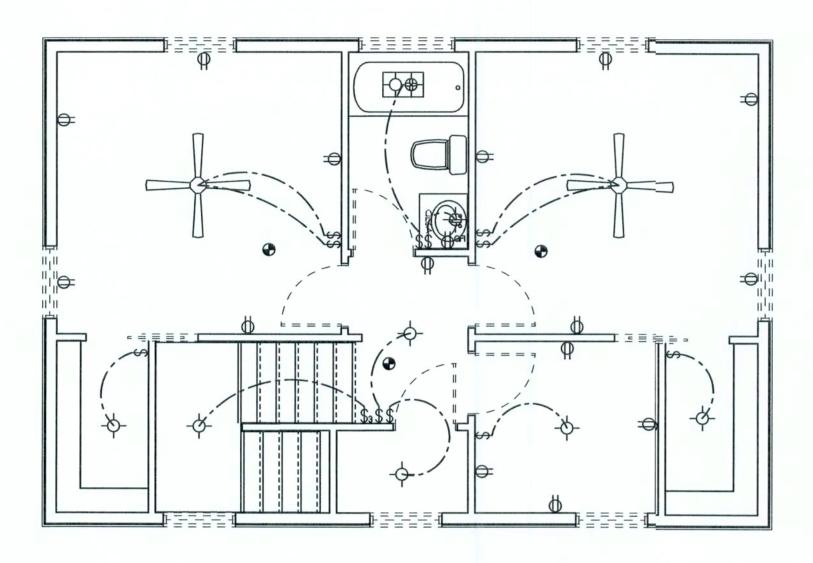
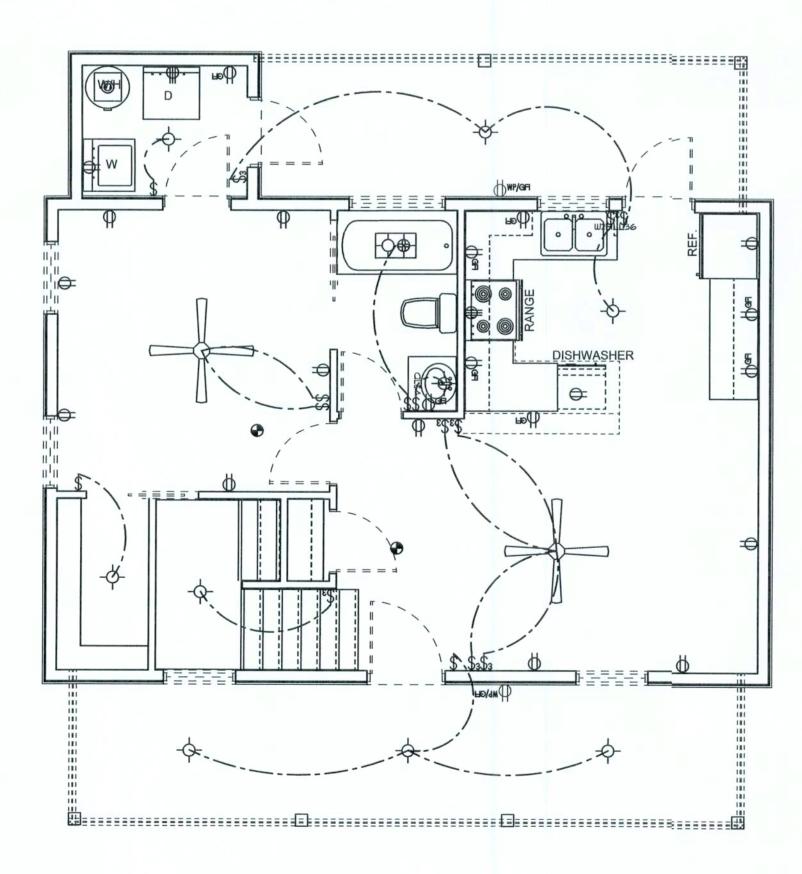


**REVISIONS** 

SOFTPIAN ARCHITECTURAL DESIGN SOFTWARE



2ND FLOOR ELECTRICAL PLAN SCALE: 1/4" = 1'-0"



2ND FLOOR ELECTRICAL PLAN SCALE: 1/4" = 1'-0"

