DATE 12/05/2006

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT
000025276

APPLICANT CHARLENE CRAWFORD PHONE 386.961.1086

ADDRESS 1964 SW CYPRESS LAKE ROAD LAKE CITY FL 32024

OWNER GATEWAY DEVELOPERS OF LAKE CITY,LLC PHONE 386.961.1086

ADDRESS 169 SW FIELDSTONE COURT LAKE CITY FL 32055

CONTRACTOR J. MACK LIPSCOMB PHONE 386.623.9141

LOCATION OF PROPERTY 90-W TO HEATHRIDGE,TL TO FIELDSTONE,TR AND IT'S THE 5TH LOT ON L.

TYPE DEVELOPMENT SFD/UTILITY ESTIMATED COST OF CONSTRUCTION 110000.00

HEATED FLOOR AREA 2200.00 TOTAL AREA 2967.00 HEIGHT STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 8'12 FLOOR CONC

LAND USE & ZONING RSF-2 MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE XPP DEVELOPMENT PERMIT NO.

PARCEL ID 33-3S-16-02438-146 SUBDIVISION EMERALD COVE

LOT 46 BLOCK PHASE 1 UNIT TOTAL ACRES 0.50

000001268		CBC1253543	Marlene Crawford
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

18"X32'MITERED	06-1036-N	BLK	JTH	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: NOC ON FILE. 1 FOOT ABOVE ROAD. LETTER OF AUTH. ON FILE.

Check # or Cash 3815

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by

Foundation _____ date/app. by

Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by

Slab _____ date/app. by

Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by

Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by

Heat & Air Duct _____ date/app. by

Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by

C.O. Final _____ date/app. by

Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by

Pool _____ date/app. by

Reconnection _____ date/app. by

Pump pole _____ date/app. by

Utility Pole _____ date/app. by

M/H Pole _____ date/app. by

Travel Trailer _____ date/app. by

Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 550.00 CERTIFICATION FEE $ 14.84 SURCHARGE FEE $ 14.84

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $

FLOOD DEVELOPMENT FEE $ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ 25.00 **TOTAL FEE** 679.68

INSPECTORS OFFICE CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

This Permit Must Be Prominently Posted on Premises During Construction

PLEASE NOTIFY THE COLUMBIA COUNTY BUILDING DEPARTMENT AT LEAST 24 HOURS IN ADVANCE OF EACH INSPECTION, IN ORDER THAT IT MAY BE MADE WITHOUT DELAY OR INCONVIENCE, PHONE 758-1008. THIS PERMIT IS NOT VALID UNLESS THE WORK AUTHORIZED BY IT IS COMMENCED WITHIN 6 MONTHS AFTER ISSUANCE.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

PREPARED BY AND RETURN TO:

TERRY McDAVID
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

Property Appraiser's
Identification Number

TM File No: 05-631

Inst:2005022542 Date:09/14/2005 Time:14:45
Doc Stamp-Deed : 1360.10
DC,P.DeWitt Cason,Columbia County B:1058 P:841

WARRANTY DEED

This Warranty Deed, made this 7th day of September, 2005, BETWEEN D D P CORPORATION, a Florida corporation, whose post office address is 4158 US Highway 90 West, Lake City, Florida 32055, of the County of Columbia, State of Florida, grantor and GATEWAY DEVELOPERS OF LAKE CITY, LLC, A Florida Limited Liability Company, whose Document number is L04000093284 and whose FEI number is 202222207 and whose post office address is 2806 West US Highway 90, Suite 101, Lake City, FL 32055, of the County of Columbia, State of Florida, grantee*.

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth: that said grantor, for and in consideration of the sum of Ten Dollars ($10.00), and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in Columbia County, Florida, to-wit:

Lots 43, 44, 45, 46 and 47, Emerald Cove, Phase 1, a subdivision according to the plat thereof recorded in Plat Book 8, Pages 35-36, public records, Columbia County, Florida.

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And subject to taxes for the current year and later years and all valid easements and restrictions of record, if any, which are not hereby reimposed; and also subject to any claim, right, title or interest arising from any recorded instrument reserving, conveying, leasing, or otherwise alienating any interest in the oil, gas and other minerals. And grantor does warrant the title to said land and will defend the same against the lawful claims of all persons whomsoever, subject only to the exceptions set forth herein.

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

D D P CORPORATION

(Signature of First Witness)

(Typed Name of First Witness)

BY: _____ (SEAL)
O. P. Daughtry, III,
President

(Corporate Seal)

(Signature of Second Witness)

(Typed Name of Second Witness)

Inst:2005022542 Date:09/14/2005 Time:14:45
Doc Stamp-Deed : 1380.10
DC,P.DeWitt Cason,Columbia County B:1058 P:842

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 7th day of September, 2005, by O. P. Daughtry, III, President of D D P Corporation, a Florida corporation, on behalf of said corporation, who is personally known to me or who has produced Personal Knowledge as identification and who did not take an oath.

My Commission Expires:

Notary Public
Printed, typed, or stamped name:

TERRY MCDAVID
MY COMMISSION EXPIRES
January 16, 2008
DD 079305
Bonded thru
Atlantic Bonding Co., Inc.
NOTARY PUBLIC STATE OF FLORIDA

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in the Notice of Commencement.

1. Description of property: Lot 46 Emerald Cove Subdivision

2. General description of improvement: Construction of Dwelling

3. Owner information:
   a. Name and address: Gateway Developers of Lake City, LLC
      872 SW Jaguar Drive
      Lake City, FL 32025

   b. Interest in property: Fee Simple

   c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: James Mack Lipscomb

5. Surety n/a
   a. Name and address:
   b. Amount of bond:

Inst:2006027717 Date:11/22/2006 Time:13:31
DC,P.DeWitt Cason,Columbia County B:1102 P:2203

6. Lender: Mercantile

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: None

8. In addition to himself, Owner designates_____ to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.

9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).

_____ Signature of Owner

The foregoing instrument was acknowledged before me this 21 day of November, 2006
by _Tom Eagle_, who are personally known to me and who did not take an oath.

_____ Notary Public

My commission expires:_____

Susan L. Holton
Commission #DD431203
Expires: MAY 19, 2009
www.AaronNotary.com

Columbia County Building Permit Application

For Office Use Only Application # 0611-51 Date Received 11.22.06 By CT Permit # 26812527

Application Approved by - Zoning Official BLK Date 05.12.06 Plans Examiner OK JTH Date 1-4-06

Flood Zone X & plat Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Den

Comments (Mod) (# 3815)

☒ NOC ☒ EH ☐ Deed or PA ☒ Site Plan N/A ☐ State Road Info ☐ Parent Parcel # ☐ Development Perm

Name Authorized Person Signing Permit Charlene Crawford Fax 386 719 9586

Address 1964 SW Cypress Rd Lake City FL. 32024 Phone 386 867 0514

Owners Name Gateway Development of Lake City LLC Phone 961-1056

911 Address 169 SW Fieldstone Ct Lake City FL.

Contractors Name James Mack Lipscomb Phone 623-9141

Address 755 SE Woods Terrance Lake City FL. 32025

Fee Simple Owner Name & Address n/a

Bonding Co. Name & Address n/a

Architect/Engineer Name & Address NTC Design Group P.O. Box 87 Lindale TX

Mortgage Lenders Name & Address n/a

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 33-3S-16-02438-146 Estimated Cost of Construction $150,000.00

Subdivision Name Emerald Cove Lot 46 Block ___ Unit ___ Phase ___

Driving Directions 90 West turn left on Heathridge Turn Right on Fieldstone, 3rd lot on left.

Type of Construction SFD

Total Acreage .5 Lot Size .5 Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Number of Existing Dwellings on Property 0

Actual Distance of Structure from Property Lines - Front 32'6" Side 25'6" Side 49'3" Rear 38' 107'

Total Building Height ___ Number of Stories 1 Heated Floor Area 2200 TOTAL 2967 Roof Pitch ___

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Authorized Person by Notarized Letter

James M Lipscomb

Contractor Signature

Contractors License Number CBC 1253523

Competency Card Number

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me this 21 day of November 2006.

Personally known ✓ or Produced Identification ___

Susan L. Holton Commission #DD431203 Expires: MAY 19, 2009 www.AaronNotary.com

NOTARY STAMP/SEAL

Susan L. Holton

Notary Signature

(Revised Sept. 2006)

JW ADVISED CHARLENE 12.5.06

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001268

DATE 12/05/2006 PARCEL ID # 33-3S-16-02438-146

APPLICANT CHARLENE CRAWFORD PHONE 386.961.1086

ADDRESS 1964 SW CYPRESS LAKE ROAD LAKE CITY FL 32024

OWNER GATEWAY DEVELOPERS OF LAKE CITY,LLC PHONE 386.961.1086

ADDRESS 169 SW FIELDSTONE COURT LAKE CITY FL 32055

CONTRACTOR J. MACK LIPSCOMB PHONE 386.623.9141

LOCATION OF PROPERTY 90-W TO HEATHRIDGE,TL TO FIELDSTONE,TR AND IT'S THE 5TH LOT ON L.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT EMERALD COVE 46 1

SIGNATURE Charlene Crawford

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

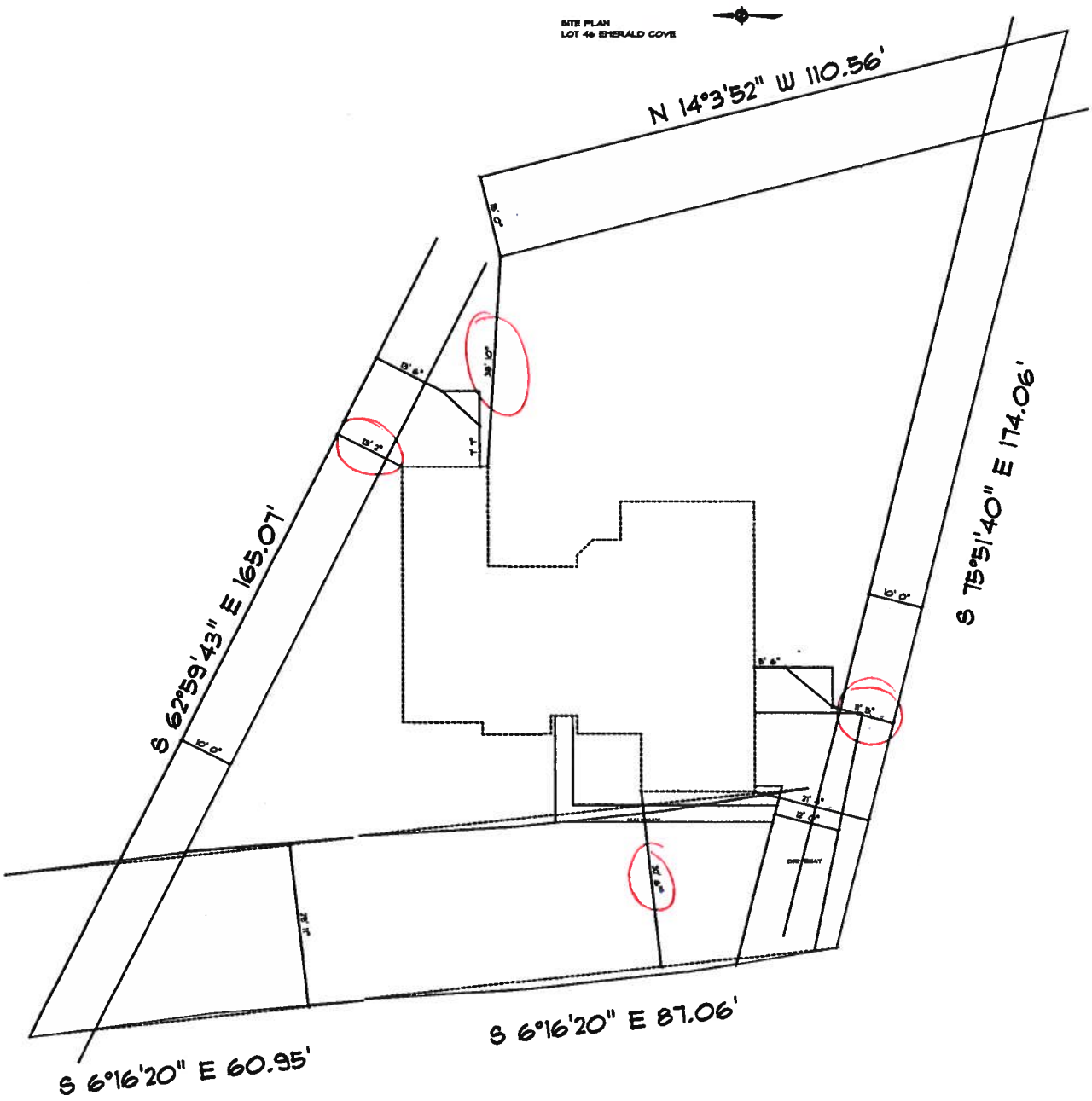
[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

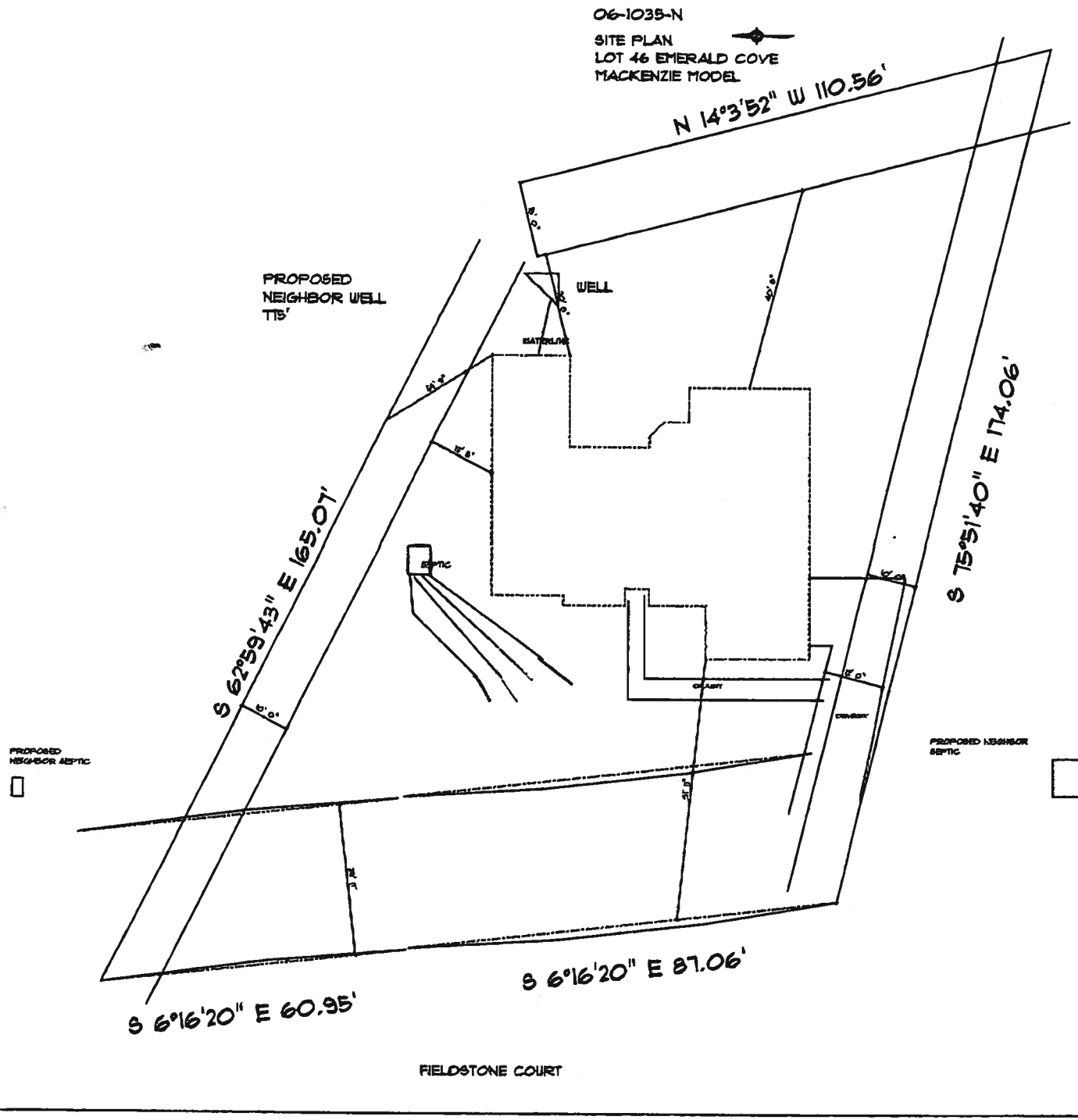
ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00


SITE PLAN
LOT 46 EMERALD COVE
N 14°3'52" W 110.56'
S 62°59'43" E 165.07'
S 75°51'40" E 174.06'
S 6°16'20" E 87.06'
S 6°16'20" E 60.95'
FIELDSTONE COURT

06-1035-N



PROPOSED WELL

1"=30'

APPROVED
Columbia CHD
12-4-06
EHS

S. Holtz
12/1/06

RECEIVED
12-4-06

No. 3083 P. 4

Dec. 1. 2006 3:10PM

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

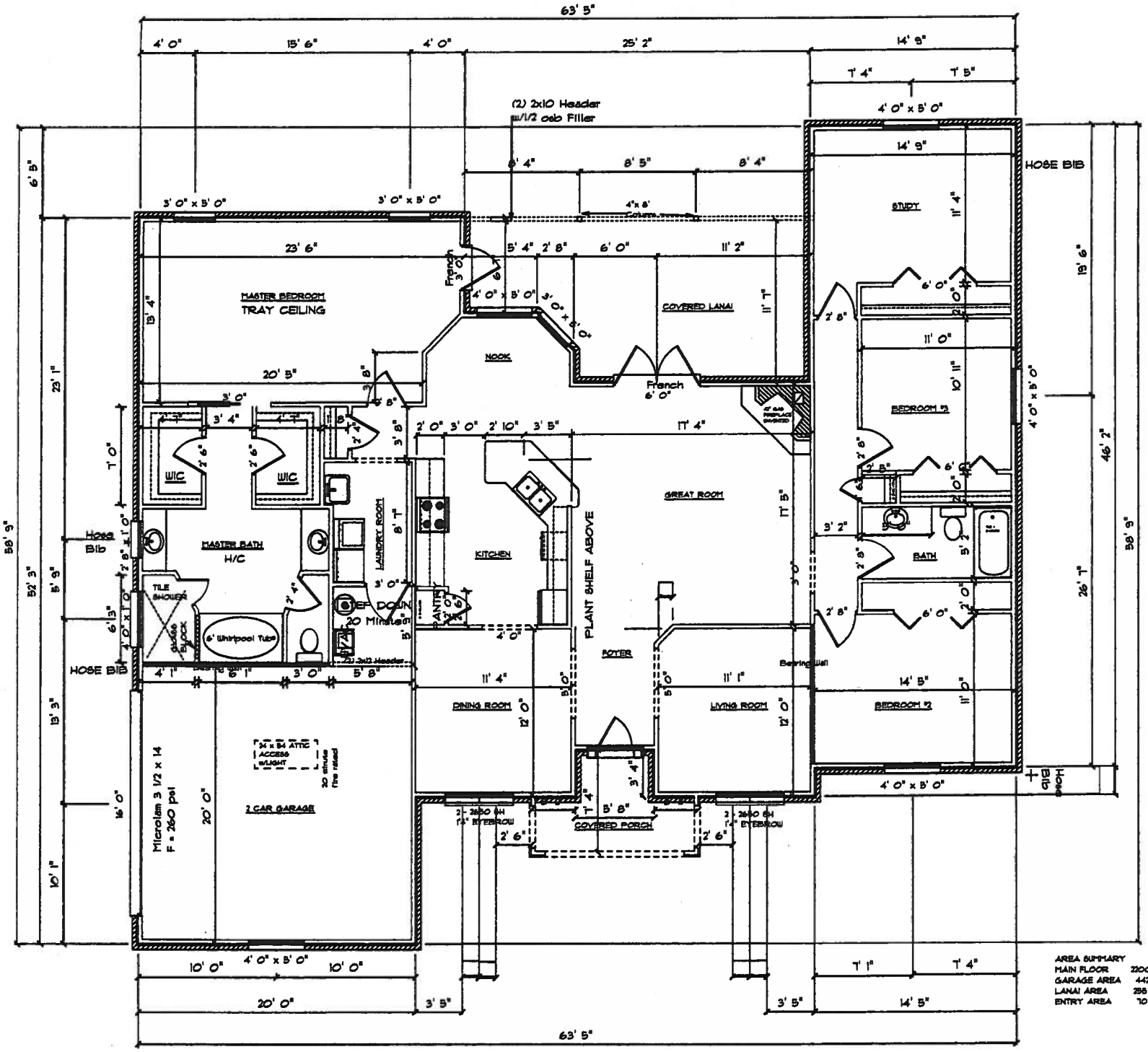
NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk


63' 5"
(2) 2x10 Header w/1/2 osb Filler
4' 0" x 5' 0"
HOSE BIB
STUDY
MASTER BEDROOM
TRAY CEILING
COVERED LANAI
NOOK
French
BEDROOM #3
GREAT ROOM
KITCHEN
LAUNDRY ROOM
MASTER BATH
H/C
WIC
TILE SHOWER
6' Whirlpool Tub
BATH
PLANT SHELF ABOVE
FOYER
DINING ROOM
LIVING ROOM
BEDROOM #2
2 CAR GARAGE
COVERED PORCH
Microlam 3 1/2 x 14
F = 260 psi
AREA SUMMARY
MAIN FLOOR 2200 SF
GARAGE AREA 442 SF
LANAI AREA 258 SF
ENTRY AREA 70 SF

FORM 600A-2004 EnergyGauge® 4.21

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**MacKenzie Residece**	Builder:	
Address:	**Lot: , Sub: , Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	25276
Owner:	**Gateway Developement, LLC**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 36.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 12.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes		
6. Conditioned floor area (ft²)	2200 ft²	c. N/A	
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)			
a. U-factor:	Description Area	13. Heating systems	
(or Single or Double DEFAULT)	7a. (Dble Default) 248.3 ft²	a. Electric Heat Pump	Cap: 36.0 kBtu/hr
b. SHGC:			HSPF: 7.00
(or Clear or Tint DEFAULT)	7b. (Clear) 248.3 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 253.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 40.0 gallons
a. Frame, Wood, Exterior	R=13.0, 1870.0 ft²		EF: 0.97
b. N/A		b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar	
10. Ceiling types		DHP-Dedicated heat pump)	
a. Under Attic	R=30.0, 2422.0 ft²	15. HVAC credits	PT,
b. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
c. N/A		HF-Whole house fan,	
11. Ducts		PT-Programmable Thermostat,	
a. Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 153.0 ft	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	

Glass/Floor Area: 0.11 Total as-built points: 29844 Total base points: 33806 **PASS**

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Gary Gall

DATE: 7/18/08

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 28206.1			**Summer As-Built Points:**					**25273.7**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 36000 btuh ,SEER/EFF(12.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)					
			25274	1.00	(1.09 x 1.147 x 0.91)	0.284	0.950	7769.2
28206.1	**0.4266**	**12032.7**	**25273.7**	**1.00**	**1.138**	**0.284**	**0.950**	**7769.2**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**17904.4**	**Winter As-Built Points:**					**22455.8**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 36000 btuh ,EFF(7.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0					
17904.4	0.6274	11233.2	22455.8	1.00	1.162	0.487	0.950	12077.7

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

Project Information for:	L166852		
Builder:	LIPSCOMB EAGLE	Date:	5/25/2006
Lot:	LOT 46	Start Number:	**1186**
Subdivision:	N/A		
County or City:	COLUMBIA COUNTY		
Truss Page Count:	52		

Truss Design Load Information (UNO) Design Program: MiTek 5.2 / 6.2

Gravity		**Wind**		**Building Code:**	**FBC2001**
Roof (psf):	**42**	Wind Standard:	**ASCE 7-98**		
Floor (psf):	55	Wind Speed (mph):	**120**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

LIPSCOMB, JAMES MACK CBC1253543
Address: 255 SE WOODS TERRACE
LAKE CITY, FL 32025 Designer: **81**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0525061186	5/25/2006	41	T20	0525061226	5/25/2006
2	CJ3	0525061187	5/25/2006	42	T21	0525061227	5/25/2006
3	CJ3A	0525061188	5/25/2006	43	T22	0525061228	5/25/2006
4	CJ5	0525061189	5/25/2006	44	T23	0525061229	5/25/2006
5	CJ5A	0525061190	5/25/2006	45	T26	0525061230	5/25/2006
6	EJ5	0525061191	5/25/2006	46	T27	0525061231	5/25/2006
7	EJ7	0525061192	5/25/2006	47	T28	0525061232	5/25/2006
8	EJ7A	0525061193	5/25/2006	48	T29	0525061233	5/25/2006
9	HJ5	0525061194	5/25/2006	49	T31	0525061234	5/25/2006
10	HJ7	0525061195	5/25/2006	50	T32	0525061235	5/25/2006
11	HJ7A	0525061196	5/25/2006	51	T33	0525061236	5/25/2006
12	PB01	0525061197	5/25/2006	52	T34	0525061237	5/25/2006
13	PB02	0525061198	5/25/2006				
14	PB03	0525061199	5/25/2006				
15	PB04	0525061200	5/25/2006				
16	PB05	0525061201	5/25/2006				
17	PB06	0525061202	5/25/2006				
18	PB07	0525061203	5/25/2006				
19	PB08	0525061204	5/25/2006				
20	PB09	0525061205	5/25/2006				
21	PB10	0525061206	5/25/2006				
22	PB11	0525061207	5/25/2006				
23	T01	0525061208	5/25/2006				
24	T02	0525061209	5/25/2006				
25	T03	0525061210	5/25/2006				
26	T04	0525061211	5/25/2006				
27	T05	0525061212	5/25/2006				
28	T06	0525061213	5/25/2006				
29	T07	0525061214	5/25/2006				
30	T08	0525061215	5/25/2006				
31	T09	0525061216	5/25/2006				
32	T10	0525061217	5/25/2006				
33	T11	0525061218	5/25/2006				
34	T12	0525061219	5/25/2006				
35	T13	0525061220	5/25/2006				
36	T15	0525061221	5/25/2006				
37	T16	0525061222	5/25/2006				
38	T17	0525061223	5/25/2006				
39	T18	0525061224	5/25/2006				
40	T19	0525061225	5/25/2006				

MAY 25 2006

Dwg.#0525061238

Licensee Details

Licensee Information

Name:	**LIPSCOMB, JAMES MACK (Primary Name)**
	LIPSCOMB & EAGLE DEVELOPMENT INC (DBA Name)
Main Address:	**255 SE WOODS TERRACE**
	LAKE CITY Florida 32025
County:	**COLUMBIA**
License Mailing:	
LicenseLocation:	**2806 U S HWY WEST SUITE 101**
	LAKE CITY FL 32055
County:	**COLUMBIA**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC1253543**
Status:	**Current,Active**
Licensure Date:	**11/02/2005**
Expires:	**08/31/2006**

Special Qualifications	Qualification Effective
Qualified Business License Required	11/02/2005

View Related License Information

View License Complaint

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061186

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	CJ1	MONO TRUSS	14	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:39 2006 Page 1

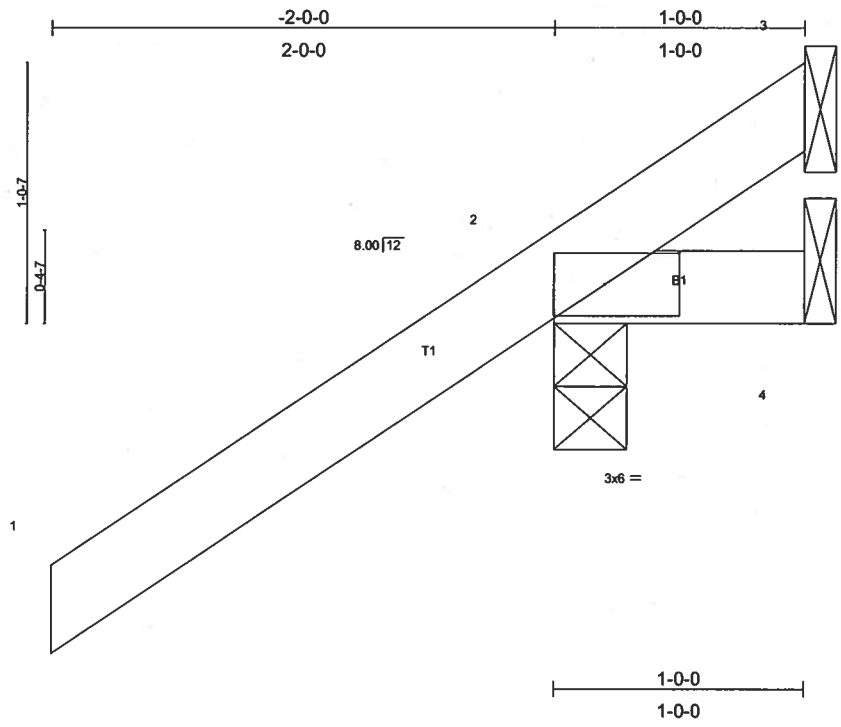


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=137(load case 5)
Max Uplift2=-357(load case 5), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=176(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-84/121
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 357 lb uplift at joint 2 and 90 lb uplift at joint 3.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061187

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	CJ3	MONO TRUSS	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:40 2006 Page 1

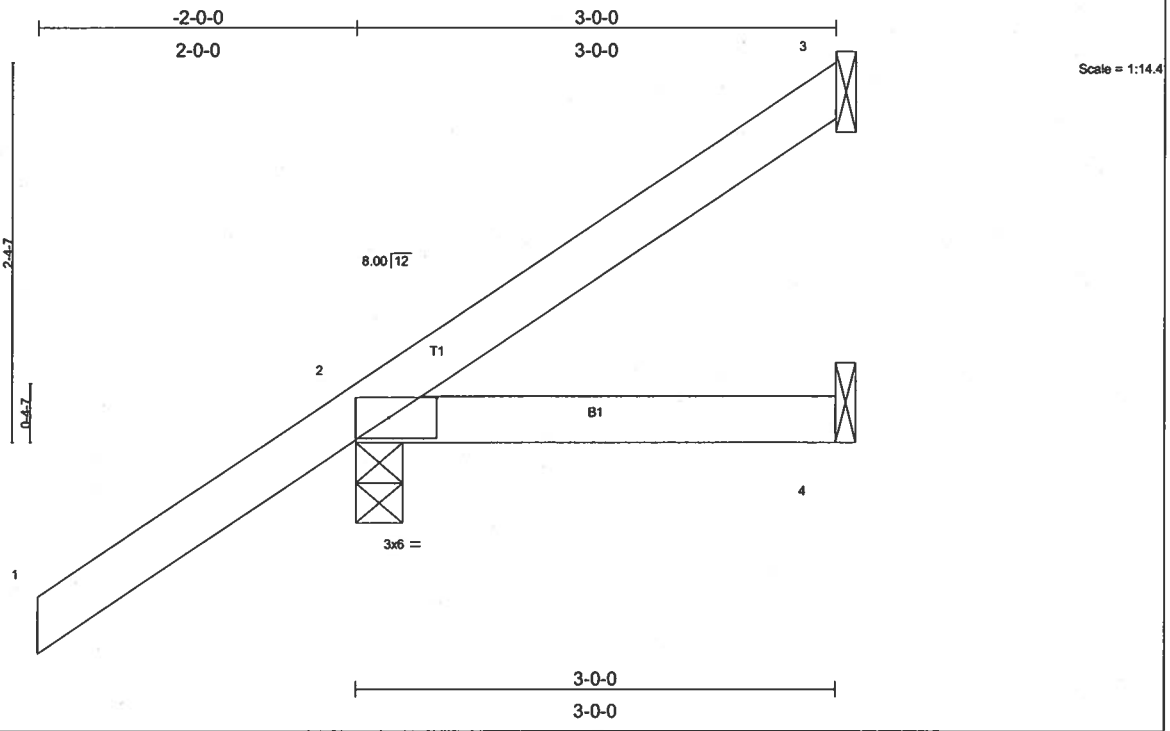


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.00	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 14 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=209(load case 5)
Max Uplift3=-35(load case 4), 2=-243(load case 5)
Max Grav 3=38(load case 3), 2=278(load case 1), 4=42(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-70/21
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 3 and 243 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061188

