



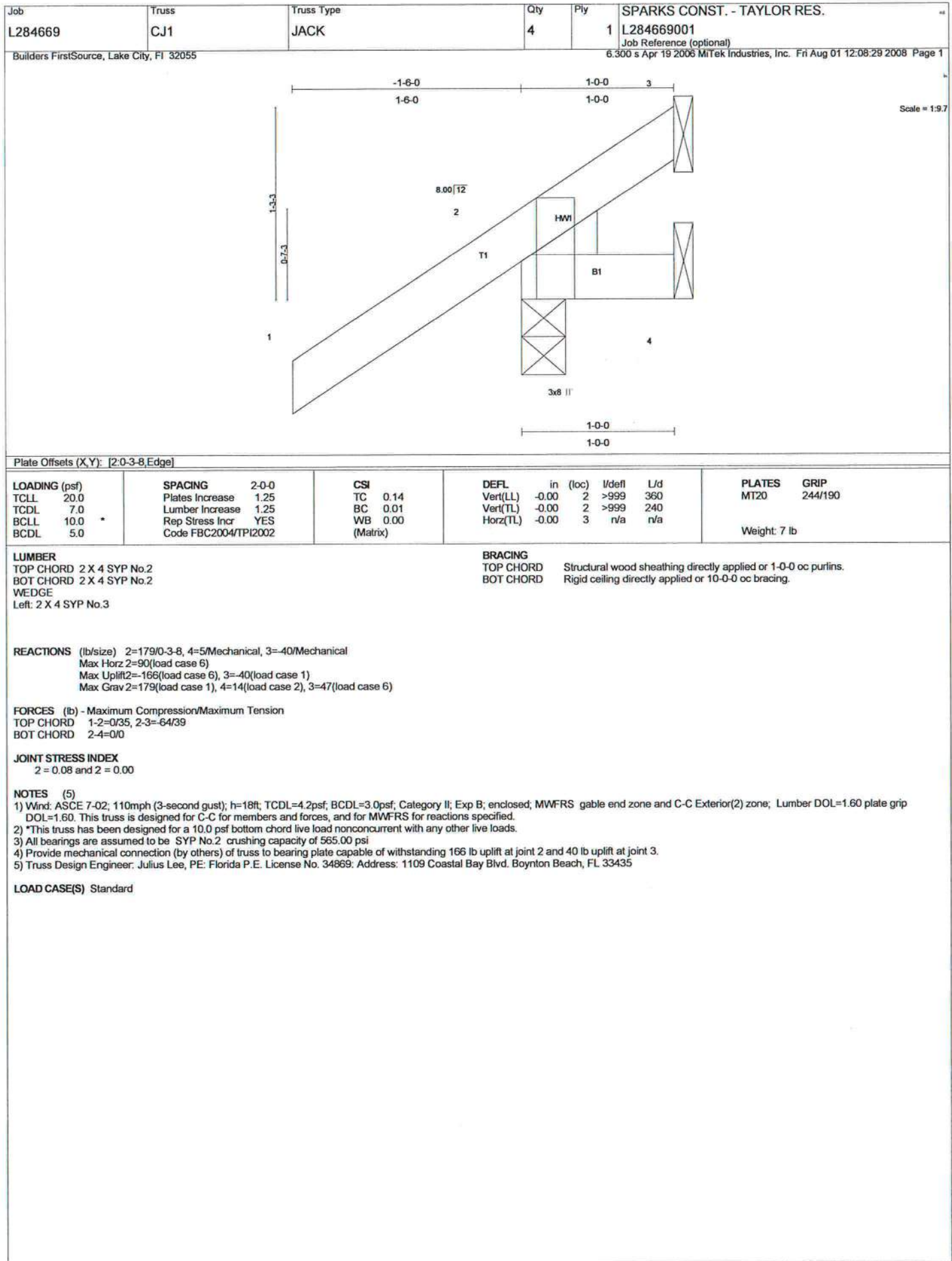
To whom it may concern,

This letter is intended to address the issue of warning notes on 7' jack trusses. I have reviewed the jack truss and it passes without modification for any jack up to 7' with a total loading not to exceed 55# and a maximum overhang of 2'. Below is a copy of note you will see on the jack. This letter will act as an approval for the truss mentioned above.

*****Design Problems*** Review Required/ Max Deflection In Panel Exceeded: A-B**



FL. PE# 34869



Job L284669	Truss CJ1A	Truss Type JACK	Qty 1	Ply 1	SPARKS CONST. - TAYLOR RES. L284669002 Job Reference (optional)
----------------	---------------	--------------------	----------	----------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:30 2008 Page 1

Plate Offsets (X,Y): [2-0-3-8,Edge]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.14	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 10.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TP12002		(Matrix)							
									Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=179/0-3-8, 4=5/Mechanical, 3=39/Mechanical
Max Horz 2=90(load case 6)
Max Uplift 2=-177(load case 6), 4=-11(load case 4), 3=-39(load case 1)
Max Grav 2=179(load case 1), 4=15(load case 2), 3=45(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=63/37
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.08 and 2 = 0.00

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 2, 11 lb uplift at joint 4 and 39 lb uplift at joint 3.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L284669	Truss CJ2	Truss Type JACK	Qty 1	Ply 1	SPARKS CONST. - TAYLOR RES. L284669003 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MITek Industries, Inc. Fri Aug 01 12:08:31 2008 Page 1		

Scale = 1:9.6

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.15	in (loc) l/defl U/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.03	Vert(LL) -0.00 2 >999 360		
BCLL 10.0 *	Rep Stress Incr YES	WB 0.00	Vert(TL) -0.00 2 >999 240		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 9 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=175/0-3-8, 4=9/Mechanical, 3=14/Mechanical

Max Horz 2=90(load case 6)

Max Uplift 2=-171(load case 6), 4=-17(load case 4), 3=-15(load case 7)

Max Grav 2=175(load case 1), 4=27(load case 2), 3=14(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-39/5

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.11

NOTES (5)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 2, 17 lb uplift at joint 4 and 15 lb uplift at joint 3.

5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-10-3 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

Job L284669	Truss CJ3	Truss Type JACK	Qty 4	Ply 1	SPARKS CONST. - TAYLOR RES. L284669004 Job Reference (optional)
----------------	--------------	--------------------	----------	----------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:31 2008 Page 1

Plate Offsets (X,Y): [2-0-3-8,Edge]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.15	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	-0.01	2-4	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 13 lb										

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

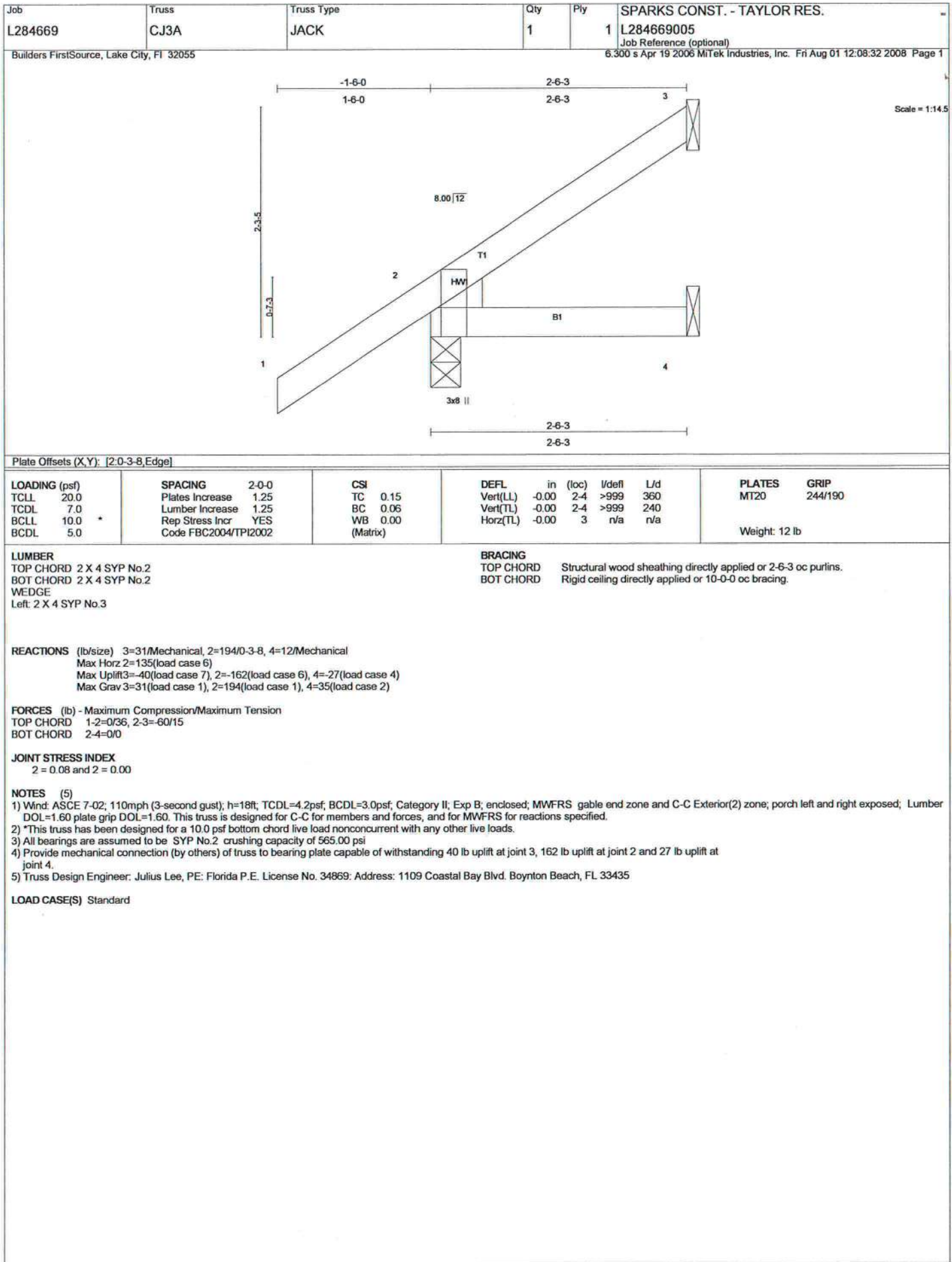
REACTIONS (lb/size) 3=49/Mechanical, 2=204/0-3-8, 4=14/Mechanical
Max Horz 2=150(load case 6)
Max Uplift 3=61(load case 6), 2=-129(load case 6)
Max Grav 3=49(load case 1), 2=204(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-64/21
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.09 and 2 = 0.00

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 3 and 129 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job L284669	Truss CJ4	Truss Type JACK	Qty 1	Ply 1	SPARKS CONST. - TAYLOR RES. L284669006 Job Reference (optional)
----------------	--------------	--------------------	----------	----------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:33 2008 Page 1

Plate Offsets (X,Y): [2-0-3-8,Edge]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.18	Vert(LL)	0.03	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.15	Vert(TL)	-0.02	2-4	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 17 lb										

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left: 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

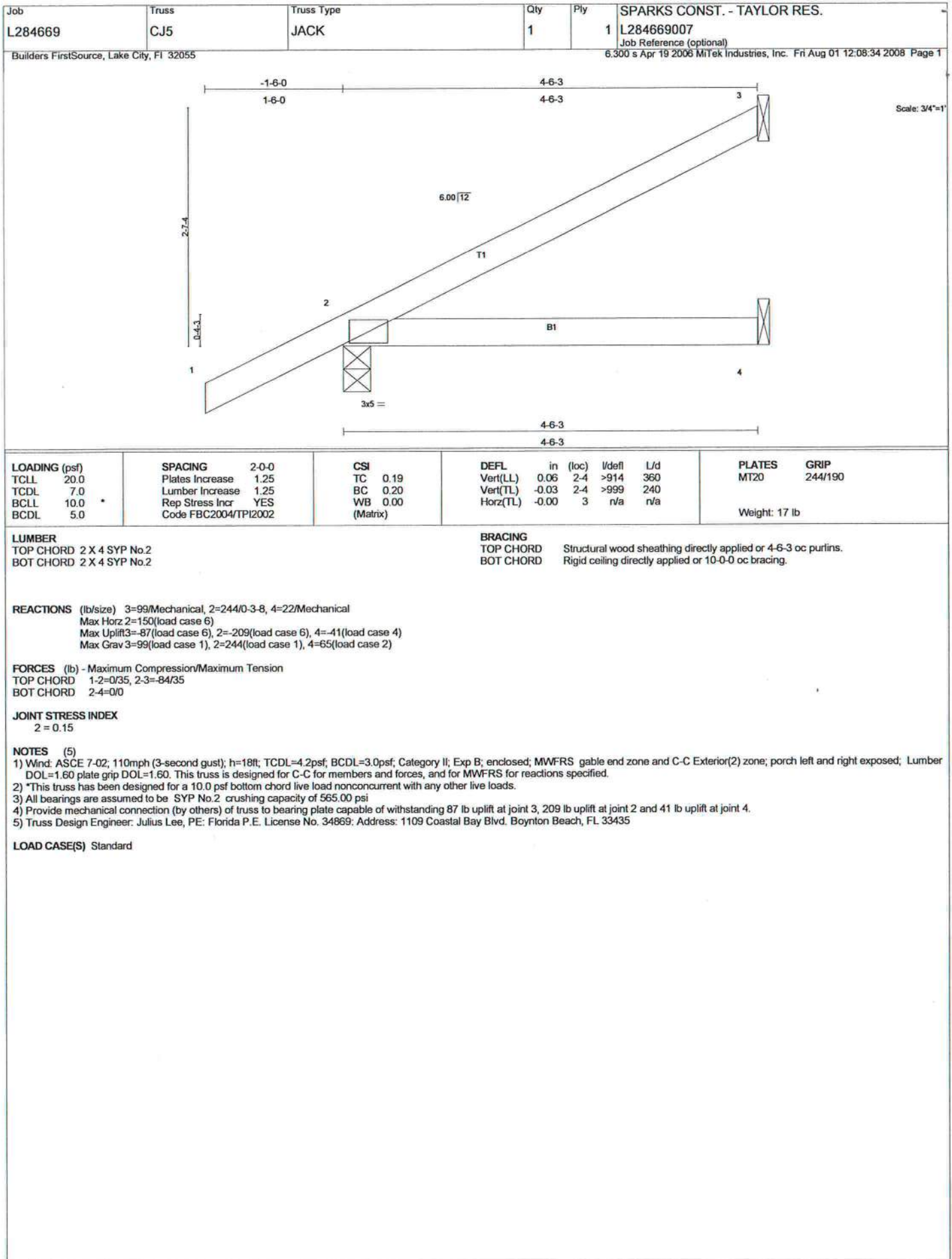
REACTIONS (lb/size) 3=84/Mechanical, 2=230/0-3-8, 4=19/Mechanical
Max Horz 2=181(load case 6)
Max Uplift 3=99(load case 6), 2=-174(load case 6), 4=-45(load case 4)
Max Grav 3=84(load case 1), 2=230(load case 1), 4=57(load case 2)

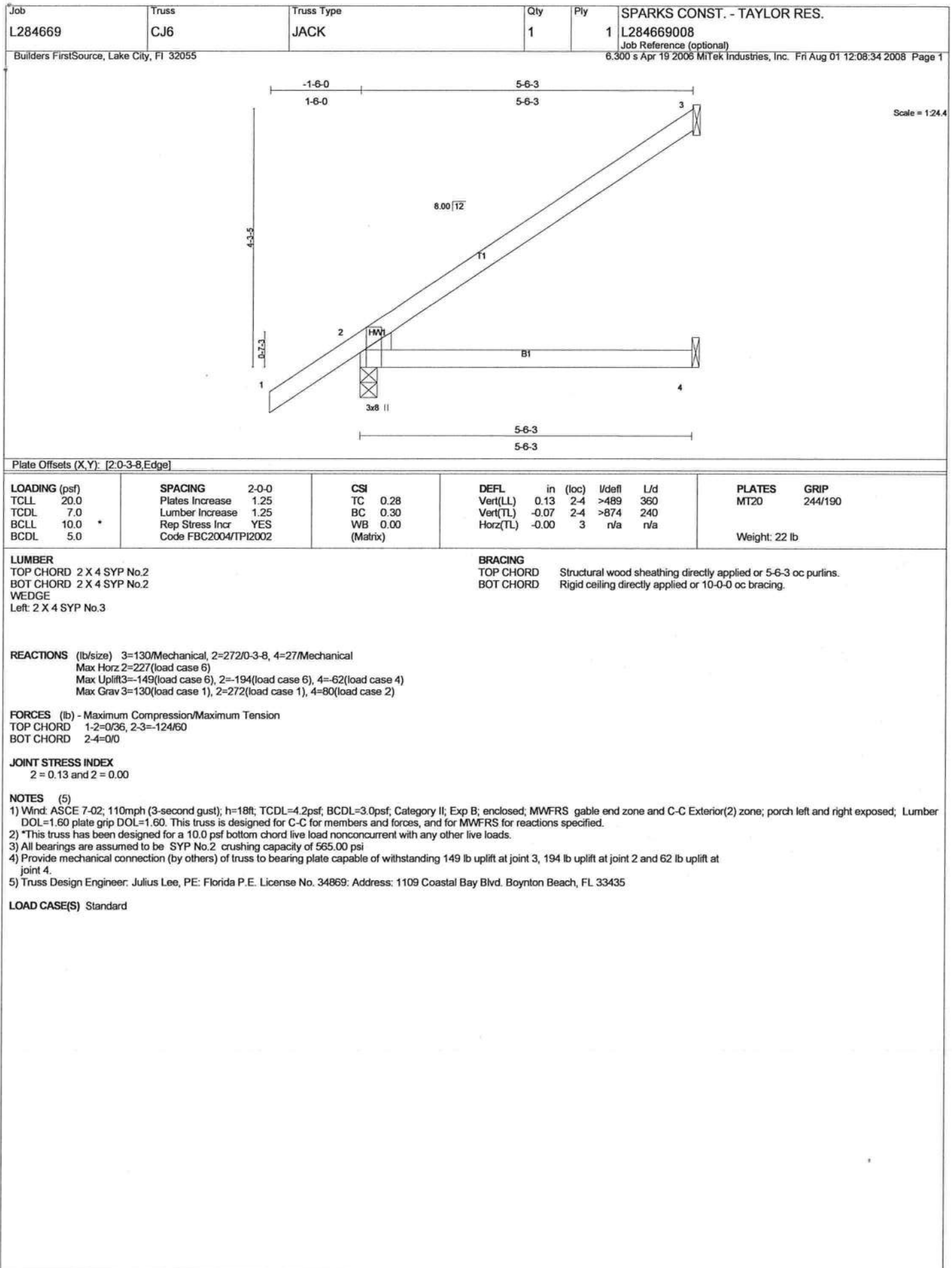
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-86/38
BOT CHORD 2-4=0/0

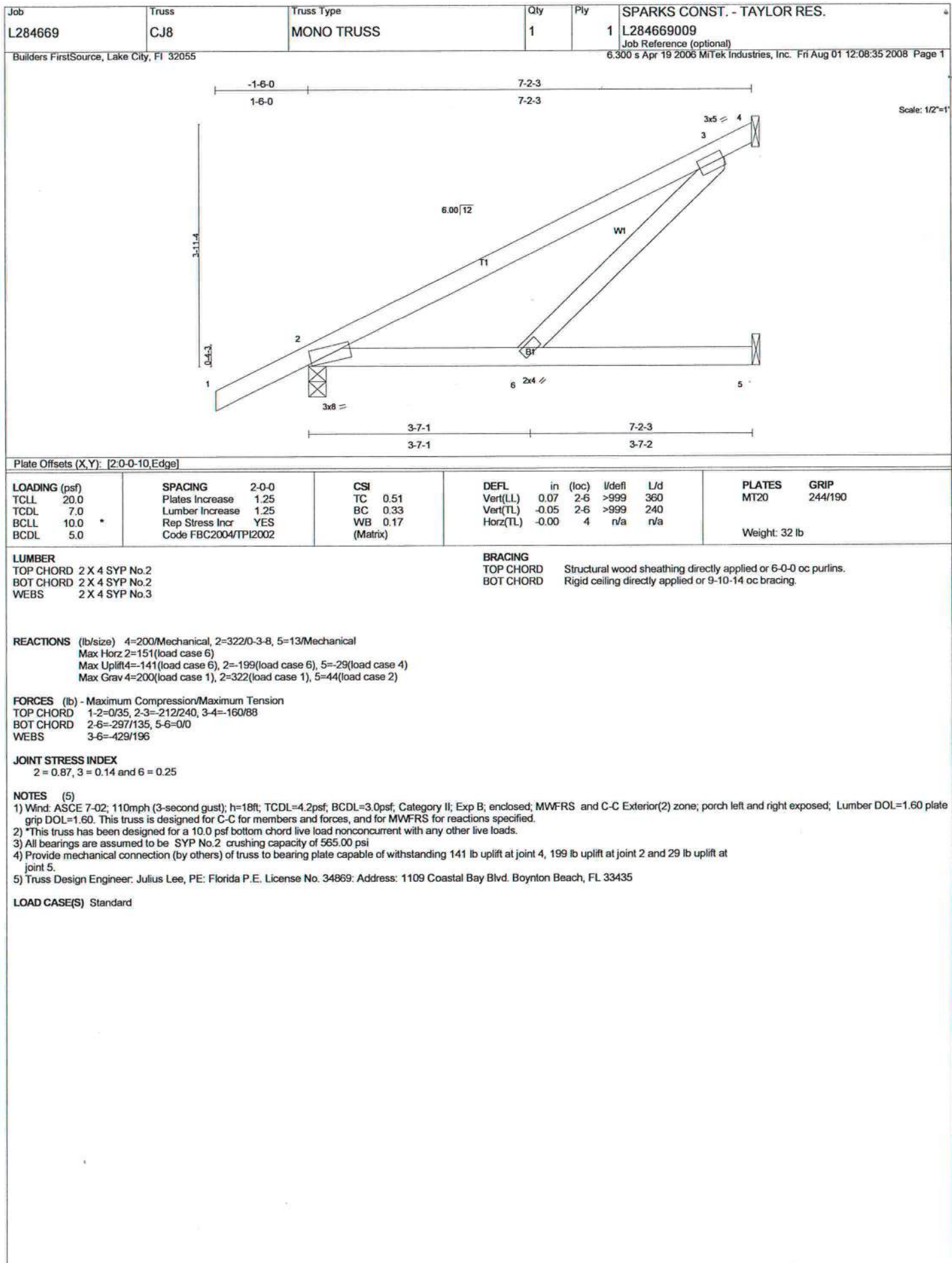
JOINT STRESS INDEX
2 = 0.10 and 2 = 0.00

