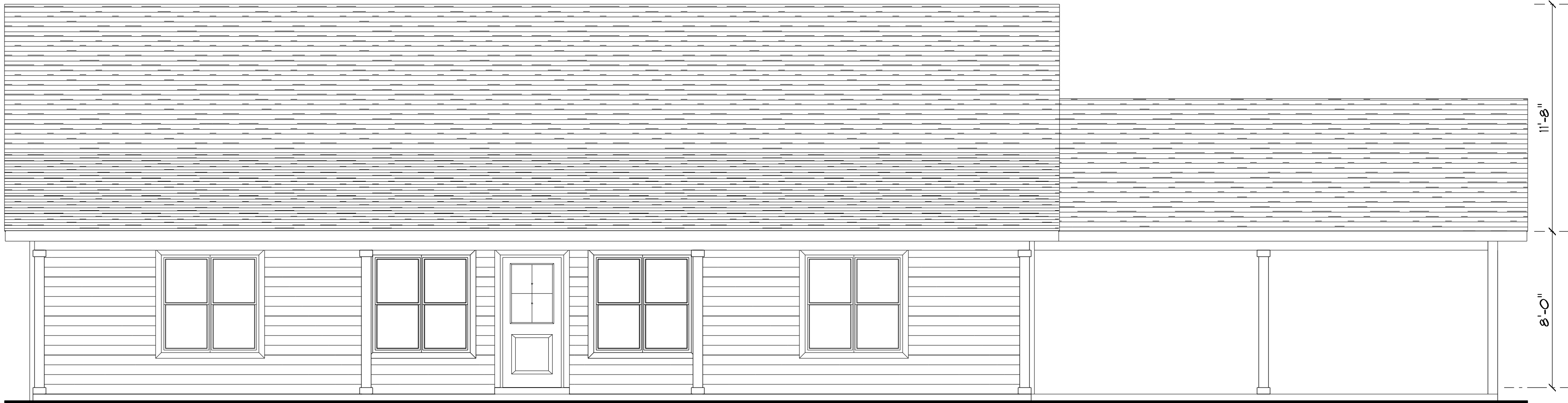
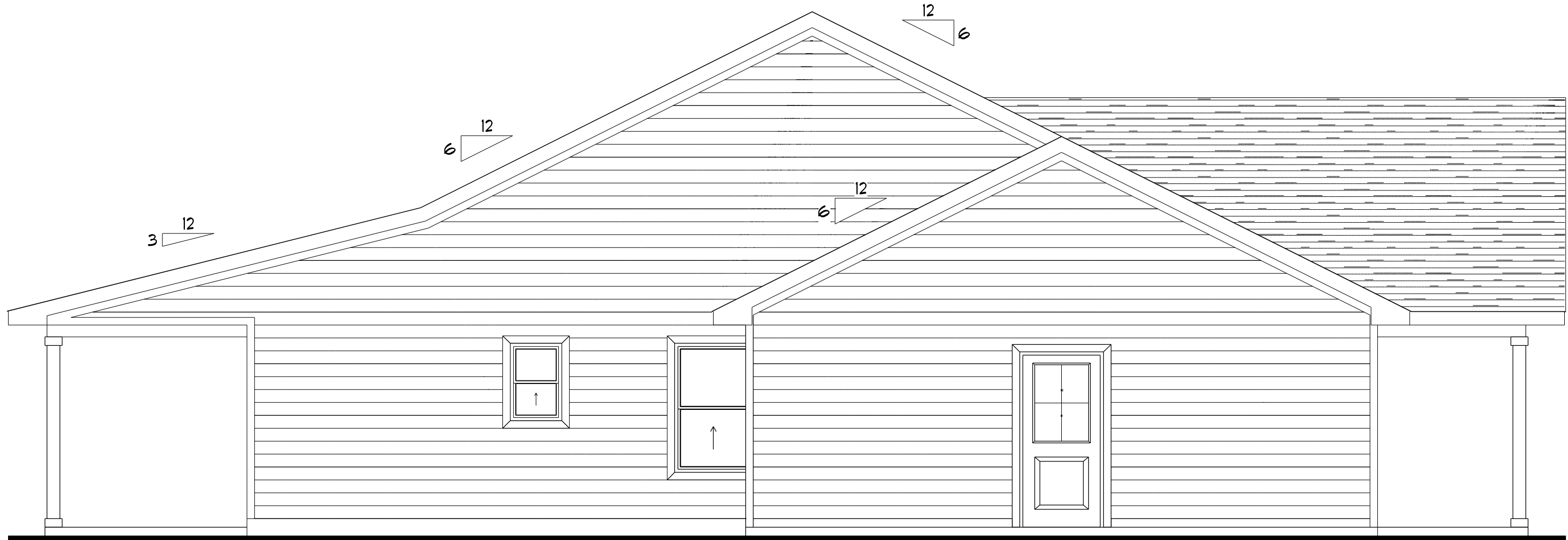




FRONT ELEVATION

SCALE: 1/4" = 1'-0"

ROOF VENTILATION:
R906.2 Minimum vent area.
The minimum net free ventilating area shall be 1/150 of the area of the vented space.
Exception: The minimum net free ventilation area shall be 1/300 of the vented space provided one or more of the following conditions are met:
1. In Climate Zones 6, 7 and 8, a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.
2. At least 40 percent and not more than 50 percent of the required ventilating area is provided by ventilators located in the upper portion of the attic or rafter space. Upper ventilators shall be located no more than 3 feet below the ridge or highest point of the space, measured vertically, with the balance of the required ventilation provided by soffit or cornice vents. Where the location of wall or roof framing members conflicts with the installation of upper ventilators, installation more than 3 feet below the ridge or highest point of the space shall be permitted.



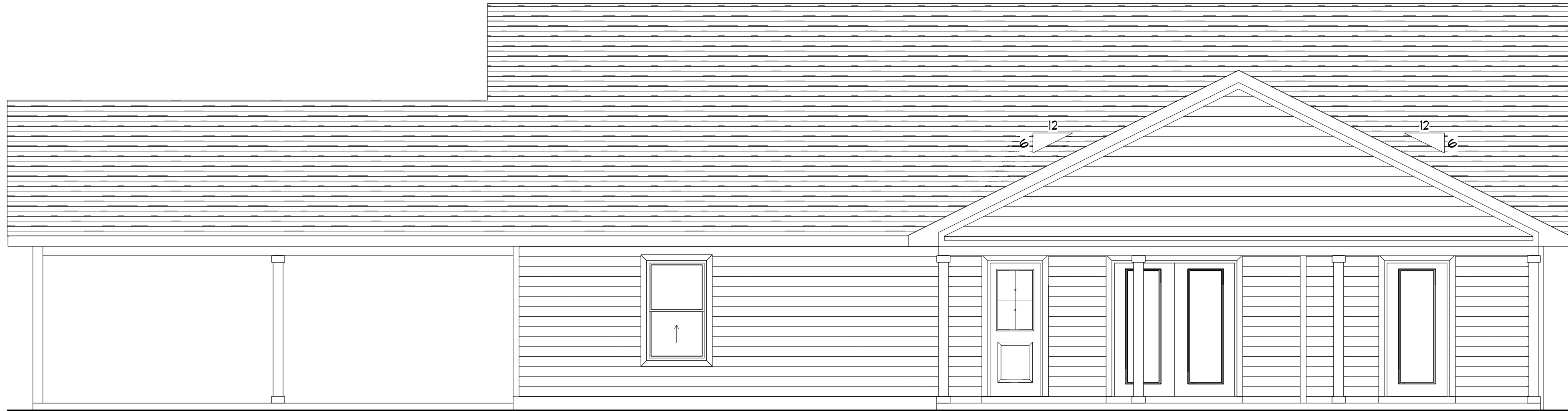
RIGHT ELEVATION

SCALE: 1/4" = 1'-0"



LEFT ELEVATION

SCALE: 1/4" = 1'-0"



REAR ELEVATION

SCALE: 1/4" = 1'-0"

Parnell Res.

PROJECT ADDRESS:
Altamira County, FL

FL PE 53915

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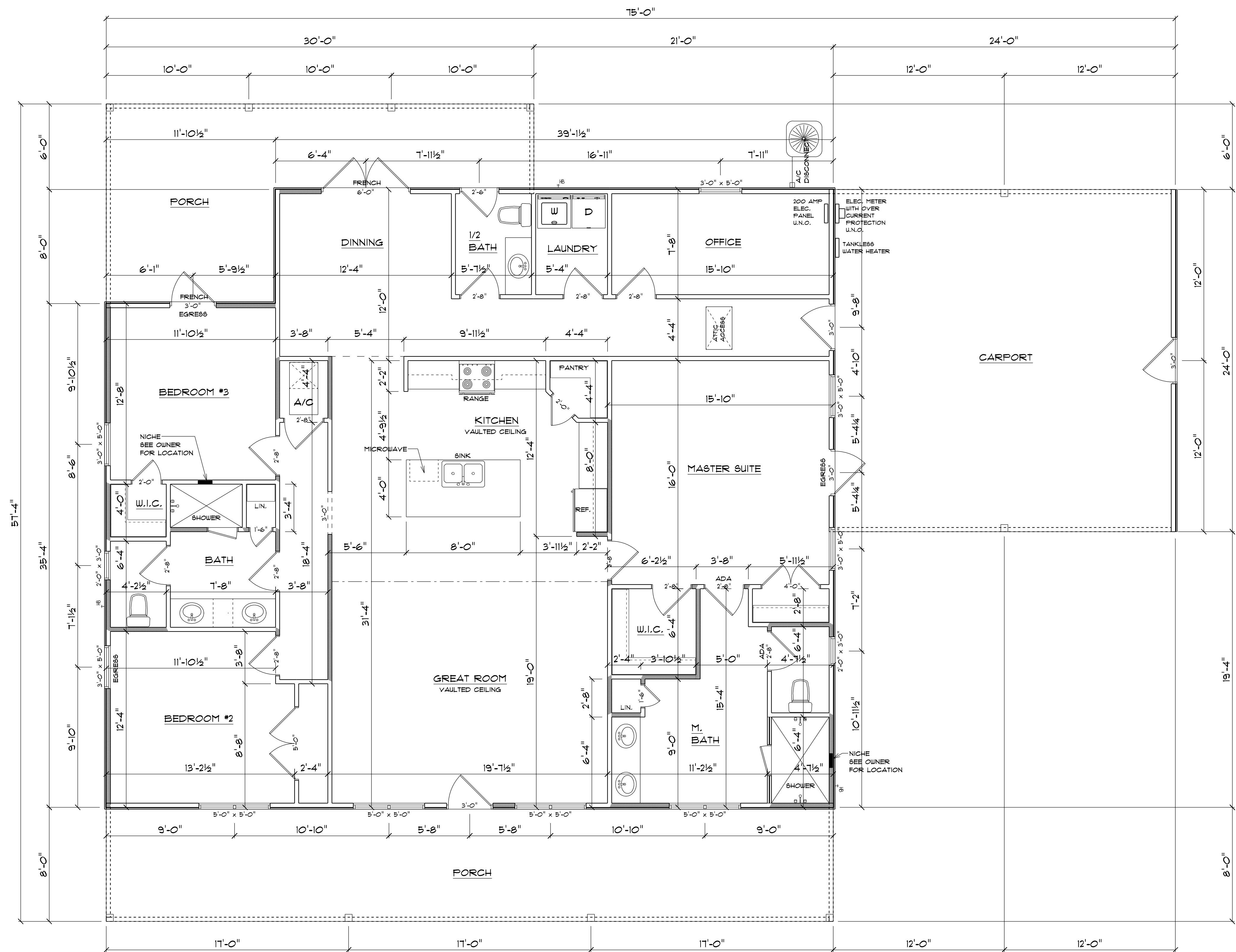
CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

Mark Disosway P.E.
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Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
240678

1
OF 6 SHEETS



SCALE: 1/4" = 1'-0"

SCALE: 1/4" = 1'-0"

ALL CEILING HEIGHTS TO BE 8'-0" UNLESS NOTED OTHERWISE

AREA SCHEDULE	
NAME	AREA
Living	2115 sq ft.
Total Living	2115 sq ft.
Front Porch	408 sq ft.
Rear Porch	275 sq ft.
Carport	576 sq ft.
Total Under Roof	3343 sq ft.