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	CJ3A	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:40 2006 Page 1

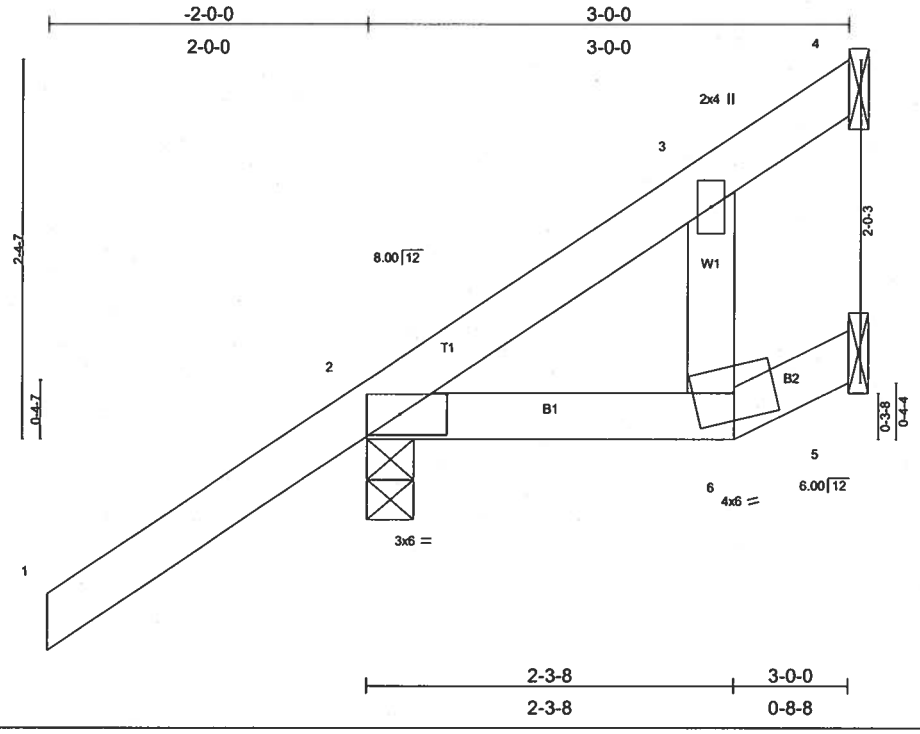


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=63/Mechanical, 2=278/0-3-8, 5=10/Mechanical
Max Horz 2=209(load case 5)
Max Uplift4=-29(load case 4), 2=-243(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-74/17, 3-4=-20/33
BOT CHORD 2-6=-4/0, 5-6=-4/4
WEBS 3-6=0/42

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 4 and 243 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061189

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	CJ5	MONO TRUSS	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:40 2006 Page 1

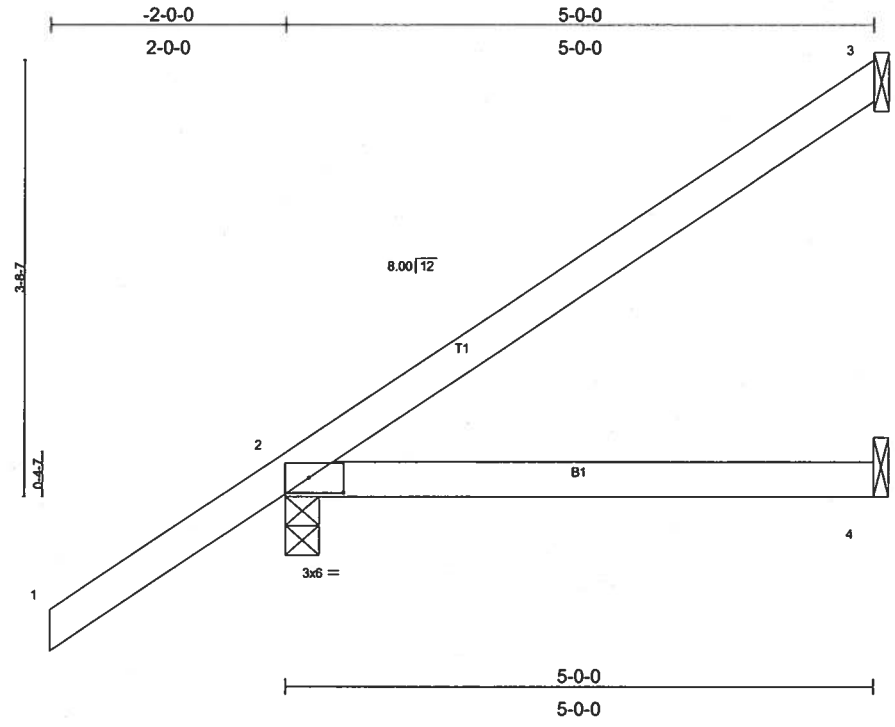


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.04	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=282(load case 5)
Max Uplift3=-129(load case 5), 2=-226(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-104/44
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 3 and 226 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061190

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	CJ5A	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:41 2006 Page 1

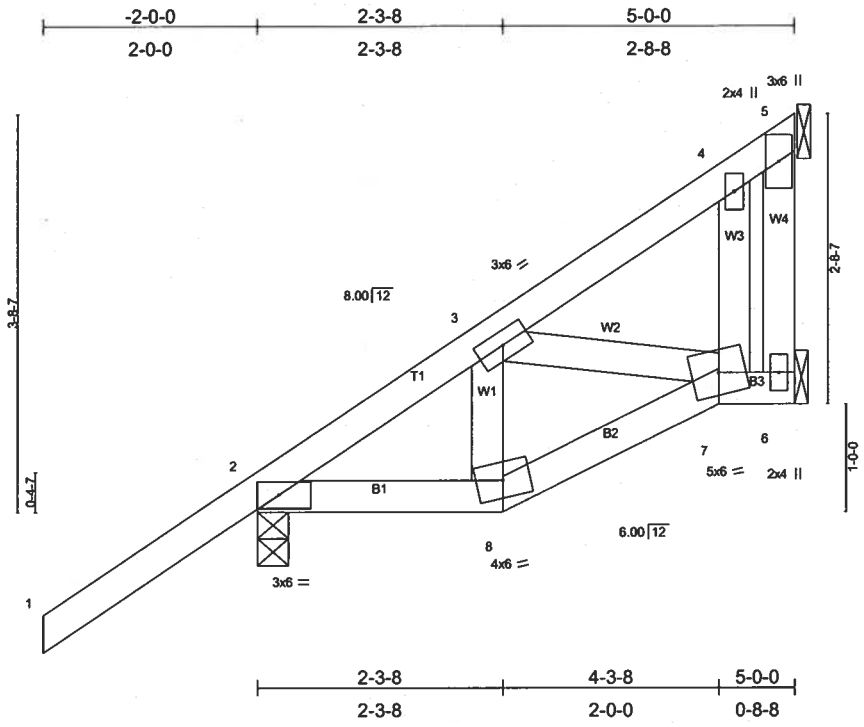


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	0.01	7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	0.01	7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 31 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=163/Mechanical, 6=8/Mechanical, 2=340/0-3-8
Max Horz 2=279(load case 5)
Max Uplift5=-113(load case 5), 2=-226(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-205/0, 3-4=-68/0, 4-5=-72/89, 5-6=0/0
BOT CHORD 2-8=-52/113, 7-8=-56/138, 6-7=-0/0
WEBS 3-8=-23/37, 4-7=-27/119, 3-7=-107/47

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 113 lb uplift at joint 5 and 226 lb uplift at joint 2.
4) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061191

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	EJ5	MONO TRUSS	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:41 2006 Page 1

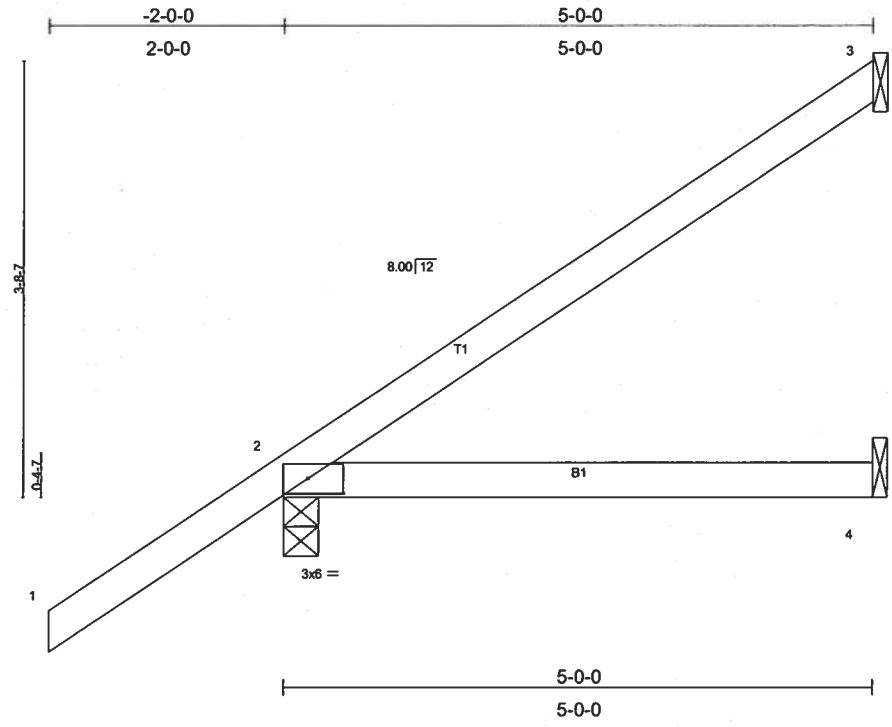


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.14	Vert(TL)	-0.04	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 20 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=282(load case 5)
Max Uplift3=-129(load case 5), 2=-226(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-104/44
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 129 lb uplift at joint 3 and 226 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061192

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	EJ7	MONO TRUSS	27	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:42 2006 Page 1

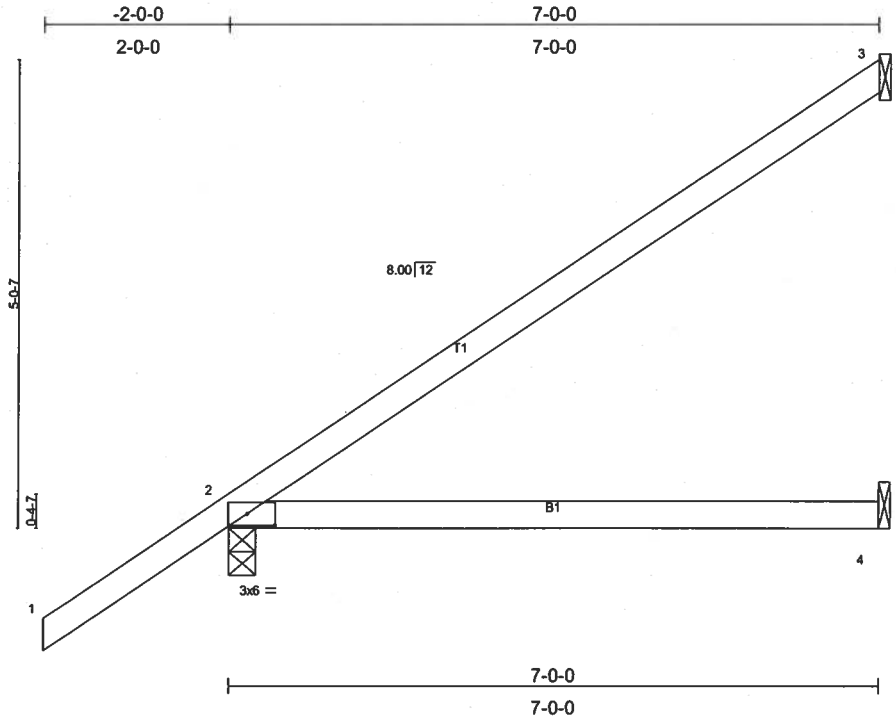


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.11	2-4	>776	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	-0.16	2-4	>517	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=165/Mechanical, 2=419/0-3-8, 4=102/Mechanical
Max Horz 2=355(load case 5)
Max Uplift3=-214(load case 5), 2=-229(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-162/73
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 214 lb uplift at joint 3 and 229 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061193

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	EJ7A	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:42 2006 Page 1

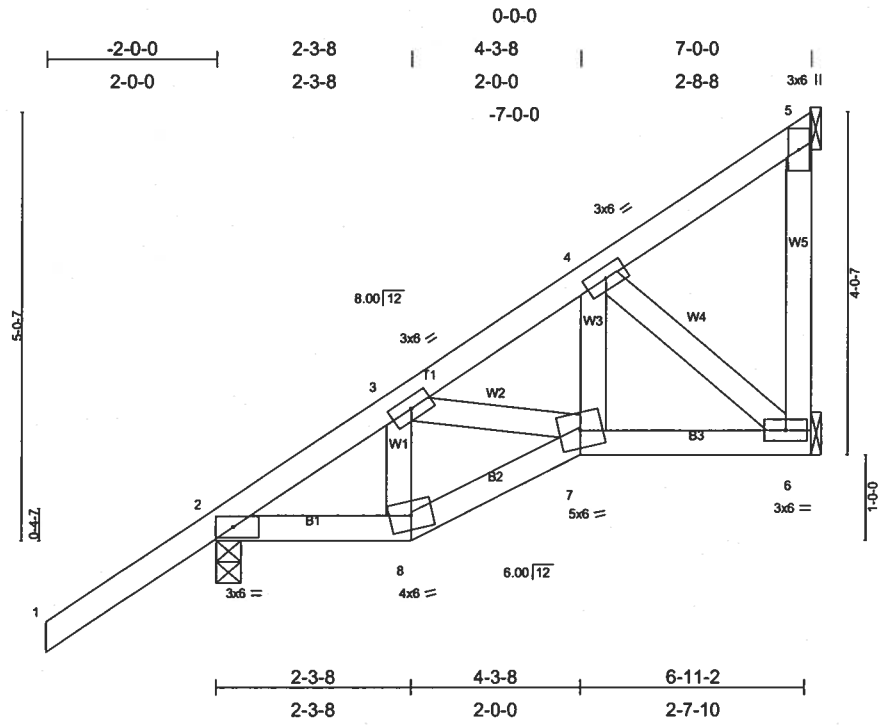


Plate Offsets (X,Y): [2:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=213/Mechanical, 2=416/0-3-8, 5=50/Mechanical
Max Horz 2=352(load case 5)
Max Uplift6=-126(load case 5), 2=-228(load case 5), 5=-64(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-319/0, 3-4=-283/27, 4-5=-49/21, 5-6=0/0
BOT CHORD 2-8=-106/197, 7-8=-114/228, 6-7=-174/211
WEBS 3-8=-36/62, 3-7=-77/29, 4-7=-25/160, 4-6=-274/226

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 6, 228 lb uplift at joint 2 and 64 lb uplift at joint 5.
4) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

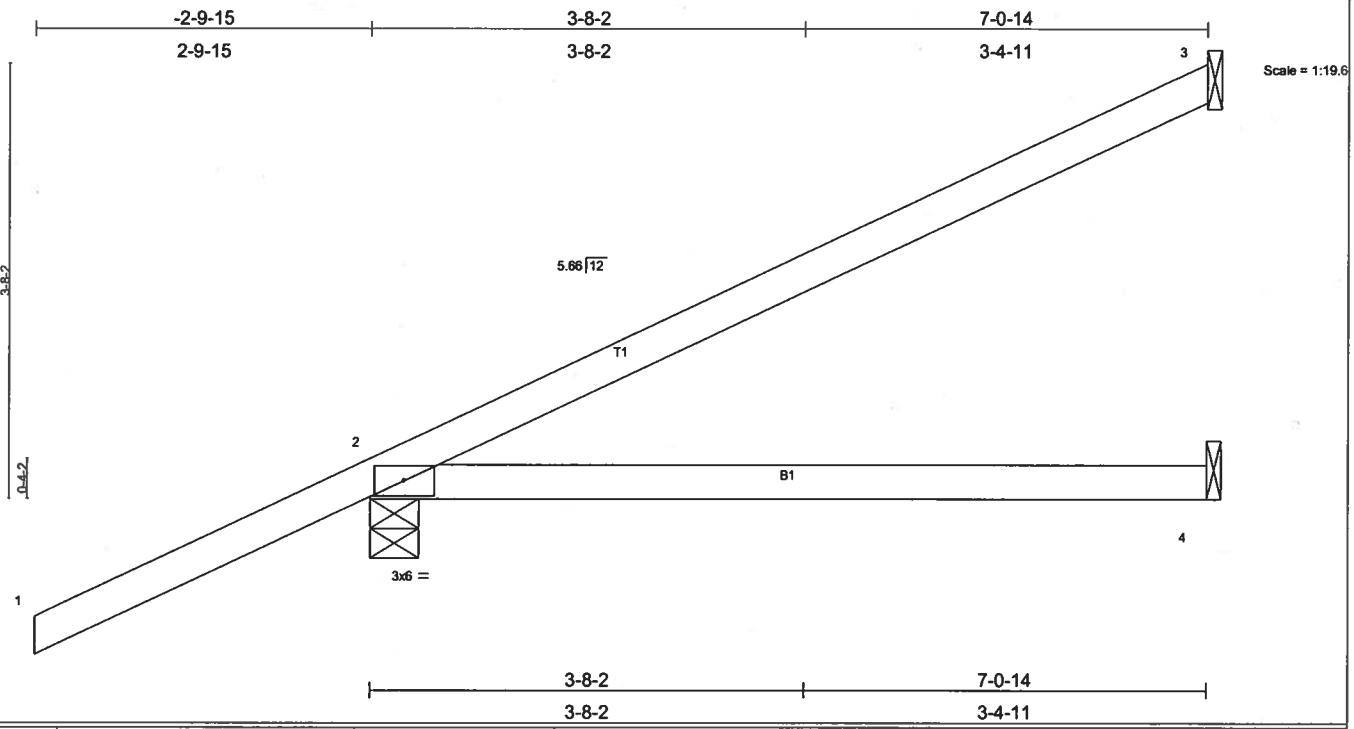
LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061194

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	HJ5	MONO TRUSS	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:43 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.09	2-4	>872	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.14	2-4	>581	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 27 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=183/Mechanical, 2=375/0-4-15, 4=120/Mechanical
Max Horz 2=263(load case 4)
Max Uplift3=-197(load case 4), 2=-279(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/64, 2-3=-110/53
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 197 lb uplift at joint 3 and 279 lb uplift at joint 2.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-4=-53(F=-12, B=-12)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061195

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	HJ7	MONO TRUSS	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:43 2006 Page 1

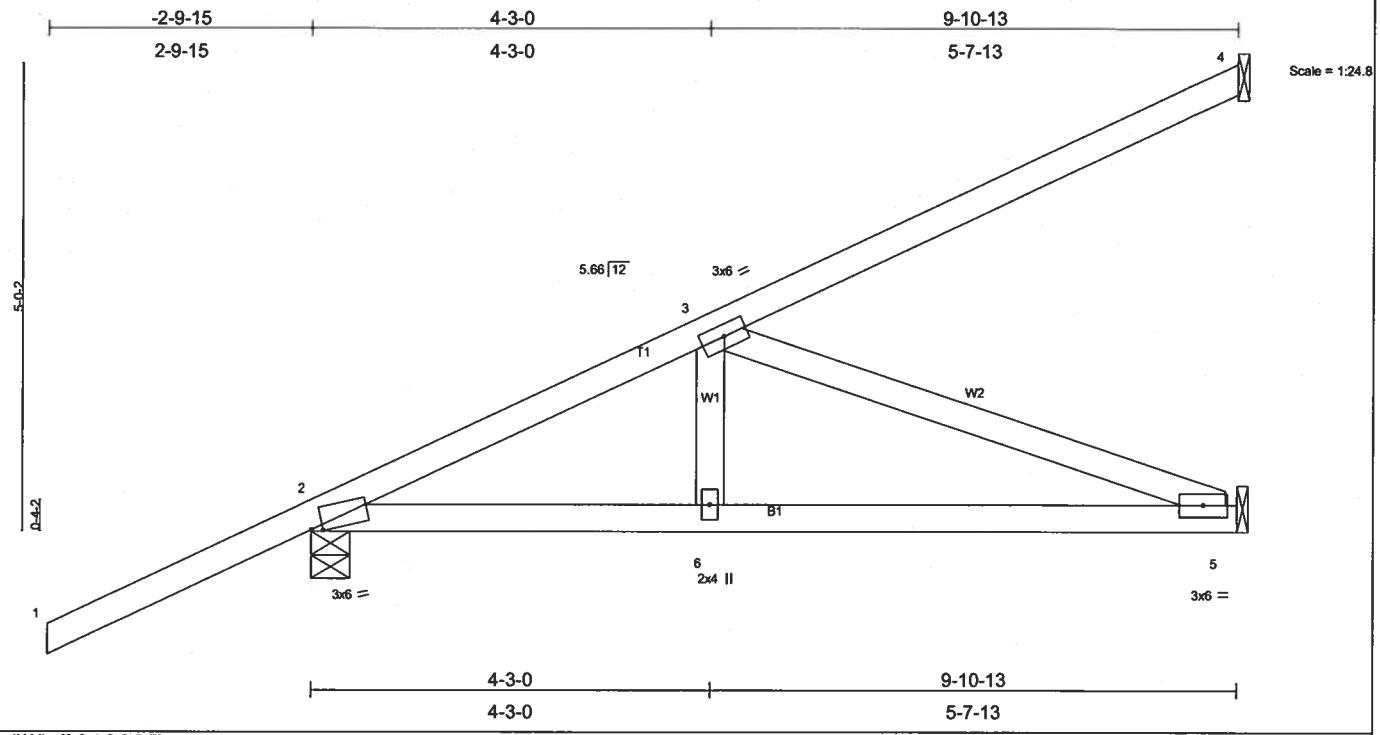


Plate Offsets (X,Y): [2:0-1-8,0-0-7]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.07	5-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.10	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.42	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 47 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-7-6 oc bracing.

REACTIONS (lb/size) 4=266/Mechanical, 2=531/0-4-15, 5=379/Mechanical
Max Horz 2=432(load case 4)
Max Uplift4=-327(load case 4), 2=-295(load case 4), 5=-111(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/64, 2-3=-700/26, 3-4=-175/83
BOT CHORD 2-6=-349/618, 5-6=-349/618
WEBS 3-6=0/165, 3-5=-659/372

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 327 lb uplift at joint 4, 295 lb uplift at joint 2 and 111 lb uplift at joint 5.
4) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061196

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	HJ7A	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:44 2006 Page 1

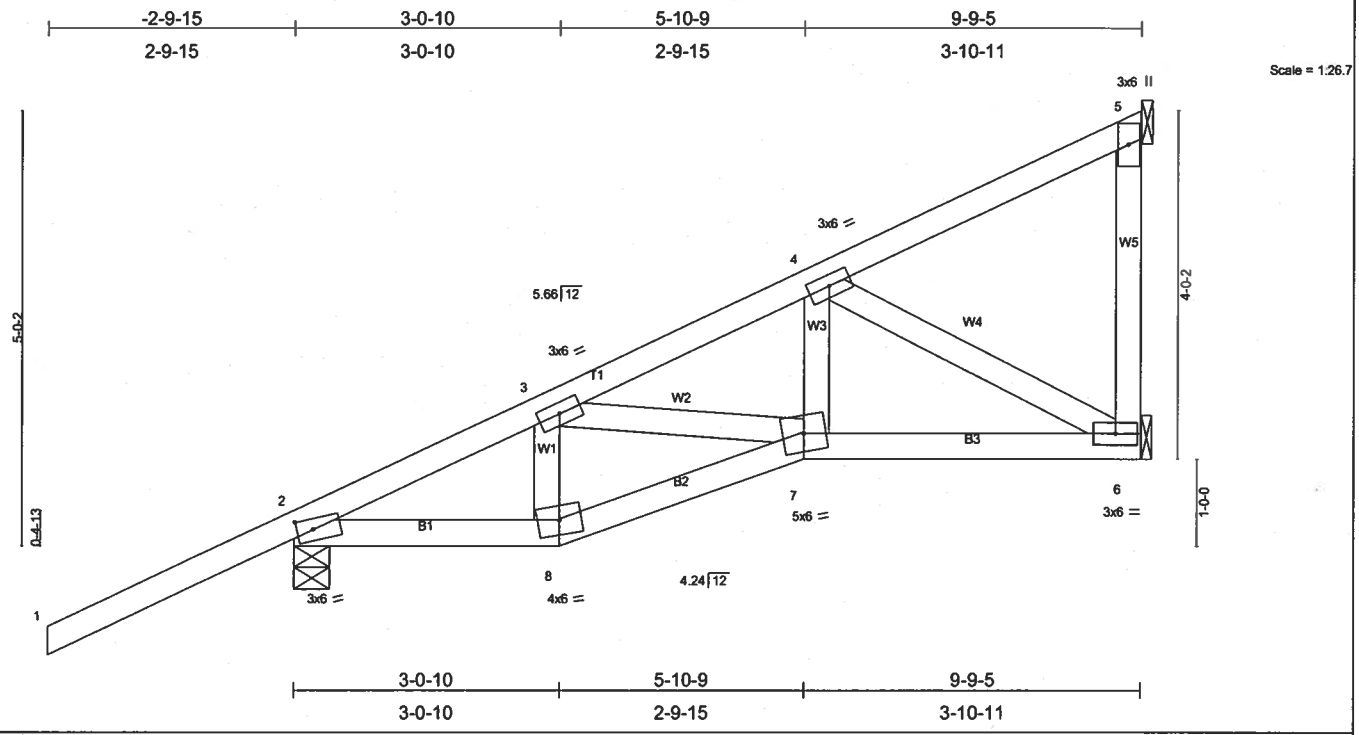


Plate Offsets (X,Y): [2:0-2-4,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.02	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	-0.02	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=187/Mechanical, 6=444/Mechanical, 2=526/0-4-15
Max Horz 2=420(load case 4)
Max Uplift5=-233(load case 4), 6=-195(load case 4), 2=-293(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/62, 2-3=-510/0, 3-4=-737/155, 4-5=-123/44, 5-6=0/0
BOT CHORD 2-8=-175/419, 7-8=-175/453, 6-7=-417/632
WEBS 3-8=-157/161, 3-7=-260/253, 4-7=0/279, 4-6=-672/443

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 5, 195 lb uplift at joint 6 and 293 lb uplift at joint 2.
4) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-5=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-8=-22(F=4, B=4), 8=-22(F=4, B=4)-to-7=-45(F=-7, B=-7), 7=-45(F=-7, B=-7)-to-6=-74(F=-22, B=-22)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

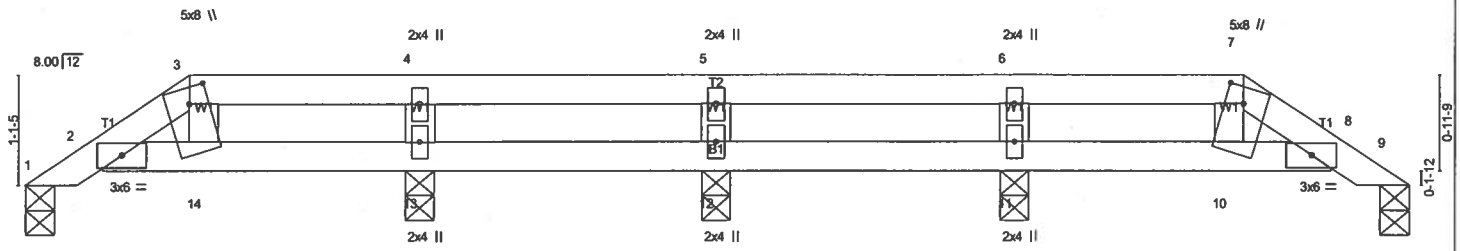
Dwg.#0525061197

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB01	HIP CAP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:44 2006 Page 1

-0	1-7-15	7-0-0	12-4-1	14-0-0	1
0	1-7-15	5-4-1	5-4-1	1-7-15	

Scale = 1:23.5



1-9-11	4-0-0	7-0-0	10-0-5	12-2-5	14-0-0
1-9-11	2-2-5	3-0-0	3-0-5	2-2-0	1-9-11

Plate Offsets (X,Y): [3:0-2-0,0-2-4], [7:0-2-0,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	0.01	14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=145/0-3-8, 9=144/0-3-8, 13=313/0-3-8, 12=213/0-3-8, 11=313/0-3-8
Max Horz 1=42(load case 4)
Max Uplift1=-57(load case 5), 9=-60(load case 6), 13=-186(load case 4), 12=-143(load case 3), 11=-176(load case 3)
Max Grav 1=145(load case 1), 9=144(load case 1), 13=315(load case 7), 12=218(load case 8), 11=314(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-71/49, 2-3=-149/87, 7-8=-148/87, 8-9=-71/46, 3-4=-122/90, 4-5=-122/91, 5-6=-122/91, 6-7=-122/90
BOT CHORD 2-14=-62/119, 13-14=-54/122, 12-13=-54/122, 11-12=-54/122, 10-11=-54/122, 8-10=-59/119
WEBS 3-14=0/38, 7-10=0/34, 4-13=-187/177, 5-12=-148/169, 6-11=-187/175

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 1, 60 lb uplift at joint 9, 186 lb uplift at joint 13, 143 lb uplift at joint 12 and 176 lb uplift at joint 11.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

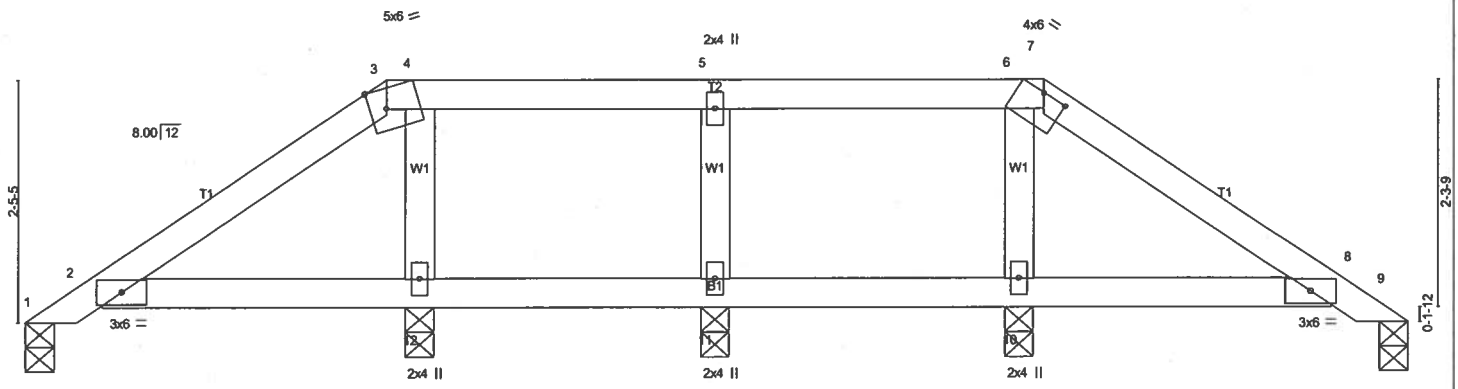
Dwg #0525061198

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB02	HIP CAP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:45 2006 Page 1

-0	3-7-15	10-4-1	14-0-0
0	3-7-15	6-8-2	3-7-15

Scale = 1:23.5



3-9-11	4-0-0	7-0-0	10-2-5	14-0-0
3-9-11	0-2-5	3-0-0	3-2-5	3-9-11

Plate Offsets (X,Y): [3:0-2-0,Edge], [7:0-3-0,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	9	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=66/0-3-8, 9=62/0-3-8, 12=400/0-3-8, 11=203/0-3-8, 10=398/0-3-8
Max Horz 1=98(load case 4)
Max Uplift1=-30(load case 6), 9=-47(load case 6), 12=-178(load case 4), 11=-176(load case 3), 10=-146(load case 6)
Max Grav 1=79(load case 7), 9=73(load case 8), 12=400(load case 1), 11=226(load case 8), 10=398(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-86/90, 2-3=-68/173, 7-8=-54/175, 8-9=-35/29, 3-4=-6/103, 4-5=-6/103, 5-6=-6/103, 6-7=-6/103
BOT CHORD 2-12=-103/112, 11-12=-103/112, 10-11=-103/112, 8-10=-103/112
WEBS 4-12=-269/206, 5-11=-151/192, 6-10=-269/207

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 1, 47 lb uplift at joint 9, 178 lb uplift at joint 12, 176 lb uplift at joint 11 and 146 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

