



Alpine, an ITW Company 6750 Forum Drive, Suite 305 Orlando, FL 32821 Phone: (800)755-6001 www.alpineitw.com

Site Information:

Customer: W. B. Howland Company, Inc.

Job Number: 20-4558

Job Description: Summers Pool House

Address:

Job Engineering Criteria:				
Design Code: FBC 2017 RES	IntelliVIEW Version: 18.02.01B through 20.01.01A			
	JRef #: 1WYc2150006			
Wind Standard: ASCE 7-10 Wind Speed (mph): 130	Roof Load (psf): 20.00-10.00- 0.00-10.00			
Building Type: Closed	Floor Load (psf): None			

This package contains general notes pages, 4 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	247.20.0816.01663	A01
3	247.20.0816.17503	A03
5	BRCLBSUB0119	
7	GBLLETIN0118	

Item	Drawing Number	Truss
2	247.20.0816.05190	A02
4	247.20.0816.28480	A04
6	A14015ENC101014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 514 Earth City Expressway, Suite 242, Earth City, MO 63045; www.alpineitw.com.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 332272 COMN Ply: 1 Job Number: 20-4558 Cust: R 215 JRef: 1WYc2150006 T6 FROM: CDM DrwNo: 247.20.0816.01663 Qty: 4 Summers Pool House Truss Label: A01 / YK 09/03/2020 5'4"5 10'2"3 19'9"13 24'7"11 30' 5'4"5 4'9"13 4'9"13 4'9"13 4'9"13 5'4"5 ∥4<u>¥</u>5 **₹**525 K ≡6X8 L ≡3X4 J ≡3X4 =4X4(A2) =4X4(A2)

30'

7'2"12

22'2"12

7'2"12

15'

Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25	Wind Criteria Wind Std: ASCE 7-10 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2	Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 2017 RES TPI Std: 2014	PP Deflection in loc L/defl L/# VERT(LL): 0.101 K 999 240 VERT(CL): 0.205 K 999 180 HORZ(LL): 0.041 J HORZ(TL): 0.084 J Creep Factor: 2.0 Max TC CSI: 0.299 Max BC CSI: 0.758	<u>ا</u> اِ
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.084 J	ľ
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.299	i
Load Duration: 1.25 Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft	Rep Fac: Yes FT/RT:20(0)/10(0)	Max Web CSI: 0.506	
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 20.01.01A.0724.11	- <u> </u>

7'9"4

7'9"4

Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL В 1336 /-/796 /237 /228 1336 /-/-/796 /237 /-Wind reactions based on MWFRS Brg Width = 3.5Min Req = 1.6 В Brg Width = 3.5 Min Req = 1.6 Bearings B & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

7'9"4

30' ▲ Maximum Reactions (lbs)

1'6"

Chords Tens.Comp. Chords Tens. Comp. 470 - 2221 404 - 1433

C - D 464 - 2013 F-G 464 - 2013 D-E 404 - 1433 G-H 470 - 2221

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2:

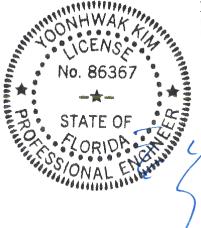
Webs: 2x4 SP #3;

The overall height of this truss excluding overhang is

Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Tens. Comp. Chords B-L 1923 - 318 K-J 1588 - 244 J - H L-K 1588 - 225 1923 - 344

Maximum Web Forces Per Ply (lbs) Tens. Comp. Webs Tens.Comp. Webs

					<u> </u>
L-D	416	- 60	K-F	179	- 531
D-K	179	- 531	F-J	416	- 59
F-K	917	- 220			



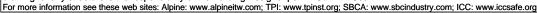
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WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

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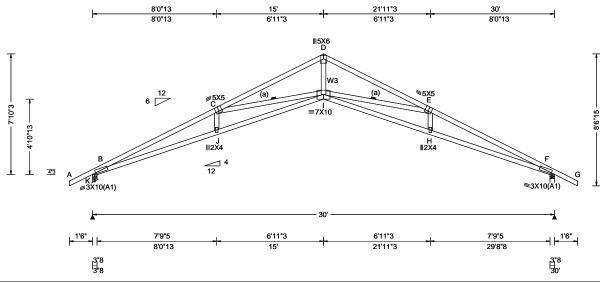
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.