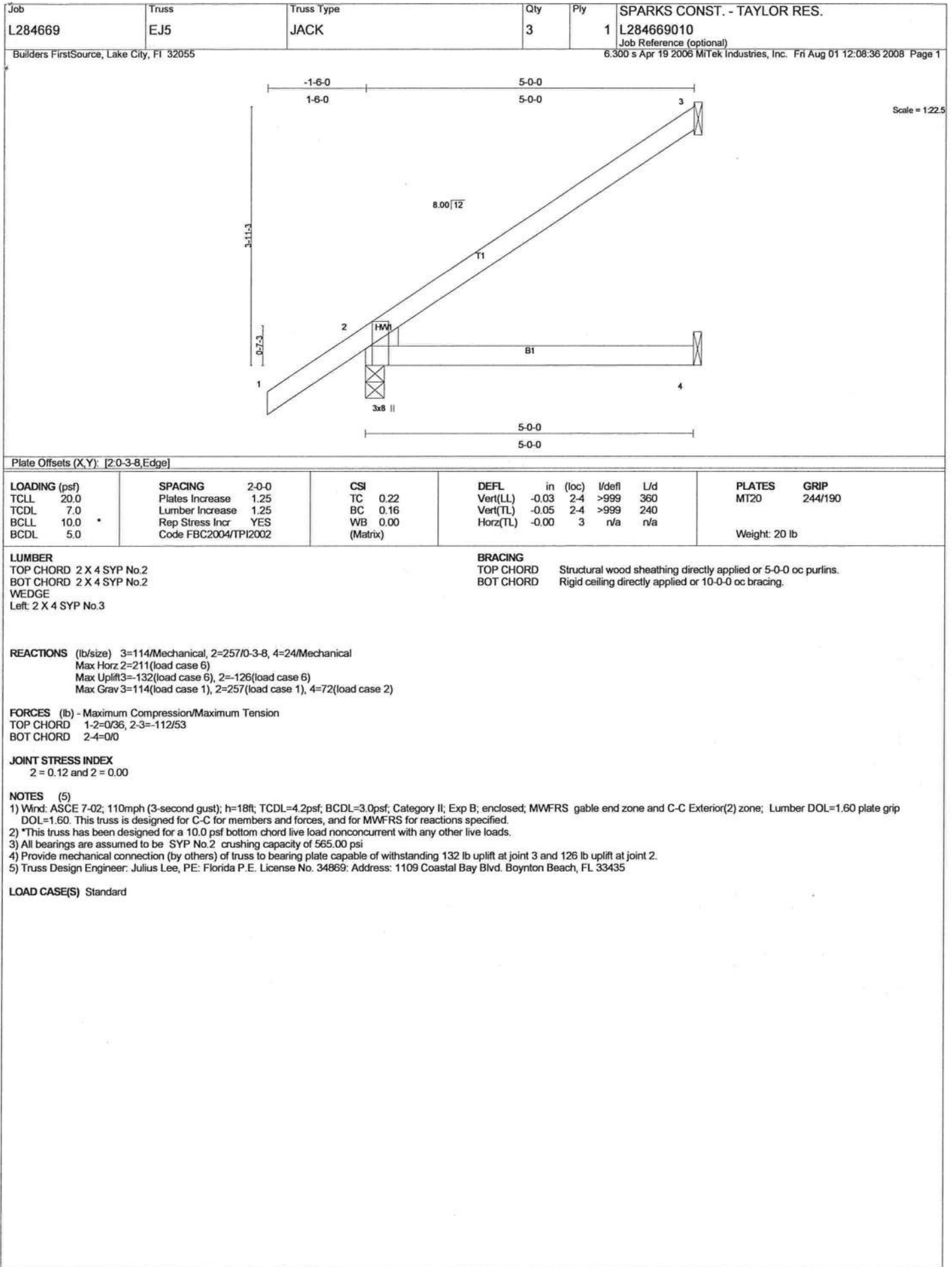
NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 3, 174 lb uplift at joint 2 and 45 lb uplift at joint 4.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard









Job L284669	Truss EJ7	Truss Type MONO TRUSS	Qty 28	Ply 1	SPARKS CONST. - TAYLOR RES. L284669011 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:37 2008 Page 1		

Scale = 1/32.7
Camber = 1/16 in

*** Design Problems ***

REVIEW REQUIRED

Max Deflection In Panel Exceeded: 2-3, 2-6
Max Vertical Deflection Exceeded In Span: 2-6

Plate Offsets (X,Y): [2-0-3-8,Edge]											
LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	2-0-0	Plates Increase	1.25	TC	0.44	in (loc)	l/defl	L/d	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(LL)	0.31	2-6	>257	360	
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Vert(TL)	-0.15	2-6	>527	240	
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)		Horz(TL)	0.00		n/a	n/a	
										Weight: 33 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

REACTIONS (lb/size) 2=310/0-3-8, 6=206/Mechanical

Max Horz 2=194(load case 6)

Max Uplift 2=162(load case 6), 6=192(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/36, 2-3=-151/68, 3-4=-2/0

BOT CHORD 2-6=0/0, 5-6=0/0

WEBS 3-6=-156/200

JOINT STRESS INDEX

2 = 0.62, 2 = 0.00, 3 = 0.10 and 6 = 0.11

NOTES (5)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 162 lb uplift at joint 2 and 192 lb uplift at joint 6.

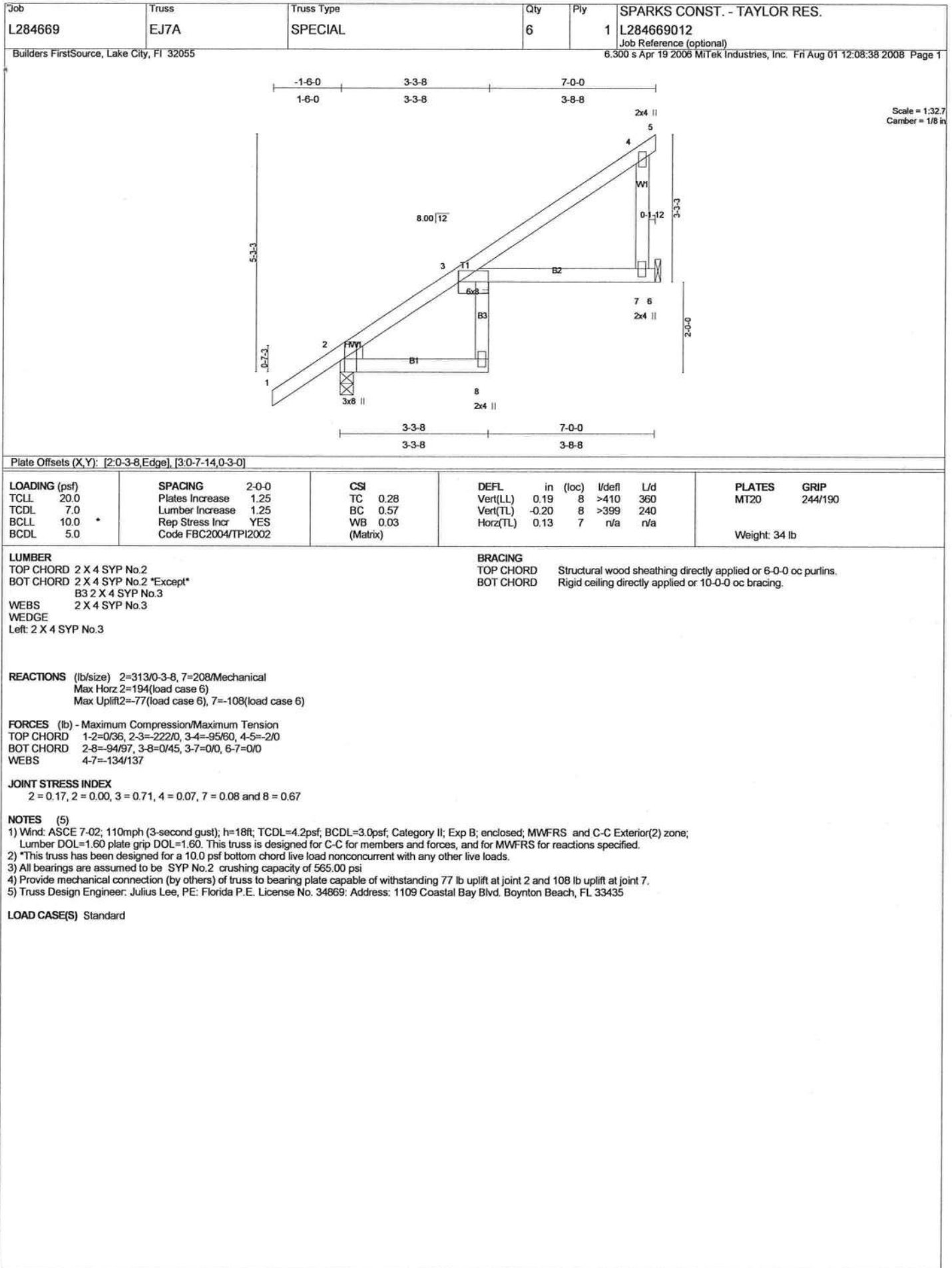
5) Truss Design Engineer, Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

LOAD CASE(S) Standard



Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:39 2008 Page 1



LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2 X 6 SYP No.1D	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3		

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/42, 2-3=-558/231, 3-4=-549/231, 4-5=-125/63, 5-6=-11/0
BOT CHORD	2-10=-497/485, 9-10=-497/485, 8-9=-472/504, 7-8=0/0
WEBS	3-10=54/56, 3-9=-60/45, 4-9=-90/203, 4-8=-604/566, 5-8=-261/251

JOINT STRESS INDEX
2 = 0.75, 3 = 0.05, 4 = 0.29, 5 = 0.14, 8 = 0.30, 9 = 0.15 and 10 = 0.04

NOTES (6-7)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDF=4.2psf, BCDL=3.0psf, Category II, Exp B; enclosed, MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 381 lb uplift at joint 2 and 582 lb uplift at joint 8.
 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 7) Use Simpson LSSU210 to attach Truss to Carrying member

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-4(F=25, B=25)-to-5=-141(F=43, B=43). 5=-101(F=43, B=43)-to-6=-105(F=45, B=45). 2=0(F=5, B=5)-to-7=-27(F=8, B=8)

Job
L284669

Truss
HJ7

Truss Type
JACK

Qty
2

Ply
1

SPARKS CONST. - TAYLOR RES.
L284669014
Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6,300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:40 2008 Page 1

Plate Offsets (X,Y): [2:0-3-8,Edge]	
LOADING (psf)	SPACING 2:0-0
TCLL 20.0	Plates Increase 1.25
TCDL 7.0	Lumber Increase 1.25
BCLL 10.0 *	Rep Stress Incr NO
BCDL 5.0	Code FBC2004/TPI2002
	CSI
	TC 0.45
	BC 0.29
	WB 0.00
	(Matrix)
	DEFL in (loc) l/defl L/d
	Vert(LL) -0.08 2-4 >999 360
	Vert(TL) -0.15 2-4 >551 240
	Horz(TL) -0.00 3 n/a n/a
	PLATES GRIP
	MT20 244/190
	Weight: 27 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEDGE

Left: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)

3=191/Mechanical, 2=284/0-5-11, 4=46/Mechanical

Max Horz 2=195(load case 5)

Max Uplift 3=181(load case 5), 2=160(load case 5)

Max Grav 3=191(load case 1), 2=284(load case 1), 4=99(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/38, 2-3=-109/60

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.32 and 2 = 0.00

NOTES (6)

1) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 3 and 160 lb uplift at joint 2.

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

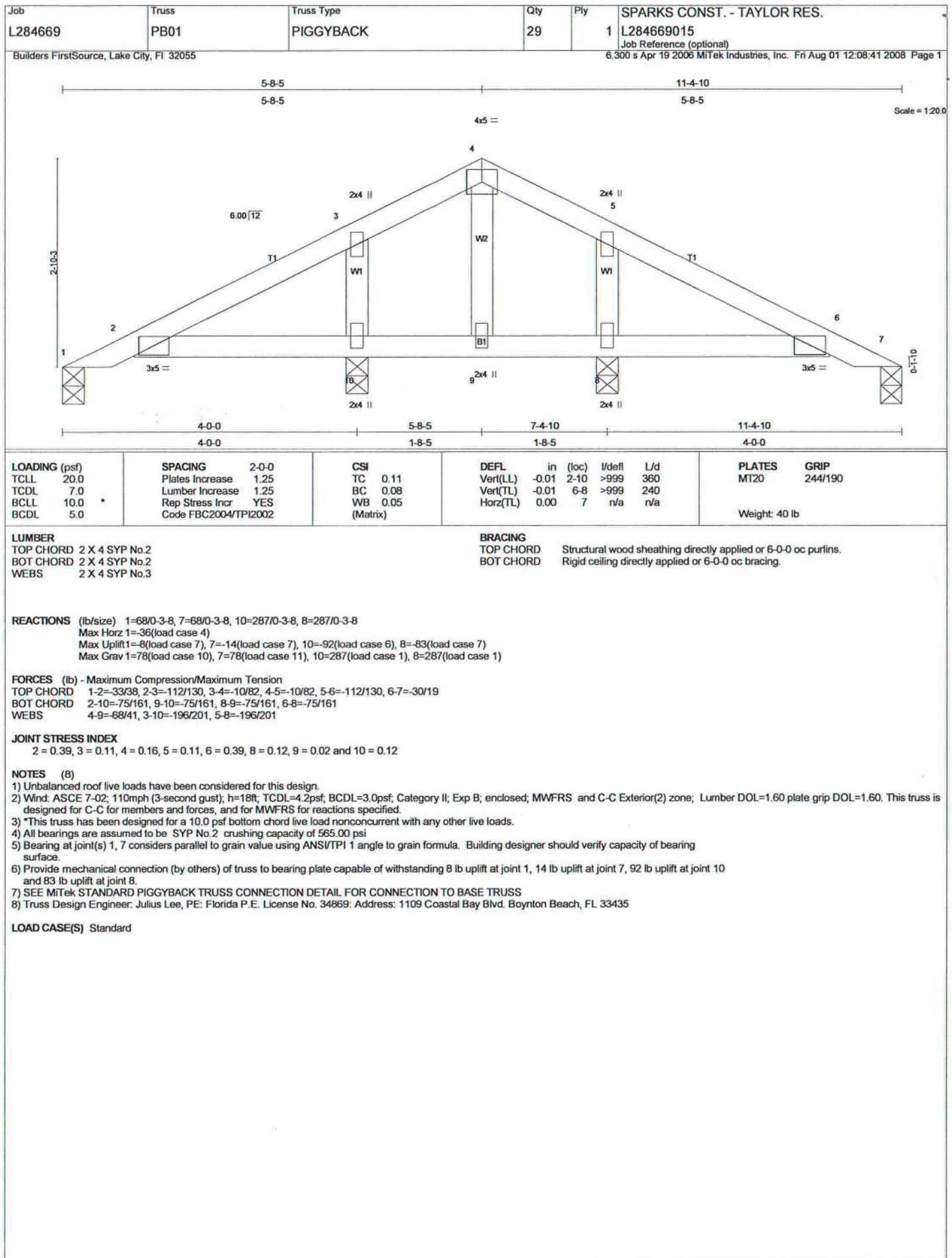
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

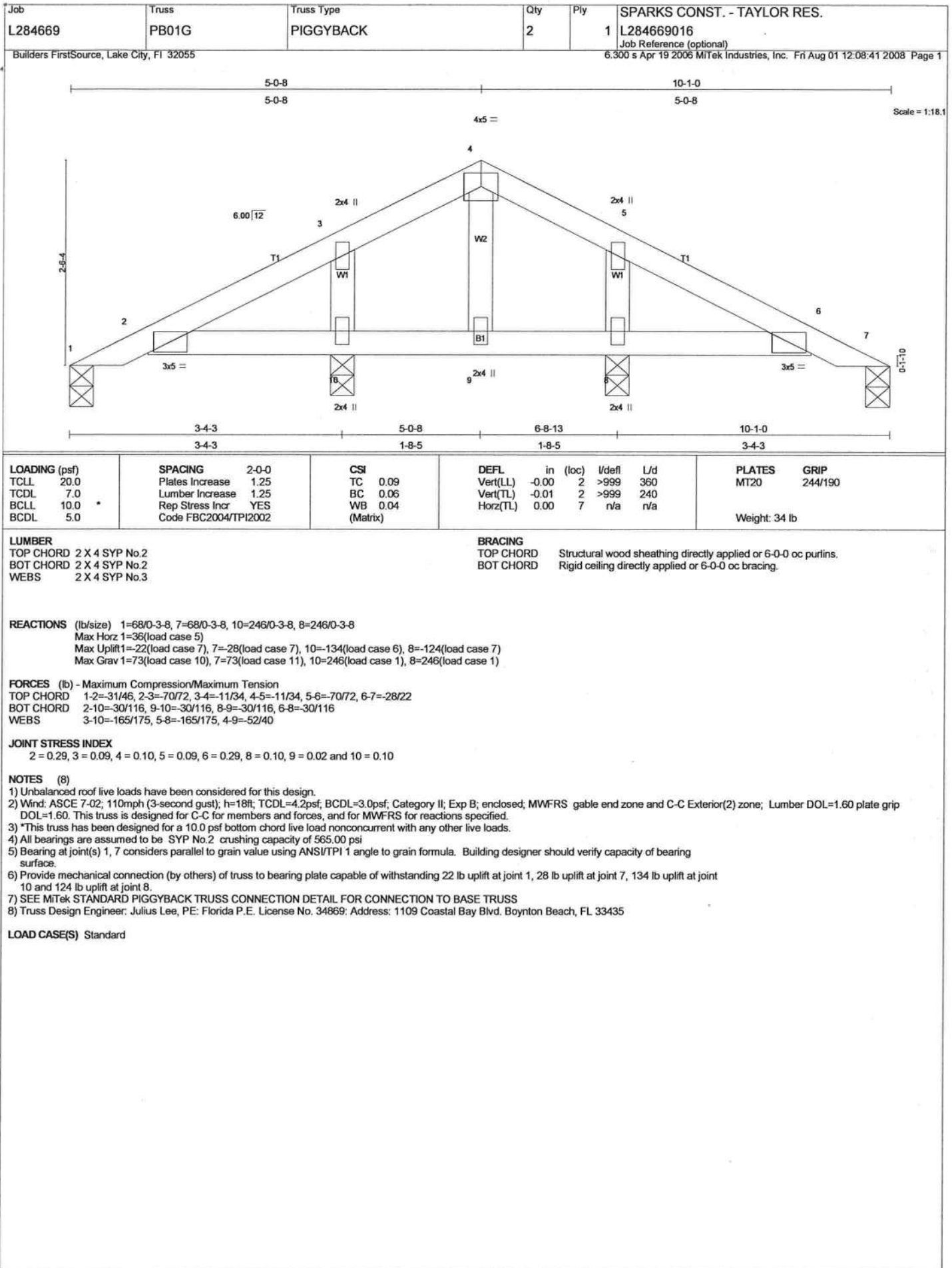
Uniform Loads (plf)

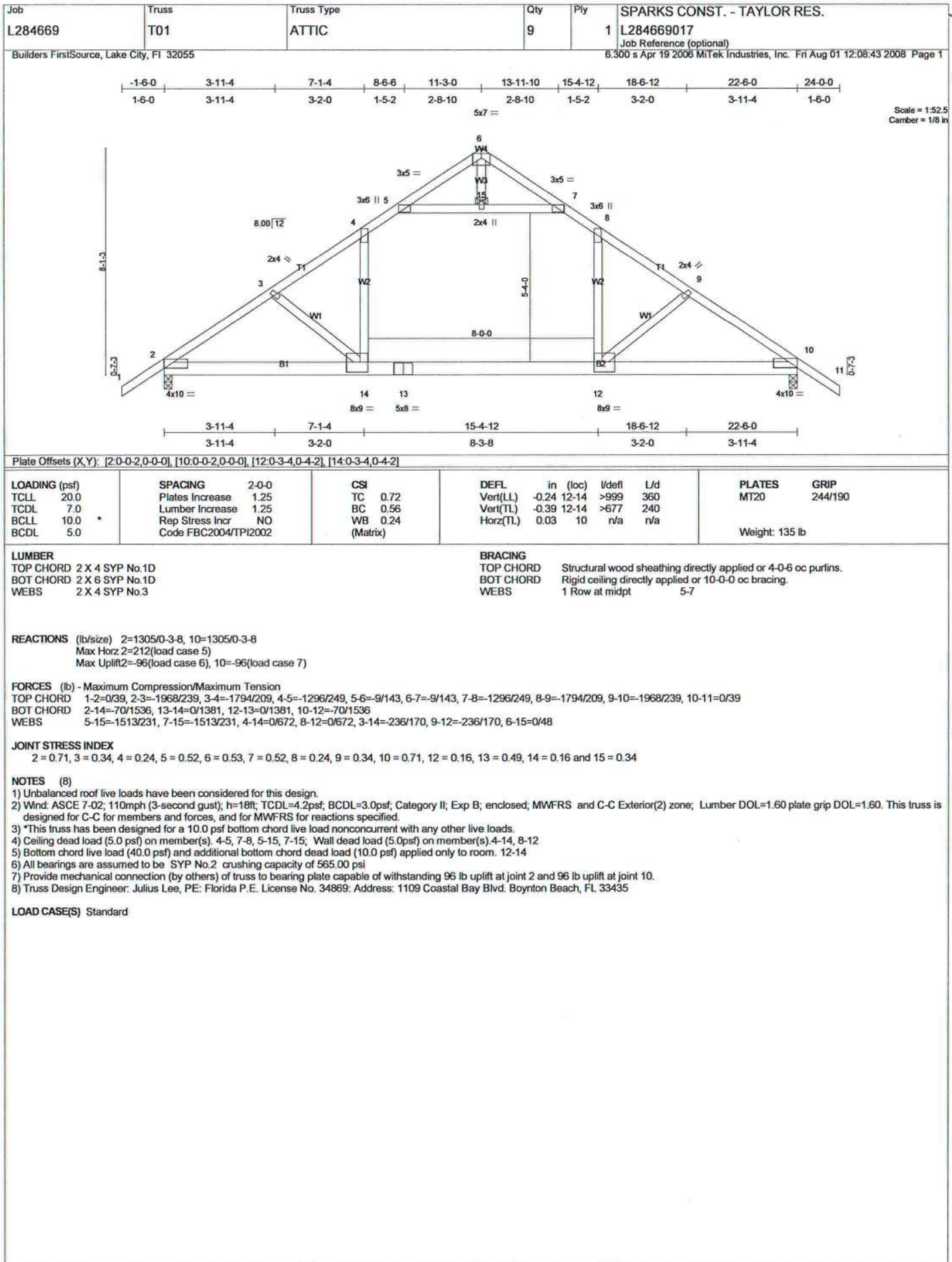
Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-3=-95(F=-21, B=21), 2=-0(F=5, B=5)-to-4=-18(F=-4, B=-4)