Parnell Res.

PROJECT ADDRESS:
Alachua County, FL

een digitally signed

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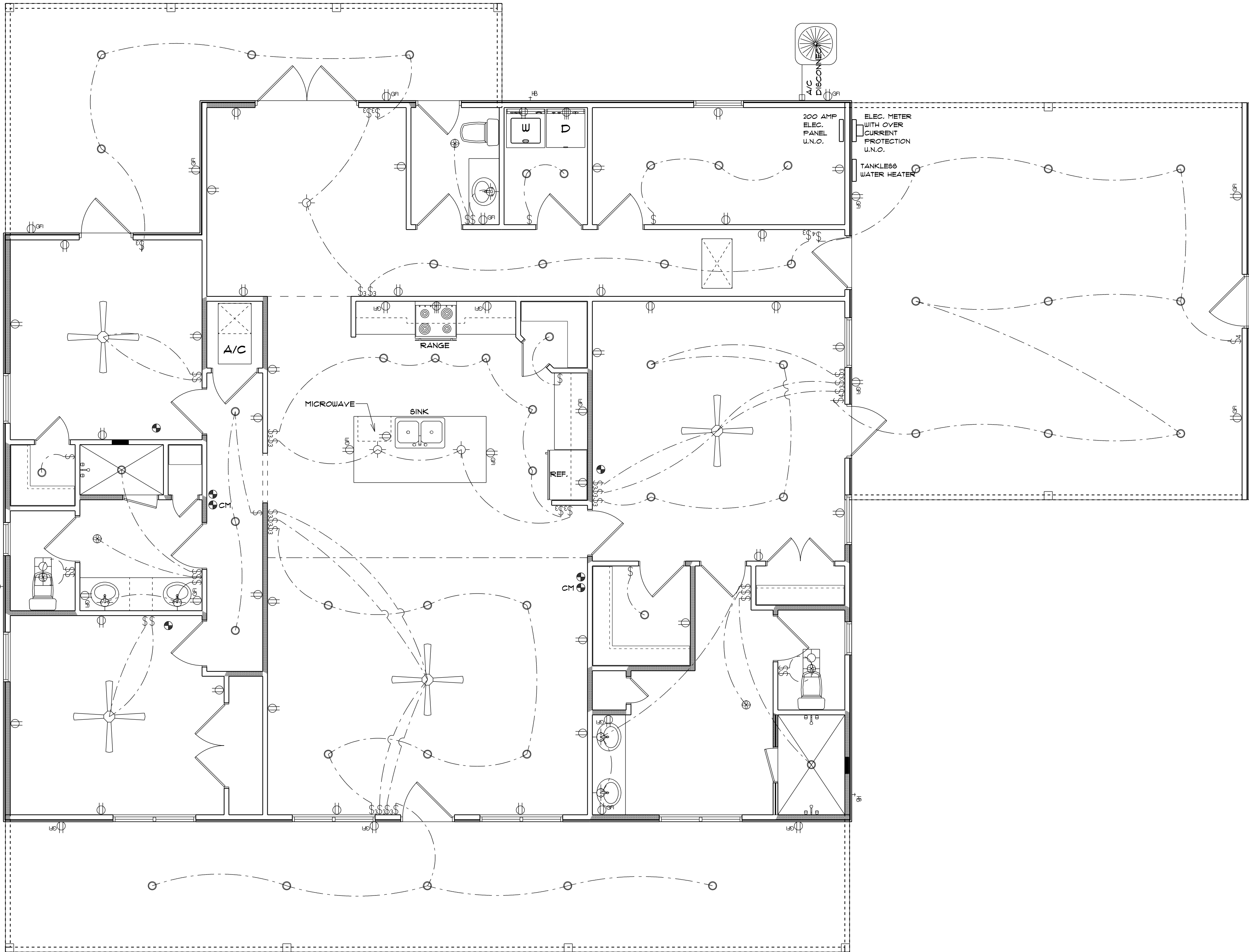
JOB NUMBER:
240678

2

OF 6 SHEETS

ELECTRICAL PLAN NOTES:	
E - 1	WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
E - 2	CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
E - 3	ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
E - 4	ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
E - 5	TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
E - 6	ELECTRICAL CONTR' SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
E - 7	ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
E - 8	ALL 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS INSTALLED IN DWELLING UNIT FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DEN'S, BEDROOMS, SUN ROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS - OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT CIRCUIT INTERRUPTER, COMBINATION-TYPE INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
E - 9	ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION.
E - 10	A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING.
E - 11	SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL.
E - 12	CARBON MONOXIDE ALARMS SHALL BE REQUIRED WITHIN 10' OF ALL ROOMS FOR SLEEPING PURPOSES IN BUILDINGS HAVING A FOSSIL FUEL BURNING HEATER OR APPLIANCE, A FIREPLACE, OR ATTACHED GARAGE.
E - 13	ALL OUTLETS LOCATED IN RESIDENTIAL TO BE TAMPER-RESISTANT PER NEC.
E - 14	A MINIMUM OF 70% OF PERMANENTLY INSTALLED LAMPS OR LIGHTING FIXTURES SHALL BE HIGH EFFICACY FBC EC SEC. R404.1

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
	RECESSED CAN LIGHT
	BATH EXHAUST FAN WITH LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR
	WALL SWITCH
	3 WAY WALL SWITCH
	4 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	PHONE JACK
	TELEVISION JACK
	GARAGE DOOR OPENER
	CARBON MONOXIDE ALARM



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

Parnell Res.

PROJECT ADDRESS:
Alachua County, FL

FL PE 53915

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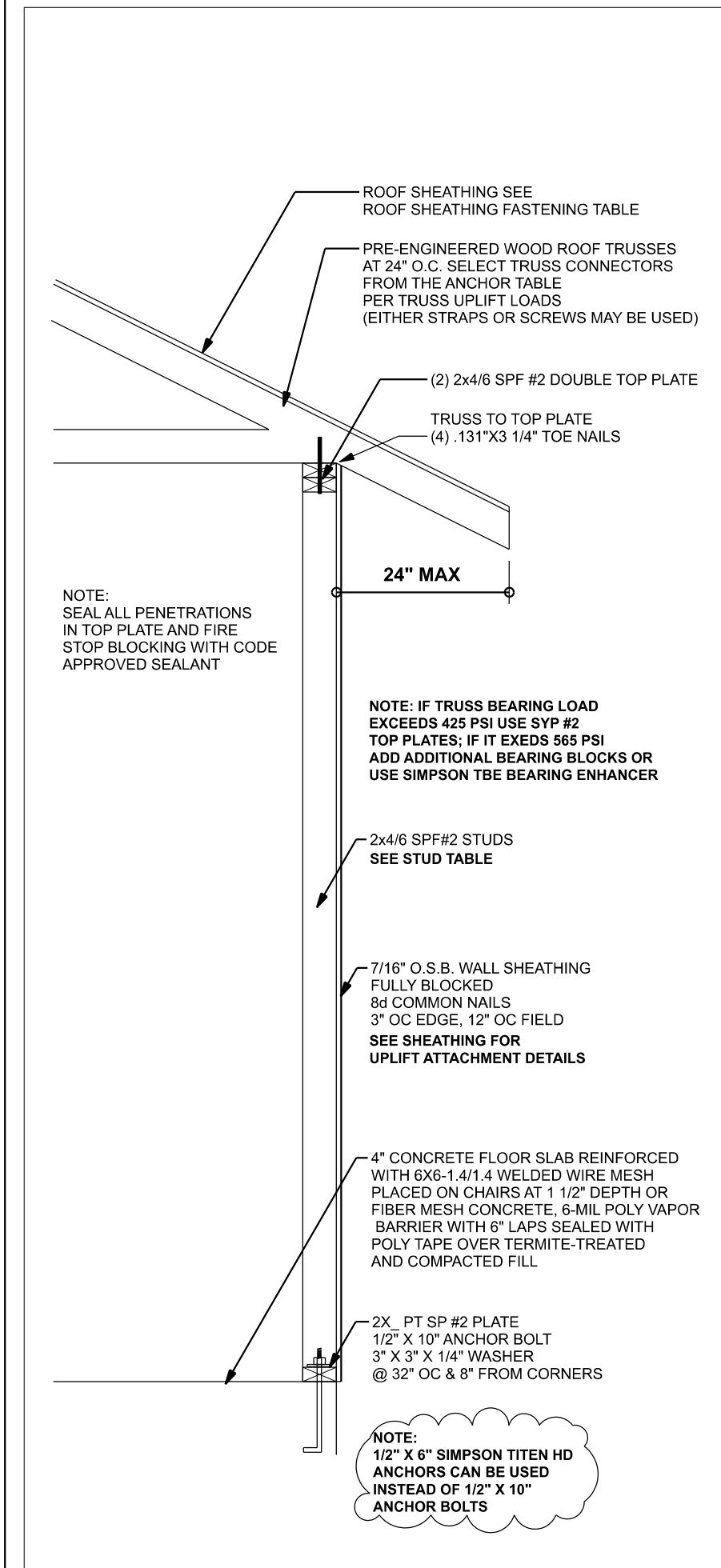
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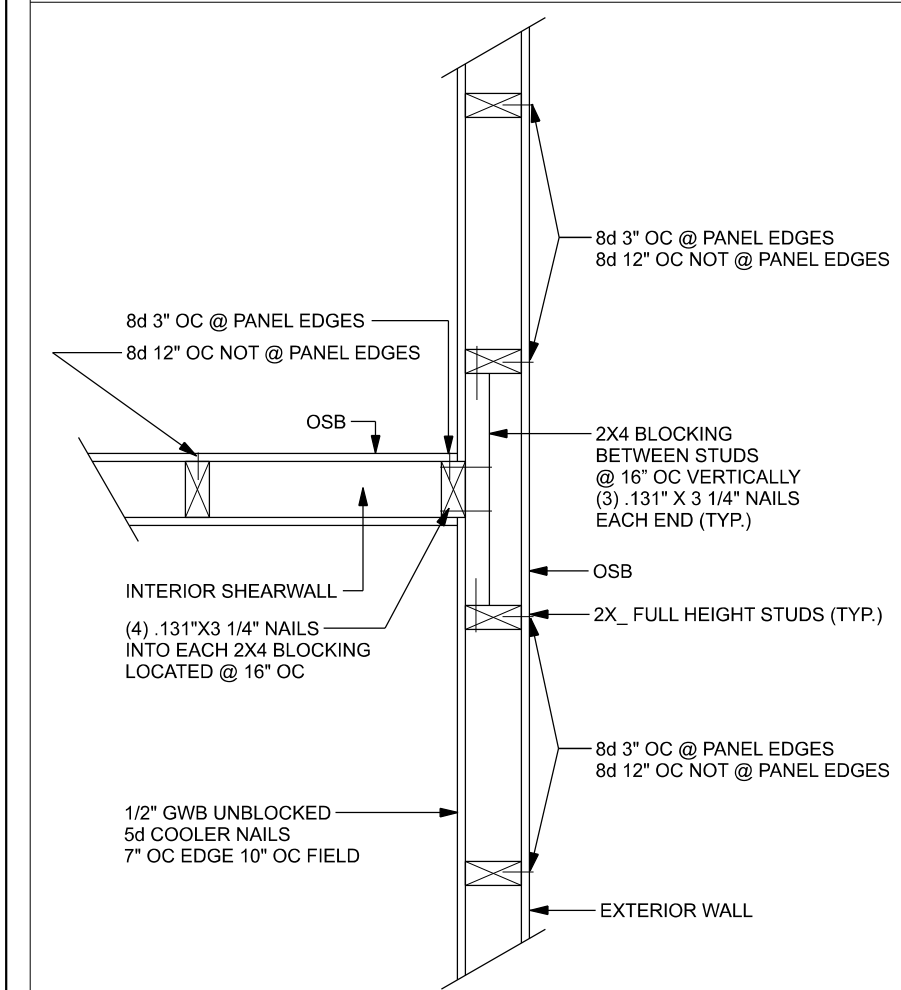
JOB NUMBER:
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3

