

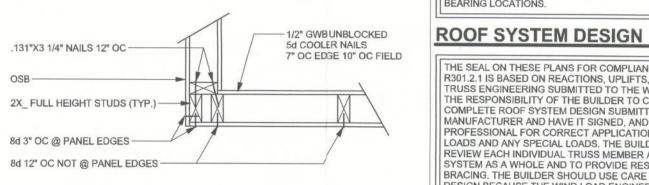
	STUD SPACING	SPECIES & GRADE OF STUD					
WIND SPEED		SPRUCE-PINE-FIR #2			SOUTHERN PINE #2		
		2x4	2x6	2x8	2x4	2x6	2x8
		MAXIMUM ALLOWABLE STUD LENGTH					
90 MPH	12" OC	13'-4"	20'-0"	20'-0"	14'-0"	20'-0"	20'-0"
	16" OC	12'-1"	19'-4"	20'-0"	12'-7"	20'-0"	20'-0"
	24" OC	10'-6"	16'-9"	20'-0"	11'-0"	17'-7"	20'-0"
100 MPH	12" OC	12'-5"	19'-10"	20'-0"	13'-0"	20'-0"	20'-0"
	16" OC	11'-2"	18'-0"	20'-0"	11'-9"	18'-10"	20'-0"
	24" OC	9'-9"	15'-7"	19'-9"	10'-2"	16'-4"	20'-0"
110 MPH	12" OC	11'-7"	18'-7"	20'-0"	12'-2"	19'-6"	20'-0"
	16" OC	10'-6"	16'-10"	20'-0"	11'-0"	17'-7"	20'-0"
	24" OC	9'-1"	14'-5"	17'-10"	9'-6"	15'-2"	19'-2"
120 MPH 130 MPH	12" OC	10'-11"	17'-6"	20'-0"	11'-5"	18'-4"	20'-0"
	16" OC	9'-10"	15'-10"	20'-0"	10'-4"	16'-7"	20'-0"
	24" OC	8'-7"	13'-2"	16'-3"	9'-0"	13'-10"	17'-5"
	12" OC	10'-4"	16'-7"	20'-0"	10'-10"	17'-4"	20'-0"
	16" OC	9'-4"	15'-0"	18'-6"	9'-9"	15'-8"	19'-11'
	24" OC	8'-1"	12'-1"	14'-11"	8'-6"	12'-8"	16'-0"

- FOR STUD LENGTH OVER 20'-0" SELECT LENGTH FROM APPROVED STUD SPAN TABLE OR PROVIDE LOAD CALCULATION SHEET MAX STUD LENGTHS IN THIS TABLE ARE BASED ON WIND LOAD ZONE 4 AND ASSUME THAT ALL STUDS ARE SHEATHED ON THE EXTERIOR WITH A MINIMUM OF 3/8" WOOD SHEATHING AND ON THE INTERIOR WITH GYPSUM WALL BOARD. STUD SPACING SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. X 0.85 = 13.6" O.C. STUDS SHALL BE CONTINUOUS BETWEEN HORIZONTAL SUPPORTS

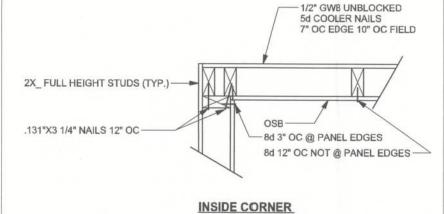
> EXTERIOR WALL STUD TABLE **BASED ON WFCM TABLE A-3.20B**

# **GRADE & SPECIES TABLE**

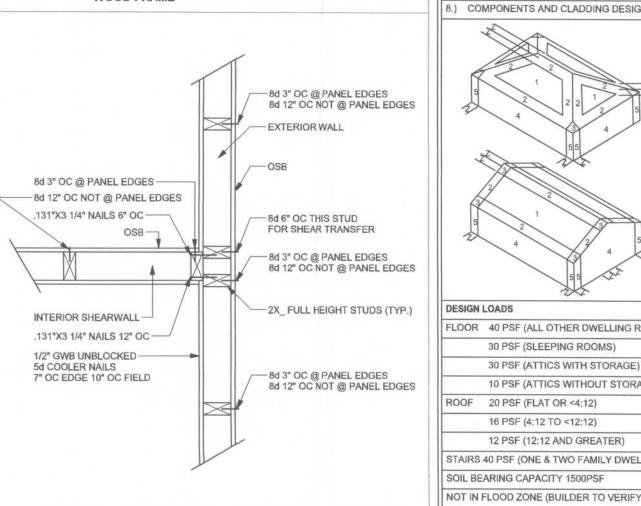
		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0



## **OUTSIDE CORNER**



(TYP.) CORNER FRAMING WOOD FRAME



(TYP.) INTERSECTING WALL FRAMING WOOD FRAME

### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE RUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTION BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS

VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116

CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN

ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS: 7/16" OSB SHEATHING UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUT FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

### **BUILDER'S RESPONSIBILITY**

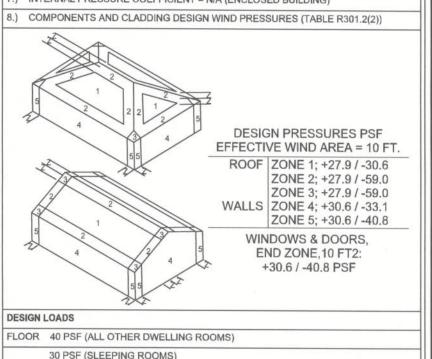
SPECIF	ILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE ICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
	RM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND ILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
WITH F	DE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY BCR 2007 REQUIREMENTS FOR THE STATED WIND VELOCITY AND N PRESSURES.
BELIE!	DE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YO E THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL ND LOAD ENGINEER IMMEDIATELY.

DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

- 1	
	THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTIO
	R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS
	THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE
1	COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS
	MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN
	PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO
	REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF
	SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL
1	BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF
1	DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE
	TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES
	RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.
- 1	THOSE STILLIO.

## DECICAL DATA

WIN	ID LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1
ME.	ICLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; AN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% OPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUI	LDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUI	LDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.)	BASIC WIND SPEED = 110 MPH
2.)	WIND EXPOSURE = C
3.)	WIND IMPORTANCE FACTOR = 1.0
4.)	BUILDING CATEGORY = II
5.)	ROOF ANGLE = 10-45 DEGREES
6.)	MEAN ROOF HEIGHT = <30 FT
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))



30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12) 16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

JOB NUMBER: 1201006 DRAWING NUMBER

OF 2 SHEETS

REVISIONS

Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: Mark Disosway, P.E. hereby expressly

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code

LIMITATION: This design is valid for one building, at specified location

to the best of my knowledge.



Hometown Homes

Mcall River Cabin

ADDRESS: Wilson Springs S/D

Fort White, Florida

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STRUCTURAL BY Evan Beamsley

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2012-01-20

FINALS DATE: