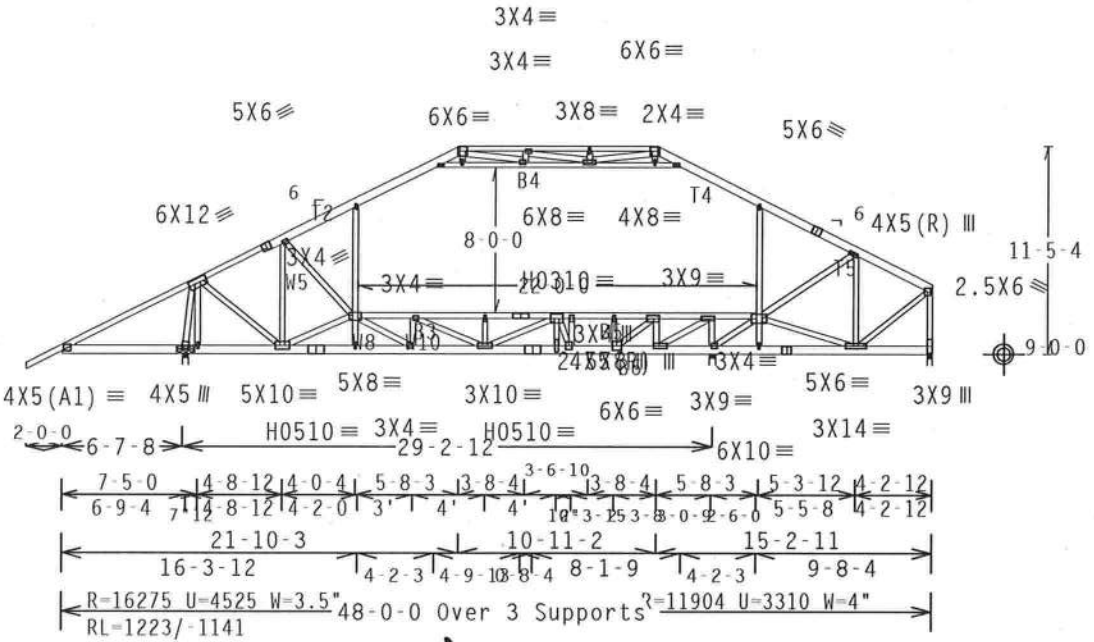
Brg block to be same size and species as bottom chord.
Refer to drawing CNNAILSP0109 for more information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 7.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/0(0) 9.02.00 QTY:1 FL/-/4/-/-/R/- Scale =.09375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44961
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025009
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	77052
DUR.FAC.	1.25		
SPACING	84.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - A-5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
:T5 2x6 SP #2:
Bot chord 2x4 SP #2 Dense :B1, B7 2x8 SP #1 Dense:
:B2 2x6 SP #2: :B6 2x6 SP SS:
Webs 2x4 SP #3 :W4, W5, W8, W10 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Calculated horizontal deflection is 0.26" due to live load and 0.20" due to dead load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Collar-tie braced with continuous lateral bracing at 24" OC.

Deflection meets L/240 live and L/180 total load.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

* WARNING* A reaction exceeds 20000 lbs.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

SPECIAL LOADS

------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	234 PLF at	-2.00 to	234 PLF at	0.00		
TC - From	218 PLF at	0.00 to	218 PLF at	10.00		
TC - From	548 PLF at	10.00 to	548 PLF at	16.31		
TC - From	627 PLF at	16.31 to	627 PLF at	19.47		
TC - From	297 PLF at	19.47 to	297 PLF at	20.49		
TC - From	218 PLF at	20.49 to	218 PLF at	21.00		
TC - From	288 PLF at	21.00 to	288 PLF at	33.63		
TC - From	218 PLF at	33.63 to	218 PLF at	34.13		
TC - From	297 PLF at	34.13 to	297 PLF at	35.16		
TC - From	522 PLF at	35.16 to	544 PLF at	38.31		
TC - From	466 PLF at	38.31 to	534 PLF at	48.00		
TC - From	394 PLF at	48.00 to	408 PLF at	50.00		
BC - From	20 PLF at	0.00 to	20 PLF at	16.31		
BC - From	120 PLF at	16.31 to	120 PLF at	38.31		
BC - From	20 PLF at	38.31 to	20 PLF at	50.00		

BC - 605 LB Conc. Load at 10.00, 11.25, 12.33, 14.33
BC - 1019 LB Conc. Load at 16.31
BC - 605 LB Conc. Load at 18.33, 20.33, 22.33, 24.33, 26.33, 28.33, 30.33, 31.71, 33.71, 35.71, 37.71, 39.79, 41.71, 43.71, 45.71

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS. THE TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

Note: All Plates Are 1.5X4 Except As Shown.

4 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @ 3.50" o.c.
Bot Chord: 1 Row @ 4.75" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Brg blocks:0.131"x3" nails

brg	x-loc	#blocks	length/blk	#nails/blk	wall plate
2	12.000'	2	16"	25	Rigid Surface

Brg block to be same size and species as bottom chord.
Refer to drawing CNNAILSP0109 for more information.

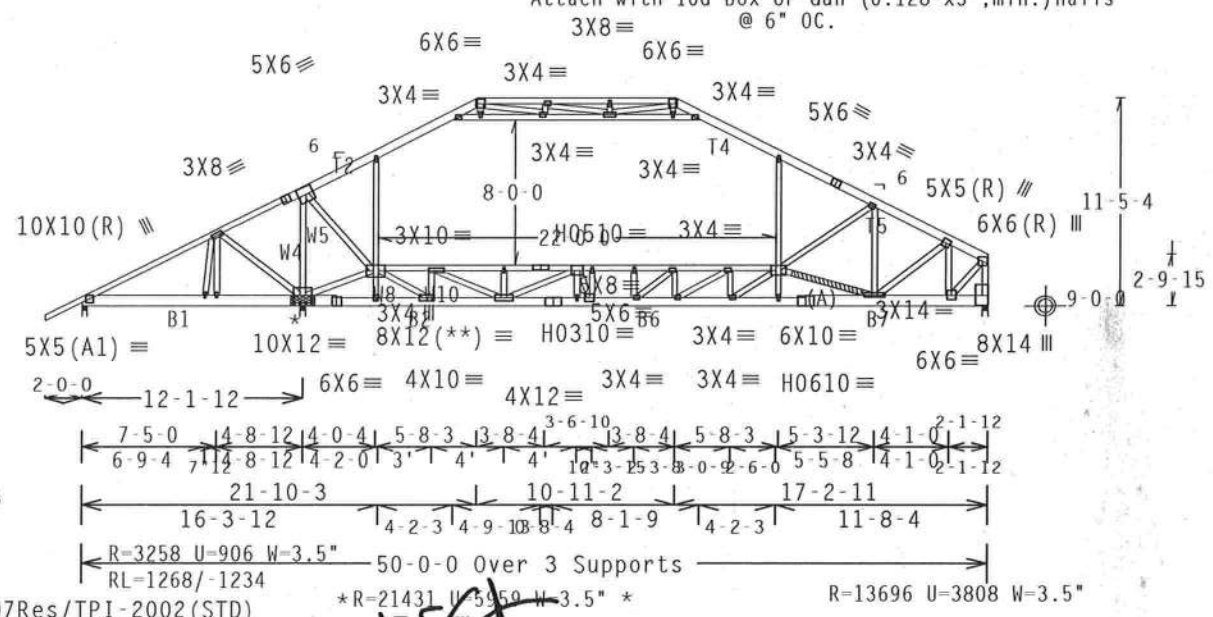
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 7.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" OC.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02.00 QTY:1 FL/-/4/-/-/R/- Scale =.09375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44962
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025003
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	40.0 PSF	SEQN-	77058
DUR.FAC.	1.25		
SPACING	84.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - F)

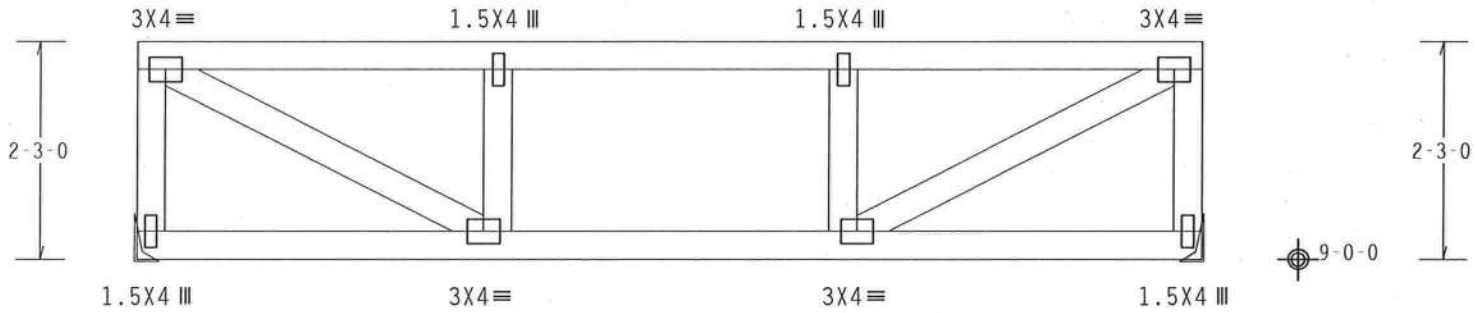
THIS DRW PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Truss must be installed as shown with top chord up.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/360 live and L/360 total load.



11-0-0 Over 2 Supports

R=605

R=605

Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/0(0)

PLT TYP. Wave

9.02.00.

QTY:23 FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	40.0 PSF	REF R8228- 44963
TC DL	10.0 PSF	DATE 01/25/10
BC DL	5.0 PSF	DRW HCUSR8228 10025007
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SEQN- 76250
DUR.FAC.	1.00	
SPACING	24.0"	JREF- 1TYR8228Z02

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 62 plf at 0.00 to 62 plf at 5.46
TC - From 62 plf at 5.46 to 62 plf at 10.93
BC - From 4 plf at 0.00 to 4 plf at 10.93

Wind reactions based on MWFRS pressures.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

MWFRS loads based on trusses located at least 21.81 ft. from roof edge.

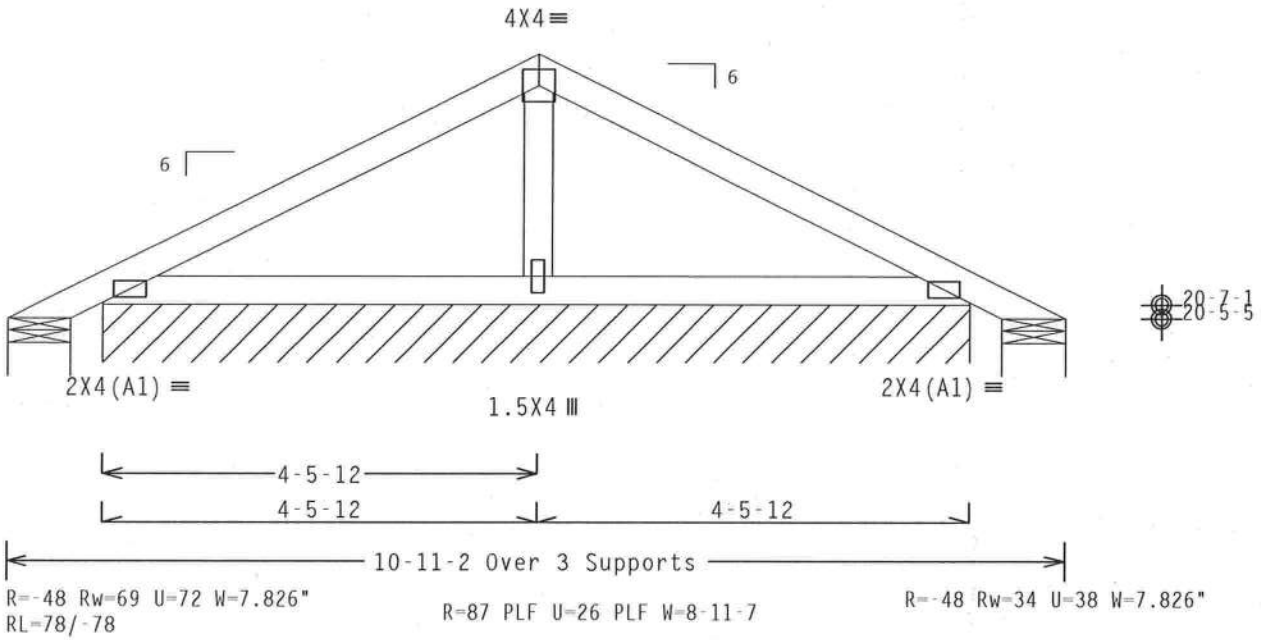
Refer to DWG PB1200109 for piggyback details.

4 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 21.81 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00.0 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLVIN, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF R8228- 44964
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025002
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76292
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - AP1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 62 plf at 0.00 to 62 plf at 5.46
TC - From 62 plf at 5.46 to 62 plf at 10.93
BC - From 4 plf at 0.00 to 4 plf at 10.93

Wind reactions based on MWFRS pressures.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

MWFRS loads based on trusses located at least 21.81 ft. from roof edge.

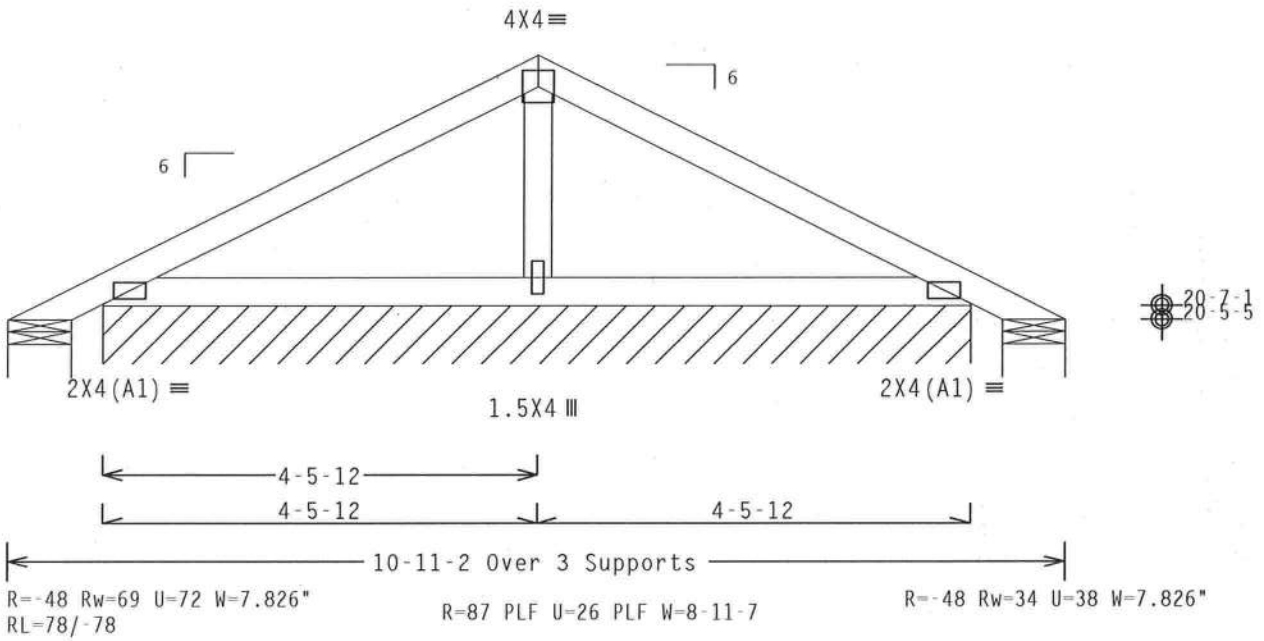
Refer to DWG PB1200109 for piggyback details.