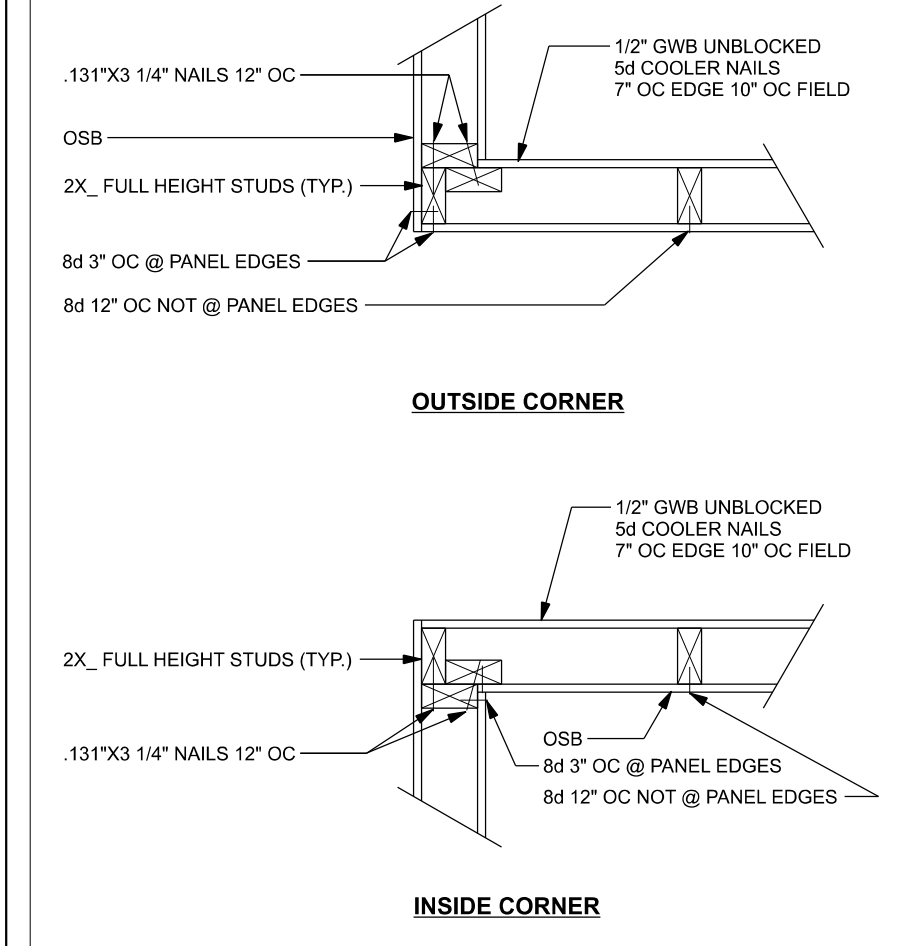
OF 6 SHEETS



ONE STORY WALL SECTION
SCALE: 3/4" = 1'-0"

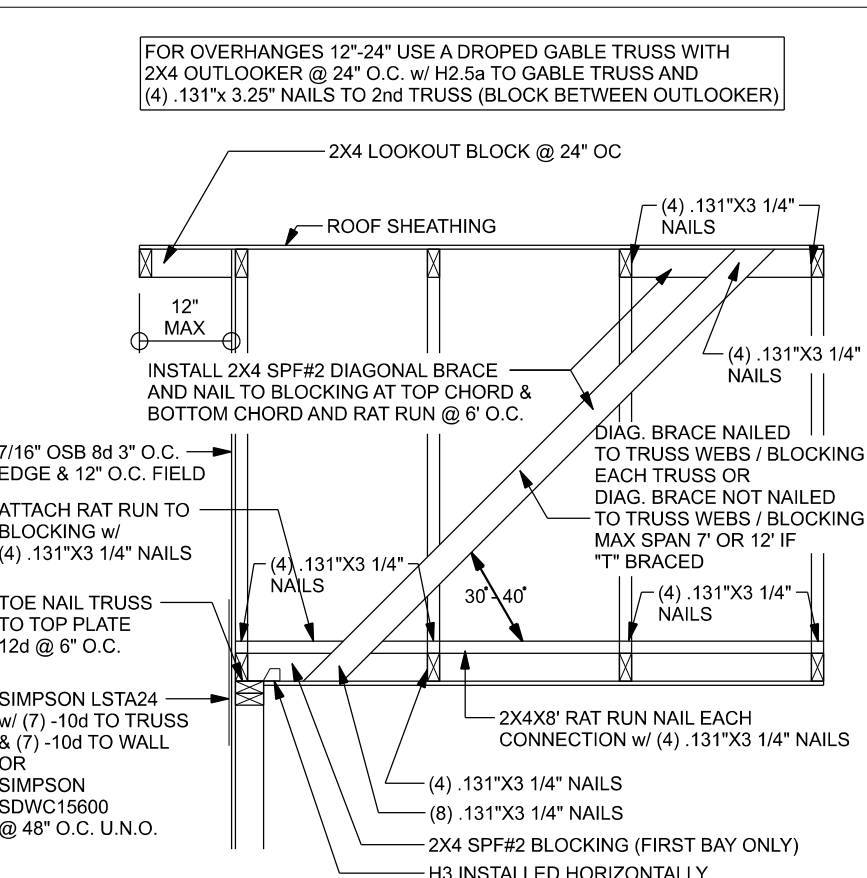


(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



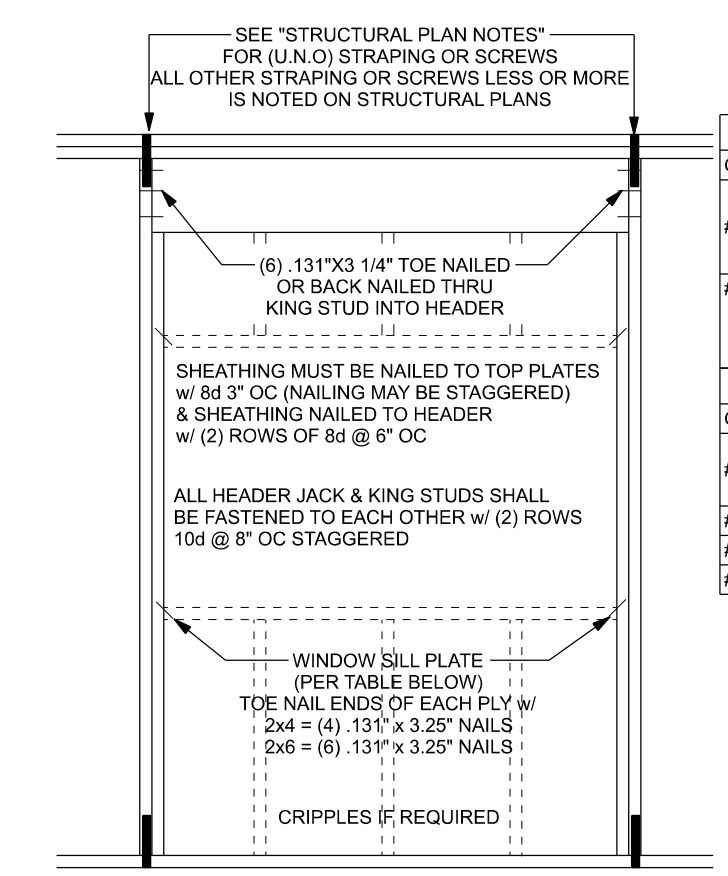
(TYP.) CORNER FRAMING
WOOD FRAME

ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
120 mph Exp. D	13/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. C	13/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. D	13/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RRSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
140 mph Exp. C	13/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	13/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc	
150 mph Exp. C	13/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc	
150 mph Exp. D	13/32"	ASTM F1667 RRSR-03 (2 1/2" x 0.131") or ASTM F1667 RRSR-04 (3" x 0.120")	6" oc	6" oc	

