

TRANSVERSE | n/a

LONGITUDINAL n/a

-6"X6" W1.4XW1.4 W.W.M. PLACED AT 2"

-4" CONCRETE SLAB

-6 MIL VAPOR BARRIER

WITH POLY TAPE

COMPACTED FILL

(2) #5 CONTINUOUS

MOINOLITHIC FOOTING

SCALIE: 1/2" = 1'-0"

IF TRUSS TO BEAM

ARE NOT REQUIRED

STRAPS ARE NAILED O BEAM SPH_

OPTION: 2 (POCKETED)

POCKETED

TOP PLATE

(DROPPED BEA.M)

ALLOWABLE UPLIFT:

WITH 6" LAPS SEALED

DEPTH ON CHAIRS OR FIBERMESH CONCRETE

STRUCTURAL PLAN

STRUCTURAL PLAN NOTES

EACH SIDE (U.N.O.)

TRUSS PACKAGE

HEADER LEGEND

(2) 2X12X0',1J 1K

WALL LEGEND

E STRUCTURAL PLAN ----

6 SYP #2 POST -

J POST BASE -

12) 16d & 5/8" ANCHOR

ONE STORY WOOD

SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

ALL LOAD BEARING FRAME WALL HEADERS

SHALL HAVE (1) JACK STUD & (1) KING STUD

ARE NOT EXACT. REFER TO ARCHITECTURAL

---HEADER/BEAM CALL-OUT (U.N.O.)

EXTERIOR WALL

ENGINEERED TRUSSES

w/ (8) 16d TO HEADER

ALLOWABLE UPLIFT

& (8) 16d TO POST

ATTACH PER TRUSS UPLIFT

INTERIOR NON-LOAD BEARING WALL

INTERIOR LOAD BEARING WALL w/ NO UPLIFT

INTERIOR LOAD BEARING WALL w/ UPLIFT

OPTION: 1 (BUCKET)

(2) 2X_ SPF#2 TOP PLATE -

18-16d TO FACE

10-10d TO JOIST 3" NOTCH-

- BEAM TO BEAR ON -

(2) 2X SPF#2 JACKS

- 2X_ PT SYP#2 PLATE -

WITHIN 3" OF STUD PACK

1/2" ANCHOR

2" WASHER

(TYP.) BEAM TO WALL

WOOD FRAME w/ STRAPS & ANCHORS

(2) MTS20

-HUC410

--- NUMBER OF KING STUDS (FULL LENGTH)

DIMENSIONS ON STRUCTURAL SHEETS

FLOOR PLAN FOR ACTUAL DIMENSIONS

-SPAN OF HEADER

SIZE OF HEADER MATERIAL

-NUMBER OF PLIES IN HEADER

SCALE: 1/4" = 1'-0"

(2) FOR SYP GIRDER & SPF STUDS

ABU44

TRUSS CONNECTOR UPLIFT SYP UPLIFT SPF F1 SYP F2 SYP F1 SPF F2 SPF TO RAFTER/TRUSS TO PLATES 4-8d x 1 1/2" 4-8d x 1 1/2 415 4-8d x 1 1/2" 4-8d x 1 1/2 H2.5 415 5-8d x 1 1/2" 5-8d x 1 1/2 H2.5A 480 110 110 110 110 5-8d x 1 1/2" 5-8d x 1 1/2 H6 950 8-8d 8-8d 5-10d x 1 1/2" 5-10d x 1 1/2 H14-1 1050 515 265 480 245 12-8d x 1 1/2" 13-8d H14-2 12-8d x 1 1/2" 15-8d H10 8-8d x 1 1/2" 8-8d x 1 1/2 H10-2 655 6-10d 6-10d H16 1265 2-10d x 1 1/2" 10-10d x 1 1/ H16-2 1265 2-10d x 1 1/2" | 10-10d x 1 1/2 LTS12 - LTS20 1000 620 6-10d x 1 1/2" 6-10d x 1 1 MTS12 - MTS30 1000 860 7-10d x 1 1/2" 7-10d x 1 1/ HTS16 - HTS30 1450 1245 12-10d x 1 1/2" | 12-10d x 1 1/2 HEAVY GIRDER TIEDOWNS TO FOUNDATION LGT2 14-16d LGT3-SDS2.5 795 410 795 410 12-SDS 1/4" x 2 1/2 26-16dS LGT4-SDS3 4060 3860 2000 675 2000 675 12-SDS 1/4" x 3" 36-16dS MGT 3965 3330 22 -10d 5/8" ANCHOR HGT-2 10980 6485 16 -10d 2-5/8" ANCHOR HGT-3 10530 9035 16 -10d 2-5/8" ANCHOR HGT-4 9250 9250 16 -10d 2-5/8" ANCHOR STUD STRAP CONNECTOR TO STUDS SSP DOUBLE TOP PLATE 435 3-10d 4 -10d SSP SINGLE SILL PLATE 455 420 1-10d 4 -10d DSP DOUBLE TOP PLATE 825 825 6-10d 8 -10d DSP SINGLE SILL PLATE 600 2-10d 8 -10d 4 -10d 6 -10d SP2 1065 605 6-10d 6 -10d SP4 885 760 6-10d x 1 1/2' SPH4 1240 1065 10-10d x 1 1/2" SP6 885 760 6-10d x 1 1/2" SPH6 1240 1065 10-10d x 1 1/2" LSTA18 1235 14-10d LSTA21 1235 1235 16-10d CS20 1030 1030 14-10d CS16 1705 22-10d STUD ANCHORS TO STUDS TO FOUNDATION LTT19 1350 1305 8-16d 1/2" ANCHOR LTTI31 2310 2310 18-10d x 1 1/2 5/8" ANCHOR HD2A 2775 2570 2-5/8" BOLTS 5/8" ANCHOR HTT16 4175 3695 18-16d 5/8" ANCHOR 5260 5250 32-16d 5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES

2200

2320

LUMBER SIZE & GRADE MINUMUM REQUIREMENTS RAFTER SPANS 20'-0" OR LESS 2X4 SYP #2 PURLINS / LATERAL BRACING 2X4 SPF #2 2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR SLEEPERS CRIPPLES & BLOCKING 2X4 SPF #2 OR BETTER SEE TRUSS DESIGN - SOUTHEREN PINE MATERIA TRUSS BELOW 2'-0" O.C. (TYP.) 4'-0" MAX SPACING -CS20 RIDGE TENSION STRAP w/ 8 - 8d OR 2X4 COLLAR TIE 3 - 16d OR 4 - .131 x3" VALLEY ROOF PLAN -2X4 VALLEY RAFTER * ATTACHMENT CAN BE MADE DIRECTLY OR THROUGH PLYWOOD SHEATHING BY CUTTING A 2" x 4" NOTCH IN SHEATHING 4'-0" MAX SPACING 4'-0' (TYP.) BEVEL RAFTER CUT AS REQ'D FOR PITCH FRAMING 2x SYP @ 24* O.C. (WHERE NO SHEATHING IS APPLIED) (NOT REQUIRED IF SLEEPERS ARE USED) SECTION CUT PARALLEL TO VALLEY RAFTER

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B4, XTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C STUD DEFLECTION LIMIT H/240 (NOT OK FOR SOME BRITTLE FINISH?). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

VALLEY ROOF PLAN MEMBER LEGEND

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) NAX

MAXIMUM ROOF AREA PER SUPPORT

16ft2 IN ZONES 2 & 3 , 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" Ø.C. X 4'-0" SPAN

16fiz IN ZONES 2 & 3 , 24fiz IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 16fiz OR 2'-0" X 8'-0" SPAN = 16fiz)
PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SIPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
-SPANS (DISTANC'S RETWEEN HEEL ST 40'-0" OR LESS.

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLE\$ 5-0" TO 10'-0" LONG NAILED w? 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGEOF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH, CRIPPLES OVER 10'-0" LONG REQURE TWO CLB'S OR BOTH FACES W/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.

NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTIER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD

INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.

INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED,

- APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE

PPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE

-SPANS (DISTANCS BETWEEN HEELS) 40'-0" OR LESS - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS -MAXIMUM WIND SPEED: 120 MPH

MAXIMUM TOTAL LOADING: 40 psf
MEETS FBC 2001/ASCE 7-98 WIND REQUIREMENTS
- EXPOSURE CATEGORY "B", I = 1.0, Kzt = 1.0

CRIPPLE, BRACING, & BLOCKING NOTES

MAXIMUM MEAN ROOF HEIGHT: 30 FEET

3-16d OR 6 - .131 x 3" TOE NAIL

- 16d OR 6 - .131 x 3" FACE NALS

- 16d OR 6 - .131 x 3" FACE NALS

4 - 16d OR 8 - .131 x 3" FACE NALS EACH TRU

6 -16d OR 12 - .131 x 3" TOE NALS

-16d OR 6 - .131 x 3" TOE NAILS

3-16d OR 6 - .131 x 3" TOE NAIL

-16d OR 6 - .131 x 3" NAILS

-16d OR 8 - .131 x 3" NAILS

3 -16d OR 6 - .131 x 3" END NAIL

3 -16d OR 6 - .131 x 3" FACE NAII

3 -16d OR 6 - .131 x 3" FACE NAIL

= = = TRUSS UNDER VALLEY FRAMING

CONNECTION REQUIREMENT NOTES

: = = = : VALLEY RAFTER OR RIDGE

TRUSS

CRIPPLE

CRIPPLE TO RAFTERS

SLEEPER TO TRUSS

RAFTER TO SLEEPER OR BLOC

RIDGE BOARD TO ROOF BLOCK

PURLIN TO TRUSS (IF CRIPPLE IS

RIDGE BOARD TO TRUSS

PURLIN TO TRUSS (TYP

TRUSS TO BLOCKING

CRIPPLE TO TRUSS

1 CRIPPLE TO PURLIN

GENERAL NOTES

MAXIMUM RAFTER SPANS

- ENCLOSED BUILDING

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE 2010 FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTION BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PS WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALI BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY: 4"OC. UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTOR ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 9/64"; WITH