Job L284669	Truss T01G	Truss Type GABLE	Qty 1	Ply 1	SPARKS CONST. - TAYLOR RES. L284669018 Job Reference (optional)
----------------	---------------	---------------------	----------	----------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:45 2008 Page 1

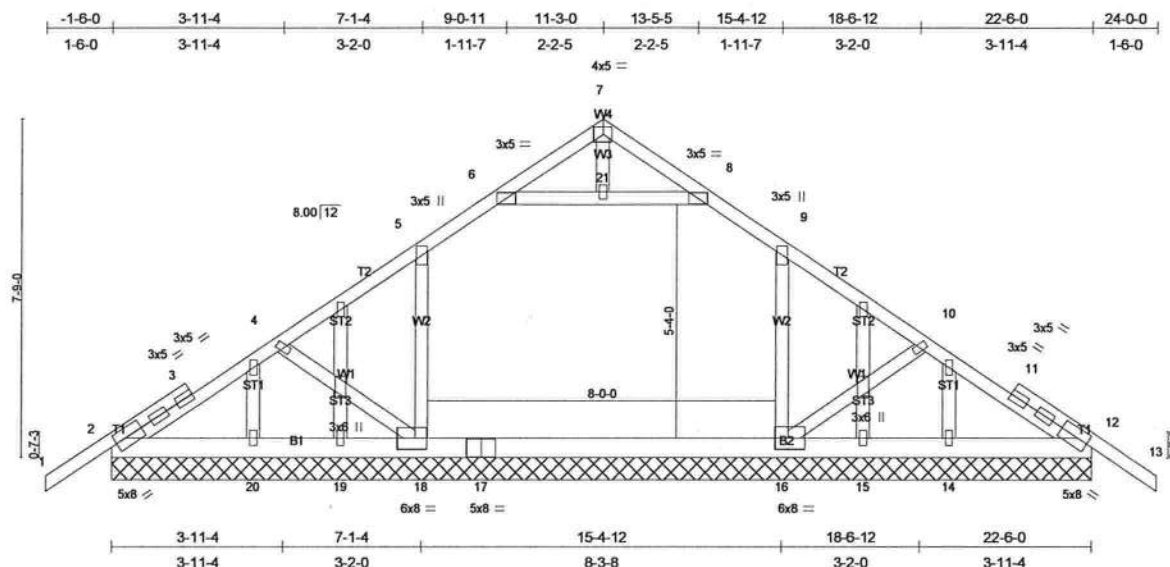


Plate Offsets (X,Y): [2-0-2-13,0-2-8], [12-0-2-13,0-2-8], [16-0-3-9,0-3-0], [18-0-3-6,0-3-2]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.26	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(LL) -0.01 13 n/r 120		
BCLL 10.0	Lumber Increase 1.25	WB 0.15	Vert(TL) 0.00 12 n/r 90		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 12 n/a n/a		
	Code FBC2004/TPI2002			Weight: 150 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=753/22-6-0, 12=753/22-6-0, 18=658/22-6-0, 16=658/22-6-0, 19=51/22-6-0, 20=102/22-6-0, 15=51/22-6-0, 14=102/22-6-0
Max Horz 2=-259(load case 4)
Max Uplift 2=-339(load case 6), 12=-344(load case 7), 18=-294(load case 6), 16=-289(load case 7), 19=-83(load case 2), 20=-7(load case 7), 15=-83(load case 2), 14=-8(load case 6)
Max Grav 2=753(load case 1), 12=753(load case 1), 18=674(load case 11), 16=674(load case 12), 20=140(load case 2), 14=140(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-10/70, 2-3=-886/291, 3-4=-789/277, 4-5=-584/196, 5-6=-595/257, 6-7=-355/142, 7-8=-355/145, 8-9=-595/257, 9-10=-584/187, 10-11=-789/267, 11-12=-886/281, 12-13=-10/70
BOT CHORD 2-20=-204/657, 19-20=-204/657, 18-19=-204/657, 17-18=-78/414, 16-17=-78/414, 15-16=-165/657, 14-15=-165/657, 12-14=-165/657
WEBS 6-21=-142/137, 8-21=-142/137, 5-18=-414/203, 9-16=-414/197, 4-18=-299/238, 10-16=-299/241, 7-21=0/30

JOINT STRESS INDEX
2 = 0.60, 3 = 0.00, 3 = 0.43, 3 = 0.43, 4 = 0.34, 5 = 0.18, 6 = 0.18, 7 = 0.28, 8 = 0.18, 9 = 0.18, 10 = 0.34, 11 = 0.00, 11 = 0.43, 11 = 0.43, 12 = 0.60, 14 = 0.34, 15 = 0.34, 16 = 0.15, 17 = 0.08, 18 = 0.16, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.66, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.66 and 27 = 0.34

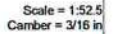
- NOTES** (12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 - *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - Ceiling dead load (5.0 psf) on member(s). 5-6, 8-9, 6-21, 8-21; Wall dead load (5.0psf) on member(s). 5-18, 9-16
 - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 2, 344 lb uplift at joint 12, 294 lb uplift at joint 18, 289 lb uplift at joint 16, 83 lb uplift at joint 19, 7 lb uplift at joint 20, 83 lb uplift at joint 15 and 8 lb uplift at joint 14.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

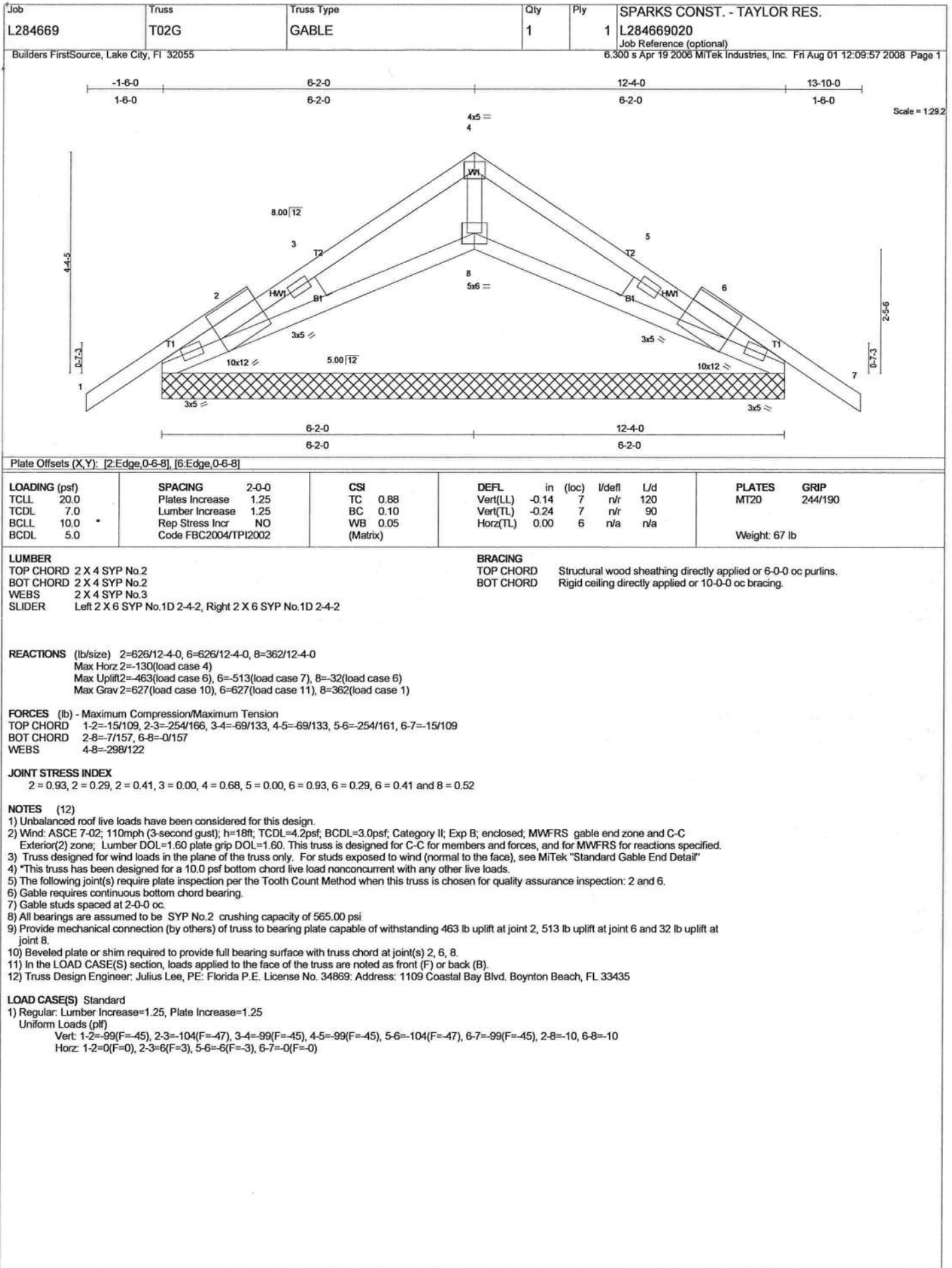
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

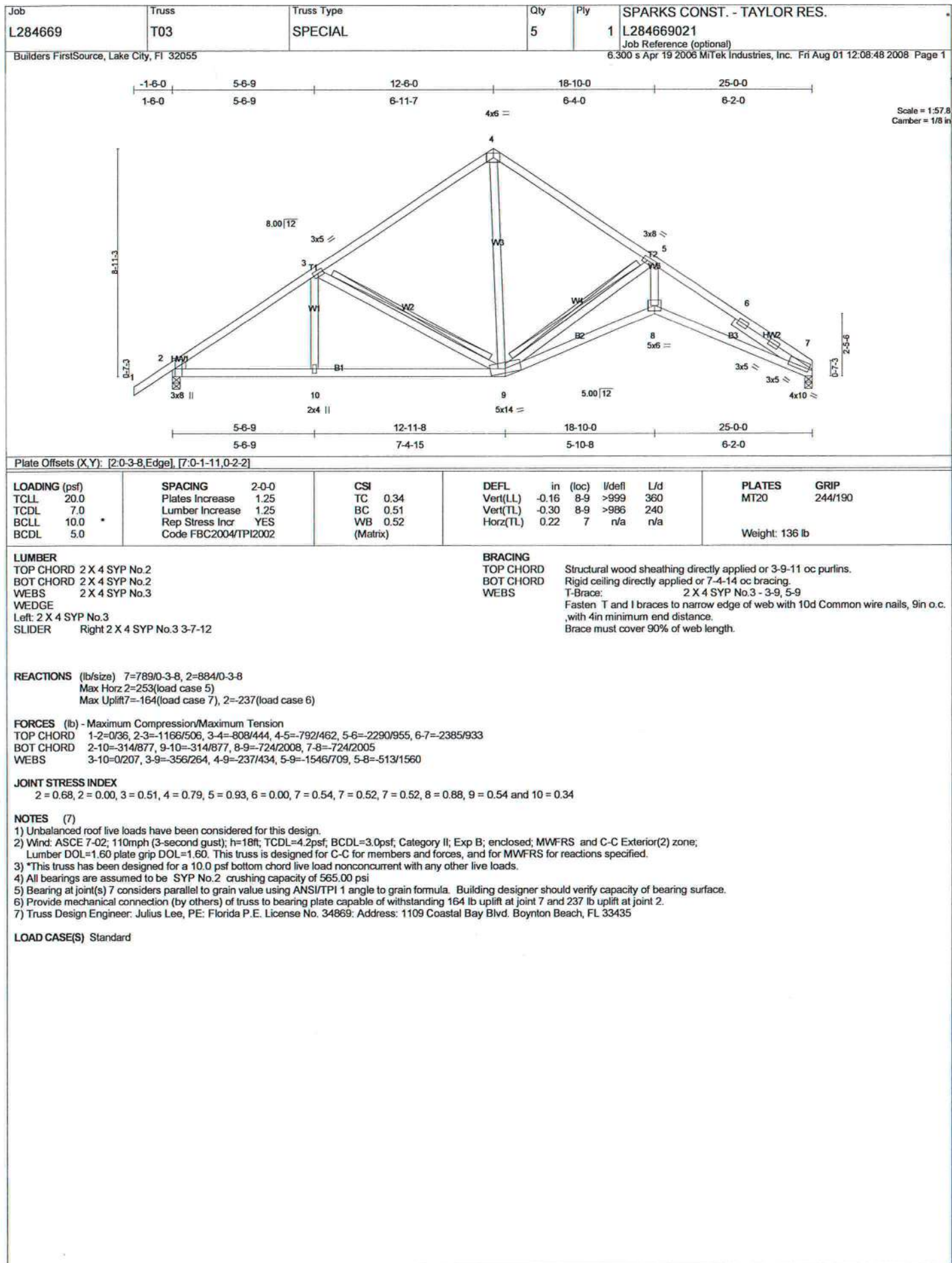
Uniform Loads (plf)

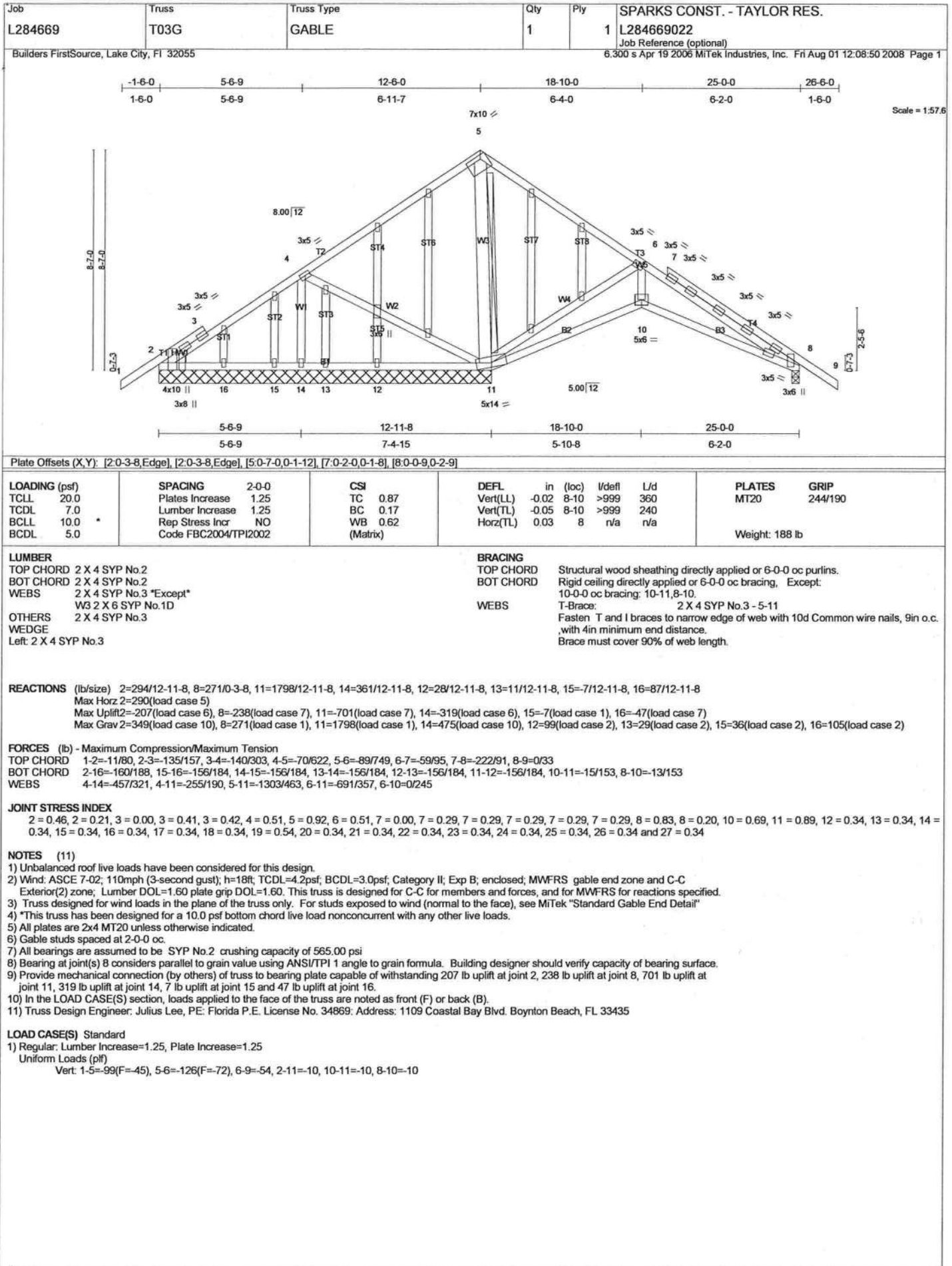
Vert: 2-12=-10, 1-5=-99(F=-45), 5-6=-109(F=-45), 6-7=-99(F=-45), 7-8=-99(F=-45), 8-9=-109(F=-45), 9-13=-99(F=-45), 6-8=-10
Drag: 5-18=-10, 9-16=-10



LOAD CASE(S) Standard







Job L284669	Truss T04	Truss Type COMMON	Qty 1	Ply 2	SPARKS CONST. - TAYLOR RES. L284669023 Job Reference (optional)
----------------	--------------	----------------------	----------	----------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:52 2008 Page 1

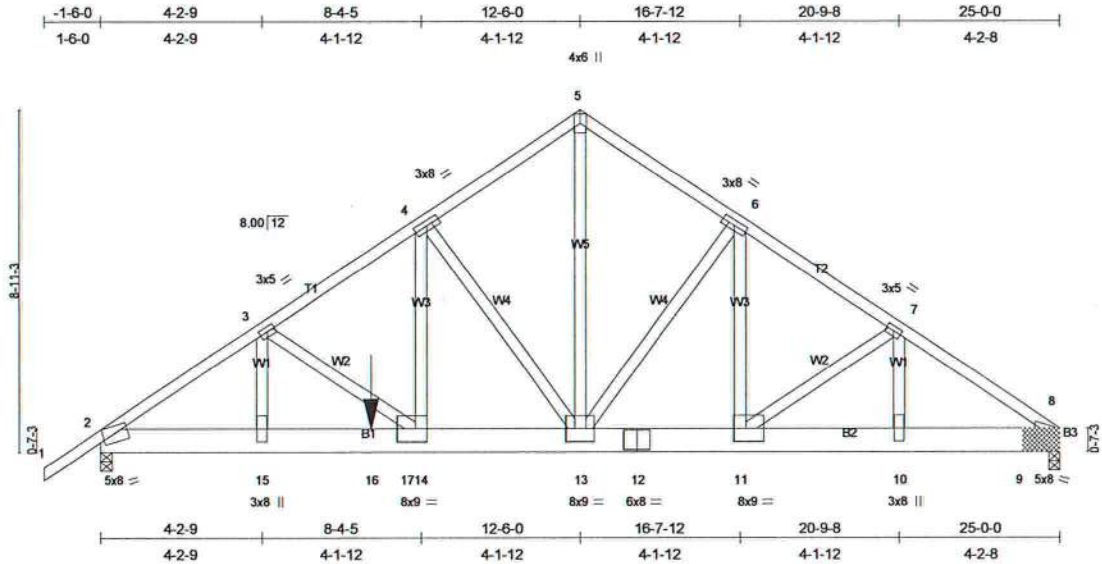


Plate Offsets (X,Y): [2:0-1-3,0-1-2], [8:Edge,0-3-14], [11:0-3-8,0-4-0], [14:0-3-8,0-4-0]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCCL 20.0	Plates Increase 1.25	TC 0.34	Vert(LL)	-0.14	11-13	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.65	Vert(TL)	-0.26	11-13	>999	240		
BCLL 10.0	Rep Stress Incr NO	WB 0.98	Horz(TL)	0.06	8	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
								Weight: 399 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=6346/0-3-12 (0-3-8 + bearing block), 2=5064/0-3-8
Max Horz 2=251(load case 4)
Max Uplift 8=1792(load case 6), 2=1440(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/42, 2-3=-8273/2254, 3-4=-7732/2172, 4-5=-5798/1706, 5-6=-5798/1702, 6-7=-7528/2142, 7-8=-9144/2573
BOT CHORD 2-15=-1872/6659, 15-16=-1872/6659, 16-17=-1872/6659, 14-17=-1872/6659, 13-14=-1760/6380, 12-13=-1674/6208, 11-12=-1674/6208, 10-11=-2035/7384, 9-10=-2035/7384, 8-9=-2035/7384
WEBS 3-15=-112/534, 3-14=-353/138, 4-14=-848/3046, 4-13=-2697/830, 5-13=-1784/6100, 6-13=-2406/781, 6-11=-790/2698, 7-11=-1451/490, 7-10=-504/1774

JOINT STRESS INDEX

2 = 0.93, 3 = 0.80, 4 = 0.91, 5 = 0.73, 6 = 0.91, 7 = 0.80, 8 = 0.85, 8 = 0.00, 8 = 0.00, 9 = 0.00, 9 = 0.00, 9 = 0.00, 10 = 0.29, 11 = 0.31, 12 = 0.59, 13 = 0.57, 14 = 0.31 and 15 = 0.29

NOTES (11)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 2 X 8 SYP No.1D bearing block 12" long at jt. 8 attached to each face with 4 rows of 10d (0.131"x3") nails spaced 3" o.c. 16 Total fasteners per block.
Bearing is assumed to be SYP.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1792 lb uplift at joint 8 and 1440 lb uplift at joint 2.
- Girder carries tie-in span(s): 29-8-8 from 8-0-0 to 25-0-0
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2268 lb down and 627 lb up at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-8=-54, 2-17=-10, 8-17=-453(F=443)
Concentrated Loads (lb)
Vert: 16=-2268(F)

Job L284669	Truss T05	Truss Type HIP	Qty 1	Ply 1	SPARKS CONST. - TAYLOR RES. L284669024 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:53 2008 Page 1		

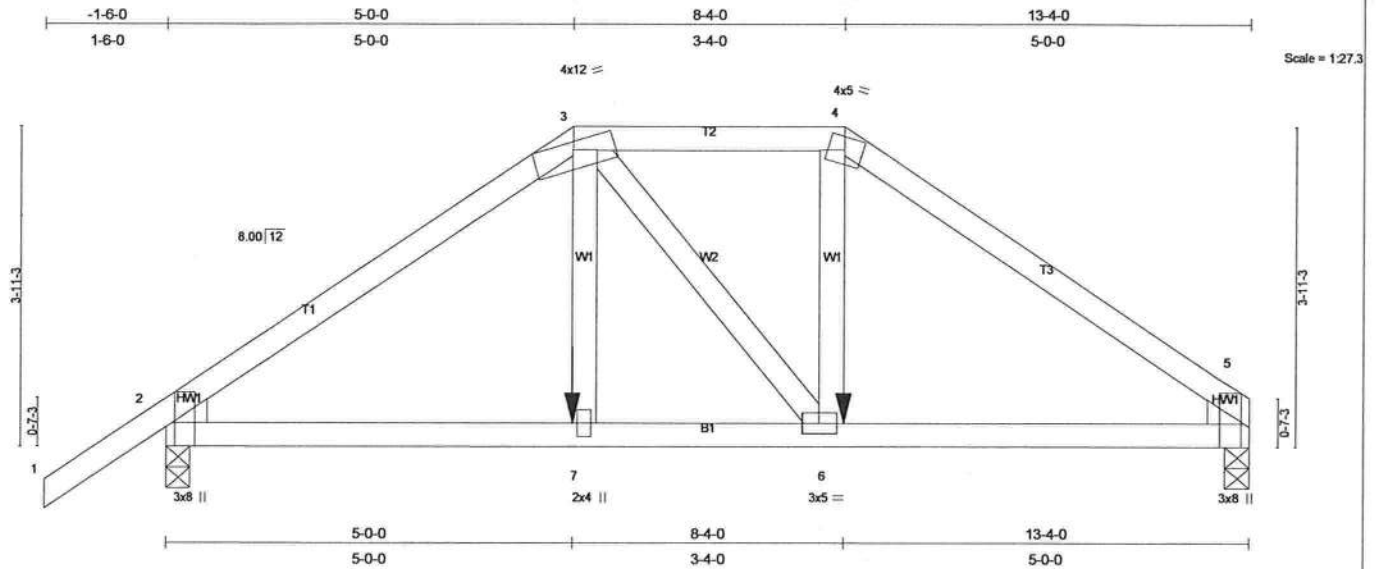


Plate Offsets (X,Y): [2-0-3-8,Edge], [5-0-3-8,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.30	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.24	Vert(LL) -0.02 5-6 >999 360		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.09	Vert(TL) -0.04 5-6 >999 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.02 5 n/a n/a		
	Code FBC2004/TP12002			Weight: 64 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)

2=770/0-3-8, 5=670/0-3-8
Max Horz 2=114(load case 4)
Max Uplift 2=-317(load case 5), 5=-241(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/36, 2-3=-966/360, 3-4=-735/347, 4-5=-976/367
BOT CHORD 2-7=-325/712, 6-7=-329/722, 5-6=-267/725
WEBS 3-7=-96/242, 3-6=-97/30, 4-6=-140/288

JOINT STRESS INDEX

2 = 0.57, 2 = 0.00, 3 = 0.54, 4 = 0.53, 5 = 0.56, 5 = 0.00, 6 = 0.22 and 7 = 0.18

NOTES (10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02, 110mph (3-second gust); h=18ft; TCDL=4.2psf, BCDL=3.0psf, Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 317 lb uplift at joint 2 and 241 lb uplift at joint 5.
- Girder carries hip end with 5-0-0 end setback.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 187 lb down and 117 lb up at 8-4-0, and 187 lb down and 117 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869. Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-91(F=-37), 4-5=-54, 2-7=-10, 6-7=-17(F=-7), 5-6=-10
Concentrated Loads (lb)
Vert: 7=-187(F) 6=-187(F)

Job L284669	Truss T06	Truss Type COMMON	Qty 3	Ply 1	SPARKS CONST. - TAYLOR RES. L284669025 Job Reference (optional)
-----------------------	---------------------	-----------------------------	-----------------	-----------------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:08:53 2008 Page 1

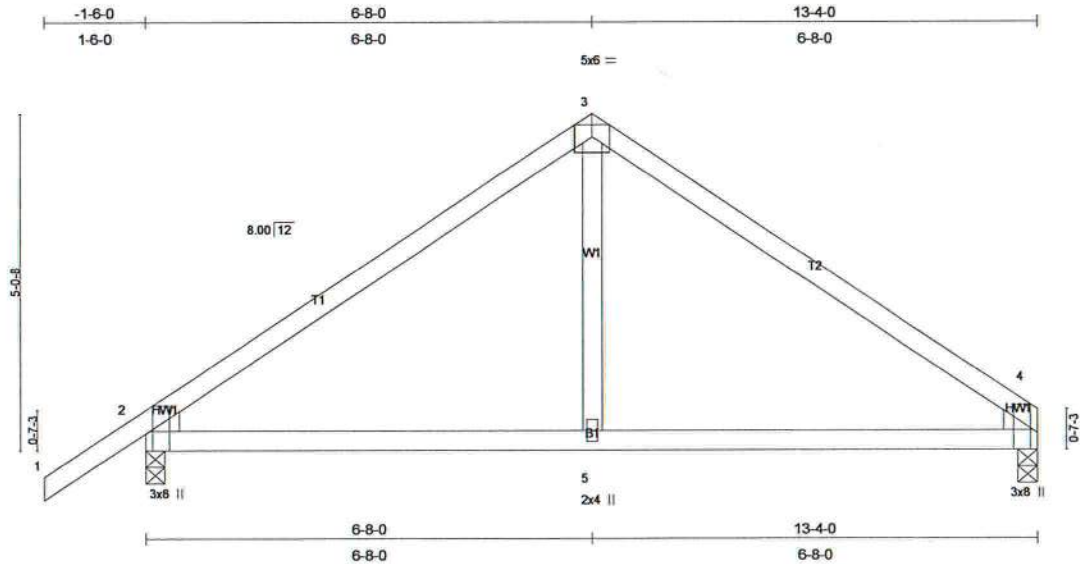


Plate Offsets (X,Y): [2'-0-3-8,Edge], [4'-0-3-8,Edge]

LOADING (psf)	SPACING	2'-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.05	4-5	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.33	Vert(TL)	-0.09	4-5	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
									Weight: 55 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3
 WEDGE
 Left: 2 X 4 SYP No.3, Right: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS

(lb/size) 2=512/0-3-8, 4=412/0-3-8
 Max Horz 2=145(load case 5)
 Max Uplift 2=161(load case 6), 4=85(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/36, 2-3=-522/236, 3-4=-519/232
 BOT CHORD 2-5=-77/347, 4-5=-77/347
 WEBS 3-5=0/231

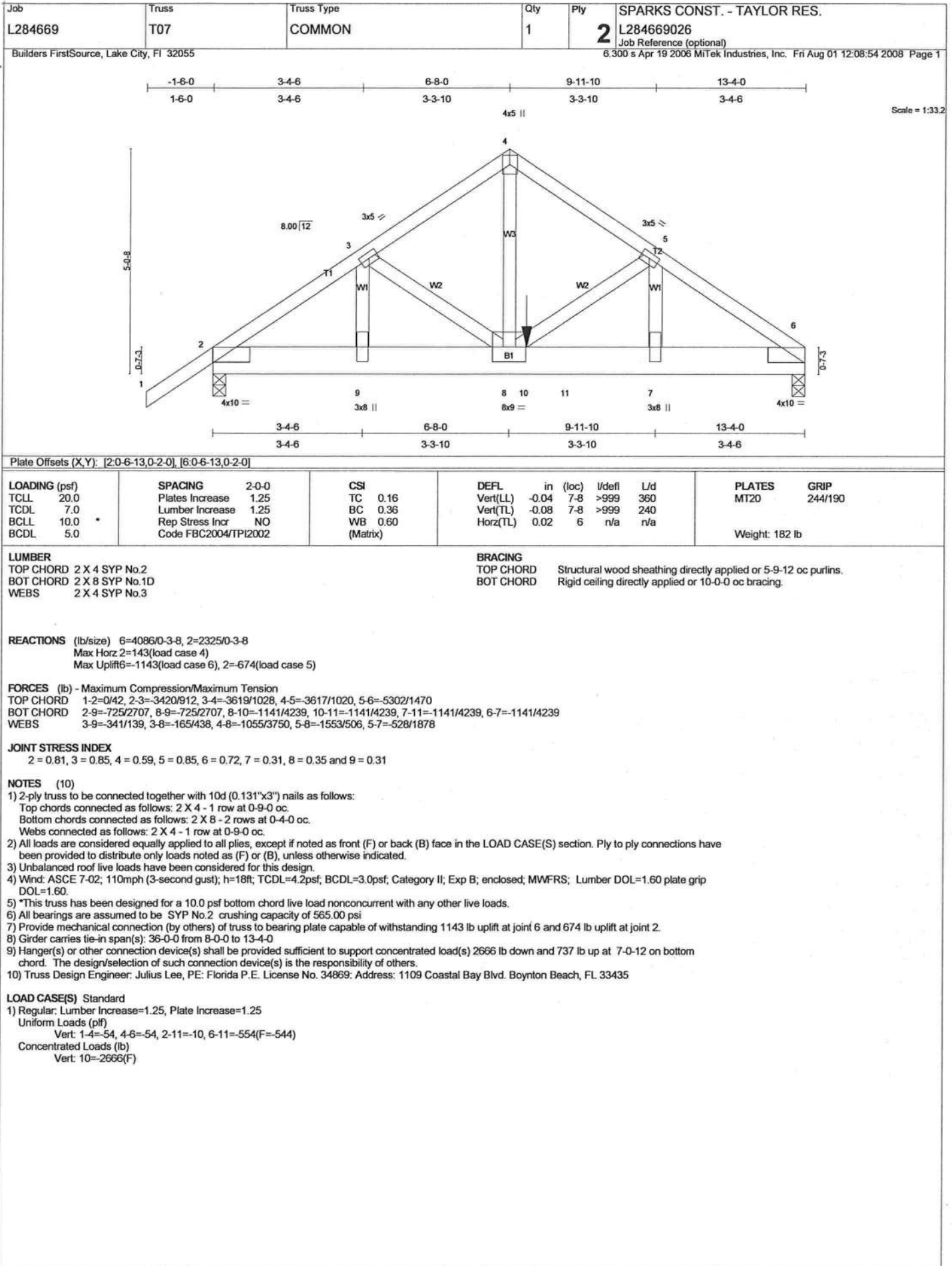
JOINT STRESS INDEX

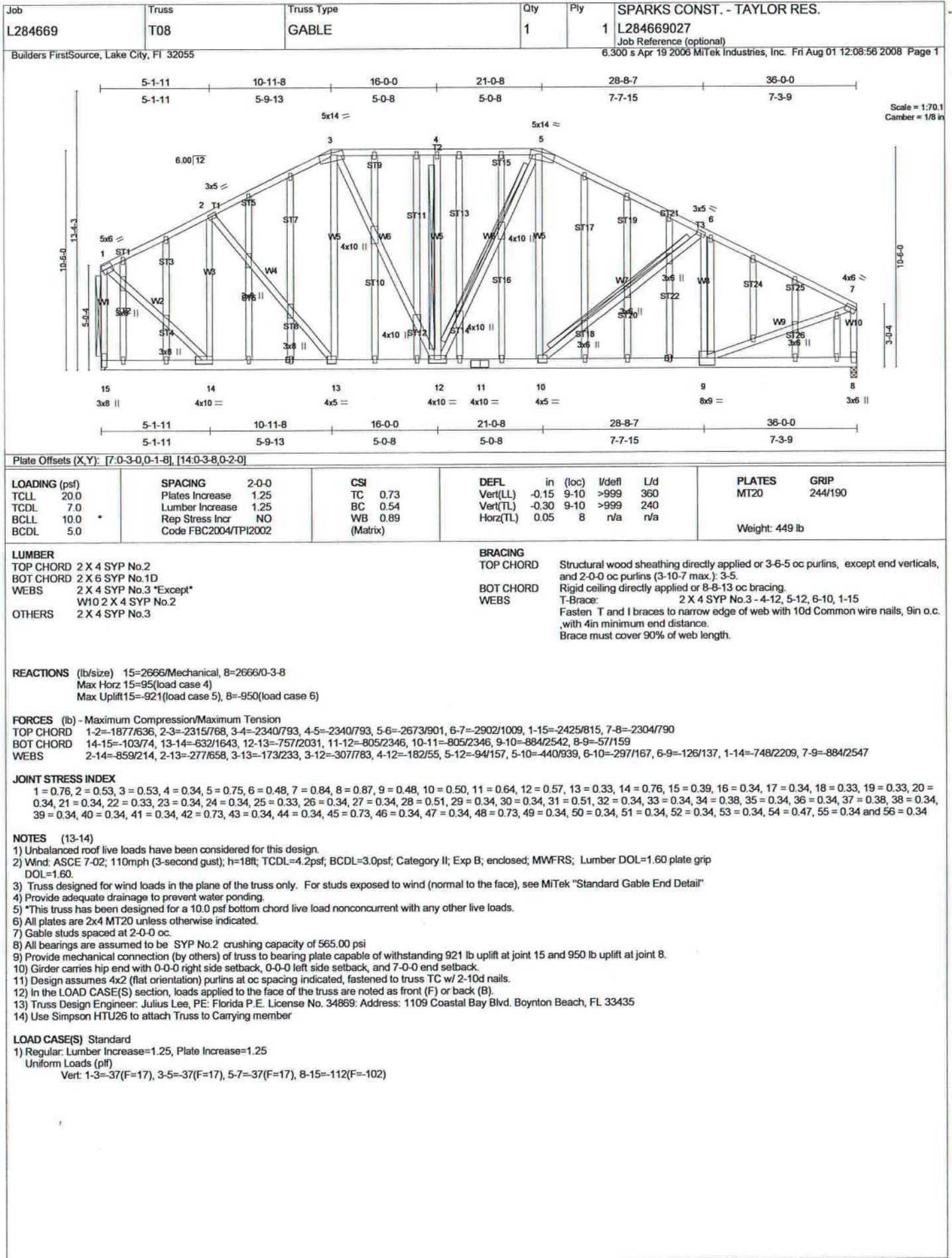
2 = 0.65, 2 = 0.00, 3 = 0.69, 4 = 0.65, 4 = 0.00 and 5 = 0.17

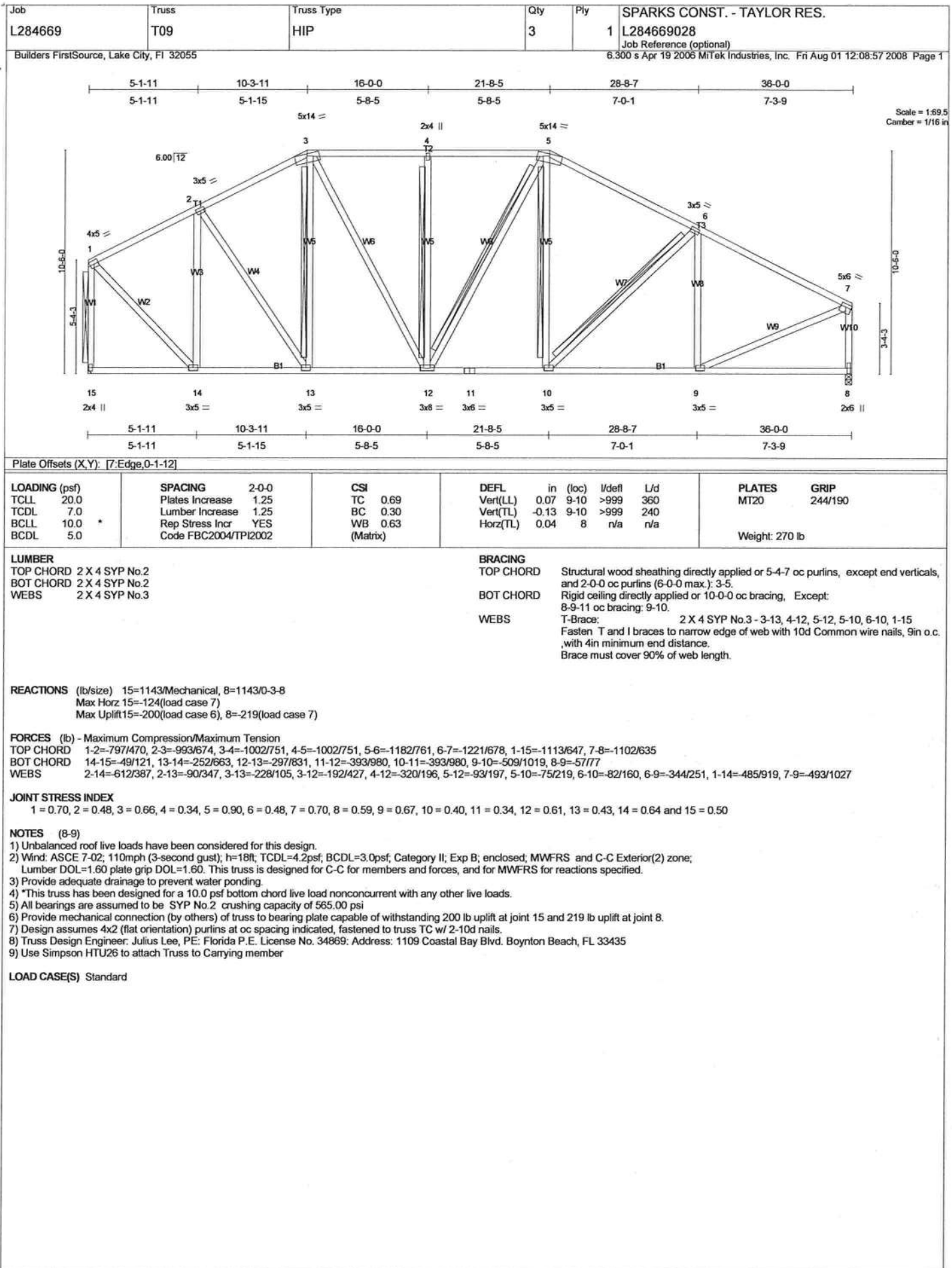
NOTES (6)

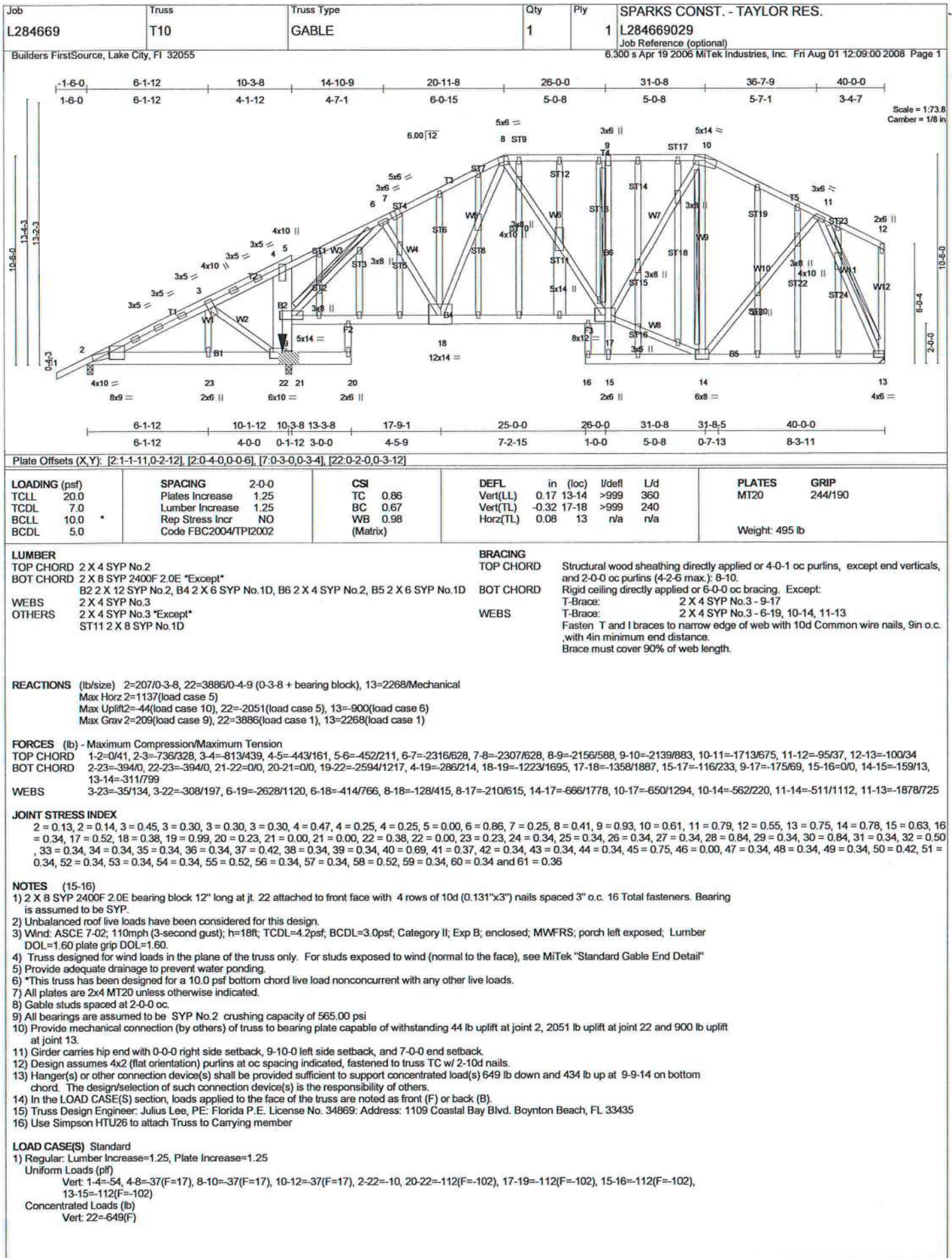
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 2 and 85 lb uplift at joint 4.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

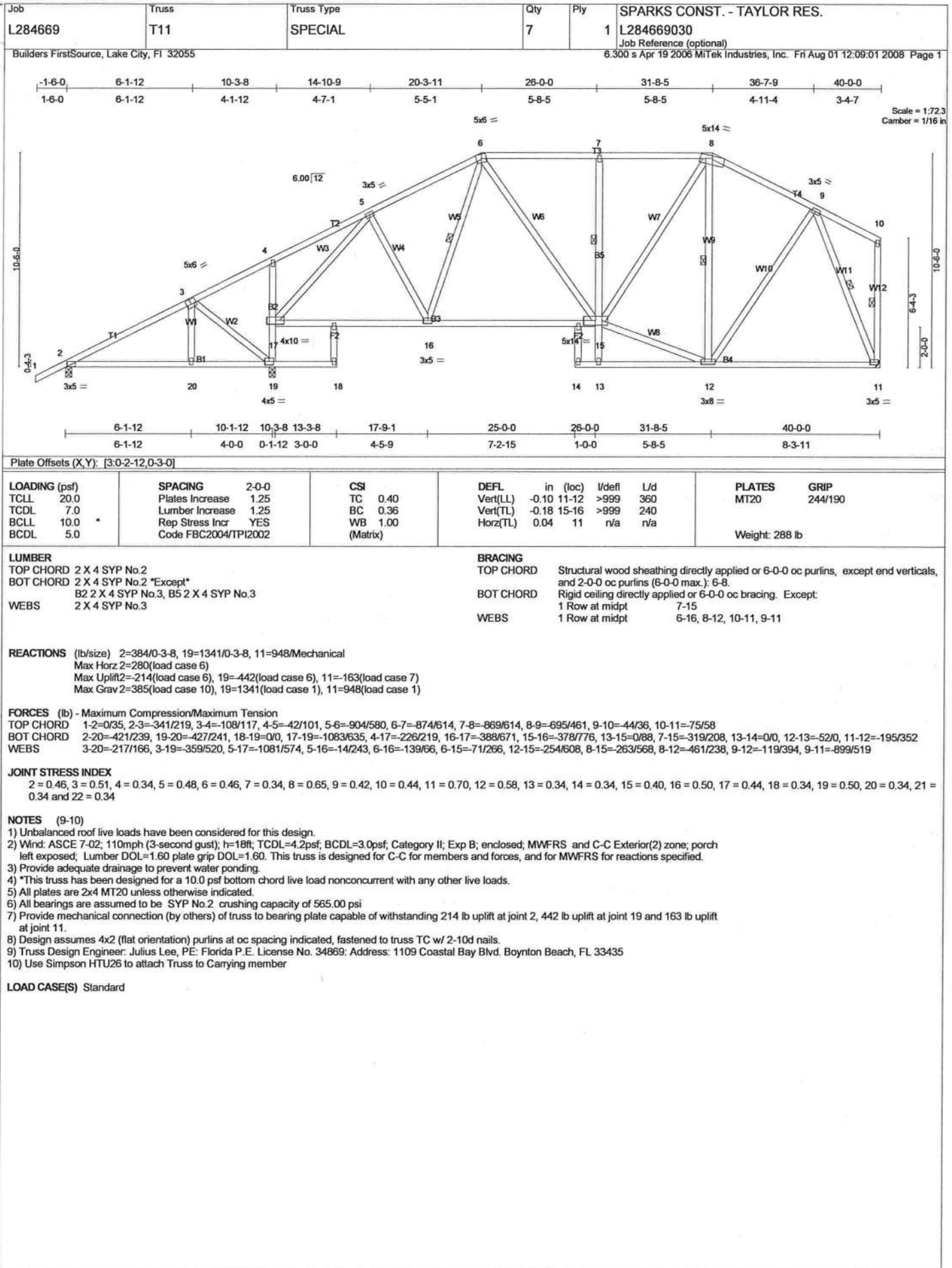
LOAD CASE(S) Standard











Job L284669	Truss T12	Truss Type HIP	Qty 1	Ply 1	SPARKS CONST. - TAYLOR RES. L284669031 Job Reference (optional)
-----------------------	---------------------	--------------------------	-----------------	-----------------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MilTek Industries, Inc. Fri Aug 01 12:09:02 2008 Page 1

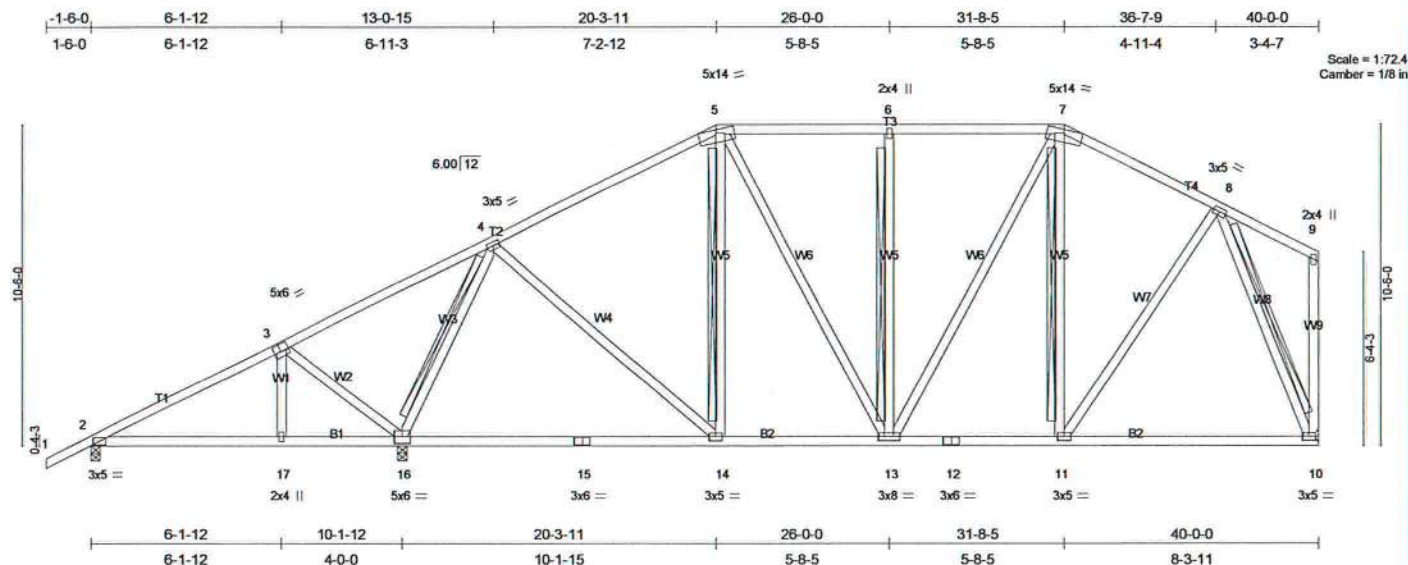


Plate Offsets (X,Y): [3-0-2-12,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.41	Vert(LL)	-0.20	14-16	>999	360	
TCDL 7.0	Plates Increase 1.25	BC 0.49	Vert(TL)	-0.36	14-16	>991	240	
BCLL 10.0	Lumber Increase 1.25	WB 0.38	Horz(TL)	0.02	10	n/a	n/a	
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2004/TPI2002							
							Weight: 274 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-16, 5-14, 6-13, 7-11, 8-10
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS

(lb/size) 2=312/0-3-8, 16=1402/0-3-8, 10=916/Mechanical
Max Horz 2=280(load case 6)
Max Uplift 2=180(load case 6), 16=507(load case 6), 10=152(load case 7)
Max Grav 2=317(load case 10), 16=1402(load case 1), 10=916(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-202/71, 3-4=-303/319, 4-5=-746/434, 5-6=-654/504, 6-7=-654/504, 7-8=-666/440, 8-9=-45/37, 9-10=-76/59
BOT CHORD 2-17=-280/122, 16-17=-288/123, 15-16=-138/271, 14-15=-138/271, 13-14=-254/583, 12-13=-241/543, 11-12=-241/543, 10-11=-185/339
WEBS 3-17=-221/105, 3-16=-403/597, 4-16=-1176/809, 4-14=-159/422, 5-14=-189/163, 5-13=-104/233, 6-13=-319/192, 7-13=-112/291, 7-11=-231/136, 8-11=-105/375, 8-10=-863/492

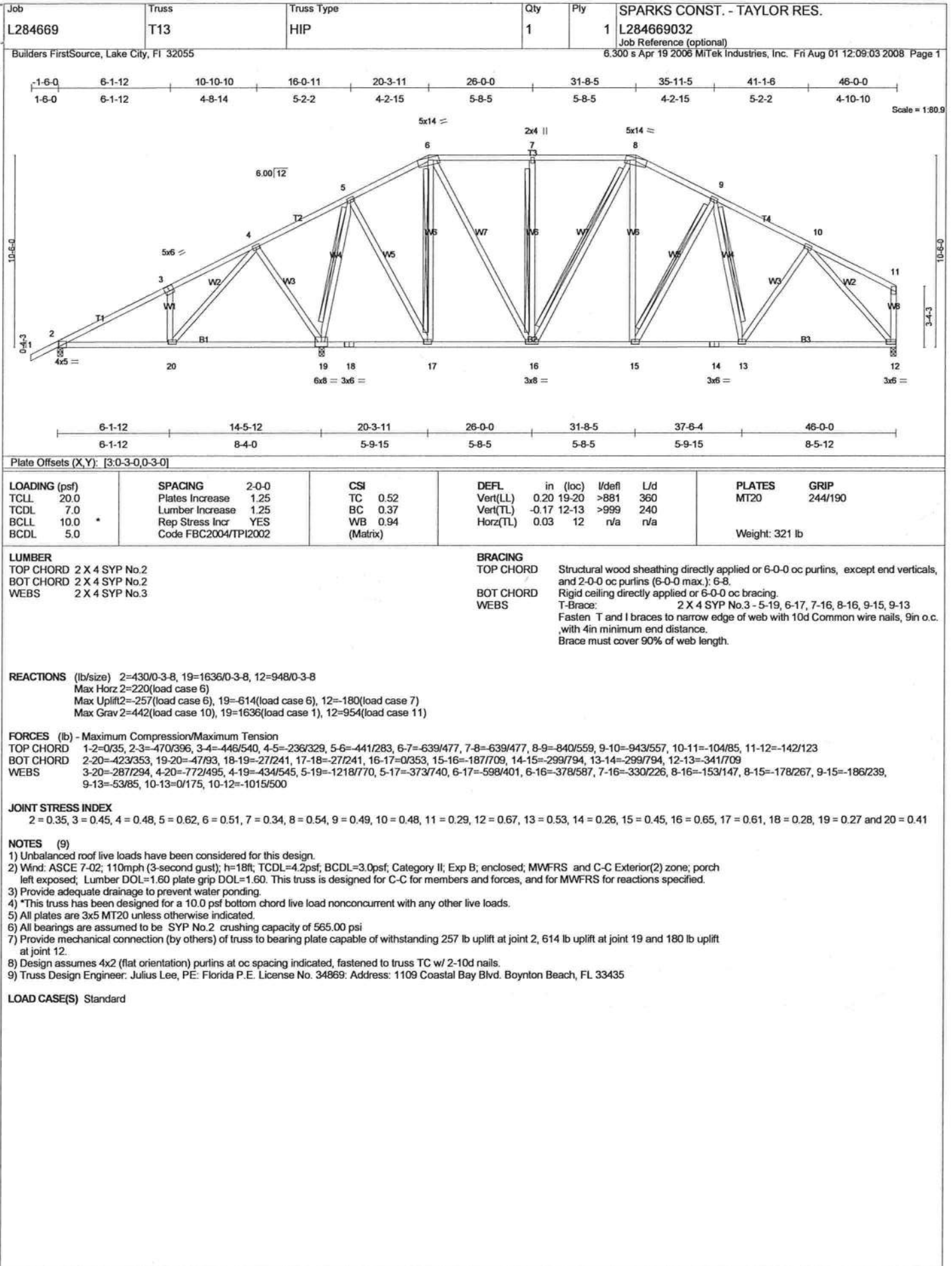
JOINT STRESS INDEX

2 = 0.44, 3 = 0.70, 4 = 0.51, 5 = 0.96, 6 = 0.34, 7 = 0.69, 8 = 0.42, 9 = 0.43, 10 = 0.69, 11 = 0.43, 12 = 0.21, 13 = 0.61, 14 = 0.40, 15 = 0.62, 16 = 0.28 and 17 = 0.34

NOTES (8-9)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 2, 507 lb uplift at joint 16 and 152 lb uplift at joint 10.
- Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	SPARKS CONST. - TAYLOR RES.
L284669	T14	SPECIAL	1	1	L284669033 Job Reference (optional)

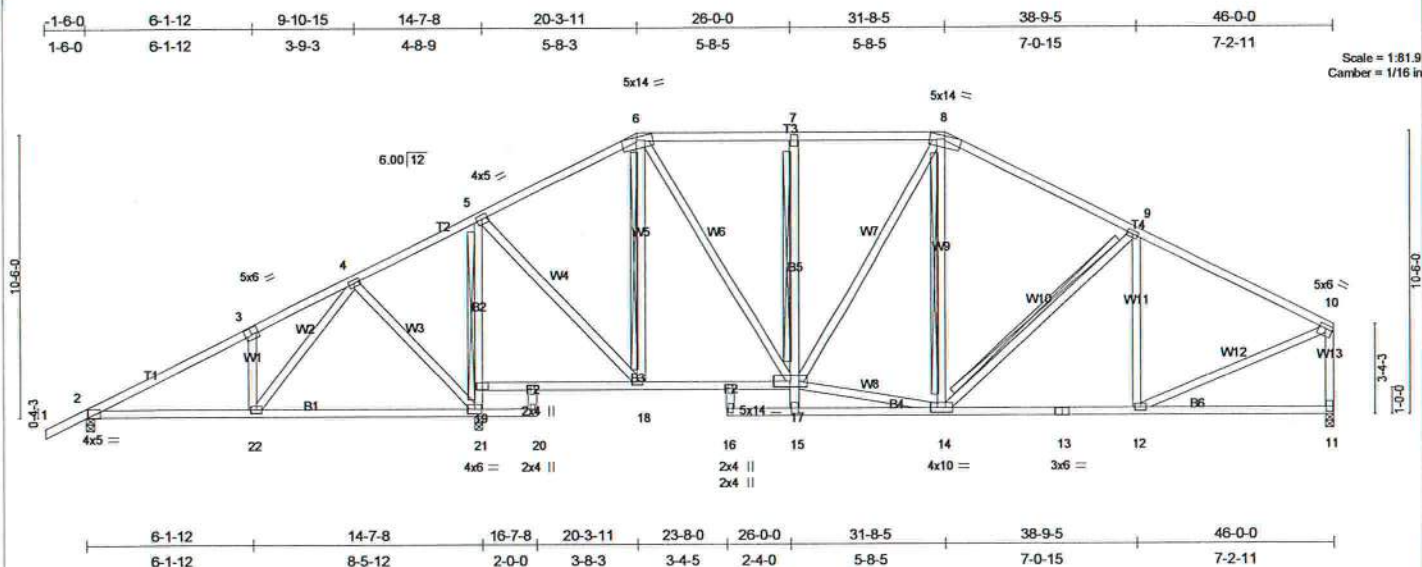


Plate Offsets (X,Y): [3.0-3.0,0-3.0], [10:Edge,0-1-12]										
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl l/d				PLATES GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.63	Vert(LL)	0.21 21-22	>835	360	MT20 244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.15 21-22	>999	240	
BCLL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.02 11	n/a	n/a	
BCDL	5.0	Code FBC2004/TP12002		(Matrix)						
									Weight: 320 lb	

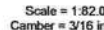
LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=504/0-3-8, 21=1566/0-3-8, 11=991/0-3-8
 Max Horz 2=220(load case 6)
 Max Uplift2=-359(load case 6), 21=-443(load case 6), 11=-226(load case 7)
 Max Grav 2=509(load case 10), 21=1566(load case 1), 11=991(load case 1)

JOINT STRESS INDEX
2 = 0.40, 3 = 0.45, 4 = 0.48, 5 = 0.43, 6 = 0.77, 7 = 0.18, 8 = 0.91, 9 = 0.48, 10 = 0.68, 11 = 0.36, 12 = 0.56, 13 = 0.34, 14 = 0.33, 15 = 0.18, 16 = 0.34, 17 = 0.34, 18 = 0.58, 19 = 0.49, 20 = 0.34, 21 = 0.79, 22 = 0.41, 23 = 0.34 and 24 = 0.34

LOAD CASE(S) Standard

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:09:06 2008 Page 1



Weight: 330 lb

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-5-5 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-8.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing. Except: T-Brace: 2 X 4 SYP No.3 - 5-18 1 Row at midpt 14-16
WEBS	T-Brace: 2 X 4 SYP No.3 - 6-17, 7-16, 8-16, 9-15 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length. 1 Brace at Jt(s): 14
JOINTS	

Max Uplift2=-359(load case 6), 11=-213(load case 7), 20=-446(load case 6)
Max Grav 2=506(load case 10), 11=1010(load case 1), 20=1568(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/35, 2-3=609/824, 3-4=577/955, 4-5=0/205, 5-6=614/536, 6-7=772/671, 7-8=772/671, 8-9=1057/698, 9-10=1225/655, 10-11=994/558
 BOT CHORD 2-21=801/476, 20-21=371/196, 19-20=0/0, 18-20=1220/554, 5-18=1181/557, 17-18=86/0, 16-17=179/478, 15-16=347/862, 14-15=513/1048, 12-14=0/232, 9-14=192/232, 12-13=0/0, 11-12=212/0
 WFRS 3-21=268/271, 4-21=702/454, 4-20=392/505, 5-17=236/830, 6-17=553/226, 6-16=249/586, 7-16=313/178, 8-16=181/69, 8-15=68/348, 9-15=246/201, 10-14=487/1032, 11-14=0/251

JOINT STRESS INDEX
2 = 0.39, 3 = 0.45, 4 = 0.48, 5 = 0.43, 6 = 0.76, 7 = 0.34, 8 = 0.72, 9 = 0.40, 10 = 0.71, 11 = 0.37, 12 = 0.60, 13 = 0.34, 14 = 0.95, 15 = 0.40, 16 = 0.40, 17 = 0.58, 18 = 0.52, 19 = 0.34, 20 = 0.79, 21 = 0.41, 22 = 0.34, 23 = 0.34, 24 = 0.34, 25 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34 and 29 = 0.34

NOTES (9)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 359 lb uplift at joint 2, 213 lb uplift at joint 11 and 446 lb uplift at joint 20.
- 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 9) Truss Design Engineer: Julius Lee, P.E.; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L284669	Truss T17	Truss Type SPECIAL	Qty 1	Ply 1	SPARKS CONST. - TAYLOR RES. L284669036 Job Reference (optional)
----------------	--------------	-----------------------	----------	----------	---

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:09:09 2008 Page 1

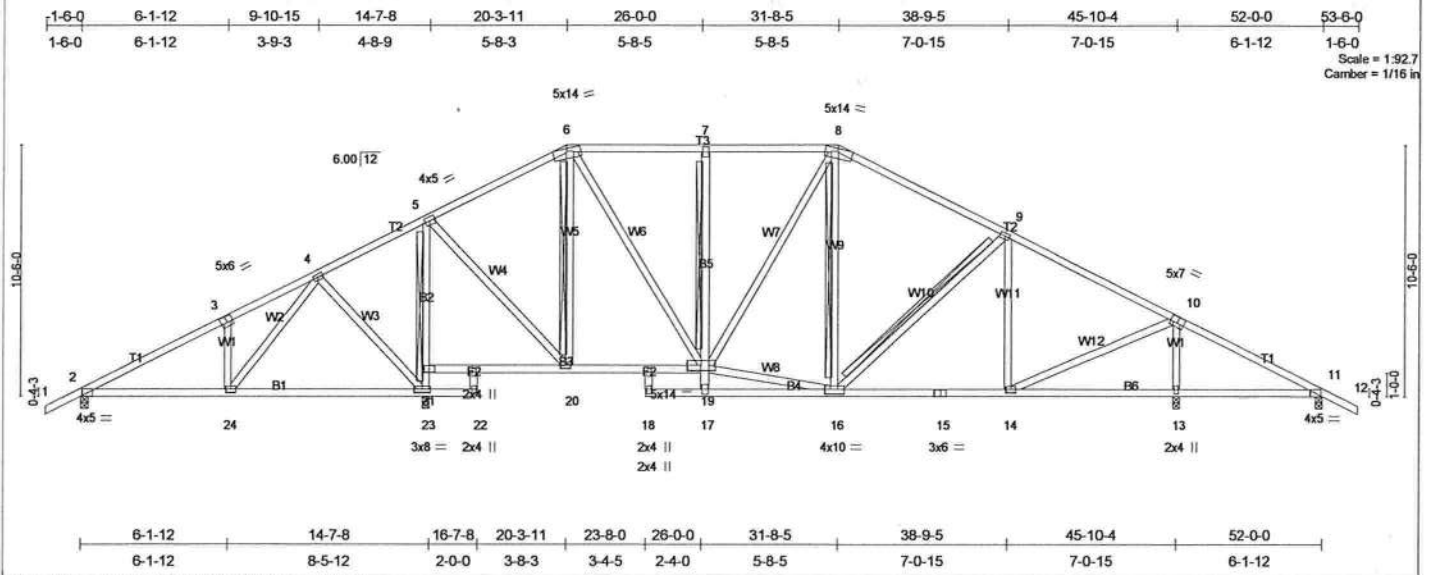


Plate Offsets (X,Y): [3.0-3.0,0-3.0], [10.0-3.8,0-3.0]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.36	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.44	Vert(LL) 0.22 23-24 >794 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.48	Vert(TL) -0.15 23-24 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 11 n/a n/a		
	Code FBC2004/TPI2002			Weight: 341 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-8.
BOT CHORD 2 X 4 SYP No.2 "Except"	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
B2 2 X 4 SYP No.3, B5 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 5-21, 7-19
WEBS 2 X 4 SYP No.3	T-Brace: 2 X 4 SYP No.3 - 6-20, 8-16, 9-16
	Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
	Brace must cover 90% of web length.