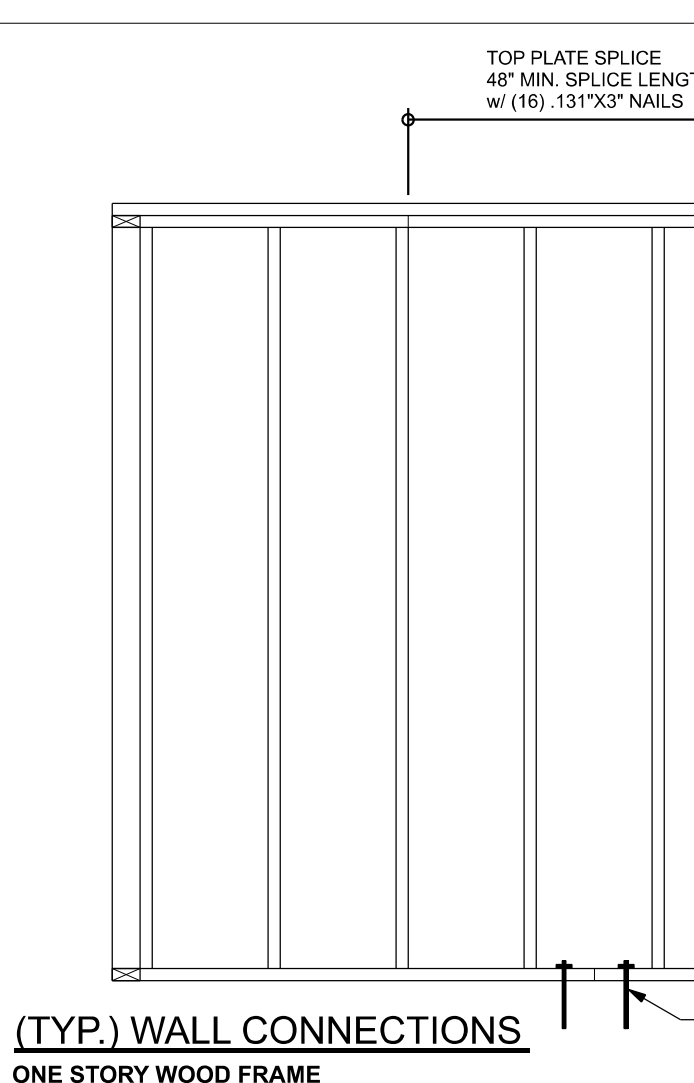


SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 130 MPH, EXP. C, ENCLOSED

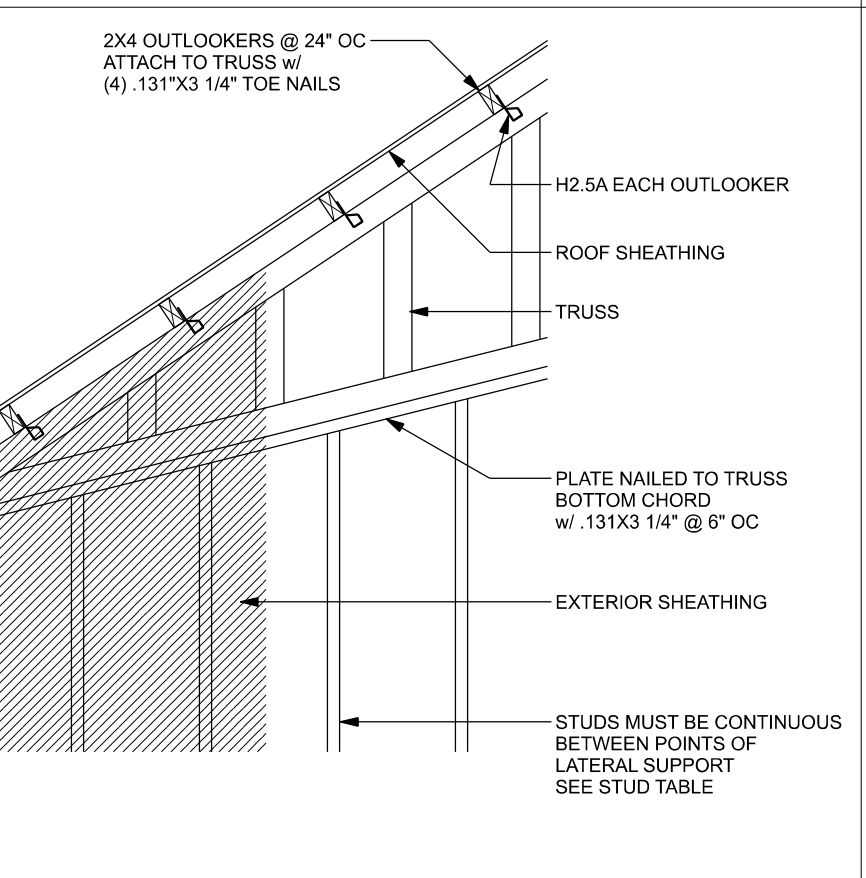
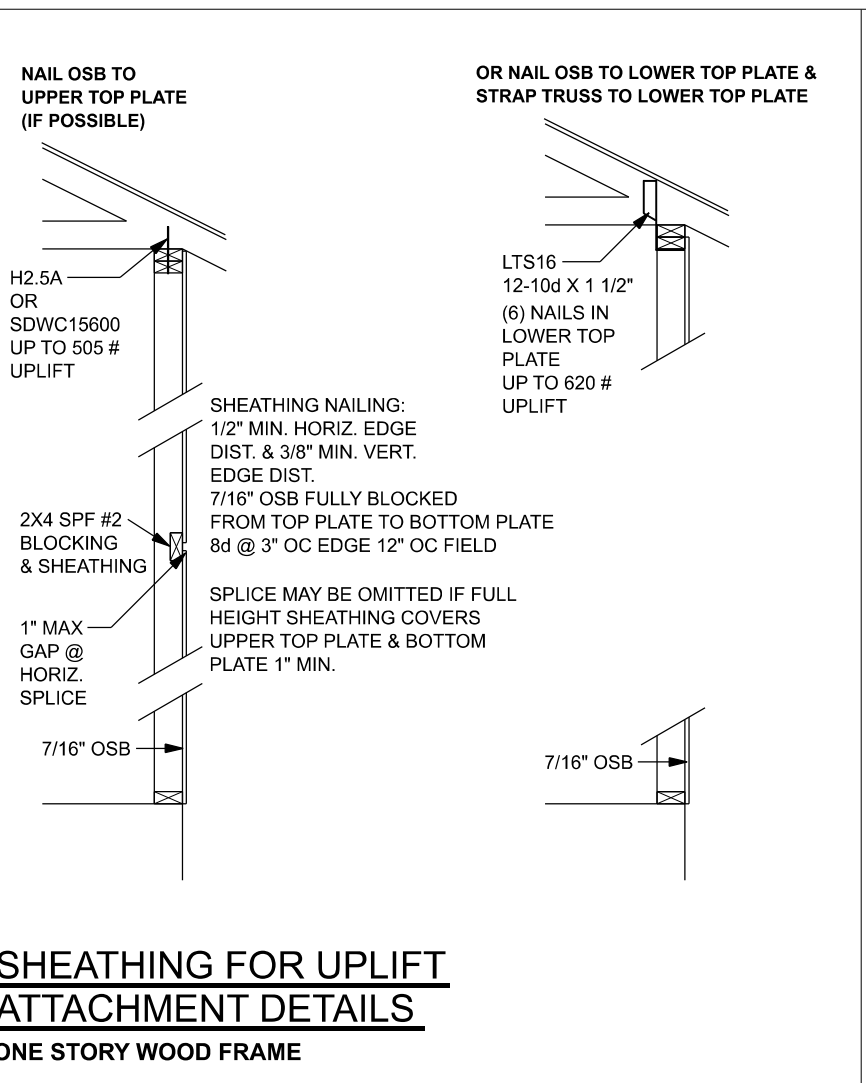
(TYP.) GABLE BRACING DETAIL
WOOD FRAME



TYPICAL HEADER STRAPPING OR SCREWS DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