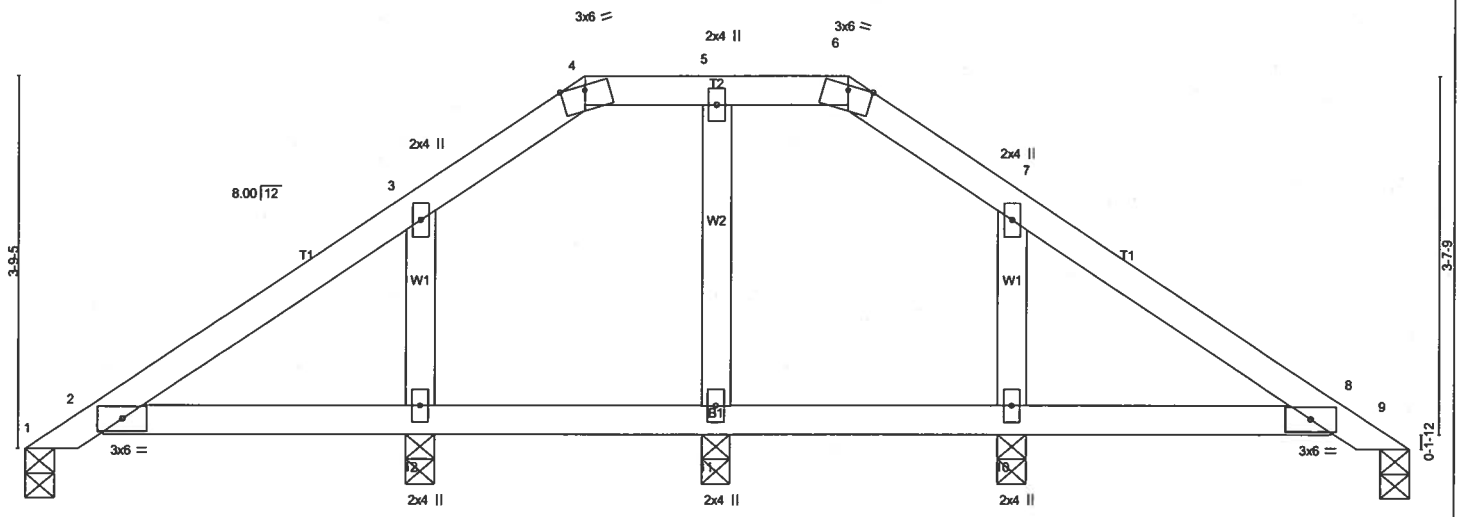
Dwg.#0525061199

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB03	HIP CAP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:45 2006 Page 1

-0	5-7-15	8-4-1	14-0-0	1
0	5-7-15	2-8-2	5-7-15	:

Scale = 1:23.5



4-0-0	5-9-11	7-0-0	8-2-5	10-0-0	14-0-0
4-0-0	1-9-11	1-2-5	1-2-5	1-9-11	4-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 53 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=80/0-3-8, 9=80/0-3-8, 11=272/0-3-8, 12=349/0-3-8, 10=349/0-3-8
Max Horz 1=153(load case 4)
Max Uplift1=-34(load case 3), 9=-27(load case 3), 11=-102(load case 4), 12=-237(load case 5), 10=-220(load case 6)
Max Grav 1=92(load case 7), 9=92(load case 8), 11=272(load case 1), 12=349(load case 1), 10=349(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-144/154, 2-3=-134/149, 3-4=0/84, 6-7=0/80, 7-8=-90/149, 8-9=-44/18, 4-5=0/73, 5-6=0/73
BOT CHORD 2-12=-72/146, 11-12=-72/146, 10-11=-72/146, 8-10=-72/146
WEBS 5-11=-207/124, 3-12=-217/229, 7-10=-217/220

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 1, 27 lb uplift at joint 9, 102 lb uplift at joint 11, 237 lb uplift at joint 12 and 220 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

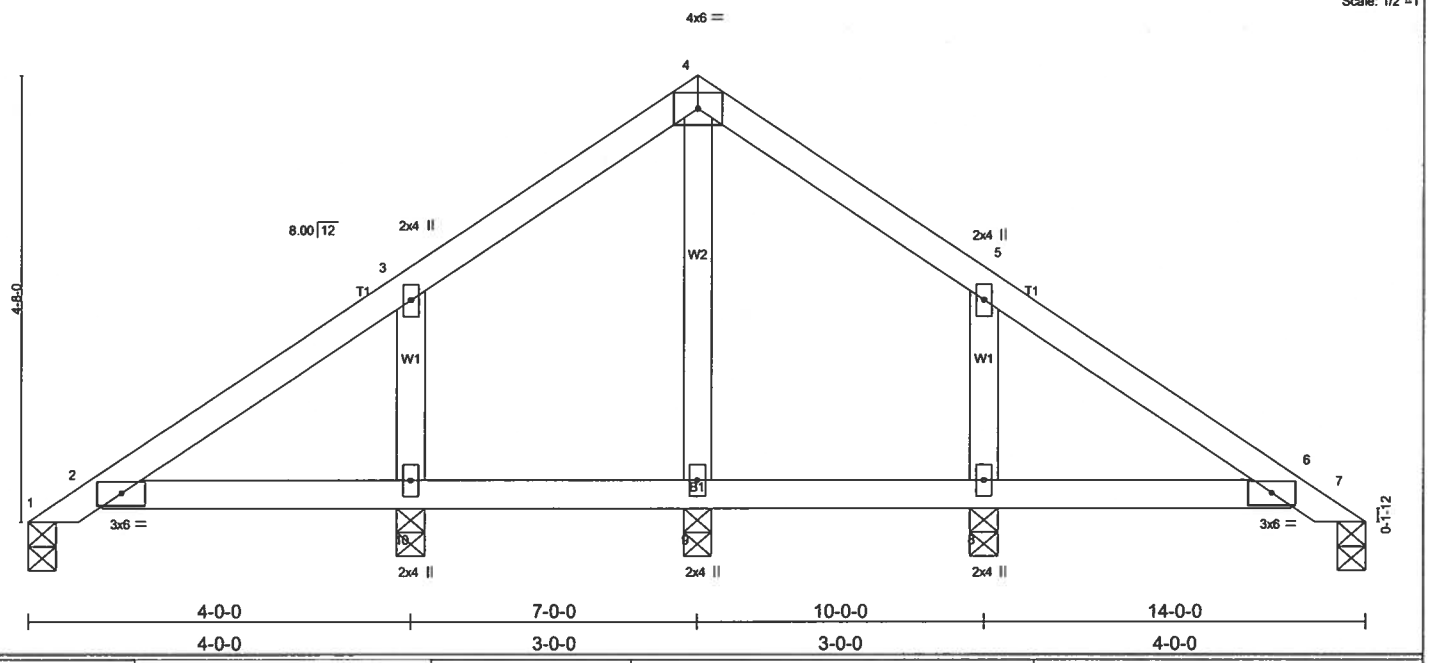
Dwg.#0525061200

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB04	PIGGYBACK	4	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:46 2006 Page 1

-2-0-0 | 7-0-0 | 14-0-0 | 16-
2-0-0 | 7-0-0 | 7-0-0 | 2-

Scale: 1/2"=1'



4-0-0 | 7-0-0 | 10-0-0 | 14-0-0
4-0-0 | 3-0-0 | 3-0-0 | 4-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.13	Vert(LL)	0.01	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=62/0-3-8, 7=62/0-3-8, 9=349/0-3-8, 10=328/0-3-8, 8=328/0-3-8
Max Horz 1=189(load case 4)
Max Uplift1=-50(load case 3), 7=-21(load case 6), 9=-54(load case 5), 10=-254(load case 5), 8=-243(load case 6)
Max Grav 1=75(load case 7), 7=75(load case 8), 9=349(load case 1), 10=336(load case 7), 8=336(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-192/193, 2-3=-158/189, 3-4=-49/168, 4-5=-13/168, 5-6=-103/189, 6-7=-36/15
BOT CHORD 2-10=-107/155, 9-10=-107/155, 8-9=-107/155, 6-8=-107/155
WEBS 4-9=-282/82, 3-10=-207/248, 5-8=-207/243

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 1, 21 lb uplift at joint 7, 54 lb uplift at joint 9 , 254 lb uplift at joint 10 and 243 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

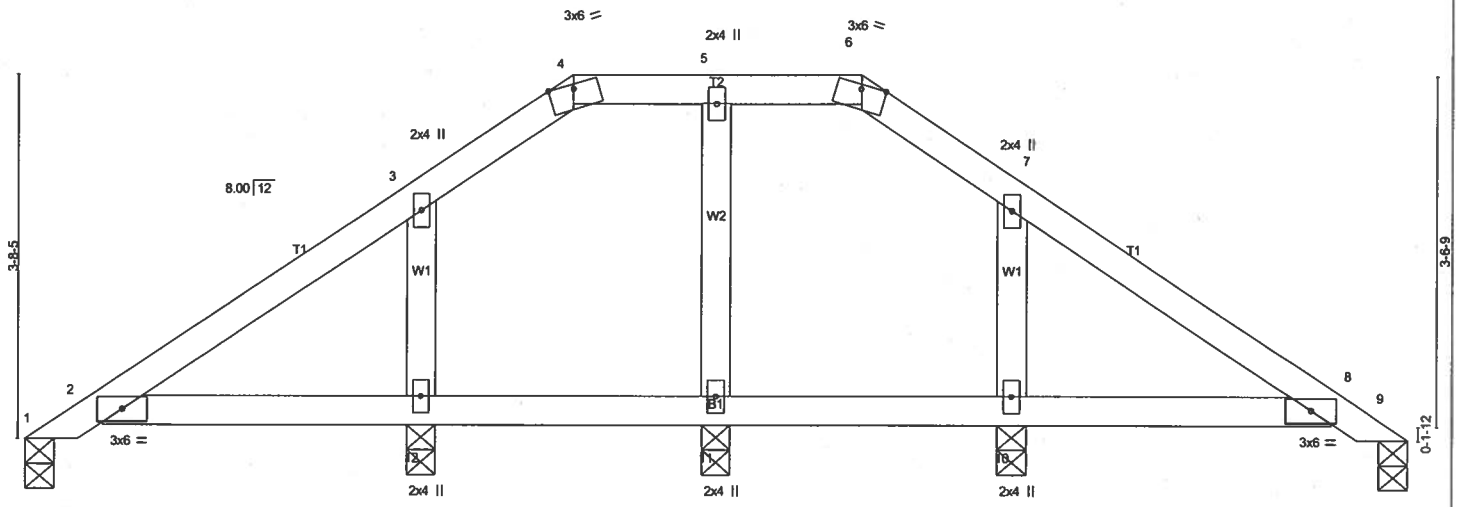
Dwg.#0525061201

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB05	HIP CAP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:46 2006 Page 1

	5-6-7	8-5-9	14-0-0	
-0	5-6-7	2-11-2	5-6-7	1

Scale = 1:23.5



	4-0-0	5-8-3	7-0-0	8-3-13	10-0-0	14-0-0
	4-0-0	1-8-3	1-3-13	1-3-13	1-8-3	4-0-0

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 53 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=80/0-3-8, 9=80/0-3-8, 12=351/0-3-8, 10=351/0-3-8, 11=266/0-3-8
Max Horz 1=150(load case 4)
Max Uplift1=-33(load case 3), 9=-28(load case 3), 12=-234(load case 5), 10=-216(load case 6), 11=-108(load case 4)
Max Grav 1=93(load case 7), 9=93(load case 8), 12=351(load case 1), 10=351(load case 1), 11=266(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-140/151, 2-3=-133/149, 3-4=-0/83, 6-7=-0/81, 7-8=-92/149, 8-9=-45/19, 4-5=0/72, 5-6=0/72
BOT CHORD 2-12=-72/148, 11-12=-72/148, 10-11=-72/148, 8-10=-72/148
WEBS 3-12=-220/226, 7-10=-220/218, 5-11=-202/130

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1, 28 lb uplift at joint 9, 234 lb uplift at joint 12, 216 lb uplift at joint 10 and 108 lb uplift at joint 11.

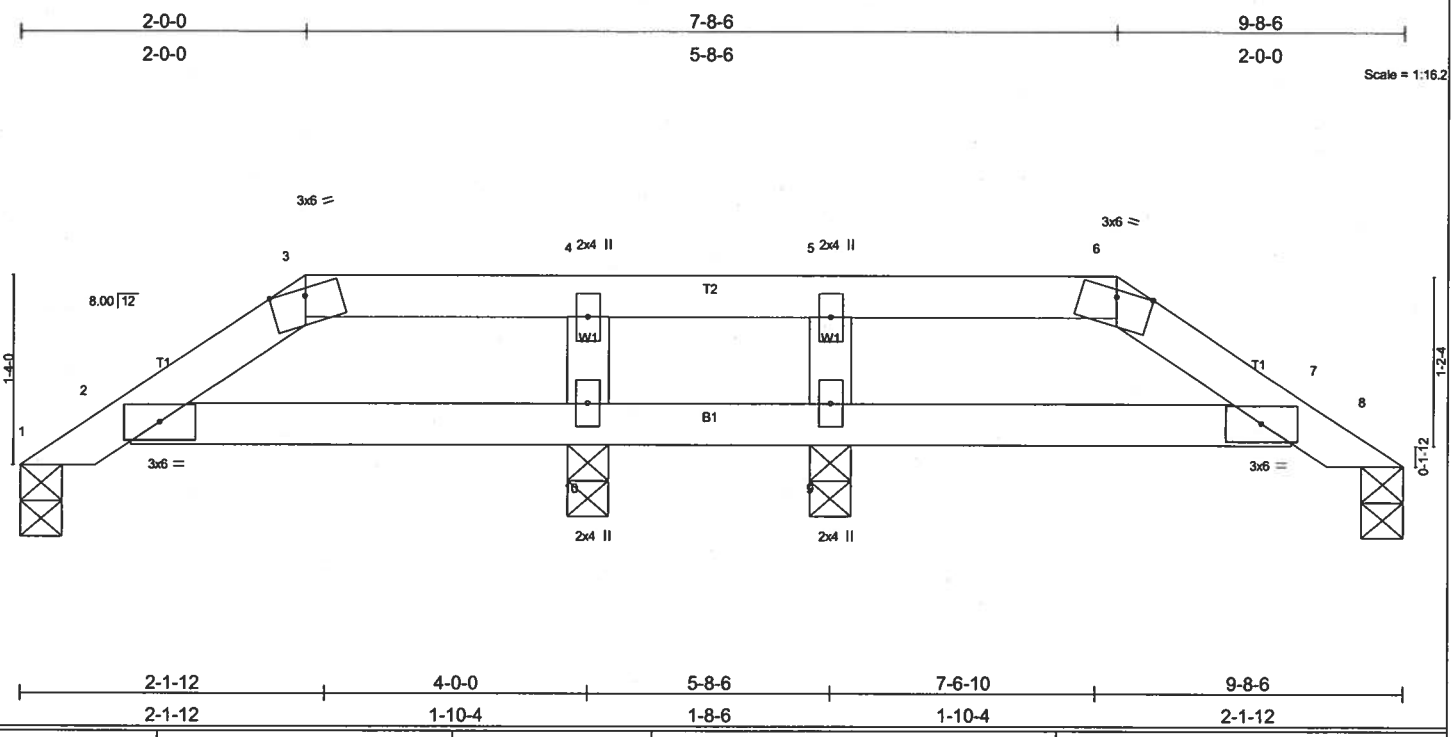
LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061202

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB06	HIP PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:47 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	0.01	7-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.01	8	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 30 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=162/0-3-8, 8=162/0-3-8, 10=221/0-3-8, 9=221/0-3-8
Max Horz 1=-52(load case 3)
Max Uplift1=-67(load case 5), 8=-71(load case 6), 10=-152(load case 4), 9=-145(load case 3)
Max Grav 1=162(load case 1), 8=162(load case 1), 10=242(load case 7), 9=242(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-80/61, 2-3=-166/151, 3-4=-136/151, 4-5=-136/151, 5-6=-136/151, 6-7=-166/151, 7-8=-80/61
BOT CHORD 2-10=-86/136, 9-10=-86/136, 7-9=-86/136
WEBS 4-10=-137/144, 5-9=-137/144

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 1, 71 lb uplift at joint 8, 152 lb uplift at joint 10 and 145 lb uplift at joint 9.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061203

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB07	HIP PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:47 2006 Page 1

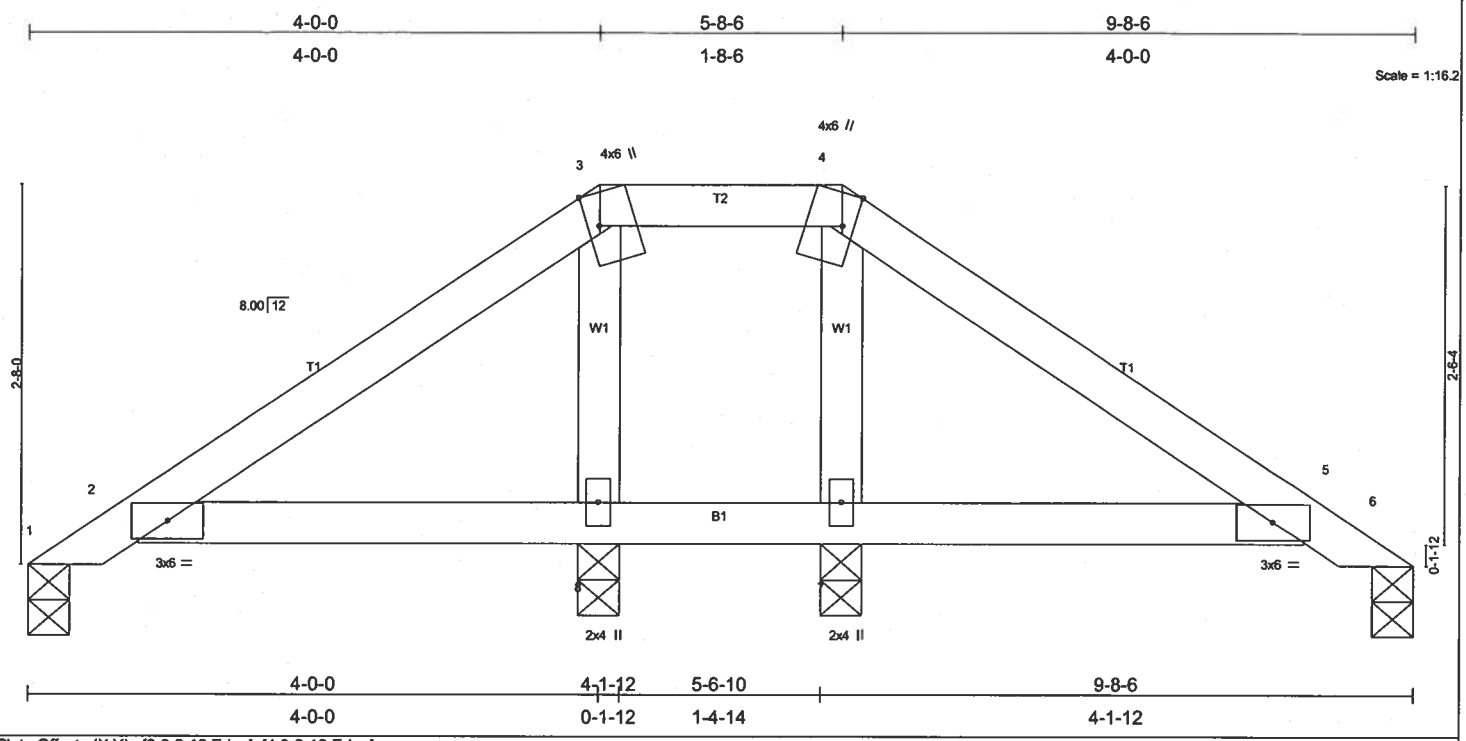


Plate Offsets (X,Y): [3:0-2-12,Edge], [4:0-2-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.11	Vert(LL)	0.01	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=29/0-3-8, 6=29/0-3-8, 8=355/0-3-8, 7=355/0-3-8
Max Horz 1=-107(load case 3)
Max Uplift1=-15(load case 3), 6=-35(load case 3), 8=-202(load case 5), 7=-174(load case 6)
Max Grav 1=51(load case 7), 6=51(load case 8), 8=368(load case 7), 7=368(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-98/105, 2-3=-129/269, 3-4=-37/177, 4-5=-124/269, 5-6=-24/23
BOT CHORD 2-8=-177/180, 7-8=-177/180, 5-7=-177/180
WEBS 3-8=-264/204, 4-7=-264/204

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 1, 35 lb uplift at joint 6, 202 lb uplift at joint 8 and 174 lb uplift at joint 7.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061204

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB08	PIGGYBACK	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:48 2006 Page 1

-2-0-0 | 4-10-3 | 9-8-6 | 11

2-0-0 | 4-10-3 | 4-10-3 | 2-

Scale = 1:17.7

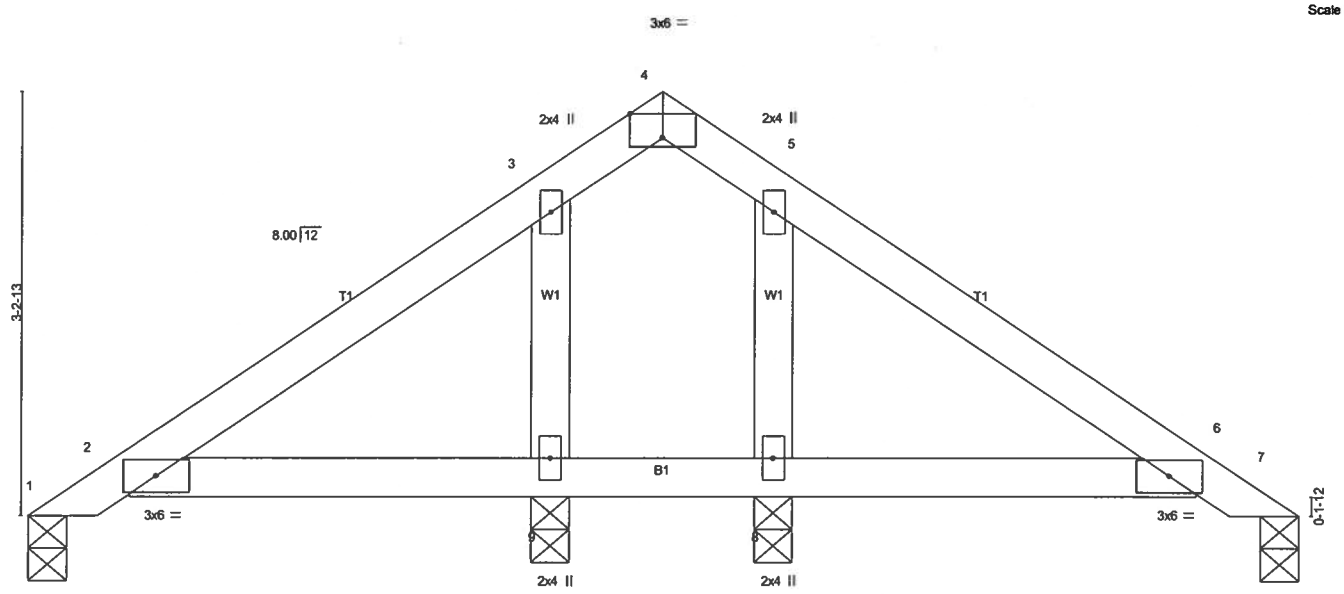


Plate Offsets (X,Y): [4:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 35 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=40/0-3-8, 7=40/0-3-8, 9=343/0-3-8, 8=343/0-3-8
Max Horz 1=-130(load case 3)
Max Uplift1=-13(load case 3), 7=-19(load case 3), 9=-230(load case 5), 8=-203(load case 6)
Max Grav 1=60(load case 7), 7=60(load case 8), 9=350(load case 7), 8=350(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-126/123, 2-3=-163/240, 3-4=-31/124, 4-5=-38/124, 5-6=-163/240, 6-7=-28/14
BOT CHORD 2-9=-149/212, 8-9=-149/212, 6-8=-149/212
WEBS 3-9=-253/217, 5-8=-253/208

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 1, 19 lb uplift at joint 7, 230 lb uplift at joint 9 and 203 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

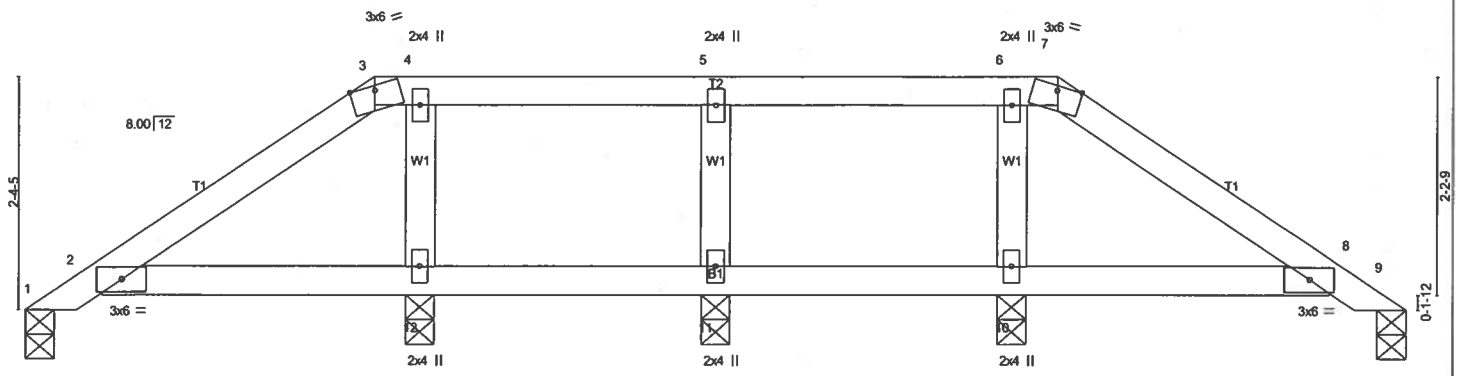
Dwg.#0525061205

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB09	HIP CAP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:48 2006 Page 1

-0	3-6-7	10-5-9	14-0-0	1
0	3-6-7	6-11-2	3-6-7	:

Scale = 1:23.5



3-8-3	4-0-0	7-0-0	10-0-0	10-3-13	14-0-0
3-8-3	0-3-13	3-0-0	3-0-0	0-3-13	3-8-3

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=79/0-3-8, 9=79/0-3-8, 12=391/0-3-8, 10=391/0-3-8, 11=188/0-3-8
Max Horz 1=95(load case 4)
Max Uplift1=-38(load case 6), 9=-57(load case 6), 12=-182(load case 4), 10=-140(load case 3), 11=-173(load case 3)
Max Grav 1=87(load case 7), 9=87(load case 8), 12=391(load case 1), 10=391(load case 1), 11=212(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-83/86, 2-3=-61/140, 7-8=-30/140, 8-9=-42/34, 3-4=-10/78, 4-5=-10/78, 5-6=-10/78, 6-7=-10/78
BOT CHORD 2-12=-78/92, 11-12=-78/92, 10-11=-78/92, 8-10=-78/92
WEBS 4-12=-256/198, 6-10=-256/198, 5-11=-142/188

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 1, 57 lb uplift at joint 9, 182 lb uplift at joint 12, 140 lb uplift at joint 10 and 173 lb uplift at joint 11.

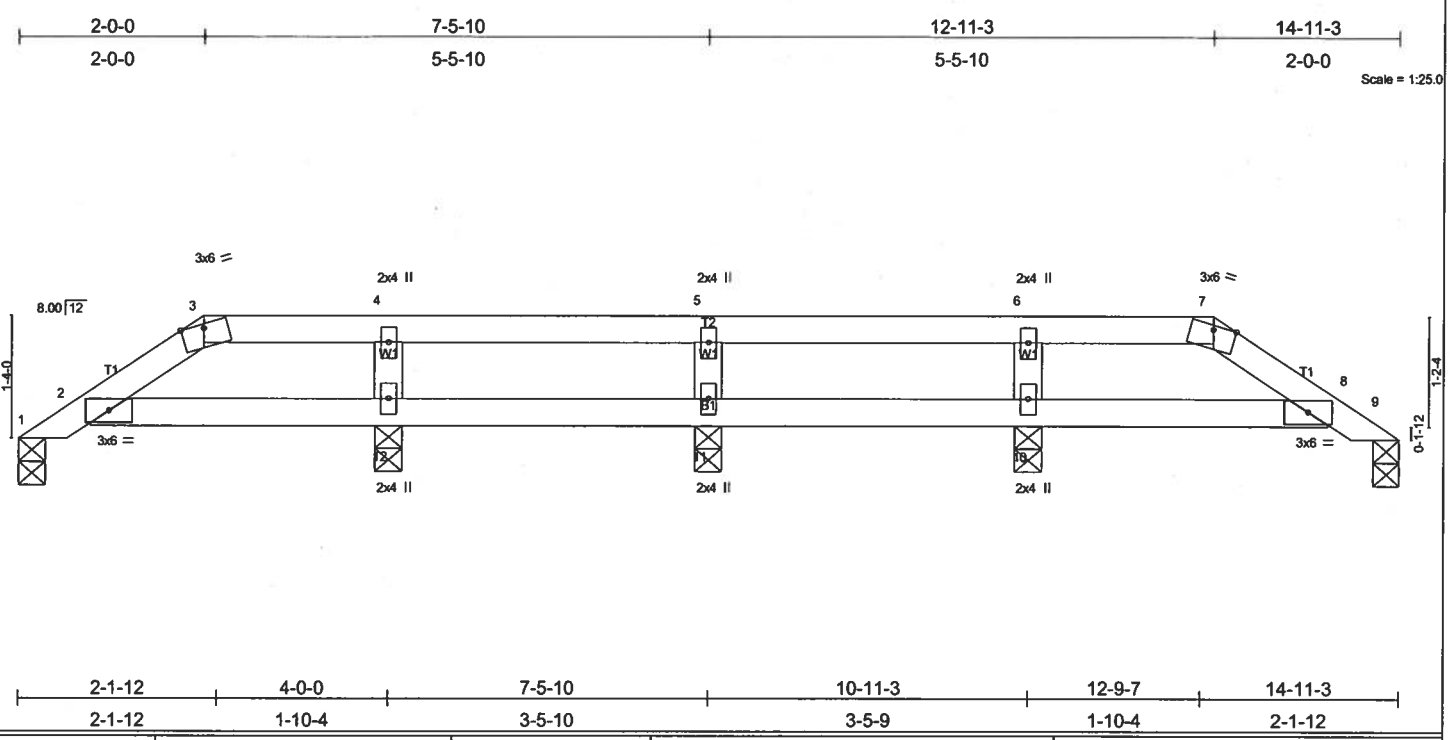
LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061206

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB10	HIP PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:49 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.12	Vert(LL)	0.01	2-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.10	Vert(TL)	0.01	2-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.01	9	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 46 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=143/0-3-8, 9=143/0-3-8, 12=327/0-3-8, 11=266/0-3-8, 10=327/0-3-8
Max Horz 1=52(load case 4)
Max Uplift1=-62(load case 5), 9=-67(load case 6), 12=-189(load case 4), 11=-180(load case 3), 10=-177(load case 3)
Max Grav 1=143(load case 1), 9=143(load case 1), 12=330(load case 7), 11=272(load case 7), 10=330(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-70/54, 2-3=-116/89, 3-4=-91/94, 4-5=-91/94, 5-6=-91/94, 6-7=-91/94, 7-8=-116/89, 8-9=-70/47
BOT CHORD 2-12=-51/91, 11-12=-51/91, 10-11=-51/91, 8-10=-51/91
WEBS 4-12=-194/192, 5-11=-183/203, 6-10=-194/185

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 62 lb uplift at joint 1, 67 lb uplift at joint 9, 189 lb uplift at joint 12, 180 lb uplift at joint 11 and 177 lb uplift at joint 10.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061207

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	PB11	PIGGYBACK	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:50 2006 Page 1