### **ELECTRICAL PLAN NOTES**

- E -1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E -2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E -3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E -4

  ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY
  BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL
  BE INTERLOCKED TOGETHER. INSTALL INSIDE AND
  NEAR ALL BEDROOMS.
- E -5
  TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE
  DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
  DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE
  SECTIONS OF NEC-LATEST EDITION.
- E -6 ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E -7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E -8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E -9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION
- A SERVICE DISCONNECT WITH OVER CURRENT PROTECTION SHALL BE INSTALLED OUTSIDE OF THE BUILDING, ON THE LOAD SIDE OF THE METER, AT THE PLACE ELECTRIC CONDUCTORS ENTER THE BUILDING.

  SERVICE ENTRANCE CONDUCTORS MAY NOT BE LOCATED INSIDE OF THE OF THE BUILDING WITHOUT SPECIAL APPROVAL OF THE BUILDING OFFICIAL

-	ELECTRICAL LEGEND
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
QP	DOUBLE SECURITY LIGHT
	2X4 FLUORESCENT LIGHT FIXTURE
0	RECESSED CAN LIGHT
→ 🛞	BATH EXAUST FAN WITH LIGHT
₩	BATH EXAUST FAN
<b>-</b>	LIGHT FIXTURE
Ф	DUPLEX OUTLET
₩	220v OUTLET
⊕an ⊕an	GFI DUPLEX OUTLET
•	SMOKE DETECTOR
\$	WALL SWITCH
\$3	3 WAY WALL SWITCH
\$4	4 WAY WALL SWITCH
∯ <sub>WP/GFI</sub>	WATER PROOF GFI OUTLET
$\nabla$	PHONE JACK
60	TELEVISION JACK
更	GARAGE DOOR OPENER
<del></del>	WALL HEATER

North Florida Framers

**Brooks Spec House** 

ADDRESS: 223 NE Gum Swamp Rd. Lake City, FL 32055

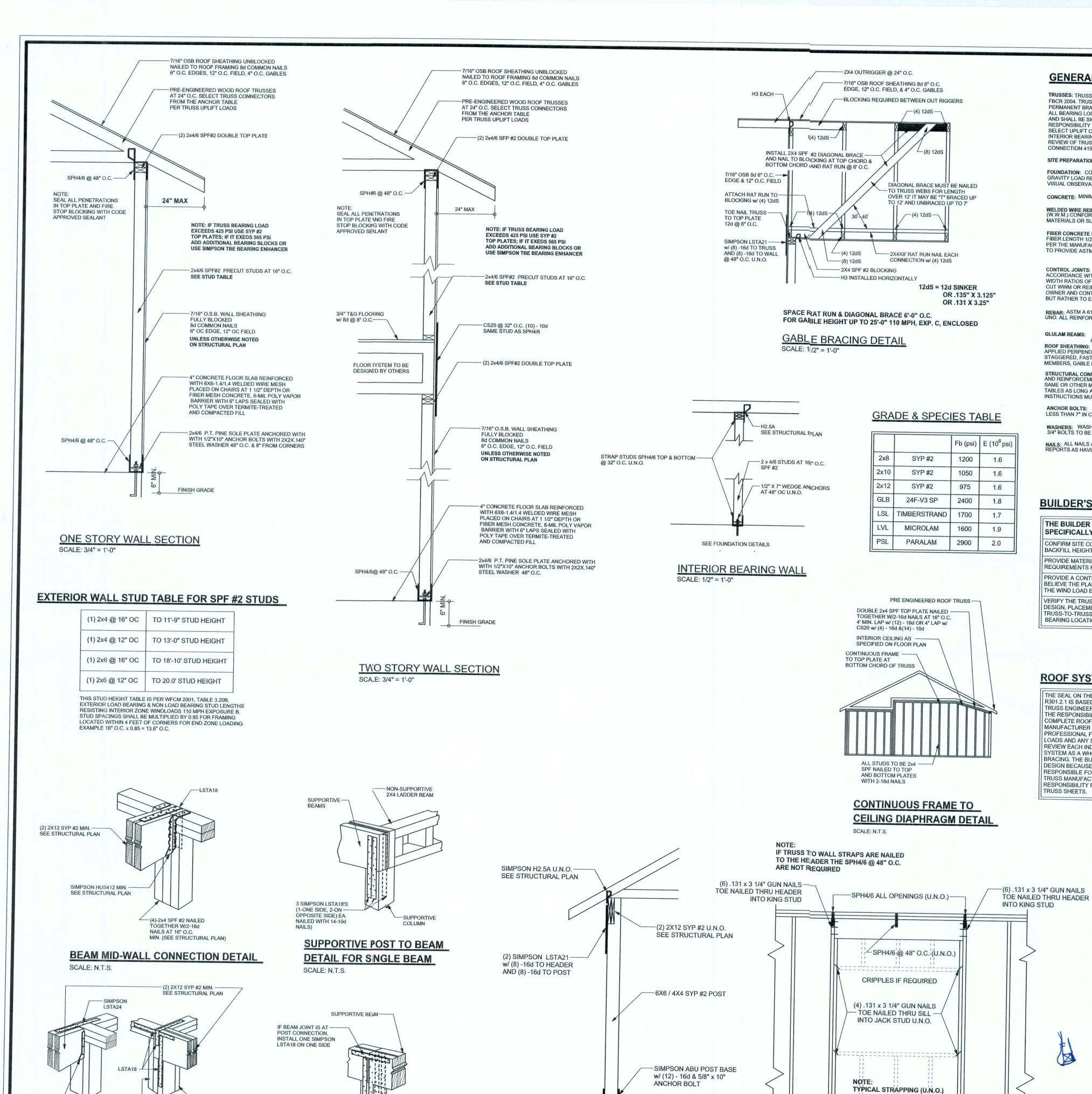
PRINTED DATE: October 26, 2007 DRAWN BY: STRUCTURAL BY:

FINALS DATE:

26 / Oct / 07 JOB NUMBER: 710171

DRAWING NUMBER

OF 2 SHEETS



NAIL THRU 2x4 INTO

BEAM MAY BE ATTACHED IN

**BEAM CORNER CONNECTION. DETAIL** 

4-SIMPSON LSTA18 -

SUPPORTIVE CENTER POST TO BEAM DETAIL

(2-ONE SIDE,2-ON OTHER SIDE)

BEAM W/4-16d

SIMPSON HUS412 MIN.

SEE STRUCTURAL PLAN

### **GENERAL NOTES:**

RUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE, STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED

MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2"  $\times$  2"  $\times$  9/64"; WITH 5/8" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 3/4" BOLTS TO BE 3"  $\times$  3"  $\times$  9/64"; WITH 7/8" BOLTS TO BE 3"  $\times$  3"  $\times$  5/16"; UNO. NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST

### **BUILDER'S RESPONSIBILITY**

REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE. PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES. ROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

### ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

MASONRY NOTES:

Mortar

Grout

3.3.E.7 | Movement joints

CMU standard

Clay brick standard

Reinforcing bars, #3 - #11

Coating for corrosion protection

Coating for corrosion protection

IN WRITING.

1.4A

2.1

2.4F

(SEE STRUCTURAL PLAN)

- SPH4/6 ALL OPENINGS (U.N.O.)

(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.

(1) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O.

(FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

YPICAL HEADER STRAPING DETAIL

-SEE FOOTING DETAILS

4X4 / 6X6 PORCH POST DETAIL

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

3.3.E.2 Pipes, conduits, and accessories Any not shown on the project drawings

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

ASTM C 90-02, Normal weight, Hollow,

bond and 12"x12" or 16"x16" column

medium surface finish, 8"x8"x16" runninç

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM

require engineering approval.

detailed on project drawings.

