

TYPICAL DESIGN WALL SECTION

NON - STRUCTURAL DATA

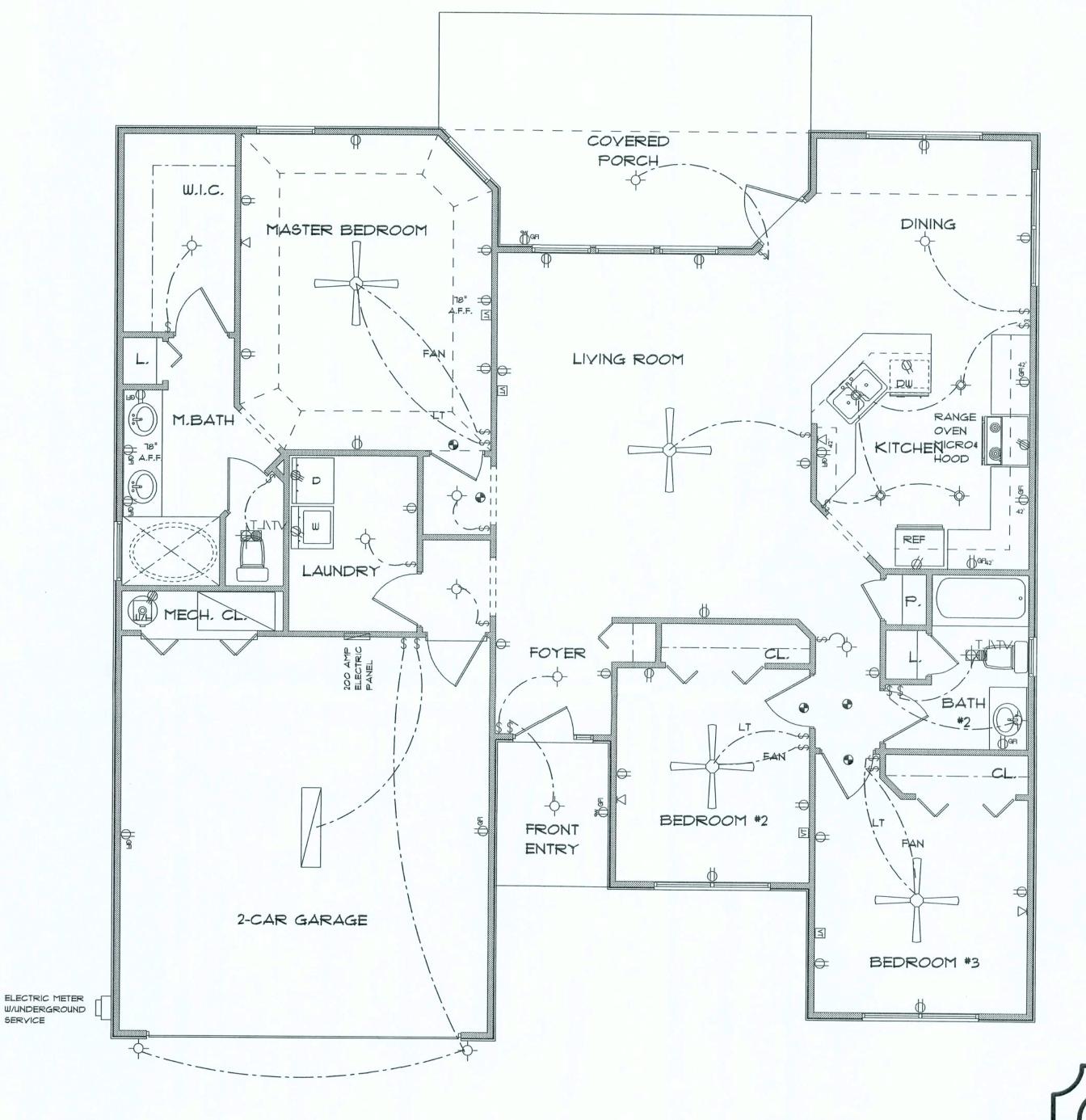
SCALE: 3/4" = 1'- 0"

ELECTRICAL	COUNT	SYMBO
ceiling fan	2	
200 amp service panel	1	200 AMP ELECTRIC PANEL
Light	2	<b></b>
cable tv outlet	4	M
ceiling fan	2	
electric meter	1	ELECTRIC HE WANDERSRO SERVICE
fluorescent It fixture		
gfi waterproof outlet	2	₽æ,
light	10	<b></b>
outlet	29	ð
outlet 220v	3_	4
outlet gfi	10	Q <sub>en</sub>
recessed can light	4	•
smoke detector	5	•
switch	21	\$
ewitch 3 way	2	\$,
telephone	4	Ø
vent light combo	2	VIA.T.