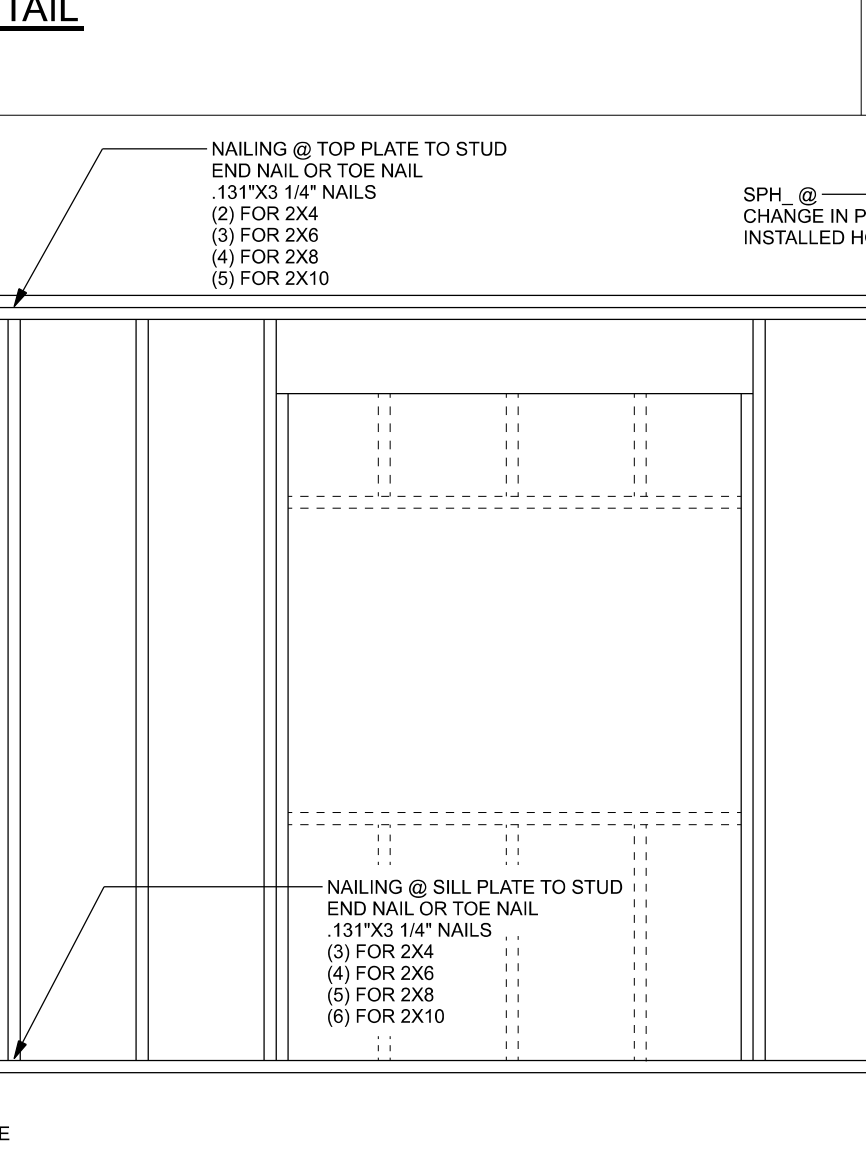


(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

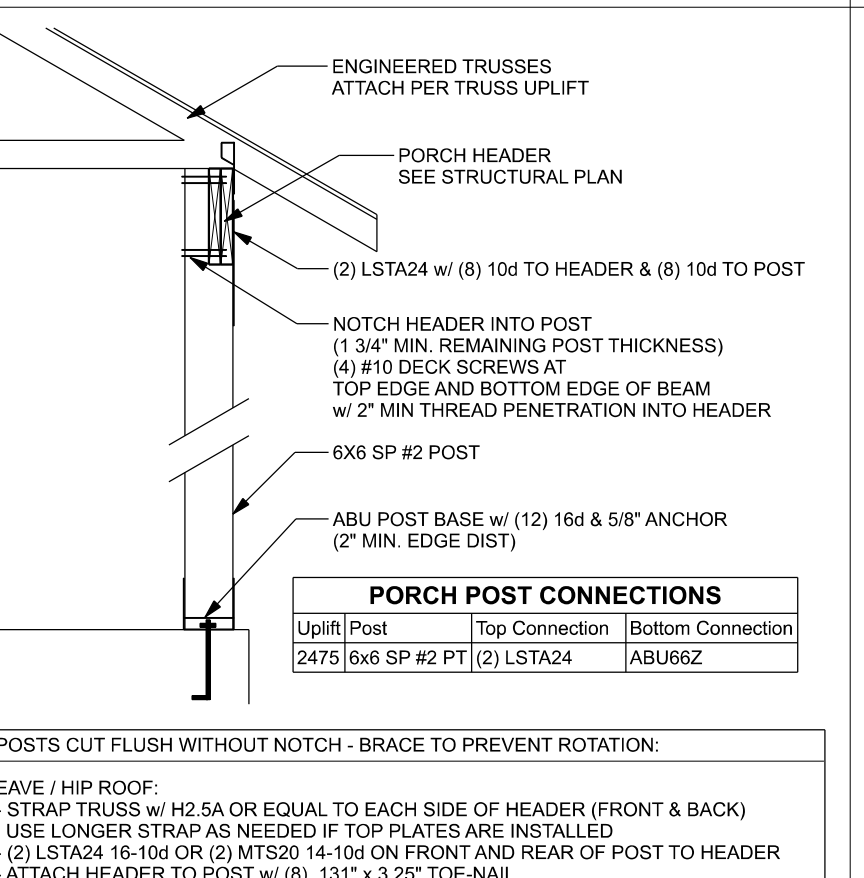
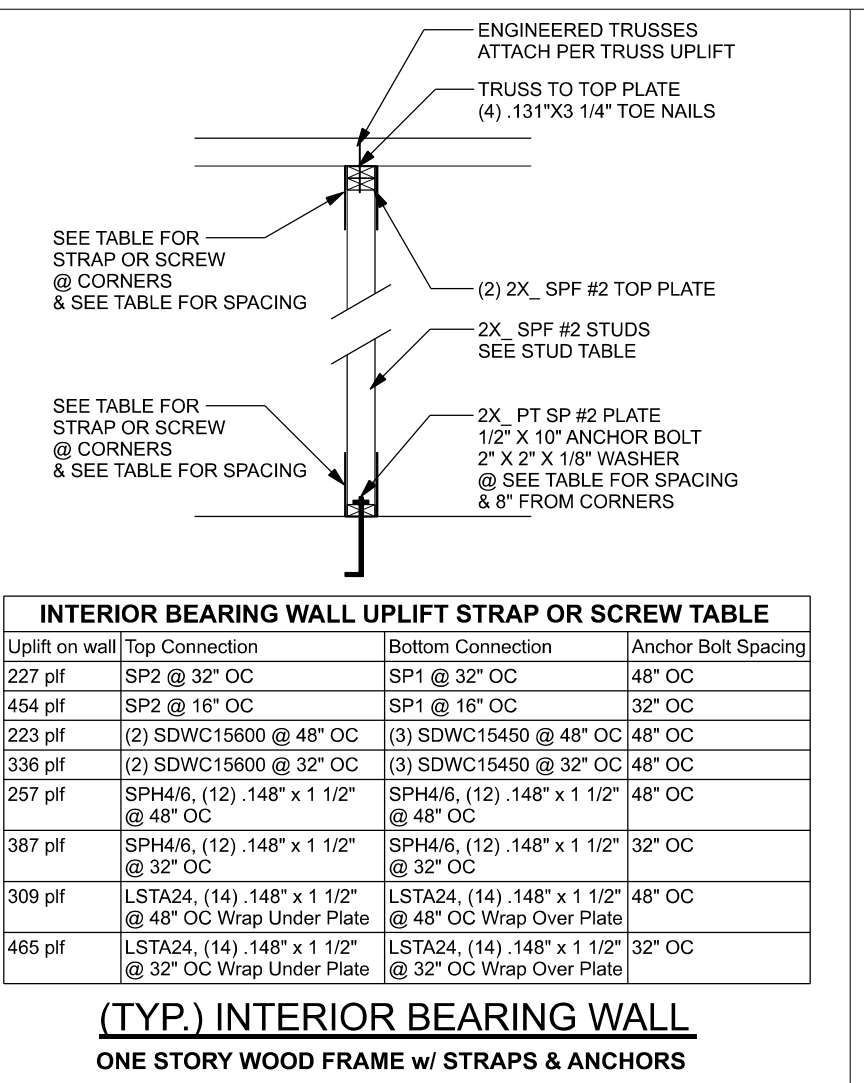
HEADER SCREWS TABLE			
Option	Uplift	Top Connection	Bottom Connection
#1	< 510	Attach king stud to top plate w/ (1) Simpson SDWC15600	Attach king stud to bottom plate w/ (2) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#2	< 895	Attach king stud to top plate w/ (2) Simpson SDWC15450	Attach king stud to bottom plate w/ (3) Simpson SDWC15450 1/2" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations

HEADER STRAP TABLE			
Option	Uplift	Top Connection	Bottom Connection
#3	< 1235	LSTA24, (14), 148" x 1 1/2" wrap over plate	LSTA24, (14), 148" x 1 1/2" wrap under plate 12" x 10" Anchor bolt w/ 3" x 3" x 1/4" washer must be located within 6" of king stud @ all door locations
#4	< 1455	MSTA24, 18-, 148"x1 1/2" header to jacks DT122	
#5	< 1800	(2) MSTA24, 18-, 148"x1 1/2" header to jacks DT122	
#6	< 2910	(2) MSTA24, 18-, 148"x1 1/2" header to jacks HTT14	

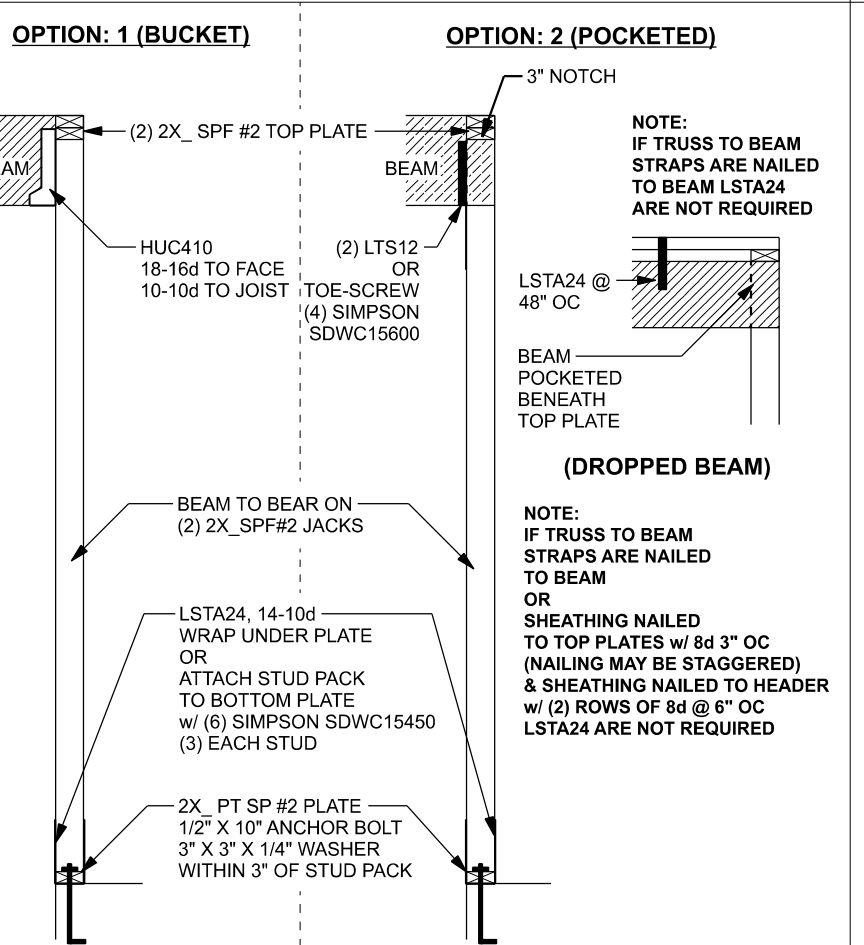
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT			
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-2.8B
(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"
			11'-3"



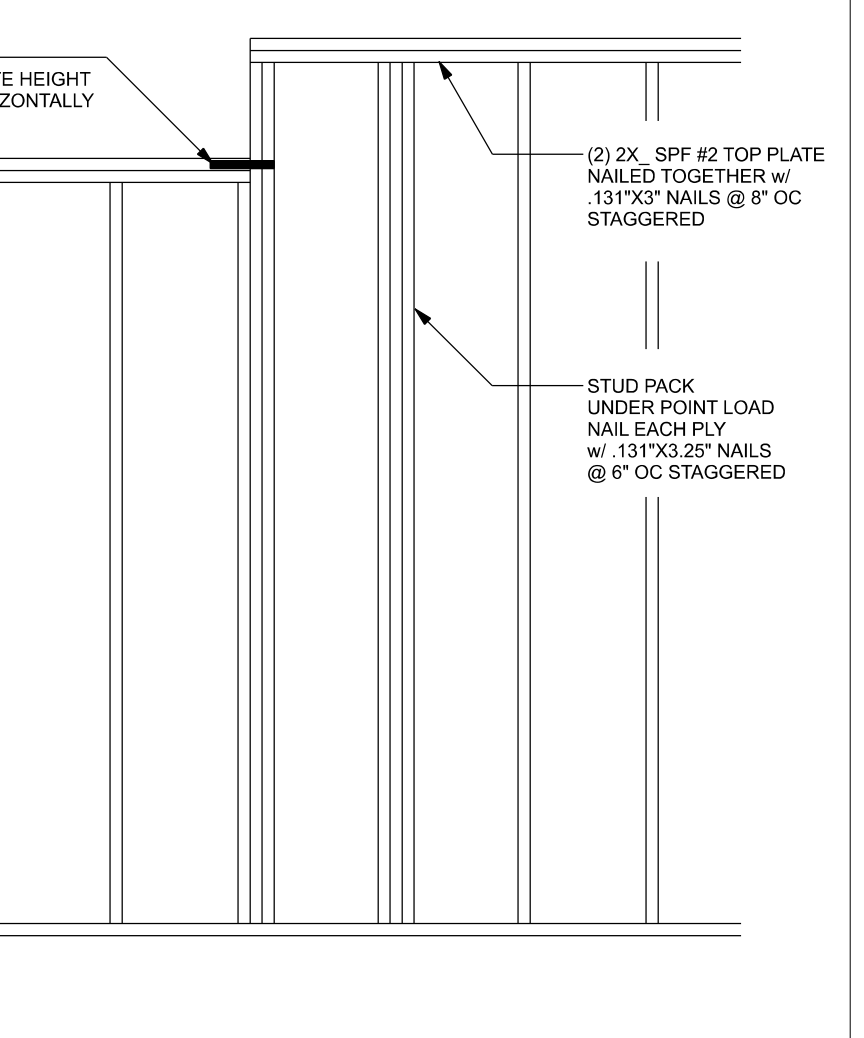
(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME



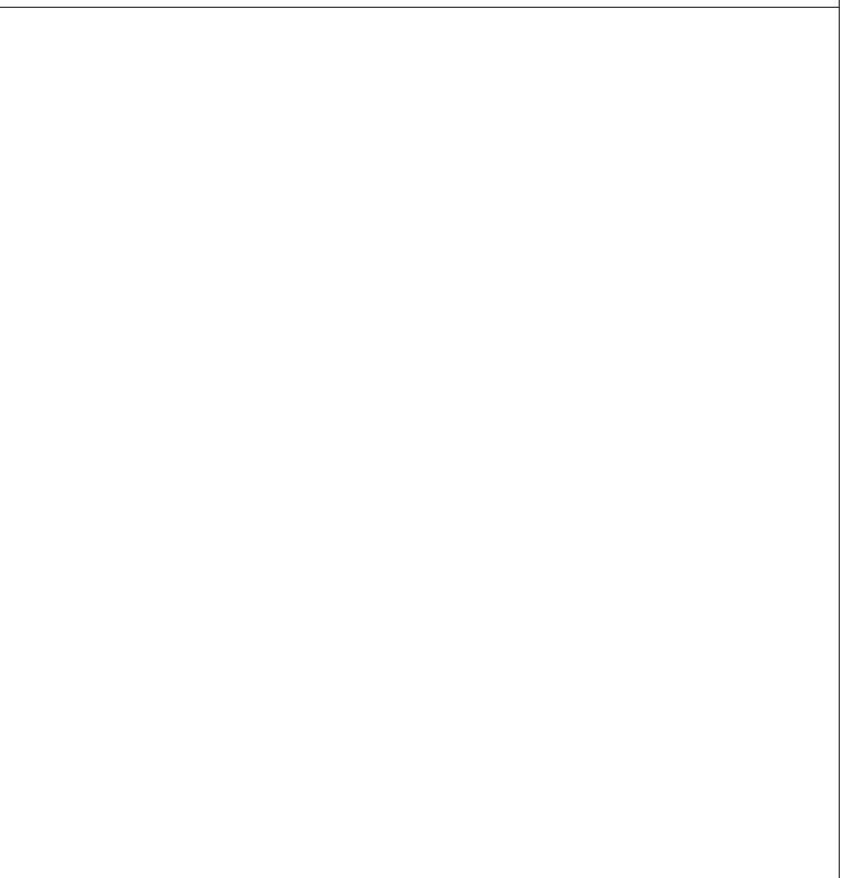
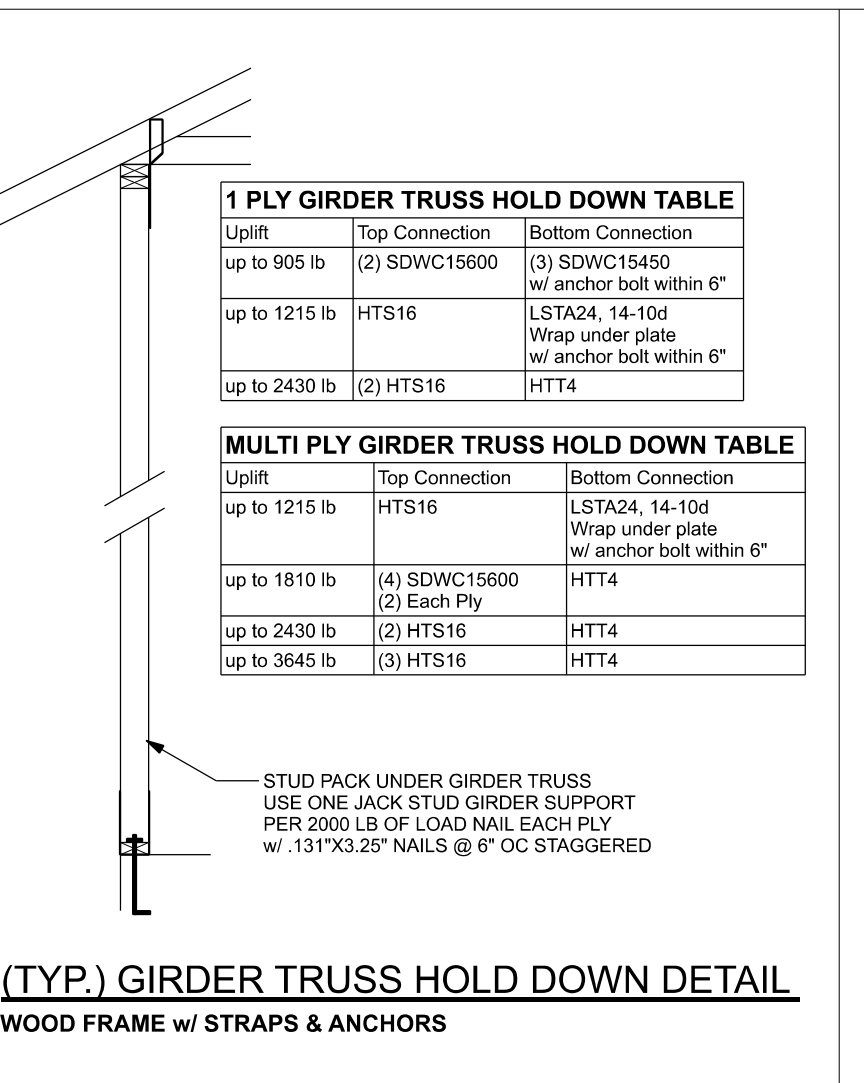
(TYP.) PORCH POST
ONE STORY WOOD



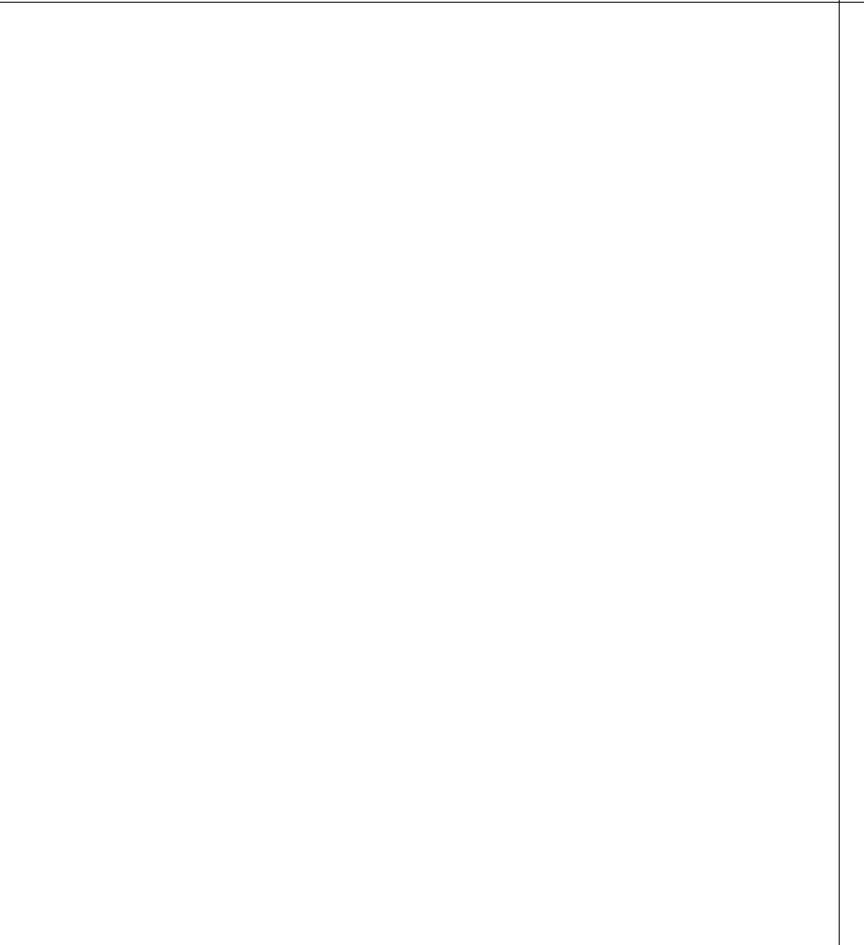
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



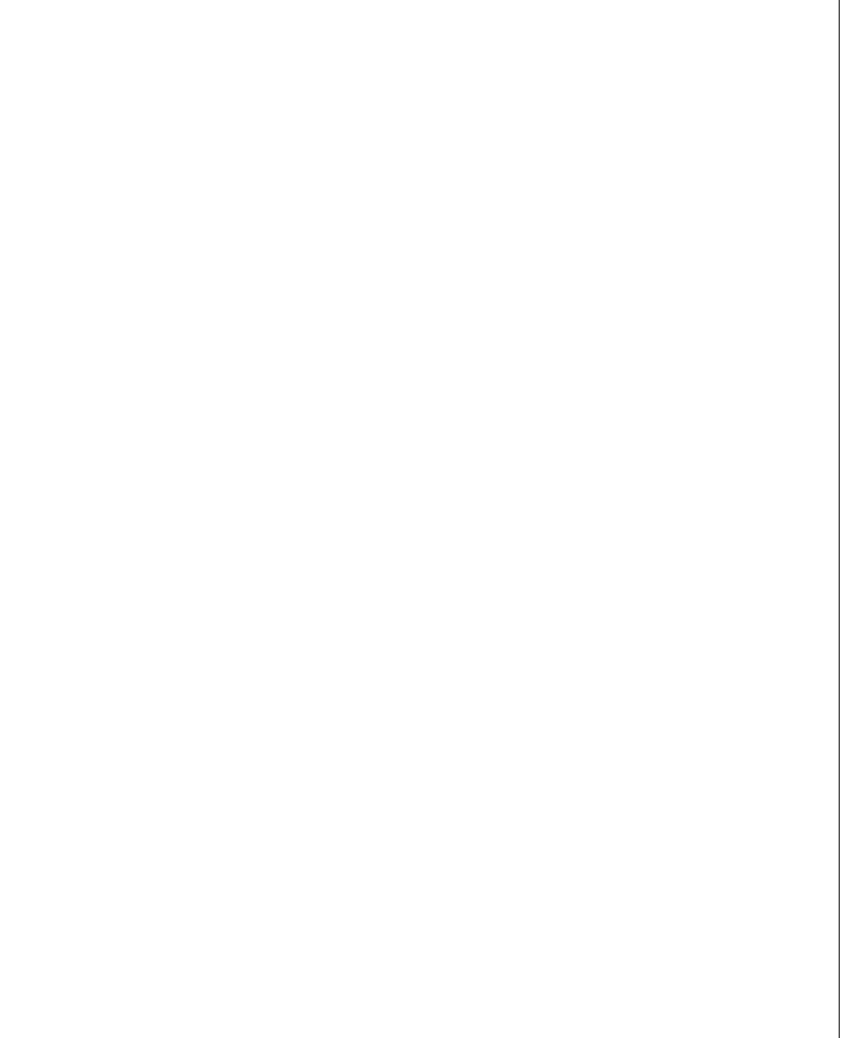
(TYP.) BEAM TO WALL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

CONNECTOR TABLE			
Uplift	Top Connection	Bottom Connection	
up to 905 lb	(2) SDWC15600	(3) SDWC15450 w/ anchor bolt within 6"	
up to 1215 lb	HTS16	LSTA24, 14-10d w/ anchor bolt within 6"	
up to 2430 lb	(2) HTS16	HTT4	

MULTI PLY GIRDER TRUSS HOLD DOWN TABLE			
Uplift	Top Connection	Bottom Connection	
up to 1215 lb	HTS16	LSTA24, 14-10d w/ anchor bolt within 6"	
up to 1810 lb	(4) SDWC15600 (2) Each Ply	HTT4	
up to 2430 lb	(2) HTS16	HTT4	
up to 3645 lb	(3) HTS16	HTT4	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:			
Uplift	Top Connection	Bottom Connection	
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT		
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT		
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT		
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT		

GRADE & SPECIES TABLE			
	SP	#2	Fd E
2x8	SP	#2	925 1.4
2x10	SP	#2	800 1.4
2x12	SP	#2	750 1.4
GLB	2x4V3 SP		2600 1.9
LVL	TIMBERSTRAND		1700 1.7
LVL	MICROLAM		2950 2.0
PSL	PARALAM		2900 2.0

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

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(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR WIND LOAD TRUSS RECTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" w/ #4 x W14, FB = 88KSI, WELDED WIRE REINFORCEMENT FABRIC (W14) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C1116. SUPPLIER TO PROVIDE ASTM C1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT WIND CORING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVAL. THE CONTROL JOINTS ARE NOT INTENDING TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPACINGS 40" DB (25" FOR #5 BARS); UNO AND ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH UDO.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO DIAPHRAGM, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE, NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FIBER REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER'S DESIGN IS NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SHEETS.