4 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3" nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 21.81 ft mean hgt, ASCE 7-05, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00.06.0006 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 2010

TC LL	20.0 PSF	REF	R8228- 44965
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025019
BC LL	0.0 PSF		HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN-	76292
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

(10-016--Fill in later dicks -- , ** - APGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.81 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

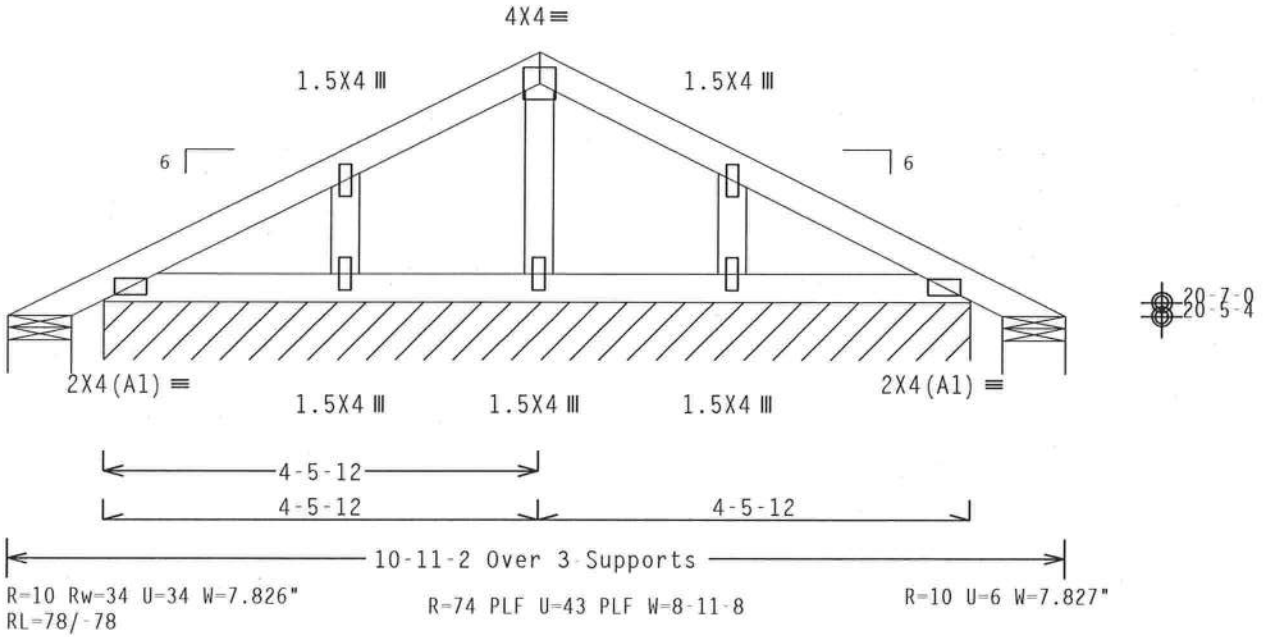
Refer to DWG PB1200109 for piggyback details.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC - From 62 plf at 0.00 to 62 plf at 5.46
TC - From 62 plf at 5.46 to 62 plf at 10.93
BC - From 4 plf at 0.00 to 4 plf at 10.93

Gable end supports 8" max rake overhang.

See DWGS A11030050109 & GBLLETIN0109 for more requirements.

Deflection meets L/240 live and L/180 total load.



Design Crit: FBC2007Res/TPI-2002(STD)
FT/RT=20%(0%)/0(0)

PLT TYP. Wave

9.02 00.[illegible] QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES N. COLLINS, JR. LICENSED No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF R8228- 44966
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025017
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 76295
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - AGE2)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense :B2, B5, B7 2x6 SP #2:
:B6 2x6 SP #1 Dense:
Webs 2x4 SP #3
:W4, W5, W7, W9, W13 2x4 SP #2 Dense:
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

Gable end supports 8" max rake overhang.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 16-3-12 to 38-3-12.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

(++) - This plate works for both joints covered.

(**) 5 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

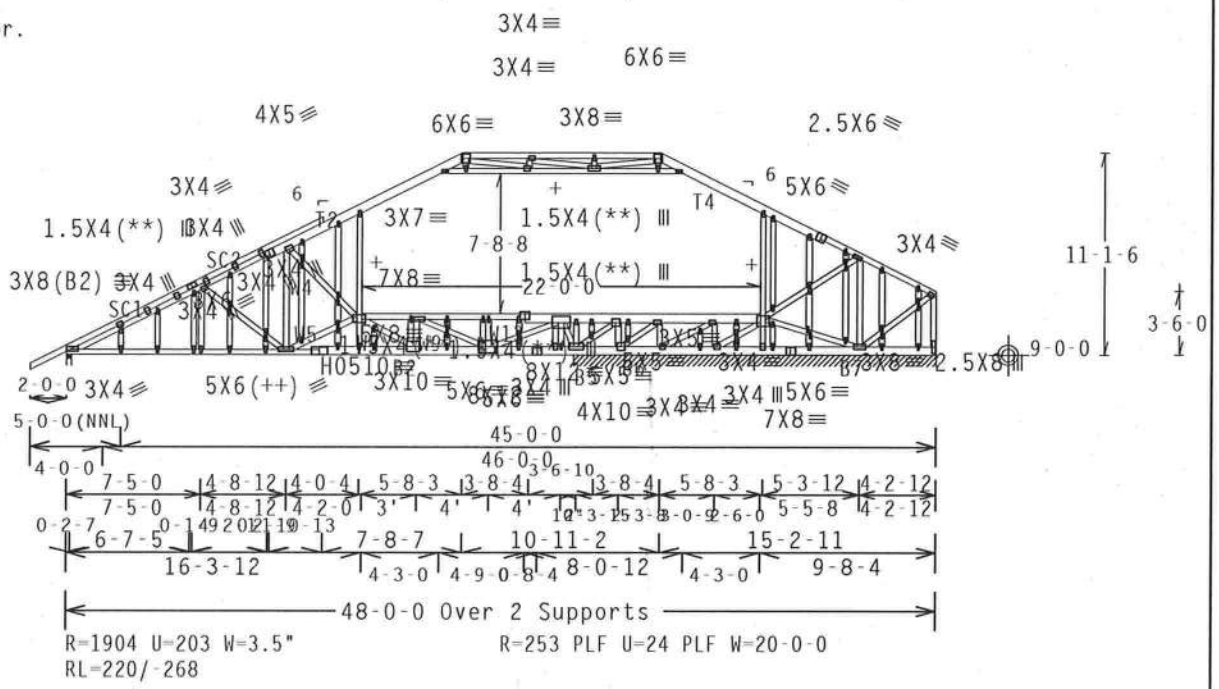
Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. 20 Gauge HS,Wave FT/RT=20%(0%)/0(0) 9.02 00 06 QTY:1 FL/-/4/-/-/R/- Scale =.09375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES A. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF R8228- 44967
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025004
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 77048
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - DOR)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