REACTIONS (lb/size) 2=502/0-3-8, 23=1551/0-3-8, 13=1275/0-3-8, 11=205/0-3-8
Max Horz 2=147(load case 6)
Max Uplift 2=389(load case 6), 23=400(load case 6), 13=379(load case 7), 11=197(load case 7)
Max Grav 2=509(load case 10), 23=1551(load case 1), 13=1275(load case 1), 11=212(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=614/1022, 3-4=582/1152, 4-5=0/403, 5-6=612/634, 6-7=766/736, 7-8=763/734, 8-9=914/735, 9-10=971/657, 10-11=8/169, 11-12=0/35
BOT CHORD 2-24=761/480, 23-24=332/200, 22-23=0/0, 21-23=1203/421, 5-21=1166/421, 20-21=78/21, 19-20=63/477, 17-19=0/150, 7-19=315/187, 17-18=0/0, 16-17=52/26, 15-16=272/792, 14-15=272/792, 13-14=110/143, 11-13=83/122
WEBS 3-24=268/268, 4-24=701/453, 4-23=392/504, 5-20=110/817, 6-20=536/138, 6-19=198/569, 16-19=135/727, 8-19=83/178, 8-16=85/106, 9-16=83/159, 9-14=318/241, 10-14=451/981, 10-13=1193/761

JOINT STRESS INDEX
2 = 0.40, 3 = 0.47, 4 = 0.48, 5 = 0.42, 6 = 0.77, 7 = 0.18, 8 = 0.92, 9 = 0.48, 10 = 0.76, 11 = 0.23, 13 = 0.44, 14 = 0.64, 15 = 0.33, 16 = 0.32, 17 = 0.18, 18 = 0.34, 19 = 0.33, 20 = 0.57, 21 = 0.50, 22 = 0.34, 23 = 0.88, 24 = 0.41, 25 = 0.34 and 26 = 0.34

NOTES (9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=18ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All plates are 3x5 MT20 unless otherwise indicated.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 389 lb uplift at joint 2, 400 lb uplift at joint 23, 379 lb uplift at joint 13 and 197 lb uplift at joint 11.
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job

L284669

Truss

T18

Truss Type

HIP

Qty

1

Ply

1

SPARKS CONST. - TAYLOR RES.

L284669037

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Aug 01 12:09:11 2008 Page 1

1-6-0

6-1-12

10-10-10

16-0-11

20-3-11

26-0-0

31-8-5

35-11-5

41-1-6

45-10-4

52-0-0

53-6-0

1-6-0

6-1-12

4-8-14

5-2-2

4-2-15

5-8-5

5-8-5

4-2-15

5-2-2

4-8-14

6-1-12

1-6-0

Scale = 1:91.5

6-1-12

14-5-12

20-3-11

26-0-0

31-8-5

37-6-4

45-10-4

52-0-0

6-1-12

8-4-0

5-9-15

5-8-5

5-8-5

5-9-15

8-4-0

6-1-12

Plate Offsets (X,Y): [3:0-3:0,0-3:0], [11:0-3:0,0-3:0]

LOADING (psf)	SPACING	2:0-0	CSI	DEFL	in (loc)	I/defl	I/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	0.20 21-22	>877	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.35	Vert(TL)	-0.15 14-15	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.81	Horz(TL)	0.03 14	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						

Weight: 342 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.); 6-8.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 5-21, 6-19, 7-18, 8-18, 9-17, 9-15, 10-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size)

2=428/0-3-8, 21=1617/0-3-8, 14=1264/0-3-8, 12=178/0-3-8

Max Horz 2=147(load case 6)

Max Uplift 2=274(load case 6), 21=587(load case 6), 14=352(load case 7), 12=171(load case 7)

Max Grav 2=441(load case 10), 21=1617(load case 1), 14=1264(load case 1), 12=185(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=-466/500, 3-4=-442/644, 4-5=-130/334, 5-6=-424/338, 6-7=-614/510, 7-8=-614/510, 8-9=-800/578, 9-10=-866/548, 10-11=0/224, 11-12=-127/239, 12-13=0/35

BOT CHORD 2-22=-299/350, 21-22=-52/212, 20-21=-35/378, 19-20=-35/378, 18-19=0/339, 17-18=-0/673, 16-17=-86/739, 15-16=-86/739, 14-15=-99/606, 12-14=-144/232

WEBS 3-22=-287/292, 4-22=-769/495, 4-21=-434/544, 5-21=-1199/690, 5-19=-311/725, 6-19=-585/345, 6-18=-343/563, 7-18=-330/226, 8-18=-129/113, 8-17=-158/235, 9-17=-143/211, 9-15=-106/48, 10-15=0/226, 10-14=-1140/520, 11-14=-307/324

JOINT STRESS INDEX

2 = 0.35, 3 = 0.57, 4 = 0.48, 5 = 0.61, 6 = 0.53, 7 = 0.34, 8 = 0.53, 9 = 0.61, 10 = 0.48, 11 = 0.57, 12 = 0.35, 14 = 0.41, 15 = 0.33, 16 = 0.32, 17 = 0.59, 18 = 0.63, 19 = 0.59, 20 = 0.32, 21 = 0.33 and 22 = 0.41

NOTES (9)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02, 110mph (3-second gust); h=18ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) All plates are 3x5 MT20 unless otherwise indicated.

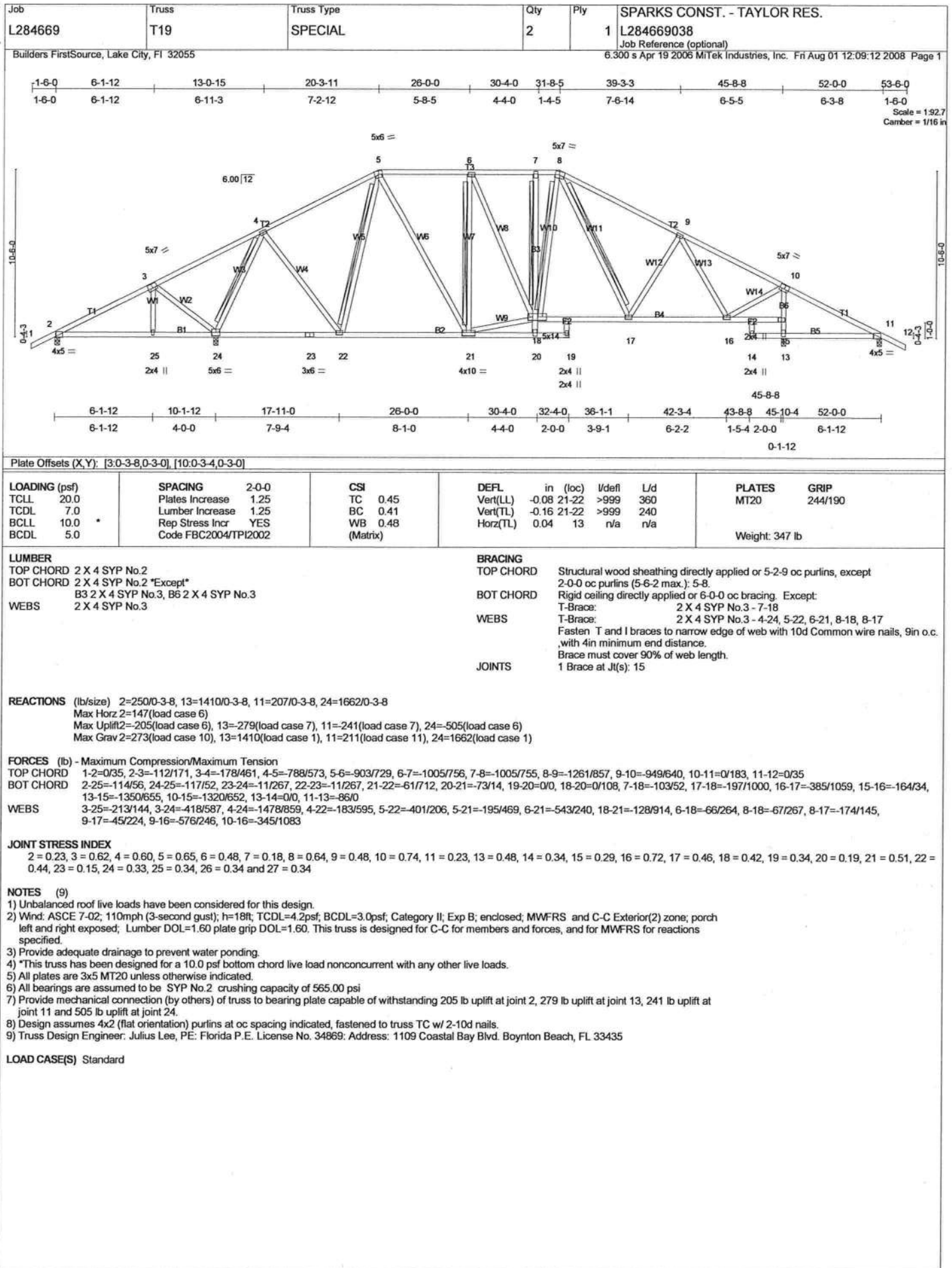
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

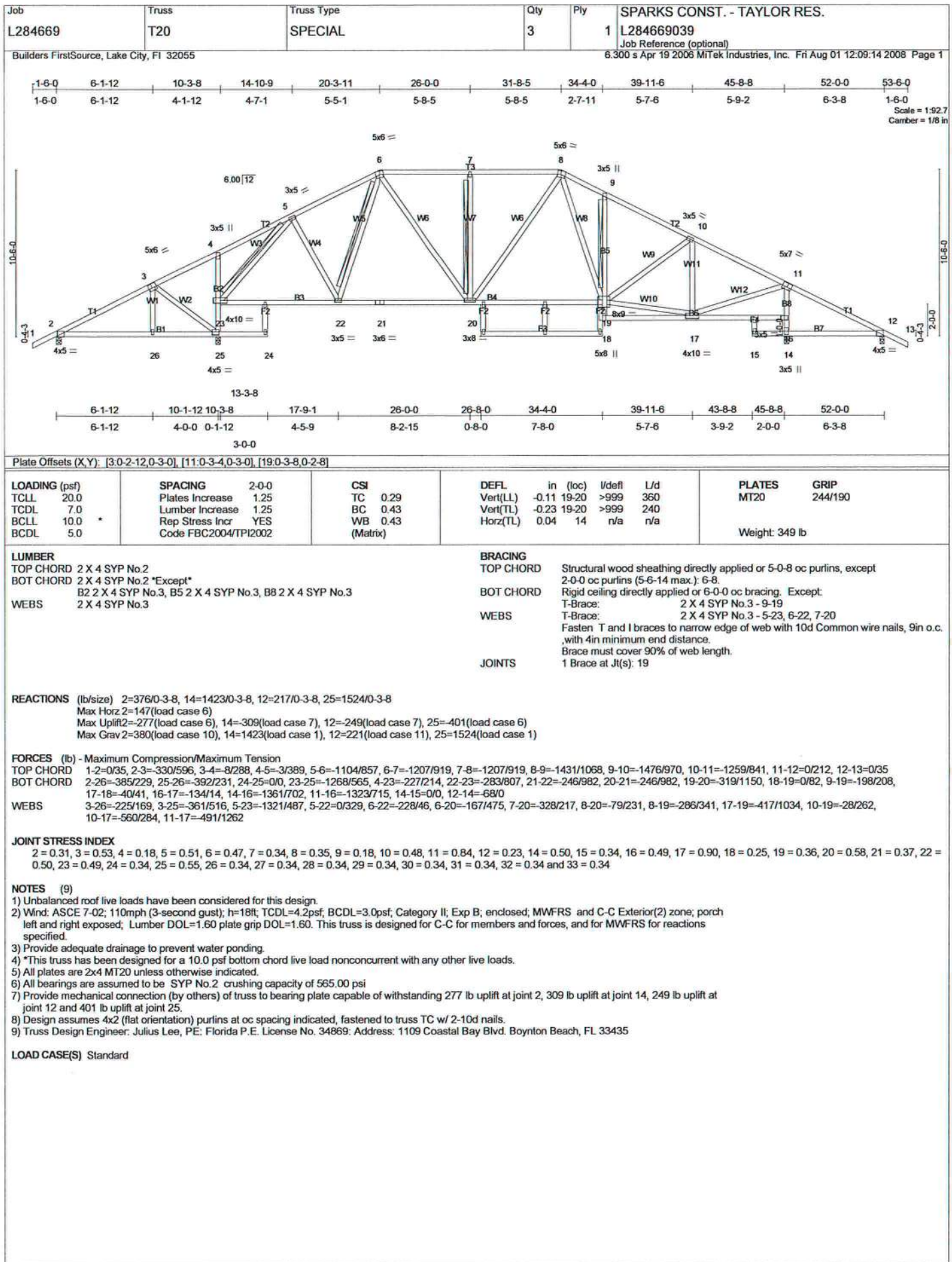
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2, 587 lb uplift at joint 21, 352 lb uplift at joint 14 and 171 lb uplift at joint 12.

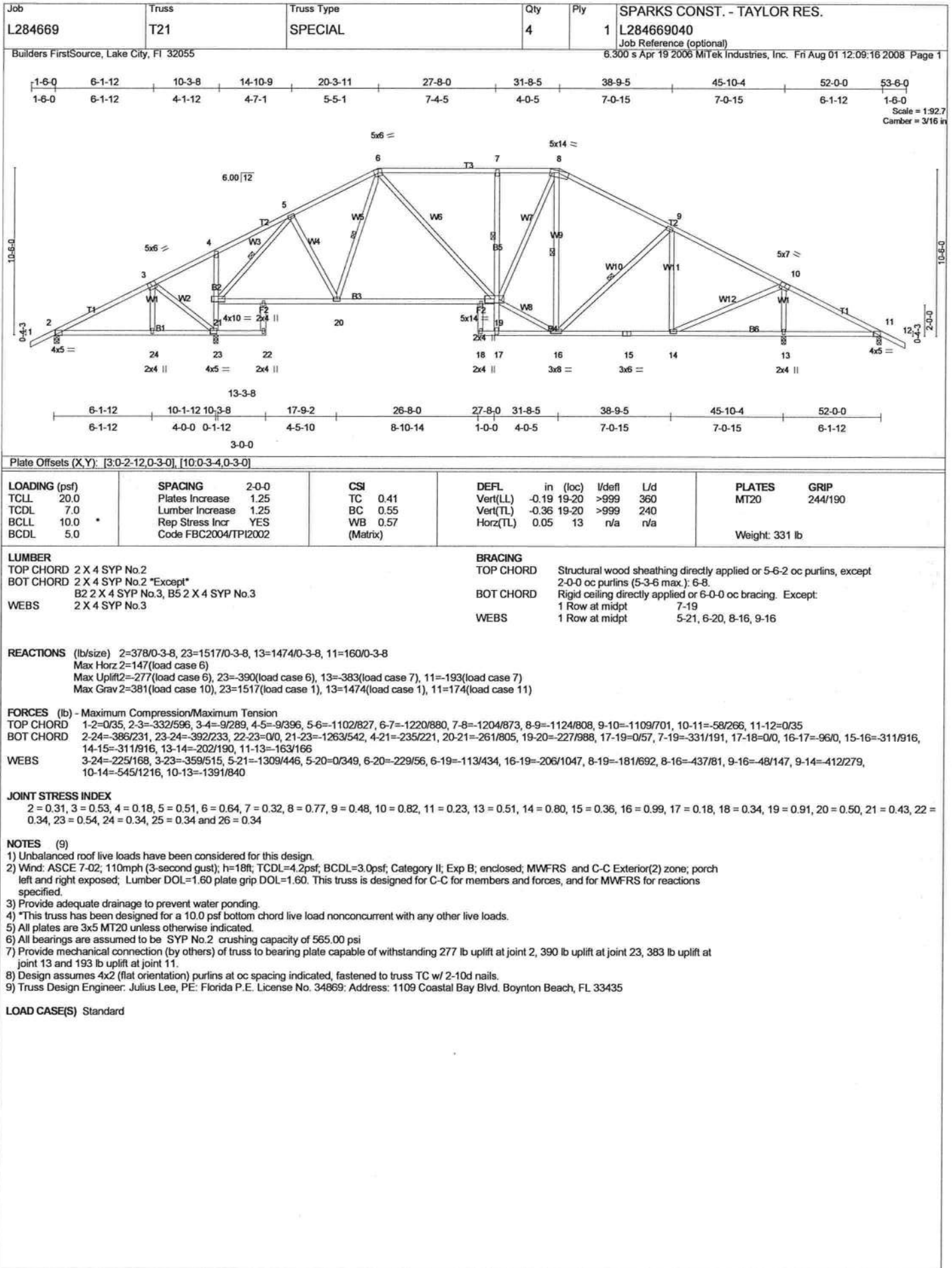
8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

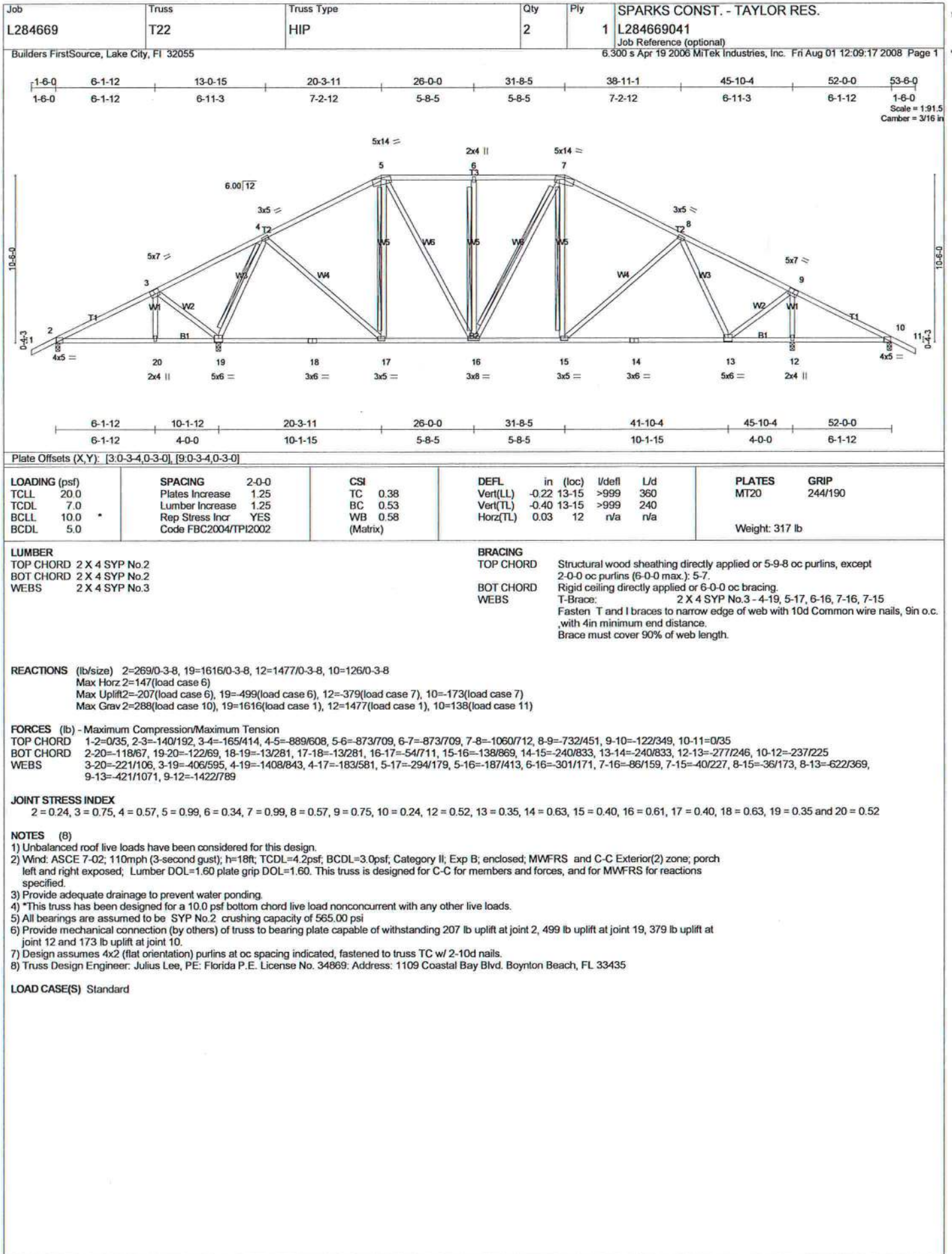
9) Truss Design Engineer: Florida Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard









TOP CHORD 2X4 SO. PINE #2 or Better 120 MPH MAX

MAX 2' TYP.
MAX -
Setback 7' or Less

UPLIFT: 400# or Less

CJ's
2' TYP.
MAX

UPLIFT: 400# or Less

[illegible]

UPLIFT™ 400# or Less



ALL HEELS TO BE STANDARD WITH NO CANTILEVER



ALL HEELS TO BE STANDEAR WITH NO CANTTLEVER

SEE FOR THE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET
SETBACK

7'0" MAX

~~UNAVAILING~~ REFERS TO REQUIRED EXTREME CARE IN FABRICATING, WELDING, SHIPPING, INSTALLING AND BRACING. REFER TO BC01-0-93 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 5869 DOWNER RD., SUITE 200, MADISON, VT 52719 AND VTOA CYCLOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LN, MADISON, WI 52719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFLICTS WITH APPLICABLE PROVISIONS OF NDS OPTIONAL DESIGN SPECIFICATIONS AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF #618/REGA /ALPS#3 ASIN A653 GRADE 6060-T6 ALUMINUM EXTRUDED CHANNELS. PROVIDE ONE HOLE TO EACH JOINT OF TRUSS AND FIELD JOINTS. COUPLER BOLTS SHALL BE PER ANEX A4 OF TPI-1-0002 SEC. 3 A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SELECTED FOR THE BRACING COMPONENT DESIGN SHOW. THE INSTALLATION AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2

CONG. ENGINEERS, P.
1065 SW 4th Avenue

	DUR.	FAC.	SPACING	2' MAX
PC	DL	20	MAX PSF	
TC	DL	20	MAX PSF	
BC	DL	10*	MAX PSF	
CC	DL	5	MAX PSF	
LC	DL	20	MAX PSF	
EC	DL	20	MAX PSF	
CC	DL	10*	MAX PSF	
CC	DL	5	MAX PSF	

REVIEWED
By Julius Ioo at 10:52 am, Jun 27, 2008

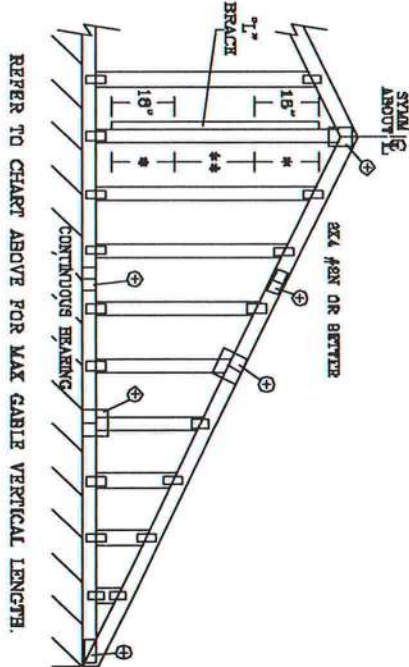
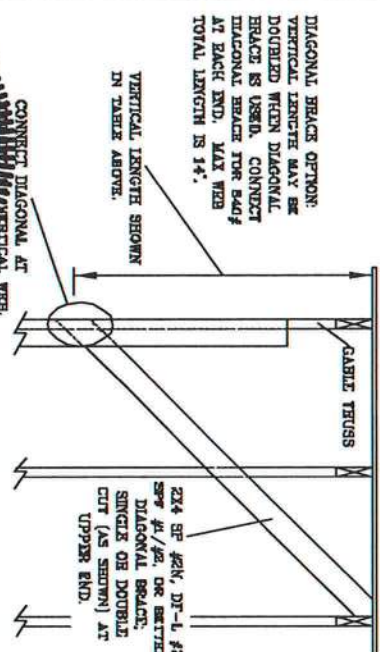


ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																
CABLE VERTICAL SPACING	2x4 SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE *		(2) 2x8 "L" BRACE *		
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"	12' 11"	12' 11"	
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"	12' 11"		
	HF	STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"	12' 11"		
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"	13' 11"		
	SP	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 11"	13' 11"		
		#2	3' 7"	6' 10"	6' 3"	6' 11"	7' 6"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 11"	13' 11"		
	DFL	#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"	13' 7"		
		STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 3"	10' 3"	10' 3"	12' 11"	13' 7"	13' 7"		
	16" O.C.	SPF	#1 / #2	3' 4"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	3' 3"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	
HF		STUD	3' 9"	5' 2"	6' 2"	6' 10"	6' 10"	8' 2"	8' 2"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"		
		STANDARD	3' 9"	5' 2"	6' 2"	7' 11"	7' 11"	8' 6"	8' 6"	10' 2"	10' 2"	13' 5"	13' 5"	14' 0"		
SP		#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
DFL		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
24" O.C.		SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	
	HF	STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 6"	9' 6"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"		
		STANDARD	3' 9"	5' 2"	6' 2"	6' 10"	6' 10"	8' 2"	8' 2"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"		
	SP	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
	DFL	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	12" O.C.	SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
HF		STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
SP		#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
DFL		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
16" O.C.		SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
	HF	STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	SP	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
	DFL	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	24" O.C.	SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
HF		STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
SP		#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
DFL		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
12" O.C.		SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
	HF	STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	SP	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
	DFL	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	16" O.C.	SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
HF		STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
SP		#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
DFL		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
24" O.C.		SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
	HF	STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	SP	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
	DFL	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	12" O.C.	SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
HF		STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
SP		#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
DFL		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
16" O.C.		SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
	HF	STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	SP	#1	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"		
	DFL	#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"	12' 5"	14' 0"		
	24" O.C.	SPF	#1 / #2	4' 3"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
			#3	4' 2"	8' 8"	7' 2"	7' 11"	8' 6"	9' 5"	9' 5"	12' 6"	12' 6"	14' 0"	14' 0"	14' 0"	
HF		STUD	4' 0"	6' 2"	6' 2"	7' 11"	8' 2"	9' 6"	9' 6"	12' 5"	12' 5"	14' 0"	14' 0"	14' 0"		
		STANDARD	4' 0"	6' 1"	6' 1"	7' 11"	7' 11"	8' 1"	8' 5"	9' 11"	9' 11"	12' 5"				

CABLE TRUSS DETAIL NOTES:

- LIVE LOAD DESTRUCTION CATEGORY IS I/240.
- PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER CONTINUOUS BEARING (6 PSF FC DEAD LOAD).
- CABLE END SUPPORTS LOAD FROM 4' 0" OUTLEAKERS WITH 8' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
- ATTACH EACH "L" BRACE WITH 104 NAILS.
- * FOR (1) "L" BRACE: SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.
- ** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
- "L" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.



CABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH	NO. SPICES	LESS THAN 4' 0"
GREATER THAN 4' 0", BUT	2X4	LESS THAN 11' 8"
GREATER THAN 11' 8"	2.5X4	

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND BEEL PLATES.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO SECS 1-403 (BUILDING COMPONENT SAFETY, DEFORMATION), PUBLISHED BY THE CHIEF ENGINEER, WITH THE 1997 EDITION OF THE 2000 INTERNATIONAL BUILDING CODE (IBC) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 ST. 4TH AVENUE
DELAWARE BEACH, FL 33441-8161

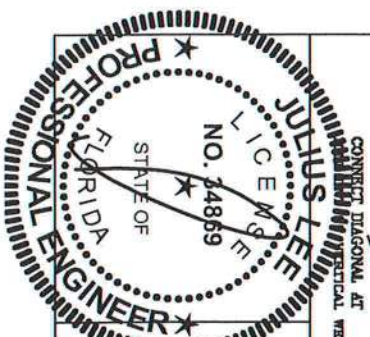
REF ASCE7-02-CAB130A15
DATE 11/26/03
DRWG MTRK STD GABLE 18 L ET
-ENG

REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

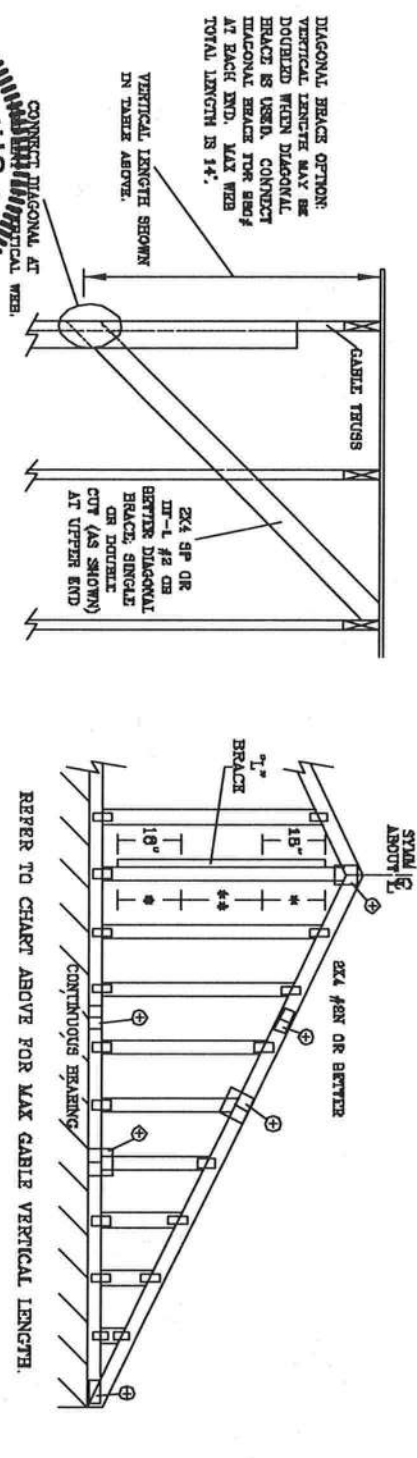
No. 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"



ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		2X4		BRACE		NO		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE *	
SPACING	VERTICAL SPECIES	GRADE	BRACE	NO	GROUP A	GROUP B	GROUP C	GROUP D	GROUP E	GROUP F	GROUP G	GROUP H	GROUP I	GROUP J	GROUP K	GROUP L	GROUP M
12" O.C.	SPF	#1 / #2	STUD	3' 2"	5' 6"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"
		#3		3' 1"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"
		STUD		3' 1"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"
		STANDARD		2' 11"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"
16" O.C.	SPF	#1 / #2	STUD	3' 2"	5' 6"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"
		#3		3' 1"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"
		STUD		3' 1"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"
		STANDARD		2' 11"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"
24" O.C.	SPF	#1 / #2	STUD	3' 2"	5' 6"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"	6' 8"
		#3		3' 1"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"
		STUD		3' 1"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"	4' 5"
		STANDARD		2' 11"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"	3' 9"



BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPRUCE-PINE-YR	42
#1 / #2 STANDARD	STUD
#3	STANDARD
STUD	
STANDARD	
GROUP B:	
SPRUCE-PINE-YR	42
#1 / #2 STANDARD	STUD
#3	STANDARD
STUD	
STANDARD	

CABLE TRUSS DETAIL NOTES:
 LIVE LOAD DEFLECTION CRITERIA IS L/240.
 PROVIDE UPLIFT CONNECTIONS PER 180 PSF OVER
 CONTINUOUS BEARING (6 PER FC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4" O"
 OUTLOOKERS WITH 2" O" OVERHANG, OR 12"
 PLAYWOOD OVERHANG.

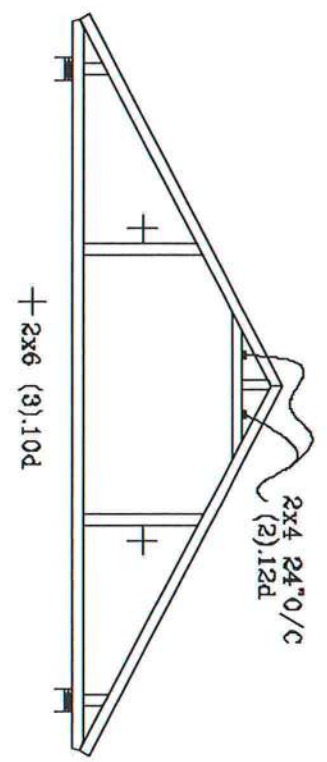
CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SERVICE
LESS THAN 4' 0"	1X4 OR BRS
GREATER THAN 4' 0", BUT	2X4
LESS THAN 11' 8"	
GREATER THAN 11' 8"	2.5X4



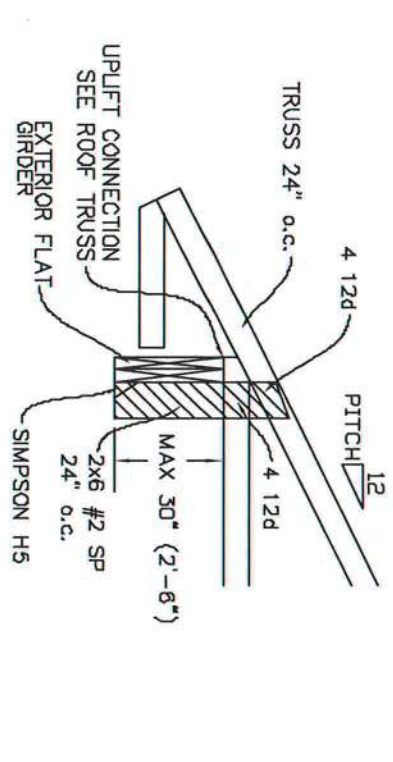
REVIEWED
 By Julius Lee at 12:00 pm, Jun 11, 2008

JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1456 SW 4th AVENUE
 DUNNELL BLVD. FL 33444-8401
 No: 34869
 STATE OF FLORIDA
 REF: ASCE 7-02-GAB130380
 DATE: 11/26/03
 DWG: LATE STD GABLE 30' x 10'
 -ENG-
 MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

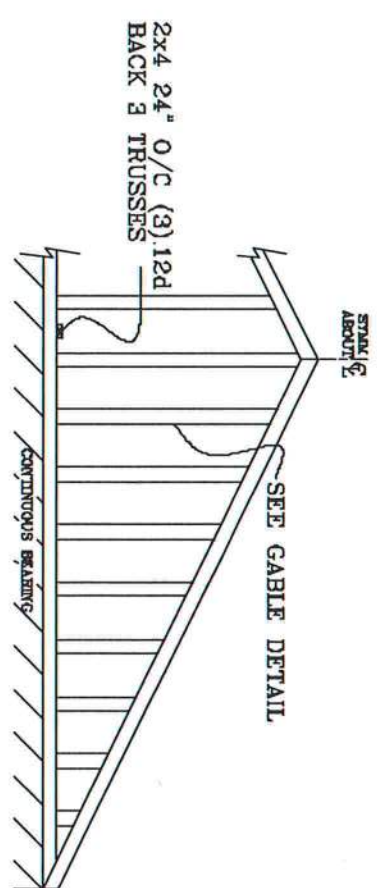
TYPICAL ATTIC TRUSS BRACING



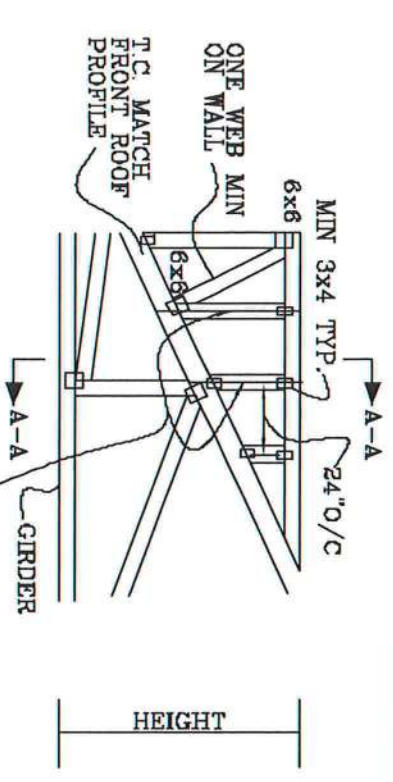
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



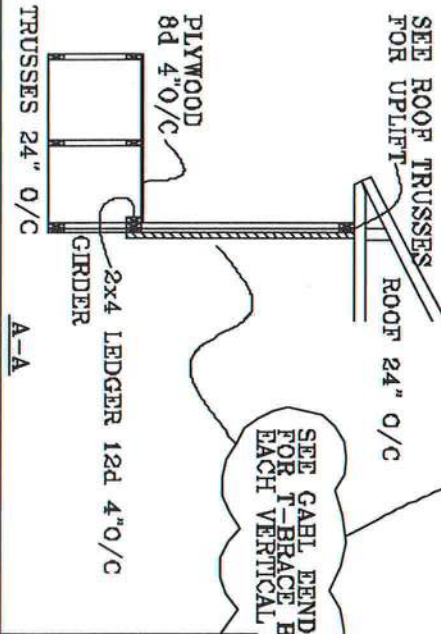
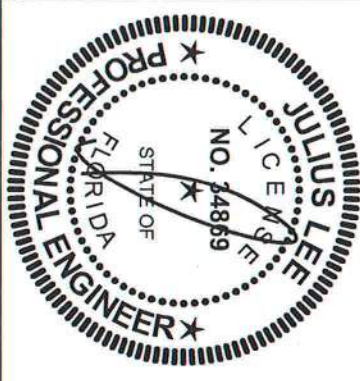
GABLE END TRUSS DETAIL



TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL

JULIUS LEE'S
CONS. ENGINEERS P.A.
1465 SW 4th AVENUE
DIKSHIT Bldg., FL 33411-2661

No. 34869
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPICES MUST BE STAGGERED SO THAT ONE SPICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

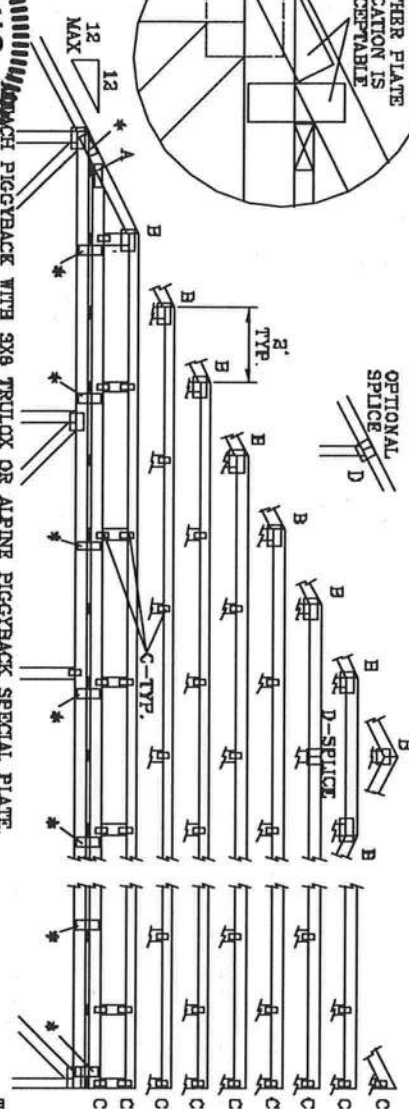
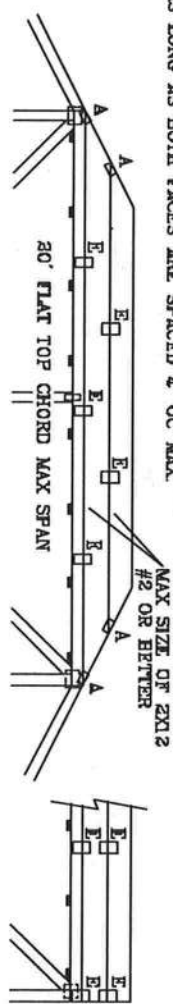
CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, ENCLOSURE BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

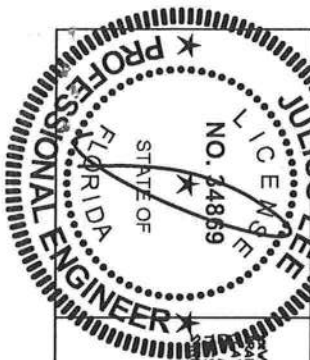
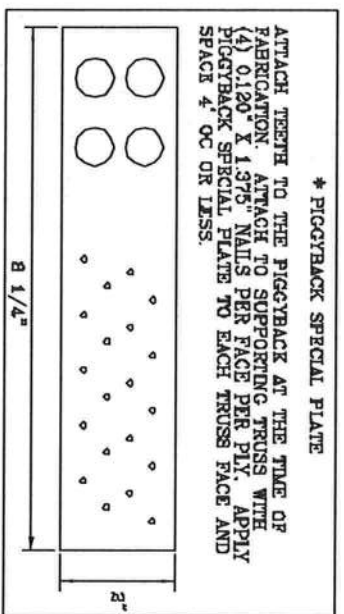


THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

JOINT TYPE	SPANS UP TO		
	30'	36'	62'
A	2X4	2.5X4	3X6
B	4X6	6X6	6X6
C	1.5X3	1.6X4	1.5X4
D	5X4	6X6	6X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY		

ATTACH TRUSS PLATES WITH (8) 0.120" X 1.375" NAILS OR EQUAL PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80X LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80X LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4TH AVENUE
ORLANDO, FL 32811-2161

No. 34869
STATE OF FLORIDA

MAX LOADING	REF
55 PSF AT 1.33 DUR. FAC.	DATE 09/12/07
50 PSF AT 1.25 DUR. FAC.	DRWG/ITEK STD PIGGY
47 PSF AT 1.15 DUR. FAC.	-ENG JL
SPACING 24.0"	

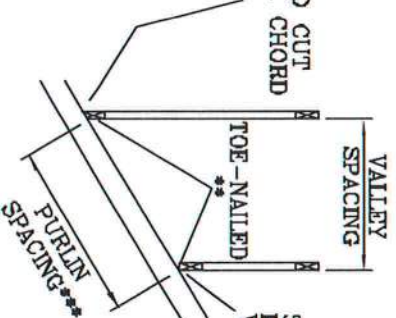
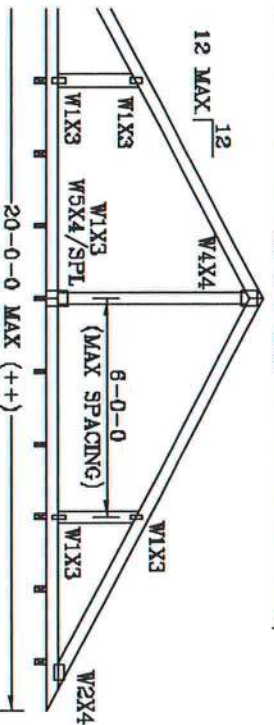
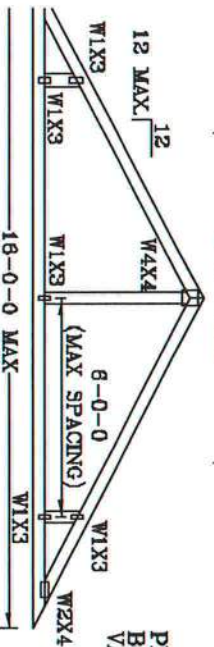
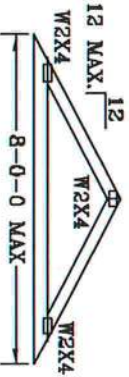
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

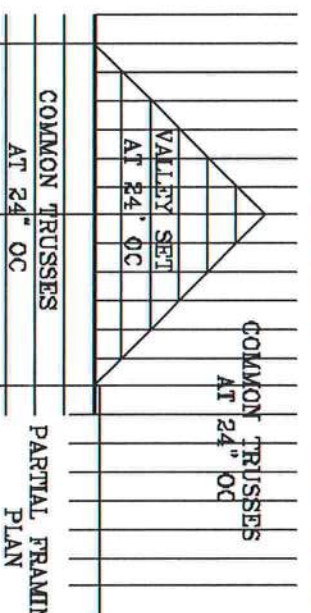
- (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
- FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
- ASCE 7-02 130 MPH WIND, 15' MEAN HEIGHT, ENCLOSED
- BUILDING, EXP. C, RESIDENTIAL, WIND TC DL-6 PSF.