(TYP.) GIRDER TRUSS HOLD DOWN DETAIL
WOOD FRAME w/ STRAPS & ANCHORS

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PROJECT ADDRESS:
Alachua County, FL

FL PE 53915

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 8th Edition Florida Building Code Residential (2023) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

DESIGN CRITERIA & LOADS:

BUILDING CODE	8TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2023)
CODE FOR DESIGN LOADS	ASCE 7-22

WINDLOADS

BASIC WIND SPEED (ASCE 7-22, SS GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT

C&C DESIGN PRESSURES SEE TABLE

FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD

SOIL BEARING CAPACITY

FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE
------------	--

COMPONENT & CLADING DESIGN PRESSURES 130 MPH (EXP C)

EFFECTIVE WIND AREA (FT ²)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(Vasd) -27.8(Vasd)	+25.6(Vasd) -34.2(Vasd)
0 - 20	+42.6(Vasd) -46.2(Vult)	+42.6(Vult) -57.0(Vult)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)

8x7 GARAGE DOOR	+22.6(Vasd) -25.5(Vasd)
16x7 GARAGE DOOR	+21.7(Vasd) -24.1(Vasd)

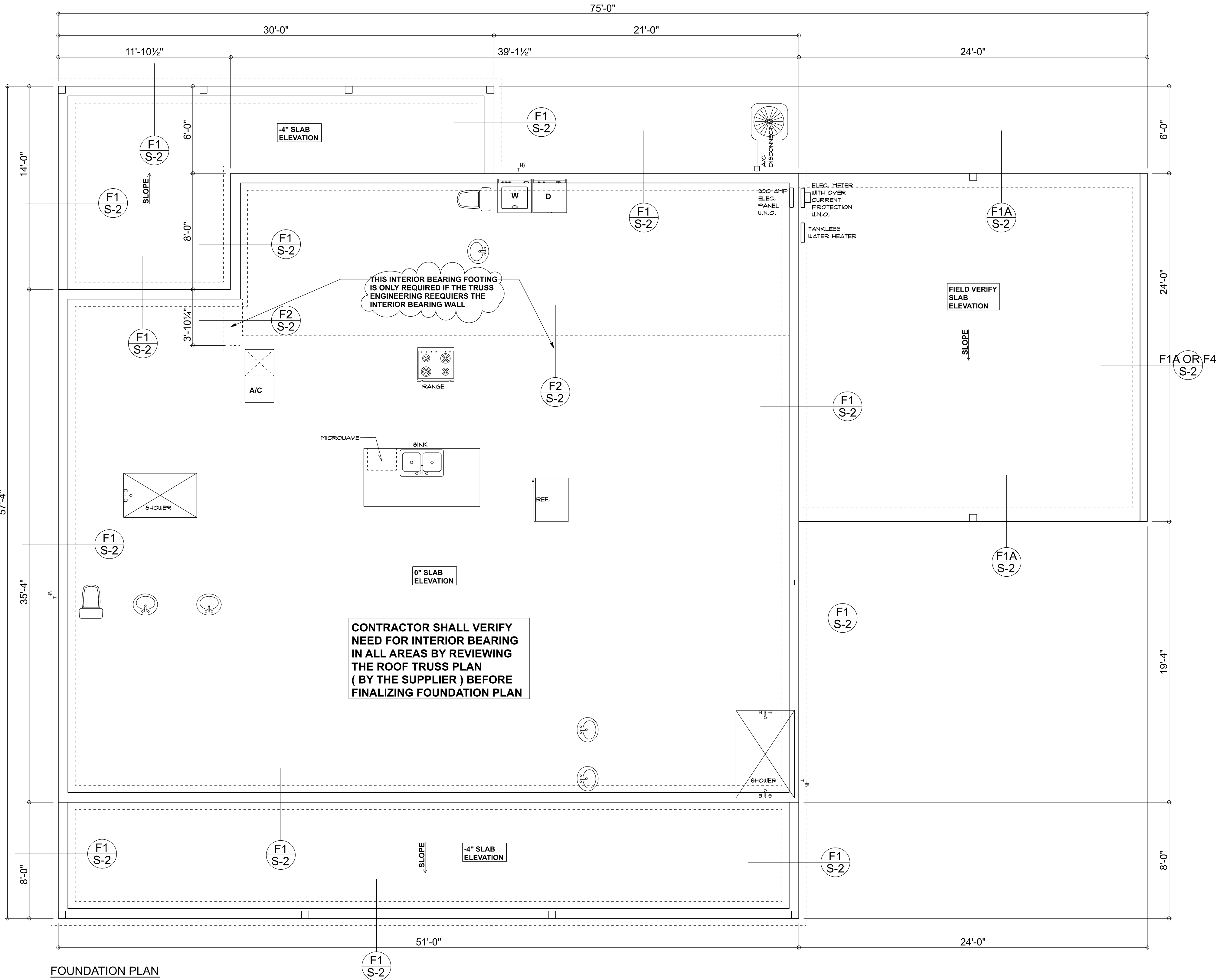
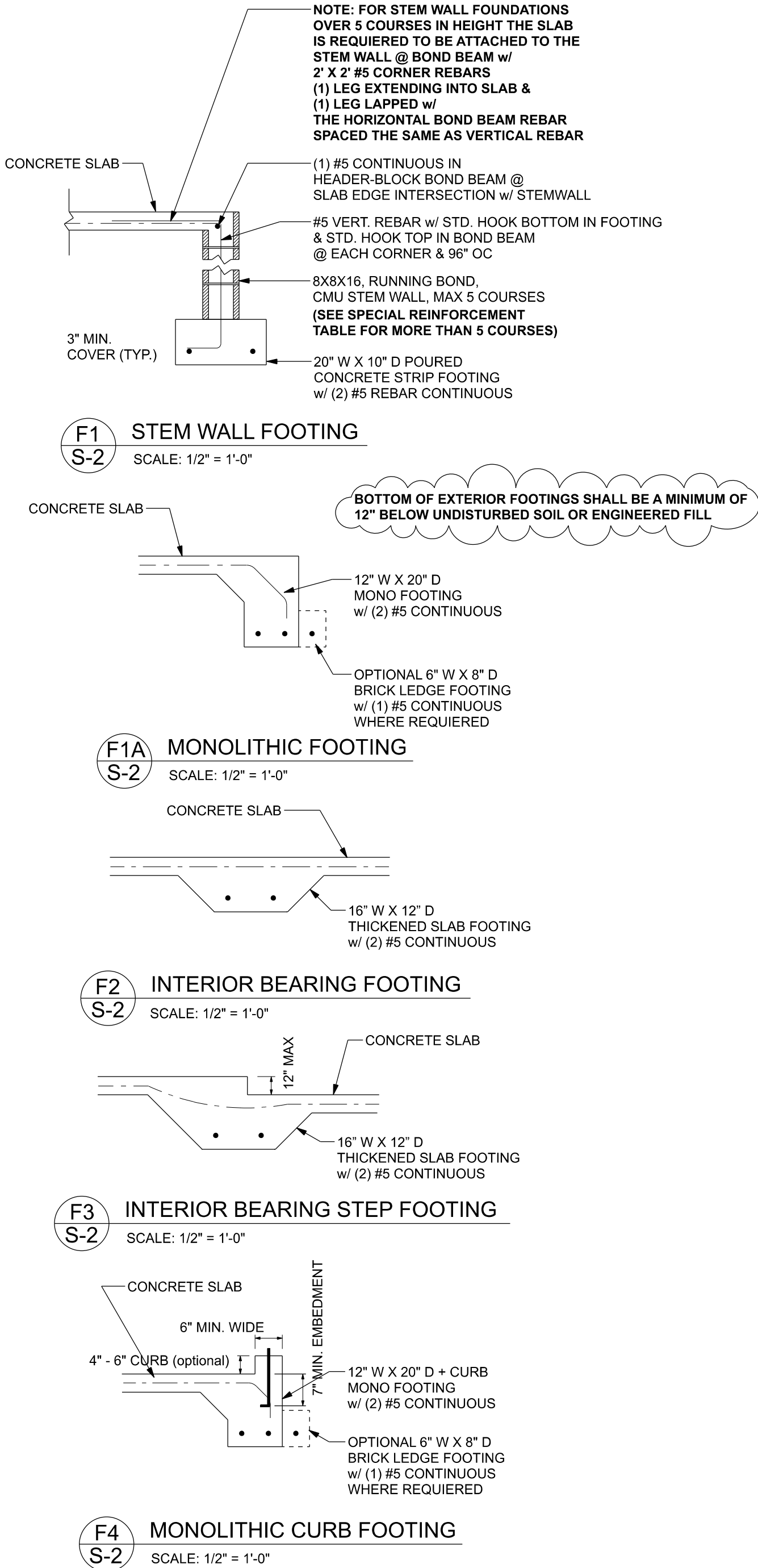
JOB NUMBER: 240678

S-1

OF 6 SHEETS

TALL STEM WALL TABLE:							
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.							
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE:	
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.	
ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM A615, Grade 60, Fy = 60 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, eheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wet feet, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE 4" CONCRETE SLAB REINFORCED w/ 6X6-14/14 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES TREATMENT METHOD CAN BE USED INSTEAD)

FOUNDATION DESIGN: Size footings per truss reactions and other loads. Locate footings per truss bearings. Interior shear walls require a thickened slab footing. For point loads > 5000 lb or repetitive loads > 3000 lb per truss provide pad footing 1' x 1' sqft, #5, 8"oc each way per 1500 lb of load.

Parnell Res.

PROJECT ADDRESS:
Alachua County, FL

FL PE 53915

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LIMITATION: This design is valid for one building, at specified location.