### ELECTRICAL PLAN NOTES

- E-1 WIRE ALL APPLIANCES, HYAC UNITS AND OTHER EQUIPMENT PER MANUFACTORS SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E-3 ALL INSTALLATIONS SHALL BE PER NATIONAL ELECTRIC CODE.
- E-4 ALL SMOKE DETECTORS SHALL BE 120V W/BATTER Y BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER, INSTALL INSIDE AND NEAR ALL BEDROJOMS.
- E-5

  TELEPHONE, TELEVYISION AND OTHER LOW YOLTAGE DEVICES
  OR OUTLETS SHALL BE AS PER THE OWNERS DIRECCTION AND IN
  ACCORDANCE WITH APPLICABLE SECTIONS OF NATTIONAL ELCT.
  CODE LATEST EDITION.
- E-6 ELECTRICAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE UNDERGROUND OR OVERHEAD) IS TO BE DETERMINED BY THE POWER COMPANY.
- ALL BEDROOM RECEPTICALS ARE TO BE AFCI (ARC FAULT CIRCUIT INTERRUPT)



# \* ELECTRICAL PLAN \*

SCALE : 1/4" = 1'-0"

"THE SAMUEL"
LOT #12
CSE

REVISIONS

11/21/05

SOFTPIAN ARCHITECTURAL DESIGN SOLIVAR

LAKE CITY FLORIDA

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> PRINTED DATE: January 18, 2006

DRAWN BY:
Teena M. Ruffo

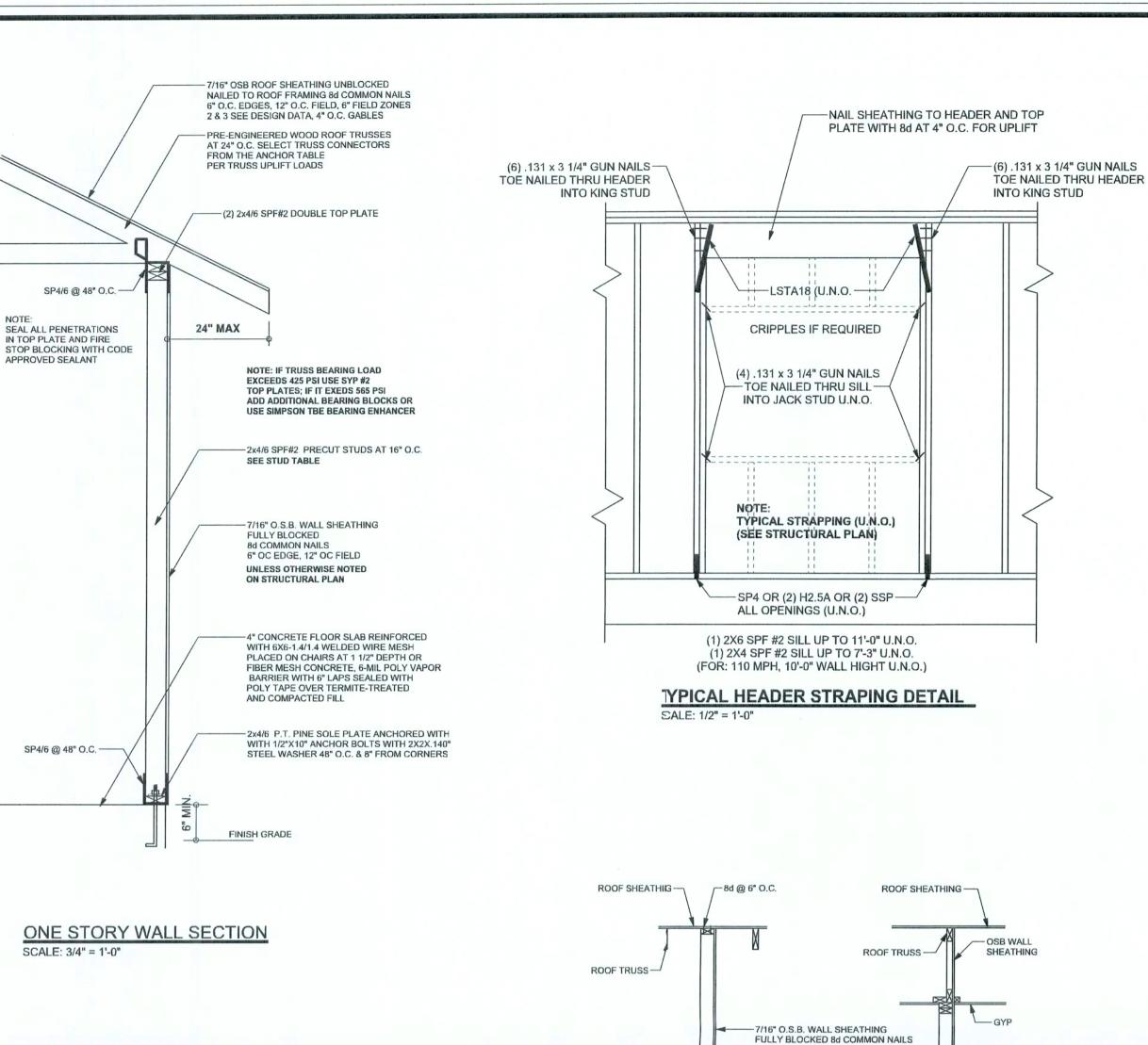
DESIGNED BT:

Jeena M. Ruffo

FINIALS DATE:

JOB NUMBER:

**A-2**OF 2 SHEETS





SIMPSON H2.5A U.N.O. -

(2) SIMPSON LSTA21 -

w/ (8) -16d TO HEADER

AND (8) -16d TO POST

SEE STRUCTURAL PLAN

(2) 2X10 SYP #2 U.N.O.

-6X6 SYP #2 POST

SEE STRUCTURAL PLAN

-SIMPSON ABU POST BASE

w/ (12) - 16d & 5/8" x 10"

-SEE FOOTING DETAILS

TYPICAL PORCH POST DETAIL SCALE: 1/2" = 1'-0" "

ANCHOR BOLT

PLIFT LBS. SYP	FT LBS. SYP UPLIFT LBS. SPF TRUSS CONNECTOR*		TO PLATES	TO RAFTER/TRUSS	TO STUDS		
< 420	< 420 < 245 H5A		3-8d	3-8d			
< 455	< 455 < 265 H5		4-8d	4-8d			
< 360	< 235	H4	4-8d	4-8d			
< 455	< 320	H3	4-8d	4-8d			
< 415	< 365	H2.5	5-8d	5-8d			
< 600	< 535	H2.5A	5-8d	5-8d			
< 950	< 820	H6	8-8d	8-8d			
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"			
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"			
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"			
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"			
< 760	< 655	H10-2	6-10d	6-10d			
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"			
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"			
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"			
< 2900	< 2490	2 - HTS24					
< 2050	< 1785	LGT2	14 -16d	14 -16d			
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION		
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED I 12" EMBEDMEN		
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED F 12" EMBEDMEN		
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED I 12" EMBEDMEN		
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED R 12" EMBEDMENT		
		STUD STRAP CONNECTOR*		_	TO STUDS		
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d	_	4 -10d		
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d		
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d		
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d	_	8 -10d		
< 885	< 760	SP4			6-10d, 1 1/2"		
< 1240	< 1065	SPH4			10-10d, 1 1/2"		
< 885	< 760	SP6			6-10d, 1 1/2"		
< 1240	< 1065	SPH6			10-10d, 1 1/2"		
< 1235	< 1165	LSTA18	14-10d				
< 1235	< 1235	LSTA21	16-10d				
< 1030	< 1030	CS20	18-8d				
< 1705	< 1705	CS16	28-8d				
		STUD ANCHORS*	TO STUDS		TO FOUNDATION		
< 1350	< 1305	LTT19	8-16d	7	1/2" AB		
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB		
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB		
< 4175	< 3695	HTT16	18 - 16d		5/8" AB		
< 1400	< 1400	PAHD42	16-16d	-			
< 3335	< 3335	HPAHD22	16-16d				
< 2200	< 2200	ABU44	12-16d	-	1/2" AB		
< 2300	< 2300	ABU66	12-16d		1/2" AB		
< 2320	< 2320	ABU88	18 - 16d	2-5/8" /			

7/16" STRUCTURAL ROOF SHEATHING 2X4 OUTRIGGER @ 48" OC. —— - HURRICANE CLIP H-2.5 OR EQUAL BLOCKING REQUIRED BETWEEN OUTRIGGERS -48" OC. - 2X4 BARGE RAFTER CONT. (3) .131 X 3 1/4 " GUN NAILS ---- SHINGLE STRIP 2X4 BLOCKING @ SHEATHING JOINT 4' FROM GABLE END -- FASCIA 2X4 SCAB CONT. TOP TO TOP CHORD OF GABLE END TRUSS - DROP 3 1/2" CHORD@ 8' FROM GABLE CONT. 2X4 SCAB FROM TOP TO 4 - 10d NAILS OR 4 - .131"x 3.25" BOTTOM CHORD @ X-BRACING TYPICAL AT ALL CONNECTIONS (PROVIDE ADDITIONAL 2X4'S @ VERTICAL IF HIGHER THAN 48", TO FORM AN "L" SHAPE.) 2X4 SCAB IF VERT. WEB IS NOT PRESENT TOE NAIL TRUSS TO DOUBLE PLATE w/ 16d COM @8" OC. CONT. 2X4X8' #2 SYP LATERAL BOTTOM CHORD OF GABLE BRACE @ 48" OC. -**END TRUSS** - 2 - 2X4 TOP PLATE 2X4 BLOCKING @ 48" OC. - SIMPSON LSTA 24 @ 48" OC. BETWEEN GABLE AND FIRST - 2X4 STUDS @16" OC. 2X4 X-BRACE @ 6'-0" OC.

ALL MEMBERS SHALL BE SYP

**GRADE & SPECIES TABLE** TYPICAL GABLE END (X-BRACING)

		Fb (psi)	E (1) <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	16
GLB	24F-V3 SP	2400	18
LSL	TIMBERSTRAND	1700	17
LVL	MICROLAM	2900	20
PSL	PARALAM	2900	20

VINDLOAD ENGINEER: Mak Disosway,

PE No.53915, POB 868, Lake City, FL

Stated dimensions superced scaled dimensions. Refer all questions to

Do not proceed without clarification.

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CERTIFICATION: I hereby cirtify that I have

examined this plan, and that he applicable

portions of the plan, relating b wind engineering comply with section R301.2.′, florida building

code residential 2004, to the jest of my

LIMITATION: This design is valid for one

MARK DISOSVAY

Cornerstone Developme

Zecher Bryan

The Samual Model

ADDRES:

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Mark Disosway P.E.

P.O. Box 368

Lake City, Florida 32056

Phone: (386) 754 - 5419

building, at specified location

Mark Disosway, P.E. for rescution

REVISIONS

SOFTPUAN

# **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 48 \* DB (30" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BIIII DEDIS DESDONSIDII ITV

THE BUILD SPECIFICA	R AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AR Y NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SIT	CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND HT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MAT REQUIREMEN	RIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 S FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE	ITINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU AN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ENGINEER IMMEDIATELY.
DESIGN, PLAC	USS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS MENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, SS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL TIONS.

#### ROOF SYSTEM DESIGN

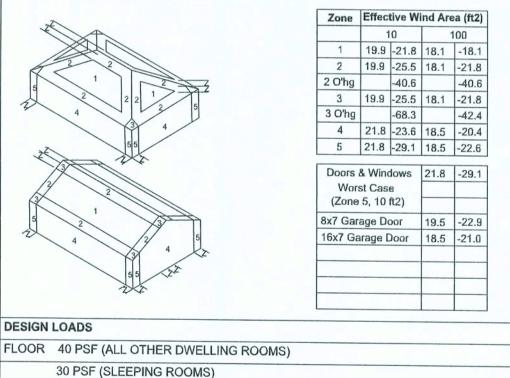
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R302.1.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS

WIN	DLOADS	PER FLC	RIDA	BUILD	ING C	ODE 20	04 PES	IDENTI	11 61	CTIO	N D2	04 2 4
	CLOSED S											
MEA	IN ROOF F	HEIGHT I	NOT EX	XCEEL	DING L	EAST F	HORIZO	NTAL D	IMEN	SION	OR 6	OFT.
ON	JPPER HA PE AND U	LF OF H	ILL OF	R ESCA	ARPM	ENT 60	FTINE	XP. B. 3	OFT II	VEXE	CA	ND >
	DING IS N								IVIILE	: VVHI	CHEV	ERI
	DING IS N					DEBRIS	REGIO	N				
1.)	BASIC WI			110 MF	PH							
2.)	WIND EXI											
3.)	WIND IMP	PORTAN	CE FA	CTOR	= 1.0							
4.)	BUILDING	CATEG	ORY =	11								
5.)	ROOF AN	GLE = 1	0-45 D	EGRE	ES							
6.)	MEAN RO	OF HEI	HT = <	<30 FT								
7.)	INTERNAL	L PRESS	SURE C	COEFF	ICIEN	T = N/A	(ENCL	OSED B	UILD	NG)		
8.)	COMPON	ENTS A	ND CLA	ADDIN	G DES	SIGN W	ND PR	ESSURI	ES (T	ABLE	R301	.2(2)
	ZE	<b></b>						Zone	Effective Wind Area (ff			
		IN						1		-21.8		100
	2	5/						2		-25.5		-21.
	5	1	2	1	3			2 O'hg		-40.6		-40.
3	1	2	2 2	5				3	19.9	-25.5	18.1	-21.
	4	1	3 4					3 O'hg	24.0	-68.3	10.5	-42
								4	21.8	-23.6	18.5	-20.



SOIL BEARING CAPACITY 1000PSF

12 PSF (12:12 AND GREATER)

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

ROOF 20 PSF (FI

	Fax: (386) 269 - 4871
NLOADS	PRINTED DATE:
R 40 PSF (ALL OTHER DWELLING ROOMS)	January 18, 2106
30 PSF (SLEEPING ROOMS)	STRUCTURAL B
30 PSF (ATTICS WITH STORAGE)	E/AN BEAMSLE
10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
20 PSF (FLAT OR <4:12)	
16 PSF (4:12 TO <12:12)	FINALS DATE:
	19 / IAN / OG

JOB NUMBER: 601181 DRAWING NUMBER

> S-1 OF 2 SHEE'S

LSTA24 NAIL THRU 2x4 INTO BEAM W/4-16d SIMPSON HUS412 MIN. SEE STRUCTURAL PLAN BEAM MAY BE ATTACHED IN EITHER METHOD SHOWN ABOVE

SEE FOUNDATION DETAILS

**INTERIOR BEARING WALL** 

STRAP STUDS -

SP4/6 TOP & BOTTOM

& BOTTOM @ 32" O.C

OR (2) H2.5A TOP

SEE STRUCTURAL PLAN

-2 x 4/6 STUDS AT 16" O.C.

- 1/2" X 7" WEDGE ANCHORS

AT 48" OC U.N.O.

SEE STRUCTURAL PLAN

SPF #2

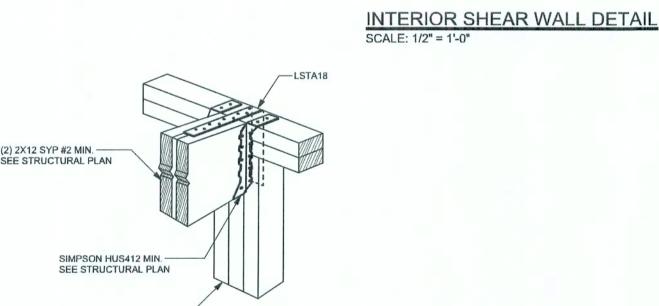
**BEAM CORNER CONNECTION. DETAIL** 

SUPPORTIVE CENTER POST TO BEAII DETAIL

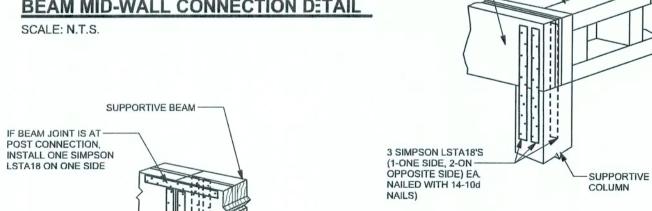
(2-ONE SIDE,2-ON OTHER SIDE)

4" O.C. TOP & BOTTOM 6" O.C. EDGE, 12" O.C. FIELD ----2 x 4/6 SPF #2 STUDS AT 16" O.C.

1/2" X 7" WEDGE ANCHORS AT 48" OC UNO -7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 4" O.C. TOP & BOTTOM 6" O.C. EDGE, 12" O.C. FIELD SEE FOUNDATION DETAILS



-(4)-2x4 SPF #2 NAILED NAILS AT 16" O.C. MIN. (SEE STRUCTURAL PLAN) **BEAM MID-WALL CONNECTION DETAIL** 



SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM

- NON-SUPPORTIVE

2X4 LADDER BEAM

SUPPORTIVE -