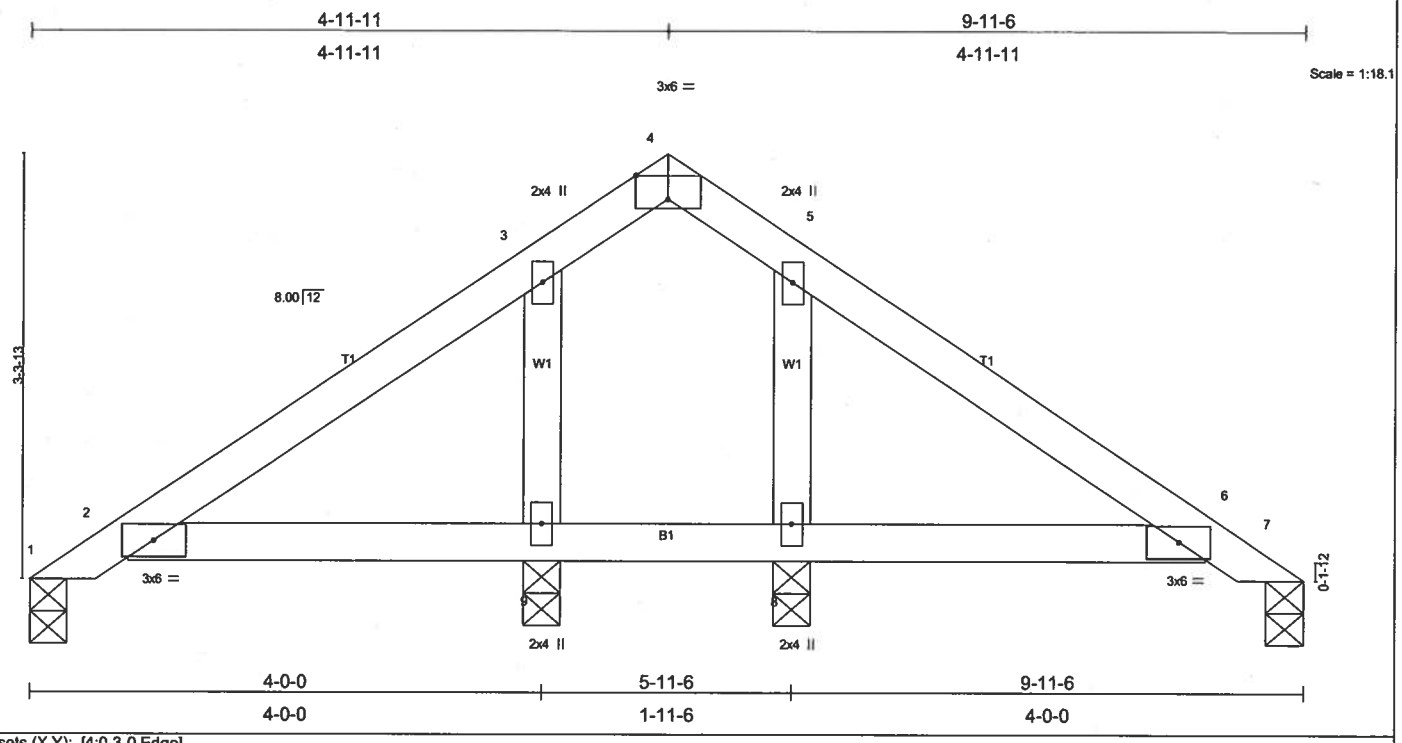


Plate Offsets (X,Y): [4:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.14	Vert(LL)	0.01	2-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.05	Horz(TL)	0.00	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 36 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=49/0-3-8, 7=49/0-3-8, 9=345/0-3-8, 8=345/0-3-8
Max Horz 1=-133(load case 3)
Max Uplift1=-12(load case 3), 7=-16(load case 3), 9=-229(load case 5), 8=-203(load case 6)
Max Grav 1=66(load case 7), 7=66(load case 8), 9=348(load case 7), 8=348(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-129/126, 2-3=-160/222, 3-4=-25/111, 4-5=-32/111, 5-6=-159/222, 6-7=-31/13
BOT CHORD 2-9=-134/209, 8-9=-134/209, 6-8=-134/209
WEBS 3-9=-251/218, 5-8=-251/212

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 1, 16 lb uplift at joint 7, 229 lb uplift at joint 9 and 203 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg #0525061208

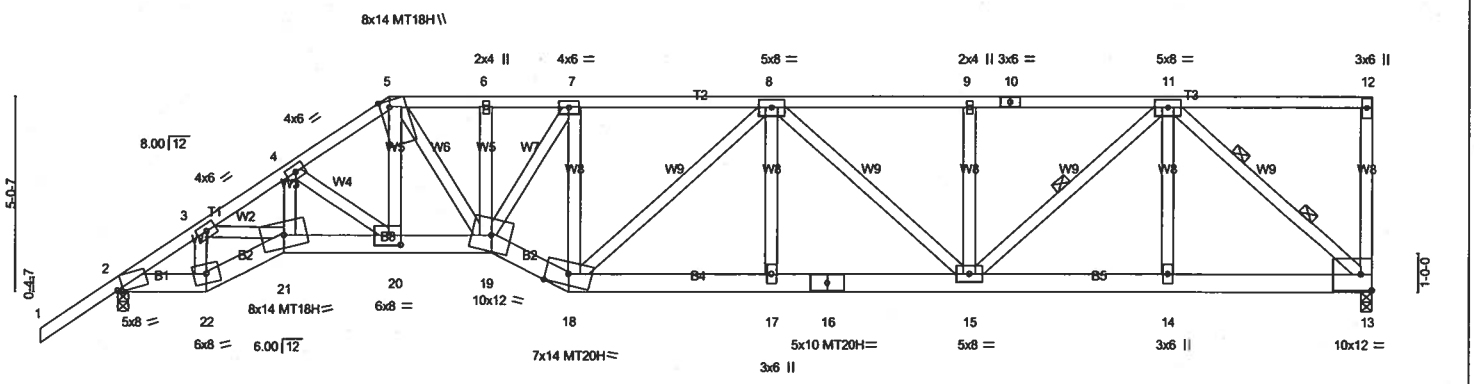
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T01	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:50 2006 Page 1

-2-0-0 | 2-3-8 | 4-3-8 | 7-0-0 | 9-7-8 | 11-7-8 | 16-10-7 | 21-11-9 | 27-0-12 | 32-3-10

2-0-0 | 2-3-8 | 2-0-0 | 2-8-8 | 2-7-8 | 2-0-0 | 5-2-15 | 5-1-3 | 5-1-3 | 5-2-15

Scale = 1:59.7



2-3-8 | 4-3-8 | 7-0-0 | 9-7-8 | 11-7-8 | 16-10-7 | 21-11-9 | 27-0-12 | 32-3-10

2-3-8 | 2-0-0 | 2-8-8 | 2-7-8 | 2-0-0 | 5-2-15 | 5-1-3 | 5-1-3 | 5-2-15

Plate Offsets (X,Y): [2:0-1-13,Edge], [5:0-2-2,Edge], [20:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.90	Vert(LL)	0.39	17	>979	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.88	Vert(TL)	-0.44	17-18	>871	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.80	Horz(TL)	0.18	13	n/a	n/a	MT18H	244/190
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 234 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-3-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-2 oc bracing.
WEBS 1 Row at midpt 11-15
2 Rows at 1/3 pts 11-13

REACTIONS (lb/size) 13=2967/0-3-8, 2=2859/0-3-8
Max Horz 2=362(load case 4)
Max Uplift13=-2278(load case 2), 2=-1863(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/62, 2-3=-4531/3063, 3-4=-6661/4684, 4-5=-5556/4027, 5-6=-5429/3985, 6-7=-5428/3986, 7-8=-4907/3653, 8-9=-4510/3433, 9-10=-4510/3433, 10-11=-4510/3433, 11-12=-63/62, 12-13=-279/361
BOT CHORD 2-22=-2622/3671, 21-22=-2737/3852, 20-21=-3912/5429, 19-20=-3437/4675, 18-19=-3919/5275, 17-18=-3940/5215, 16-17=-3940/5214, 15-16=-3940/5214, 14-15=-2152/2813, 13-14=-2152/2813
WEBS 3-22=-1571/1156, 3-21=-1489/2039, 4-21=-717/1108, 4-20=-1005/688, 5-20=-1117/1620, 5-19=-1128/1340, 6-19=-85/195, 7-19=-800/1067, 7-18=-1426/1308, 8-18=-411/384, 8-17=0/293, 8-15=-955/707, 9-15=-579/768, 11-15=-1735/2300, 11-14=0/332, 11-13=-3727/2833

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2278 lb uplift at joint 13 and 1863 lb uplift at joint 2.
5) Girder carries hip end with 0-0-0 right side setback, 7-0-0 left side setback, and 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 506 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-54, 5-12=-118(F=-64), 2-22=-30, 21-22=-30, 20-21=-30, 19-20=-65(F=-35), 18-19=-65(F=-35), 13-18=-65(F=-35)
Concentrated Loads (lb)
Vert: 20=-539(F)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061209

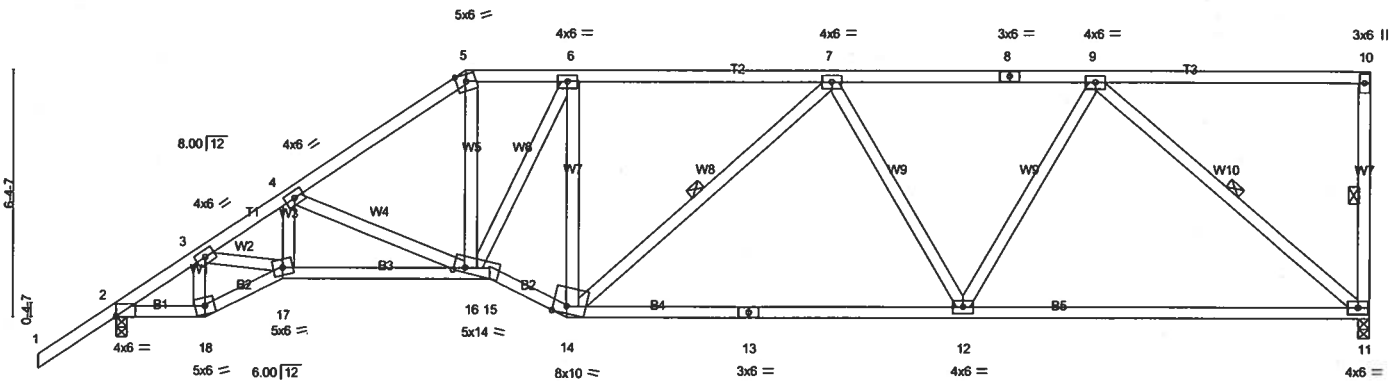
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T02	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:51 2006 Page 1

-2-0-0 | 2-3-8 | 4-3-8 | 9-0-0 | 11-7-8 | 18-5-1 | 25-2-9 | 32-3-10

2-0-0 | 2-3-8 | 2-0-0 | 4-8-8 | 2-7-8 | 6-9-9 | 6-9-9 | 7-1-1

Scale = 1:59.7



2-3-8 | 4-3-8 | 9-7-8 | 11-7-8 | 21-9-13 | 32-3-10

2-3-8 | 2-0-0 | 5-4-0 | 2-0-0 | 10-2-5 | 10-5-13

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [14:0-4-3,Edge], [16:0-3-9,0-1-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.23	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.33	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.11	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 197 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-8-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-12 oc bracing.
WEBS 1 Row at midpt 10-11, 7-14, 9-11

REACTIONS (lb/size) 11=1341/0-3-8, 2=1464/0-3-8
Max Horz 2=432(load case 5)
Max Uplift11=-714(load case 3), 2=-571(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2049/796, 3-4=-3008/1444, 4-5=-2123/1054, 5-6=-1737/959, 6-7=-1679/890, 7-8=-1572/774, 8-9=-1572/774, 9-10=-57/18, 10-11=-179/175
BOT CHORD 2-18=-873/1596, 17-18=-920/1714, 16-17=-1403/2452, 15-16=-881/1670, 14-15=-961/1785, 13-14=-935/1747, 12-13=-935/1747, 11-12=-660/1217
WEBS 3-18=-728/466, 3-17=-606/995, 4-17=-228/623, 4-16=-806/496, 6-16=-226/153, 6-14=-382/373, 7-14=-91/230, 7-12=-359/330, 9-12=-243/727, 9-11=-1542/854, 5-16=-428/951

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 714 lb uplift at joint 11 and 571 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061210

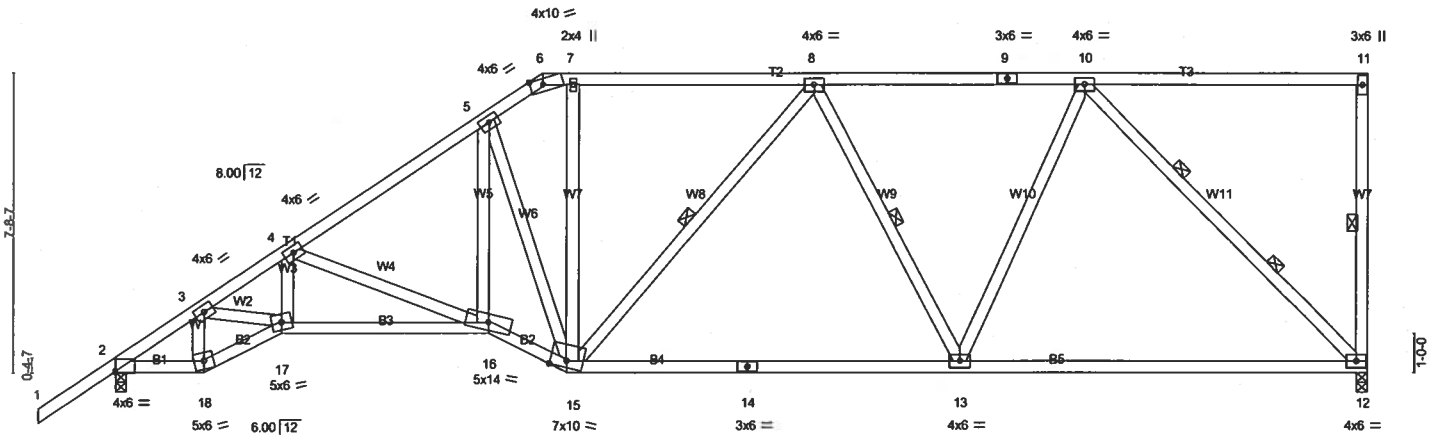
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T03	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:51 2006 Page 1

-2-0-0 | 2-3-8 | 4-3-8 | 9-7-8 | 11-0-0 | 18-0-1 | 25-0-1 | 32-3-10

2-0-0 | 2-3-8 | 2-0-0 | 5-4-0 | 1-4-8 | 7-0-1 | 7-0-1 | 7-3-9

Scale = 1:59.7



2-3-8 | 4-3-8 | 9-7-8 | 11-7-8 | 21-9-13 | 32-3-10

2-3-8 | 2-0-0 | 5-4-0 | 2-0-0 | 10-2-5 | 10-5-13

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.73	Vert(LL)	-0.23	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.62	Vert(TL)	-0.34	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.11	12	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 210 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-7-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-5 oc bracing.
WEBS 1 Row at midpt 11-12, 8-15, 8-13
2 Rows at 1/3 pts 10-12

REACTIONS (lb/size) 12=1341/0-3-8, 2=1464/0-3-8
Max Horz 2=505(load case 5)
Max Uplift12=-670(load case 3), 2=-589(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2031/752, 3-4=-3066/1458, 4-5=-2045/985, 5-6=-1498/797, 6-7=-1360/750, 7-8=-1360/750, 8-9=-1290/652, 9-10=-1290/652, 10-11=-44/14, 11-12=-181/177
BOT CHORD 2-18=-904/1578, 17-18=-958/1708, 16-17=-1492/2505, 15-16=-1066/1805, 14-15=-796/1441, 13-14=-796/1441, 12-13=-562/1024
WEBS 3-18=-757/494, 3-17=-671/1075, 4-17=-257/667, 4-16=-941/588, 5-16=-647/1137, 5-15=-881/622, 8-15=-127/292, 8-13=-330/316, 10-13=-226/675, 10-12=-1411/793, 7-15=-153/409

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 670 lb uplift at joint 12 and 589 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061211

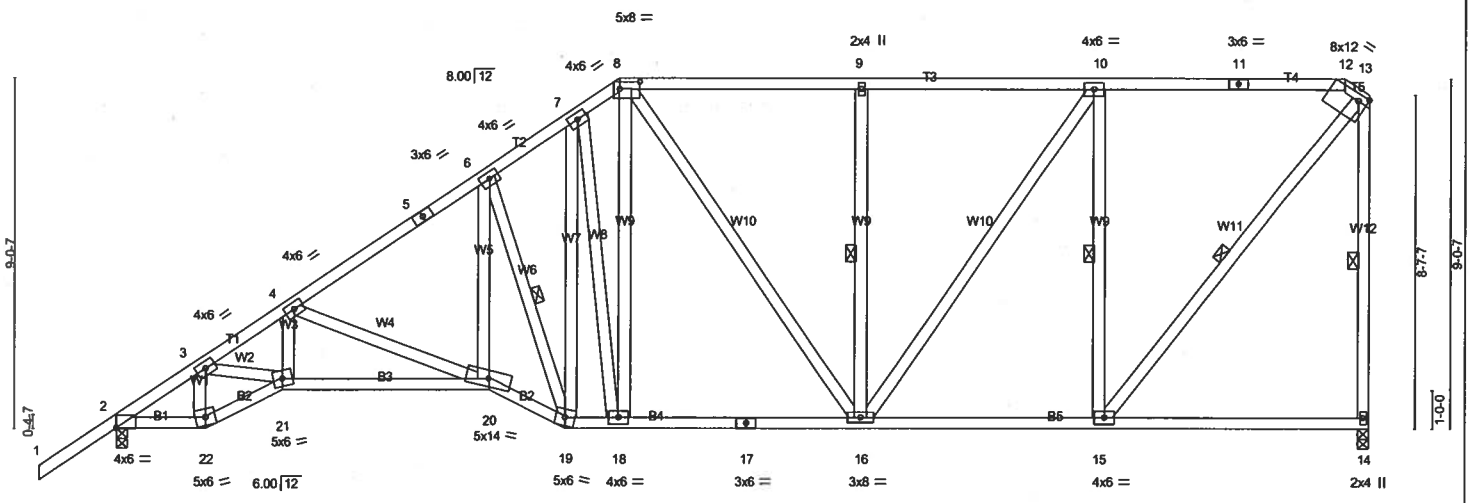
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T04	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:52 2006 Page 1

-2-0-0 | 2-3-8 | 4-3-8 | 9-7-8 | 11-7-8 | 13-0-0 | 19-3-5 | 25-4-13 | 31-8-2 | 32-3-10

2-0-0 | 2-3-8 | 2-0-0 | 5-4-0 | 2-0-0 | 1-4-8 | 6-3-5 | 6-1-9 | 6-3-5 | 0-7-8

Scale = 1:59.7



2-3-8 | 4-3-8 | 9-7-8 | 11-7-8 | 13-0-0 | 19-3-5 | 25-4-13 | 32-3-10

2-3-8 | 2-0-0 | 5-4-0 | 2-0-0 | 1-4-8 | 6-3-5 | 6-1-9 | 6-10-13

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [8:0-6-4,0-2-4], [13:Edge,0-2-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.16	20-21	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.23	20-21	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.10	14	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 257 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W12 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-8-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-11 oc bracing.
WEBS 1 Row at midpt 6-19, 9-16, 10-15, 13-14, 13-15

REACTIONS (lb/size) 2=1464/0-3-8, 14=1341/0-3-8
Max Horz 2=567(load case 5)
Max Uplift2=-603(load case 5), 14=-740(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2037/724, 3-4=-3044/1440, 4-5=-2058/949, 5-6=-1954/961, 6-7=-1575/839, 7-8=-1377/748, 8-9=-1207/678, 9-10=-1207/678, 10-11=-847/483, 11-12=-847/483, 12-13=-803/487, 13-14=-1248/756
BOT CHORD 2-22=-935/1586, 21-22=-991/1715, 20-21=-1520/2482, 19-20=-1111/1902, 18-19=-723/1246, 17-18=-689/1206, 16-17=-689/1206, 15-16=-485/847, 14-15=-7/9
WEBS 3-22=-759/513, 3-21=-671/1044, 4-21=-277/669, 4-20=-890/575, 6-20=-660/1281, 6-19=-1346/845, 7-19=-355/423, 7-18=-272/301, 8-18=-263/438, 8-16=-233/150, 9-16=-337/349, 10-16=-360/627, 10-15=-837/641, 13-15=-766/1340

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 603 lb uplift at joint 2 and 740 lb uplift at joint 14.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061212

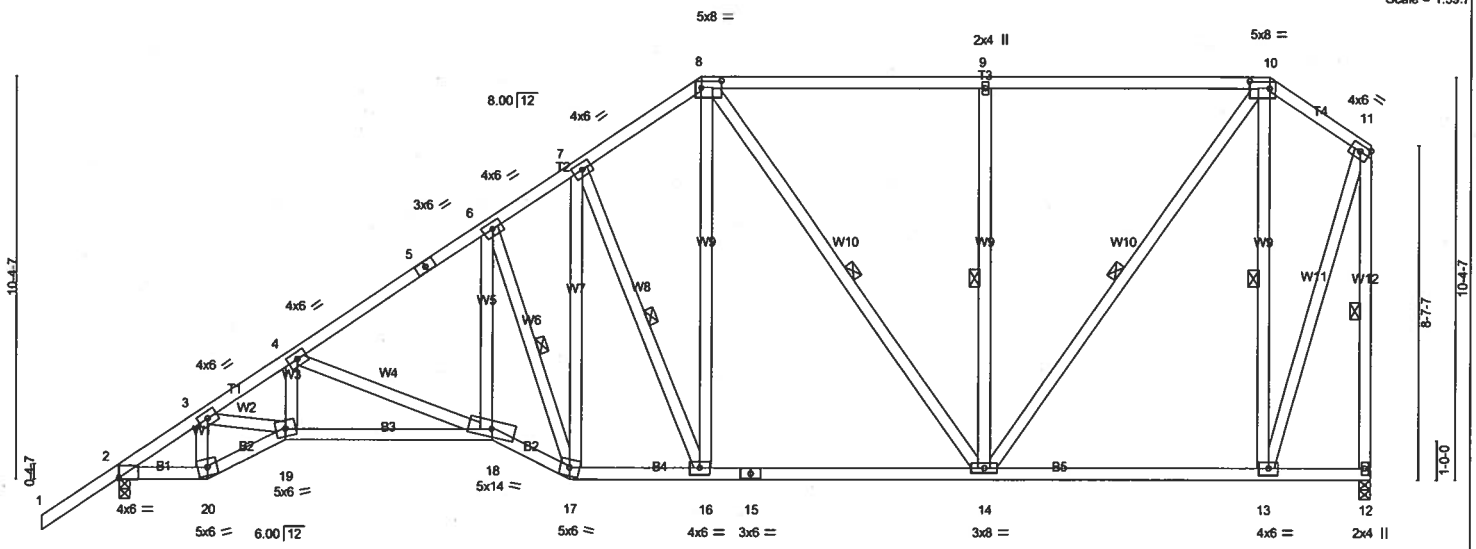
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:53 2006 Page 1

-2-0-0 | 2-3-8 | 4-3-8 | 9-7-8 | 11-7-8 | 15-0-0 | 22-4-1 | 29-8-2 | 32-3-10

2-0-0 | 2-3-8 | 2-0-0 | 5-4-0 | 2-0-0 | 3-4-8 | 7-4-1 | 7-4-1 | 2-7-8

Scale = 1:59.7



2-3-8 | 4-3-8 | 9-7-8 | 11-7-8 | 15-0-0 | 22-4-1 | 29-8-2 | 32-3-10

2-3-8 | 2-0-0 | 5-4-0 | 2-0-0 | 3-4-8 | 7-4-1 | 7-4-1 | 2-7-8

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [8:0-6-4,0-2-4], [10:0-6-4,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.16	18-19	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.23	18-19	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.10	12	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 267 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W12 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-3-15 oc bracing.
WEBS 1 Row at midpt 6-17, 7-16, 8-14, 9-14, 10-14, 10-13, 11-12

REACTIONS (lb/size) 2=1464/0-3-8, 12=1341/0-3-8
Max Horz 2=599(load case 5)
Max Uplift2=-621(load case 5), 12=-600(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2037/715, 3-4=-3045/1426, 4-5=-2057/938, 5-6=-1953/950, 6-7=-1573/825, 7-8=-1303/727, 8-9=-946/585, 9-10=-946/585, 10-11=-419/234, 11-12=-1327/687
BOT CHORD 2-20=-927/1586, 19-20=-983/1715, 18-19=-1509/2484, 17-18=-1099/1897, 16-17=-722/1253, 15-16=-595/1085, 14-15=-595/1085, 13-14=-168/331, 12-13=-3/4
WEBS 3-20=-759/509, 3-19=-668/1046, 4-19=-274/670, 4-18=-893/580, 6-18=-655/1274, 6-17=-1325/825, 7-17=-324/480, 7-16=-447/387, 8-16=-313/581, 8-14=-242/233, 9-14=-424/446, 10-14=-592/1049, 10-13=-923/605, 11-13=-594/1140

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 621 lb uplift at joint 2 and 600 lb uplift at joint 12.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061213

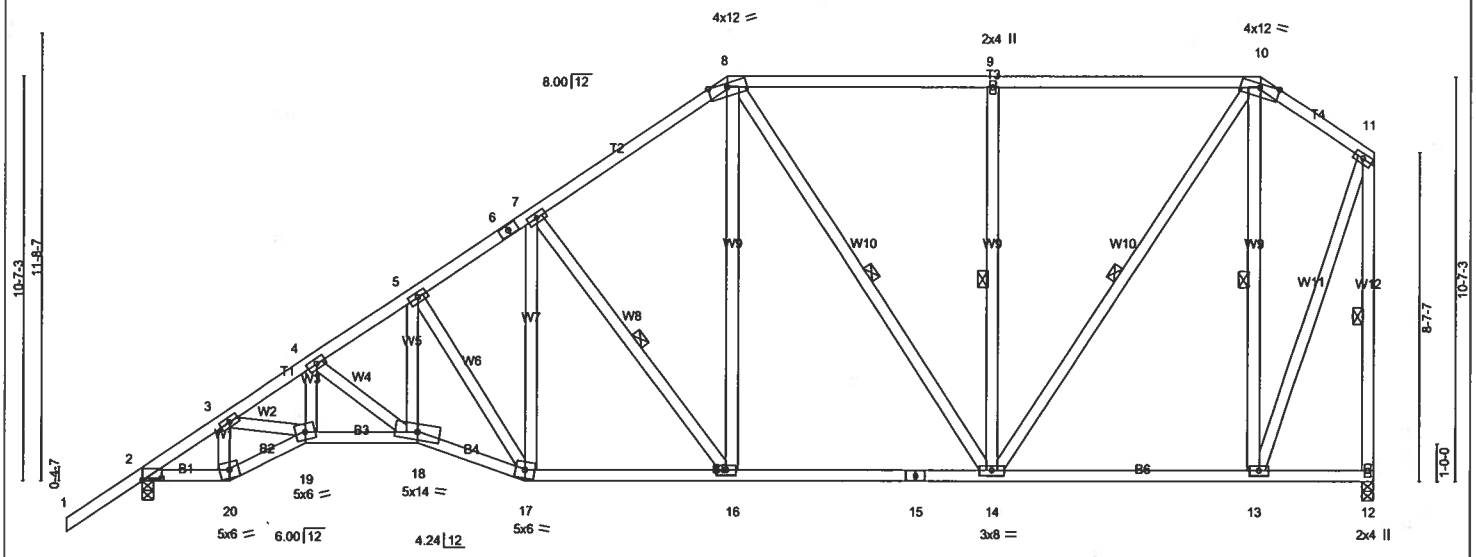
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 11:21:41 2006 Page 1

-2-0-0 | 2-3-8 | 4-3-8 | 7-2-5 | 10-0-4 | 15-4-1 | 22-4-1 | 29-4-1 | 32-3-10

2-0-0 | 2-3-8 | 2-0-0 | 2-10-13 | 2-9-15 | 5-3-13 | 7-0-0 | 7-0-0 | 2-11-9

Scale = 1:60.6



2-3-8 | 4-3-8 | 7-2-5 | 10-0-4 | 15-4-1 | 22-4-1 | 29-4-1 | 32-3-10

2-3-8 | 2-0-0 | 2-10-13 | 2-9-15 | 5-3-13 | 7-0-0 | 7-0-0 | 2-11-9

Plate Offsets (X,Y): [2:0-6-3,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	0.12	18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.51	Vert(TL)	-0.18	14-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.08	12	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 259 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W12 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-8-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-15 oc bracing.
WEBS 1 Row at midpt 7-16, 8-14, 10-14, 10-13, 11-12, 9-14

REACTIONS (lb/size) 2=1464/0-3-8, 12=1341/0-3-8
Max Horz 2=604(load case 5)
Max Uplift2=-623(load case 5), 12=-577(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2054/721, 3-4=-2996/1405, 4-5=-2312/1088, 5-6=-1682/791, 6-7=-1598/794, 7-8=-1338/708, 10-11=-463/263, 11-12=-1317/689, 8-9=-920/580, 9-10=-920/580
BOT CHORD 2-20=-939/1605, 19-20=-997/1723, 18-19=-1469/2427, 17-18=-1230/2066, 16-17=-799/1361, 15-16=-580/1065, 14-15=-580/1065, 13-14=-178/358, 12-13=-3/4
WEBS 3-20=-736/524, 3-19=-618/964, 4-19=-303/633, 4-18=-651/416, 5-18=-628/1130, 5-17=-1084/673, 7-17=-158/375, 7-16=-496/406, 8-16=-280/578, 8-14=-263/247, 10-14=-569/1007, 10-13=-878/565, 11-13=-562/1102, 9-14=-407/434

JOINT STRESS INDEX
2 = 0.77, 3 = 0.34, 4 = 0.32, 5 = 0.57, 6 = 0.29, 7 = 0.32, 8 = 0.79, 9 = 0.27, 10 = 0.64, 11 = 0.70, 12 = 0.39, 13 = 0.70, 14 = 0.57, 15 = 0.44, 16 = 0.27, 17 = 0.61, 18 = 0.66, 19 = 0.67 and 20 = 0.45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are 3x6 MT20 unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 623 lb uplift at joint 2 and 577 lb uplift at joint 12.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061214

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T07	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue May 30 11:21:46 2006 Page 1

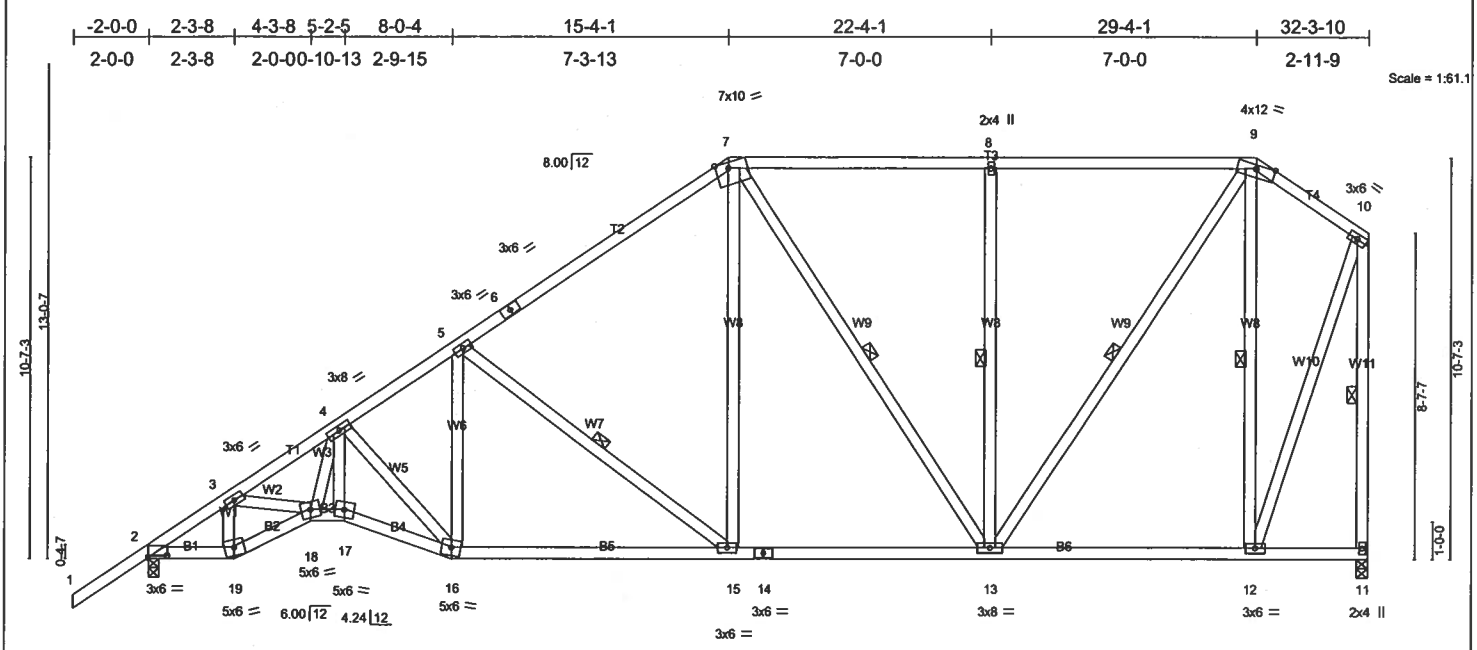


Plate Offsets (X,Y): [2:0-6-3,0-0-10], [7:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.15	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.22	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.79	Horz(TL)	0.08	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 250 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W11 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-8-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-5-0 oc bracing.
WEBS 1 Row at midpt 5-15, 7-13, 9-12, 10-11, 8-13, 9-13