A525, Class G60, 0.60 oz/ft2 or 304SS

Joint reinforcement in walls exposed to

moisture or wire ties, anchors, sheet metal

ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2

Contractor assumes responsibility for type

and location of movement joints if not

ASTM C 270, Type N, UNO

5.5"x2.75"x11.5"

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

ANCHOR TABLE

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TOSTUDO
< 420	< 245	H5A	3-8d	3-8d	TO STUDS
< 455	< 265	H5	4-8d	3-8d 4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	НЗ	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"		
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	2-10d, 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	7-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	12-100 1 1/2	12-10d 1 1/2"	
< 2050	< 1785	LGT2	14 -16d	44 464	
			14-10u	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
10.7		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		OIO TID
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	2000	ABOOO	12-10u		1/2" AB

**DESIGN DATA** WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION 1.) BASIC WIND SPEED = 110 MPH

.) WIND EXPOSURE = B

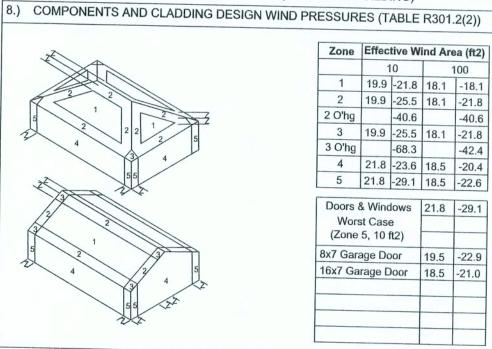
WIND IMPORTANCE FACTOR = 1.0

BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)



### DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS) 30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE) 10 PSF (ATTICS WITHOUT STORAGE, <3:12) ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER) STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

Fax: (386) 269 - 4871

October 26, 2007 STRUCTURAL BY:

JOB NUMBER:

710171 DRAWING NUMBER S-1

OF 3 SHEETS

lo.53915, POB 868, Lake City, FL 32056, 386-754-5419 DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

REVISIONS

SOFTPLAN

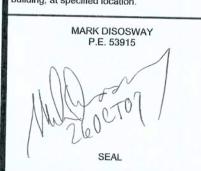
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xamined this plan, and that the applicable

/INDLOAD ENGINEER: Mark Disosway

ortions of the plan, relating to wind engine omply with section R301.2.1, florida buildin ode residential 2004, to the best of my

IMITATION: This design is valid for one ouilding, at specified location.



North Florida Framers

Spec House

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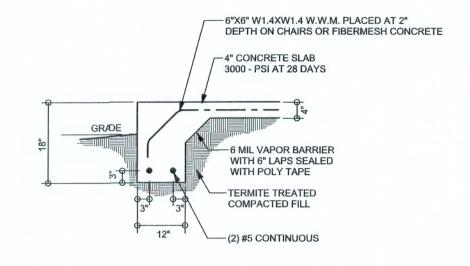
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419

PRINTED DATE:

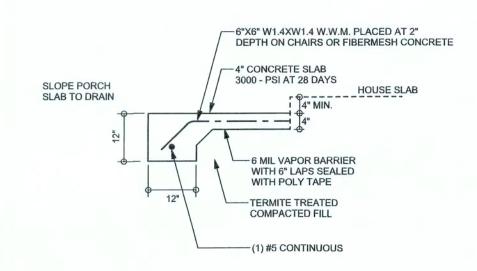
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**REVISIONS** 