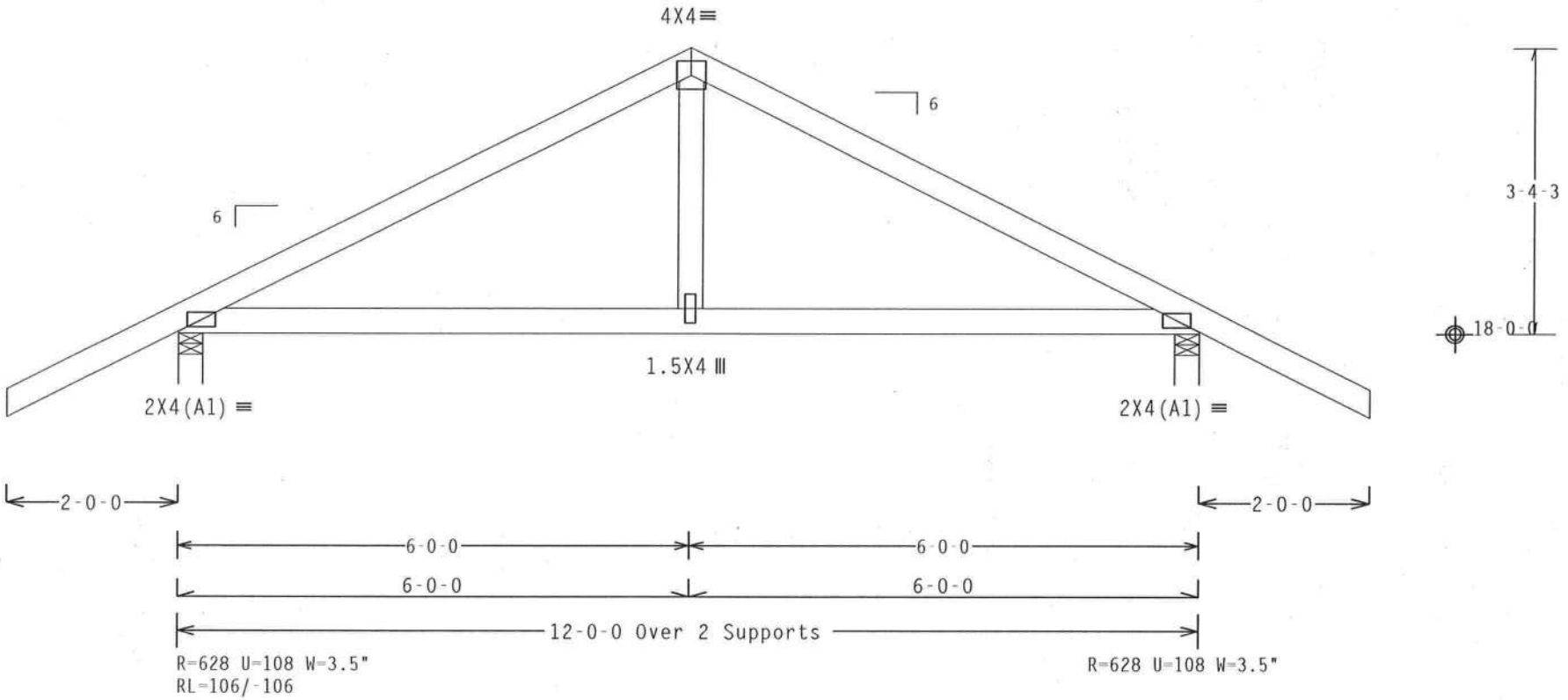
Bottom chord checked for 10.00 psf non-concurrent live load.

Left and right bottom chords exposed to wind.

110 mph wind, 19.35 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



Design Crit: CUSTOM/TPI-2002(STD)
FT/RT=20%(0%)/0(0)

PLT TYP. Wave

9.02.00

QTY:14 FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES R. TOLLING, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 25 '10

TC LL	20.0 PSF	REF R8228- 44968
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025005
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 77061
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - DORGE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

See DWGS A11030050109 & GBLLETIN0109 for more requirements.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Left and right bottom chords exposed to wind.

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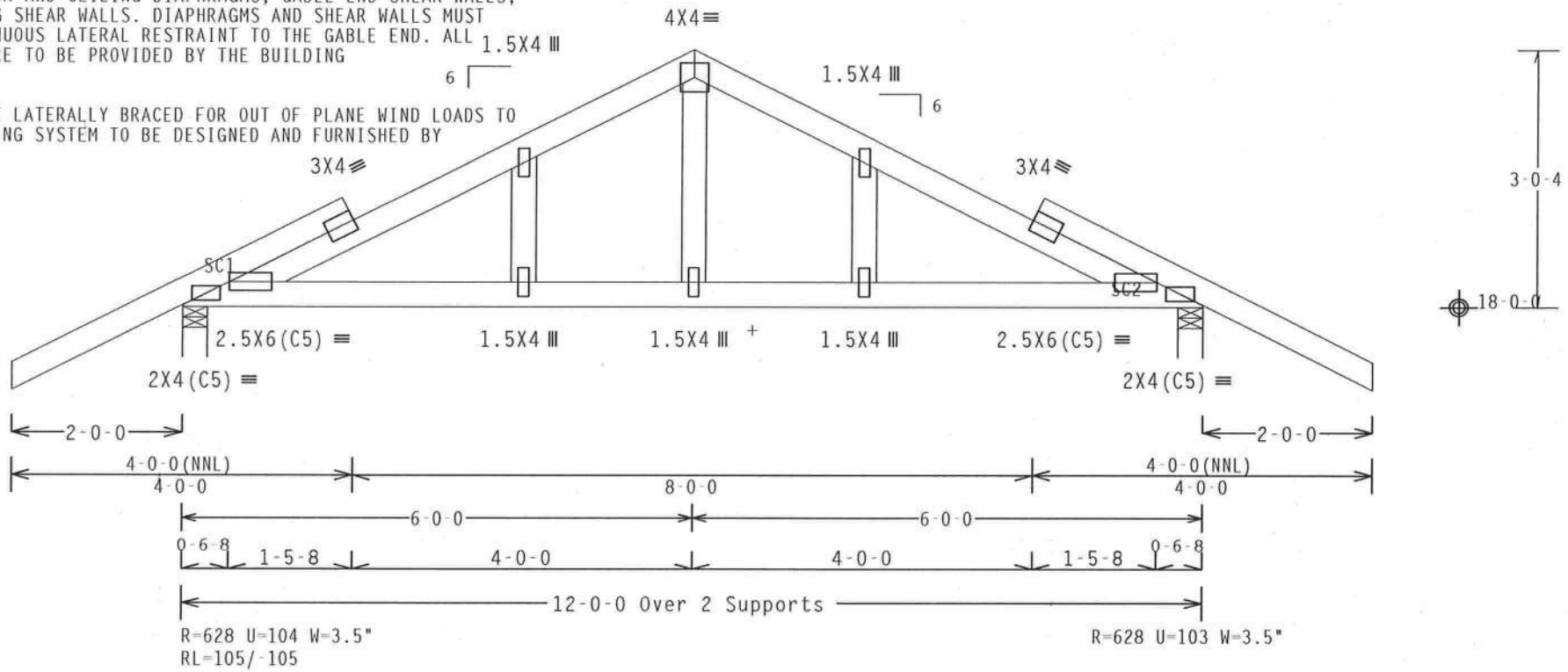
+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