REACTIONS (lb/size) 2=1464/0-3-8, 11=1341/0-3-8
Max Horz 2=604(load case 5)
Max Uplift2=-623(load case 5), 11=-577(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2065/731, 3-4=-2959/1385, 4-5=-1821/803, 5-6=-1385/655, 6-7=-1283/686, 9-10=-462/262, 10-11=-1318/689, 7-8=-918/578, 8-9=-918/578
BOT CHORD 2-19=-956/1620, 18-19=-1015/1734, 17-18=-1347/2231, 16-17=-1460/2406, 15-16=-901/1506, 14-15=-587/1074, 13-14=-587/1074, 12-13=-178/359, 11-12=-3/4
WEBS 3-19=-732/539, 3-18=-563/903, 4-18=-351/664, 4-17=-461/763, 4-16=-1161/721, 5-16=-40/293, 5-15=-550/440, 7-15=-218/548, 7-13=-284/267, 9-12=-878/567, 10-12=-563/1105, 8-13=-407/443, 9-13=-567/1001

JOINT STRESS INDEX
2 = 0.78, 3 = 0.31, 4 = 0.58, 5 = 0.29, 6 = 0.24, 7 = 0.71, 8 = 0.27, 9 = 0.66, 10 = 0.70, 11 = 0.39, 12 = 0.70, 13 = 0.57, 14 = 0.51, 15 = 0.26, 16 = 0.82, 17 = 0.73, 18 = 0.73 and 19 = 0.45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 623 lb uplift at joint 2 and 577 lb uplift at joint 11.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
~~STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196~~
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

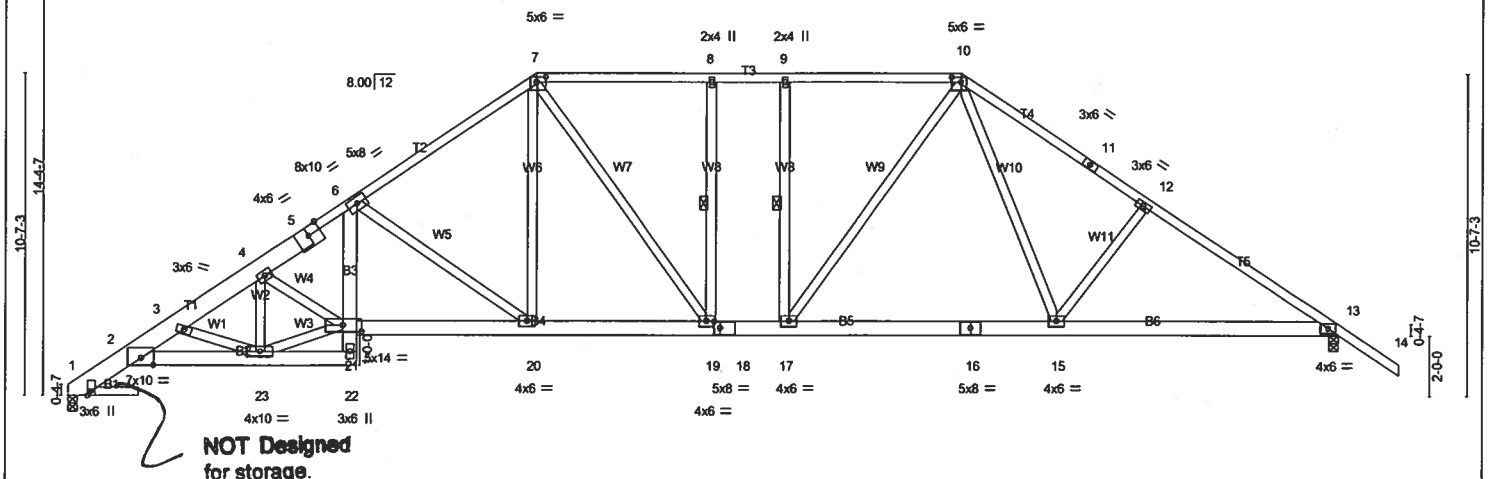
Dwg.#0525061215

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:55 2006 Page 1

-2-0-0	2-3-8	9-5-3	15-2-10	15-4-1	21-0-0	23-8-2	29-4-1	29-6-8	35-4-14	41-8-2	43-8-2
2-0-0	2-3-8	7-1-11	5-9-6	0-1-8	5-7-15	2-8-2	5-7-15	0-2-7	5-10-6	6-3-4	2-0-0

Scale = 1:76.1



NOT Designed for storage.

2-3-8	9-5-3	15-2-10	21-0-0	23-8-2	32-5-11	41-8-2
2-3-8	7-1-11	5-9-6	5-9-6	2-8-2	8-9-9	9-2-7

Plate Offsets (X,Y): [7:0-3-12,0-2-0], [10:0-3-12,0-2-0], [18:0-2-3,0-2-8], [21:0-7-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.27	20-21	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.38	20-21	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.22	13	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 305 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-4-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-5 oc bracing.
WEBS 1 Row at midpt 8-19, 9-17

REACTIONS (lb/size) 1=1701/0-3-8, 13=1856/0-3-8
Max Horz 1=388(load case 4)
Max Uplift1=-607(load case 5), 13=-713(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-864/439, 2-3=-4369/2036, 3-4=-3631/1697, 4-5=-3570/1663, 5-6=-3523/1679, 6-7=-2592/1310, 10-11=-2426/1262, 11-12=-2515/1236, 12-13=-2710/1248, 13-14=0/62, 7-8=-2177/1225, 8-9=-2177/1225, 9-10=-2177/1225
BOT CHORD 2-23=-1902/4214, 22-23=-333/658, 21-22=-23/104, 6-21=-396/1023, 20-21=-1325/2979, 19-20=-917/2070, 18-19=-894/2177, 17-18=-894/2177, 16-17=-601/1823, 15-16=-601/1823, 13-15=-806/2172
WEBS 6-20=-1084/665, 7-20=-389/882, 7-19=-442/379, 8-19=-233/286, 9-17=-267/303, 10-17=-507/731, 10-15=-246/555, 12-15=-265/365, 4-23=-123/108, 4-21=-154/189, 21-23=-1062/2471, 3-23=-1307/667

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 607 lb uplift at joint 1 and 713 lb uplift at joint 13.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061216

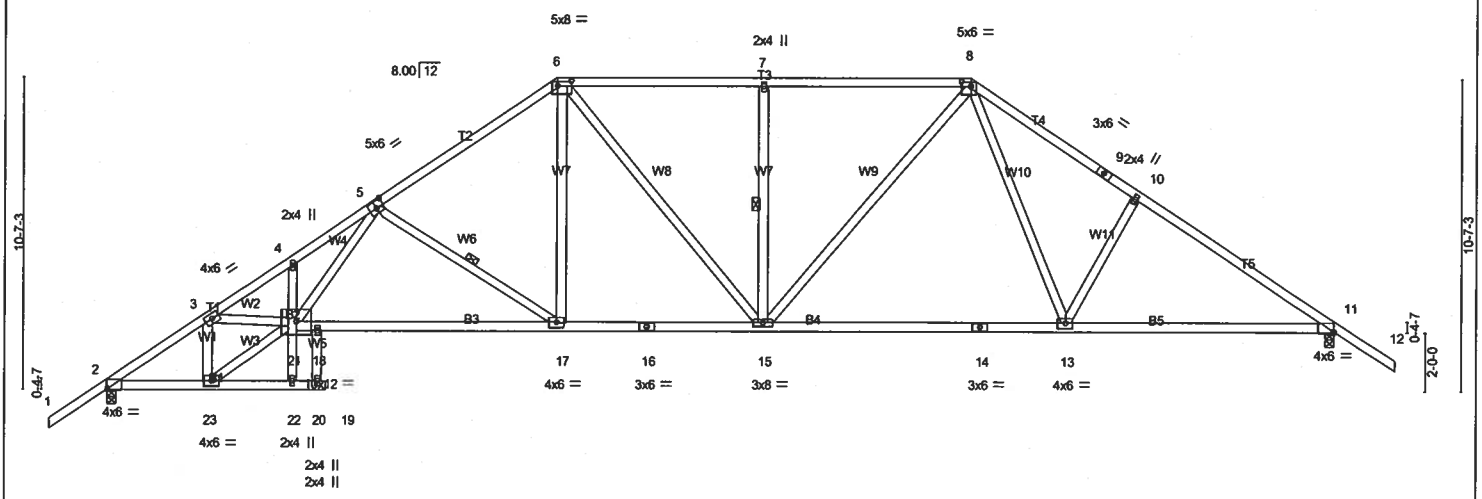
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:55 2006 Page 1

-2-0-0 | 3-5-0 | 6-5-3 | 15-4-1 | 22-4-1 | 28-7-13 | 29-4-1 | 34-11-8 | 41-8-2 | 43-8-2

2-0-0 | 3-5-0 | 3-0-3 | 8-10-14 | 7-0-0 | 6-3-12 | 0-8-4 | 5-7-7 | 6-8-10 | 2-0-0

Scale = 1:78.6



3-5-0 | 6-5-3 | 15-4-1 | 22-4-1 | 32-7-6 | 41-8-2

3-5-0 | 3-0-3 | 8-10-14 | 7-0-0 | 10-3-5 | 9-0-12

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [5:0-3-0,0-3-0], [6:0-5-12,0-2-0], [8:0-3-12,0-2-0], [11:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.40	17-18	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.57	17-18	>867	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.23	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 254 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-9-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-13 oc bracing.
WEBS 1 Row at midpt 5-17, 7-15

REACTIONS (lb/size) 2=1861/0-3-8, 11=1856/0-3-8
Max Horz 2=424(load case 4)
Max Uplift2=-760(load case 5), 11=-712(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2758/1177, 3-4=-4871/2052, 4-5=-4872/2116, 5-6=-2539/1261, 8-9=-2356/1274, 9-10=-2509/1250, 10-11=-2683/1215, 11-12=0/59, 6-7=-2192/1221, 7-8=-2192/1221
BOT CHORD 2-23=-992/2199, 22-23=-61/31, 20-22=0/0, 19-20=0/0, 21-22=0/183, 4-21=-79/100, 18-21=-1319/2947, 17-18=-1319/2947, 16-17=-897/2053, 15-16=-897/2053, 14-15=-599/1800, 13-14=-599/1800, 11-13=-765/2134
WEBS 3-23=-1497/688, 21-23=-1136/2644, 3-21=-739/1814, 5-17=-1082/657, 6-15=-418/378, 7-15=-396/434, 8-15=-473/711, 8-13=-278/588, 10-13=-256/363, 18-20=-71/0, 6-17=-319/827, 5-21=-725/1961

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 760 lb uplift at joint 2 and 712 lb uplift at joint 11.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

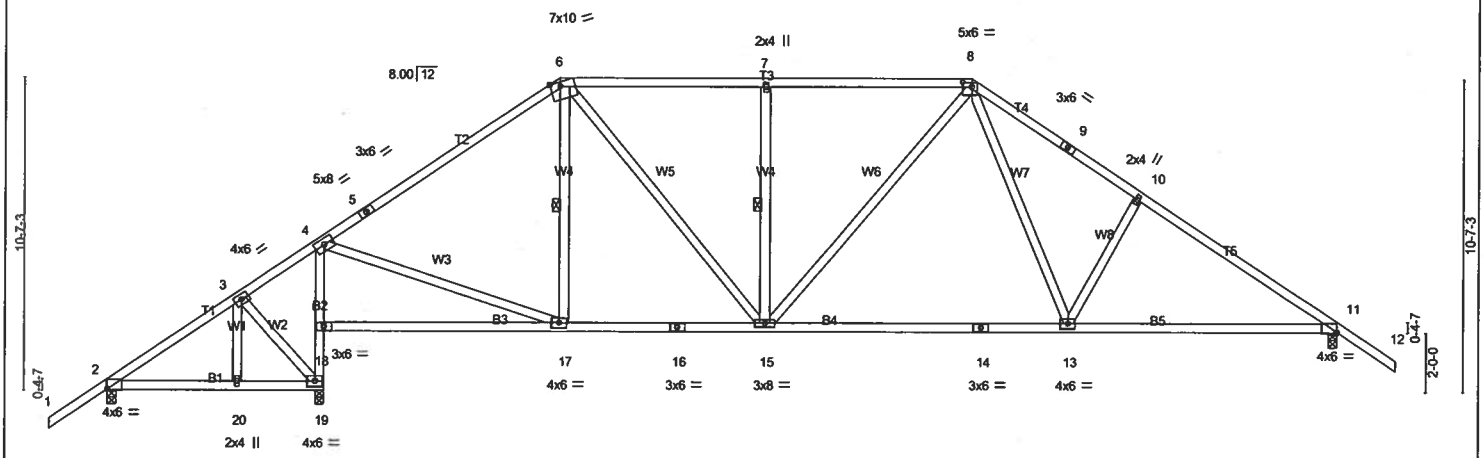
Dwg.#0525061217

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T10	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:56 2006 Page 1

-2-0-0	7-4-3	15-4-1	22-4-1	28-7-13	29-4-1	34-11-8	41-8-2	43-8-2
2-0-0	7-4-3	7-11-14	7-0-0	6-3-12	0-8-4	5-7-7	6-8-10	2-0-0

Scale = 1:78.6



7-2-7	7-4-3	15-4-1	22-4-1	32-7-6	41-8-2
7-2-7	0-1-12	7-11-14	7-0-0	10-3-5	9-0-12

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-4-0,Edge], [8:0-3-12,0-2-0], [11:Edge,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.25	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.61	Vert(TL)	-0.36	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(TL)	0.05	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 244 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except* B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-2-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-4-1 oc bracing.
WEBS 1 Row at midpt 7-15, 6-17

REACTIONS (lb/size) 2=378/0-3-8, 11=1551/0-3-8, 19=1780/0-3-8
Max Horz 2=424(load case 4)
Max Uplift2=-292(load case 6), 11=-679(load case 6), 19=-760(load case 4)
Max Grav 2=381(load case 7), 11=1551(load case 1), 19=1780(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-299/392, 3-4=-275/429, 4-5=-1438/769, 5-6=-1315/803, 8-9=-1820/1066, 9-10=-1974/1042, 10-11=-2146/1007, 11-12=0/59, 6-7=-1487/946, 7-8=-1487/946
BOT CHORD 2-20=-95/110, 19-20=-95/110, 18-19=-1589/696, 4-18=-1455/720, 17-18=-110/69, 16-17=-493/1091, 15-16=-493/1091, 14-15=-410/1353, 13-14=-410/1353, 11-13=-594/1693
WEBS 4-17=-439/1113, 6-15=-423/693, 7-15=-397/441, 8-15=-302/331, 8-13=-279/593, 10-13=-263/363, 3-20=0/135, 3-19=-232/89, 6-17=-156/188

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 lb uplift at joint 2, 679 lb uplift at joint 11 and 760 lb uplift at joint 19.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061218

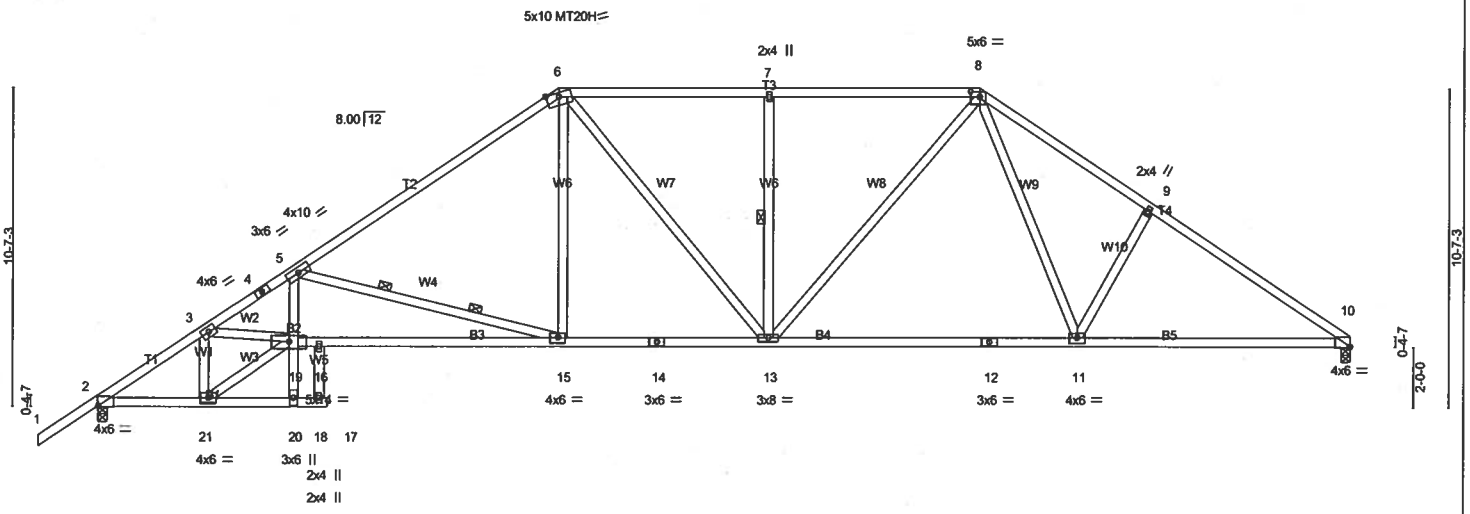
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T11	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:57 2006 Page 1

-2-0-0 | 3-6-4 | 6-7-10 | 15-4-1 | 22-4-1 | 28-7-13 | 29-4-1 | 34-11-8 | 41-8-2

2-0-0 | 3-6-4 | 3-1-6 | 8-8-7 | 7-0-0 | 6-3-12 | 0-8-4 | 5-7-7 | 6-8-10

Scale = 1:77.0



3-6-4 | 6-7-10 | 15-4-1 | 22-4-1 | 32-7-6 | 41-8-2

3-6-4 | 3-1-6 | 8-8-7 | 7-0-0 | 10-3-5 | 9-0-12

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [8:0-3-12,0-2-0], [10:Edge,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.70	Vert(LL)	-0.38	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-0.54	15-16	>917	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.98	Horz(TL)	0.26	10	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 247 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-10-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-9-13 oc bracing.
WEBS 1 Row at midpt 7-13
2 Rows at 1/3 pts 5-15

REACTIONS (lb/size) 10=1737/0-3-8, 2=1864/0-3-8
Max Horz 2=451(load case 4)
Max Uplift10=-564(load case 6), 2=-760(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2765/1174, 3-4=-4783/2117, 4-5=-4712/2122, 5-6=-2604/1271, 8-9=-2534/1329, 9-10=-2687/1268, 6-7=-2197/1242, 7-8=-2197/1242
BOT CHORD 2-21=-1011/2198, 20-21=-178/234, 18-20=0/0, 17-18=0/0, 19-20=-180/245, 5-19=-499/1463, 16-19=-1867/4121, 15-16=-1857/4112, 14-15=-940/2070, 13-14=-940/2070, 12-13=-658/1812, 11-12=-658/1812, 10-11=-921/2160
WEBS 3-21=-1346/630, 19-21=-1050/2420, 3-19=-757/1776, 5-15=-2126/1150, 7-13=-391/446, 8-13=-468/703, 8-11=-319/610, 9-11=-269/386, 16-18=-233/269, 6-15=-254/768, 6-13=-429/379

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 564 lb uplift at joint 10 and 760 lb uplift at joint 2.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061219

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:57 2006 Page 1

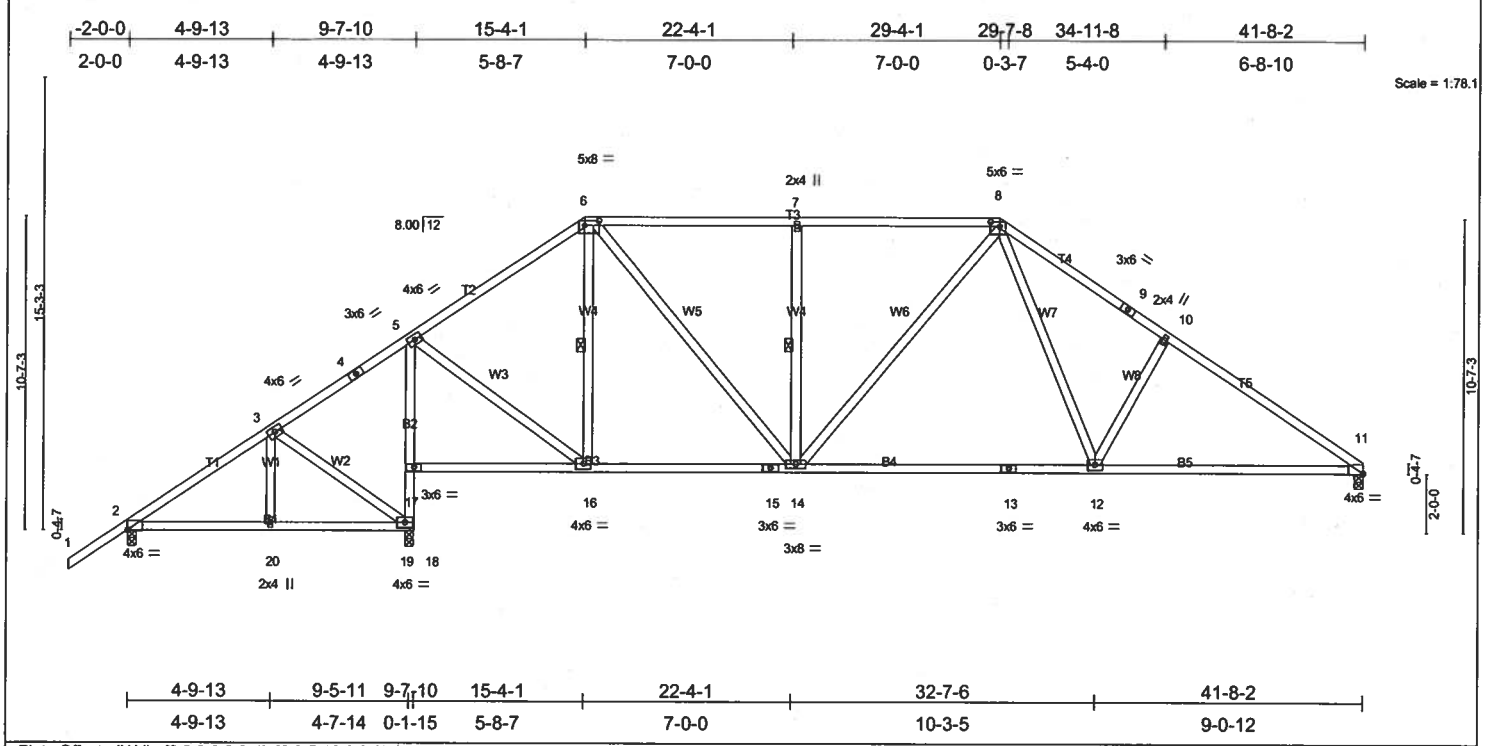


Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-5-12,0-2-0], [8:0-3-12,0-2-0], [11:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.76	Vert(TL)	-0.31	12-14	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.03	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 243 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-6-3 oc bracing.
WEBS 1 Row at midpt 6-16, 7-14

REACTIONS (lb/size) 2=493/0-3-8, 18=1763/0-3-8, 11=1337/0-3-8
Max Horz 2=451(load case 4)
Max Uplift2=-258(load case 5), 18=-787(load case 4), 11=-507(load case 6)
Max Grav 2=496(load case 7), 18=1763(load case 1), 11=1337(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-407/331, 3-4=-280/350, 4-5=-260/369, 5-6=-1034/659, 8-9=-1679/1031, 9-10=-1833/1007, 10-11=-1984/969, 6-7=-1284/854, 7-8=-1284/854
BOT CHORD 2-20=-198/275, 19-20=-198/275, 18-19=-29/33, 17-18=-1497/702, 5-17=-1406/718, 16-17=-28/52, 15-16=-369/784, 14-15=-369/784, 13-14=-405/1223, 12-13=-405/1223, 11-12=-675/1582
WEBS 5-16=-438/993, 6-16=-408/295, 6-14=-429/804, 7-14=-402/432, 8-14=-248/231, 8-12=-324/617, 10-12=-281/390, 3-20=0/164, 3-19=-358/231

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 258 lb uplift at joint 2, 787 lb uplift at joint 18 and 507 lb uplift at joint 11.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

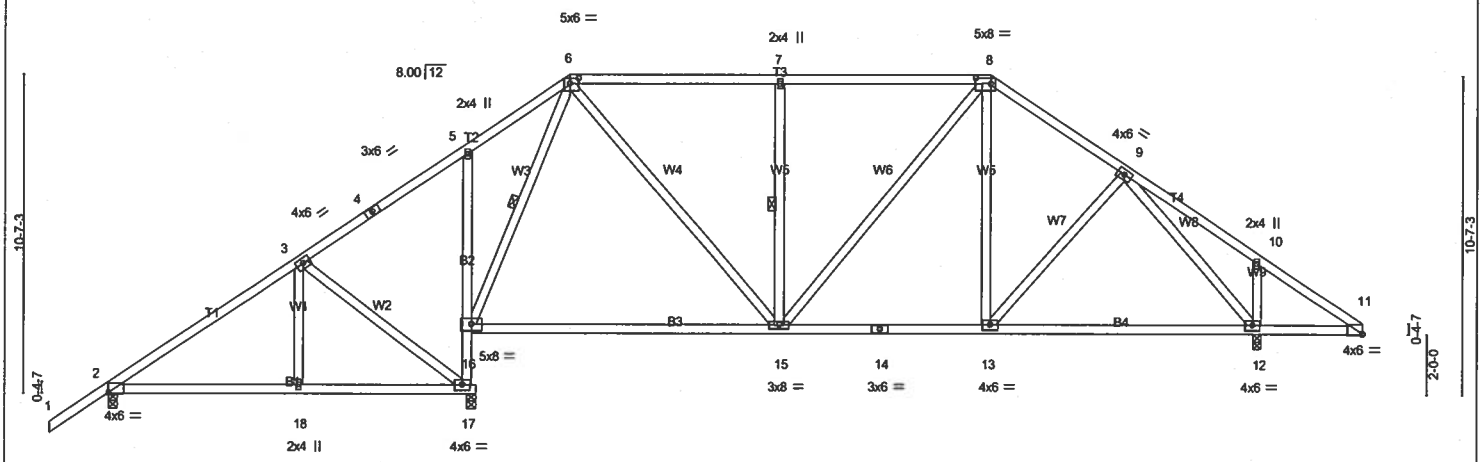
Dwg #0525061220

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T13	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:58 2006 Page 1

-2-0-0	6-3-10	12-2-6	15-4-1	22-4-1	29-4-1	33-9-4	38-2-6	41-8-2
2-0-0	6-3-10	5-10-12	3-1-11	7-0-0	7-0-0	4-5-2	4-5-2	3-5-12

Scale = 1:77.0



6-3-10	12-2-6	22-4-1	29-4-1	38-2-6	41-8-2
6-3-10	5-10-12	10-1-11	7-0-0	8-10-5	3-5-12

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-12,0-2-0], [8:0-6-4,0-2-4], [11:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.20	15-16	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.30	15-16	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.02	12	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 255 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except* B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 11-12.
WEBS 1 Row at midpt 6-16, 7-15

REACTIONS (lb/size) 2=548/0-3-8, 17=1674/0-3-8, 12=1383/0-3-8
Max Horz 2=451(load case 4)
Max Uplift2=-331(load case 5), 17=-773(load case 4), 12=-573(load case 6)
Max Grav 2=549(load case 7), 17=1674(load case 1), 12=1383(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-444/399, 3-4=-218/371, 4-5=-204/395, 5-6=-114/467, 6-7=-805/680, 7-8=-805/680, 8-9=-949/648, 9-10=-2/170, 10-11=-104/172
BOT CHORD 2-18=-269/294, 17-18=-269/294, 16-17=-1315/653, 5-16=-219/308, 15-16=-226/303, 14-15=-285/746, 13-14=-285/746, 12-13=-238/643, 11-12=-91/134
WEBS 3-18=0/212, 3-17=-486/341, 6-16=-1010/575, 6-15=-347/784, 7-15=-409/433, 8-15=-203/164, 8-13=-71/170, 9-13=-165/214, 9-12=-1148/553, 10-12=-241/304

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 331 lb uplift at joint 2, 773 lb uplift at joint 17 and 573 lb uplift at joint 12.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

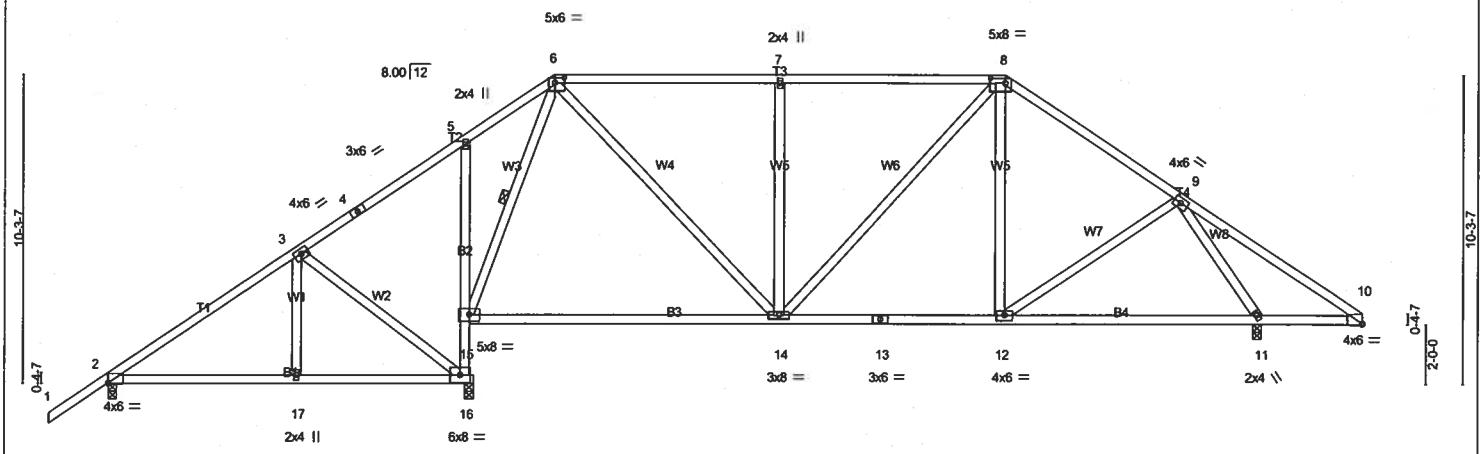
Dwg.#0525061221

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T15	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:59 2006 Page 1

-2-0-0	6-3-10	12-2-6	14-10-8	22-4-1	29-9-10	35-6-7	41-8-2
2-0-0	6-3-10	5-10-12	2-8-2	7-5-9	7-5-9	5-8-13	6-1-11

Scale = 1:77.0



6-3-10	12-2-6	22-4-1	29-9-10	38-2-6	41-8-2
6-3-10	5-10-12	10-1-11	7-5-9	8-4-12	3-5-12

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-12,0-2-0], [8:0-5-12,0-2-0], [10:Edge,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.36	Vert(LL)	-0.20	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.29	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.81	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 247 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except* B2 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-15