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

THE BUILDER AND OWN	ER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH AF
SPECIFICALLY NOT PAR	T OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDIT	ONS, FOUNDATION BEARING CAPACITY, GRADE AND
BACKFILL HEIGHT, WIN	D SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS A WITH 2010 FBCR REQUI DESIGN PRESSURES.	ND CONSTRUCTION TECHNIQUES, WHICH COMPLY REMENTS FOR THE STATED WIND VELOCITY AND
DESIGN PRESSURES.	REMENTS FOR THE STATED WIND VELOCITY AND JS LOAD PATH FROM TRUSSES TO FOUNDATION. I

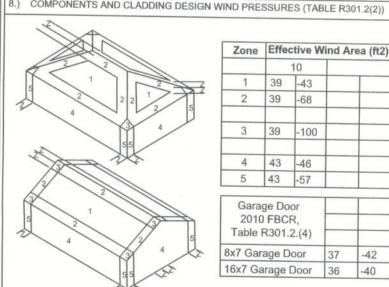
BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED OADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING, THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

WIN	ND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SE
(EN	ICLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPEI AN ROOF HEIGHT
BU	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. 0
2.)	WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY
3.)	TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIF
4.)	RISK CATEGORY = II, (MRI = 700 YR)
5.)	ROOF ANGLE = 7-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUIL
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES



~						
-	344	4	43	-46		
2/		5	43	-57		
	2 2 3 5	201	age Do 0 FBC R301	R,		
1	4	8x7 Ga	rage [Door	37	-42
	55 22	16x7 G	arage	Door	36	-40
_	LOADS		arage	Door	36	-40
40	PSF (ALL OTHER DWELLING ROO		arage	Door	36	-40
	OADS 40 PSF (ALL OTHER DWELLING ROOMS)		arage	Door	36	-40

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

JOB NUMBER: 1207031

OF 1 SHEETS

P.E. 53815 **ECTION R301.2.1** ED, OR GABLE ROOFS July 17, 2012

Stanley Crawford Construction

PE No.53915, POB 868, Lake City, FL

Stated dimensions supercede scaled dimensions. Refer all questions to

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form or manner without first the express written

permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have

examined this plan, and that the applicable

portions of the plan, relating to wind engine

LIMITATION: This design is valid for one

Building Code Residential

to the best of my knowledge.

building, at specified location

comply with section R301.2.1, 2010 Florida

Mark Disosway, P.E. for resolution

Do not proceed without clarification

32056, 386-754-5419

IMENSIONS:

REVISIONS

SOFTPLAN

Mark Robinson

ADDRESS:

Addition

Lot 10 The Preserves S/D Lake City, Florida Mark Disosway P.E.

P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 PRINTED DATE:

> July 17, 2012 STRUCTURAL BY: David Disosway

FINALS DATE: 17Jul12

> DRAWING NUMBER S-1

ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

-2X4 OUTRIGGER @ 24" O.C. - 7/16" OSB UNBLOCKED НЗ ЕАСН — -BLOCKING REQUIRED BETWEEN OUT RIGGERS -(4) .131"X3 1/4"-NAILS (4) .131"X3 1/4" --(4) .131"X3 1/4" INSTALL 2X4 SPF#2 DIAGONAL BRACE-AND NAIL TO BLOCKING AT TOP CHORD & BOTTOM CHORD AND RAT RUN @ 6' O.C DIAGONAL BRACE MUST EDGE & 12" O.C. FIELD BE NAILEDTO TRUSS WEBS FOR LENGTHOVER 12' IT ATTACH RAT RUN TO-MAY BE "T" BRACED UP BLOCKING w/ TO 12' AND UNBRACED (4) .131"X3 1/4" NAILS TOE NAIL TRUSS-(4) .131"X3 1/4" · O TOP PLATE NAILS

12d @ 6" O.C. SIMPSON LSTA21 w/ (8) -16d TO TRUSS -2X4X8' RAT RUN NAIL EACH CONNECTION w/ (4) .131"X3 1/4" NAILS & (8) -16d TO WALL @ 48" O.C. U.N.O. -(4) .131"X3 1/4" NAILS -(8) .131"X3 1/4" NAILS - 2X4 SPF#2 BLOCKING - H3 INSTALLED HORIZONTALLY

SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C. FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

(TYP.) GABLE BRACING DETAIL WOOD FRAME

12-16d 5/8" ANCHOR 12-16d 5/8" ANCHOR

18-16d

2-5/8" ANCHOR

2200

2300

2320

RETROFIT ROOF OVER FRAMING & BRACING DETAIL