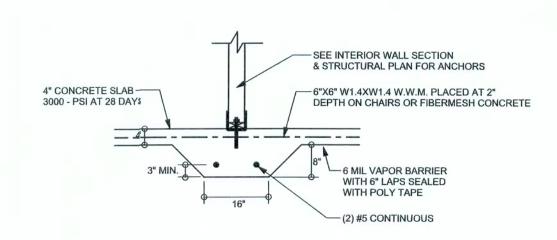
SOFTPIX



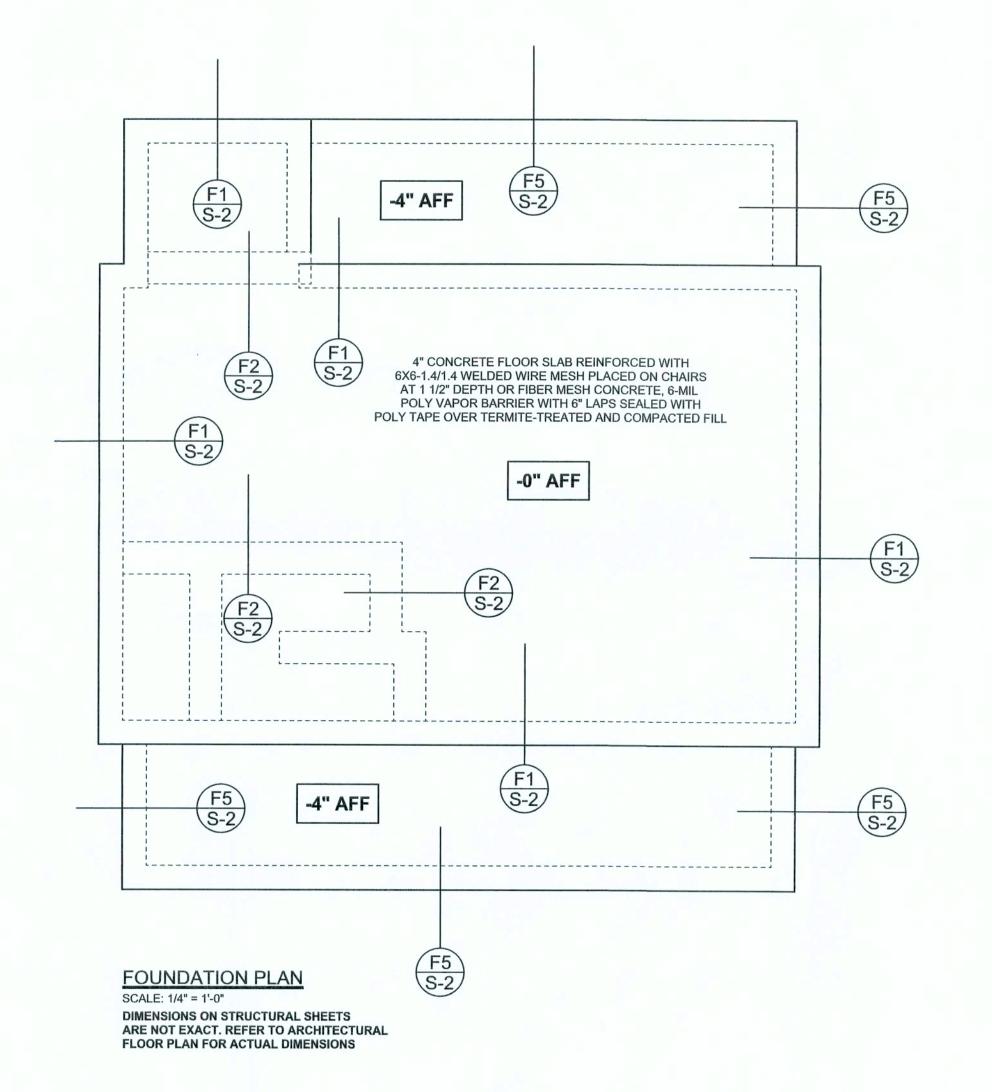
# F1 MONOLITHIC FOOTING S-2 SCALE: 1/2" = 1'-0"



# F5 PORCH FOOTING S-2 SCALE: 1/2" = 1'-0"



F2 INTERIOR BEARING FOOTING
S-2 SCALE: 1/2" = 1'-0"



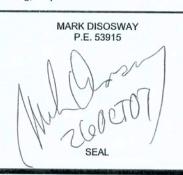
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
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permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my

LIMITATION: This design is valid for one building, at specified location.