REACTIONS (lb/size) 2=548/0-3-8, 16=1679/0-3-8, 11=1378/0-3-8
Max Horz 2=509(load case 4)
Max Uplift2=-502(load case 5), 16=-1000(load case 4), 11=-670(load case 6)
Max Grav 2=549(load case 7), 16=1679(load case 1), 11=1378(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/60, 2-3=-445/550, 3-4=-214/347, 4-5=-199/371, 5-6=-128/436, 6-7=-848/665, 7-8=-848/666, 8-9=-1008/599, 9-10=-283/295
BOT CHORD 2-17=-580/294, 16-17=-580/294, 15-16=-1319/666, 5-15=-192/283, 14-15=-208/274, 13-14=-280/769, 12-13=-280/769, 11-12=-222/542, 10-11=-168/321
WEBS 3-17=-277/212, 3-16=-487/603, 6-15=-1032/613, 6-14=-383/842, 7-14=-438/466, 8-14=-229/190, 8-12=-5/174, 9-12=-225/307, 9-11=-1305/895

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 502 lb uplift at joint 2, 1000 lb uplift at joint 16 and 670 lb uplift at joint 11.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061222

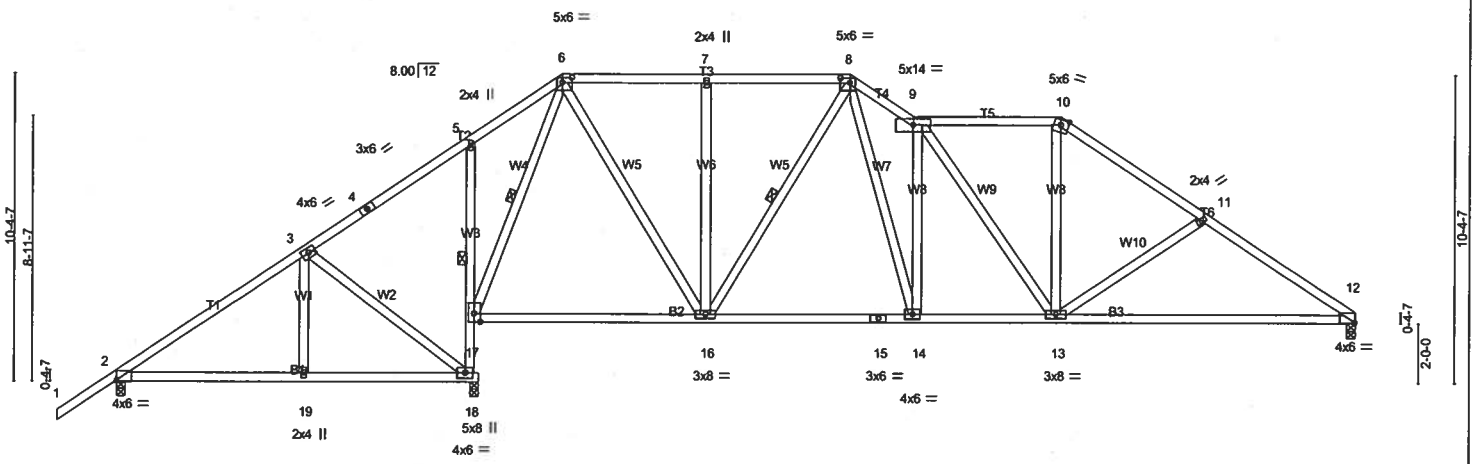
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:56:59 2006 Page 1

-2-0-0 | 6-3-10 | 12-2-6 | 15-0-0 | 19-10-3 | 24-8-6 | 26-9-14 | 31-9-10 | 36-6-7 | 41-8-2

2-0-0 | 6-3-10 | 5-10-12 | 2-9-10 | 4-10-3 | 4-10-3 | 2-1-8 | 4-11-12 | 4-8-13 | 5-1-11

Scale = 1:78.1



6-3-10 | 12-2-6 | 19-10-3 | 26-9-14 | 31-9-10 | 41-8-2

6-3-10 | 5-10-12 | 7-7-13 | 6-11-11 | 4-11-12 | 9-10-8

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-12,0-2-0], [8:0-3-12,0-2-0], [12:0-0-3,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.25	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.59	Vert(TL)	-0.37	12-13	>956	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.02	12	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 266 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-5-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-8-13 oc bracing.
WEBS 1 Row at midpt 6-17, 8-16, 5-18

REACTIONS (lb/size) 12=1237/0-3-8, 2=601/0-3-8, 18=1755/0-3-8
Max Horz 2=442(load case 4)
Max Uplift12=-559(load case 6), 2=-332(load case 5), 18=-784(load case 4)
Max Grav 12=1237(load case 1), 2=604(load case 7), 18=1755(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-542/392, 3-4=-225/361, 4-5=-211/385, 5-6=-96/484, 6-7=-914/710, 7-8=-914/710, 8-9=-1692/1117, 9-10=-1268/804, 10-11=-1587/872, 11-12=-1815/953
BOT CHORD 2-19=-265/374, 18-19=-265/374, 16-17=-190/392, 15-16=-389/1108, 14-15=-389/1108, 13-14=-543/1378, 12-13=-689/1466
WEBS 3-19=0/215, 3-18=-483/341, 6-17=-1149/531, 6-16=-402/1018, 7-16=-273/299, 8-16=-381/207, 8-14=-624/1070, 9-14=-916/631, 9-13=-212/216, 10-13=-233/560, 11-13=-254/308, 17-18=-1401/663, 5-17=-234/325

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 559 lb uplift at joint 12, 332 lb uplift at joint 2 and 784 lb uplift at joint 18.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061223

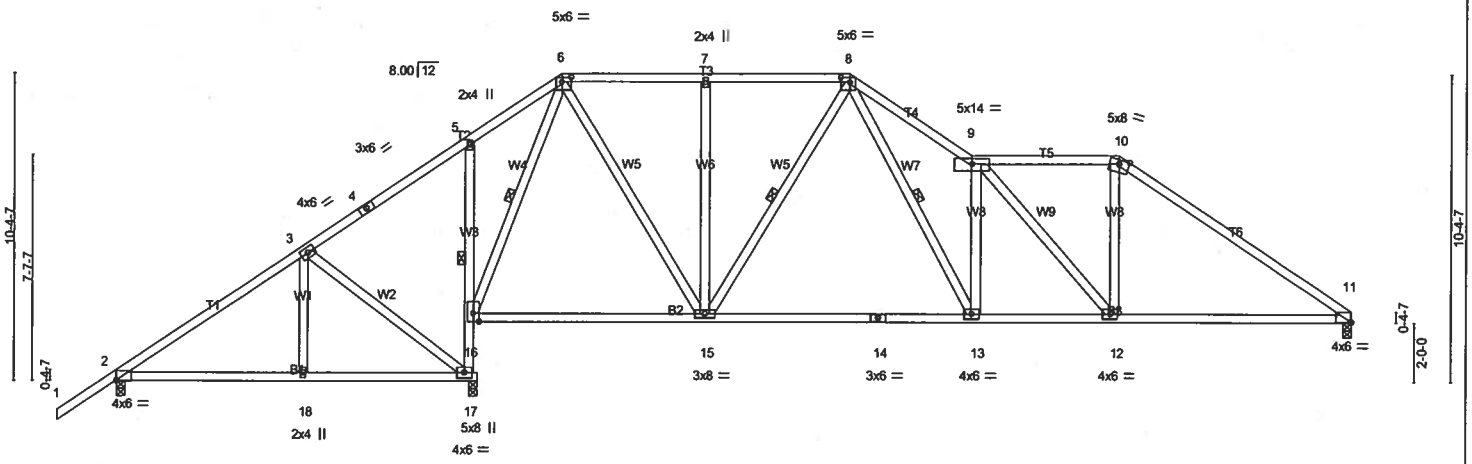
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:00 2006 Page 1

-2-0-0 | 6-3-10 | 12-2-6 | 15-0-0 | 19-10-3 | 24-8-6 | 28-9-14 | 33-9-10 | 41-8-2

2-0-0 | 6-3-10 | 5-10-12 | 2-9-10 | 4-10-3 | 4-10-3 | 4-1-8 | 4-11-12 | 7-10-8

Scale = 1:78.0



6-3-10 | 12-2-6 | 19-10-3 | 28-9-14 | 33-9-10 | 41-8-2

6-3-10 | 5-10-12 | 7-7-13 | 8-11-11 | 4-11-12 | 7-10-8

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-12,0-2-0], [8:0-3-12,0-2-0], [11:0-0-7,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.20	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.29	13-15	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 253 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-4-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-7-14 oc bracing.
WEBS 1 Row at midpt 6-16, 8-15, 8-13, 5-17

REACTIONS (lb/size) 11=1236/0-3-8, 2=599/0-3-8, 17=1758/0-3-8
Max Horz 2=442(load case 4)
Max Uplift11=-559(load case 6), 2=-331(load case 5), 17=-762(load case 4)
Max Grav 11=1236(load case 1), 2=603(load case 7), 17=1758(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-539/391, 3-4=-227/360, 4-5=-212/383, 5-6=-97/482, 6-7=-915/706, 7-8=-915/706, 8-9=-2111/1300, 9-10=-1421/872, 10-11=-1809/898
BOT CHORD 2-18=-264/371, 17-18=-264/371, 15-16=-179/391, 14-15=-391/1101, 13-14=-391/1101, 12-13=-719/1701, 11-12=-592/1407
WEBS 3-18=0/215, 3-17=-482/341, 6-16=-1159/505, 6-15=-398/1023, 7-15=-276/312, 8-15=-365/225, 8-13=-742/1341, 9-13=-1004/715, 9-12=-416/228, 10-12=-147/538, 16-17=-1405/641, 5-16=-234/325

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 559 lb uplift at joint 11, 331 lb uplift at joint 2 and 762 lb uplift at joint 17.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

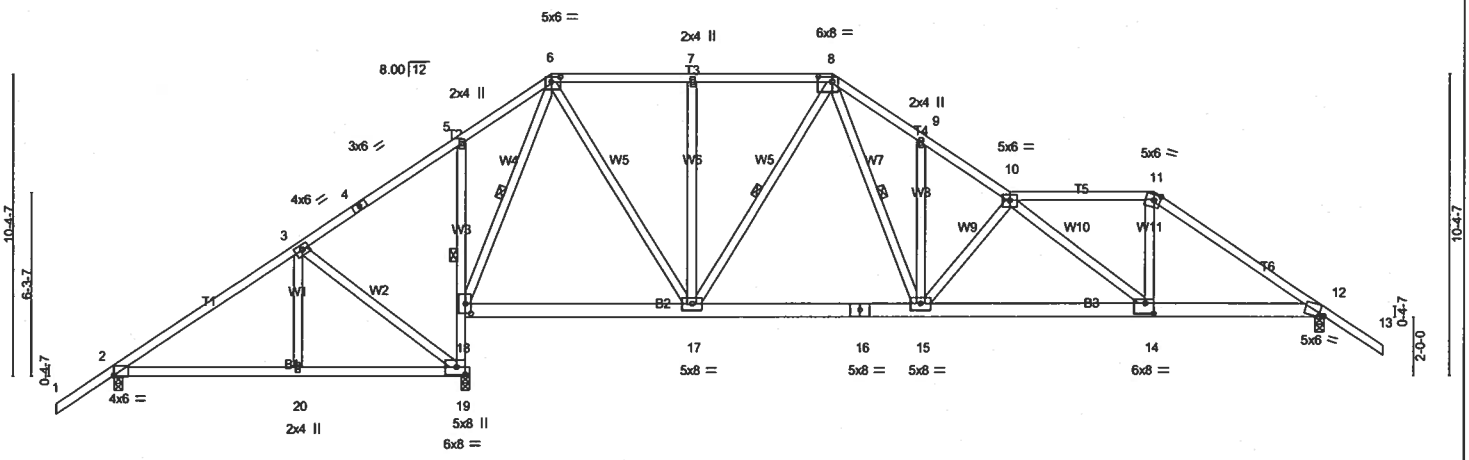
Dwg.#0525061224

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:01 2006 Page 1

-2-0-0	6-3-10	12-2-6	15-0-0	19-10-3	24-8-6	27-9-2	30-9-14	35-9-10	41-8-2	43-8-2
2-0-0	6-3-10	5-10-12	2-9-10	4-10-3	4-10-3	3-0-12	3-0-12	4-11-12	5-10-8	2-0-0

Scale = 1:79.6



6-3-10	12-2-6	19-10-3	27-9-2	35-8-2	35-9-10	41-8-2
6-3-10	5-10-12	7-7-13	7-10-15	7-11-0	0-1-8	5-10-8

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [6:0-3-12,0-2-0], [8:0-5-12,0-2-0], [11:0-2-8,Edge], [12:0-2-0,Edge], [14:0-3-8,0-4-4], [18:0-4-0,0-2-4], [19:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.19	14-15	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.26	14-15	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.94	Horz(TL)	0.03	12	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 284 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-1-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-8-14 oc bracing.
WEBS 1 Row at midpt 6-18, 8-17, 8-15, 5-19

REACTIONS (lb/size) 2=592/0-3-8, 19=2057/0-3-8, 12=2542/0-3-8
Max Horz 2=412(load case 3)
Max Uplift2=-354(load case 5), 19=-893(load case 3), 12=-1305(load case 5)
Max Grav 2=596(load case 6), 19=2057(load case 1), 12=2542(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-527/502, 3-4=-199/472, 4-5=-185/495, 5-6=-71/593, 6-7=-1201/767, 7-8=-1201/767, 8-9=-2800/1528, 9-10=-2829/1396, 10-11=-3291/1661, 11-12=-3980/1898, 12-13=0/62
BOT CHORD 2-20=-257/361, 19-20=-257/361, 17-18=-224/496, 16-17=-591/1577, 15-16=-591/1577, 14-15=-1556/3534, 12-14=-1447/3220
WEBS 3-20=0/215, 3-19=-481/340, 6-18=-1472/648, 6-17=-576/1363, 7-17=-273/295, 8-17=-728/343, 8-15=-1070/2052, 9-15=-129/211, 10-15=-1978/1061, 10-14=-312/165, 11-14=-899/1884, 18-19=-1705/773, 5-18=-238/327

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 354 lb uplift at joint 2, 893 lb uplift at joint 19 and 1305 lb uplift at joint 12.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1483 lb down and 886 lb up at 35-8-2 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-54, 6-8=-54, 8-10=-54, 10-11=-54, 11-13=-54, 2-19=-30, 12-18=-30
Concentrated Loads (lb)
Vert: 14=-1483(F)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061225

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:01 2006 Page 1

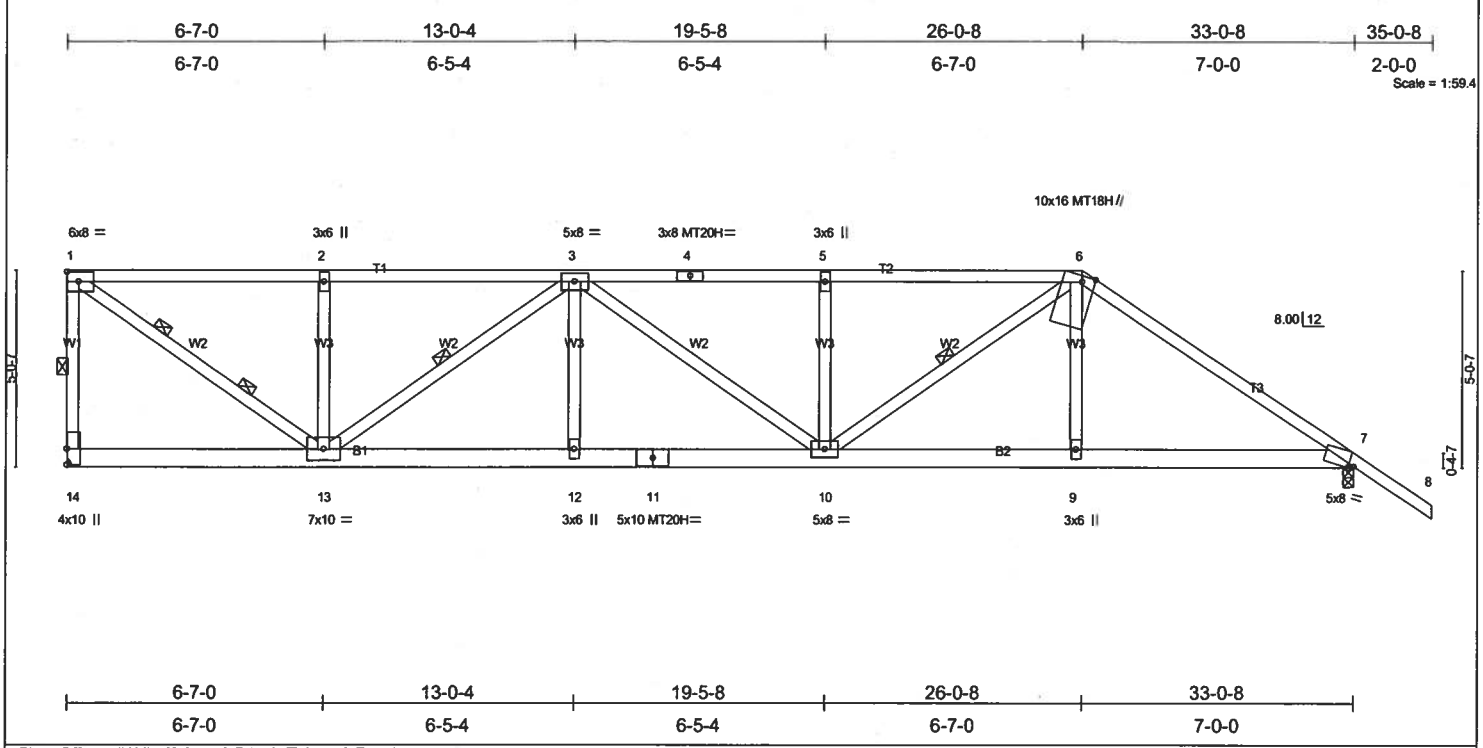


Plate Offsets (X,Y): [6:0-1-13,Edge], [7:0-1-13,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	0.39	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.43	10-12	>906	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.10	7	n/a	n/a	MT18H	244/190
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 209 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T3 2 X 4 SYP No.2D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.2D *Except*
W3 2 X 4 SYP No.3, W3 2 X 4 SYP No.3, W3 2 X 4 SYP No.3, W3 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-3-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-2-1 oc bracing.
WEBS 1 Row at midpt 1-14, 3-13, 6-10
2 Rows at 1/3 pts 1-13

REACTIONS (lb/size) 14=3034/Mechanical, 7=2927/0-3-8
Max Horz 14=-362(load case 5)
Max Uplift14=-2331(load case 3), 7=-1905(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-14=-2812/2321, 1-2=-3495/2669, 2-3=-3495/2669, 3-4=-5371/4045, 4-5=-5371/4045, 5-6=-5372/4043, 6-7=-4665/3251, 7-8=0/62
BOT CHORD 13-14=-99/276, 12-13=-3977/5246, 11-12=-3977/5246, 10-11=-3977/5246, 9-10=-2678/3815, 7-9=-2666/3788
WEBS 1-13=-3162/4181, 2-13=-781/1054, 3-13=-2162/1627, 3-12=0/431, 3-10=-165/155, 5-10=-773/1088, 6-10=-1661/1905, 6-9=-440/873

NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) All plates are MT20 plates unless otherwise indicated.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2331 lb uplift at joint 14 and 1905 lb uplift at joint 7.
6) Girder carries hip end with 7-0-0 right side setback, 0-0-0 left side setback, and 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 506 lb up at 26-0-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-6=-118(F=-64), 6-8=-54, 9-14=-65(F=-35), 7-9=-30
Concentrated Loads (lb)
Vert: 9=-539(F)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

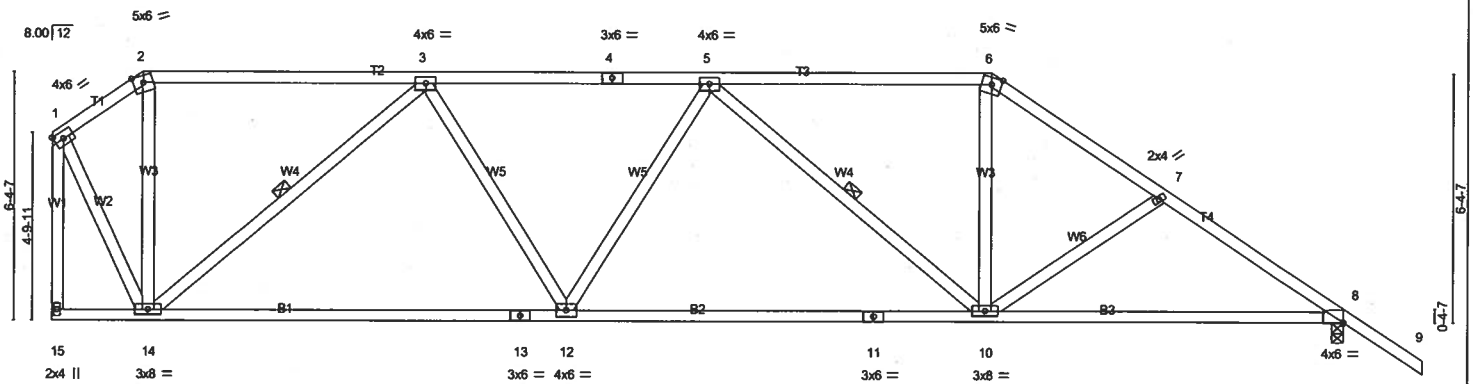
Dwg.#0525061226

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T20	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:02 2006 Page 1

2-4-2	9-6-15	16-9-11	24-0-8	28-4-1	33-0-8	35-0-8
2-4-2	7-2-13	7-2-13	7-2-13	4-3-9	4-8-7	2-0-0

Scale = 1:59.2



2-4-2	13-2-5	24-0-8	33-0-8
2-4-2	10-10-3	10-10-3	9-0-0

Plate Offsets (X,Y): [8:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.59	Vert(LL)	-0.25	10-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.36	10-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 192 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 4-5-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-14, 5-10

REACTIONS (lb/size) 15=1372/Mechanical, 8=1495/0-3-8
Max Horz 15=-385(load case 6)
Max Uplift15=-667(load case 4), 8=-624(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-651/314, 2-3=-555/328, 3-4=-1798/980, 4-5=-1798/980, 5-6=-1532/835, 6-7=-1877/922, 7-8=-2058/952, 8-9=0/59, 1-15=-1440/658
BOT CHORD 14-15=-55/382, 13-14=-844/1535, 12-13=-844/1535, 11-12=-933/1864, 10-11=-933/1864, 8-10=-665/1647
WEBS 2-14=0/123, 3-14=-1297/831, 3-12=-145/519, 5-12=-132/223, 5-10=-515/486, 6-10=-228/681, 7-10=-159/244, 1-14=-631/1233

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 667 lb uplift at joint 15 and 624 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

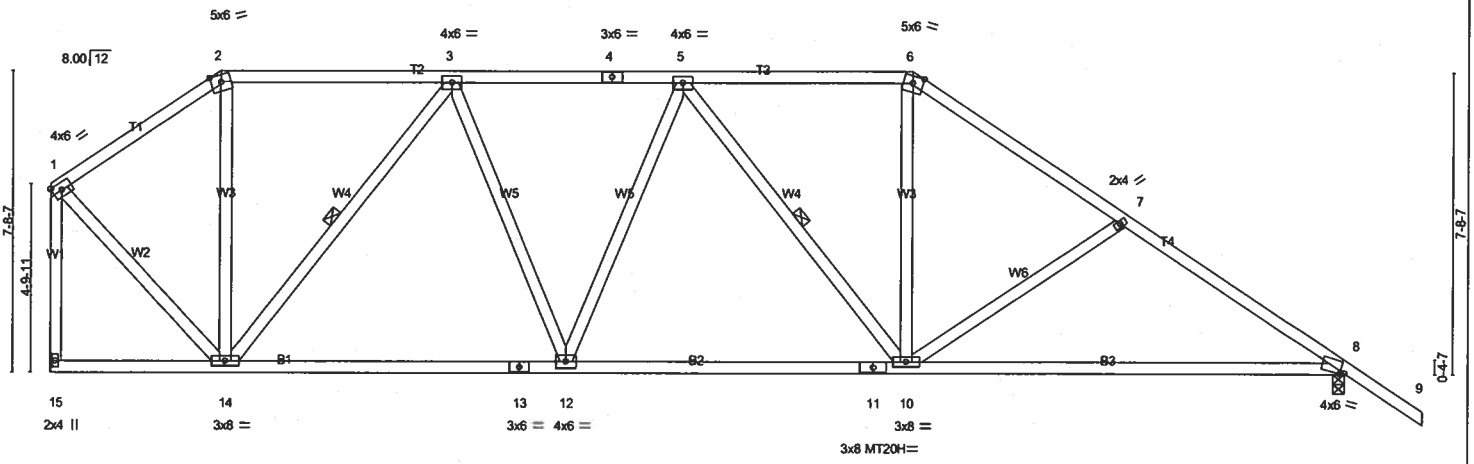
Dwg.#0525061227

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T21	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:02 2006 Page 1

4-4-2	10-2-15	16-1-11	22-0-8	27-4-1	33-0-8	35-0-8
4-4-2	5-10-13	5-10-13	5-10-13	5-3-9	5-8-7	2-0-0

Scale = 1:59.2



4-4-2	13-2-5	22-0-8	33-0-8
4-4-2	8-10-3	8-10-3	11-0-0

Plate Offsets (X,Y): [8:0-1-5,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.31	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.47	8-10	>845	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.06	8	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 203 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 4-2-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-14, 5-10

REACTIONS (lb/size) 15=1372/Mechanical, 8=1495/0-3-8
Max Horz 15=-417(load case 6)
Max Uplift15=-563(load case 4), 8=-613(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-928/480, 2-3=-734/474, 3-4=-1462/829, 4-5=-1462/829, 5-6=-1400/808, 6-7=-1754/864, 7-8=-2005/945, 8-9=0/59, 1-15=-1337/669
BOT CHORD 14-15=-118/408, 13-14=-663/1300, 12-13=-663/1300, 11-12=-685/1524, 10-11=-685/1524, 8-10=-573/1613
WEBS 2-14=-75/254, 3-14=-946/617, 3-12=-146/448, 5-12=-178/201, 5-10=-305/381, 6-10=-204/628, 7-10=-272/330, 1-14=-507/1044

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 563 lb uplift at joint 15 and 613 lb uplift at joint 8.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061228

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:03 2006 Page 1

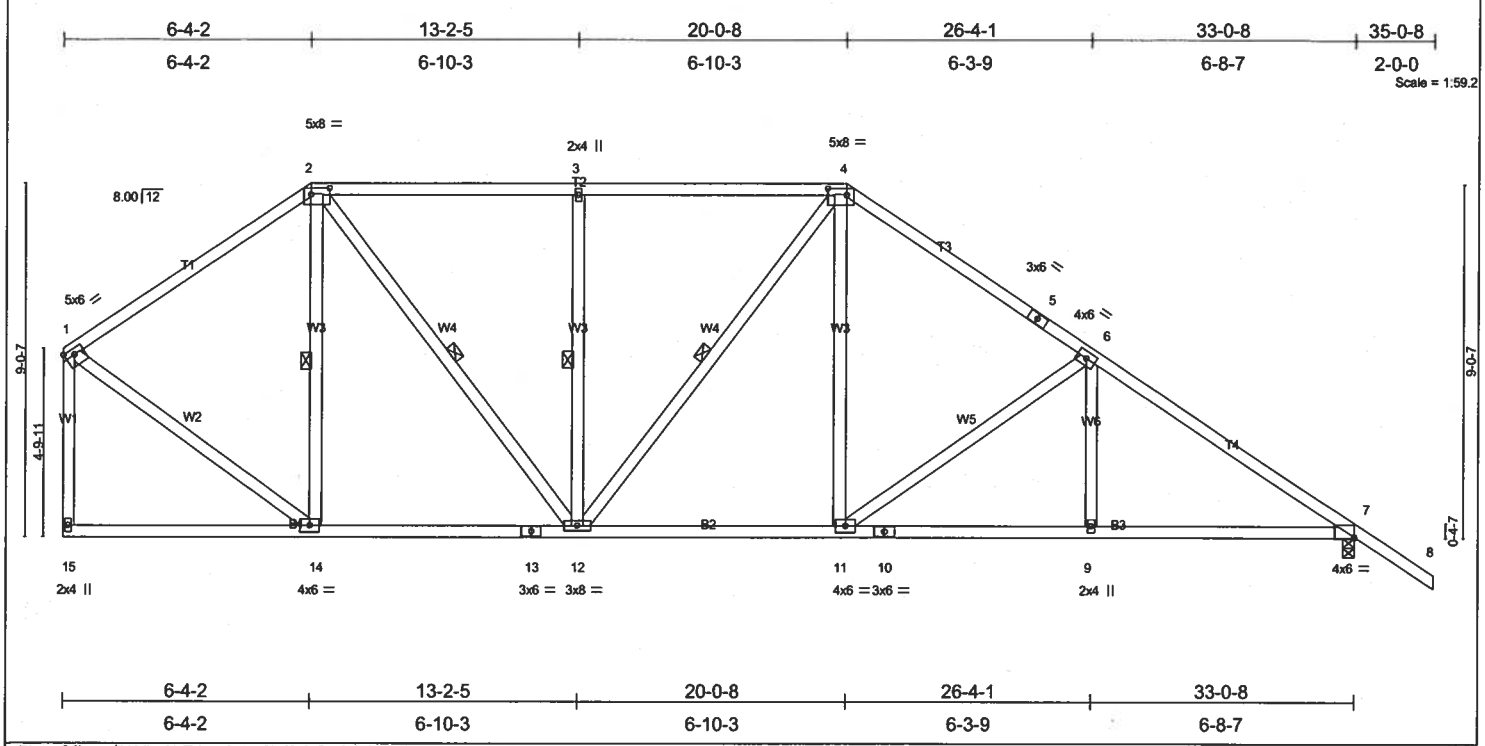


Plate Offsets (X,Y): [1:Edge,0-1-12], [2:0-5-12,0-2-0], [4:0-5-12,0-2-0], [7:0-0-0,0-0-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.62	Vert(LL)	-0.10	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.15	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.52	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 213 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 4-3-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-2-1 oc bracing.
WEBS 1 Row at midpt 2-14, 2-12, 3-12, 4-12

REACTIONS (lb/size) 15=1372/Mechanical, 7=1495/0-3-8
Max Horz 15=-449(load case 6)
Max Uplift15=-453(load case 4), 7=-636(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1087/570, 2-3=-1266/787, 3-4=-1266/787, 4-5=-1427/833, 5-6=-1597/806, 6-7=-2087/899, 7-8=0/59, 1-15=-1280/669
BOT CHORD 14-15=-184/430, 13-14=-420/827, 12-13=-420/827, 11-12=-364/1257, 10-11=-507/1641, 9-10=-507/1641, 7-9=-507/1641
WEBS 2-14=-371/302, 2-12=-481/758, 3-12=-386/422, 4-12=-284/154, 4-11=-180/471, 6-11=-481/381, 6-9=0/212, 1-14=-453/972

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 453 lb uplift at joint 15 and 636 lb uplift at joint 7.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061229