110 mph wind, 19.18 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Gable end supports 8" max rake overhang.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02.00.0061 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES R. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan

TC LL	20.0 PSF	REF R8228- 44969
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025006
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN- 77065
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - DOR1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

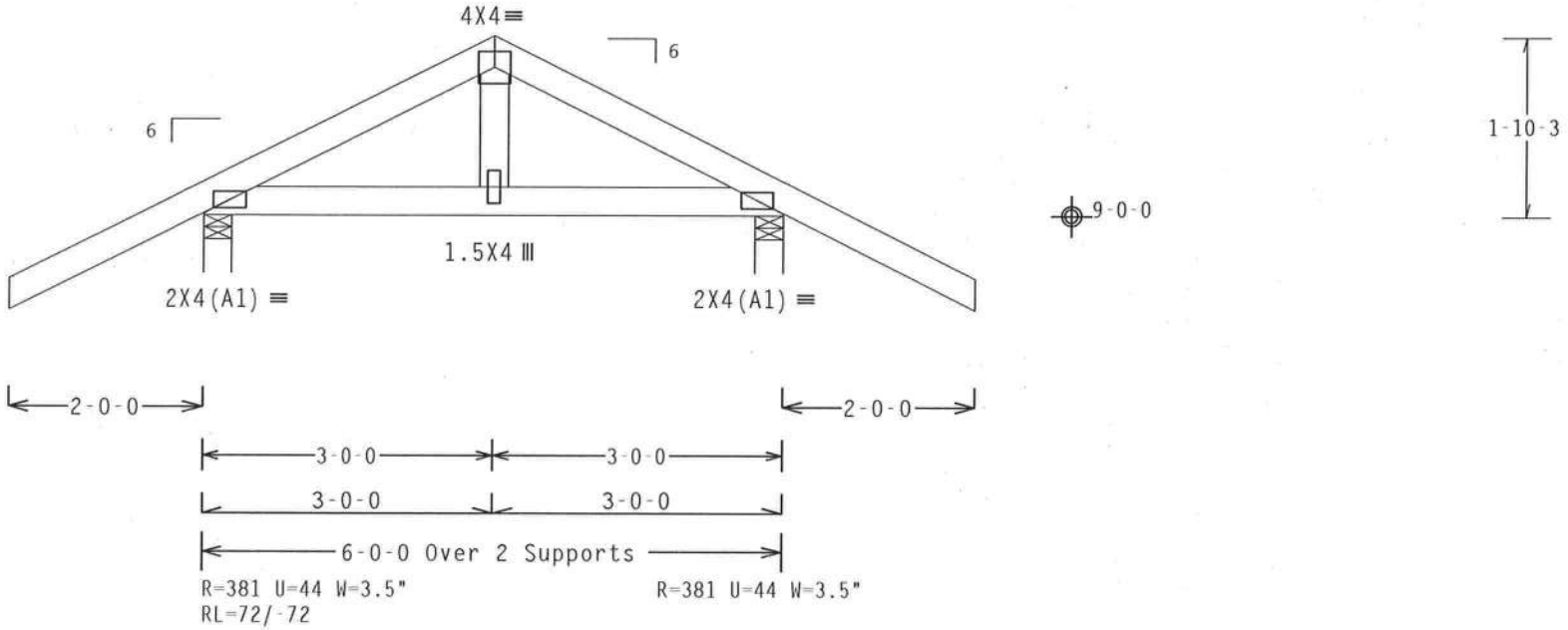
Bottom chord checked for 10.00 psf non-concurrent live load.

Left and right bottom chords exposed to wind.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load.



Design Crit: CUSTOM/TPI-2002(STD)
FT/RT=20%(0%)/0(0)

PLT TYP. Wave

9.02.00.0615

QTY:14 FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 2[illegible]

TC LL	20.0 PSF	REF R8228- 44970
TC DL	10.0 PSF	DATE 01/25/10
BC DL	10.0 PSF	DRW HCUSR8228 10025007
BC LL	0.0 PSF	HC-ENG DF/DF *
TOT.LD.	40.0 PSF	SEQN- 77068
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TYR8228Z02

(10-016--Fill in later dicks -- , ** - DOR1GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense::Stack Chord SC2 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load.

Left and right bottom chords exposed to wind.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF, FLOOR AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. DIAPHRAGMS AND SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS ARE TO BE PROVIDED BY THE BUILDING DESIGNER.

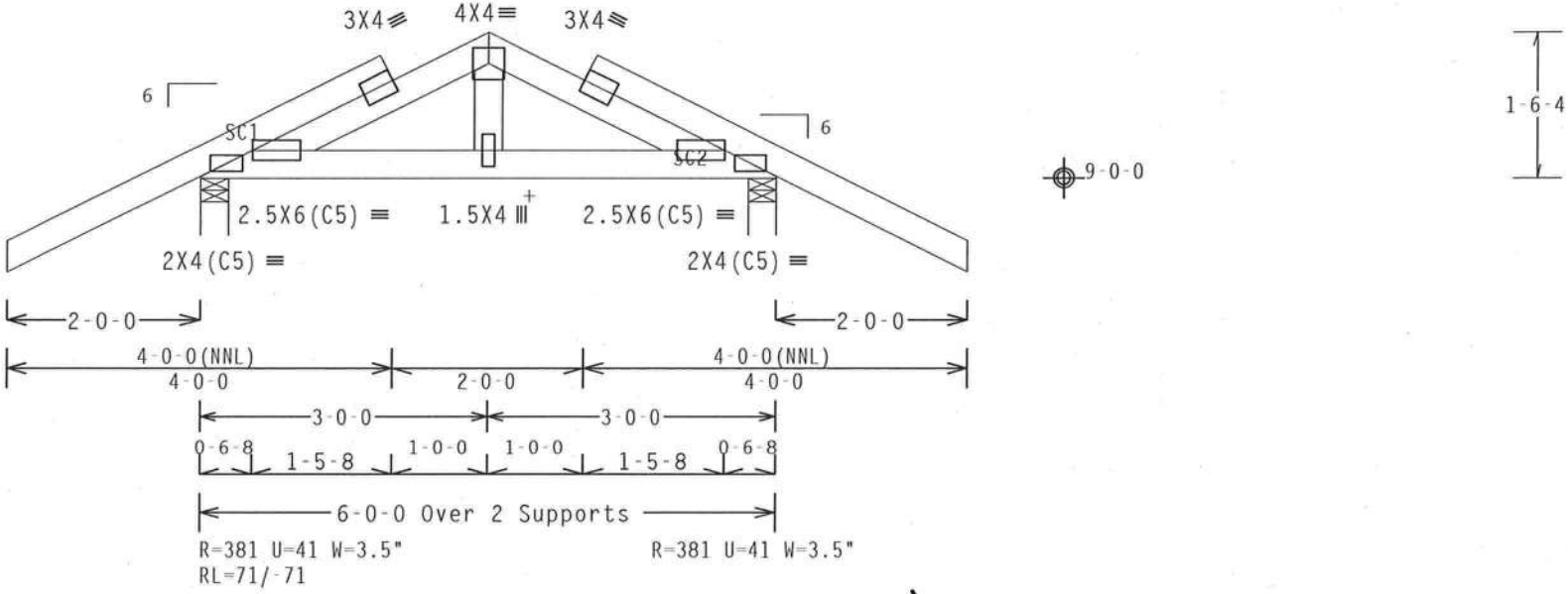
+ MEMBER TO BE LATERALLY BRACED FOR OUT OF PLANE WIND LOADS TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

See DWGS A11015050109 & GBLLETIN0109 for more requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.



Design Crit: FBC2007Res/TPI-2002(STD)

PLT TYP. Wave FT/RT=20%(0%)/0(0) 9.02 00 D61 QTY:2 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES H. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

TC LL	20.0 PSF	REF	R8228- 44971
TC DL	10.0 PSF	DATE	01/25/10
BC DL	10.0 PSF	DRW	HCUSR8228 10025008
BC LL	0.0 PSF		HC-ENG DF/DF
TOT.LD.	40.0 PSF	SEQN-	77072
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1TYR8228Z02

NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH,GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

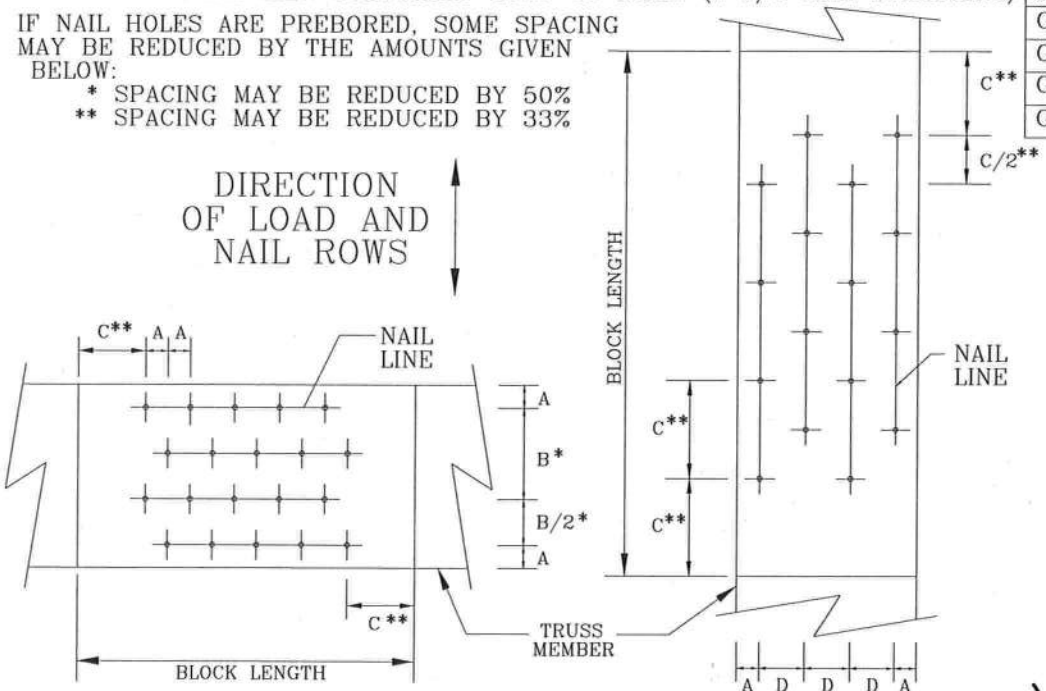
LOAD PERPENDICULAR TO GRAIN
- A – EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B – SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C – END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN
- A – EDGE DISTANCE (6 NAIL DIAMETERS)
- C – SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
- D – SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
- * SPACING MAY BE REDUCED BY 50%
- ** SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN LOAD APPLIED PARALLEL TO GRAIN

ITW Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3 & B7. See this job's general notes page for more information.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

REF	NAIL SPACE
DATE	1/1/09
DRWG	CNNAILSP0109

GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

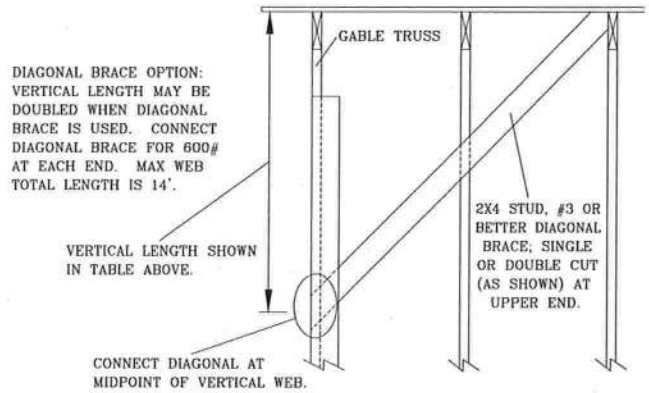
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

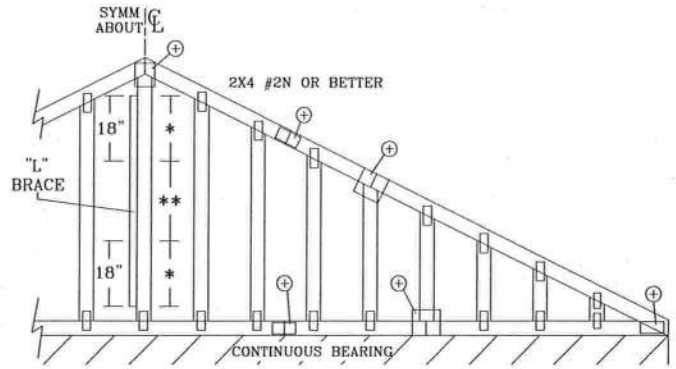
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX. GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS. (0.128"x3" min)

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ITW Building Components Group Inc.

Earth City, MO 63045

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ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

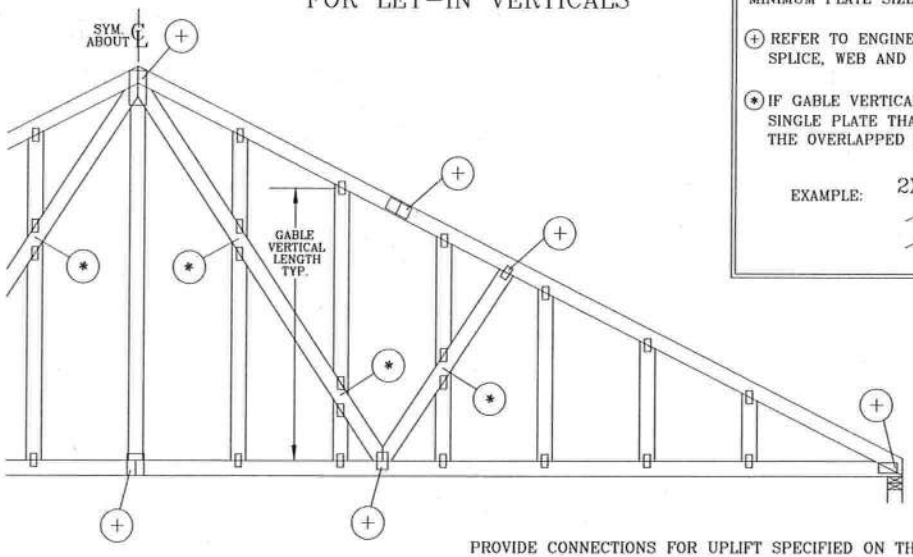
Jan 25 '10

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-05-GAB11015
DATE	1/1/09
DRWG	A11015050109

GABLE DETAIL
FOR LET–IN VERTICALS



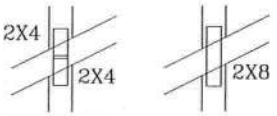
GABLE TRUSS PLATE SIZES

REFER TO APPROPRIATE ITW GABLE DETAIL FOR MINIMUM PLATE SIZES FOR VERTICAL STUDS.

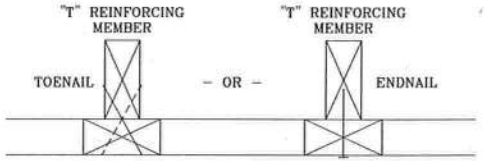
(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

(*) IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE THAT COVERS THE TOTAL AREA OF THE OVERLAPPED PLATES TO SPAN THE WEB.

EXAMPLE:



"T" REINFORCEMENT ATTACHMENT DETAIL



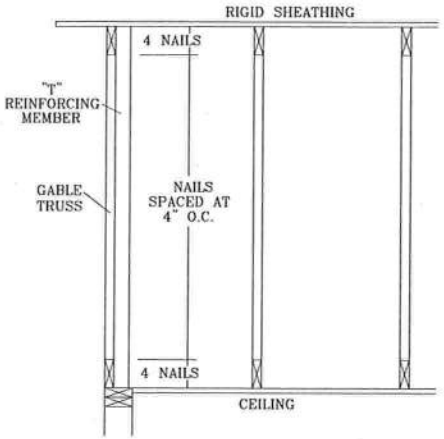
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" INCREASE BY LENGTH (BASED ON APPROPRIATE ITW GABLE DETAIL).

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	"T" INCREASE
140 MPH	2x4	10 %
15 FT	2x6	50 %
140 MPH	2x4	10 %
30 FT	2x6	50 %
130 MPH	2x4	10 %
15 FT	2x6	50 %
130 MPH	2x4	10 %
30 FT	2x6	50 %
120 MPH	2x4	10 %
15 FT	2x6	50 %
120 MPH	2x4	10 %
30 FT	2x6	40 %
110 MPH	2x4	10 %
15 FT	2x6	40 %
110 MPH	2x4	10 %
30 FT	2x6	50 %
100 MPH	2x4	20 %
15 FT	2x6	30 %
100 MPH	2x4	10 %
30 FT	2x6	40 %
90 MPH	2x4	20 %
15 FT	2x6	20 %
90 MPH	2x4	20 %
30 FT	2x6	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT, Kzt = 1.00
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
END DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) NAILS AT 4" O.C. PLUS
(4) NAILS IN TOP AND BOTTOM CHORD.
TOENAILED NAILS:
10d COMMON (0.148"x3",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ITW GABLE DETAIL FOR ASCE WIND LOAD.
ASCE 7–98 GABLE DETAIL DRAWINGS
A13015980109, A12015980109, A11015980109, A10015980109, A13030980109, A12030980109, A11030980109, A10030980109
ASCE 7–02 GABLE DETAIL DRAWINGS
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109, A13030020109, A12030020109, A11030020109, A10030020109, A14030020109
ASCE 7–05 GABLE DETAIL DRAWINGS
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109, A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

SEE APPROPRIATE ITW GABLE DETAIL FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

ITW Building Components Group Inc.
Earth City, MO 63045

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
20 25 '10

MAX TOT. LD. 60 PSF
DUR. FAC. ANY
MAX SPACING 24.0"

REF	LET–IN VERT
DATE	1/1/09
DRWG	GBLLETIN0109

GABLE STUD REINFORCEMENT DETAIL

ASCE 7-05: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C, Kzt = 1.00

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"
		#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STUD	4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 10"	5' 10"	7' 8"	7' 8"	10' 3"	10' 4"	11' 11"	11' 11"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 8"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

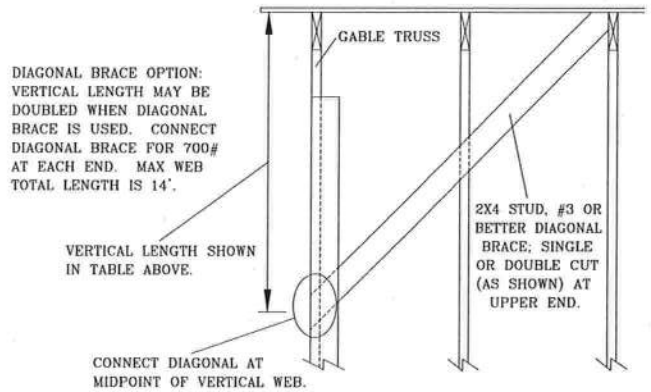
SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

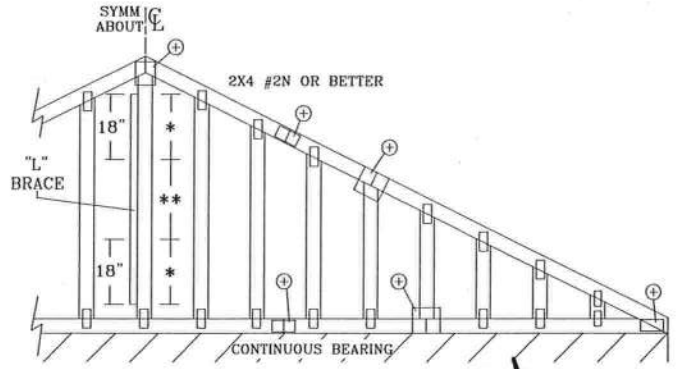
DOUGLAS FIR-LARCH	SOUTHERN PINE
#3	#3
STUD	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE	DOUGLAS FIR-LARCH
#1	#1
#2	#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS. (0.128"x3" min)

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2.5X4
GREATER THAN 11' 6"	3X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ITW Building Components Group Inc.

Earth City, MO 63045

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ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

JAN 25 '10

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-05-GAB11030
DATE	1/1/09
DRWG	A11030050109

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

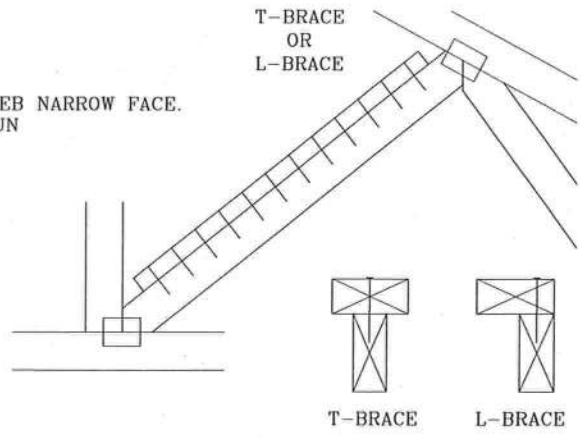
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

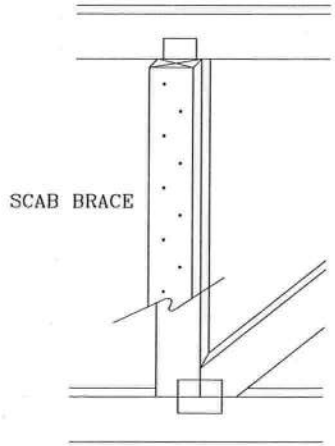
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128"x 3.",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



Building Components Group Inc.

Earth City, MO 63045

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 '10

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

120 PIGGYBACK DETAIL

UP TO 120 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, ENCLOSED BLDG. LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND DL= 5.0 PSF KZT=1.0.