SEQN: 317046 COMN Ply: 1 Job Number: 20-4558 Cust: R 215 JRef: 1WYc2150006 T1 FROM: CDM Qty: 13 DrwNo: 247.20.0816.05190 Summers Pool House Truss Label: A02 / YK 09/03/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)		▲ Maximum Reactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.577 I 618 240	Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 1.181 I 301 180	K 1352 /- /- /805 /231 /229
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): 0.476 H	F 1352 /- /- /805 /231 /-
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.975 H	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 15.00 ft	Building Code:	Creep Factor: 2.0	K Brg Width = 3.5 Min Req = 1.5
Soffit: 2.00	TCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.562	F Brg Width = 3.5 Min Req = 1.5
Load Duration: 1.25	BCDL: 5.0 psf	TPI Std: 2014	Max BC CSI: 0.592	Bearings K & F are a rigid surface.
	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.708	Members not listed have forces less than 375#
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Wax Web Col. 0.700	Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: Any	FT/RT:20(0)/10(0)		Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	Plate Type(s):		D 0 4054 5500 D 5 4040 4404
	Wind Duration: 1.60	WAVE	VIEW Ver: 18.02.01B.0321.09	B - C 1954 - 5503 D - E 1319 - 4124 C - D 1351 - 4124 E - F 1983 - 5503
Lumbor	·	·	·	C-D 1351 -4124 E-F 1983 -5503

Lumber

Top chord: 2x4 SP M-31; Bot chord: 2x4 SP M-31; Webs: 2x4 SP #3; W3 2x4 SP #2;

(a) Continuous lateral restraint equally spaced on

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

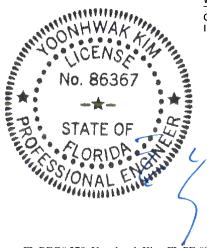
The overall height of this truss excluding overhang is



Chords	Tens.Comp.	Chords	Tens. Comp.
B - J	5095 - 1666	I - H	5114 - 1699
J - I	5114 - 1657	H - F	5095 - 1708

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. (Comp.
C - I I - F	771 - 1216 767 - 1216	D-I	3138	- 863



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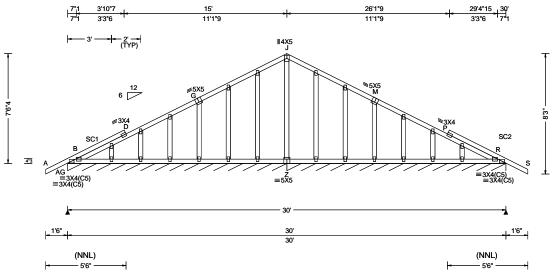
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.





SEQN: 332275 GABL Ply: 1 Job Number: 20-4558 Cust: R 215 JRef: 1WYc2150006 T7 FROM: CDM DrwNo: 247.20.0816.17503 Qty: 1 Summers Pool House Truss Label: A03 / YK 09/03/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)		▲ Maximum Reactions (Ib	• •	
TCLL: 20.00	Wind Std: ASCE 7-10		PP Deflection in loc L/defl L/#	Gravity Loc R+ /R- /Rh	Non-Gravity / Rw / U / I	/ RL
TCDL: 10.00	Speed: 130 mph Enclosure: Closed	Pf: NA Ce: NA	VERT(LL): 0.002 P 999 240			
BCLL: 0.00	Risk Category: II	Lu: NA Cs: NA	VERT(CL): 0.004 P 999 180	AG*94 /- /-	/57 /- /3	-
BCDL: 10.00 Des Ld: 40.00	EXP: C Kzt: NA	Snow Duration: NA	HORZ(LL): 0.001 D HORZ(TL): 0.001 D	R* 84 /- /- Wind reactions based on M	/51 /- /- IWFRS	•
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	AG Brg Width = 180 R Brg Width = 180	Min Req = - Min Req = -	
Soffit: 2.00 Load Duration: 1.25	BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2	FBC 2017 RES TPI Std: 2014	Max TC CSI: 0.278 Max BC CSI: 0.041	Bearings AG & Z are a rigio	surface.	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.110	Members not listed have for	rces less than 375	5#
	Loc. from endwall: Any	FT/RT:20(0)/10(0)				
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 20.01.01A.0724.11			

Lumber

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 SP #2;

Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is



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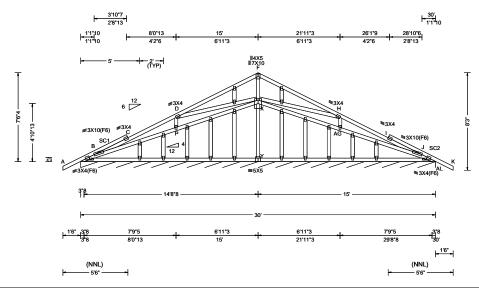
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For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



SEQN: 317056 GABL Ply: 1 Job Number: 20-4558 Cust: R 215 JRef: 1WYc2150006 T3 FROM: CDM Qty: 1 DrwNo: 247.20.0816.28480 Summers Pool House Truss Label: A04 / YK 09/03/2020



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	DefI/CSI Criteria	١.
TCLL: 20.00	Wind Std: ASCE 7-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.072 I 999 240	١.
BCLL: 0.00	Enclosure: Closed	Lu: NA Cs: NA	VERT(CL): 0.148 I 999 180	١.
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.033 I	,
Des Ld: 40.00	EXP: C Kzt: NA		HORZ(TL): 0.068 I	١.
NCBCLL: 10.00	Mean Height: 15.00 ft TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	
Soffit: 2.00	BCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.674	
Load Duration: 1.25	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.383	
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.399	
-	Loc. from endwall: Any	FT/RT:20(0)/10(0)		
	GCpi: 0.18	Plate Type(s):		l
	Wind Duration: 1.60	WAVE	VIEW Ver: 18.02.01B.0321.09	
Lumber		The project engineer shall p		
Top chord: 2x4 SP #2;		stability per Florida Building		
Bot chord: 2x4 SP #2;			erally braced as specified by	

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2: Webs: 2x4 SP #3;

Stack Chord: SC1 2x4 SP #2; Stack Chord: SC2 2x4 SP #2;

Plating Notes

All plates are 2X4 except as noted.

Purlins

Laterally brace BC at 24" oc in lieu of rigid ceiling. Laterally brace BC above filler at 24" oc.

In lieu of structural panels use purlins to brace TC @ 24" oc.

Wind loads based on MWFRS with additional C&C member design.

Additional Notes

See DWGS A14015ENC101014 & GBLLETIN0118 for gable wind bracing and other requirements.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" oc intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" oc. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The overall height of this truss excluding overhang is 7-6-4.

the project engineer. This truss will not provide lateral support of the endwall.

▲ Maximum Reactions (lbs), or *=PLF

Gravity Non-Gravity Loc R+ /Rh /Rw /U /RL AL* 90 /-/-/45

Wind reactions based on MWFRS AL Brg Width = 360 Min Req = -

Bearing B is a rigid surface.

Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)

Chords Tens.Comp. Chords Tens. Comp. B - C 590 - 647 1 - J 592 - 643

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. C	omp.
P-D	338 - 430	X - Y	221	- 502
F-X	299 - 485	H -AG	338	- 430



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For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

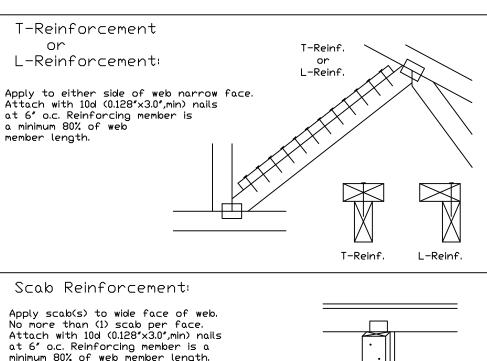
Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

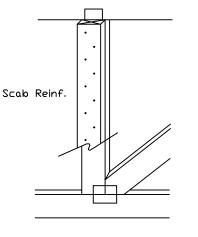
Web Member	Specified CLR	Alternative Reir	
Size	Restraint	T- or L- Reinf.	
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1-2×6
2×6	2 rows	2×6	2-2×4(*)
2×8	1 row	2×6	1-2×8
2×8	2 rows	2×6	2-2×6(*/)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(**) Center scab on wide face of web. Apply (1) scab to each face of web.



No. 86367



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For more information see this job's general notes page and these web sites 932202.

ALPINE; www.alpineitw.com; TPI; www.tpinst.org; SBCA; www.sbcindustry.org; ICC; www.iccsafe.org; Yoonhwak Kim, FL PE #86367

TC LL PSF REF CLR Subst.

DATE 01/02/19

BC DL PSF DRWG BRCLBSUB0119

BC LL PSF

DUR. FAC.

SPACING

514 Earth City Expressway Suite 242 Earth City, MO 63045

Gable Stud Reinforcement Detail

ASCE 7-10: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D. Kzt = 1.00

		2x4 Vertica	Brace	No	(1) 1×4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 *L	Brace **
Max Gable Vertical Length	Spacing	Species	Grade		Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
	24" o.c.	SPF	#1 / #2	4′ 3″	7′ 3″	7' 7"	8′ 7 ″	8′ 11″	10′ 3″	10′ 8 ″	13′ 6″	14' 0"	14′ 0″	14′ 0″
			#3	4′ 1″	6′ 7 ″	7′ 1″	8′ 6 ″	8′ 10 ″	10′ 1″	10′ 6 ″	13′ 4″	13′ 10 ″	14′ 0″	14′ 0″
		HF	Stud	4′ 1″	6′ 7 ″	7′ 0 ″	8′ 6 ″	8′ 10 ″	10′ 1″	10′ 6″	13′ 4″	13′ 10 ″	14′ 0″	14′ 0″
			Standard	4′ 1″	5′ 8 ″	6′ 0 ″	7′ 7″	8′ 1 ″	10′ 1″	10′ 6″	11′ 10″	12′ 8″	14′ 0″	14′ 0″
		SP	#1	4′ 6″	7′ 4″	7′ 8 ″	8′ 8 ″	9′ 0″	10′ 4″	10′ 9 ′	13′ 8″	14′ 0″	14′ 0″	14′ 0″
			#2	4′ 3″	7′ 3″	7′ 7″	8′ 7 ″	8′ 11 ″	10′ 3″	10′ 8 ″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
		DFL	#3	4′ 2″	6′ 0″	6′ 4″	7′ 11″	8′ 6 ″	10′ 2″	10′ 7″	12′ 5″	13′ 4″	14′ 0″	14′ 0″
			Stud	4′ 2″	6′ 0″	6′ 4″	7′ 11″	8′ 6 ″	10′ 2″	10′ 7″	12′ 5″	13′ 4″	14′ 0″	14′ 0″
			Standard	4′ 0″	5′ 3 ″	5′ 7 ″	7′ 0 ″	7′ 6″	9′ 6″	10′ 2 ′	11′ 0″	11′ 10″	14′ 0″	14′ 0″
	16" o.c.	SPF HF	#1 / #2	4′ 11″	8′ 4″	8′ 8 ″	9′ 10″	10′ 3″	11′ 8″	12′ 2″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			#3	4′ 8″	8′ 1 ″	8′ 8″	9′ 8″	10′ 1″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Stud	4′ 8 ″	8′ 1″	8′ 6 ″	9′ 8″	10′ 1″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	4′ 8 ″	6′ 11″	7′ 5 ′	9′ 3 ″	9′ 11″	11′ 7″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
		SP	#1	5′ 1 ″	8′ 5 ″	8′ 9 ″	9′ 11″	10′ 4″	11′ 10″	12′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			#2	4′ 11″	8′ 4″	8′ 8 ″	9′ 10″	10′ 3″	11′ 8″	12′ 2 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
		DFL	#3	4′ 9″	7′ 4″	7′ 9″	9′ 9″	10′ 2″	11′ 8″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Stud	4′ 9 ″	7′ 4″	7′ 9 ″	9′ 9″	10′ 2″	11′ 8″	12′ 1″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	4′ 8″	6′ 5 ″	6′ 10 ″	8′ 7 ″	9′ 2″	11′ 7″	12′ 1″	13′ 6″	14′ 0″	14′ 0″	14′ 0″
	12″ o.c.	SPF HF	#1 / #2	5′ 5 ″	9′ 2″	9′ 6″	10′ 10″	11′ 3″	11′ 8″	13′ 5 ′	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			#3	5′ 1 ′	9′ 0″	9′ 4″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Stud	5′ 1 ″	9′ 0″	9′ 4″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	5′ 1 ″	8′ 0″	8′ 6″	10′ 8″	11′ 1″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
		SP	#1	5′ 8 ″	9′ 3″	9′ 8″	10′ 11″	11′ 4″	13′ 0″	13′ 6 ″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			#2	5′ 5 ″	9′ 2″	9′ 6″	10′ 10″	11′ 3″	12′ 11″	13′ 5 ′	14′ 0″	14′ 0″	14′ 0″	14′ 0″
		DFL	#3	5′ 3 ″	8′ 5 ″	9′ 0″	10′ 9″	11′ 2″	12′ 10″	13′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Stud	5′ 3 ″	8′ 5 ″	9′ 0″	10′ 9″	11′ 2″	12′ 10″	13′ 4″	14′ 0″	14′ 0″	14′ 0″	14′ 0″
			Standard	5′ 1 ″	7′ 5″	7′ 11″	9′ 11″	10′ 7″	12′ 9″	13′ 3″	14′ 0″	14′ 0″	14′ 0″	14′ 0″

Bracing Group Species and Grades: Group A: Spruce-Pine-Fir <u>He</u>m-Fir #1 / #2 Standard #2 Stud #3 Stud #3 Standard D<u>ouglas Fir-Lar</u>ch Southern Pine*** #3 Stud Stud Standard Standard Group B: Hem-Fir #1 & Btr Douglas Fir-Larch Southern Pine*** #1 #1 #2

1x4 Braces shall be SRB (Stress-Rated Board) **For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards, Group B values may be used with these grades.

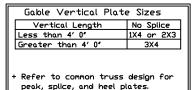
Gable Truss Detail Notes: Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

Attach "L" braces with 10d (0.128"x3.0" min) nails. * For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones. ₩¥For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

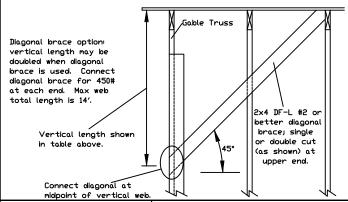


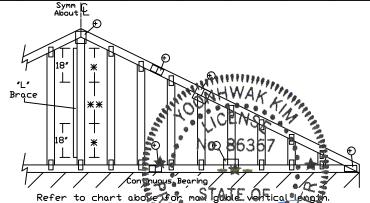
Refer to the Building Designer for conditions not addressed by this detail.

DATE 10/01/14

DRWG A14015ENC101014

ASCE7-10-GAB14015





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MAX, TOT, LD, 60 PSF MAX. SPACING 24.0"

514 Earth City Expressway Suite 242 Earth City, MO 63045

Gable Detail For Let-in Verticals Gable Truss Plate Sizes Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs. (+) Refer to Engineered truss design for peak, splice, web, and heel plates. ₩If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web. Gable Example: Length typ. (*)

Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:

10d Common (0.148"x 3.", min) Nails at 4" o.c. plus

(4) nails in the top and bottom chords.

10d Common (0.148"x3".min) Toenails at 4" o.c. plus

(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A14013ENC100118,

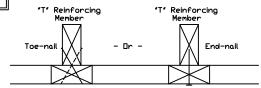
A18015ENC100118, A12015ENC100118, A12015ENC100118, A12015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A120015ENC100118, A12003ENC100118, A12003ENC100118, A120030ENC100118, A120030ENC100118,

\$18015ENC100118, \$20015ENC100118, \$20015END100118, \$20015PED100118 \$11530ENC100118, \$12030ENC100118, \$14030ENC100118, \$12030ENC100118)

\$18030ENC100118, \$20030ENC100118, \$20030END100118, \$20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical

"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf.	"T"				
Mbr. Size	Increase				
2×4	30 %				
2x6	20 %				

Example:

ASCE 7-10 Wind Speed = 120 mph Mean Roof Height = 30 ft, Kzt = 1.00 Gable Vertical = 24°o.c. SP #3 "T" Reinforcing Member Size = 2x4

"T" Brace Increase (From Above) = 30% = 1.30

(1) 2x4 "L" Brace Length = 8' 7"

Maximum "T" Reinforced Gable Vertical Length $1.30 \times 8' \ 7'' = 11' \ 2''$

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REF LET-IN VERT DATE 01/02/2018 DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

514 Earth City Expressway Suite 242 Earth City, MO 63045

Rigid Sheathing

Ceiling

4 Nails

Nails

Spaced At

4 Nails

Reinforcing Member

Gable

Truss

DUR. FAC. ANY MAX. SPACING 24.0"