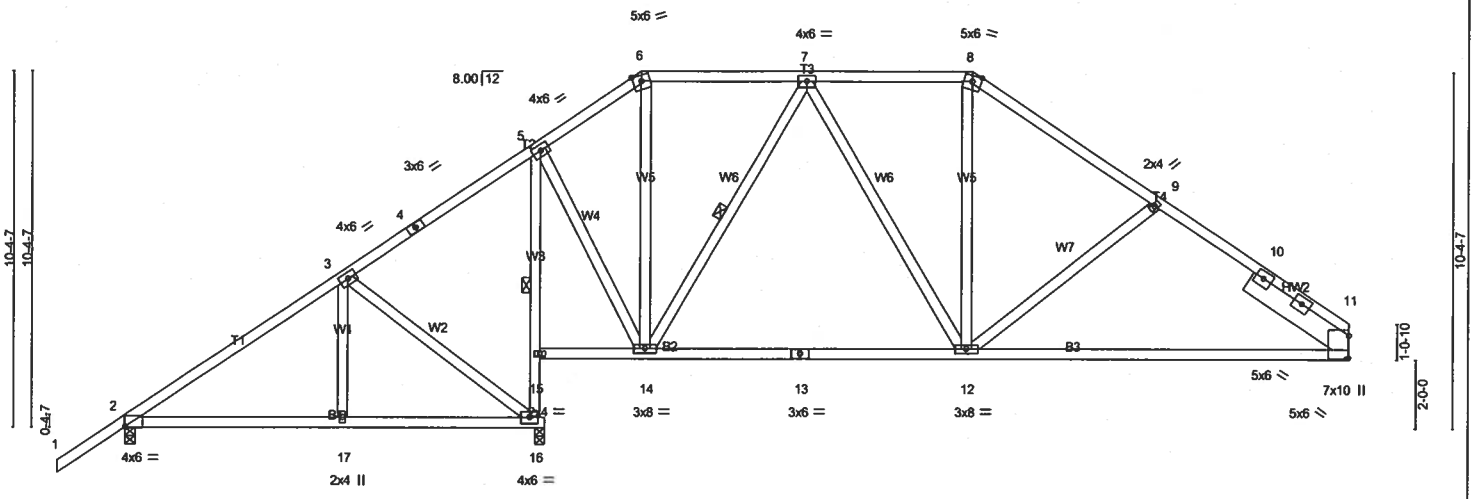
Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T23	SPECIAL	3	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:04 2006 Page 1

-2-0-0 | 6-3-10 | 12-2-6 | 15-0-0 | 19-10-3 | 24-8-6 | 30-0-8 | 35-8-2

2-0-0 | 6-3-10 | 5-10-12 | 2-9-10 | 4-10-3 | 4-10-3 | 5-4-2 | 5-7-10

Scale = 1:67.5



6-3-10 | 12-2-6 | 15-0-0 | 24-8-6 | 35-8-2

6-3-10 | 5-10-12 | 2-9-10 | 9-8-6 | 10-11-12

Plate Offsets (X,Y): [2:0-0-0,0-0-4], [11:0-7-15,0-0-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.19	11-12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.29	11-12	>993	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.02	11	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 226 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2D
BOT CHORD	2 X 4 SYP No.2D
WEBS	2 X 4 SYP No.3
SLIDER	Right 2 X 8 SYP No.1D 3-6-7

BRACING

TOP CHORD	Sheathed or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 14-15.
WEBS	1 Row at midpt 7-14, 5-16

REACTIONS (lb/size) 11=993/Mechanical, 2=610/0-3-8, 16=1498/0-3-8
Max Horz 2=442(load case 4)
Max Uplift11=-433(load case 6), 2=-334(load case 5), 16=-649(load case 4)
Max Grav 11=993(load case 1), 2=610(load case 7), 16=1498(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/60, 2-3=-553/399, 3-4=-222/368, 4-5=-207/391, 5-6=-481/500, 6-7=-358/457, 7-8=-775/607, 8-9=-1004/625, 9-10=-1087/686, 10-11=-1245/665
BOT CHORD	2-17=-268/383, 16-17=-268/383, 14-15=-6/12, 13-14=-242/641, 12-13=-242/641, 11-12=-407/936
WEBS	3-17=0/215, 3-16=-485/341, 5-14=-285/761, 6-14=-158/116, 7-14=-571/302, 7-12=-68/269, 8-12=-89/232, 9-12=-211/321, 15-16=-1142/528, 5-15=-1138/528

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 433 lb uplift at joint 11, 334 lb uplift at joint 2 and 649 lb uplift at joint 16.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061230

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T26	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:04 2006 Page 1

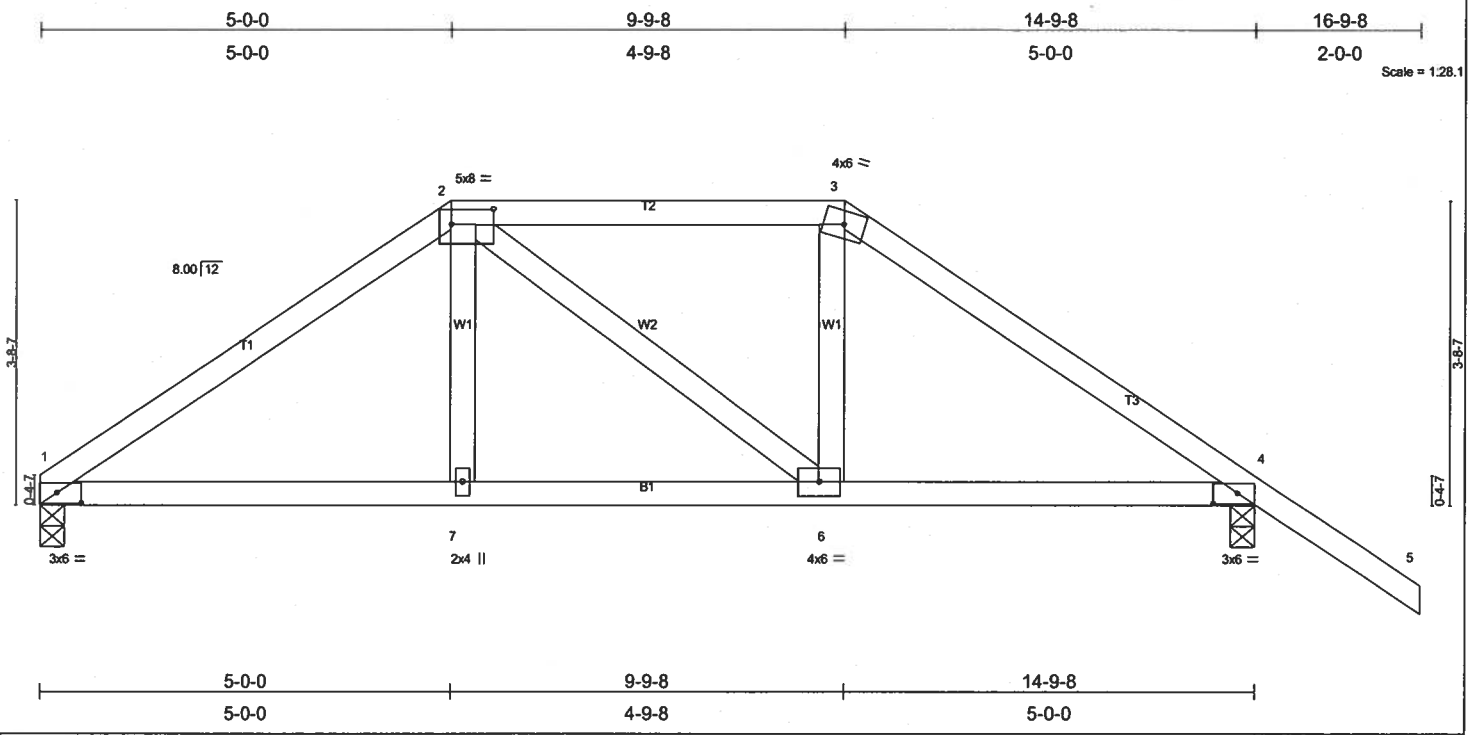


Plate Offsets (X,Y): [1:0-3-9,0-1-8], [2:0-6-4,0-2-4], [4:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.04	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.36	Vert(TL)	-0.06	6-7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.16	Horz(TL)	0.02	4	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 68 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-2-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-4 oc bracing.

REACTIONS (lb/size) 1=979/0-3-8, 4=1112/0-3-8
Max Horz 1=-179(load case 5)
Max Uplift1=-563(load case 4), 4=-729(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1476/892, 2-3=-1170/790, 3-4=-1473/880, 4-5=0/59
BOT CHORD 1-7=-787/1173, 6-7=-796/1193, 4-6=-673/1151
WEBS 2-7=-212/458, 2-6=-113/139, 3-6=-244/485

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 563 lb uplift at joint 1 and 729 lb uplift at joint 4.
5) Girder carries hip end with 5-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 230 lb up at 9-9-8, and 245 lb down and 230 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-3=-91(F=-37), 3-5=-54, 1-7=-30, 6-7=-50(F=-20), 4-6=-30
Concentrated Loads (lb)
Vert: 7=-245(F) 6=-245(F)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061231

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T27	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:05 2006 Page 1

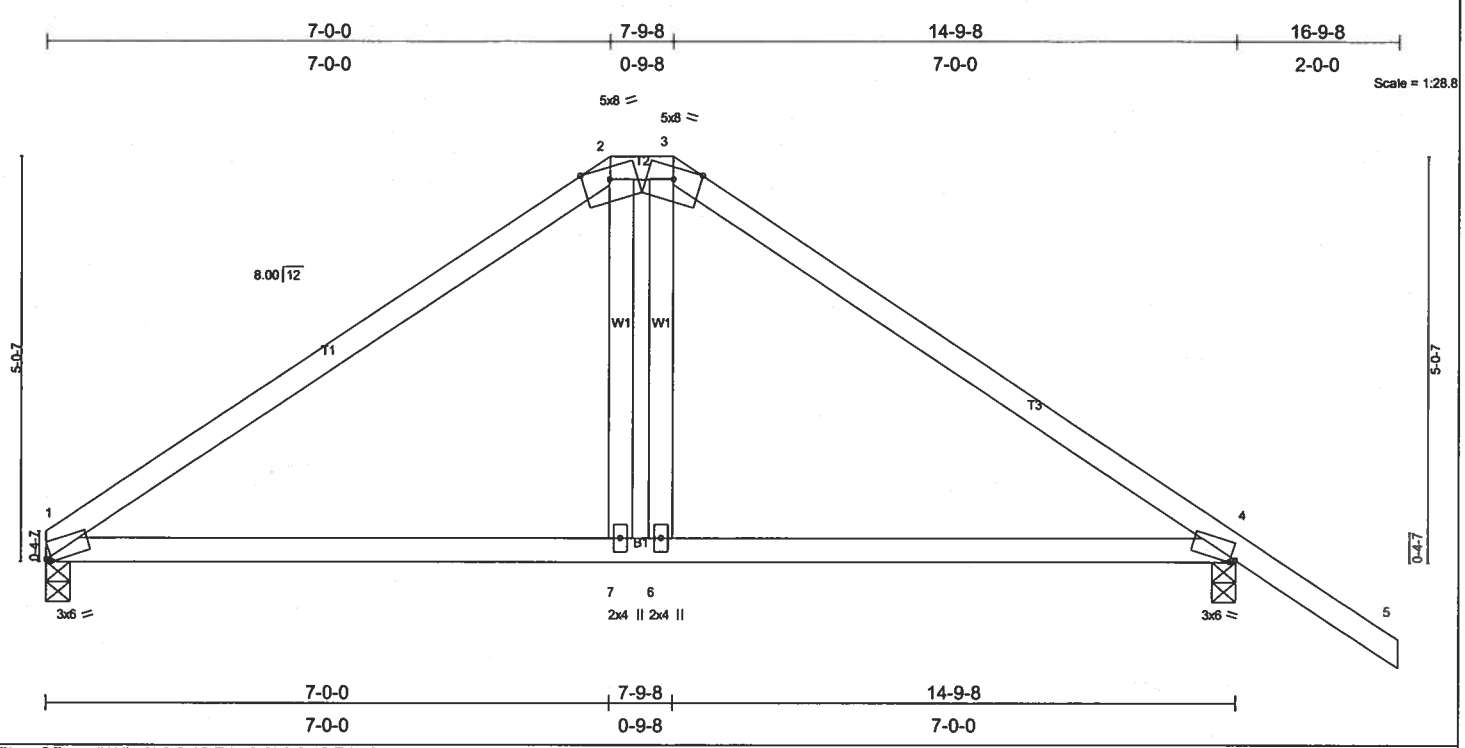


Plate Offsets (X,Y): [1:0-0-12,Edge], [4:0-0-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.12	1-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.42	Vert(TL)	-0.16	1-7	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.11	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 66 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=600/0-3-8, 4=733/0-3-8
Max Horz 1=-231(load case 3)
Max Uplift1=-224(load case 5), 4=-389(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-740/315, 2-3=-538/377, 3-4=-745/324, 4-5=0/59
BOT CHORD 1-7=-126/534, 6-7=-121/538, 4-6=-115/534
WEBS 2-7=-269/323, 3-6=-282/336

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 1 and 389 lb uplift at joint 4.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061232

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T28	COMMON	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:05 2006 Page 1

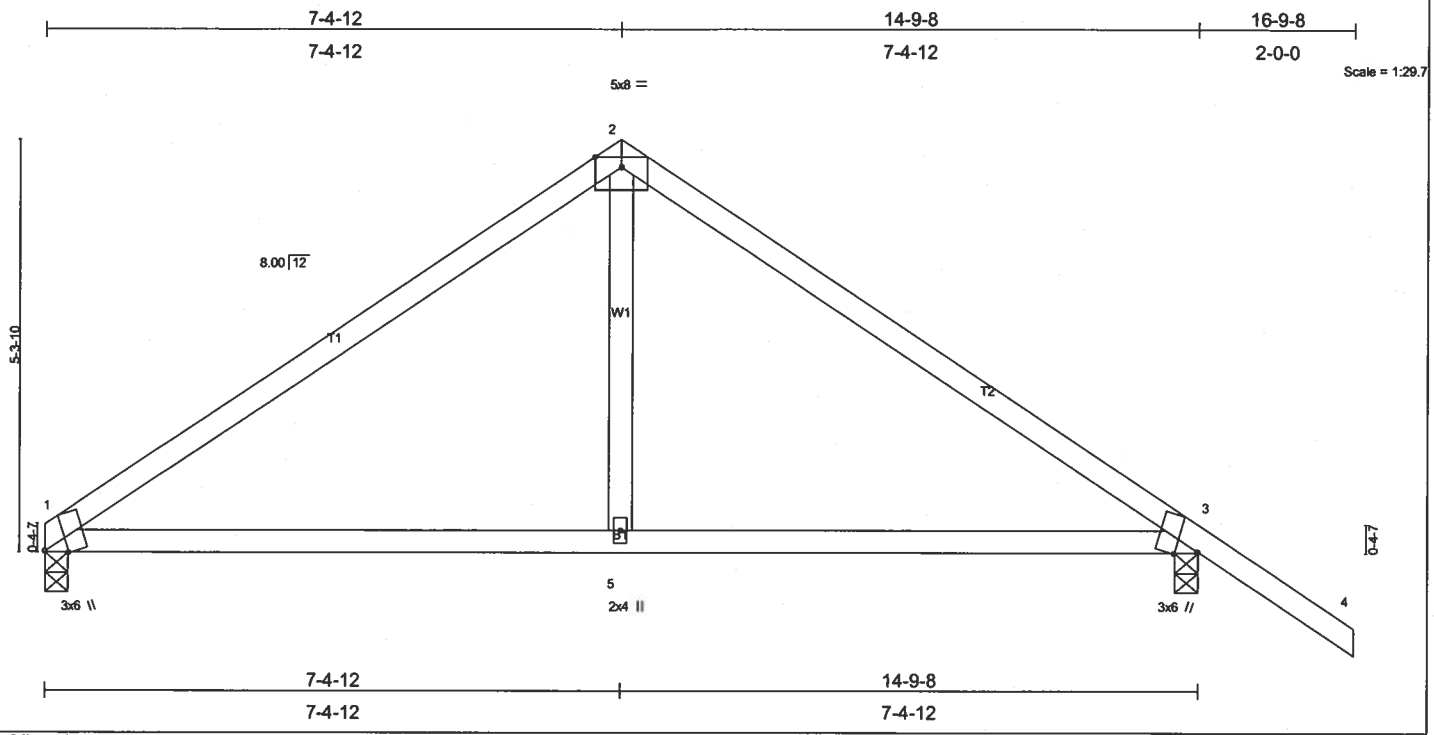


Plate Offsets (X,Y): [1:0-1-5,Edge], [3:0-1-5,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	0.11	1-5	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.15	1-5	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 60 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=600/0-3-8, 3=733/0-3-8
Max Horz 1=-241(load case 3)
Max Uplift1=-227(load case 5), 3=-392(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-739/311, 2-3=-744/319, 3-4=0/59
BOT CHORD 1-5=-129/533, 3-5=-129/533
WEBS 2-5=0/284

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 227 lb uplift at joint 1 and 392 lb uplift at joint 3.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061233

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T29	COMMON	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:06 2006 Page 1

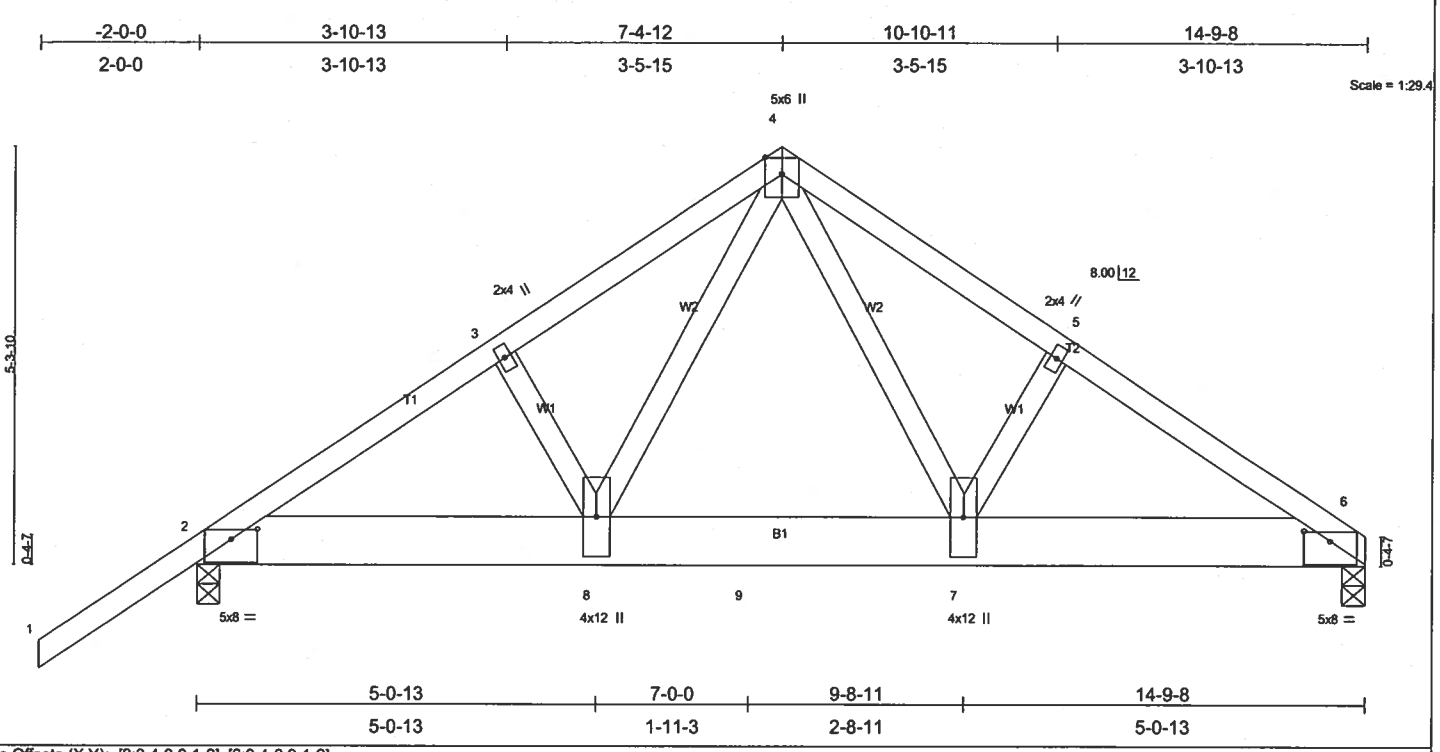


Plate Offsets (X,Y): [2:0-4-0,0-1-9], [6:0-4-0,0-1-9]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.32	Vert(LL)	-0.12	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.16	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.87	Horz(TL)	-0.02	2	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 196 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-10-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=5670/0-3-8, 2=3635/0-3-8
Max Horz 6=239(load case 2)
Max Uplift6=-2731(load case 4), 2=-1815(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 4-5=-7636/3737, 5-6=-7732/3697, 1-2=0/65, 2-3=-6392/2996, 3-4=-6280/3021
BOT CHORD 2-8=-2403/5249, 8-9=-1822/3996, 7-9=-1822/3996, 6-7=-3084/6399
WEBS 5-7=-129/201, 4-7=-2695/5402, 4-8=-1335/2816, 3-8=-91/169

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2731 lb uplift at joint 6 and 1815 lb uplift at joint 2.
6) Girder carries tie-in span(s): 33-0-8 from 7-0-0 to 14-9-8
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3034 lb down and 1462 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 4-6=-54, 1-4=-54, 2-9=-30, 6-9=-676(F=-646)
Concentrated Loads (lb)
Vert: 9=-3034(F)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061234

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T31	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:06 2006 Page 1

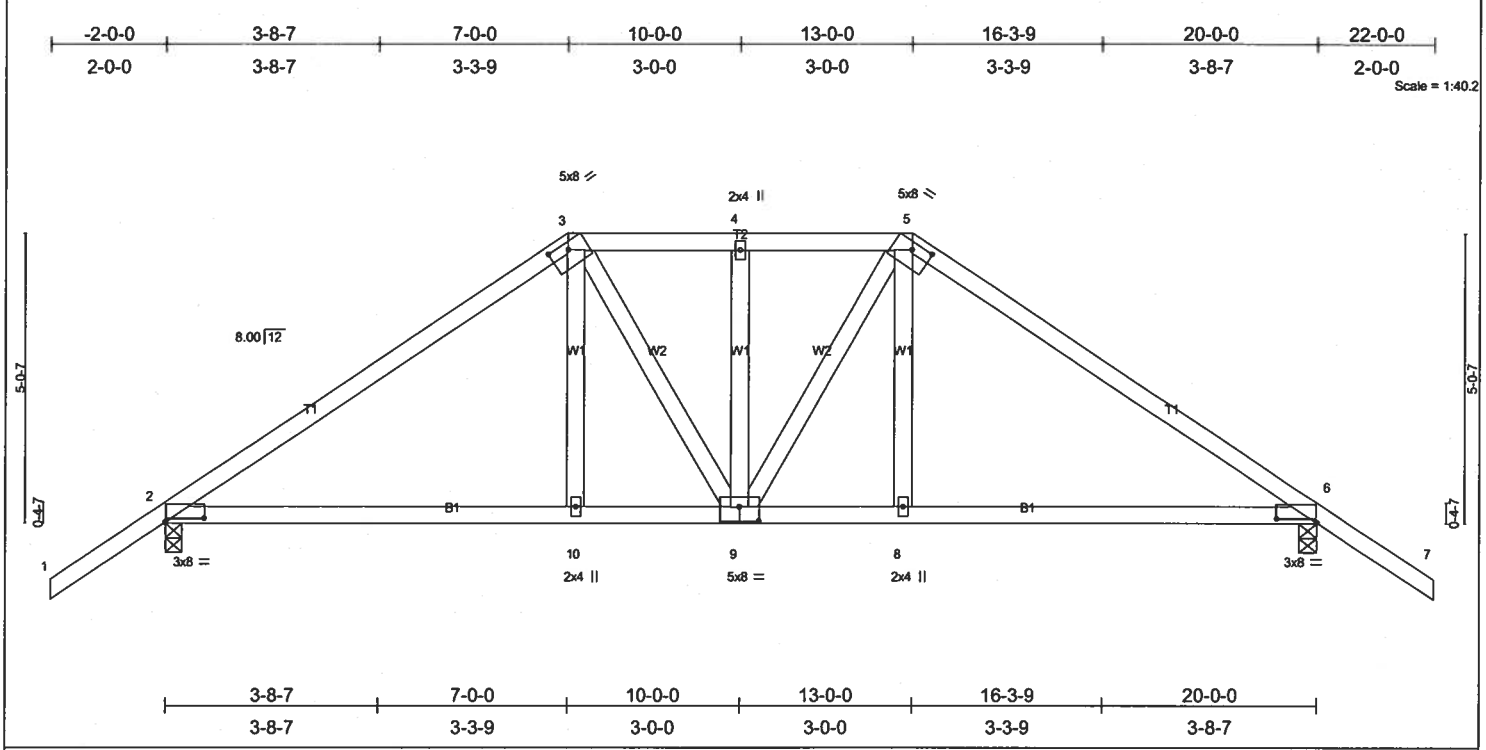


Plate Offsets (X,Y): [2:0-8-3,0-0-14], [3:0-4-0,0-1-9], [5:0-4-0,0-1-9], [6:0-8-3,0-0-14], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.11	2-10	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.16	2-10	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.25	Horz(TL)	0.05	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-8-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-3-15 oc bracing.

REACTIONS (lb/size) 2=1774/0-3-8, 6=1774/0-3-8
Max Horz 2=203(load case 3)
Max Uplift2=-1227(load case 4), 6=-1227(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-2558/1691, 3-4=-2147/1565, 4-5=-2147/1564, 5-6=-2558/1692, 6-7=0/59
BOT CHORD 2-10=-1462/2026, 9-10=-1476/2050, 8-9=-1332/2050, 6-8=-1319/2026
WEBS 3-10=-467/794, 3-9=-421/298, 4-9=-261/490, 5-9=-422/298, 5-8=-467/794

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1227 lb uplift at joint 2 and 1227 lb uplift at joint 6.
5) Girder carries hip end with 7-0-0 end setback.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 506 lb up at 13-0-0, and 539 lb down and 506 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-118(F=-64), 5-7=-54, 2-10=-30, 8-10=-65(F=-35), 6-8=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061235

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T32	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:07 2006 Page 1

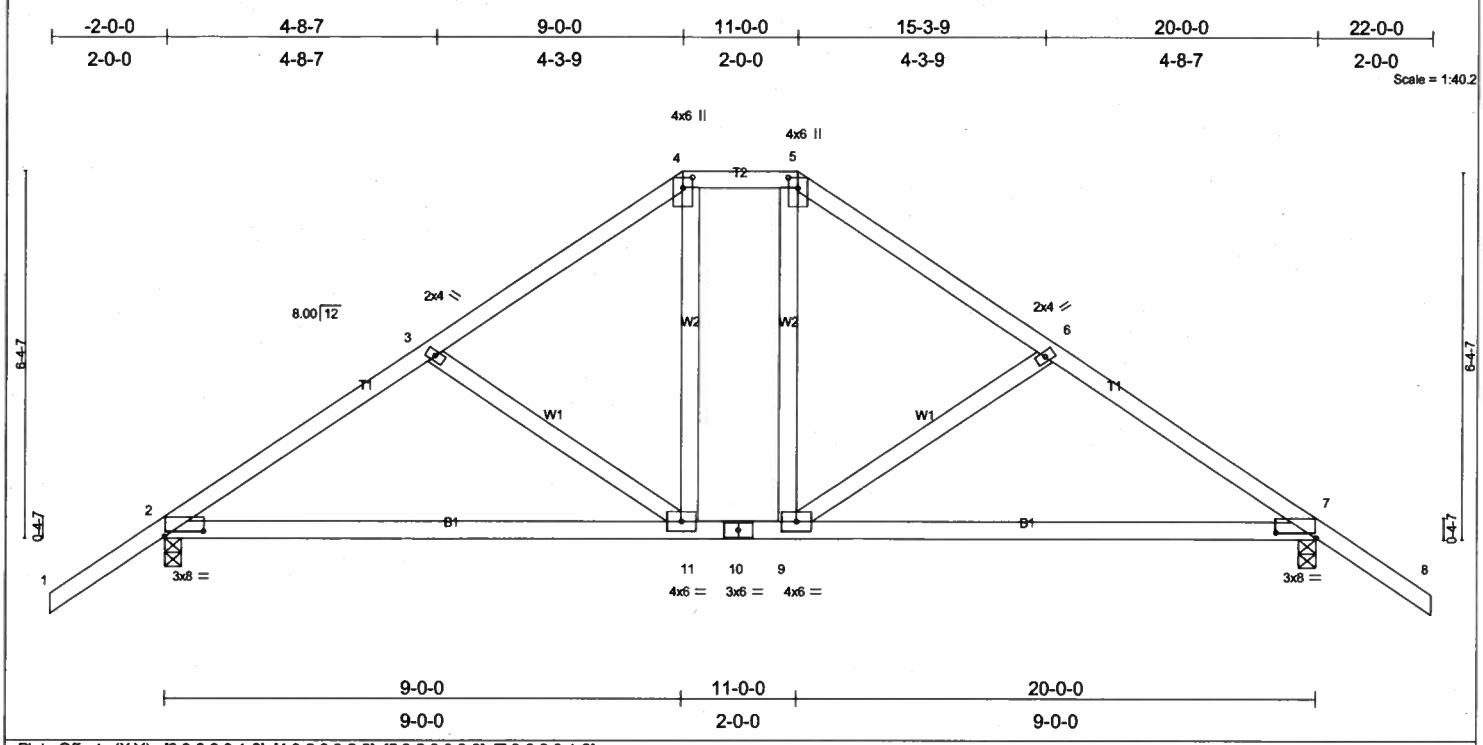


Plate Offsets (X,Y): [2:0-8-3,0-1-2], [4:0-2-3,0-2-0], [5:0-2-3,0-2-0], [7:0-8-3,0-1-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.16	2-11	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.23	2-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.02	7	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 105 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-11-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=944/0-3-8, 7=944/0-3-8
Max Horz 2=259(load case 4)
Max Uplift2=-461(load case 5), 7=-461(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-1096/494, 3-4=-888/429, 4-5=-683/426, 5-6=-888/429, 6-7=-1096/494, 7-8=0/59
BOT CHORD 2-11=-324/868, 10-11=-121/683, 9-10=-121/683, 7-9=-201/868
WEBS 3-11=-254/271, 4-11=-90/293, 5-9=-90/293, 6-9=-254/272

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 461 lb uplift at joint 2 and 461 lb uplift at joint 7.

LOAD CASE(S) Standard

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061236

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T33	COMMON	5	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:07 2006 Page 1

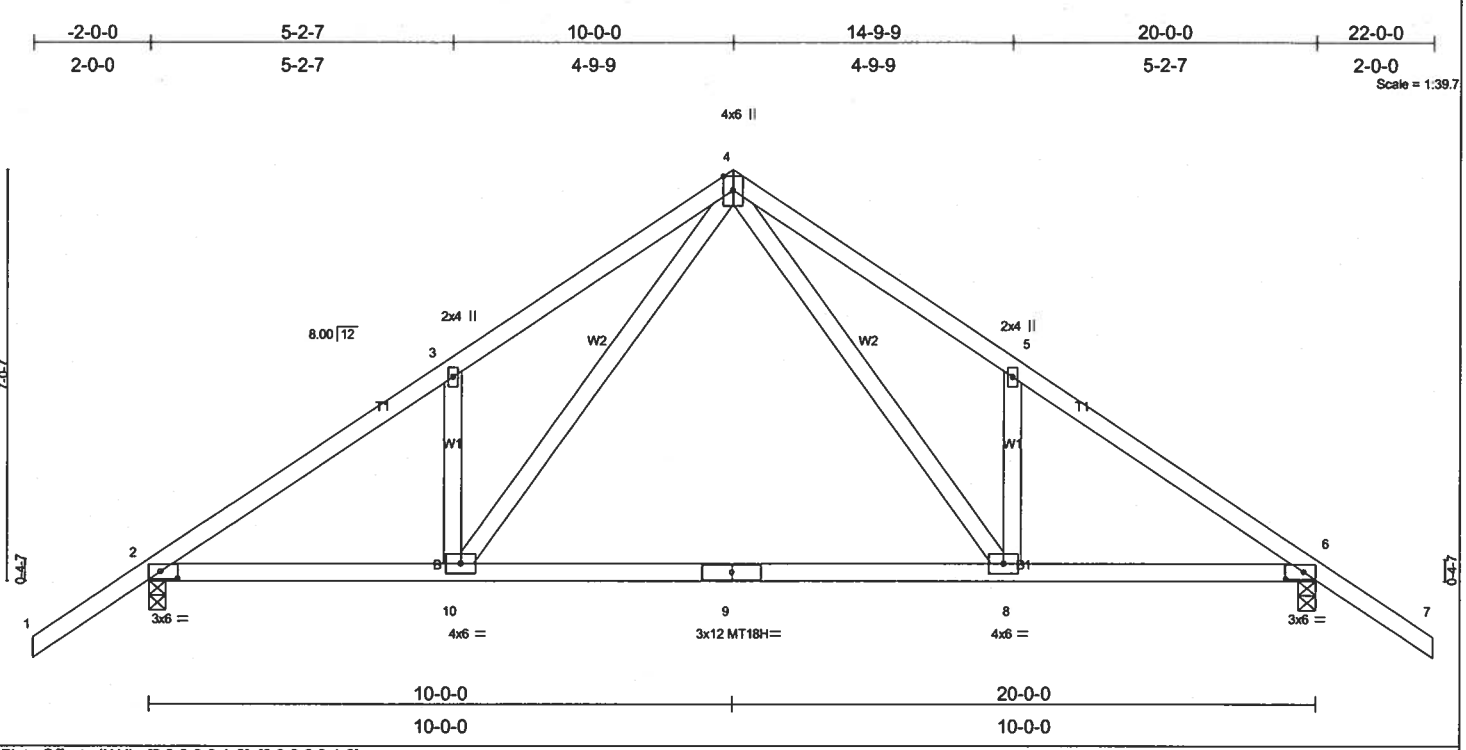


Plate Offsets (X,Y): [2:0-3-9,0-1-8], [6:0-3-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.47	8-10	>498	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.68	8-10	>349	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	NO	WB	0.59	Horz(TL)	0.03	6	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 107 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-7-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-0-15 oc bracing.