SQUARE CUT
BOTTOM CHORD
VALLEY

OPTIONAL STUB
END DETAIL

OPTIONAL HIP
JOINT DETAIL



PARTIAL FRAMING
PLAN

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:

PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

OR PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

WARNING: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DETAIL 1-10 BUILDING DEPARTMENT SPECIFICATIONS, PUBLISHED BY THE TRUSS ASSOCIATION, 5800 ENTERPRISE DR., SUITE 200, MADISON, VA 52759 AND AISC TRUSS COUNCIL, AMERICA, 6500 ENTERPRISE DR., SUITE 200, MADISON, VA 52759 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S

CONS. ENGINEERS P.A.

1455 SW 4TH AVENUE
DEALAT BLVD, FL 33444-0001

NO. 34869

STATE OF FLORIDA

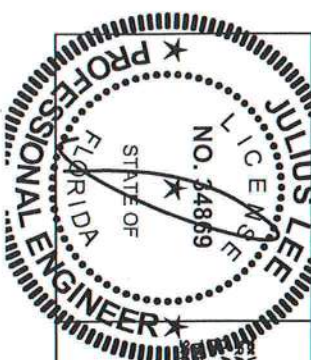
REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

THIS DRAWING REPLACES DRAWING A105

TC IL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC IL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		

DURFAC 1.25	1.25
SPACING	24"



TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

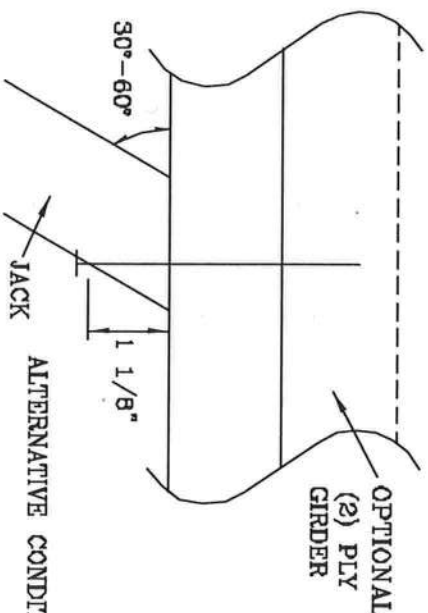
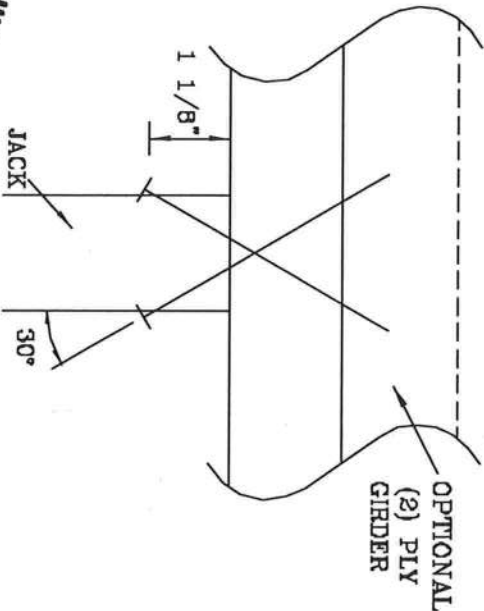
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

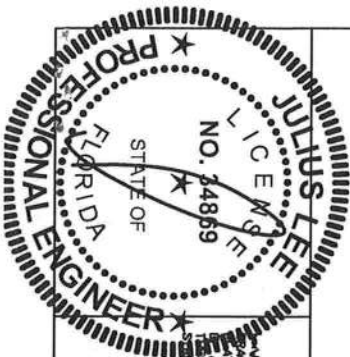
NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES
2	187#	256#	181#	234#	156#	203#	154#	189#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND SPACING. REFER TO BCST 1-43 CRUISING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE CRUISING INSTITUTE, 288 PRINCE DR., SUITE 200, NANTUCKET, MA 02559. FOR SAFETY PRACTICES PRIOR TO PERFORMING TRUSS CONSTRUCTION. IF CHORD NAIL HAVE PRESENTLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PRESENTLY ATTACHED



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

1495 BY 4TH AVENUE
DELMAR BEACH, FL 33441-2161

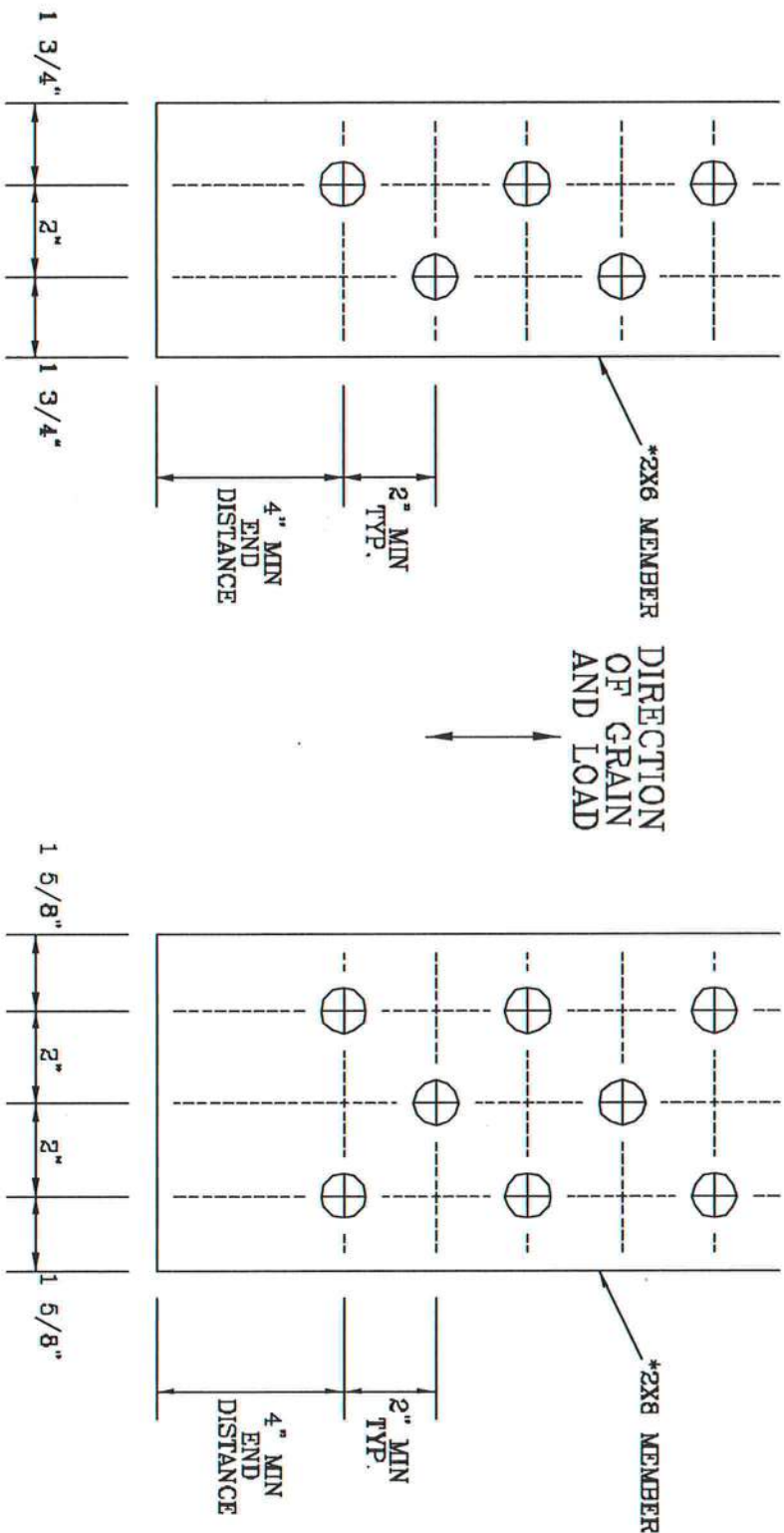
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONALL103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

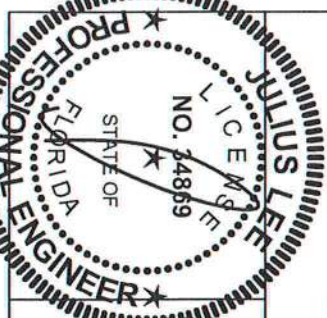
THIS DRAWING REPLACES DRAWING A628.016

WARNING: THESE REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTION. REFER TO POST-100 BUILDING DEPARTMENT SAFETY DEPARTMENT, ESTABLISHED BY THE CLASS ACTION LITIGATION, 1998-1999, AND THE CLASS ACTION LITIGATION, 2000-2001, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4TH AVENUE
DELMAR BEACH, FL 33444-2161

STATE OF

NO. 34869



REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

TRULOX CONNECTION DETAIL

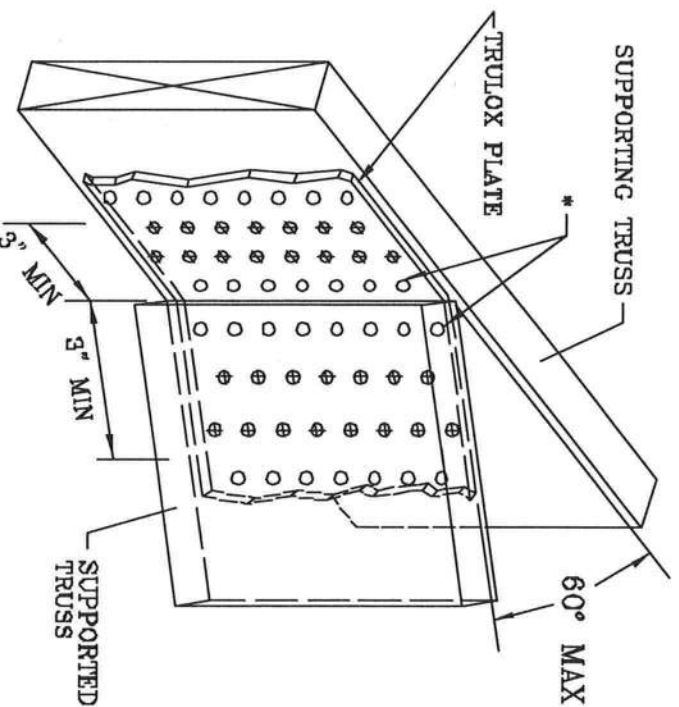
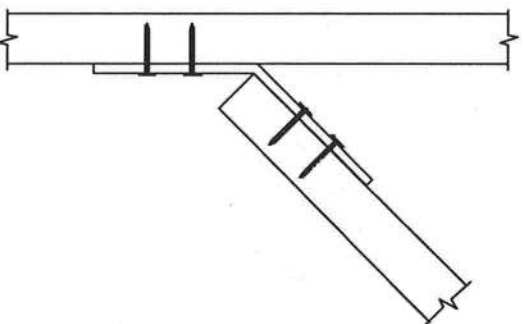
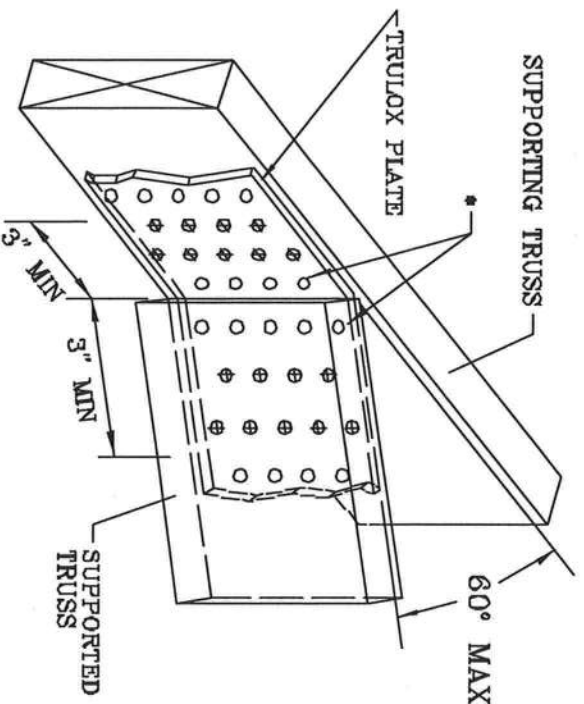
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FULL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

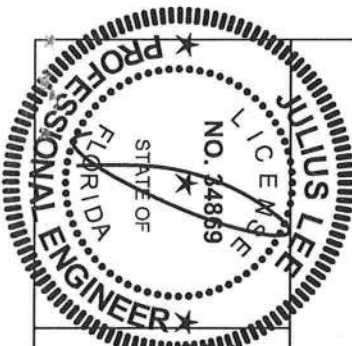


MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#

MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,988/R 1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31-1-93 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 283 DOWNSIDE DR., SUITE 200, MADISON, VT 05750 AND VITA CODE TRUSS COUNCIL, 6500 ENTERPRISE LN, MADISON, VT 05715 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-3181

Nbr: 34869
STATE OF FLORIDA

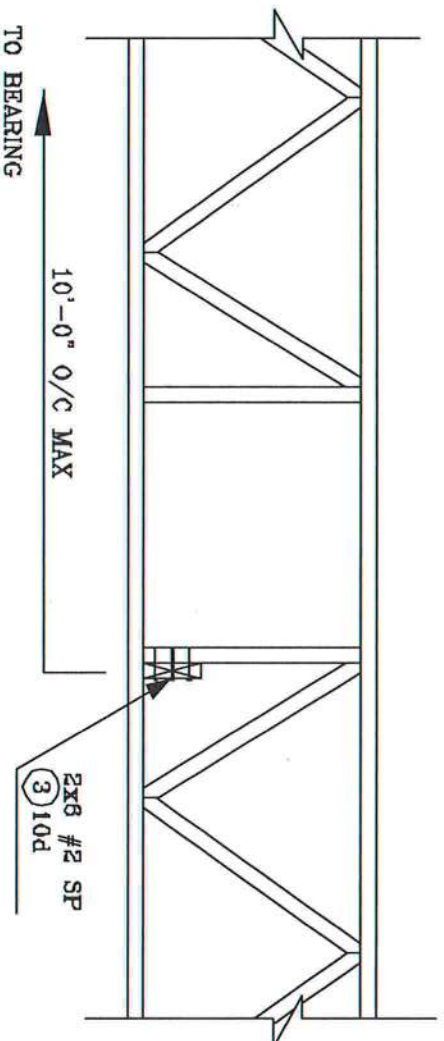
REF TRULOX

DATE 11/26/03

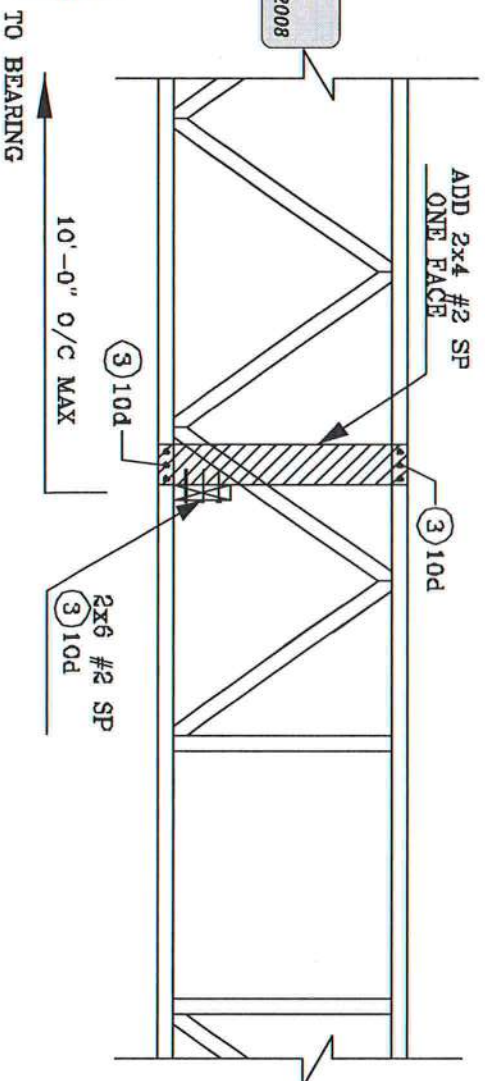
DRWG CNTRULOX1103

-ENG JL

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS

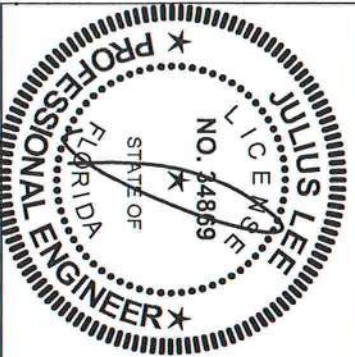


ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED

By Julius Lee at 11:58 am, Jun 11, 2008



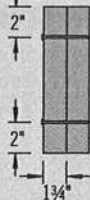
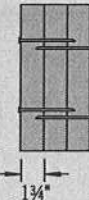
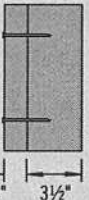
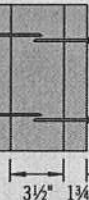
JULIUS LEE'S
CONS. ENGINEERS P.A.

1426 SW 4th Avenue
Dixie Beach, FL 33441-2641

No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/4" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 3/16" maximum.

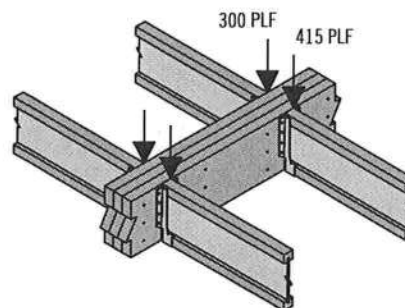
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

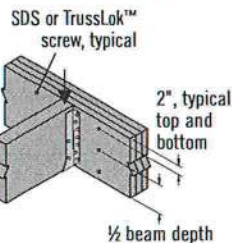
(2) 6" long screws required.

(3) 5" long screws required.

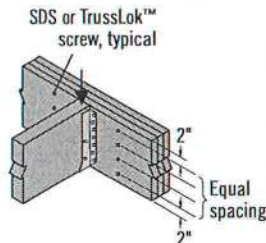
(4) 3 1/2" and 3 3/4" long screws must be installed on both sides.

Connections

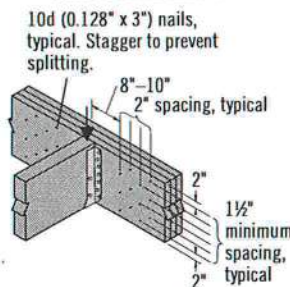
4 or 6 or Screw Connection



8 Screw Connection

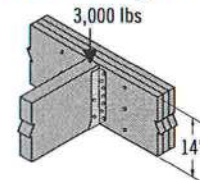


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

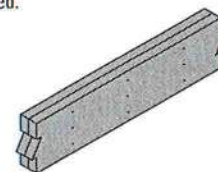
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"