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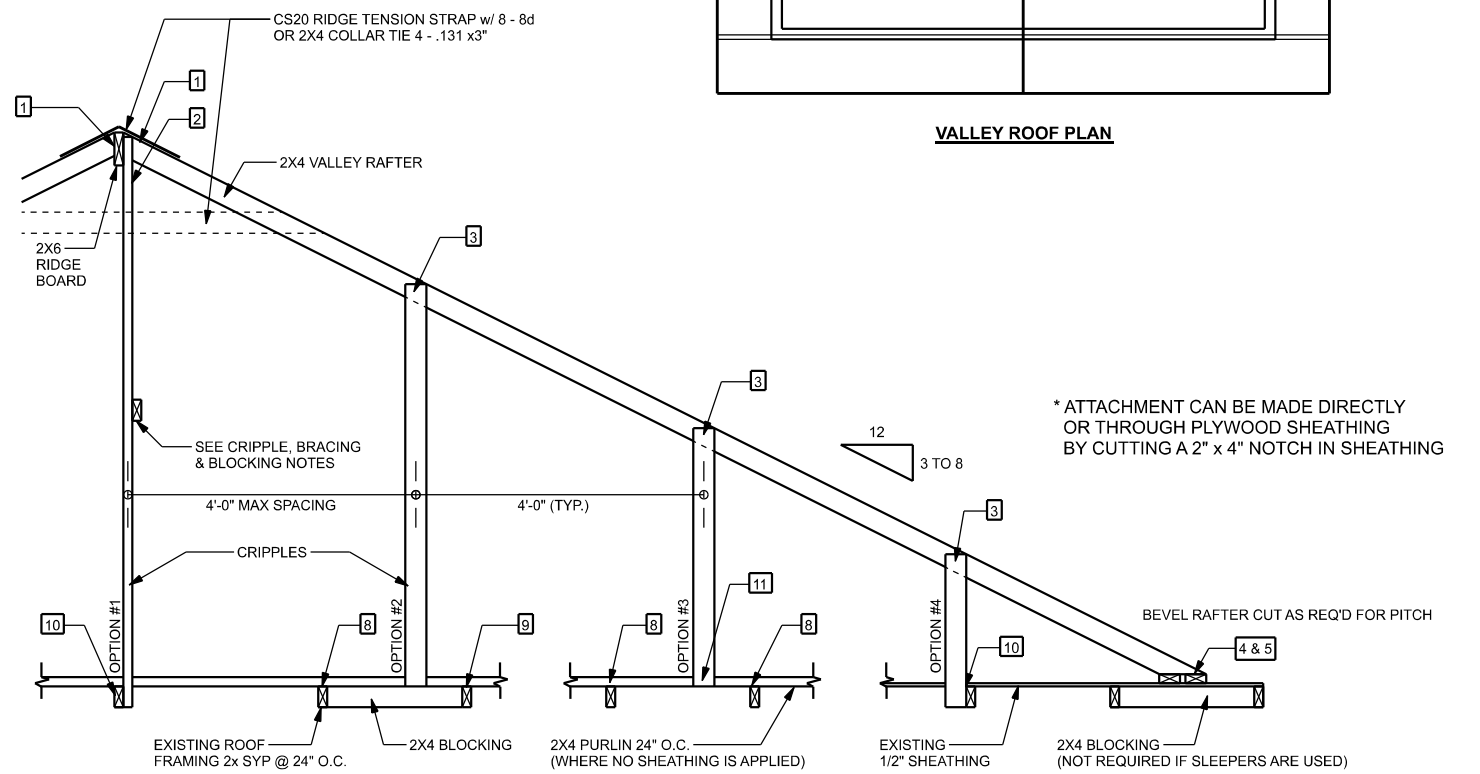
JOB NUMBER:
240678

S-2

OF 6 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RIDGE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLINS / LATERAL BRACING	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER BEAT CUT) SYP #2 OR 2 PARALLEL 2X4 SYP #4
CRIPPLES & BLOCKING	2X4 SYP #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



SECTION CUT PARALLEL TO VALLEY RAFTER

VALLEY ROOF PLAN MEMBER LEGEND

=====	TRUSS
=====	TRUSS UNDER VALLEY FRAMING
=====	VALLEY RAFTER OR RIDGE
=====	CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

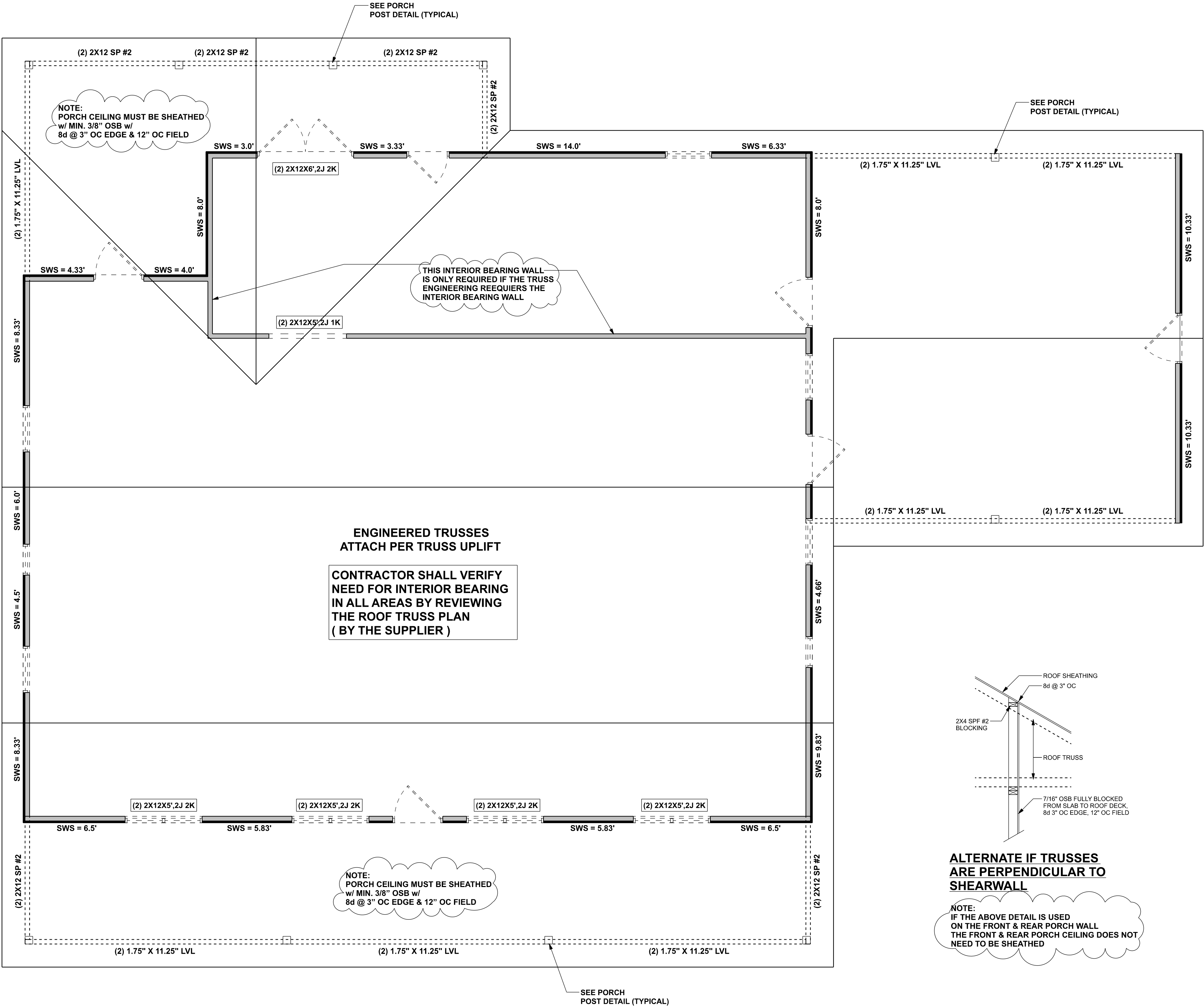
CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	4- 131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	4- 131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	4- 131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	4- 131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4- 131 x 3" TOE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	4- 131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	4- 131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	4- 131 x 3" NAILS
9	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4- 131 x 3" NAILS
10	TRUSS TO BLOCKING	4- 131 x 3" END NAILS
11	CRIPPLE TO TRUSS	4- 131 x 3" FACE NAILS
12	CRIPPLE TO PURLIN	4- 131 x 3" FACE NAILS

ROOF OVER FRAMING & BRACING DETAIL SCALE: N.T.S.

GENERAL NOTES

MAXIMUM RAFTER SPANS:
6'-0" FOR 2X4, 8'-0" FOR 2X6 SYP #2 OR SYP #2
MAXIMUM ROOF AREA PER SUPPORT:
1800 IN SQUARES (8.3' x 14.1' IN DIM.) (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1600 OR 2'-0" X 8'-0" SPAN = 1600)
PURLIN REQUIRED: 10'-0" O.C. IF EXISTING SHEATHING IS REMOVED
PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM
IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 3-16 COMMON WIRE NAILS
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
- SPANS DISTANCES BETWEEN WALLS 16'-0" OR LESS
- MAXIMUM VALLEY HEIGHT: 16'-0" OR LESS
- MAXIMUM WIND SPEED: 130 MPH
- MAXIMUM TOTAL LOADING: 40 psf
- MEETS IRC 402.2 WIND REQUIREMENTS
- EXPOSURE CATEGORY "C" (1 = 1.0, Kd = 1.0)
- ENCLOSED BUILDING
CRIPPLE BRACING & BLOCKING NOTES
- 2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 8'-0" TO 10'-0" LONG
- NAILS @ 2'-0" MAXES OR 2X4 1" OR 2X6 1 1/2" BRACE NAIL TO FLAT EDGE OF CRIPPLE
- WITH 8 NAILS @ 8" O.C. 1" OR 2X6 1 1/2" OR 2X4 1" MUST BE 80% OF CRIPPLE LENGTH CRIPPLES
- OVER 10'-0" LONG REQUIRE TWO CLBS OR BOTH FACES 1" OR 2X6 1 1/2" OR 2X4 1" USE STRESS
- GRADED LUMBER & BOX OR COMMON NAILS
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE
- INSTALL INSIDE ROOF BOARD
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN
- LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED
- APPLY ALL NAILING IN ACCORDANCE TO NDS-197 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE



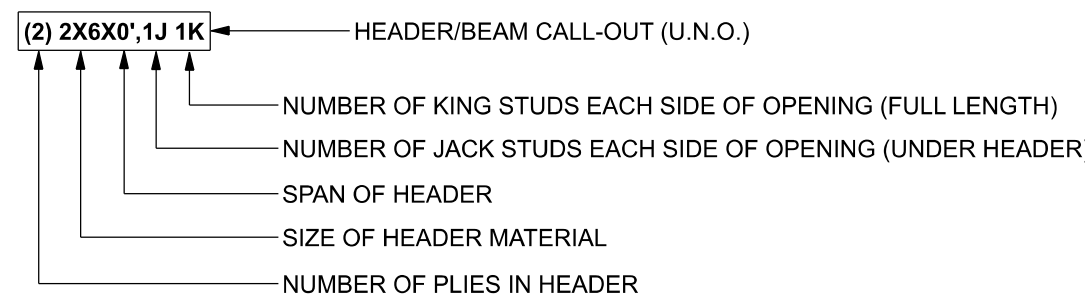
STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

UNLESS NOTED OTHERWISE (MINIMUM REQUIRERMENTS) ***SEE STRUCTURAL PLAN FOR ANY SPECIFIC CALL OUTS***	
BEAM / HEADERS (SIZE)	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (UNO)
HEADERS (JACK & KING STUDS)	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (UNO)
HEADERS (STRAPPING)	ALL HEADERS w/ UPLIFT TO BE STRAPPED OR SCREWED DOWN w/ MIN. OPTION #1 OR OPTION #3 (SEE DETAIL ON SHEET S-1) (U.N.O.) 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
JACK STUDS UNDER GIRDER TRUSS	USE ONE JACK STUD GIRDER SUPPORT PER 2000 LB LOAD

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	18794 LBF	13116 LBF
REQUIRED	16714 LBF	10361 LBF

ALTERNATE IF TRUSSES ARE PERPENDICULAR TO SHEARWALL

NOTE: IF THE ABOVE DETAIL IS USED ON THE FRONT & REAR PORCH WALL THE FRONT & REAR PORCH CEILING DOES NOT NEED TO BE SHEATHED

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Alachua County, FL

FL PE 53915
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JOB NUMBER:
240678
S-3
OF 6 SHEETS