REACTIONS (lb/size) 2=1183/0-3-8, 6=1183/0-3-8
Max Horz 2=-286(load case 3)
Max Uplift2=-584(load case 5), 6=-584(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/59, 2-3=-1732/677, 3-4=-1679/858, 4-5=-1679/858, 5-6=-1732/677, 6-7=0/59
BOT CHORD 2-10=-511/1345, 9-10=-221/809, 8-9=-221/809, 6-8=-397/1345
WEBS 4-10=-499/942, 4-8=-500/942, 3-10=-200/319, 5-8=-200/319

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 584 lb uplift at joint 2 and 584 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

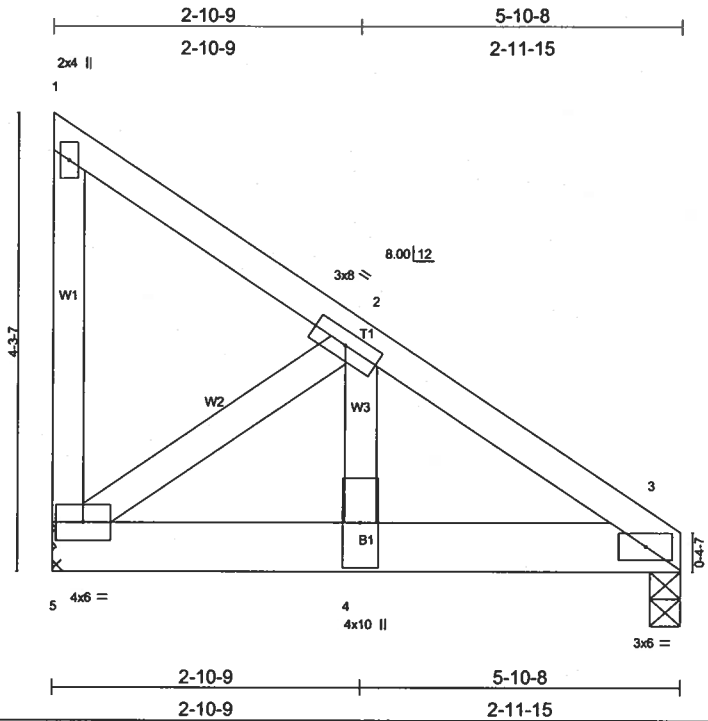
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (pff)
Vert: 1-4=-54, 4-7=-54, 2-10=-30, 8-10=-80(F=-50), 6-8=-30

MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0525061237

Job	Truss	Truss Type	Qty	Ply	LIPSCOMB EAGLE-LOT 45 EMERALD COVE
L166852	T34	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Thu May 25 11:57:08 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.17	Vert(LL)	0.02	3-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.02	3-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	3	n/a	n/a		
BCDL	5.0	Code	FBC2001/ANSI95	(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-3-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=1483/Mechanical, 3=1483/0-3-8
Max Horz 5=-208(load case 4)
Max Uplift5=-806(load case 4), 3=-665(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-5=-64/75, 1-2=-55/27, 2-3=-1501/620
BOT CHORD 4-5=-475/1212, 3-4=-475/1212
WEBS 2-5=-1492/823, 2-4=-704/1535

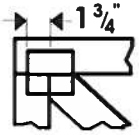
NOTES
1) Wind: ASCE 7-98; 120mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 806 lb uplift at joint 5 and 665 lb uplift at joint 3.
4) Girder carries tie-in span(s): 23-5-0 from 0-0-0 to 5-10-8
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-477(F=-447)

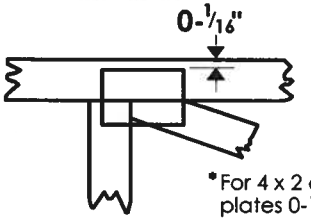
MAY 25, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

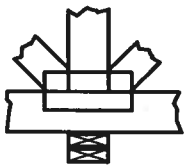
PLATE SIZE

4 X 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

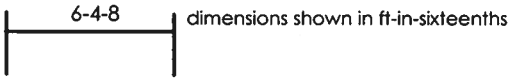


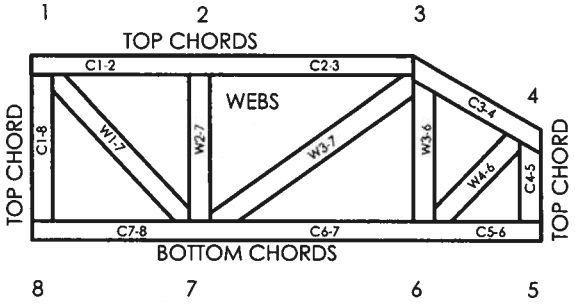
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System





JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

GTC DESIGN GROUP

PROJECT NAME: GATEWAY DEVELOPEMT
PROJECT NUMBER: PF05-033

WIND LOAD AND STRUCTURAL CALCULATIONS FOR

GATEWAY DEVELOPMENT, LLC
"MacKenzie" MODEL HOME
LOT 46
EMERALD COVE SUBDIVISION

INDEX

GENERAL INFORMATION		
	DESIGN BASIS	PAGE 2
	CALCULATION / DESIGN SUMMARY	PAGE 3
	DESIGN LOADS	PAGE 4
WALL COMPONENTS		
	SHEARWALL DESIGN – N/S	PAGE 5 - 7
	SHEARWALL DESIGN – E/W	PAGE 8 - 10
STRUCTURAL ITEMS		
	WIND LOADS – ASCE 7-98	PAGE 11 - 16

GARY GILE, PE
GTC DESIGN GROUP, LLC
P.O. BOX 187
LIVE OAK, FL 32064
386-362-3678
386-362-6133 (FAX)
AUTH. # 9461

Project name: MacKenzie Windload Analysis
Project: PF05-034
Client Gateway Dev
Calculations: Gary Gill, PE
Date: 4/7/2005

Design Basis

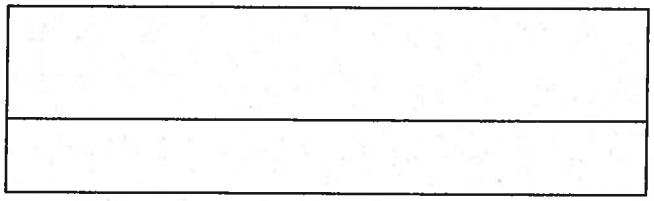
Design Loads

Wind Load	110	
Floor Live Load		
	Sleep Areas =	30 psf
	All Others =	40 psf
Floor Dead Load		10 psf
Wall Dead Load		10 psf
Roof Live Load		20 psf
Roof Dead Load		10 psf

Load Combinations

DL + LL(floor) + LL (roof)
DL + LL(floor) +WL
DL + WL
Wind load

Exposure B



Building Information

Shape	Rectangle
Length	46.17 ft
Width	56.4 ft
Type	1 storey sog

References

2004 Florida Building Code
ASCE 7-98 Minimum Design Loads for Buildings and Other Structures
AITC Timber Construction Manual

WIND98 v3-02

Wind Load Design per ASCE 7-98

Description: MacKinsey -
Analysis by: Gary Gill

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	33.69	Deg
Type of Roof	Gabled	
Kd (Directonality Factor)	0.85	
Eave Height (Eht)	11.00	ft
Ridge Height (RHt)	26.25	ft
Mean Roof Height (Ht)	22.00	ft
Width Perp. To Wind Dir (B)	63.46	ft
Width Paral. To Wind Dir (L)	42.40	ft
Damping Ratio (beta)	0.02	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.52
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^0.63))^0.5	0.9002	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8661	
Gust Factor Summary			
G	Since this is not a flexible structure the lessor of Gust1 or Gust2 are used	0.85	

 www.mecaconsulting.com

WIND98 v3-02

Wind Load Design per ASCE 7-98

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev	Kz	Kzt	qz	Pressure (lb/ft^2)	
				Windward Wall*	
ft			lb/ft^2	+GCpi	-GCpi
26.25	0.67	1.00	17.76	9.04	15.11
22	0.64	1.00	16.88	8.44	14.52
20	0.62	1.00	16.43	8.13	14.21
15	0.57	1.00	15.13	7.25	13.33

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

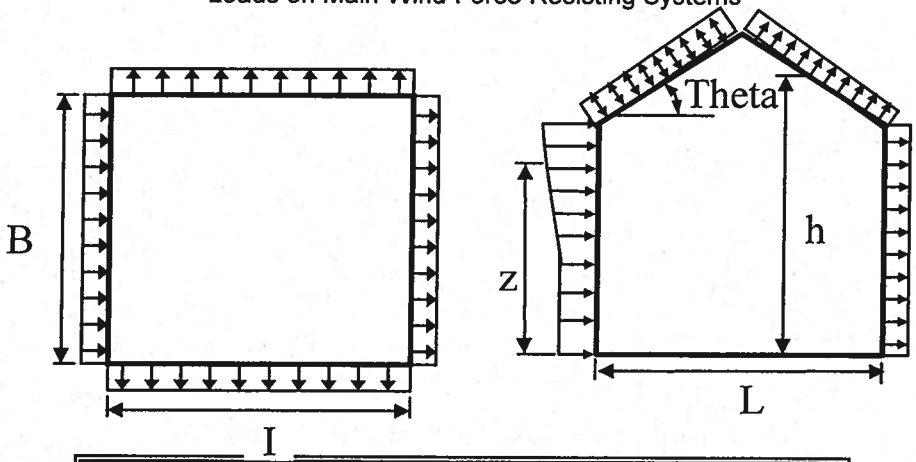
Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	0.64	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*I*Kh*Kht*Kd	16.88	psf
Khcc	Comp & Clad: Table 6-5 Case 2	0.70	
Qhcc	.00256*V^2*I*Khcc*Kht*Kd	18.45	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.8

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Calculations for Wind Normal to 63.46 ft Face *Additional Runs may be req'd for other wind directions*	Cp	Pressure (psf) +GCpi	 -GCpi
Leeward Walls (Wind Dir Normal to 63.46 ft wall)	-0.50	-10.21	-4.14
Side Walls	-0.70	-13.08	-7.01
Roof - Wind Normal to Ridge (Theta>=10) - for Wind Normal to 63.46 ft face			
Windward - Max Negative	-0.20	-5.92	0.15
Windward - Max Positive	0.27	0.85	6.93
Leeward Normal to Ridge	-0.60	-11.65	-5.57
Overhang Top (Windward)	-0.20	-2.88	-2.88
Overhang Top (Leeward)	-0.60	-8.61	-8.61
Overhang Bottom (Applicable on Windward only)	0.80	10.29	10.29
Roof - Wind Parallel to Ridge (All Theta) - for Wind Normal to 63.46 ft face			
Dist from Windward Edge: 0 ft to 11 ft	-0.92	-16.17	-10.09
Dist from Windward Edge: 11 ft to 22 ft	-0.89	-15.85	-9.77

* Horizontal distance from windward edge

WIND98 v3-02

Wind Load Design per ASCE 7-98

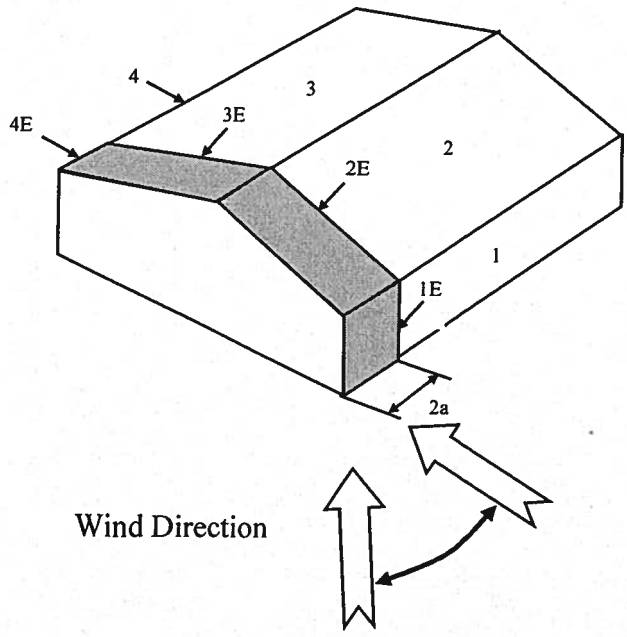
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.64
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	16.88

Case A						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	0.56	0.18	-0.18	16.88	6.42	12.49
2	0.21	0.18	-0.18	16.88	0.51	6.58
3	-0.43	0.18	-0.18	16.88	-10.30	-4.22
4	-0.37	0.18	-0.18	16.88	-9.29	-3.21
5	0.00	0.18	-0.18	16.88	-3.04	3.04
6	0.00	0.18	-0.18	16.88	-3.04	3.04
1E	0.69	0.18	-0.18	16.88	8.61	14.69
2E	0.27	0.18	-0.18	16.88	1.52	7.60
3E	-0.53	0.18	-0.18	16.88	-11.99	-5.91
4E	-0.48	0.18	-0.18	16.88	-11.14	-5.06
5E	0.00	0.18	-0.18	16.88	-3.04	3.04
6E	0.00	0.18	-0.18	16.88	-3.04	3.04

* p = qh * (GCpf - GCpi)



WIND98 v3-02

Wind Load Design per ASCE 7-98

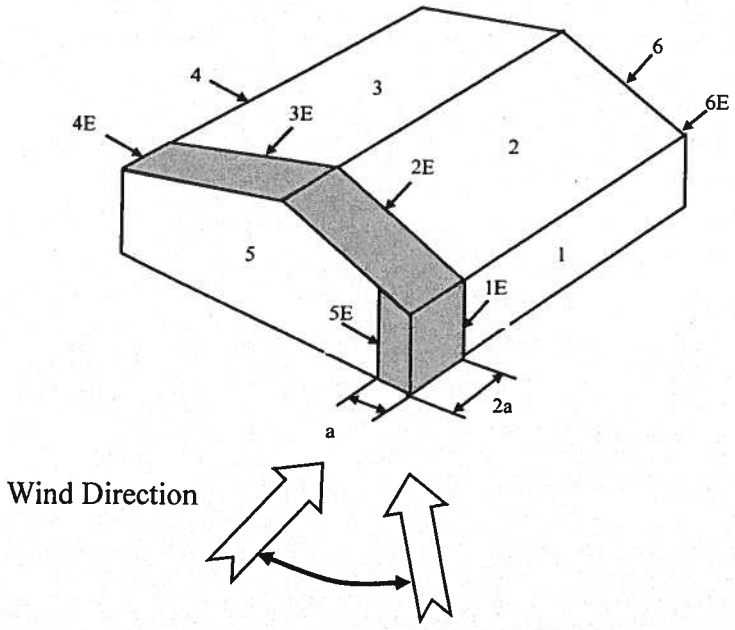
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.64
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	16.88

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	16.88	-10.64	-4.56
2	-0.69	0.18	-0.18	16.88	-14.69	-8.61
3	-0.37	0.18	-0.18	16.88	-9.29	-3.21
4	-0.45	0.18	-0.18	16.88	-10.64	-4.56
5	0.40	0.18	-0.18	16.88	3.71	9.79
6	-0.29	0.18	-0.18	16.88	-7.93	-1.86
1E	-0.48	0.18	-0.18	16.88	-11.14	-5.06
2E	-1.07	0.18	-0.18	16.88	-21.10	-15.02
3E	-0.53	0.18	-0.18	16.88	-11.99	-5.91
4E	-0.48	0.18	-0.18	16.88	-11.14	-5.06
5E	0.61	0.18	-0.18	16.88	7.26	13.34
6E	-0.43	0.18	-0.18	16.88	-10.30	-4.22

* p = qh * (GCpf - GCpi)

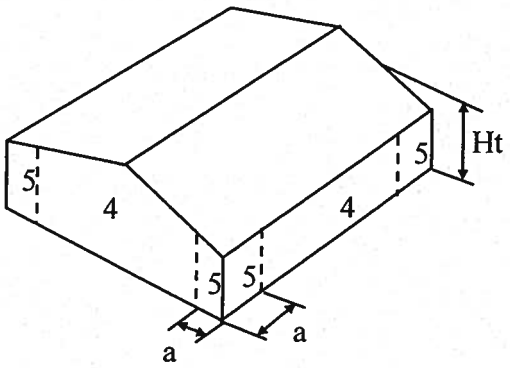


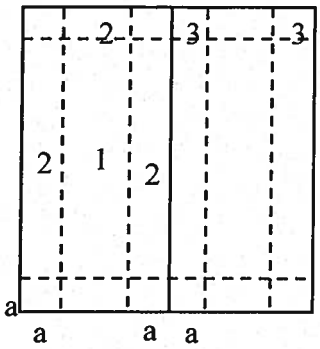
WIND98 v3-02

Wind Load Design per ASCE 7-98

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Gabled Roof

10 < Theta <= 45

a = 4.24 ==> 4.24 ft

Component	Width (ft)	Span (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
ROOF	10	1	10.00	1	0.90	-1.00	19.92	-21.77
Walls	10	1	10.00	4	1.00	-1.10	21.77	-23.61
roof edge	10	1	10.00	2	0.90	-1.20	19.92	-25.46
Wall edge	10	1	10.00	5	1.00	-1.40	21.77	-29.15
Roof overhang	10	1	10.00	2H	0.90	-2.00	16.60	-36.89

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

 www.mecaconsulting.com

Shearwall Design - N/S Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)		8	Roof Dia	14.42	
Vertical Roof height		14.00	Length of Building	63	
2nd Floor height	0		Width of Building	31.6	
1st Floor height	8				

Wind Pressure per ASCE 7- Normal to surface Case A

Windward Roof - Surface 2	6.58	psf	Wall - Leeward	12.49 psf
Leeward Roof - Surface 3	-4.22	psf	Wall - Surface 4	-3.21 psf
			Total Wall	15.7 psf

Horizontal loads from wind perpendicular to ridge (N / S)

Roof Pressure (interior)

Windward Roof Horz.(psf)	3.65
Leeward Roof Horz.(psf)	-2.34
Total	5.99
Tributary area (roof)	661.50
Roof shear values	**3962.89**

Wall Pressure - 2nd Floor

Sum. of wind. & lee. (psf)	15.7
Tributary area to each Shearwall (sf)	0.00
Wall shear values to each shearwall	**0.00**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	15.7
Tributary area to each Shearwall (sf)	317.50
Wall shear values to each shearwall	**4984.75**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**8947.64**

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1	
Full wall length	32.625	46.17	
Shearwall #1 length	26.33	37.2	
Shearwall #2 length	0	0	
Wall height ratio (h/b)	2004 Florida B	0.22	
Rigidities of shearwalls	#VALUE!	15.26	
Lateral load on shearwall column (lbs) based on rigidity	**#VALUE!**	**#VALUE!**	
Percent Full-Height Sheathing Shearwall #1	80.70%	80.57%	
Shear capacity adjustment	0.9	0.9	
Shearwall rating (plf) w/ 1.4 increase for wind	483	483	
Design Shear Capacity	**11445.65**	**16170.84**	
Stress Ratio	#VALUE!	#VALUE!	
uplift at shear ends	**#VALUE!**	**#VALUE!**	
shear and uplift between holddown, v and u	**#VALUE!**	**#VALUE!**	

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

Shearwall Design - E/W Direction

Rigid Diaphragm Analysis

Wind load acting on building

General Data

Roof Pitch (x:12)	8	Roof Dia	14.42
Vertical Roof height	14.00	Length of Building	63
2nd Floor height	0	Width of Building	31.6
1st Floor height	8		

Wind Pressure per ASCE 7- Normal to surface Case B

Windward Wall - Surface 5	9.79	psf
Leeward Wall - Surface 6	-1.86	psf
Total Wall	11.65	

Horizontal loads from parallel to ridge (E/W)

Number of Shearwall columns = 2

Roof Pressure (interior)	
Windward Roof Horz.(psf)	9.79
Leeward Roof Horz.(psf)	-1.86
Total	11.65
Tributary area (roof) to each shearwall (sf)	322.00
Roof shear values to each shearwall	**3751.30**

Wall Pressure - 1st Floor

Sum. of wind. & lee. (psf)	11.65
Tributary area to each Shearwall (sf)	294.00
Wall shear values to each shearwall	**3425.10**

Total shear to top of 2nd floor (lb) per wall (actual)	**0.00**
Total shear to top of 1st floor (lb) per wall (actual)	**7176.40**

1st Floor shearwall (ft)

Number of shearwall segments in each column	1	1
Shearwall #1 length	17.5	14
Shearwall #2 length	11.75	0
Wall height ratio (h/b)	0.46	0.57
Rigidities of shearwalls	6.82	5.26
Lateral load on shearwall column (lbs) based on rigidity	**4050.50**	**3125.90**
Percent Full-Height Sheathing Shearwall #1	0.58	0.74
Shear capacity adjustment	0.75	0.85
Shearwall rating (plf) w/ 1.4 increase for wind	483	483
Design Shear Capacity	**2004 Florida**	**5747.70**
Stress Ratio	#VALUE!	0.54
uplift at shear ends	**3703.32**	**3152.16**
shear and uplift between holddown, v and u	**184.64**	**262.68**

Anchor Bolt Shear Capacity plf			
Bolt size / spacing	24"	36"	48"
1/2" dia	422.5	281.67	211.25
5/8" dia	660	440.00	330
3/4" dia	930	620.00	465

EMBER MASTER™

VENT-FREE GAS LOG HEATERS

The FMI Ember Master Gas Log Heaters offer you what ordinary gas logs and wood cannot...the warmth, cleanliness and economy of vent-free space heating combined with the beauty and convenience of gas logs. FMI's unique, clean burning design and precision positioned logs permit vent-free operation with the fireplace damper closed. With no venting required, there is no heat loss up the chimney. Plus they require no electricity so you always have heat - even in the event of power outage.

A Model To Fit Every Need

FMI's Ember Master Gas Log Heaters are available in two log sizes: 18" and 24", each model is available in propane and natural gas. Choose from two types of controls: *Variable Manually Controlled Models* - Allow you to choose any heat setting and flame height desired by simply turning the control knob. *Remote Control "Ready" Models* - Offer you heat at the push of a button or the flip of a switch. FMI's Remote Control Ready Heaters offer you the choice of four types of remote controls as well as the ability to operate the log heater with a variable manual control. Optional remote accessories include: the hand held thermostatic remote control, the hand held on/off remote control, the wall mount thermostat control and the wall mount on/off switch.

FMI Vent-Free Gas Logs install in any masonry, manufactured solid fuel burning fireplace, or AGA or UL certified Vent-Free Universal Firebox. All (manual variable) log heaters are also listed to the ANSI Z21.60 Vented Log Decorative Standard.

Safety Features Built Into Every Gas Log Heater

FMI Vent-Free Gas Log Heaters are designed certified by the American Gas Association (ANSI Z21.11.2) and meet or exceed all regulations and safety performance standards for vent-free gas heaters. Additionally, these log heaters perform well within nationally recognized guidelines for indoor air quality.

The dual-purpose safety pilot system protects against oxygen depletion and any interruption in the fuel supply. If either occurs, it shuts off the gas flow to the burner turning the heater off. An internal pressure regulator controls fluctuations in your gas pressure. These features ensure clean and reliable heat without the worry and inconvenience of vented gas logs or burning wood.

Log Size	No. of Logs	Part/Model Number		Btu Output	Shipping Wt.
		Natural	Propane		
Variable Manually Controlled Models					
18"	5	01984/VFN18MV	01985/VFP18MV	16,000 to 26,000	26 Lbs.
24"	6	01986/VFN24MV	01987/VFP24MV	20,000 to 33,000	28 Lbs.
Remote Control Ready/Millivolt Models					
18"	5	01988/VFN18R	01989/VFP18R	16,000 to 26,000	32 Lbs.
24"	6	01990/VFN24R	01991/VFP24R	20,000 to 33,000	34 Lbs.
Remote Control Accessories (Must be purchased Separately)					
Part/Model Number		Description			
01994/FHRCT		Receiver and Hand Held Thermostat Remote Control Kit			3 Lbs.
01995/FHRC		Receiver and Hand Held On/Off Remote Control Kit			3 Lbs.
01996/FWMT1		Wall Mount Thermostat Control Kit			1Lb.
01997/FWMS2		Wall Mount - On/Off Switch Kit			1Lb.
Accessories					
01244/HDABK		Hood - Flat Black Enamel - Adjustable 28" to 49"			5 Lbs.
01245/HDABR		Hood - Polished Solid Brass - Adjustable 28" to 49"			5 Lbs.

Log Sizing Requirements

Minimum Firebox Size					Gas Connection
Log Size	Height	Depth	Front Width	Rear Width at 14" Depth	
18"	17"	14"	20"	20"	1/2" NPT
24"	17"	14"	26"	21"	1/2" NPT

A fireplace hood accessory may be required to deflect heat away from the mantel shelf. Fits fireplace openings from 28" to 49" wide. Installs easily with glass doors or to the lintel of the fireplace opening. Refer to the Ember Master Vent Free Gas Log Owner's Manual for more details.

IMPORTANT

- Installation must be done by qualified service persons
- Read Owners Manual before using.
- Check local codes and ordinances for permitted uses.
- Approved for manufactured (mobile) home installation. Not for use in recreational vehicles.
- We reserve the right to amend product specifications without notice.
- Use with adequate air (ventilation) only. Humidifies while it heats.
- Provides water vapor in the area heated. Refer to Owner's Manual for specifics.
- Operating heater at very high elevations could cause nuisance outage. Product cannot be converted between fuel types.
- The only warranty we offer is our standard warranty.
- Please read the warranty for any limitations or disclaimers.
- All products carry a one year warranty

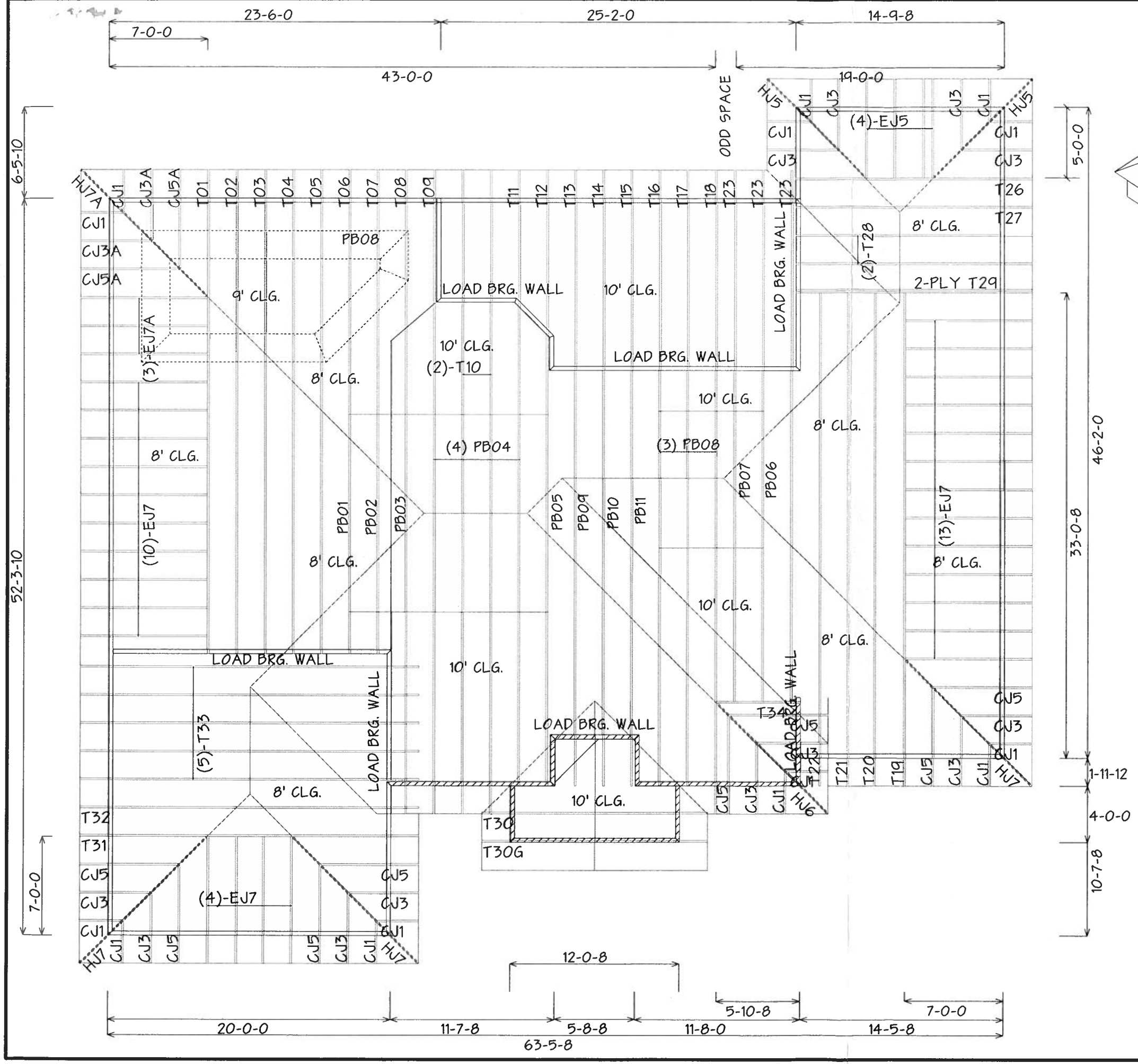
Made in U.S.A

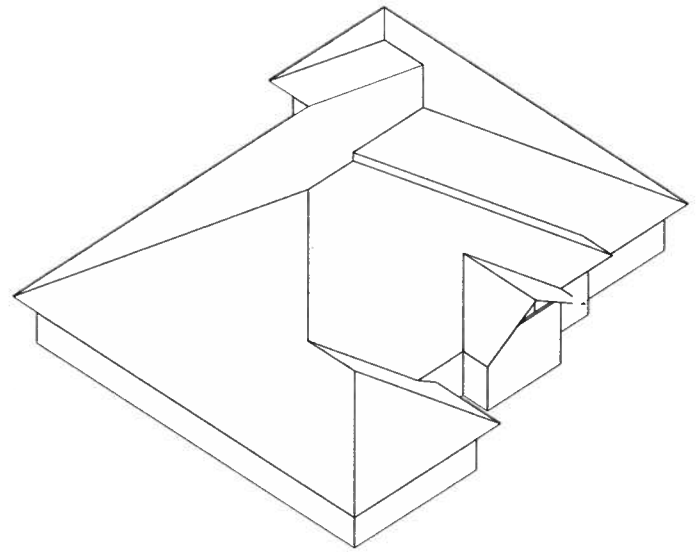
gama

Fireplace Manufacturers Incorporated

For More Information Call 1-800-888-2050

P.N.#55486 5/99 Printed in U.S.A.
http://www.fmionline.com





BEARING HEIGHT SCHEDULE

8'-0"

10'-0"

ROOF
PITCH
8/12
OVERHANG
EAVE 1'-6"
GABLE 1'-0"

HANGER SCHEDULE
(8) HUS26
(148) P.B. PLATES

NOTES:

1.) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.

2.) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V10 FOR ALTERNATE BRACING REQUIREMENTS.

3.) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.

4.) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.

5.) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.

6.) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.

7.) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.

8.) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: LIPSCOMB EAGLE

LEGAL ADDRESS: COLUMBIA CO.

MODEL: ARBOR GREEN

REVISION:

SCALE: NTS

DATE: 9-7-05

DRAWN BY: JRD

JOB #: L130832