MAXIMUM TRUSS SPACING IS 24" O.C.
DETAIL IS NOT APPLICABLE IF CAP SUPPORTS ADDITIONAL LOADS SUCH AS CUPOLA, STEEPLE, CHIMNEY OR DRAG STRUT LOADS.

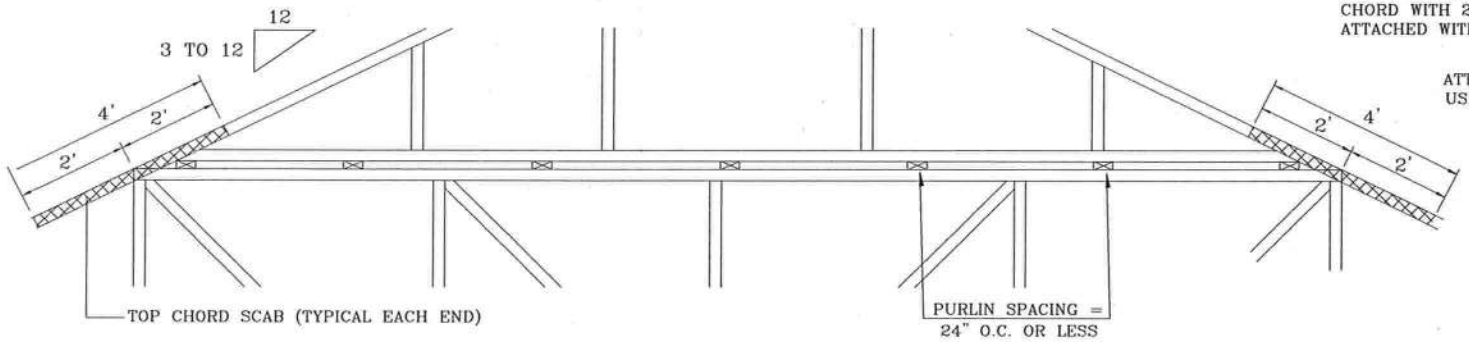
NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATELY BRACED BY SHEATHING OR PURLINS. THE BUILDING ENGINEER OF RECORD SHALL PROVIDE DIAGONAL BRACING, LATERAL BRACING FOR OUT OF PLANE LOADS OVER GABLE ENDS, OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

** REFER TO ENGINEER'S SEALED TRUSS DESIGN DRAWING FOR PIGGYBACK AND BASE TRUSS SPECIFICATIONS.

DETAIL A : PURLIN SPACING = 24" O.C. OR LESS

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

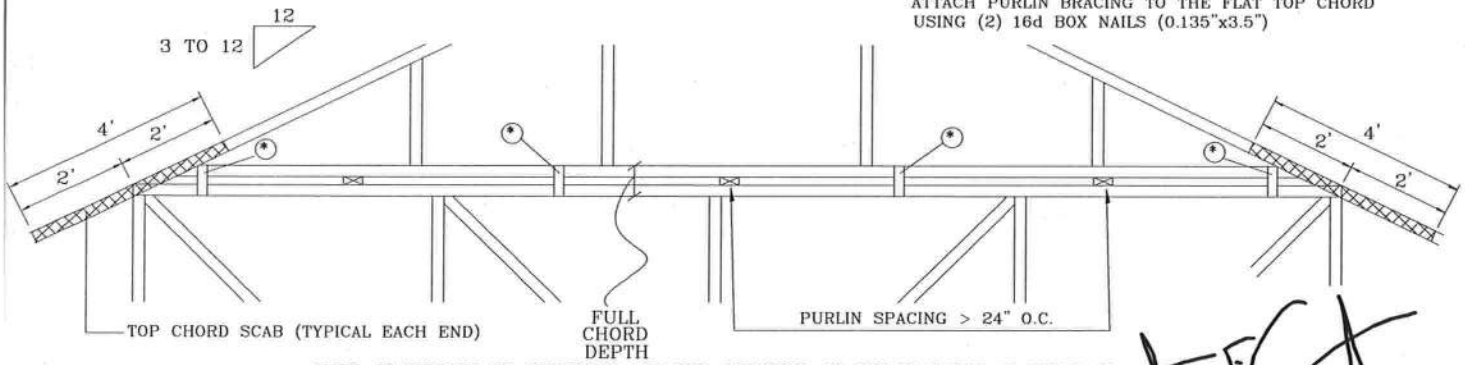
ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")



DETAIL B : PURLIN SPACING > 24" O.C.

PIGGYBACK CAP TRUSS SLANT NAILED TO ALL TOP CHORD PURLIN BRACING WITH (2) 16d BOX NAILS (0.135"x3.5") AND SECURE TOP CHORD WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY AT EACH END) ATTACHED WITH 2 ROWS OF 10d BOX NAILS (0.128"x3.0") AT 4" O.C.

ATTACH PURLIN BRACING TO THE FLAT TOP CHORD USING (2) 16d BOX NAILS (0.135"x3.5")



NOTE: IF PURLINS OR SHEATHING ARE NOT SPECIFIED ON THE FLAT TOP OF THE BASE TRUSS, PURLINS MUST BE INSTALLED AT 24" O.C. MAX. AND USE DETAIL A

* IN ADDITION, PROVIDE CONNECTION WITH ONE OF THE FOLLOWING METHODS:

TRULOX
USE 3x8 TRULOX PLATES FOR 2x4 CHORD MEMBER, AND 3x10 TRULOX PLATES FOR 2x6 AND LARGER CHORD MEMBERS. ATTACH TO EACH FACE @ 8' O.C. WITH (4) 0.120"x1.375" NAILS INTO CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. TRULOX PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

PLYWOOD GUSSET
8"x8"x1/2" RATED SHEATHING GUSSETS (EACH FACE). ATTACH @ 8' O.C. WITH (8) 6d COMMON (0.113"x2") NAILS PER GUSSET, (4) IN CAP BOTTOM CHORD AND (4) IN BASE TRUSS TOP CHORD. GUSSETS MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

2x4 VERTICAL SCABS
2x4 SPF#2, FULL CHORD DEPTH SCABS @ 8' O.C. EACH FACE, STAGGERED 4' O.C. ATTACH WITH (3) 10d BOX NAILS (0.128"x3") INTO BOTH CHORDS (TOTAL OF 6 NAILS PER SCAB).

28PB WAVE PIGGYBACK PLATE
ONE 28PB WAVE PIGGYBACK PLATE TO EACH FACE @ 8' O.C. ATTACH TEETH TO PIGGYBACK AT TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120"x1.375" NAILS PER FACE PER PLY. PIGGYBACK PLATES MAY BE STAGGERED 4' O.C. FRONT TO BACK FACES.

ITW
Building Components Group Inc.
Earth City, MO 63045

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 25 10

SPACING 24.0"

REF	PIGGYBACK
DATE	10/01/09
DRWG	PB1201009

Notice of Inspection and/or Treatment

10-22-10 #28448

Date of Inspection

Date of Treatment

Termidor 80WG 5535

Pesticide Used · Square Feet Sprayed

Subterranean

Wood-Destroying Organism Treated

Pursuant to Chapter 482, Florida Statutes, 482.226(6), this notice is required to be posted. Any licensee who performs control of any wood-destroying organism shall post notice of said treatment immediately adjacent to the access to the attic or crawl area or other readily accessible area of the property treated.

Gamble Pest Control

12314 Bass Road
Live Oak, Florida 32060
(386) 362-7519