North Florida Framers

Spec House

ADDRESS: 223 NE Gum Swamp Rd. Lake City, FL 32055

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

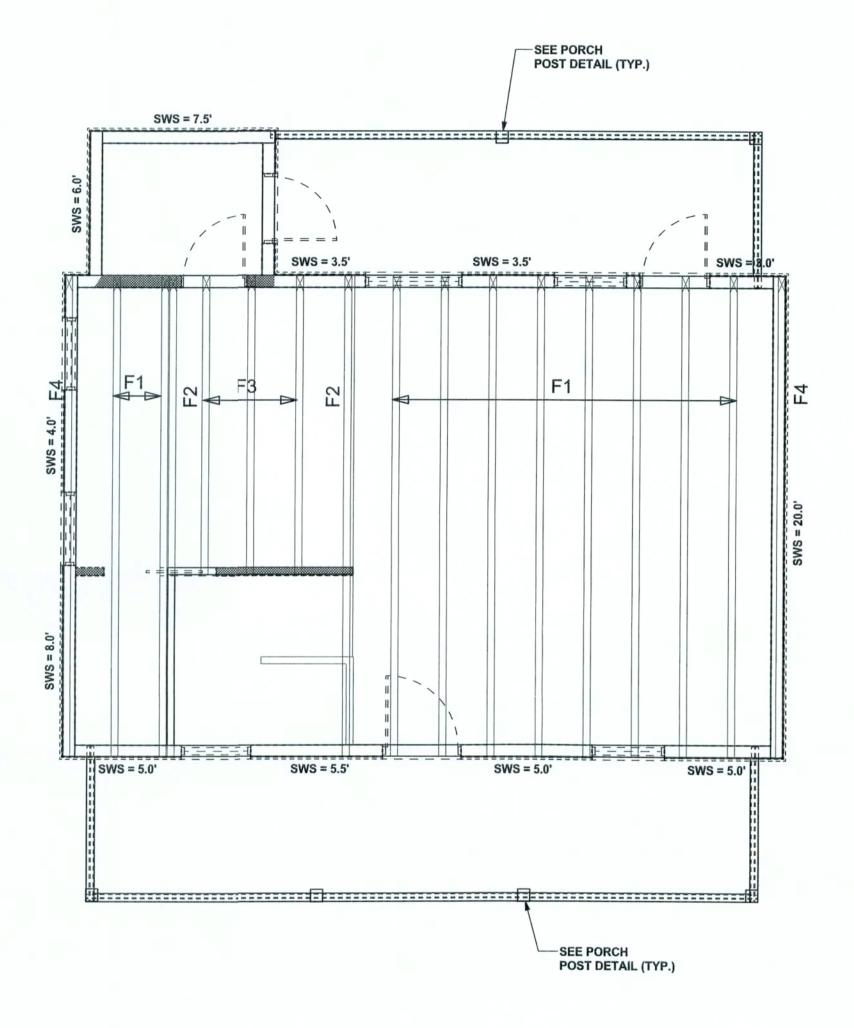
PRINTED DATE:
October 26, 2007
STRUCTURAL BY

FINALS DATE:

JOB NUMBER: 710171 DRAWING NUMBER

> S-2 OF 3 SHEETS

SOFTPIAN



STRUCTURAL FLOOR PLAN SCALE: 1/4" = 1'-0"

### STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARNG FRAME WALL & PORCH HEADERS SHALL BE A MINMUM OF (2) 2X12 SYP #2 (U.N.O.)

SN-2 ALL LOAD BEARNG FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

DIMENSIONS ON STRUCTURAL SHEETS SN-3

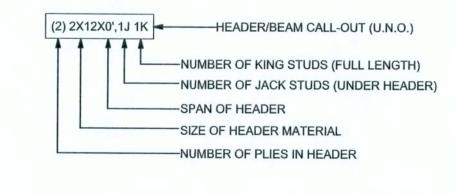
ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS \$HOWN ON THE SEALED TRUSS DRAWINGS. SN-4 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHEDBY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

## WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR WALL
IBW 2000000000000000000000000000000000000	1ST FLOOR INTERIOR BEARING WALL
IBW	2ND FLOOR INTERIOR BEARING WALL

## **HEADER LEGEND**



### 1ST FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	33.7	38.0'
LONGITUDINAL	25.4'	34.5'

USE H2.5A (535lb) FOR ALL TRUSS TO WALL FRAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE	sv	VS = 4.5	. /	V   II		S = 5.0'		X	X I	sws	= 5.0			SWS =	5.0'	A
F = = SWS = 8.5		⊎ USE CON	H2.5A	\ (535 IONS	b) FC UNLE	R ALL	. TRU		O WA	LL FR SE	AME	AND	PORC	H BE	<b>A</b> M	SWS = 8.5'
							  -  -									SWS = 8.5'

STRUCTURAL ROOF PLAN
SCALE: 1/4" = 1'-0"

### 2ND FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	14.3'	34.0'
LONGITUDINAL	12.2'	40.5'

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419 Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification. COPYRIGHTS AND PROPERTY RIGHTS: COPYRIGHTS AND PROPERTY RIGHTS:
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permission and consent of Mark Disosway. CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge. LIMITATION: This design is valid for one building, at specified location. MARK DISOSWAY P.E. 53915

North Florida Framers

Spec House

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