



AA C00066

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TOWNEPLACE
SUITES
BY MARIOTT

TOWNEPLACE SUITES HOTEL
LAKE CITY HOTELS, INC.
NW COMMERCIAL DRIVE, LAKE CITY, FLORIDA

REVISION

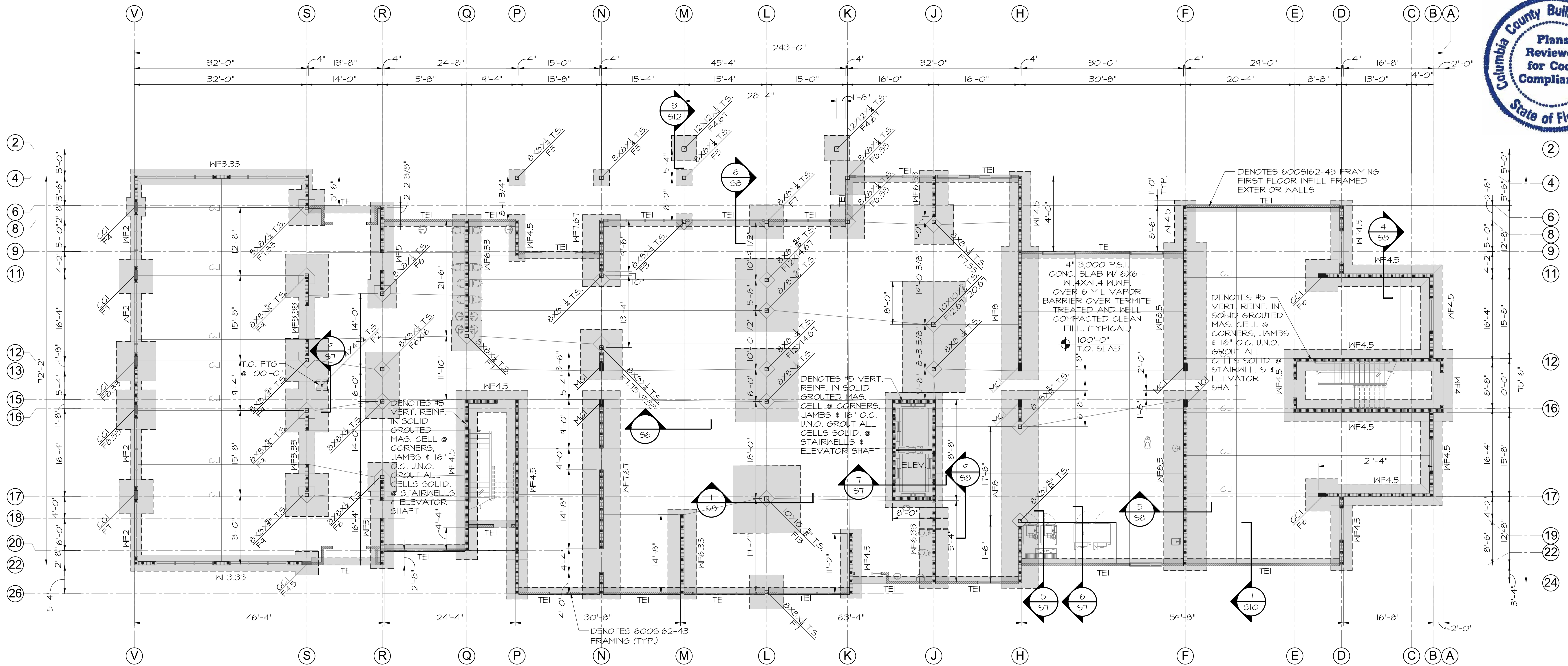
JOB NO.

DATE

SHEET

S1

1 OF



1 FOUNDATION PLAN
S1 SCALE: 1/8"=1'-0"

FOUNDATION SCHEDULE

MARK	SIZE	DEPTH	REINFORCEMENT	T.O.FTG.
F2	2'-0" SQ.	1'-0"	#5 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F3	3'-0" SQ.	1'-0"	#5 REINF. @ 12" O.C. E.W. BOT.	9'-8" U.N.O.
F4.0	4'-0" SQ.	1'-0"	#5 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F4.5	4'-6" SQ.	1'-0"	#5 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F4.67	4'-8" SQ.	1'-8"	#5 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F5.33	5'-4" SQ.	1'-2"	#5 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F5.67	5'-4" SQ.	1'-2"	#5 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F6	6'-0" SQ.	1'-2"	#6 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F6.33	6'-4" SQ.	1'-4"	#6 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F6x16	6'-0" X 16'-0"	1'-2"	#6 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F6.67	6'-8" SQ.	1'-4"	#6 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F7	7'-0" SQ.	1'-4"	#6 REINF. @ 12" O.C. E.W. BOT.	9'-8"
F7.33	7'-4" SQ.	1'-6"	#6 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F8	8'-0" SQ.	1'-6"	#8 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F8.33	8'-4" SQ.	1'-6"	#8 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F9	9'-0" SQ.	1'-6"	#8 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F11.67	11'-8" SQ.	1'-6"	#9 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F12x14.67	12'-0" X 14'-8"	1'-6"	#9 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F13	13'-0" SQ.	2'-0"	#9 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
F12.67X20.67	12'-8" X 20'-8"	1'-10"	#9 REINF. @ 12" O.C. E.W. TOP & BOT.	9'-8"
ELEV	12'-8" X 20'-4"	1'-2"	#6 REINF. @ 12" O.C. E.W. BOT.	9'-8" *
WF2	2'-0" CONT.	1'-0"	(3) #5 REINF. CONT. BOT.	9'-8"
WF3.33	3'-4" CONT.	1'-0"	(4) #5 REINF. CONT. BOT.	9'-8"
WF4.5	4'-6" CONT.	1'-0"	(5) #5 REINF. CONT. BOT.	9'-8"
WF5	5'-0" CONT.	1'-0"	(6) #6 REINF. CONT. BOT.	9'-8"
WF6.33	6'-4" CONT.	1'-0"	(6) #6 REINF. CONT. BOT.	9'-8"
WF7.67	7'-8" CONT.	1'-2"	(8) #7 REINF. CONT. BOT.	9'-8"
WF8	8'-0" CONT.	1'-2"	(9) #7 REINF. CONT. W/ #5 TRANS. @ 24" O.C. BOT.	9'-8"
WF8.5	8'-6" CONT.	1'-2"	(9) #7 REINF. CONT. W/ #5 TRANS. @ 24" O.C. BOT.	9'-8"
TEI	1'-4" CONT.	1'-4"	(2) #5 REINF. @ BOT.	100'-0"

* VERIFY DEPTH OF ELEVATOR PIT W/ ELEV. MANUF.

TENSION REINF. LAP SPLICE SCHEDULE WALLS, SLABS & FOOTING

BAR SIZE	LAP SPLICE LENGTH (in)	
	TOP	OTHERS
#3	22"	17"
#4	29"	22"
#5	37"	28"
#6	43"	33"
#7	63"	49"
#8	72"	55"
#9	81"	62"
#10	90"	69"

fy=60,000 psi, f'c > 3,000 psi, UNCOATED REINF.

1. REINFORCING BARS CLASSIFIED AS TOP BARS WHEN MORE THAN 12" OF CONCRETE IS CAST BENEATH RESPECTIVE REINFORCING BARS.

MAS. REINF. LAP SPLICE SCHEDULE (TENSION & COMPRESSION)

BAR SIZE	LAP SPLICE LENGTH (in)
48 x (BAR DIA.)	
#3	18"
#4	24"
#5	30"
#6	36"
#7	42"
#8	48"
#9	54"
#10	60"

f'm = 1500 psi, fy=60 ksi

COMPRESSION REINF. LAP SPLICE SCHEDULE

BAR SIZE	LAP SPLICE LENGTH (in)
30 x (BAR DIA.)	
#3	18"
#4	24"
#5	25"
#6	36"
#7	42"
#8	48"
#9	54"
#10	60"

fy=60,000 psi, f'c > 3,000 psi

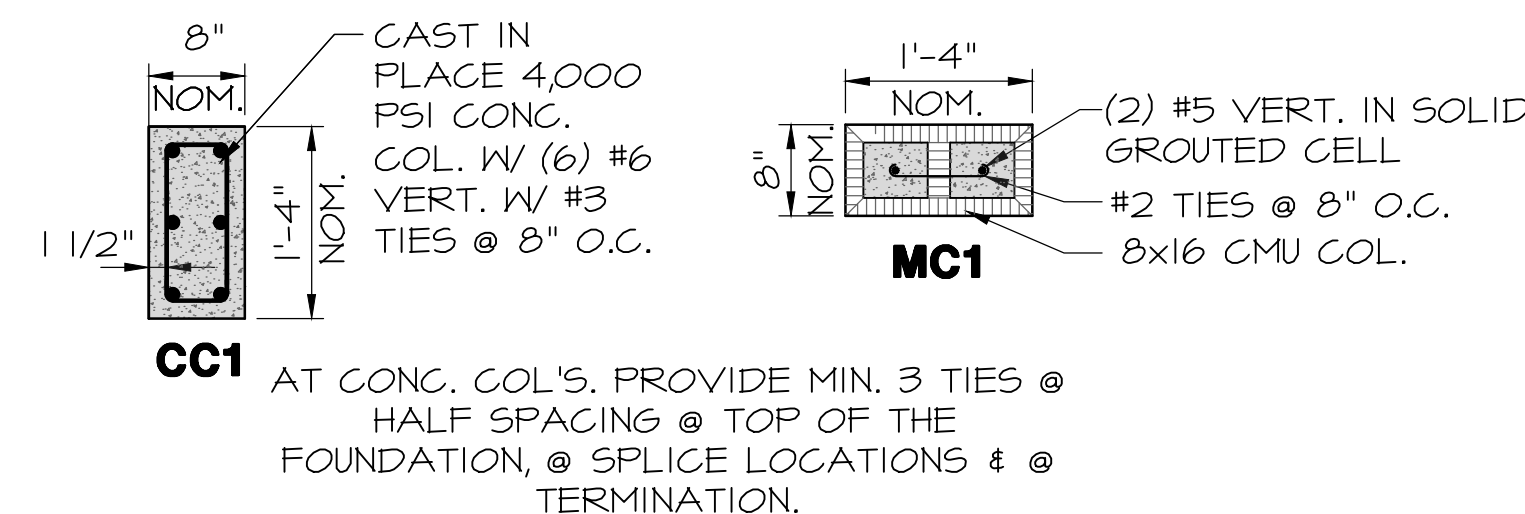
NOTE:

- 1ST FLOOR WALLS USE #5 VERT. REINF. IN SOLID GROUTED MASONRY CELLS @ 24" O.C. IN WALLS PERP. TO HOLLOWCORE PLANKS. (UNLESS NOTED OTHERWISE.)
- COORDINATE MASONRY OPENINGS WITH ARCH DRWS. FOR SIZE AND LOCATIONS.
- COORDINATE CHASES W/ M.E.P. AND ARCH. DRAWINGS FOR SIZE & LOCATIONS

NOTE:

USE A SPECIFIED COMPRESSIVE STRENGTH OF MASONRY f'm = (2,500 PSI) FOR ALL FIRST FLOOR THRU THIRD FLOOR MASONRY WALLS.

MASONRY & CONCRETE COLUMN SCHEDULE



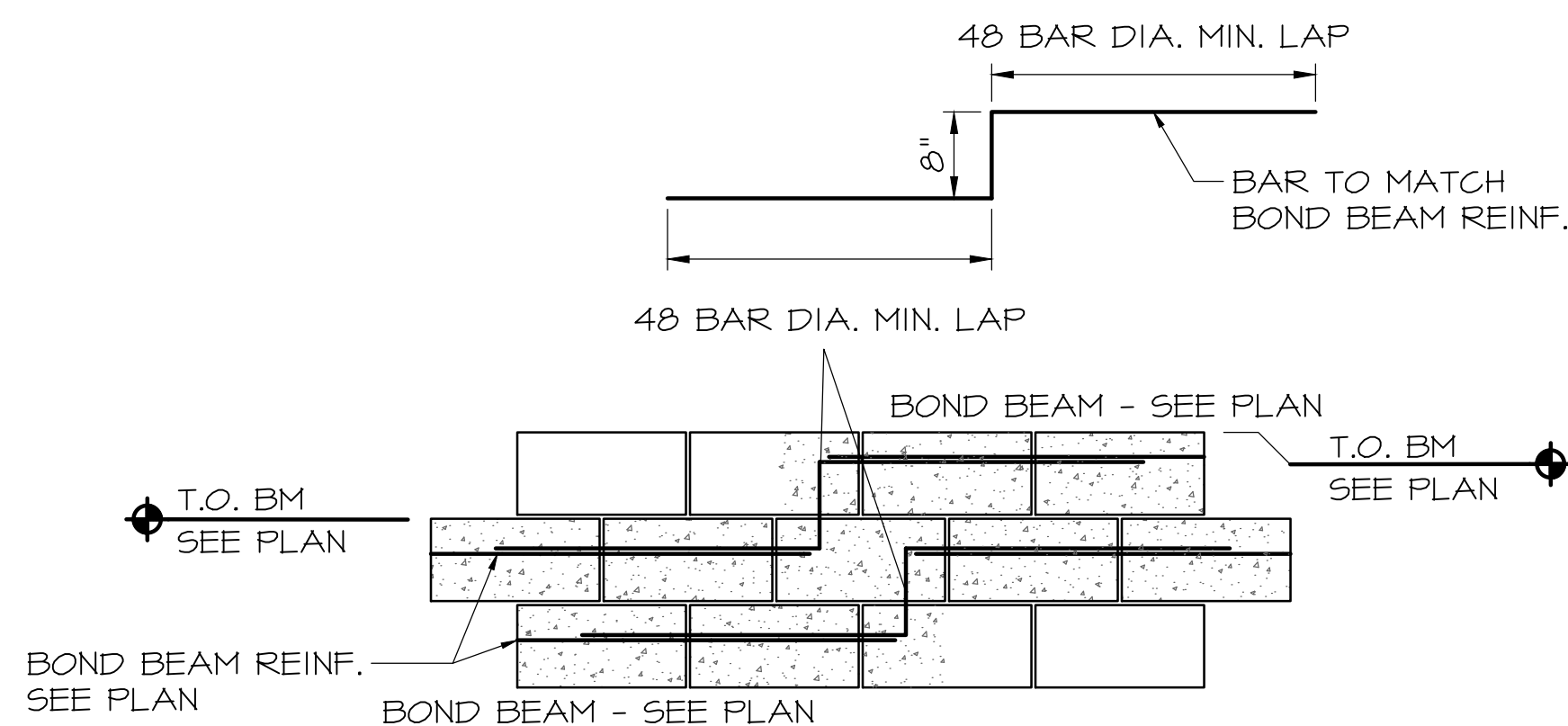
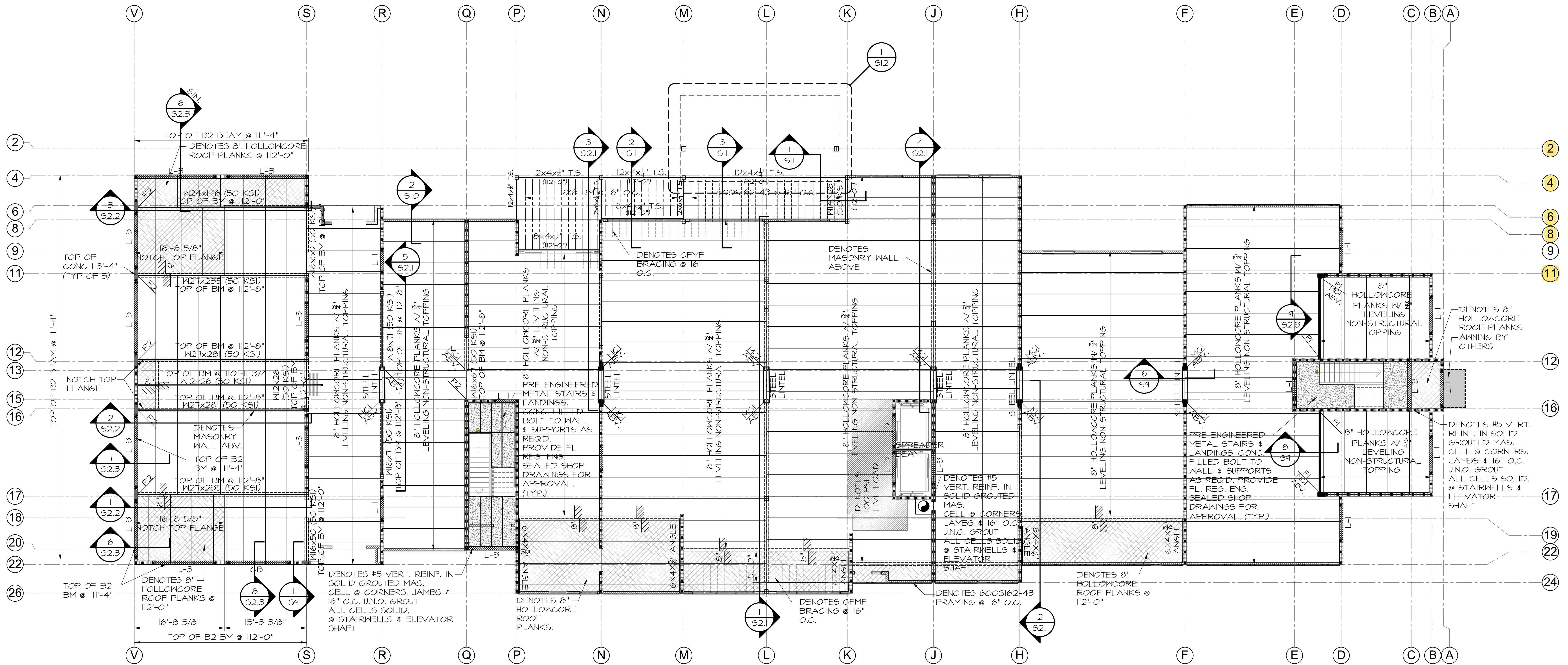
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HB ASSOCIATES, LLC
STRUCTURAL/CIVIL ENGINEERING
FLORIDA CA # 7045
HARRY BRUMLEY III, PE # 41837

377 MAITLAND AVENUE, SUITE # 2007
ALTAMONTE SPRINGS, FL 32701-5442 (407) 740-5444



2 BOND BEAM ELEV. CHANGE
SCALE: 3/4" = 1'-0"

PRECAST LINTEL SAFE LOAD SCHEDULE					
MARK	DESIGNATION	GRAVITY (#/FT)	UPLIFT (#/FT)	MAX. CLEAR SPAN	REMARKS
L-1	8F16-1B/1T	1366		10'-8"	SEE SECTION
L-3	8F24-1B/1T	2423		10'-8"	SEE SECTION

NOTES:
1. ALL LINTELS BY CAST-CRETE OR APPROVED EQUAL.
2. OTHER LINTELS ACCEPTABLE BASED UPON CAPACITIES STATED IN LINTEL SCHEDULE.
3. SEE MANUFACTURER'S SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

F=FILLED WITH GROUT / U=UNFILLED

QUANTITY OF #5 REBAR AT BOTTOM OF LINTEL CAVITY

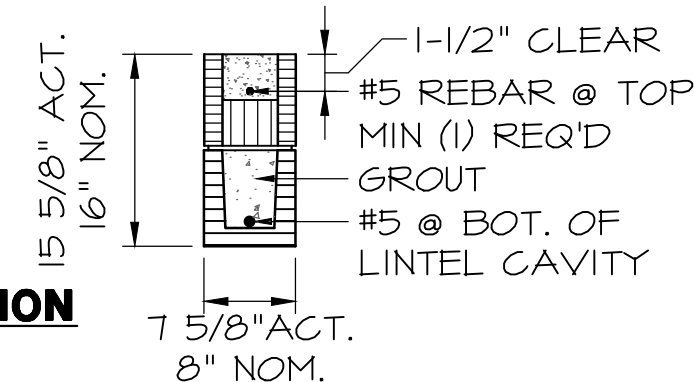
8F16-1B/1T

NOMINAL WIDTH

NOMINAL HEIGHT

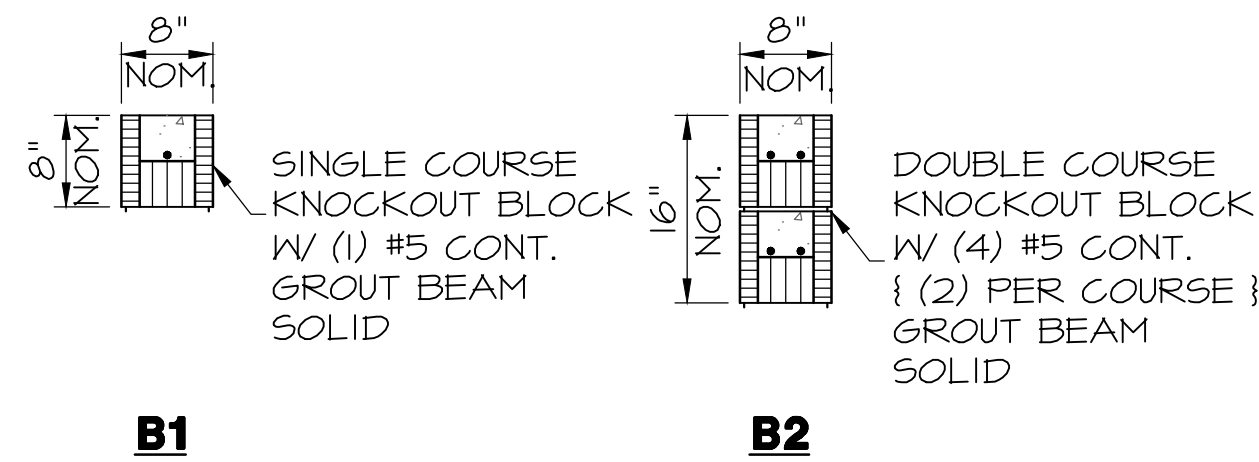
QUANTITY OF #5 REBAR AT TOP

LINTEL TYPE DESIGNATION

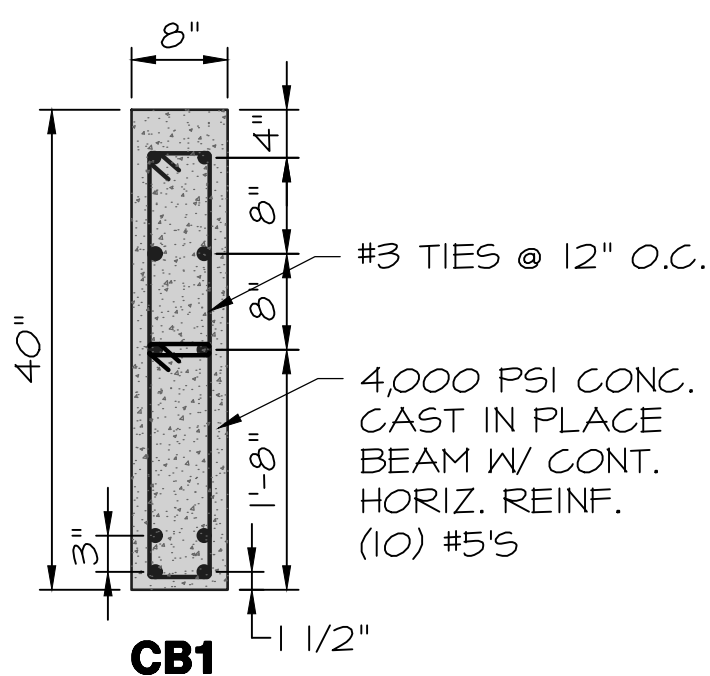


1 2ND FLR FRAMING PLAN
SCALE: 1/8" = 1'-0"

MASONRY BEAM SCHEDULE



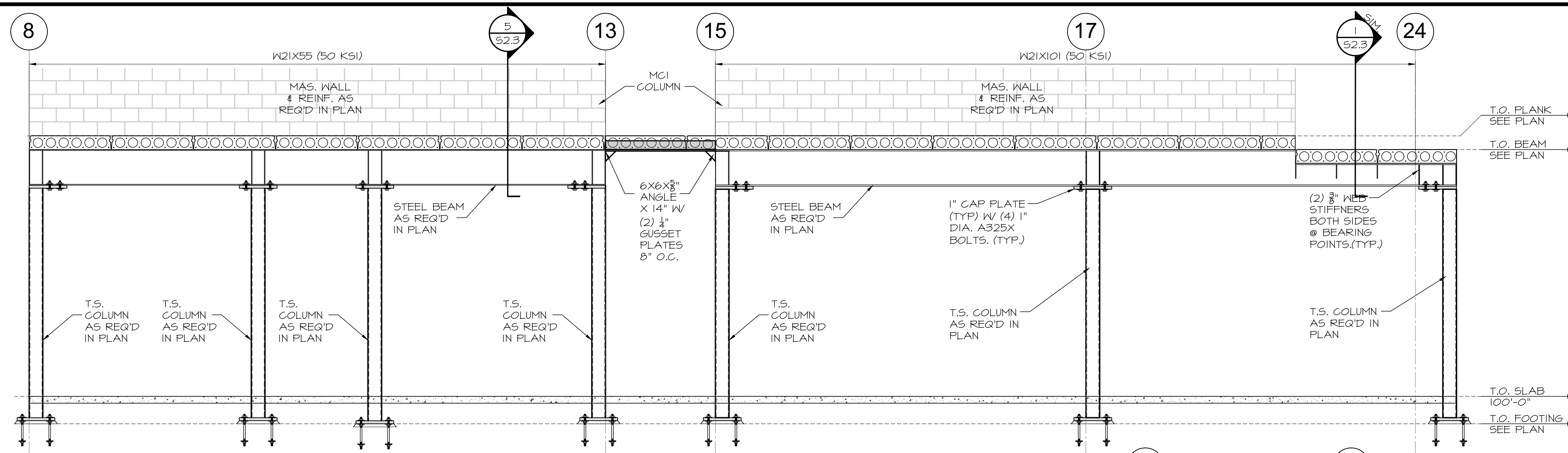
CONCRETE BEAM SCHEDULE



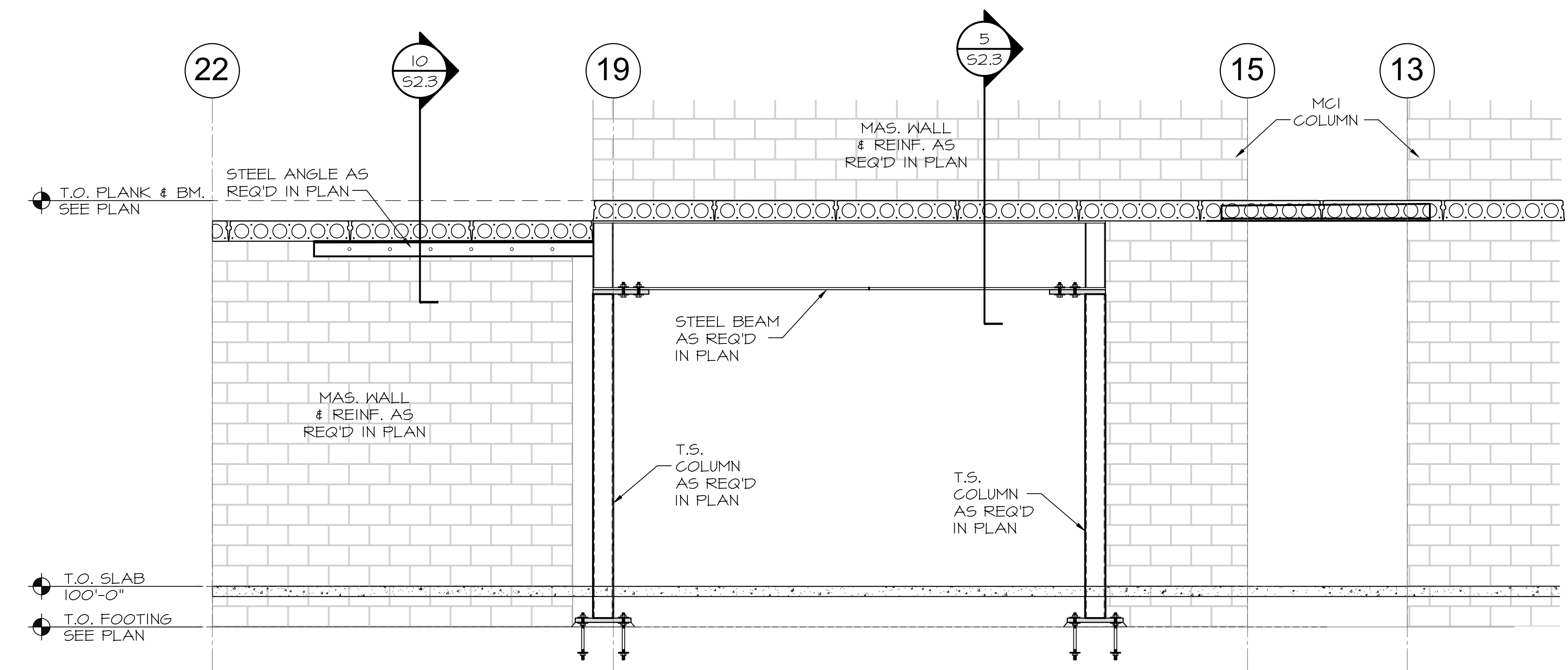
TOP OF 2ND FLR. PLANKS = 112'-8" U.N.O.

NOTE:

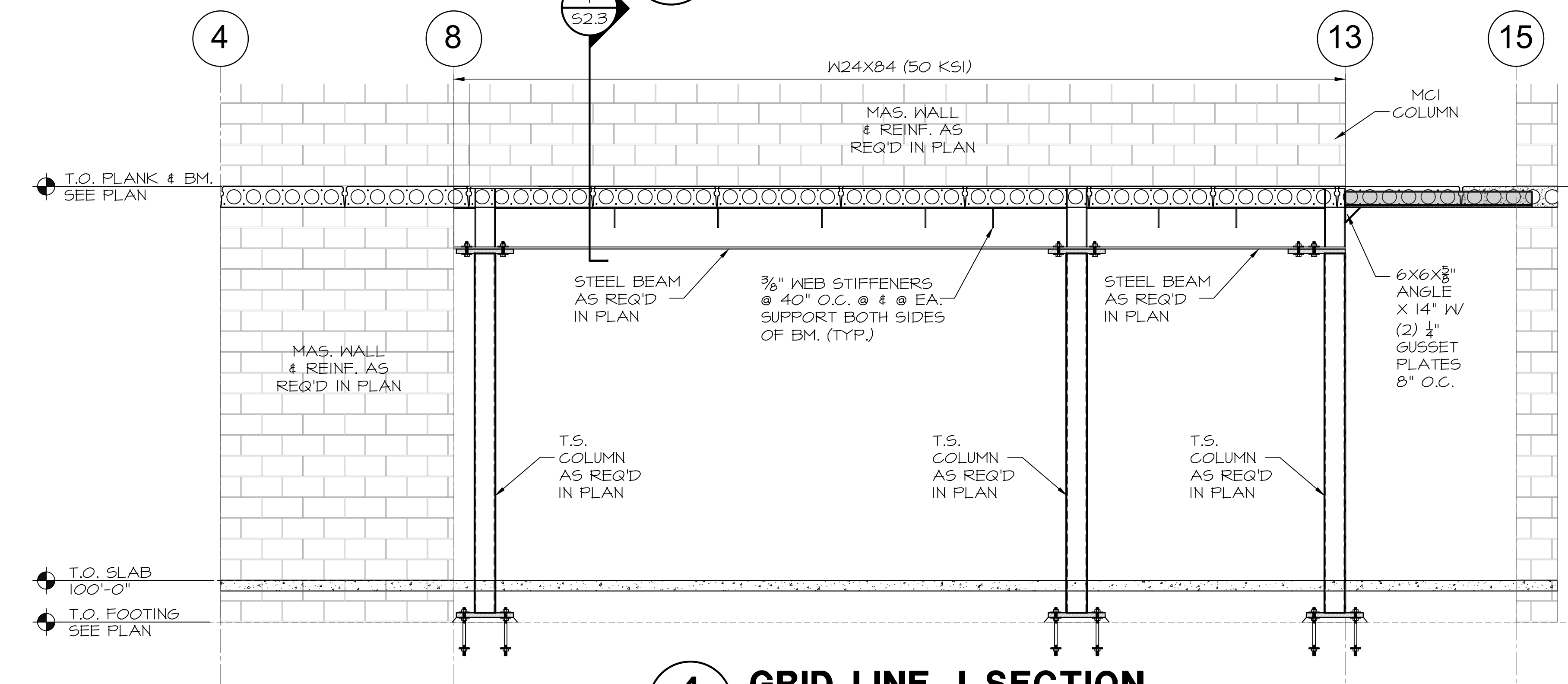
- * TOP OF B2 BEAM @ 112'-0" @ ALL MASONRY WALLS U.N.O.
- * MASONRY WALLS ABOVE 2ND FLR. USE #5 VERT. REINF. IN SOLID GROUTED MASONRY CELLS @ 24" O.C. UNLESS NOTED OTHERWISE.
- * COORDINATE MASONRY OPENINGS WITH ARCH DWGS FOR SIZE AND LOCATIONS.
- * COORDINATE CHASES W/ M.E.P. AND ARCH. DRAWINGS FOR SIZE & LOCATIONS.



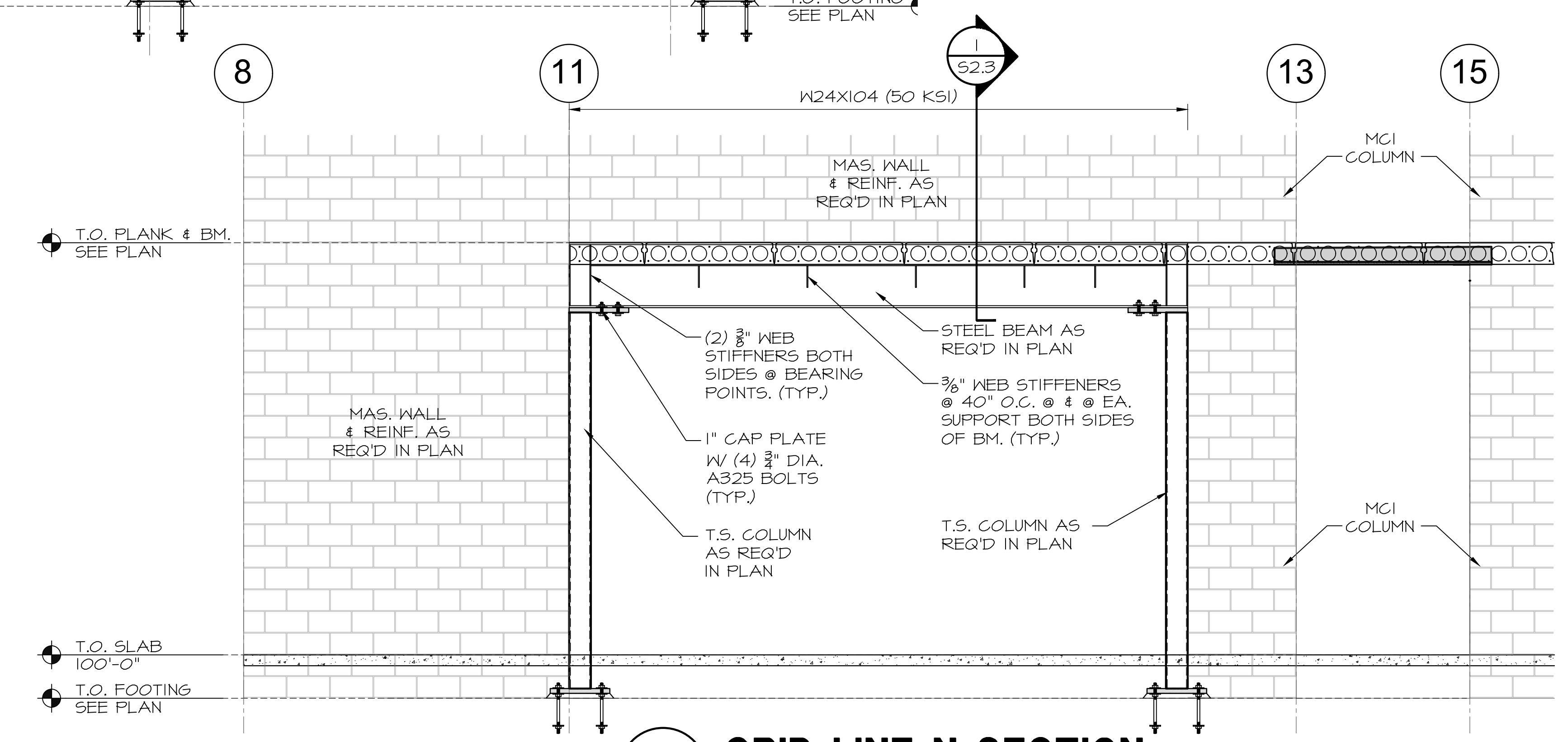
1 GRID LINE L SECTION
S2.1
SCALE: 3/8"=1'-0"



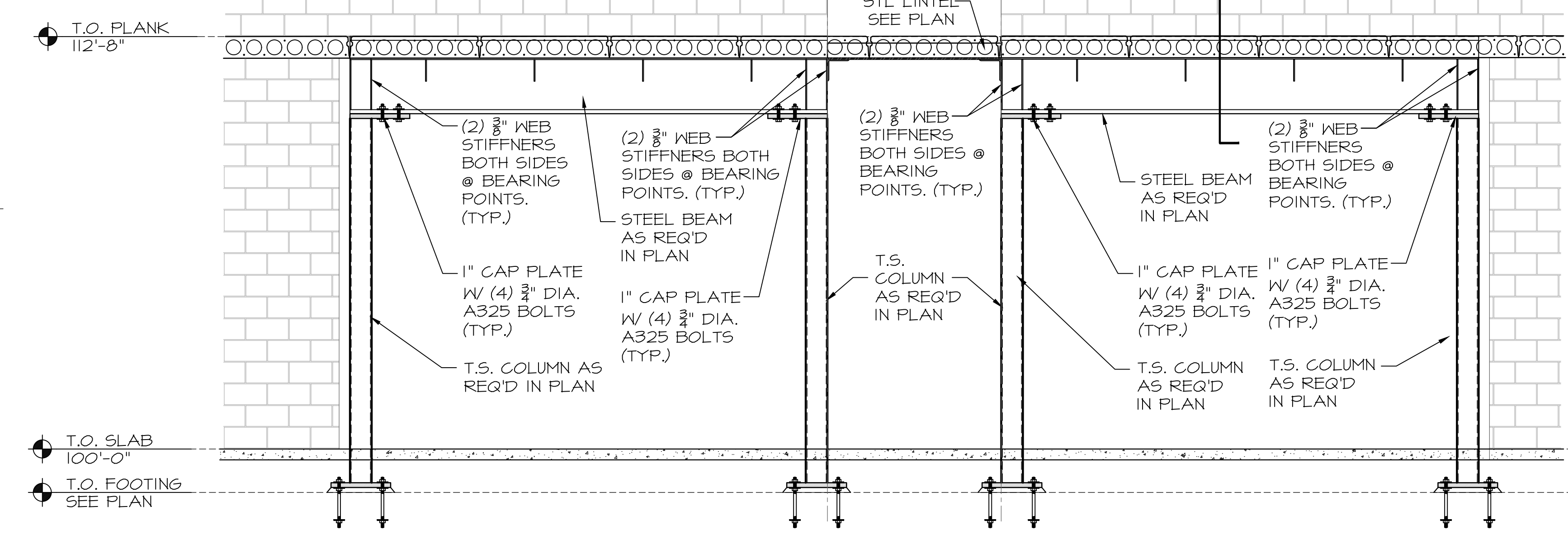
2 GRID LINE H SECTION
S2.1
SCALE: 3/8"=1'-0"



4 GRID LINE J SECTION
S2.1
SCALE: 3/8"=1'-0"



3 GRID LINE N SECTION
S2.1
SCALE: 3/8"=1'-0"



5 GRIDLINE P SECTION
S2.1
SCALE: 3/8"=1'-0"

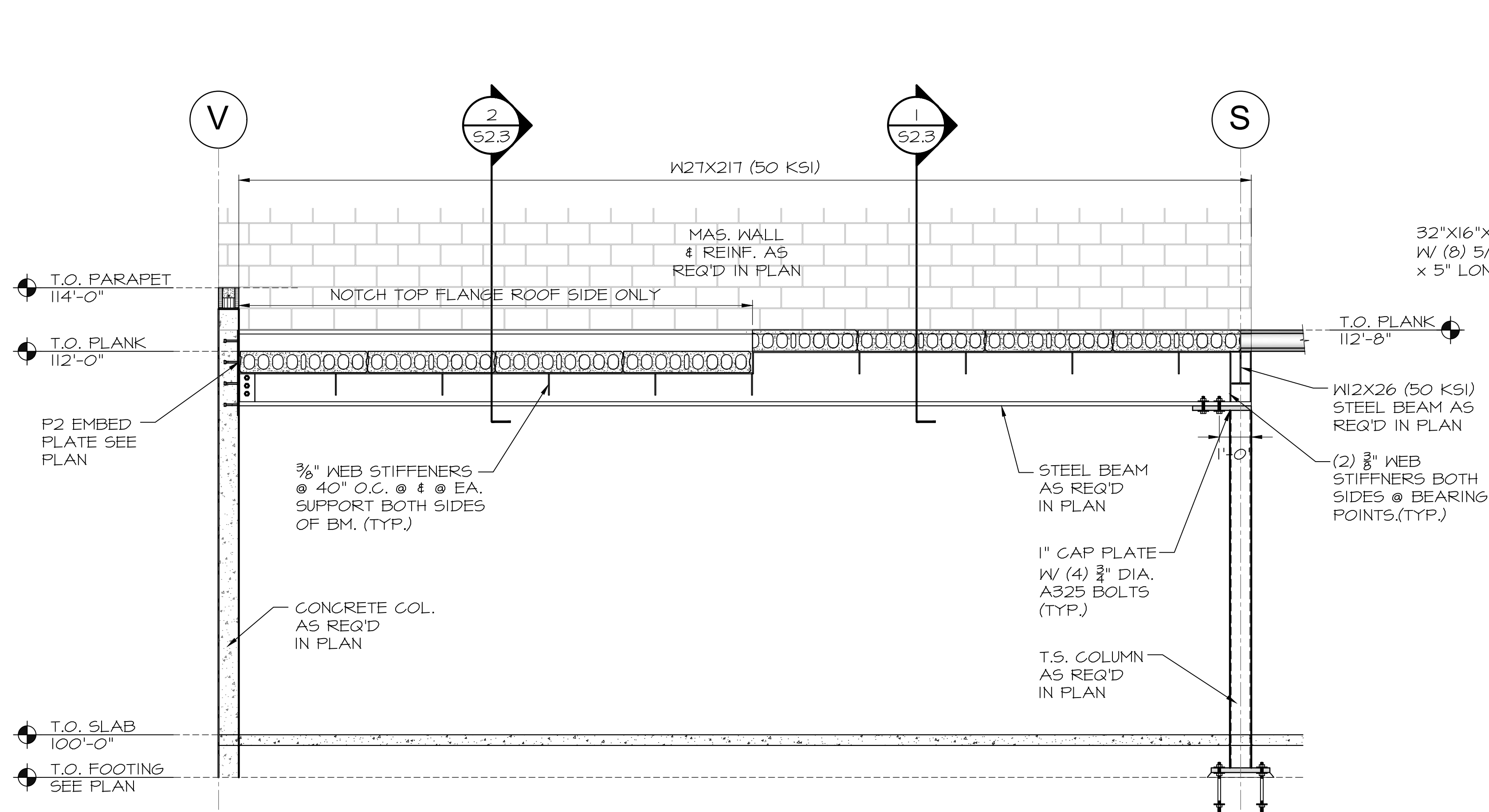
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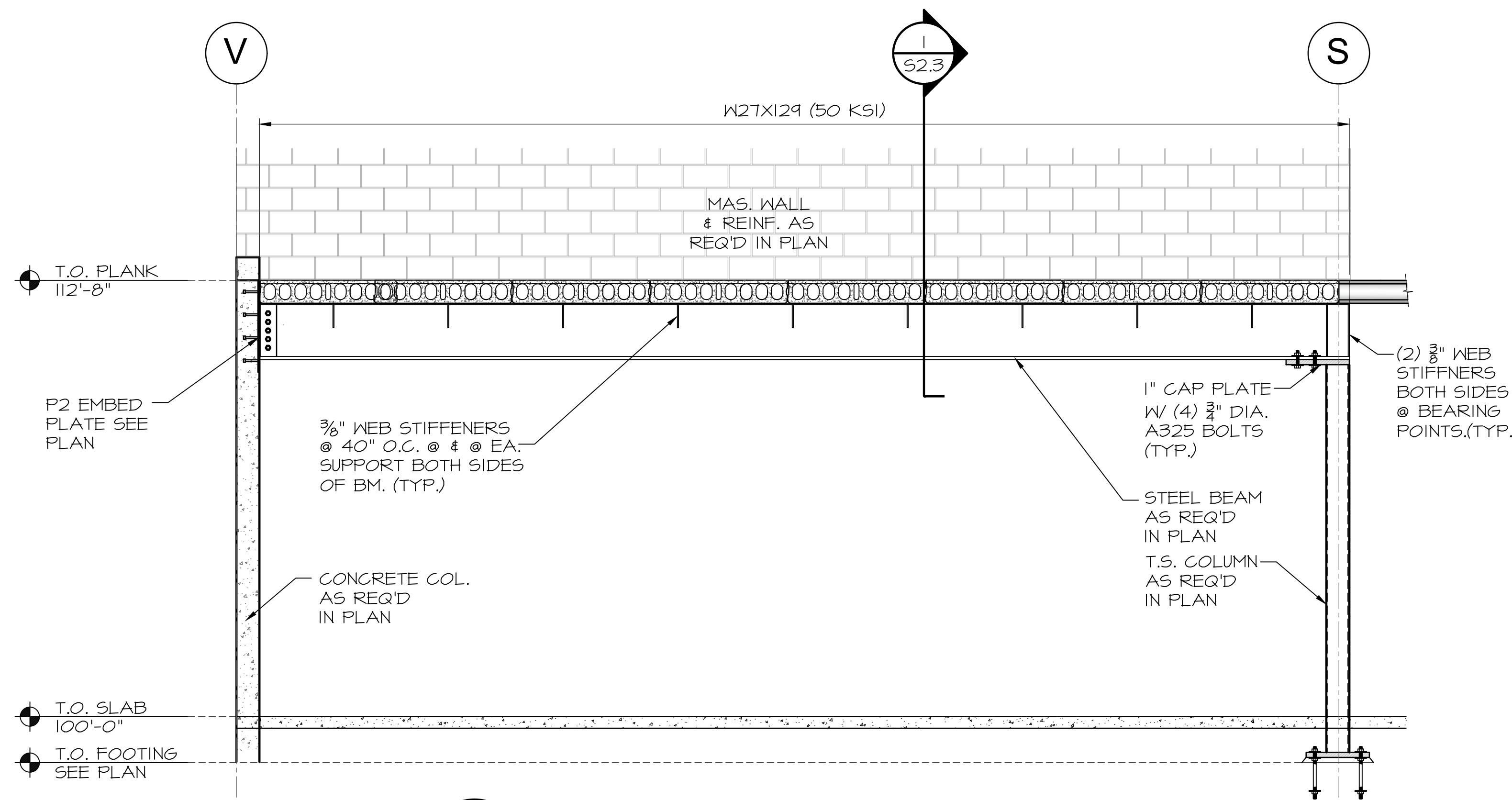
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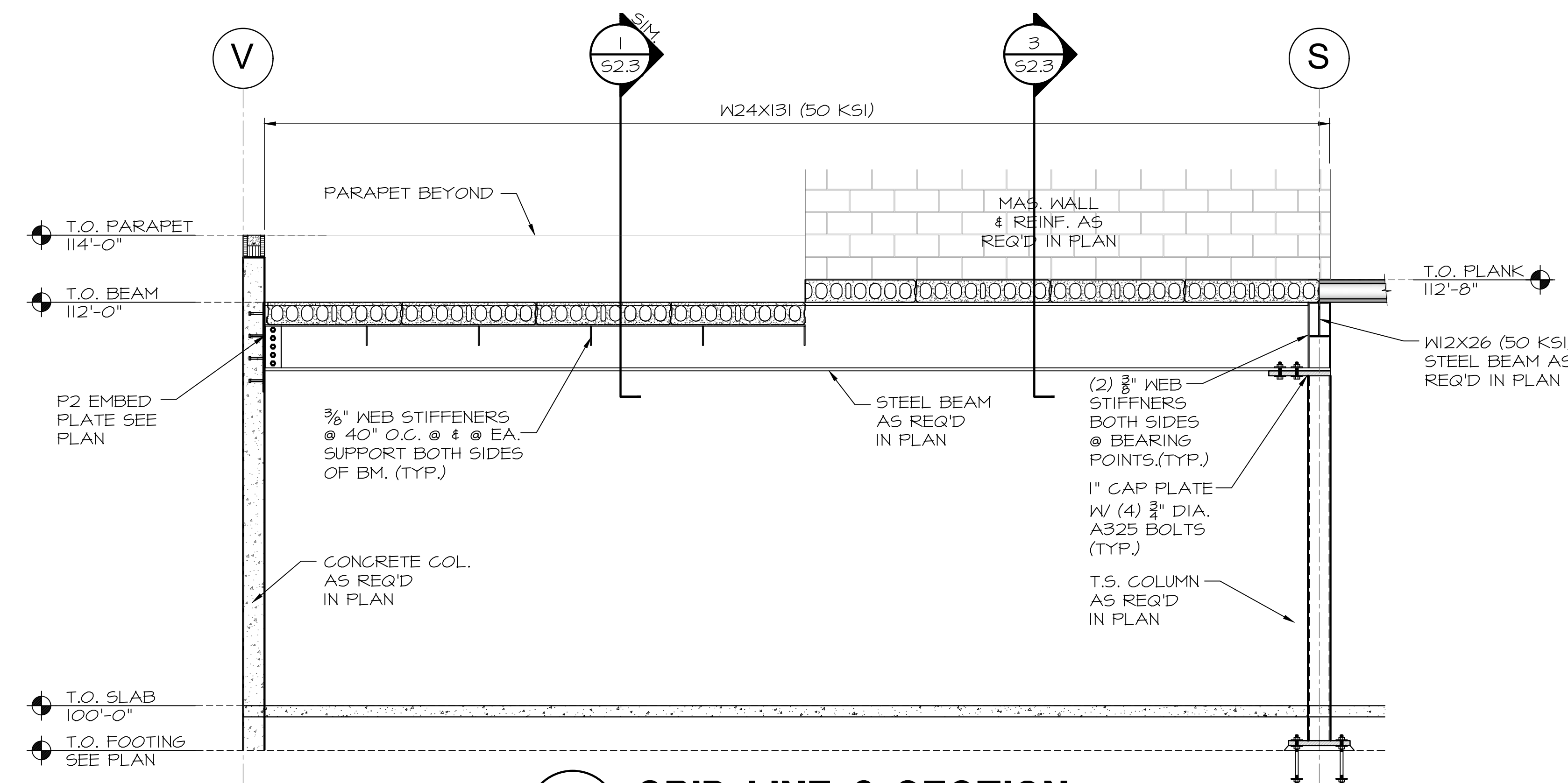
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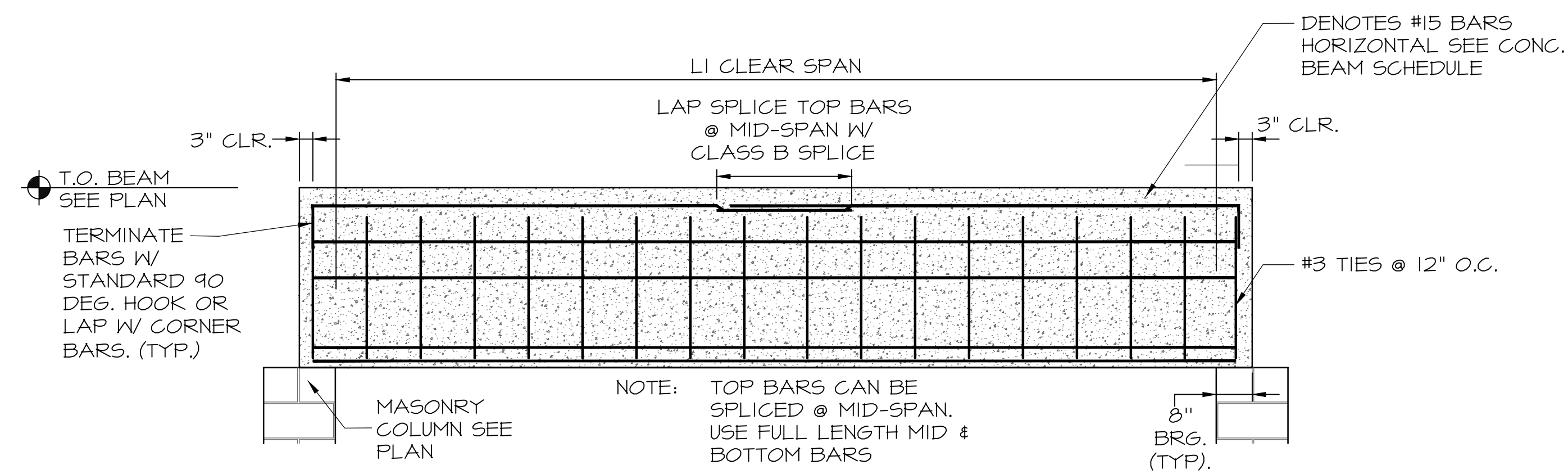
1
S2.2 **GRID LINE 17 & 11 SECTION**
SCALE: 3/8"=1'-0"



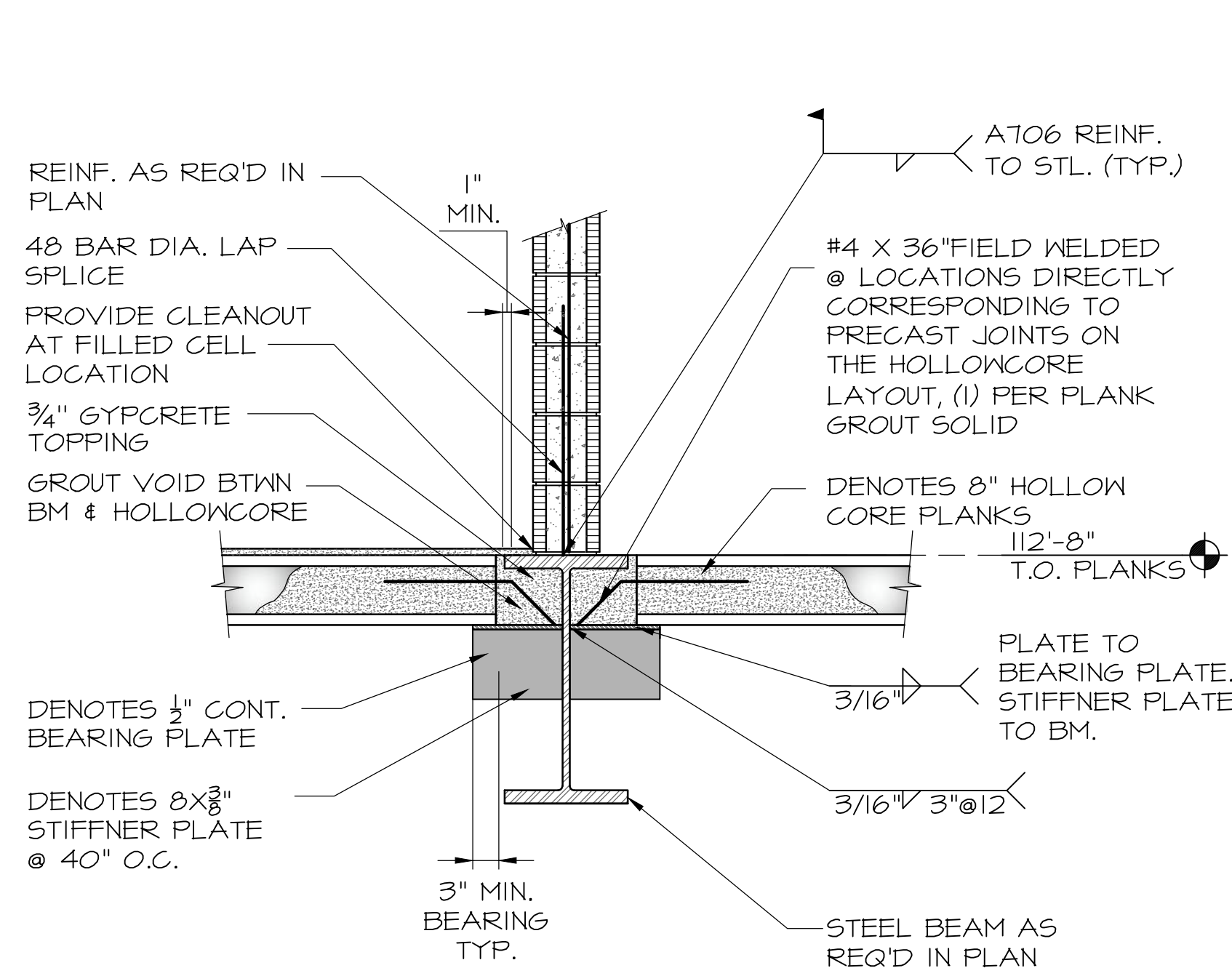
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S2.2 **GRID LINE 16 & 12 SECTION**
SCALE: 3/8"=1'-0"



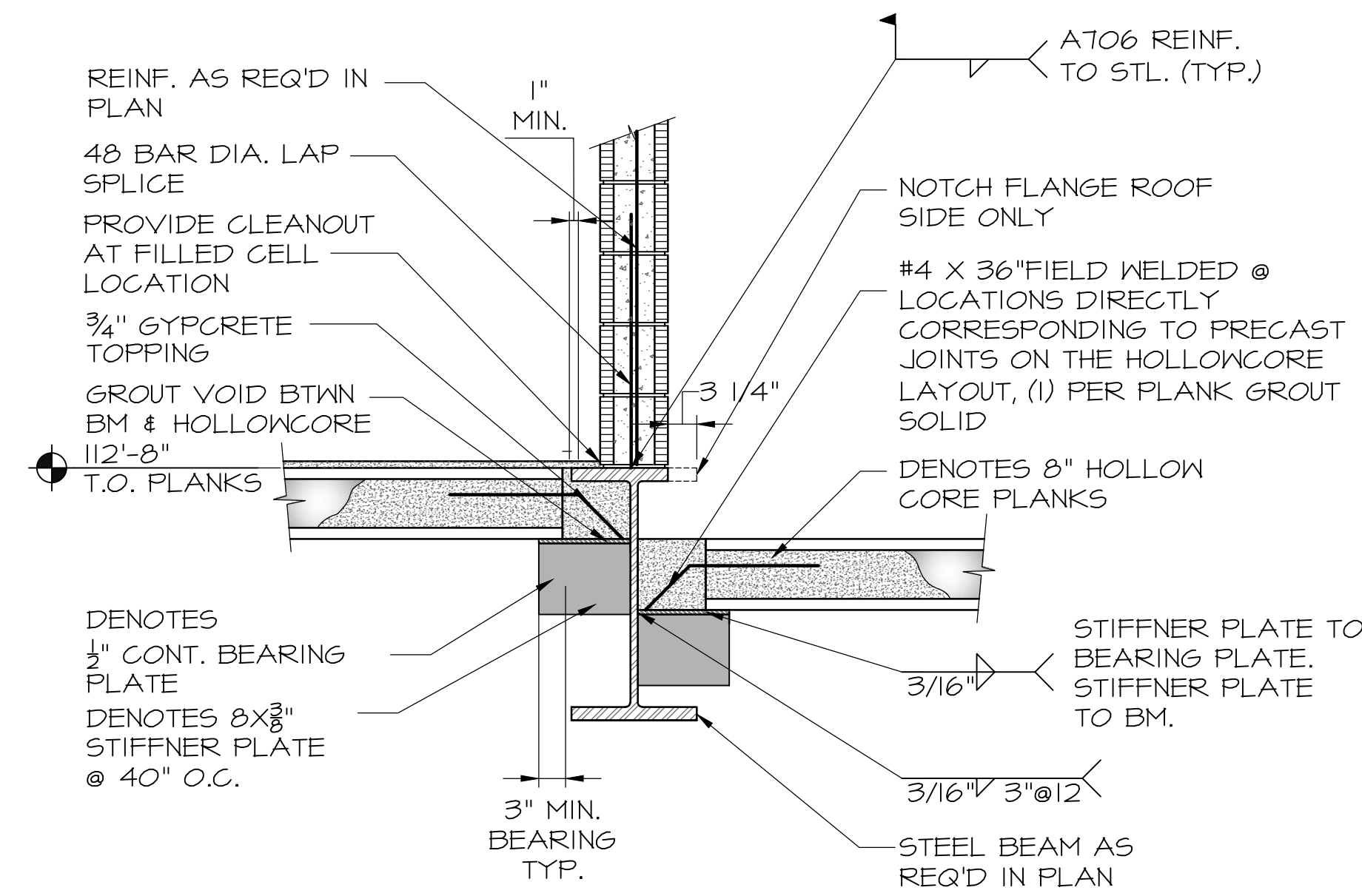
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S2.2 **GRID LINE 6 SECTION**
SCALE: 3/8"=1'-0"



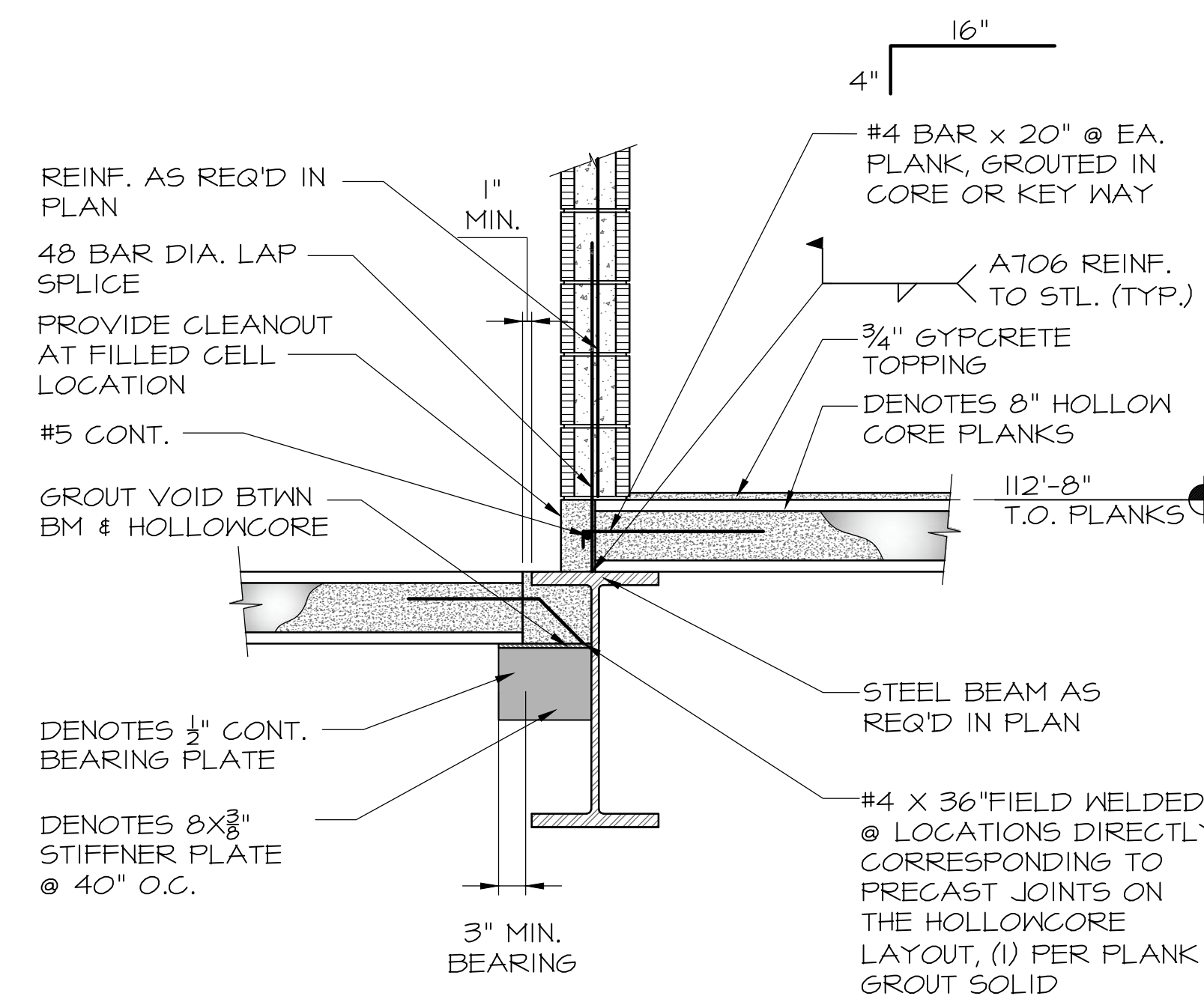
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S2.2 **CONCRETE BM DIAGRAM**
SCALE: 1/2"=1'-0"



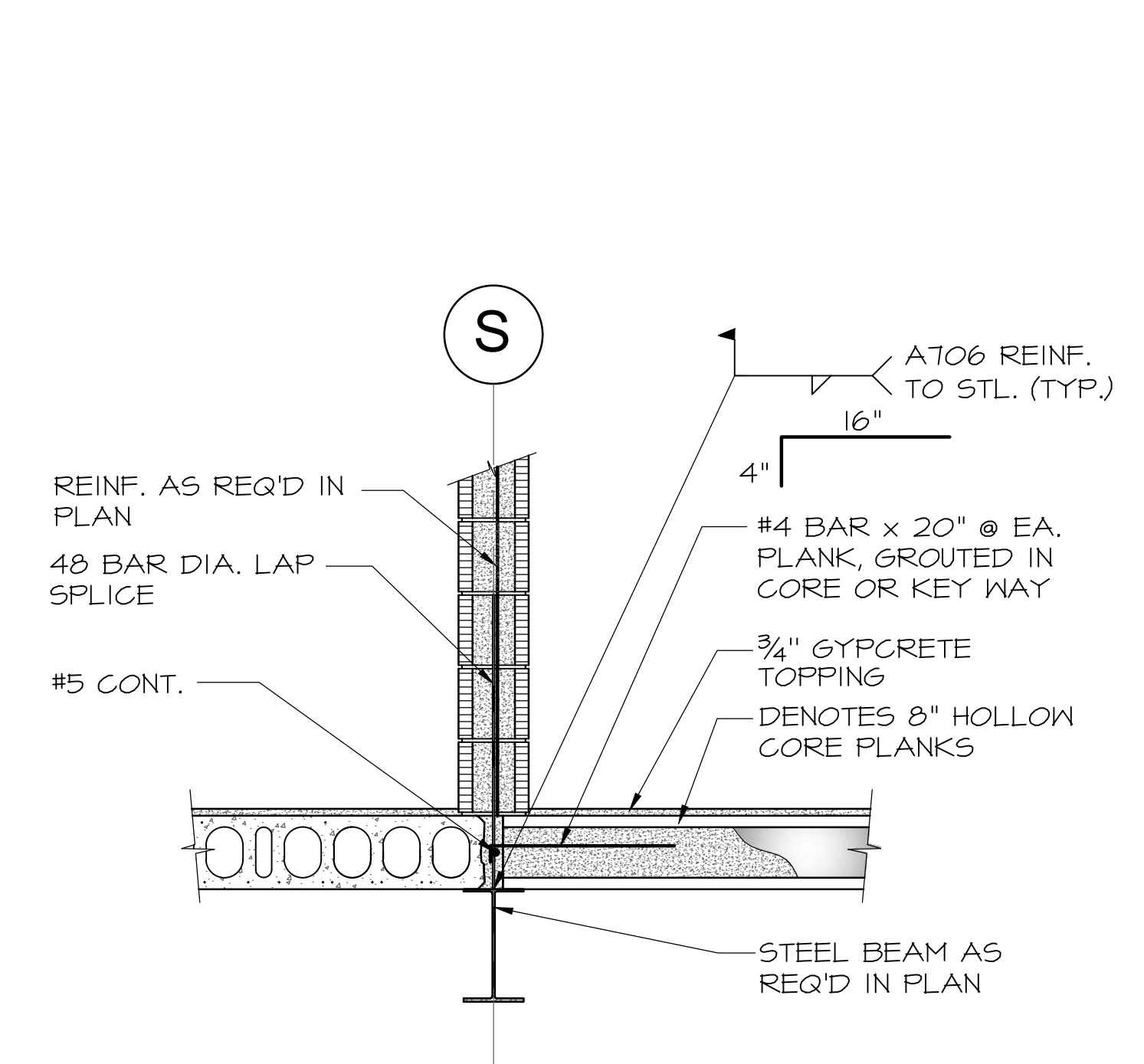
1 INTER. BEARING DETAIL
S2.3 SCALE: 3/4"=1'-0"



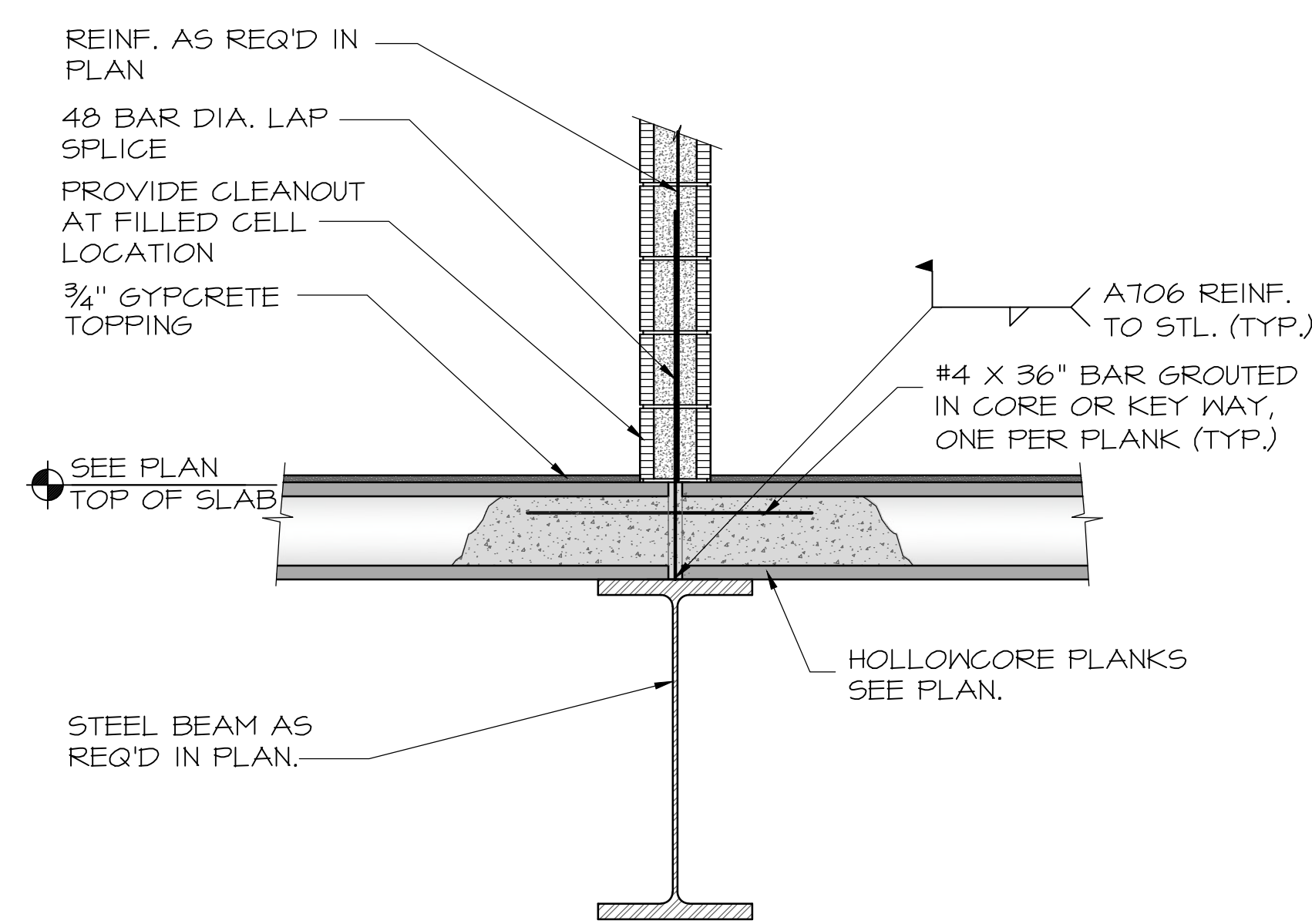
2 INTER. BEARING DETAIL
S2.3 SCALE: 3/4"=1'-0"



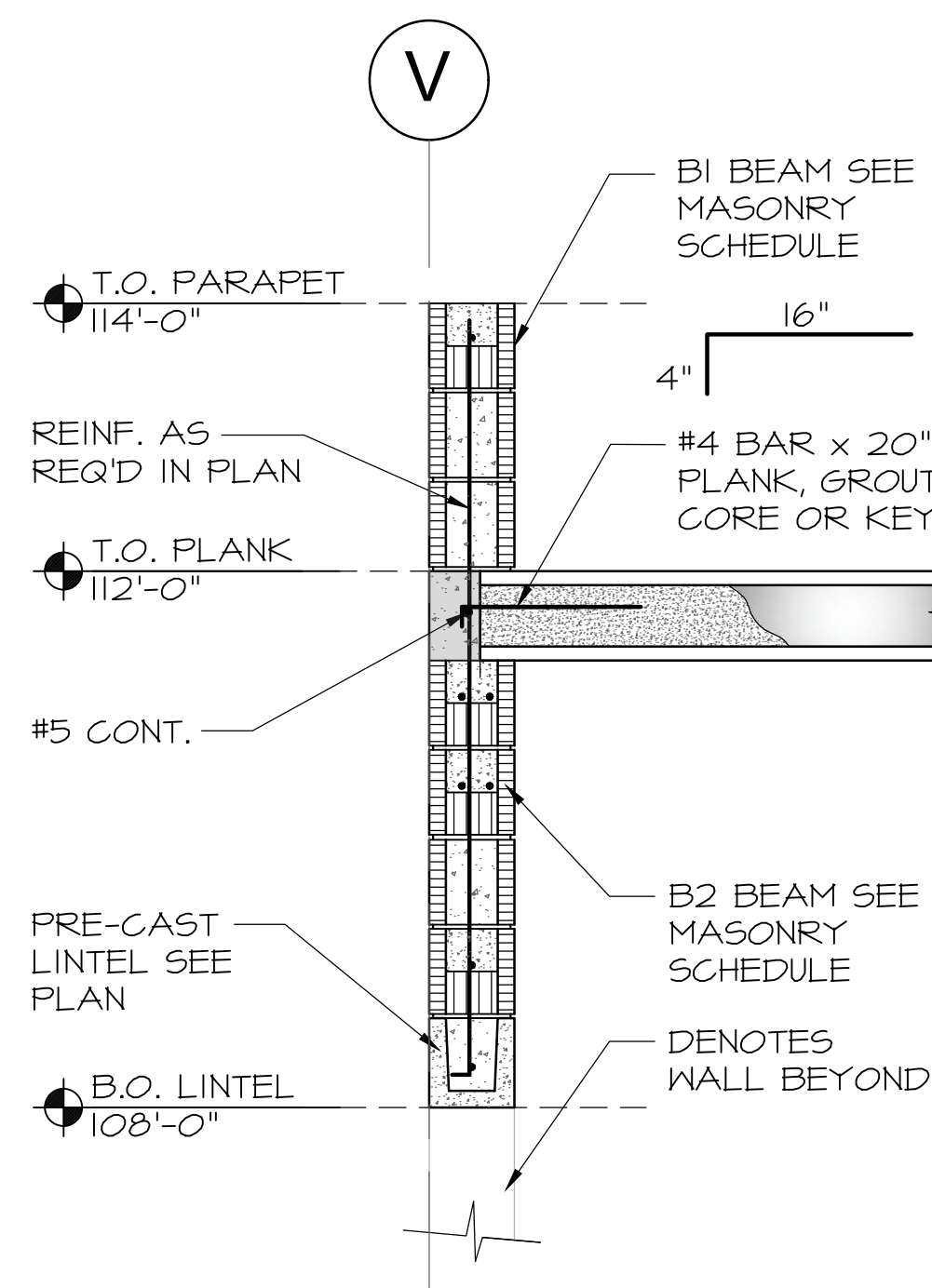
3 INTER. BEARING DETAIL
S2.3 SCALE: 3/4"=1'-0"



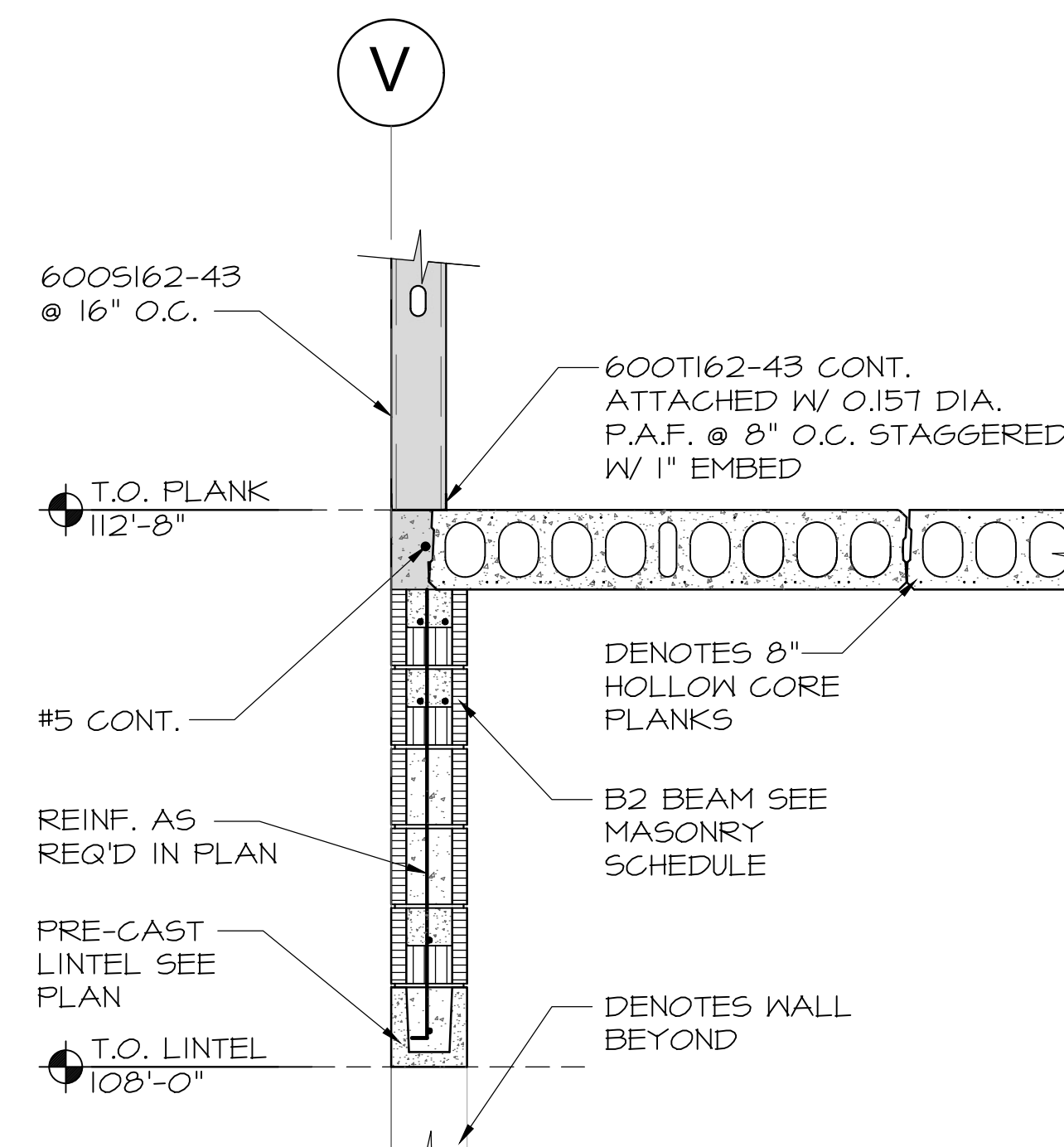
4 INTER. BEARING DETAIL
S2.3 SCALE: 3/4"=1'-0"



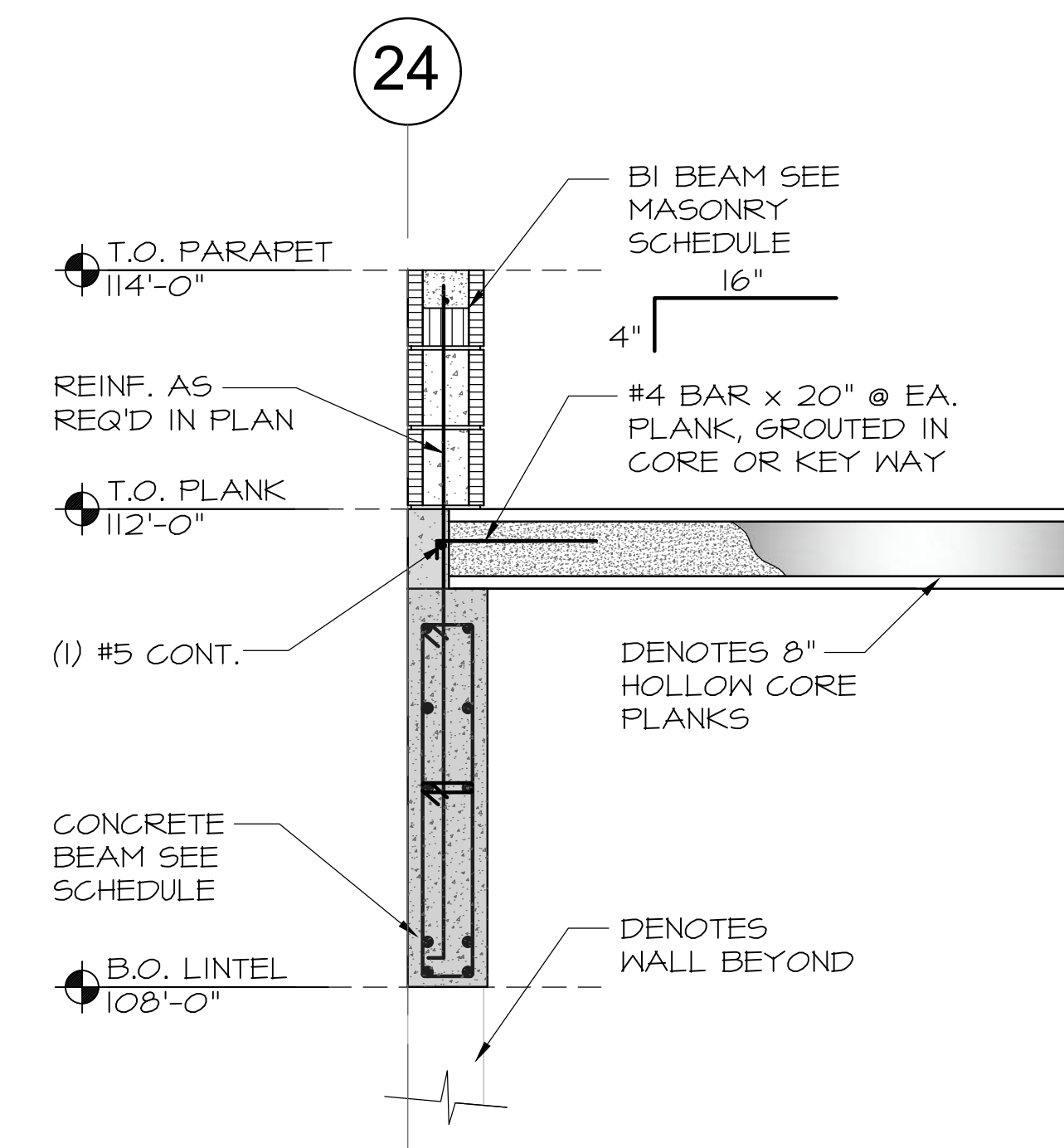
5 HOLLOWCORE PLANK STEEL BM CONNECTION DETAIL
S2.3 SCALE: 3/4"=1'-0"



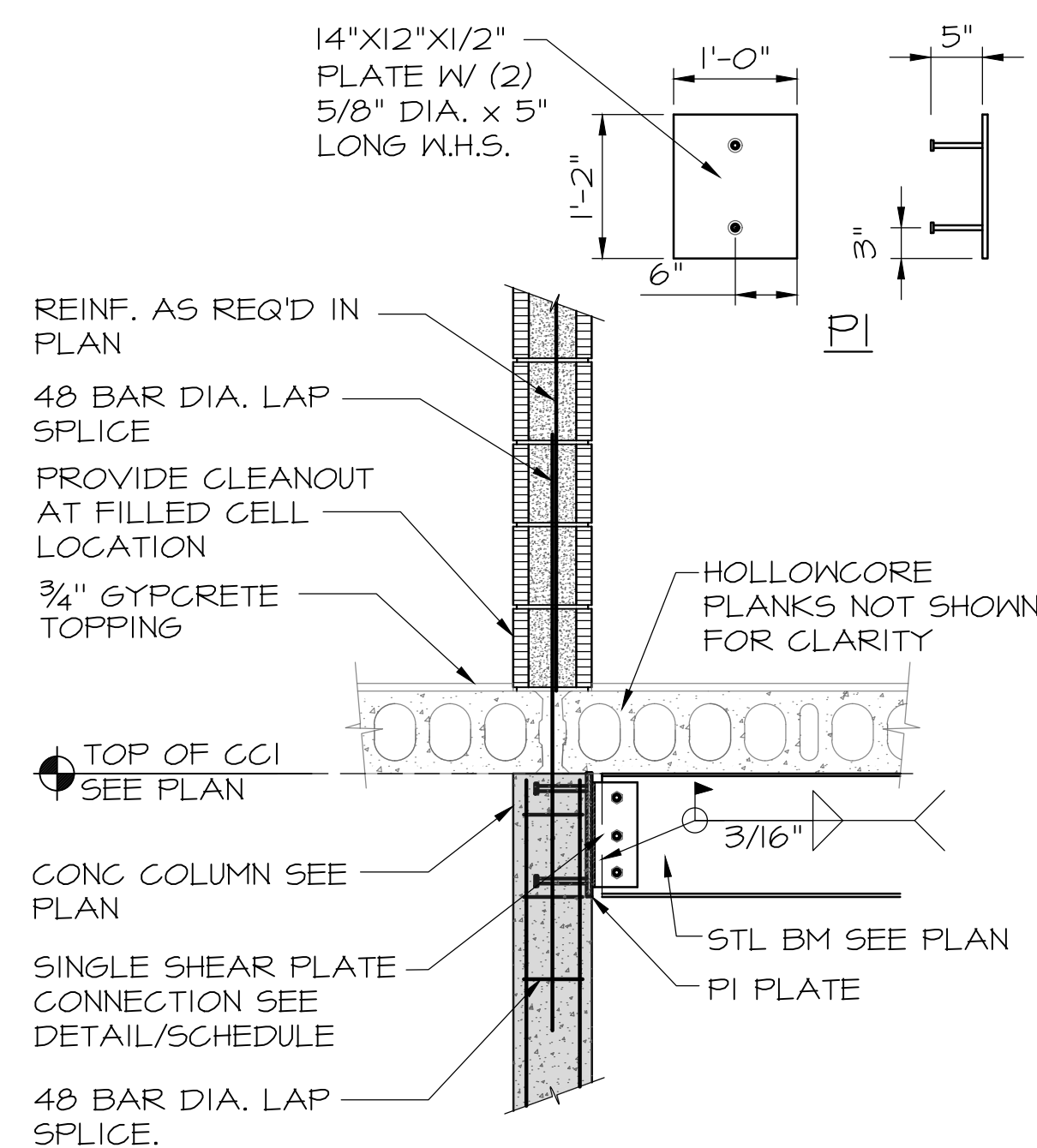
6 PARAPET DETAIL
S2.3 SCALE: 3/4"=1'-0"



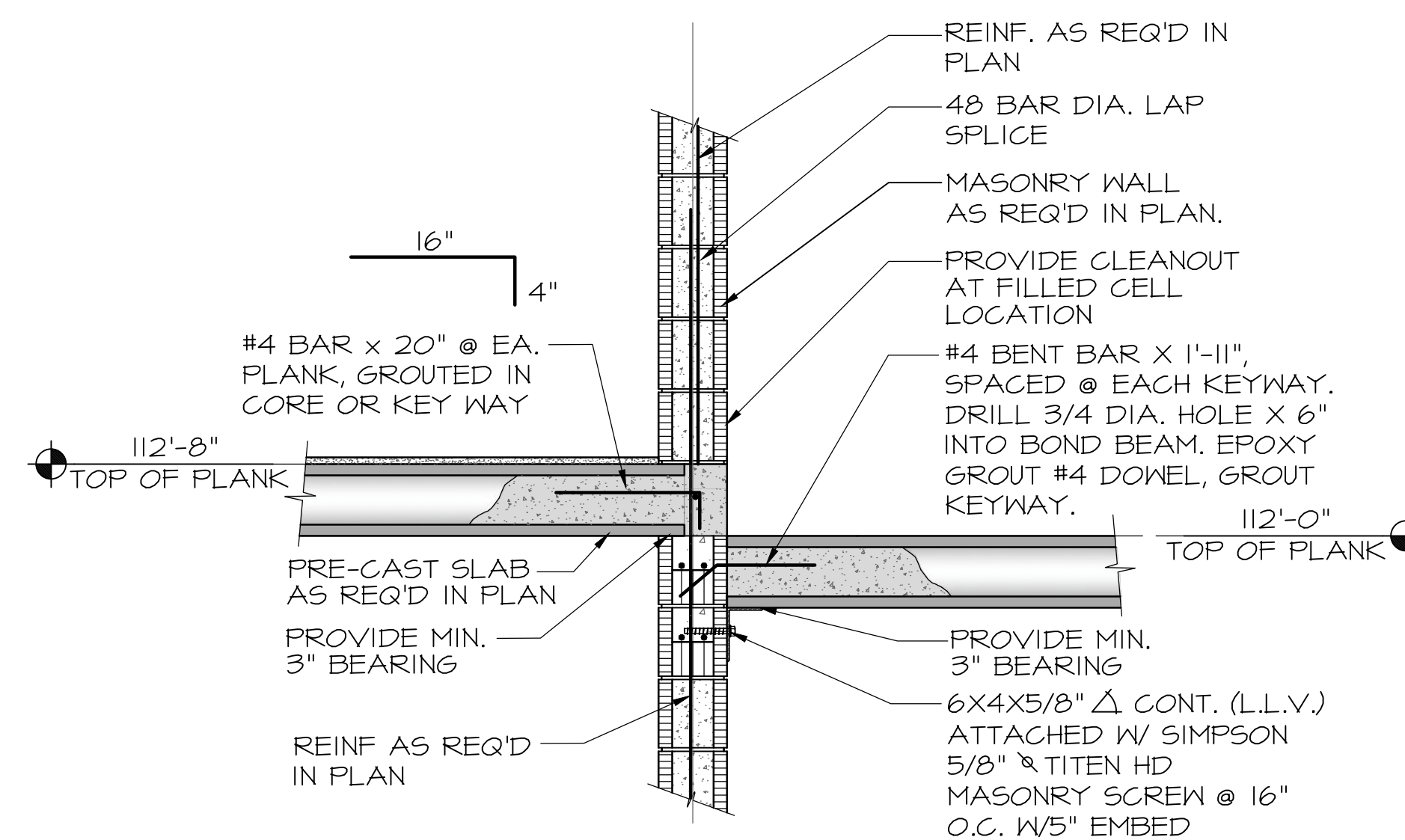
7 WALL DETAIL
S2.3 SCALE: 3/4"=1'-0"



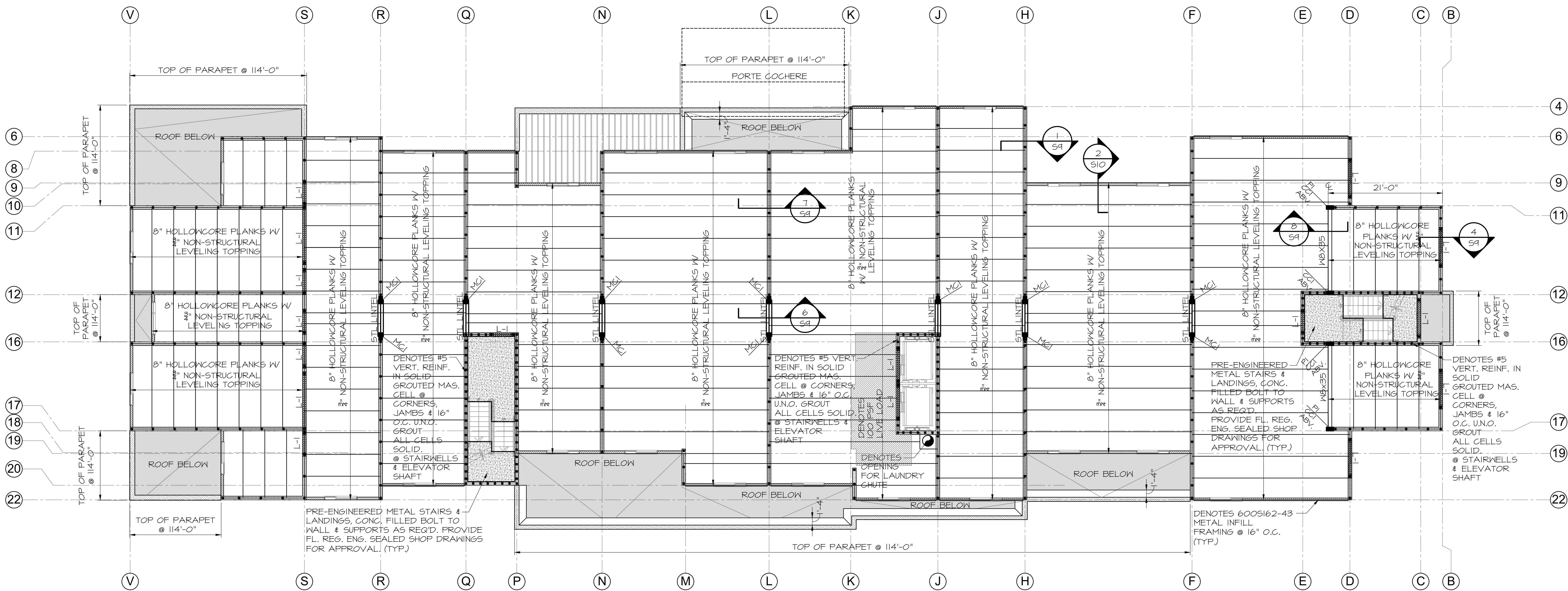
8 PARAPET DETAIL
S2.3 SCALE: 3/4"=1'-0"



9 SECTION
S2.3 SCALE: 3/4"=1'-0"



10 EXTER. BRG DETAIL
S2.3 SCALE: 3/4"=1'-0"



1 3RD-5TH FLR FRAM'G PLAN
S3 SCALE: 1/8"=1'-0"

- TOP OF PLANKS 3RD FLOOR = 122'-8"
- TOP OF PLANKS 4TH FLOOR = 132'-8"
- TOP OF PLANKS 5TH FLOOR = 142'-8"

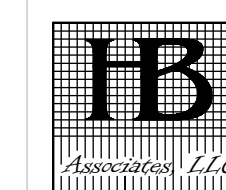
NOTE:

- * TOP OF B2 BEAM @ 122'-0" @ 132'-0" @ 142'-0"
- MASONRY WALLS ABV. 3RD. FLR. USE #5 VERT. REINF. IN SOLID GROUTED MASONRY CELLS @ 48" O.C. IN WALLS PERP. TO HOLLOWCORE PLANKS. (UNLESS NOTED OTHERWISE.)
- COORDINATE MASONRY OPENINGS WITH ARCH DRWS. FOR SIZE AND LOCATIONS.
- COORDINATE CHASES W/ M.E.P. AND ARCH. DRAWINGS FOR SIZE & LOCATIONS

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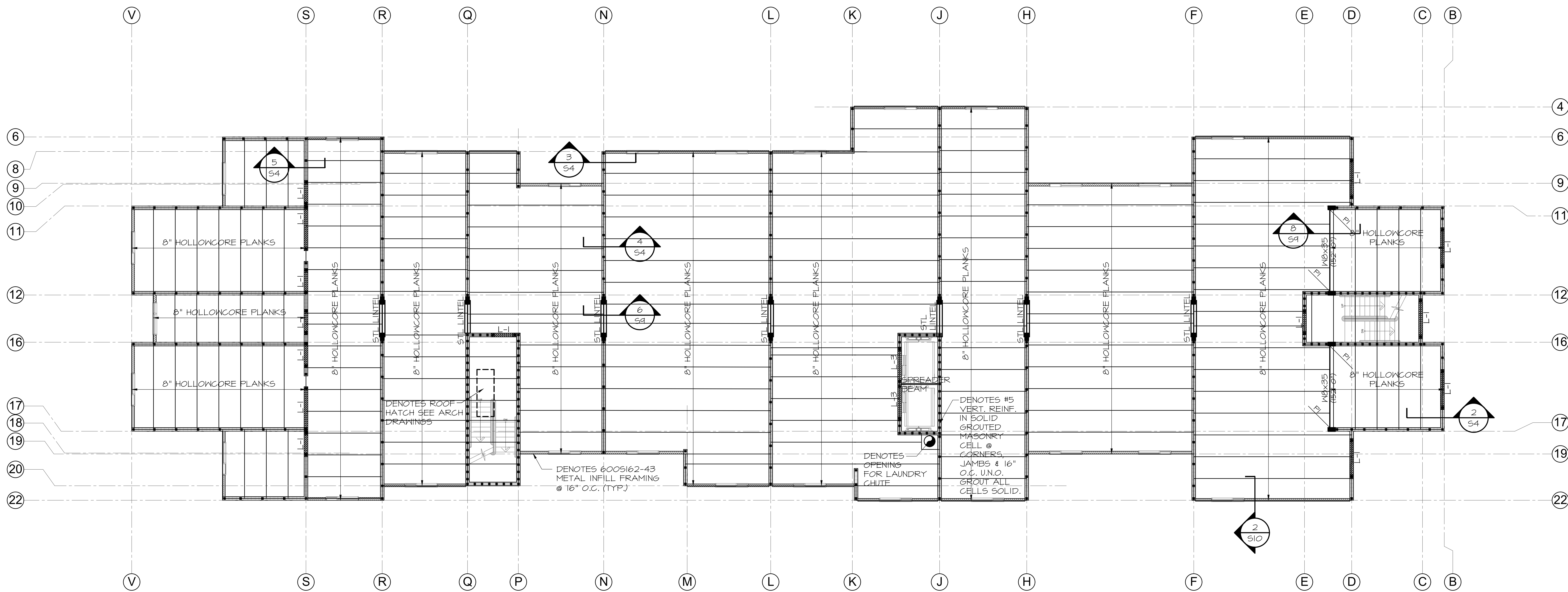
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6 OF



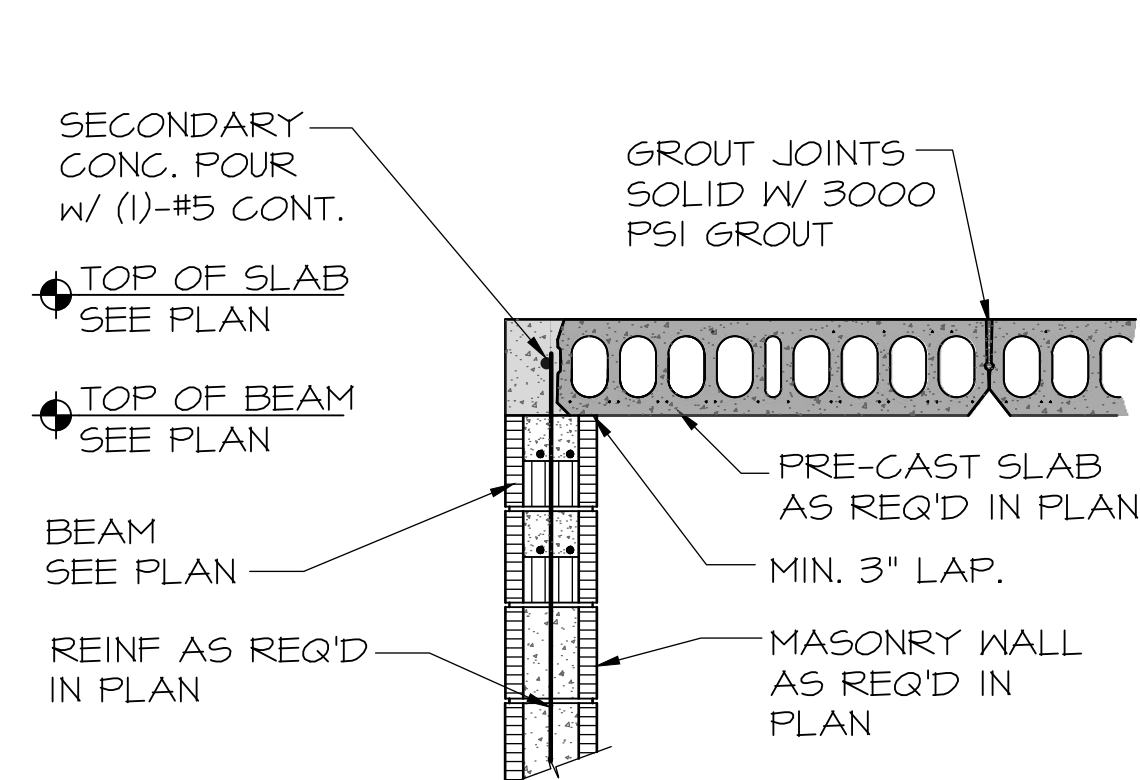
TOP OF PLANKS ROOF = 152'-8"

NOTE:

- * TOP OF B2 BEAM @ 152'-0" @ ALL MASONRY WALLS.
- * COORDINATE CHASES W/ M.E.P. AND ARCH. DRAWINGS FOR SIZE & LOCATIONS.
- * COORDINATE MASONRY OPENINGS WITH ARCH DWGS FOR SIZE AND LOCATIONS.

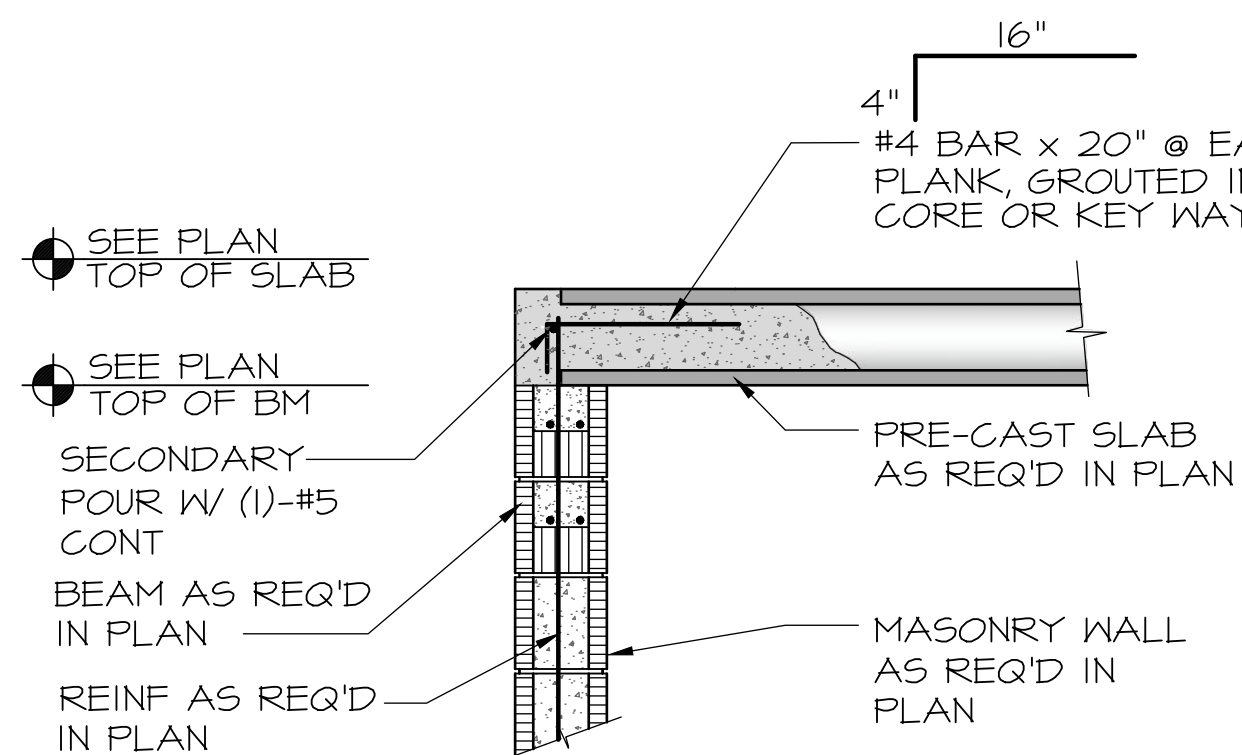
1
S4 **ROOF FRAMING PLAN**

SCALE: 1/8"=1'-0"



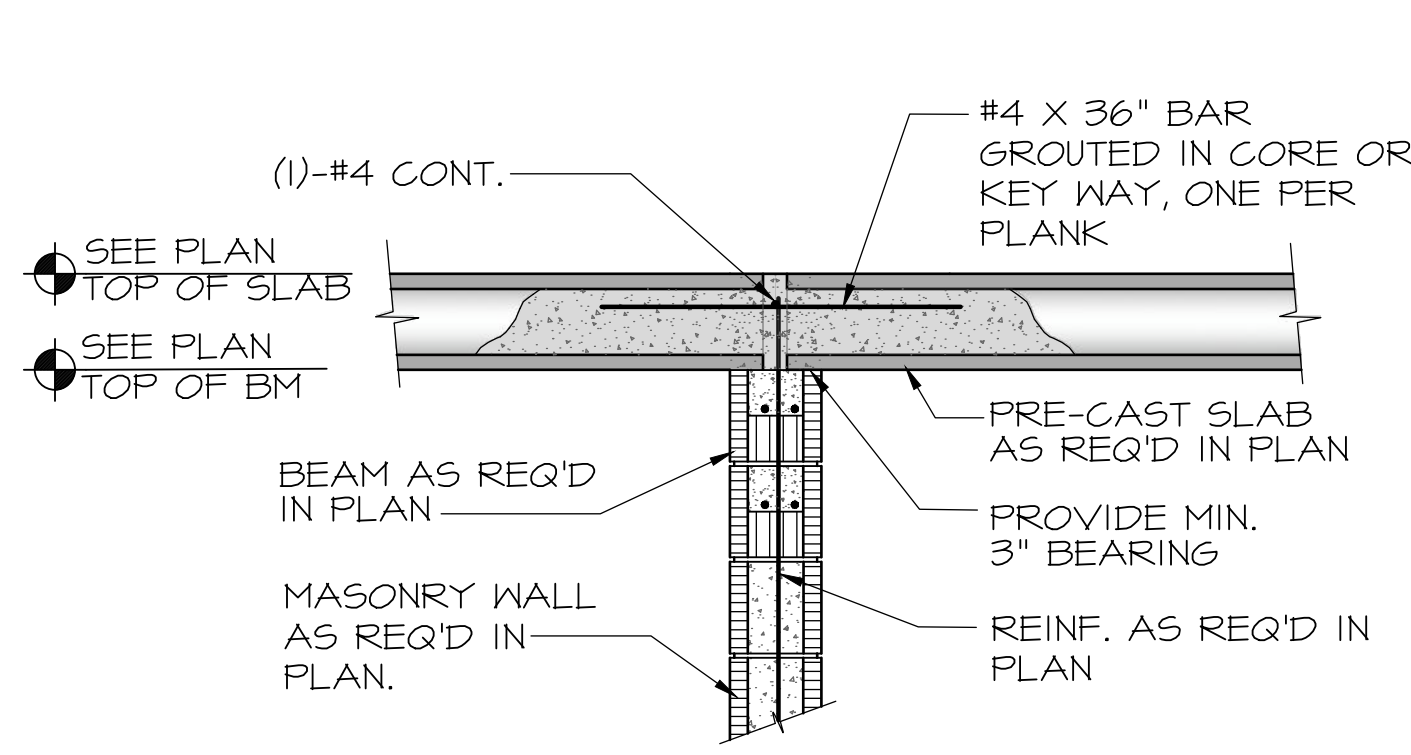
2
S4 **HOLLOWCORE PLANK EXTER. WALL DETAIL**

SCALE: 3/4"=1'-0"



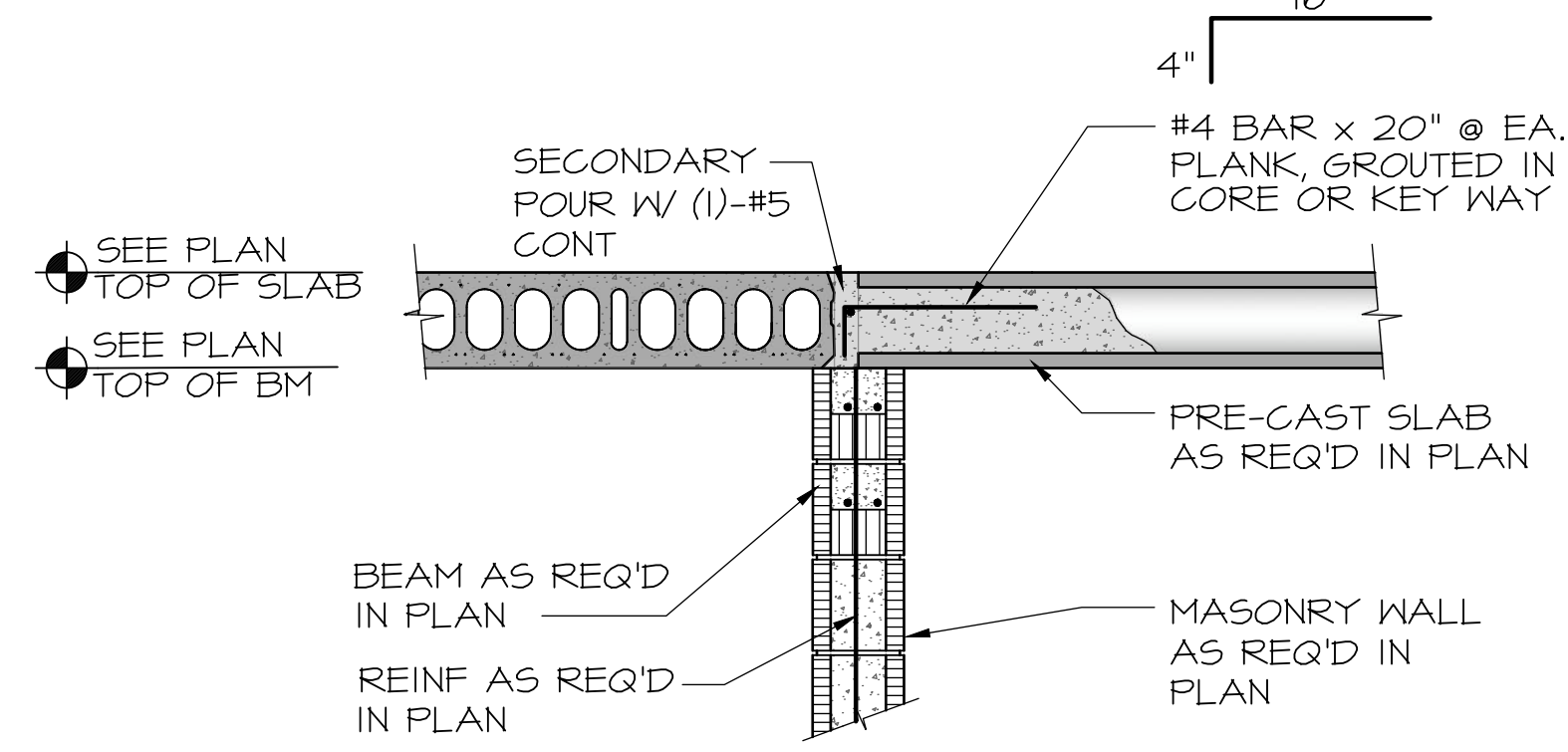
3
S4 **HOLLOWCORE PLANK EXTER. BRG. DETAIL**

SCALE: 3/4"=1'-0"



4
S4 **HOLLOWCORE PLANK INTER. BRG DETAIL**

SCALE: 3/4"=1'-0"



5
S4 **HOLLOWCORE PLANK INTER. BRG DETAIL**

SCALE: 3/4"=1'-0"

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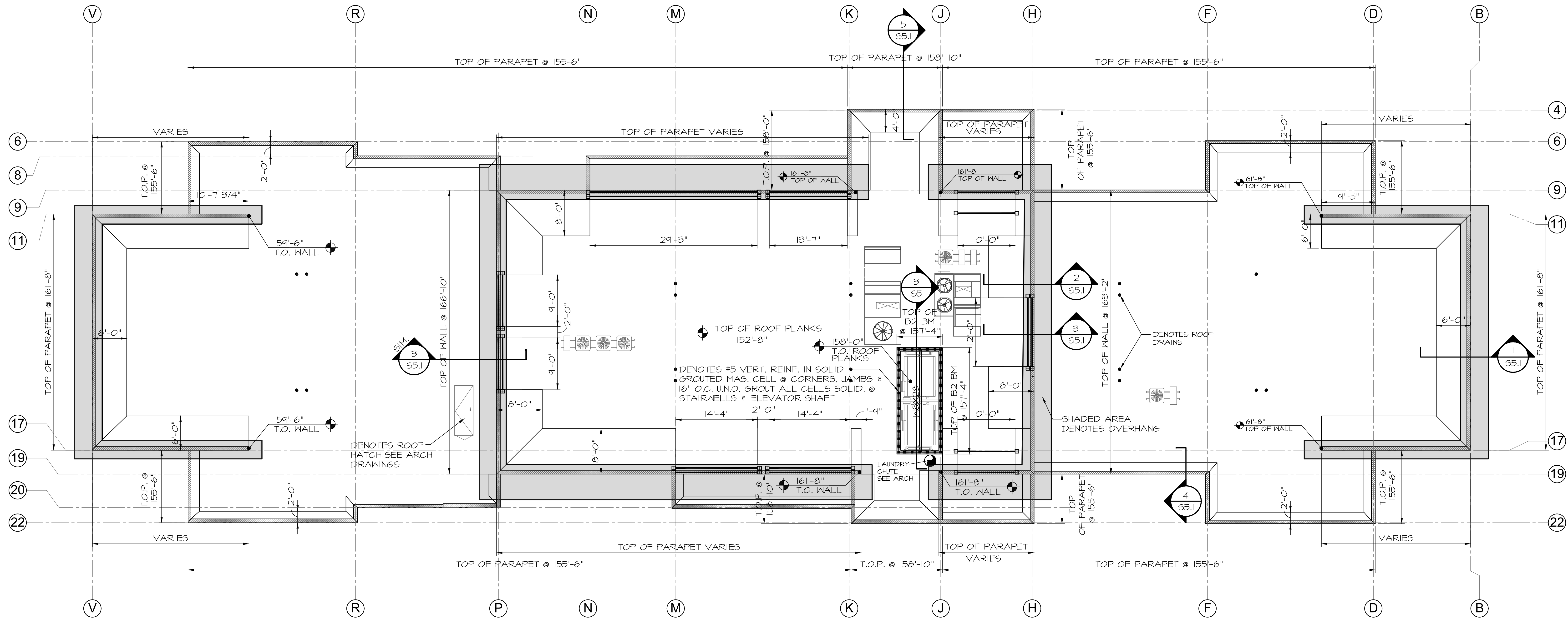
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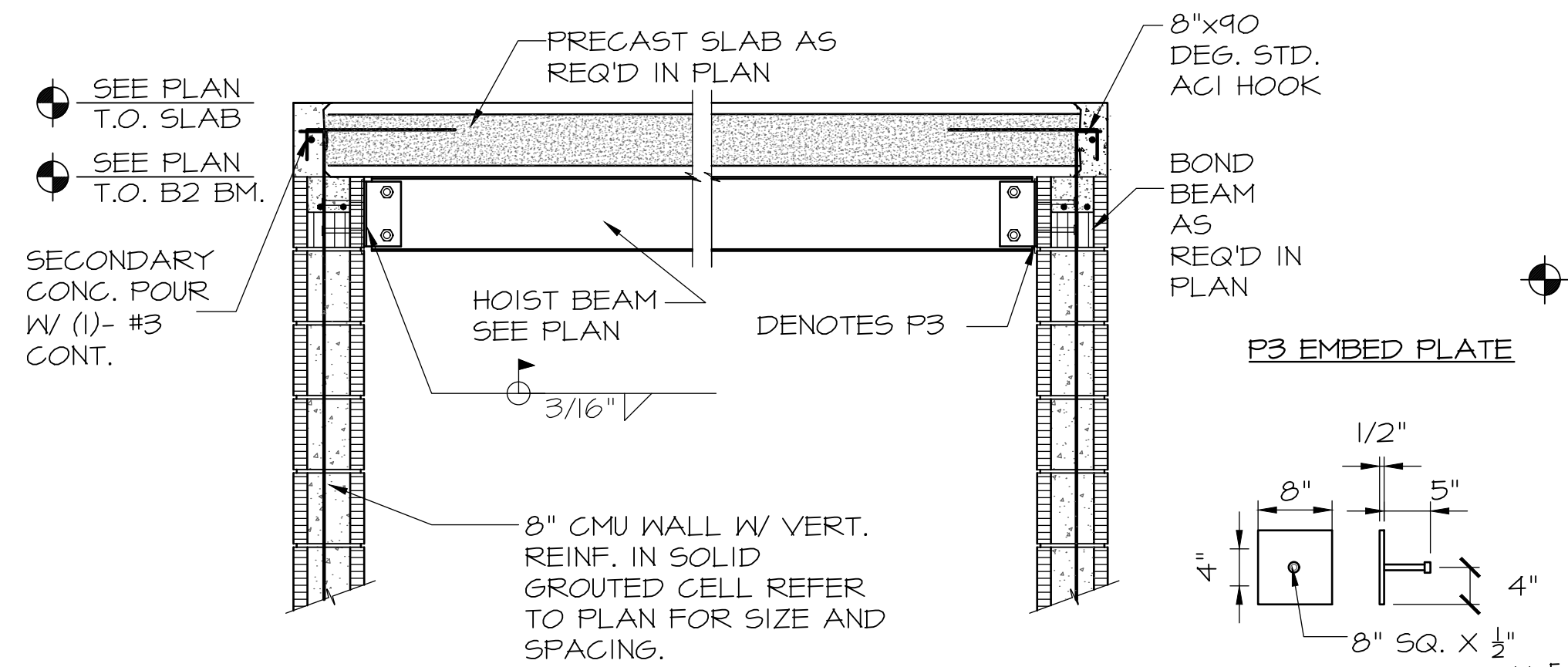
DATE

SHEET
S4

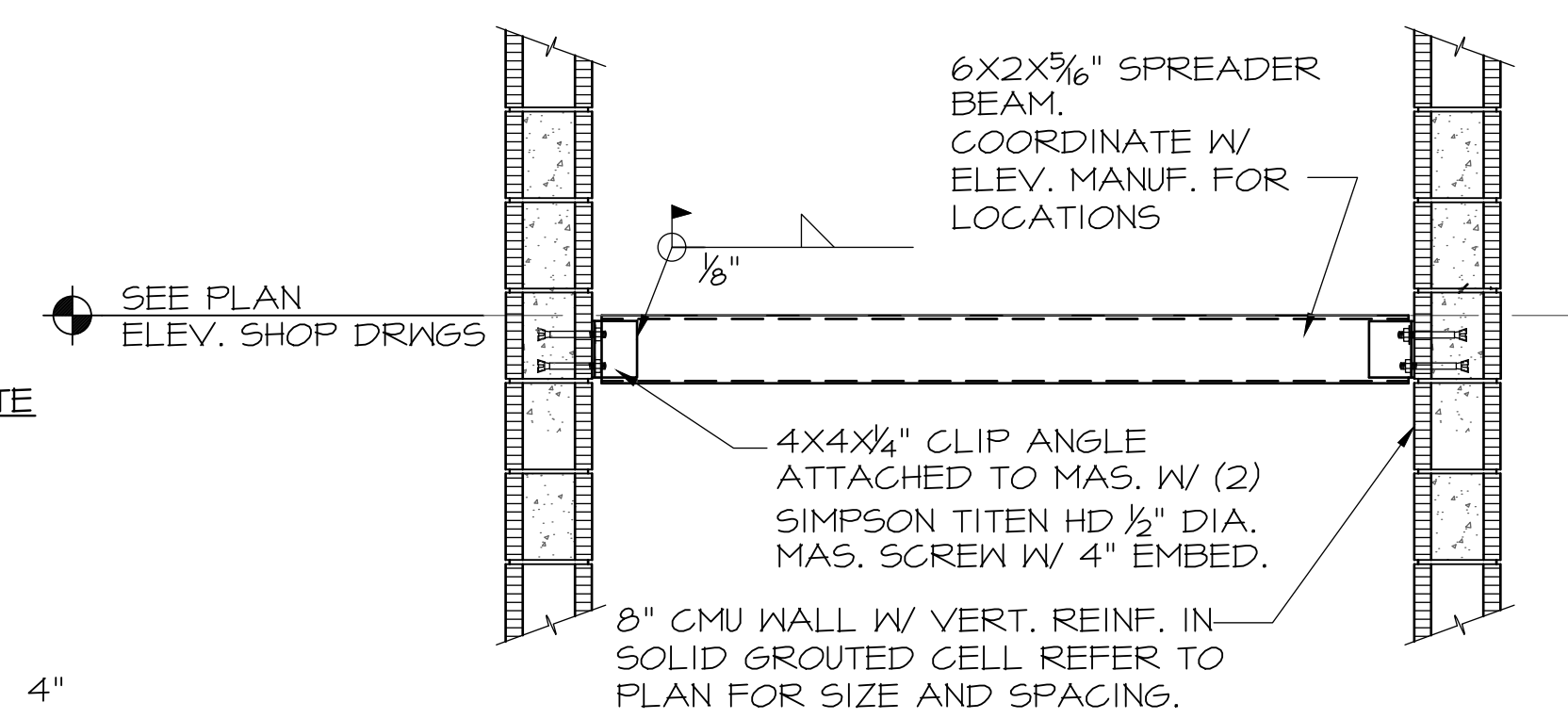
7 OF



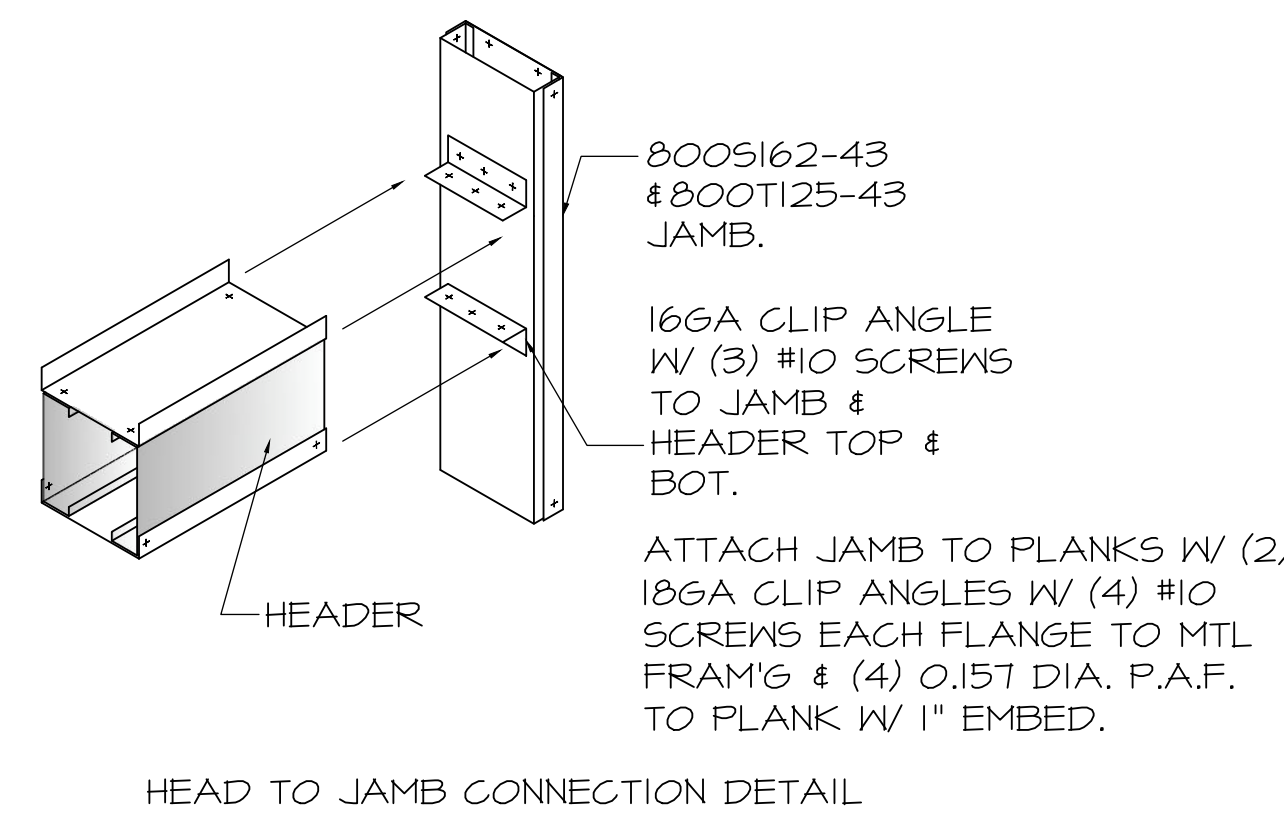
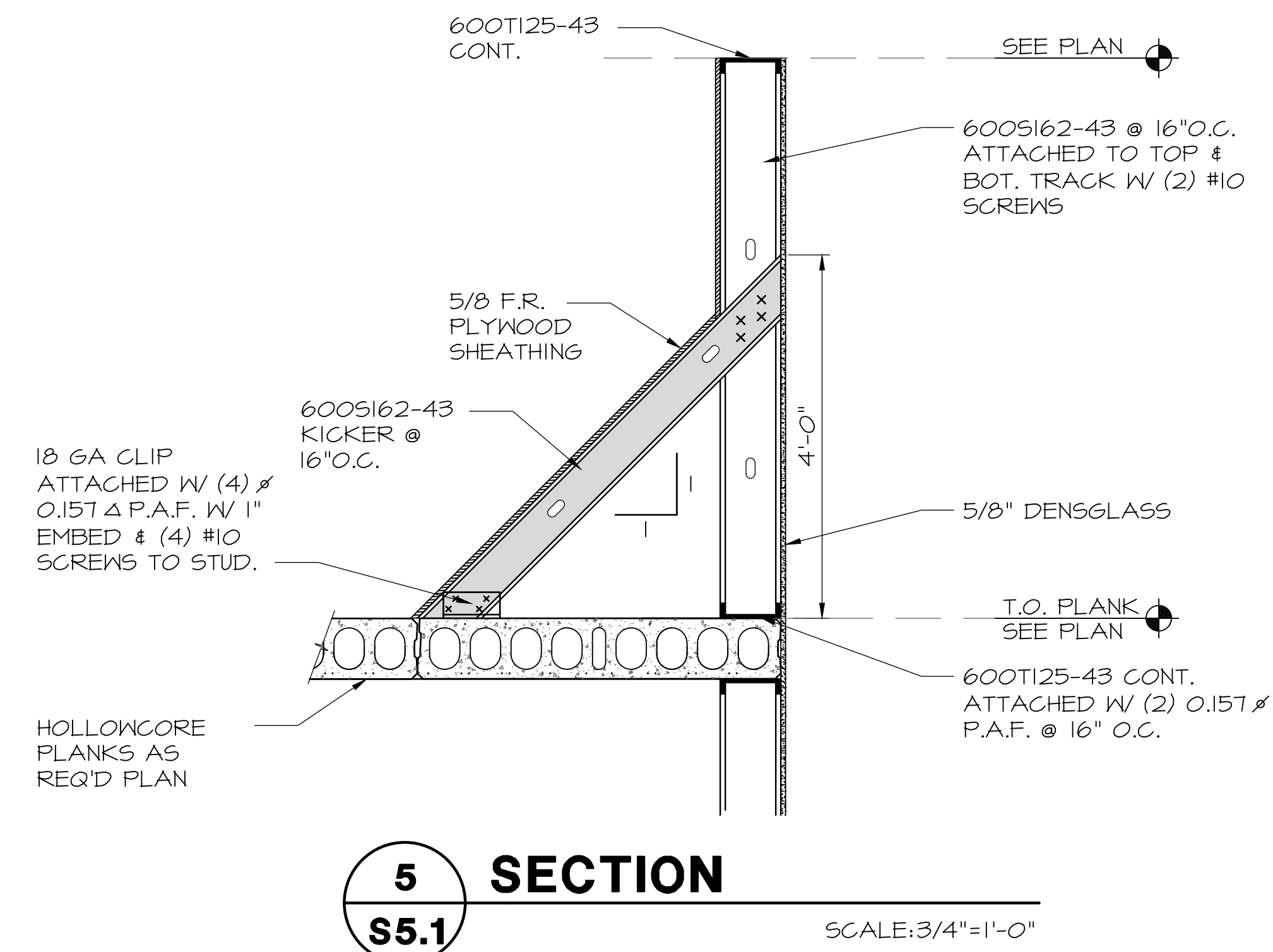
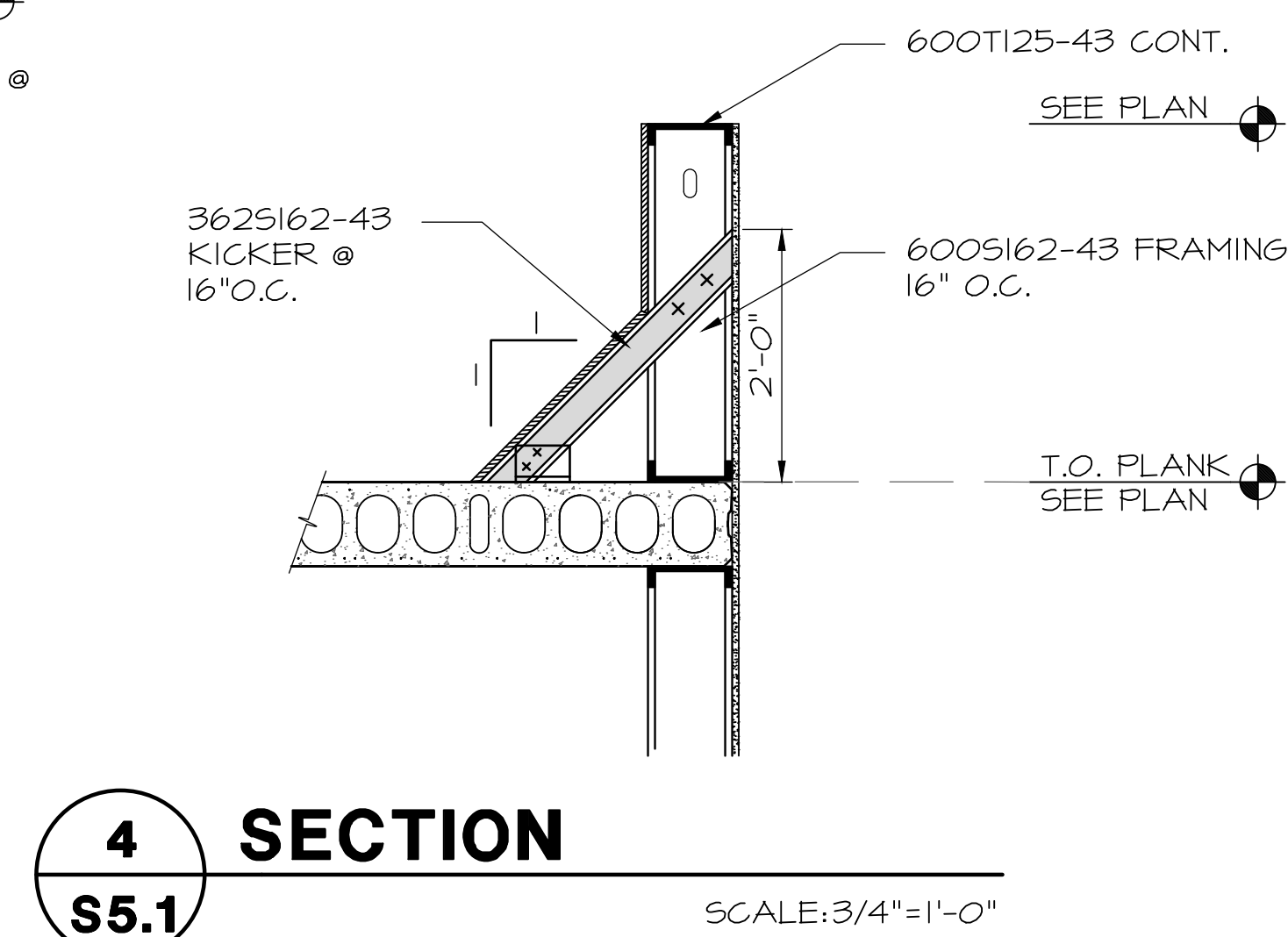
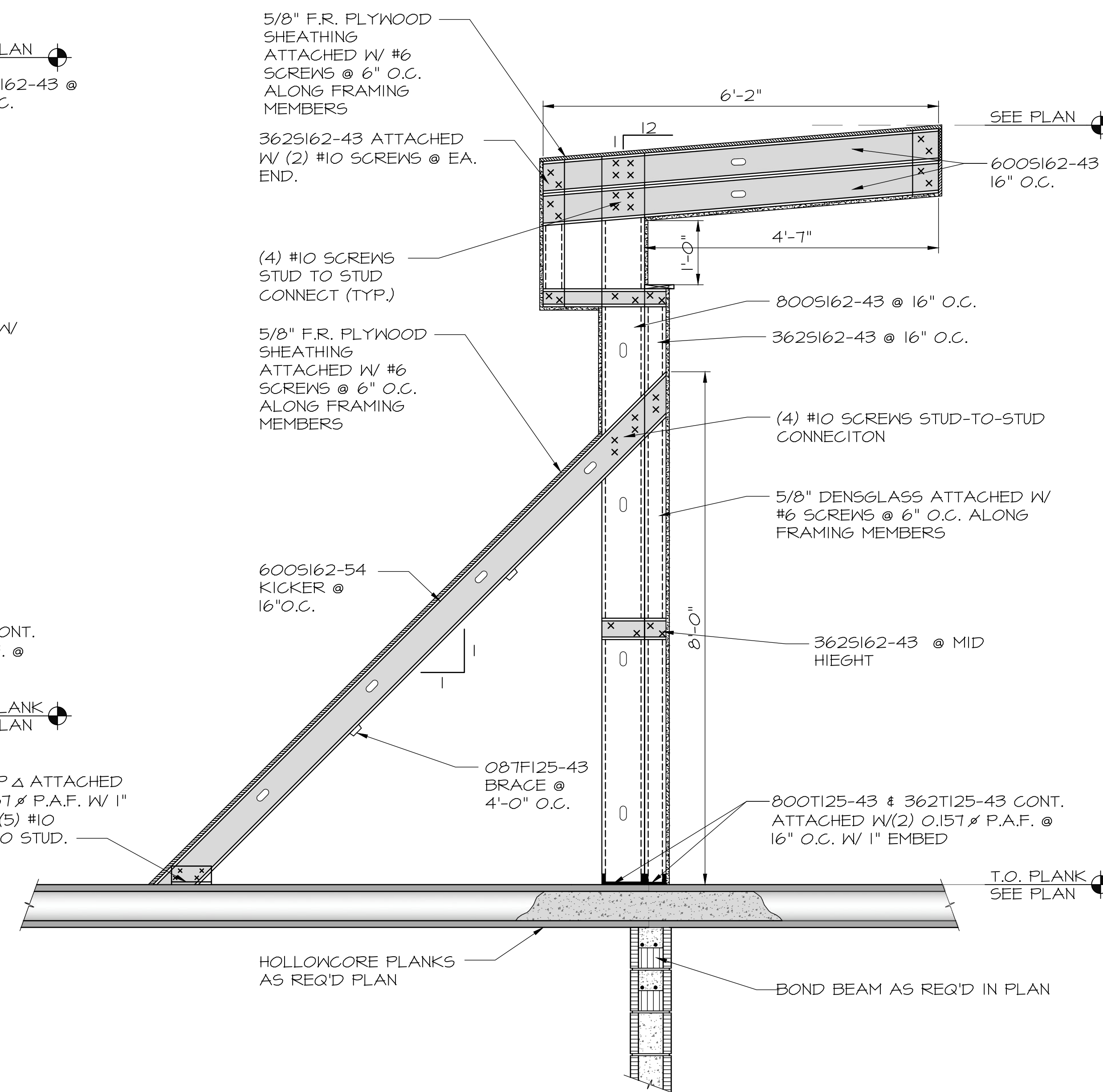
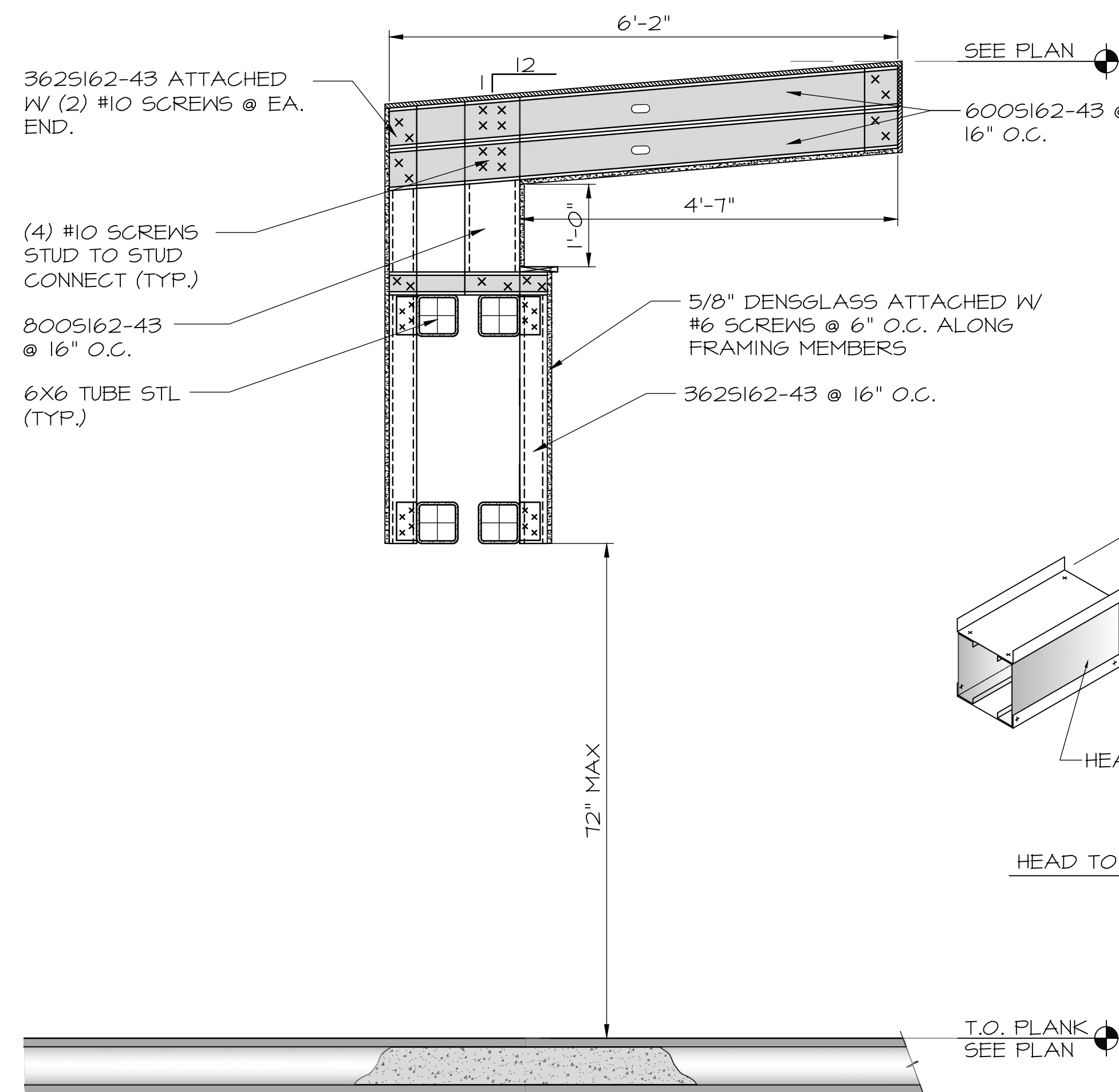
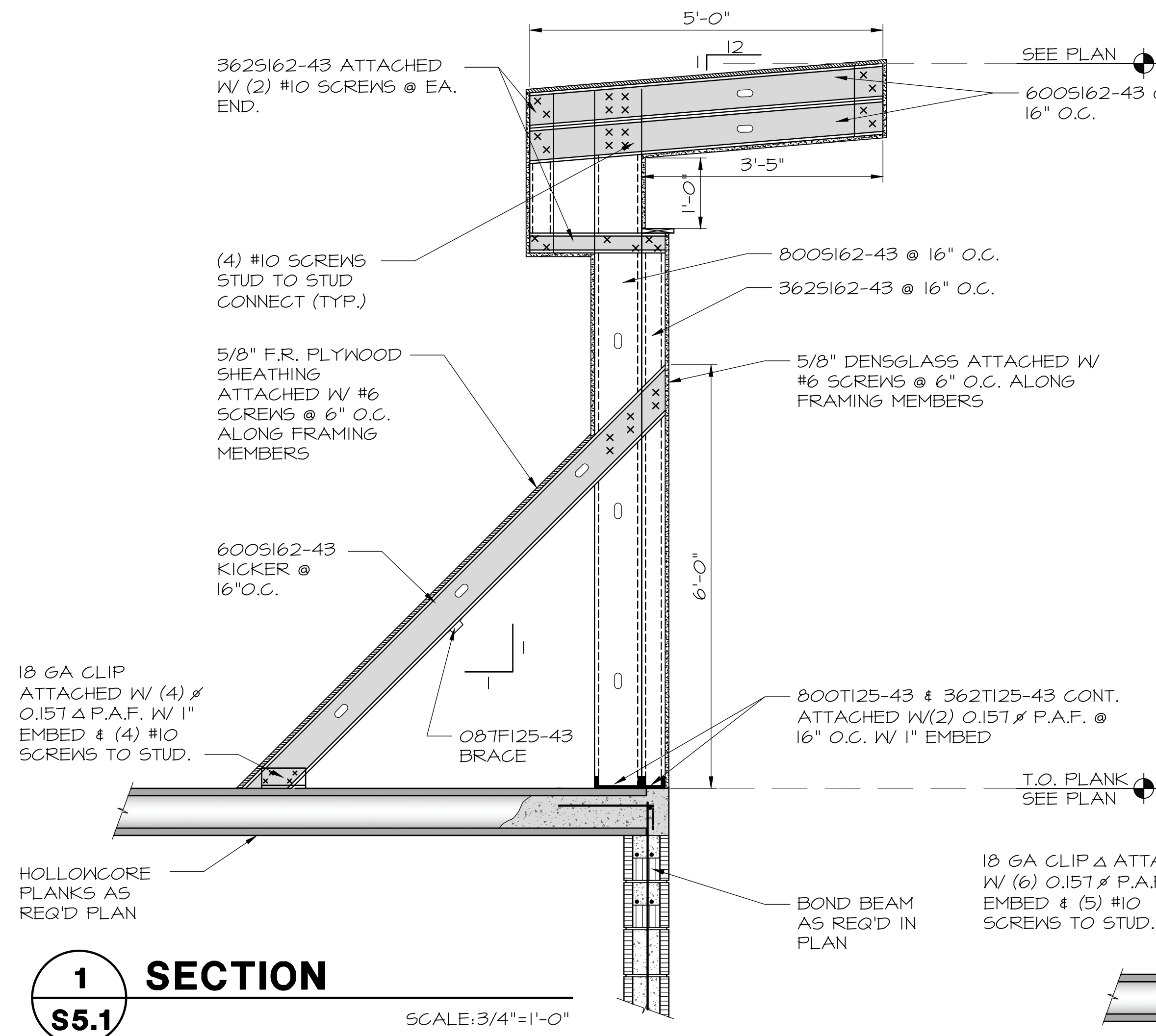
1 PARAPET FRAMING PLAN
SCALE: 1/8"=1'-0"



2 ELEV. HOIST BEAM DETAIL
SCALE: 3/4"=1'-0"



3 ELEV. SPREADER BM DETAIL
SCALE: 3/4"=1'-0"



STRUCTURAL DESIGN CRITERIA
GOVERNING CODES AND SPECIFICATIONS

1. 2020 Florida Building Code, 7th Edition.
2. Specifications for Structural Steel Buildings, June 22, 2014
3. Building Code Masonry Structures, TMS 402-2016
4. Building Code Requirements for Structural Concrete, ACI 318-14.
5. North American Specification for the Design of Cold-formed Steel Structural Members, AISI S100-16

GRAVITY DESIGN LOADS

- | | |
|---------------------------------------|---------|
| 1. Roof Dead Load | 65 PSF |
| 2. Roof Live Load | 20 PSF |
| 3. Canopy Roof Dead Load | 20 PSF |
| 4. Floor Dead Load | 75 PSF |
| 5. Floor Live Load | 40 PSF |
| 6. Corridors Serv'g Public Rms | 100 PSF |
| 7. Corridors Serv'g Private Rms | 40 PSF |
| 8. Stairs & Exits | 100 PSF |

WIND DESIGN DATA

- | | |
|------------------------------------|-----------|
| 1. Ultimate Design Wind Speed..... | 120 MPH |
| Nominal Design Wind Speed..... | 93 MPH |
| Risk Category..... | II |
| Wind Exposure..... | C |
| Enclosure Classification | ENCLOSED |
| Mean Roof Height..... | 52.67' FT |

Wind Loads - Components & Cladding - h ≤ 60'

Roof Angle (θ) = 0.0 deg
Type of roof = Monoslope

Area	10 sf	20 sf	50 sf	100 sf	200 sf	500 sf	1000 sf
Negative Zone 1	-39.1	-38.5	-33.1	-30.5	-27.9	-24.5	-21.5
Negative Zone 2	-22.5	-22.5	-22.5	-22.5	-18.5	-15.2	-12.1
Positive Zone 1 & 2	-51.6	-48.2	-43.9	-40.5	-37.2	-34.6	-32.8
Positive Zone 3 & 4	-10	10	10	10	10.0	10.0	10.0
Overhang Zone 1 & 2	-47.8	-43.4	-37.6	-33.1	-28.7	-25.1	-22.9
Overhang Zone 3	-47.8	-43.4	-37.6	-33.1	-28.7	-25.1	-22.9

User Input	25 sf	50 sf
Case A	-35.7	-33.1
Case B	-47.2	-43.9
Case C	-47.2	-43.9
Case D	-47.2	-43.9
Case E	-47.2	-43.9
Case F	-47.2	-43.9
Case G	-47.2	-43.9
Case H	-47.2	-43.9
Case I	-47.2	-43.9
Case J	-47.2	-43.9
Case K	-47.2	-43.9
Case L	-47.2	-43.9
Case M	-47.2	-43.9
Case N	-47.2	-43.9
Case O	-47.2	-43.9
Case P	-47.2	-43.9
Case Q	-47.2	-43.9
Case R	-47.2	-43.9
Case S	-47.2	-43.9
Case T	-47.2	-43.9
Case U	-47.2	-43.9
Case V	-47.2	-43.9
Case W	-47.2	-43.9
Case X	-47.2	-43.9
Case Y	-47.2	-43.9
Case Z	-47.2	-43.9

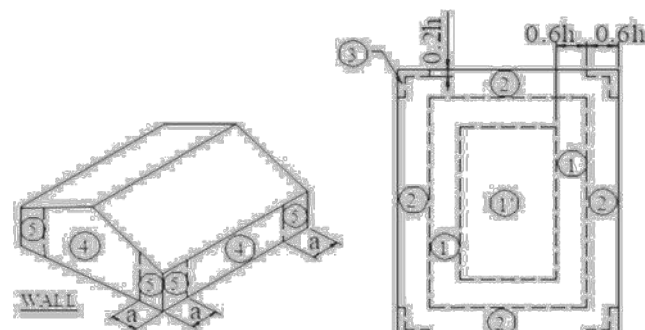
Parapet - op = 21.4 psf

Area	10 sf	20 sf	50 sf	100 sf	200 sf	500 sf	1000 sf
Case A	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case B	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case C	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case D	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case E	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case F	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case G	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case H	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case I	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case J	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case K	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case L	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case M	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case N	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case O	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case P	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case Q	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case R	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case S	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case T	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case U	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case V	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case W	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case X	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case Y	68.6	64.2	58.3	53.9	49.4	43.5	38.6
Case Z	68.6	64.2	58.3	53.9	49.4	43.5	38.6

Area	10 sf	20 sf	50 sf	100 sf	200 sf	500 sf	1000 sf
Negative Zone 1	-1.44	-1.12	-1.03	-0.90	-0.79	-0.73	-0.67
Negative Zone 2	-1.08	-0.92	-0.87	-0.81	-0.72	-0.65	-0.59
Positive Zone 1 & 2	-1.08	-0.92	-0.87	-0.81	-0.72	-0.65	-0.59
Positive Zone 3 & 4	-1.08	-0.92	-0.87	-0.81	-0.72	-0.65	-0.59
Overhang Zone 1 & 2	-1.08	-0.92	-0.87	-0.81	-0.72	-0.65	-0.59
Overhang Zone 3	-1.08	-0.92	-0.87	-0.81	-0.72	-0.65	-0.59

Note: GCP reduced by 10% due to roof angle < 10 deg

Location of C&C Wind Pressure Zones - ASCE 7-16



GENERAL STRUCTURAL NOTES
FOUNDATION

1. Foundation design is based on an allowable soil bearing pressure of 2500 PSF and on evaluations made by Geo-Tech, Inc. within soils project #20-5543.03.16 dated June 12, 2020.
2. Refer to the recommendations below from the soils report referenced above regarding soil conditions, temporary groundwater control, general site preparation, fill placement and compaction requirements.
3. Contractor shall provide complete reports, prepared by a Florida Registered Geotechnical Engineer, of all monitoring and testing performed to bring the foundation strata within conformance of design recommendations. Monitoring and testing shall be performed in accordance with the project specifications.
4. Provide 6 mil vapor barrier over compacted subgrade for slab on grade. Vapor barrier shall be lapped a minimum of 6 inches in direction of concrete pour. Care shall be exercised to prevent damage or rupture of barrier. Where columns, pipe conduit, etc. penetrate membrane, cut around projection and seal with joint tape.

Recommendation

Building Area

Geo-Tech's borings for the structure, show a loose to very loose zone of slightly clayey sand zone from the ground elevation to a depth of approximately thirteen and a half (13 1/2) feet below the existing ground surface. The primary concern is the settlement of the floor slabs and column pads due to the very loose to loose sandy soils.

Geo-Tech's is to utilizing auger cast piles or stone columns to support the column pads. Geo-Tech would anticipate that the auger cast piles would be bearing piles. If stone columns option is selected the anticipated depth of the stone columns would be in the range of fifteen (15) feet below the existing site grade. The size, type and locations of these piles or columns should be determined by a specialty foundation engineer licensed in the State of Florida. It is recommended that once the design is finalized, each column location should be drilled prior to the start of construction to verify pile requirements.

In addition, the entire foot print of the floor slab should be over excavated to a depth four (4) feet below the bottom of the floor slab and footing column pads. The bottom of the floor slab excavation should be then proof rolled in accordance with the "Proof Rolling" section of this report.

Backfilling of the floor slab over excavation should use material conforming to the "Structural Fill Material" section of this report. The backfill should be compacted in accordance with the "Compaction of Fill Soils" section of this report.

All excavations should extend a minimum of two (2) feet beyond the edge of perimeter footings and column pads.

The over excavated material can be re-used provided it meets the requirements of the "Structural Fill Material" section of this report. All compacted fill should be compacted in accordance with the "Compaction of Fill Soils" section of this report.

Recommendations

Building Area

Geo-Tech's borings for the structure, show a loose to very loose zone slightly clayey sand zone from the ground elevation to a depth of approximately thirteen and a half (13 1/2) feet below the existing ground surface. The primary concern is settlement of the floor slab and column pads.

Geo-Tech recommends utilizing auger cast piles or stone columns to support the column pads. The size, type and locations of these piles or columns should be determined by a specialty foundation engineer licensed in the State of Florida. It is recommended that once the design is finalized, each column location should be drilled prior to the start of construction to verify pile requirements.

In Geo-Tech's opinion the foot print of the floor slab should be over excavated four (4) feet below the bottom of the floor slab.

The bottom of the floor slab excavation should be proof rolled in accordance with the "Proof Rolling" section of this report.

The over excavated material can be re-used provided it meets the requirements of the "Structural Fill Material" section of this report. All compacted fill should be compacted in accordance with the "Compaction of Fill Soils" section of this report.

Geo-Tech also that additional borings be done at each column pad location to verify sub-surface conditions at each location.

Recommended Site Preparation

Building Area

Stripping and Grubbing

The "footprint" of the proposed building, plus an additional horizontal margin of ten (10) feet, should be stripped of the existing vegetation, stumps, surface debris, or other deleterious materials as found. Expect clearing and grubbing to depths of about eight (8) to twelve (12) inches. Deeper clearing and grubbing depths may be encountered in heavily vegetated areas where major root systems are found. Actual depth(s) of stripping and grubbing must be determined by visual observation and judgment during the earthwork operation.

Proof-Rolling

If utilizing Option 2 in the Recommendations section of this report, proof-rolling of the cleared surface is recommended for 1) locate any soft areas or unsuitable surface or near surface soils; 2) increase the density of the in-situ soils to at least ninety-five (95) percent of the maximum density as determined by the Modified Proctor Test Method (ASTM D-1557) maximum dry density value; and 3) prepare the existing surface for the addition of fill soils (if required). Proof-rolling of the building areas should consist of at least ten (10) passes of a self-propelled static compactor. Each pass of the compactor should overlap the preceding pass by thirty (30) percent to insure complete coverage. If deemed necessary, in areas continuing to "yield," remove all deleterious material and replace with a clean, compacted sand backfill. Proof-rolling should occur after cutting and before filling. Vibratory compaction equipment should not be used within one hundred (100) feet of neighboring structures.

Structural Fill Material

Structural fill material should be free of organic material such as roots and/or vegetation. Geo-Tech recommends using sand fill with between three (3) to twelve (12) percent by dry weight of material passing the U.S. Standard No. 200 sieve size. All structural fill should be pre-qualified prior to importing and placing.

Upper fine sands and slightly clayey sands found on site should meet these requirements and can be used if kept separate from the clayey soils during the earthwork phase of construction. Clayey soils are typically not used for structural fill due to inherent nature to retain moisture and the natural weight of the material makes compaction requirements difficult to achieve. However, the clayey soils can be utilized for other non-structural gradings as desired.

Compaction of Fill Soils

Structural fill should be placed in level lifts not thicker than twelve (12) inches (uncompacted). Each lift in the proposed building areas should be compacted to at least ninety-eight (98) percent of the maximum density as determined by the Modified Proctor Test Method (ASTM D-1557) maximum dry density value. If hand-held compaction equipment is used, reduce the uncompacted lift thickness to six (6) inches. Filling and compaction operation should continue in lifts until the desired elevation is attained.

Foundation Support

Foundations for the proposed structure may consist of shallow foundations placed on compacted engineered fill material. Such footings may be designed for maximum allowable soils contact pressures of two thousand five hundred (2,500) pounds per square foot. For purposes of confinement, exterior footings should be embedded at least twenty-four (24) inches below the lowest adjacent grade as measured to the base of the footing. Interior footings should be embedded a minimum of eighteen (18) inches below the lowest adjacent grade as measured to the base of the footing.

Moisture entry from the underlying subgrade soils should be minimized. An impervious membrane placed between the subgrade soils and floor slab will help to accomplish this. A polyethylene film (six [6] mil) is commonly used for this purpose. Care should be used so that the membrane is not punctured when placing reinforcing steel (or mesh) and concrete.

Quality Control

Geo-Tech recommends establishing a comprehensive quality control program to ensure that site preparation and foundation construction is conducted according to the plans and specifications. Materials testing and inspection services should be provided by Geo-Technologies, Inc. An engineering technician should be on-site to monitor all stripping and grubbing, to verify that all deleterious materials have been removed.

Density testing should be performed during backfill and below all footings and floor slabs to check the required compaction. Field density values should be compared to laboratory proctor moisture-density results for each different natural and fill soil encountered.

If excavating to attain the recommended separation, Geo-Tech recommends that we be notified to verify the depth of excavation, daylight gravity drain (if required), compaction of backfill and foundation is properly located within boundaries of excavation.

Geotechnical engineering design does not end with the advertisement of construction documents. The design is an ongoing process throughout construction. Because of Geo-Tech's familiarity with the site conditions and the intent of the engineering design, we are most qualified to address problems that might arise during construction in a timely and cost effective manner.

CONCRETE

1. All concrete work shall be accomplished in accordance with "Specifications for Structural Concrete for Buildings", ACI 301-II.
2. All concrete shall have a specified minimum compressive strength of 3000 PSI at 28 days.
3. Concrete aggregate shall be 3/4" in accordance with ASTM C33.
4. Submit mix design for review prior to concrete pour.

PRE-CAST SLABS

1. Pre-cast roof and floor slabs shall be pre-tensioned, hollow core, flat plank machine-cast at factory with closely controlled mixing, casting and curing procedures.
2. Slabs shall be designed and engineered for all loads prescribed herein.
3. Reinforcement shall be multiple strand, stress relieved, high-tensile steel wires with ultimate strength of 250,000 psi.
4. Contractor shall submit shop drawings, signed and sealed by a Florida registered structural engineer, for review by the structural engineer of record prior to fabrication. Shop drawings shall also be reviewed by the mechanical, plumbing and electrical contractors for coordination of opening requirements prior to submittal to the engineer of record.
5. Provide 3-1/2" minimum bearing on masonry and 2-1/2" on steel.
6. Coordinate exact opening sizes and locations with mechanical, plumbing and electrical contractors.
7. Openings not shown on the shop drawings shall not be cut without the prior notification of the pre-cast engineer and the engineer of record.
8. Grout joints with a 3:1 sand/portland cement mix with minimum grout strength of 2,500 psi. Clean joints prior to grouting and remove grout from ceilings before hardening.

MASONRY

1. All concrete masonry work shall be in accordance with "Specifications for Concrete Masonry Structures", ACI 530J-II.
2. Concrete masonry units shall be in accordance with ASTM C90, Grade N, Type II and shall be of a type such that the cores line up vertically.
3. Compressive strength of masonry assemblage shall be 1500 PSI minimum. U.N.O.
4. Masonry mortar shall be in accordance with ASTM C270, Type S or M.
5. Grout for masonry work shall be in accordance with ASTM C476, Coarse Type, and shall attain a compressive strength of 2,000 PSI at 28 days.
6. Provide 9 gauge ladder type joint reinforcing at 16" on center vertically in all masonry construction. Lap joint reinforcement a minimum of 6" and discontinue at all wall joints.
7. Submit manufacturer's test data of masonry units and assemblage for review prior to construction.
8. Provide cleanout at the base of all grouted masonry cells. Clean masonry void to be grouted of all debris and mortar protrusions.
9. Provide masonry precast lintel with minimum 8" bearing over all masonry openings.

REINFORCEMENT

1. Reinforcing steel shall be in accordance with ASTM A615, Grade 60.
2. Masonry joint reinforcement shall be hot-dip galvanized steel wire in accordance with ASTM A82.
3. Welded wire fabric shall be in accordance with ASTM A185 and shall be placed in the upper 1/3 of the slab.

STRUCTURAL STEEL

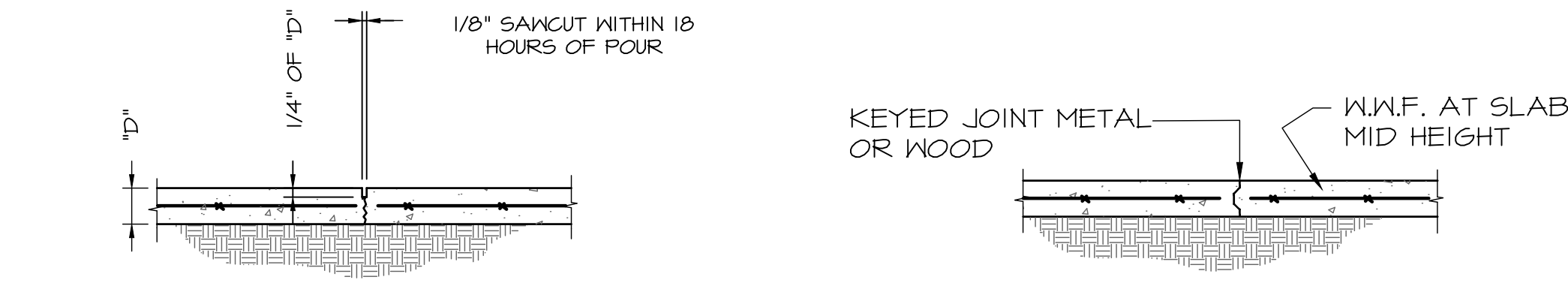
1. All structural shapes, except structural tubing, shall be in accordance with ASTM A36.
2. Structural tubing shall be in accordance with ASTM A500, Grade B.
3. Bolted connections, unless otherwise stated, shall be in accordance with ASTM A325X.
4. Anchor bolts shall be in accordance with ASTM A307.
5. Headed studs shall be attached to plates such that the connection develops the full tensile strength of the stud.
6. Welding shall be in accordance with AWS D1.1 with E70XX electrodes.
7. Submit structural steel shop drawings for review prior to fabrication.

COLD-FORMED STRUCTURAL STEEL

1. All cold-formed structural steel members (sheet metal studs) shall be of the type, size, gage and spacing as shown on the structural and architectural plans.
2. All Light Gauge Steel Members 54 Mil (16 Gauge) and heavier shall be formed from steel corresponding to a type listed in the AISI Specifications for the Design of Cold-Formed Steel Structural Members, with a minimum yield strength of 50 KSI.
3. All Light Gauge Steel Members 43 Mil (18 Gauge) and lighter shall be formed from criteria listed above, with a minimum yield strength of 33 KSI, unless noted otherwise.
4. All framing products shall be formed from steel processing a "G-60" Zinc Coating corresponding to the minimum requirements of ASTM A653.
5. Submit cold formed structural steel shop drawings for review prior to fabrication.

MISCELLANEOUS

1. Contractor shall brace all construction against gravity, lateral and uplift loads until structure is complete.
2. Coordinate structural work with architectural plans and all other trades.
3. All construction shall be in accordance with the Florida Building Code, 2020 Edition.



CONTROL JOINT

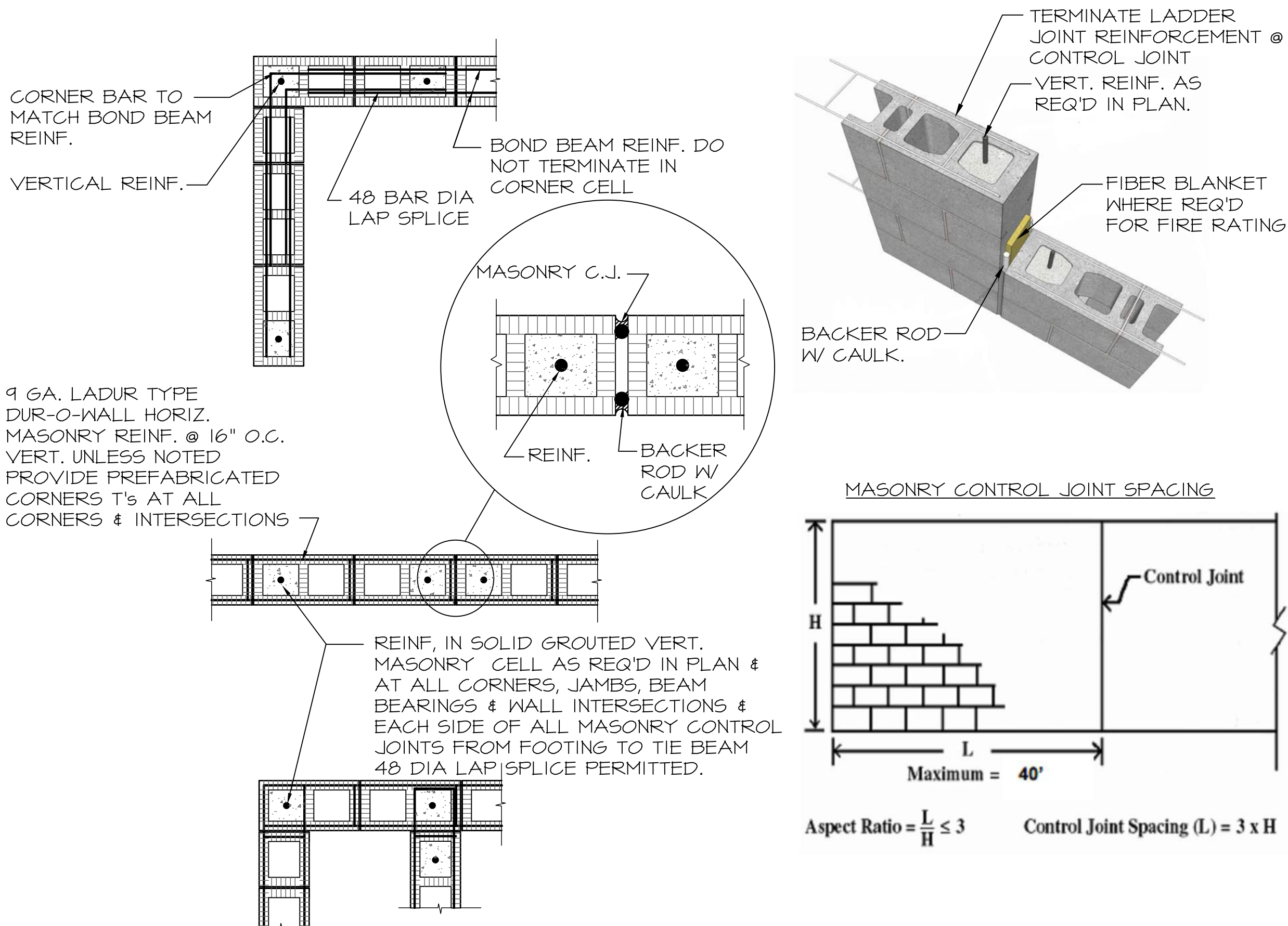
SAW CONTROL JOINT NOTES

1. MAKE SAW CUT AS SOON AS SLAB IS ABLE TO SUPPORT WEIGHT OF WORKERS AND SAWING EQUIPMENT WITHOUT DAMAGE TO FINISH SURFACE OF SLAB.
2. ALL SAW CUT CONTROL JOINTS SHALL BE 'SOFT CUT' WITHIN 2 HOURS AFTER FINAL FINISHING.
3. CLEAN JOINT PRIOR TO FILLING THE JOINT.

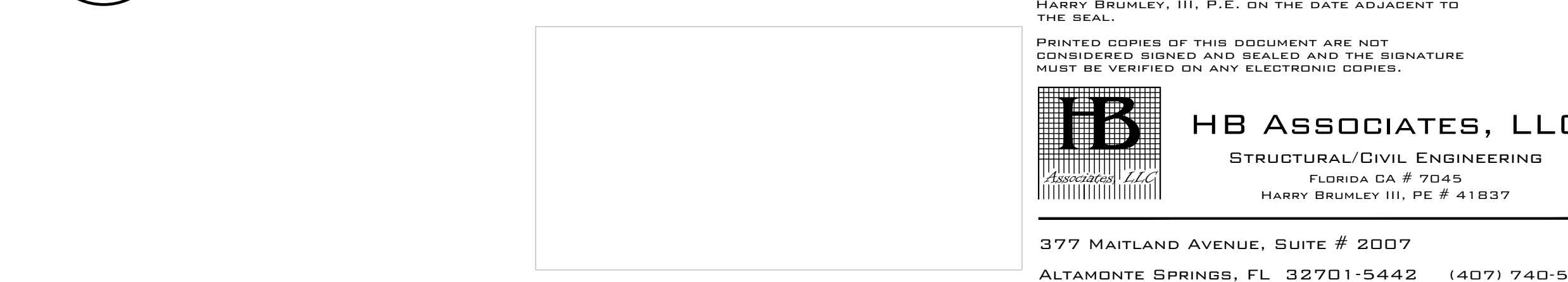
JOINT FILLER MATERIAL NOTES

1. FILLER MATERIAL USED SHALL HAVE A MINIMUM SHORE A HARDNESS OF 35, AND SHALL CONFORM TO ASTM D2240. APPROVED JOINT FILLER IS VULKEM 245 AS MANUFACTURED BY MAMECO INTERNATIONAL OR APPROVED EQ.
2. WHERE POSSIBLE FILLER MATERIAL SHALL BE APPLIED WHEN BUILDING IS UNDER PERMANENT TEMPERATURE CONTROL. THIS SHALL BE EITHER AT THE END OF CONSTRUCTION OF THE COMPLETE BUILDING SHELL OR A MINIMUM OF 30 DAYS AFTER SLAB CONSTRUCTION.
3. FOLLOW STRICTLY THE MANUFACTURERS RECOMMENDED PROCEDURES FOR APPLYING THE JOINT FILLER.

1 TYP. SLAB ON GRADE JOINT DETAILS
SCALE: 3/4" = 1'-0"



2 TYP. WALL REINF. DETAIL
SCALE: N.T.S.



TO THE BEST OF THE ENGINEER'S KNOWLEDGE THE PLANS & SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AS SPECIFIED WITHIN.

THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY HARRY BRUMLEY, III, P.E. ON THE DATE ADJACENT TO THE SEAL.

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.



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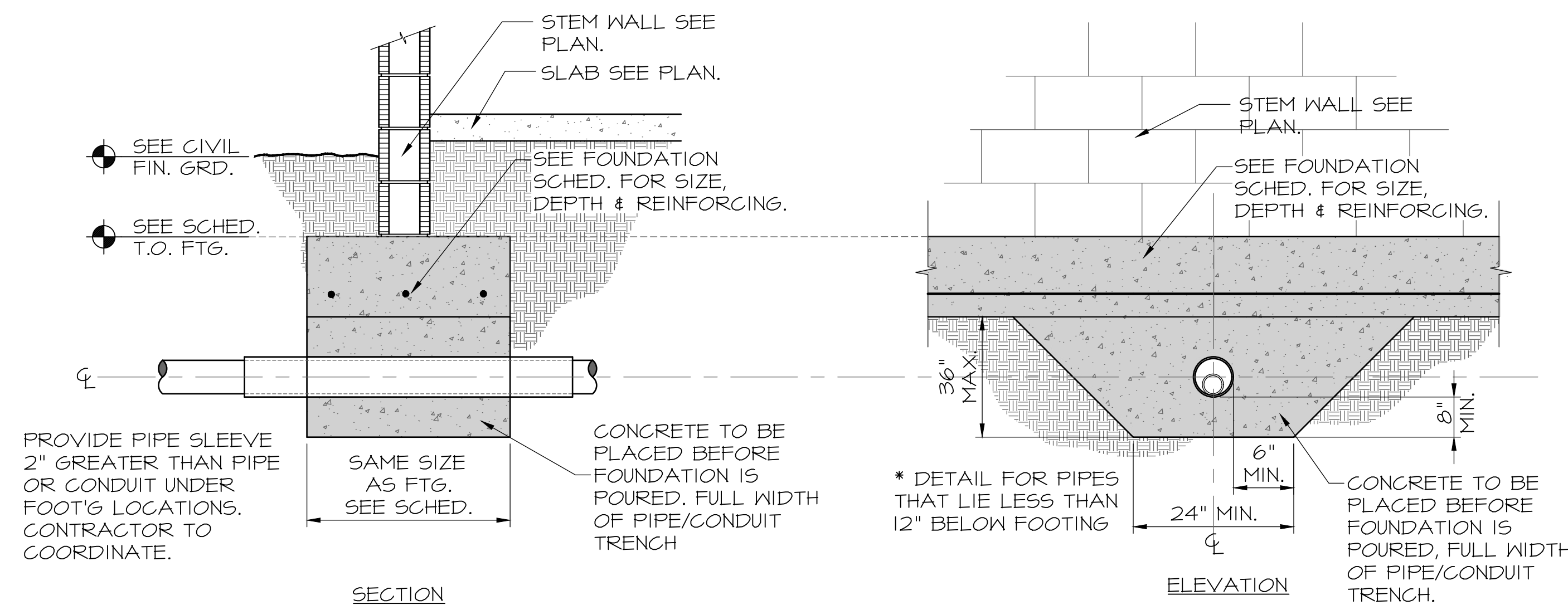
JOB NO.

DATE

SHEET

S6

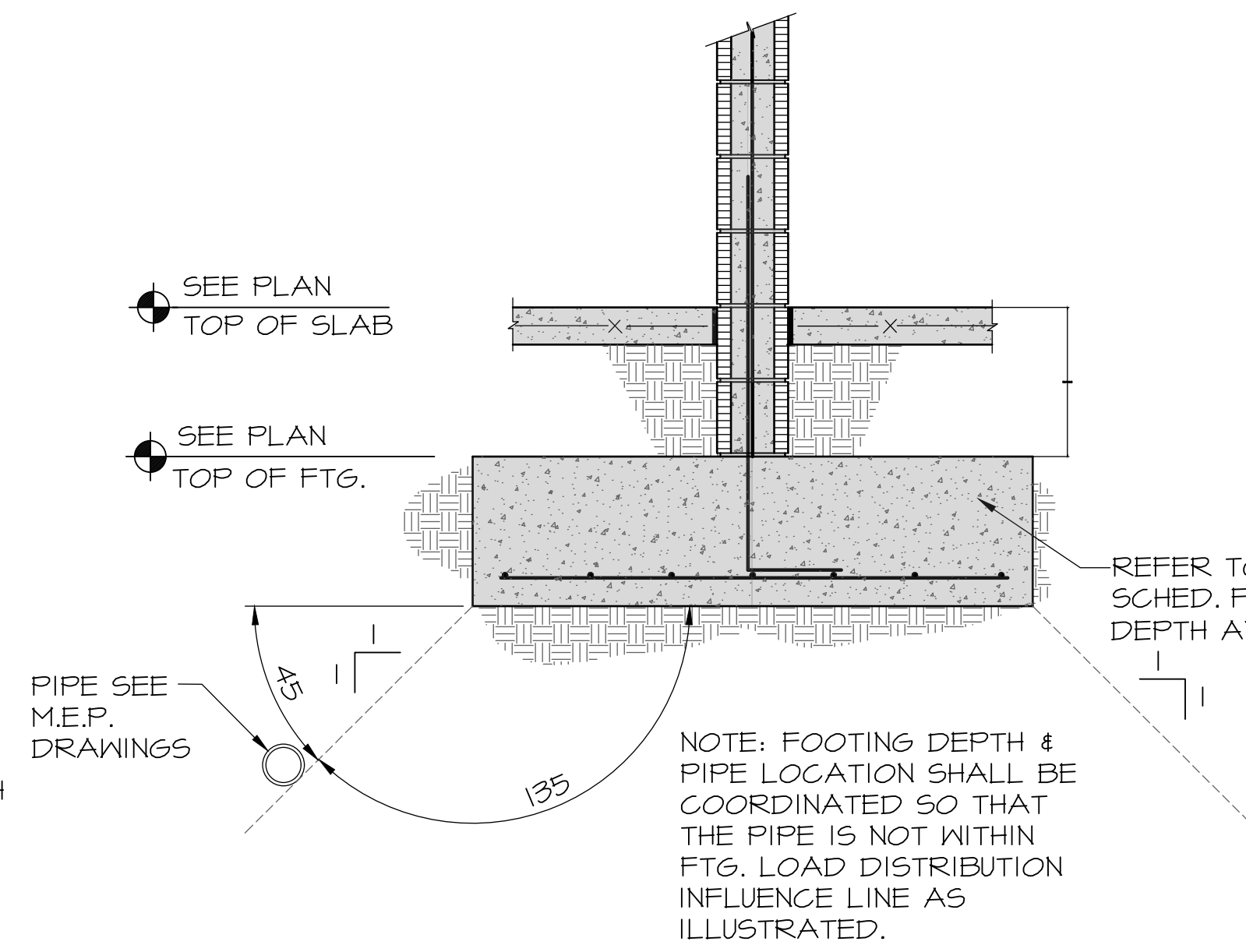
10 OF



TYP. PIPE SLEEVE PENETRATION UNDER FOOTING DETAIL

1
S7

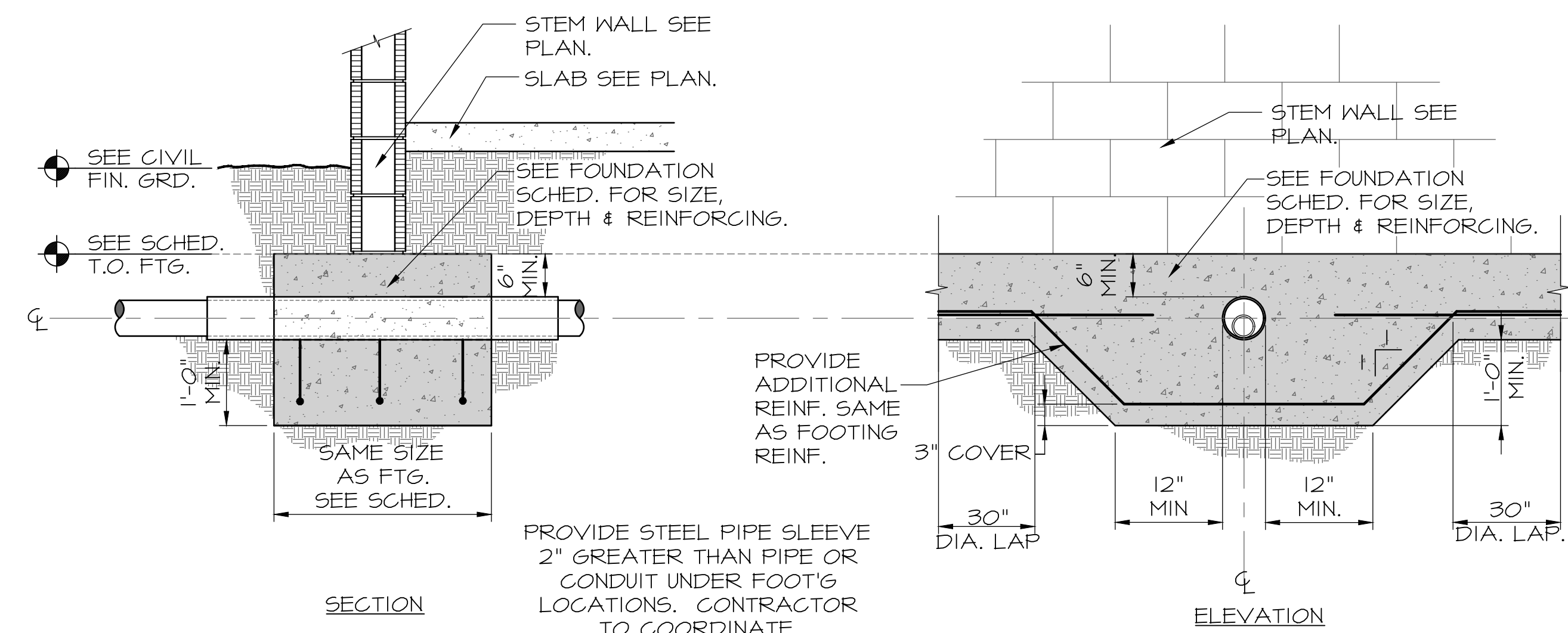
SCALE: 3/4"=1'-0"



TYP. PIPE PARALLEL TO WALL FOOTING

4
S7

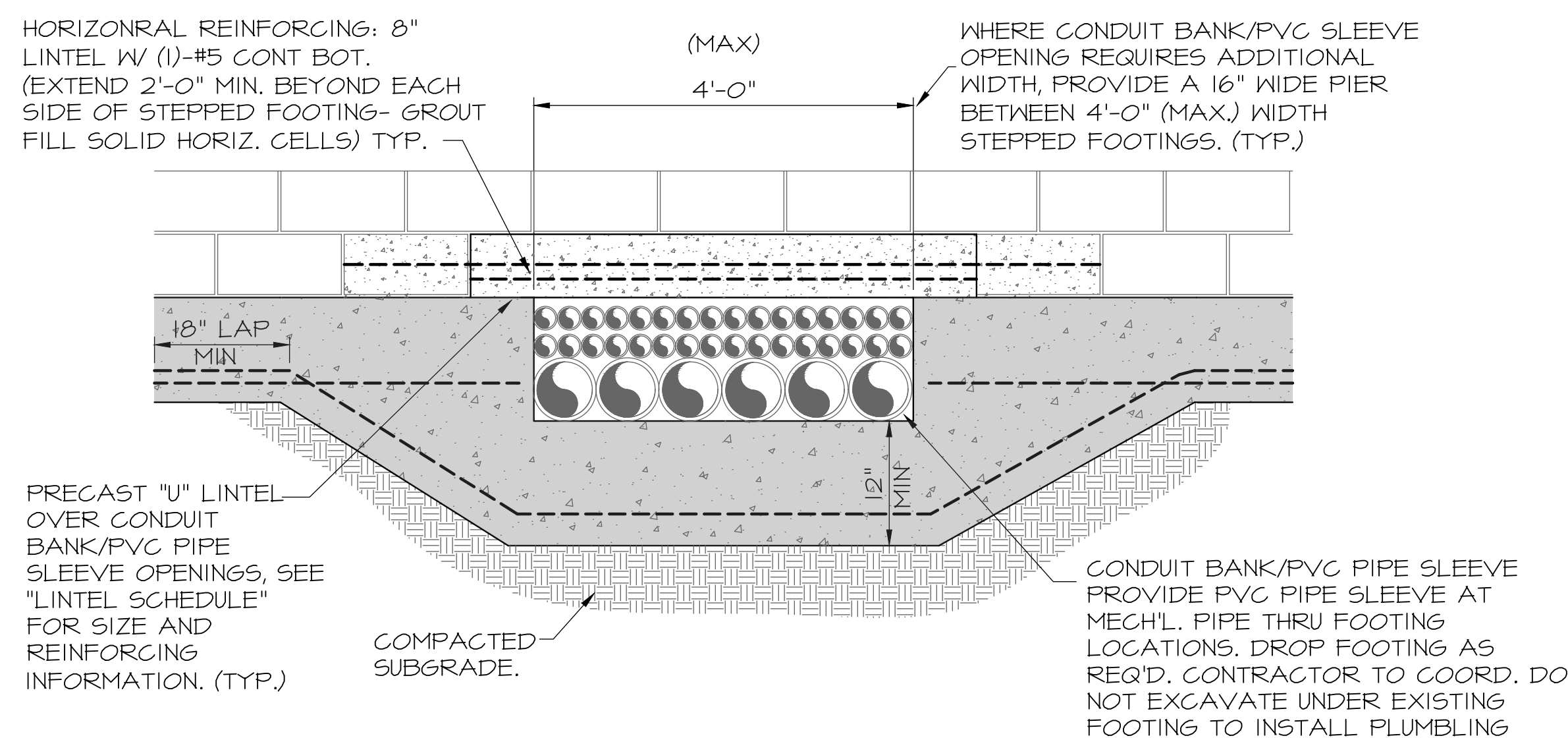
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TYP. PIPE SLEEVE PENETRATION THRU FOOTING DETAIL

2
S7

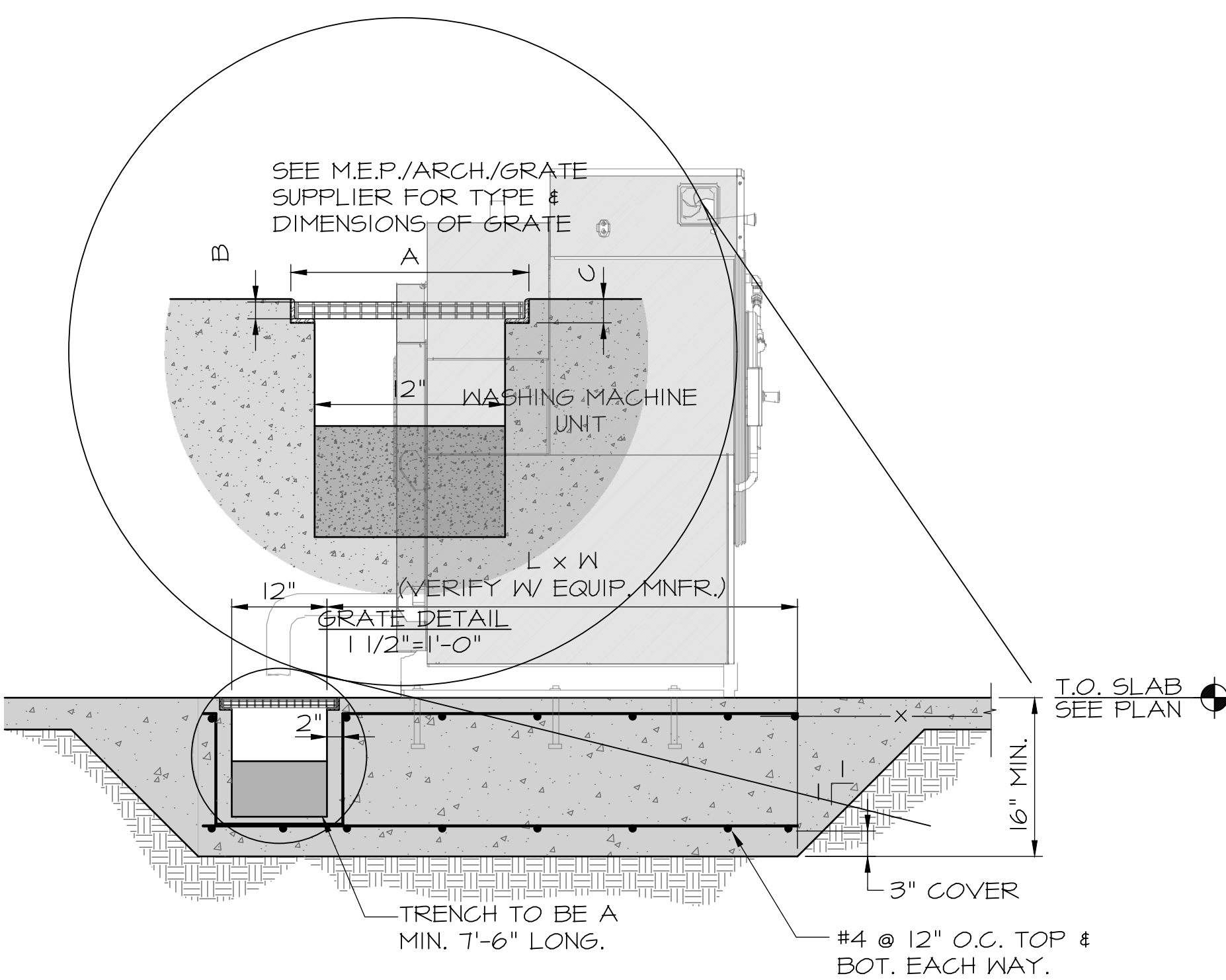
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TYP. PRECAST LINTEL @ CONDUIT BANK DETAIL

3
S7

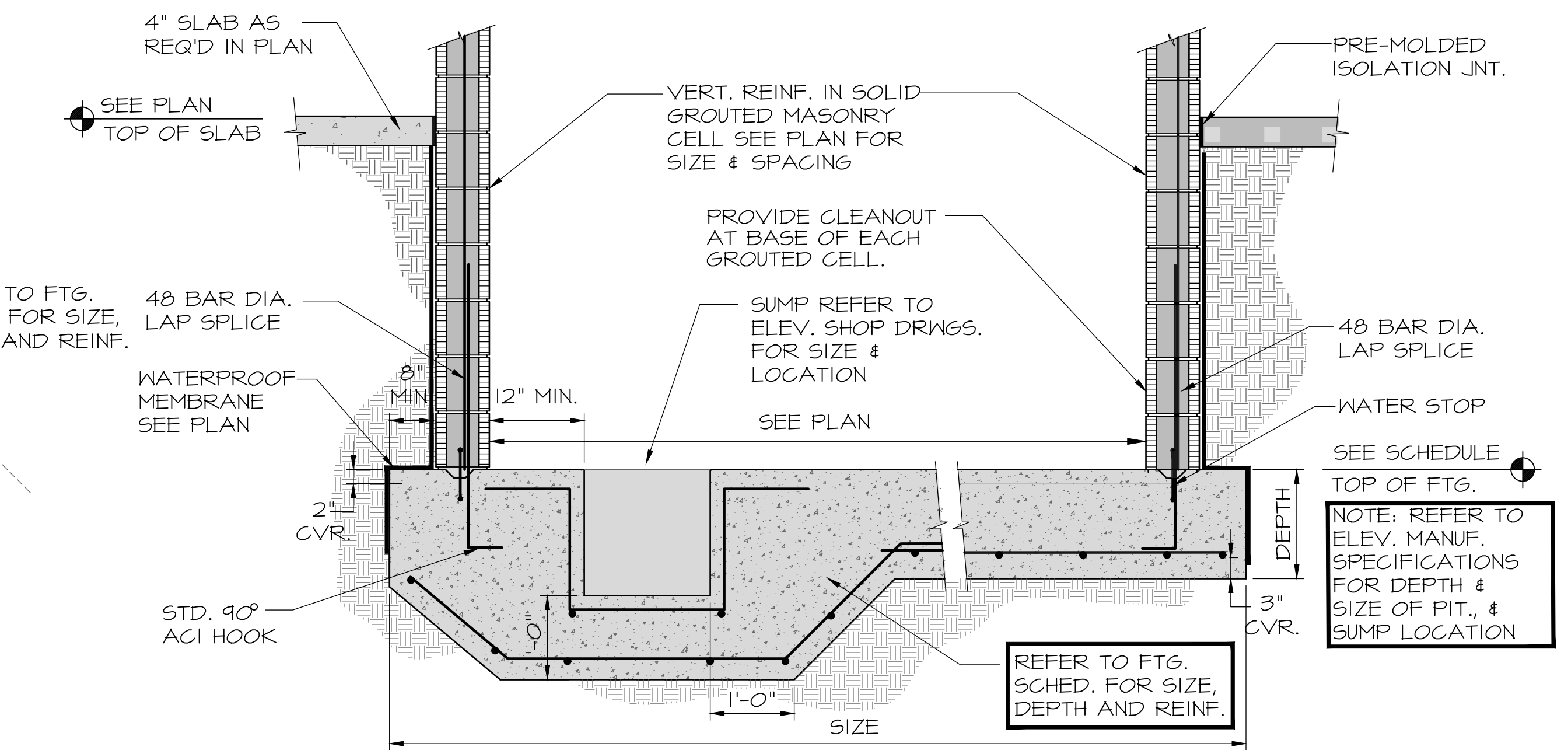
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WASH'G MACHINE PAD & TRENCH DRAIN DETAIL

6
S7

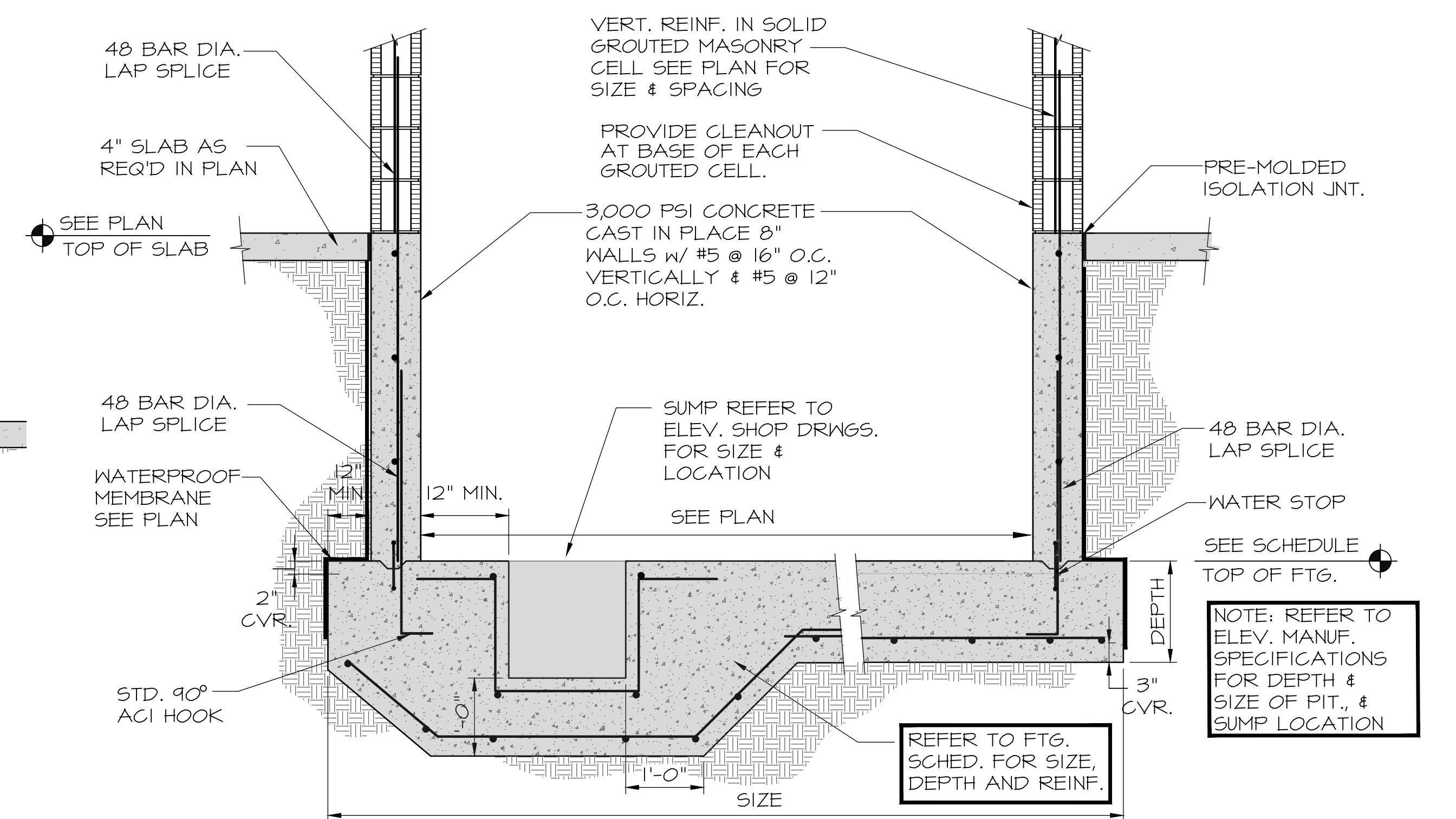
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ELEVATOR PIT DETAIL

7
S7

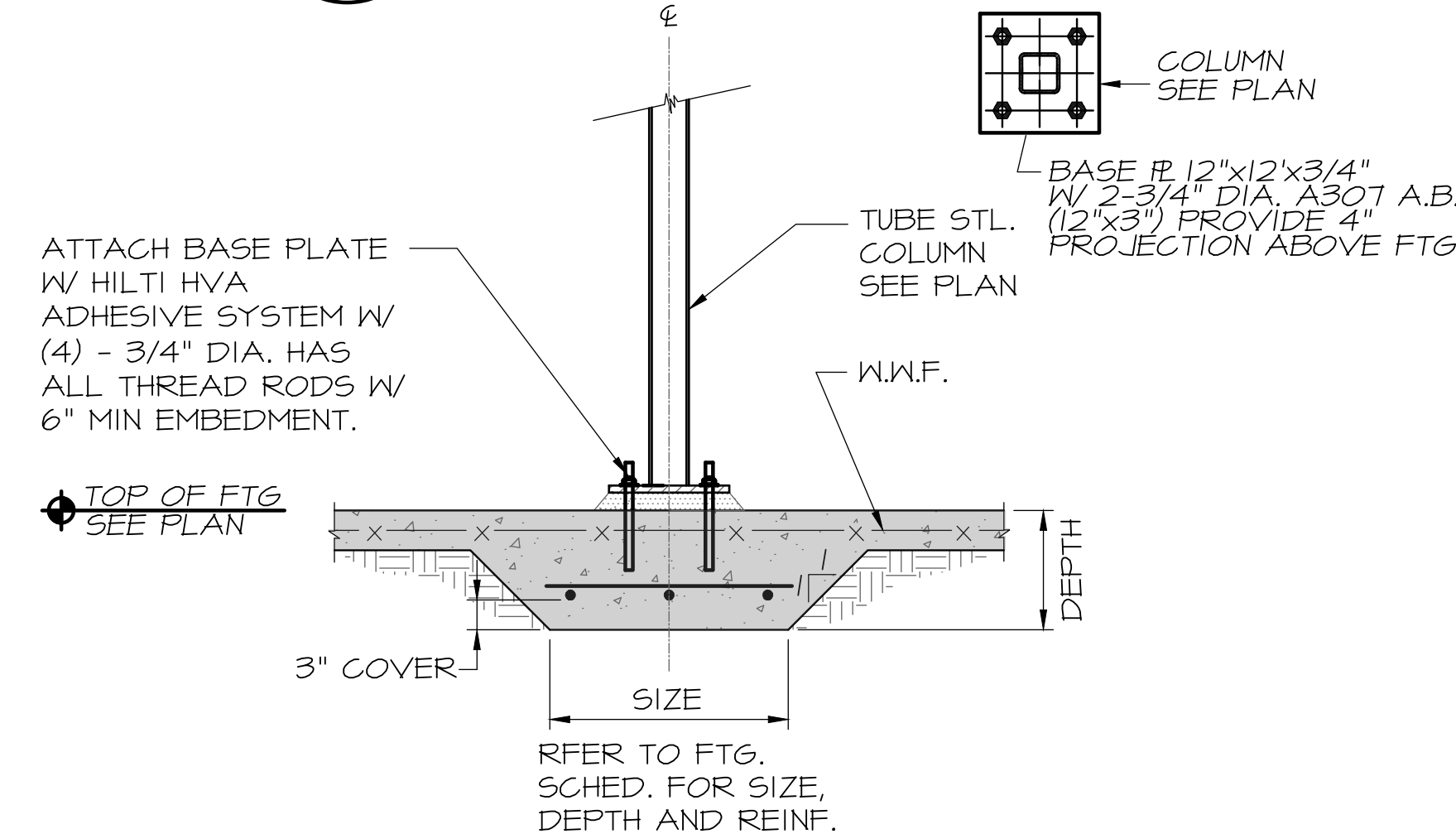
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ELEVATOR PIT DETAIL ALT.

8
S7

SCALE: 3/4"=1'-0"



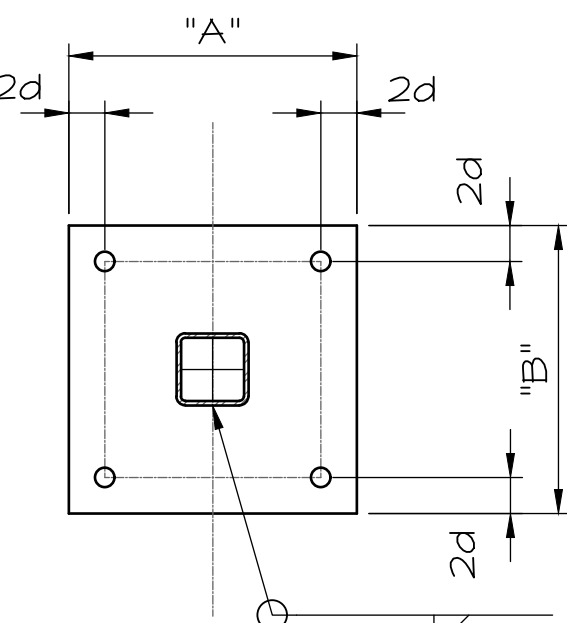
SECTION

9
S7

SCALE: 3/4"=1'-0"

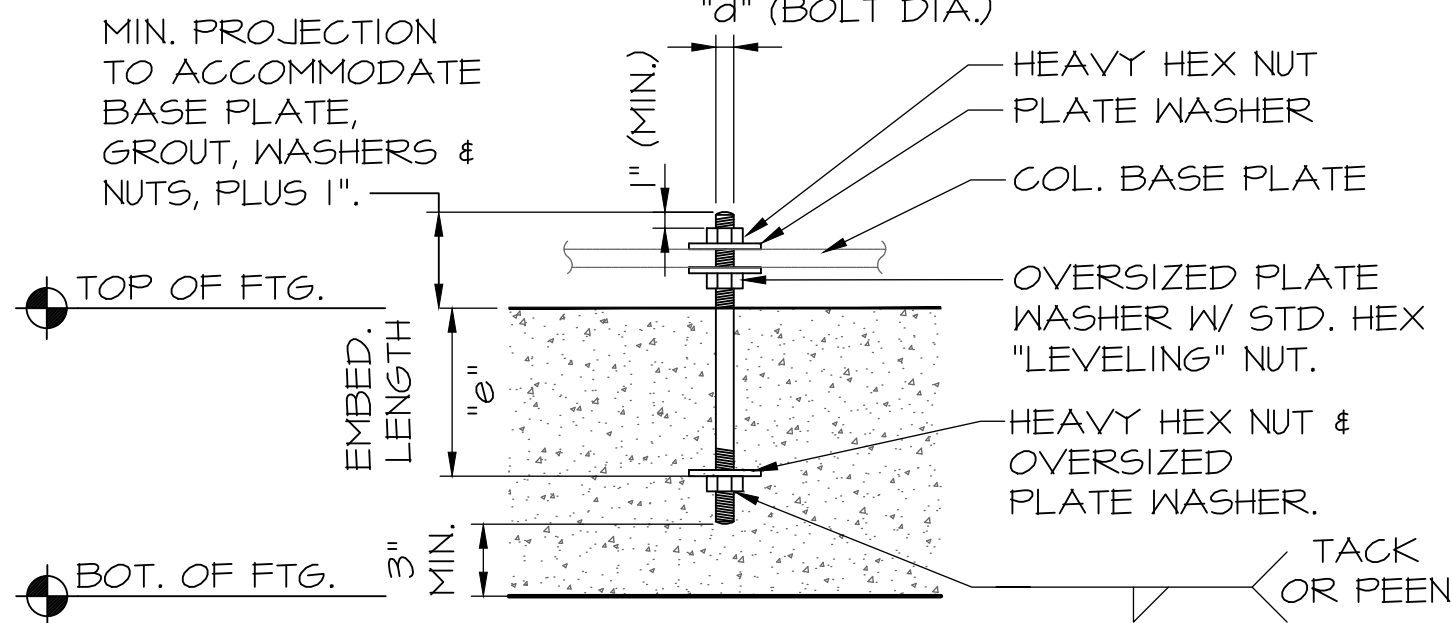
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HB **HB ASSOCIATES, LLC**
STRUCTURAL/CIVIL ENGINEERING
FLORIDA CA # 7045
HARRY BRUMLEY III, PE # 41837
377 MAITLAND AVENUE, SUITE # 2007
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STEEL COLUMN & BASE PLATE SCHEDULE									
COL. MARK	COL. SIZE	B.P. MARK	BASE PLATE SIZE (INCHES)			ANCHOR BOLTS (INCHES)			COMMENTS
			A	B	T	d	•	•	
	4X4 T.S.		12	12	1	4	3/4	8	
	8X8 T.S.		16	16	1 3/8	4	1 1/8	12	
	10X10 T.S.		18	18	1 1/2	4	1 1/8	12	



BASE PLATE
DETAIL

N.T.S.

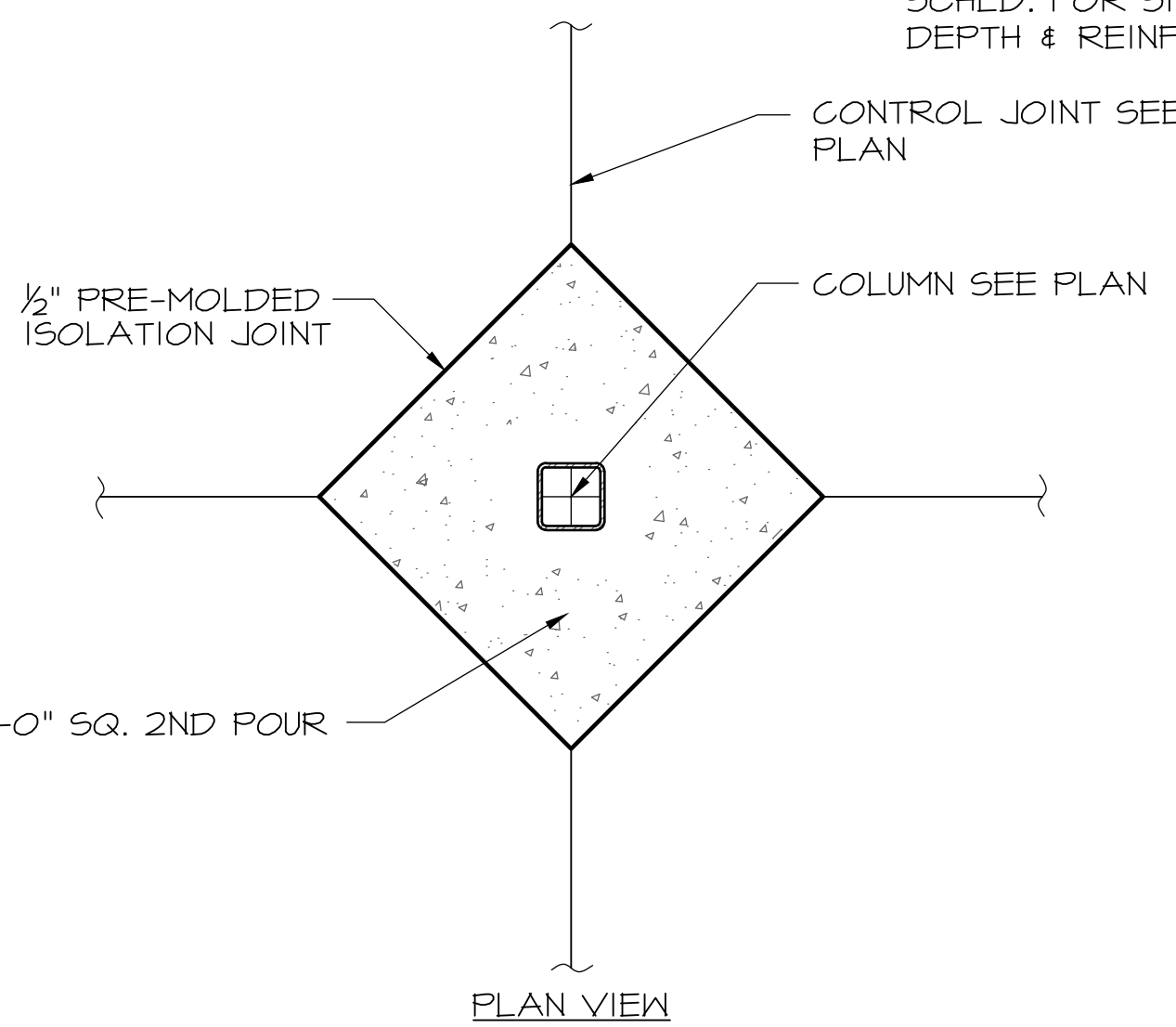
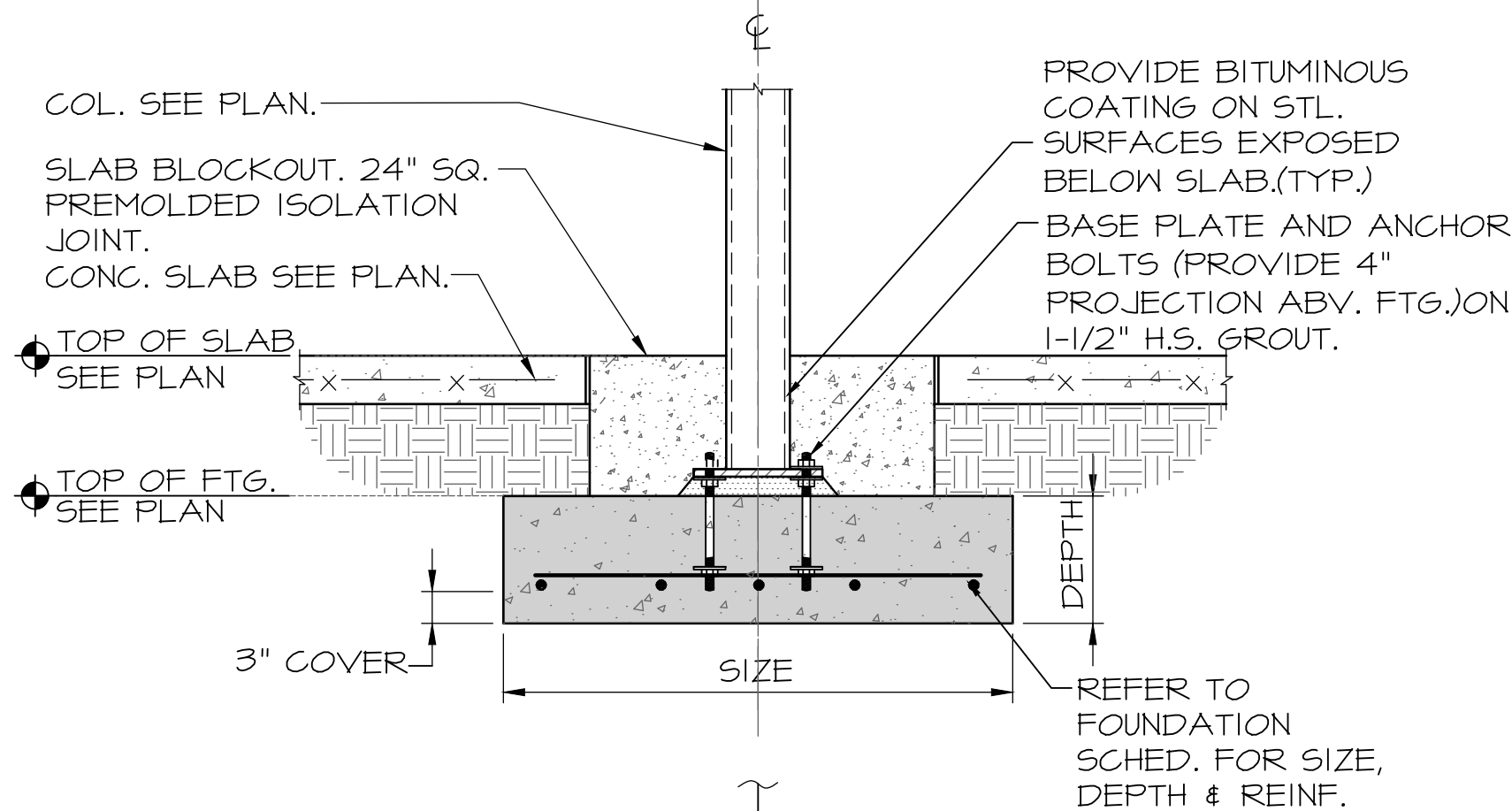


TYP. ANCHOR
BOLT DETAIL

N.T.S.

NOTE:

- UNLESS NOTED OTHERWISE, ALL BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT CONDITION" AS DEFINED BY AISC, AFTER THE CONCRETE IS AT LEAST 14 DAYS OLD.
- THE HOLE IN THE BASE PLATE AND PLATE WASHER SHALL BE 5/16" LARGER THAN THE BOLT DIAMETER. PLATE WASHERS SHALL BE CIRCLE OR SQUARES.
- USE ASTM F1554 GRADE 55 KSI THREADED RODS



1 COLUMN DETAIL

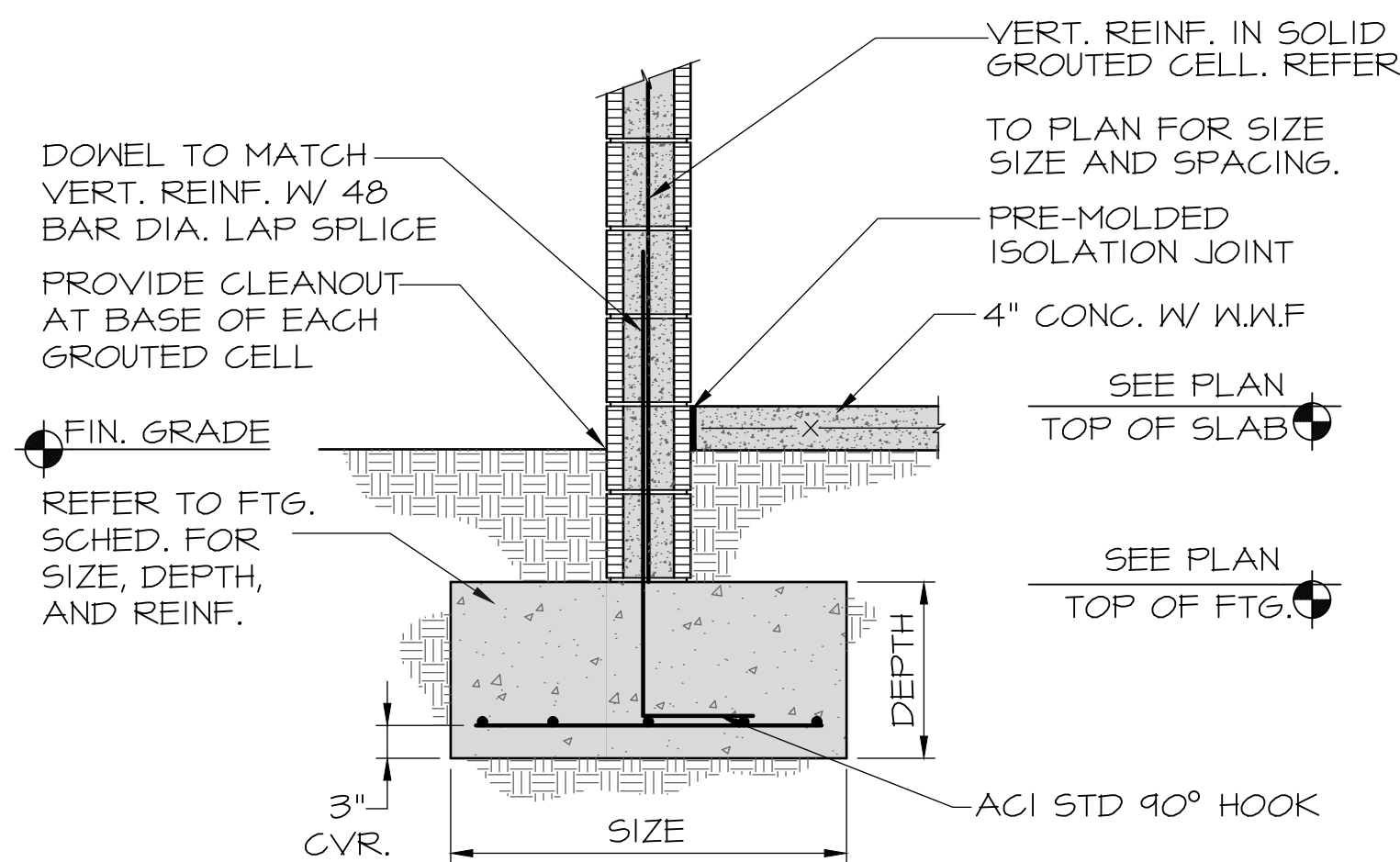
S8

SCALE: 3/4"=1'-0"

2 BEAM TO BEAM/COL SHEAR CONN.

S8

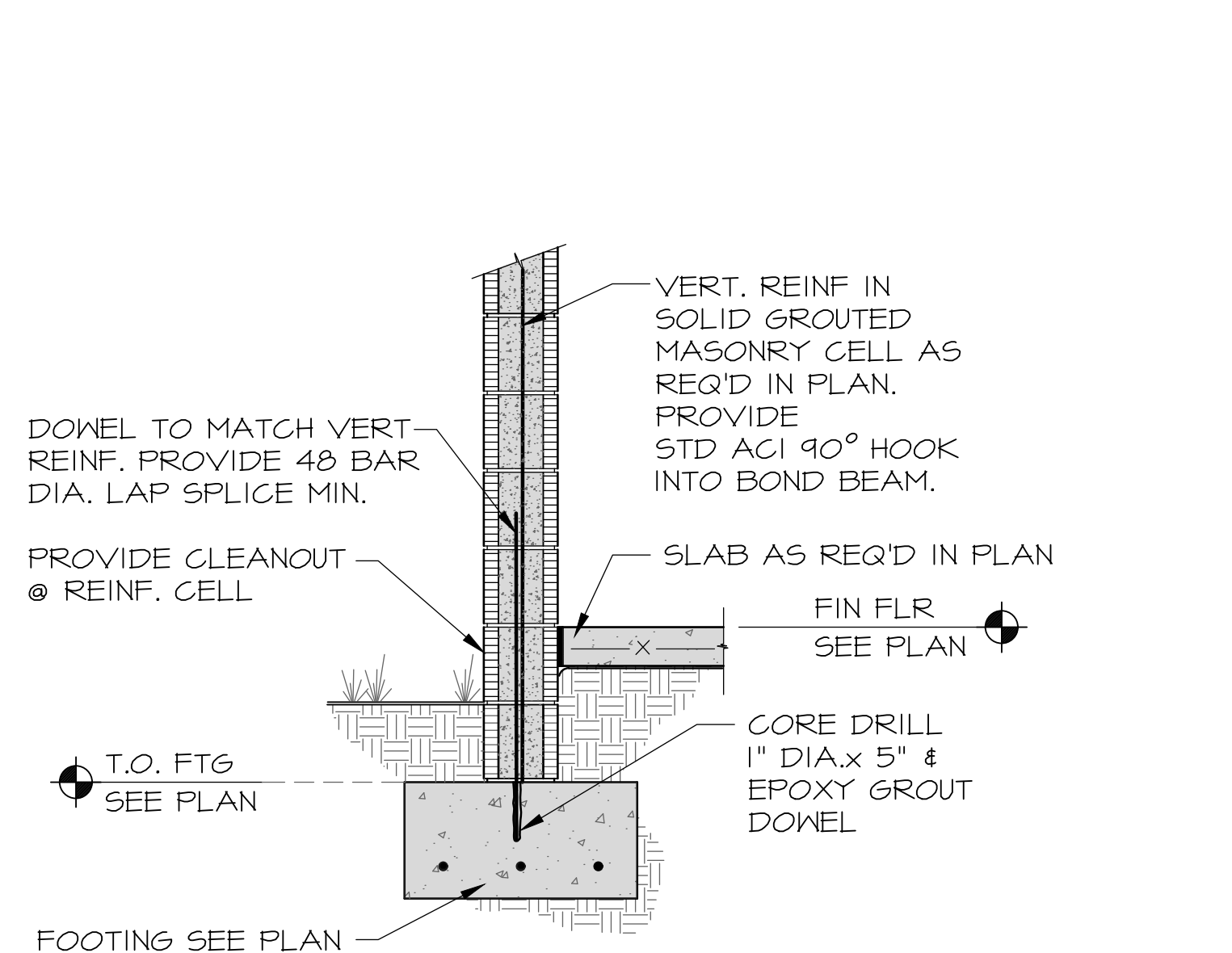
SCALE: 3/4"=1'-0"



4 TYP. EXT. WALL-FTG. DETAIL

S8

SCALE: 3/4"=1'-0"



7 MISSED DOWEL
RETRO-FIT DETAIL

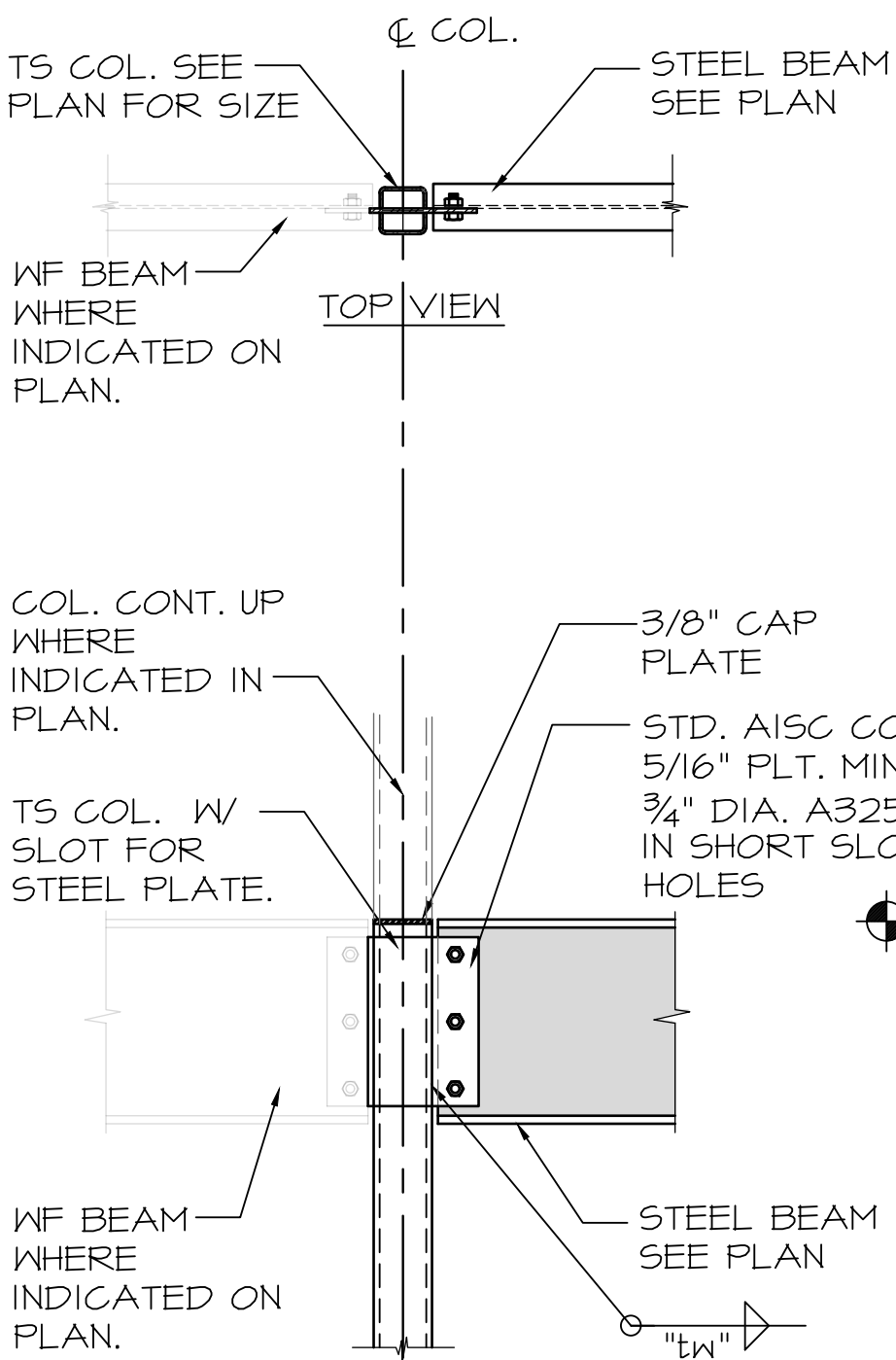
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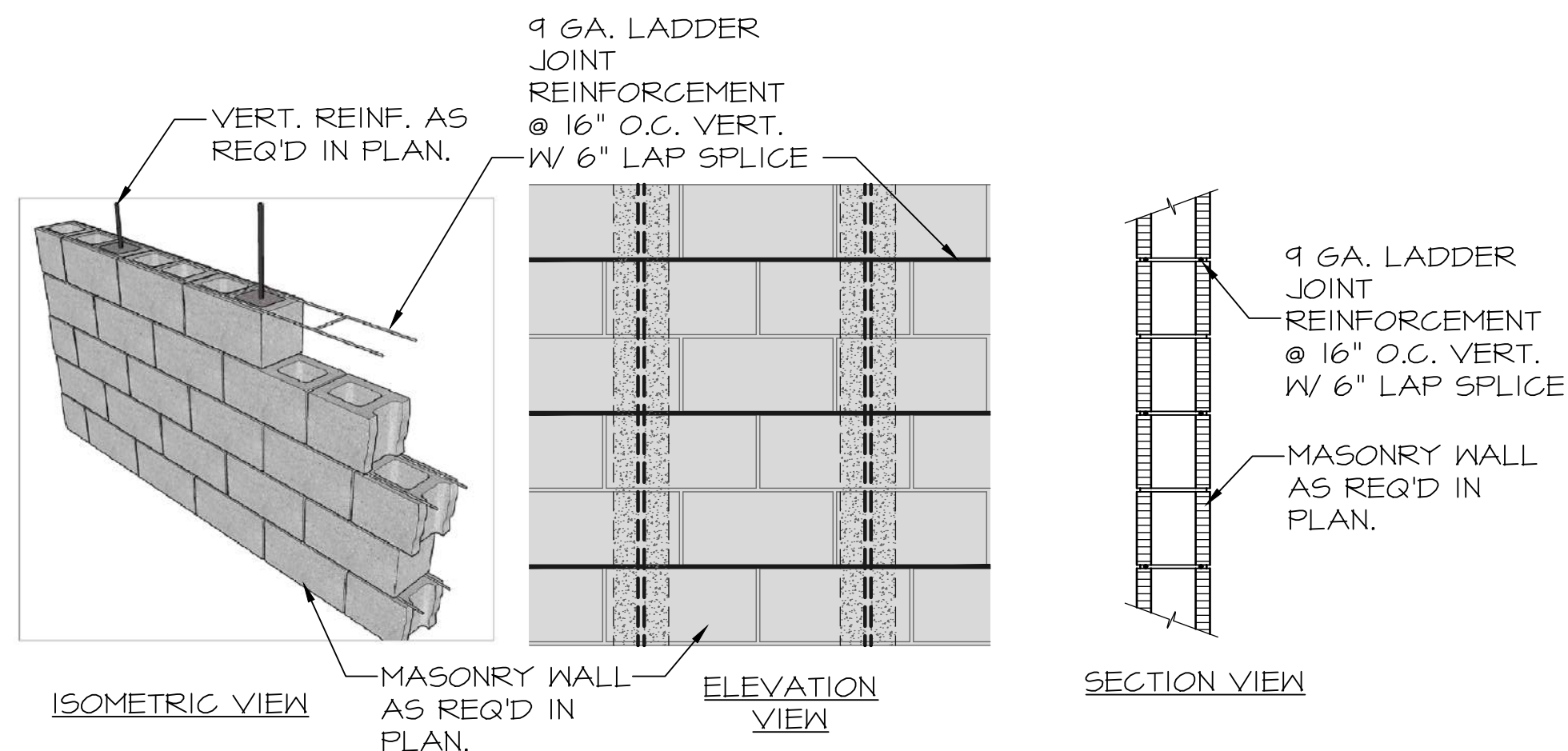
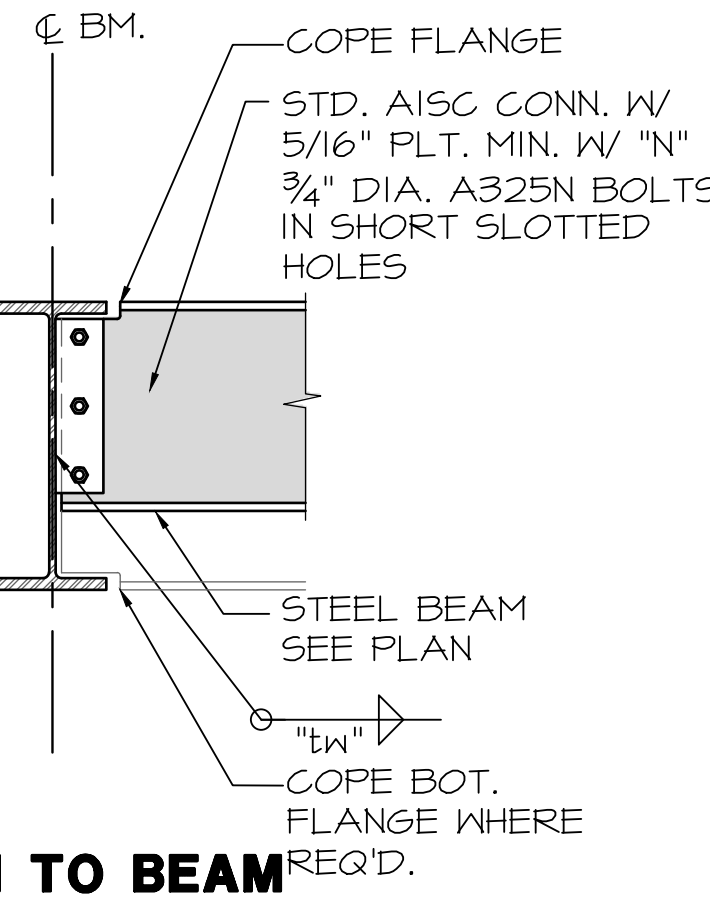


SINGLE PLATE SHEAR
CONNECTIONS

BEAM	'tw' WELD	'N' BOLTS	ALLOW. LOAD
W8, W10	3/16"	2	8.2 K
W12, W14	3/16"	3	16.3 K
W16, W18	3/16"	4	26.1 K
W21, W24	3/16"	6	46.3 K
W27	3/16"	7	56.4 K



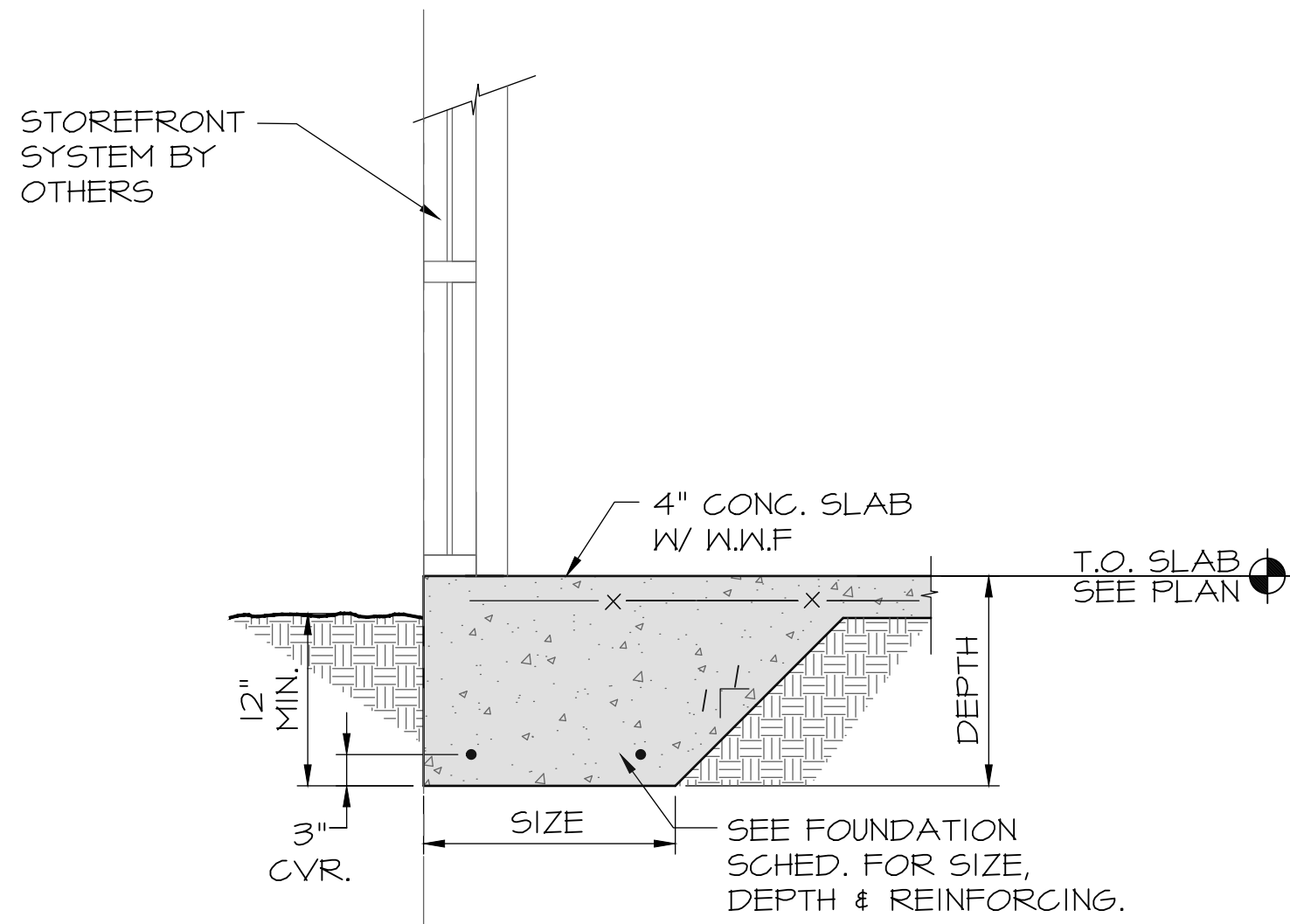
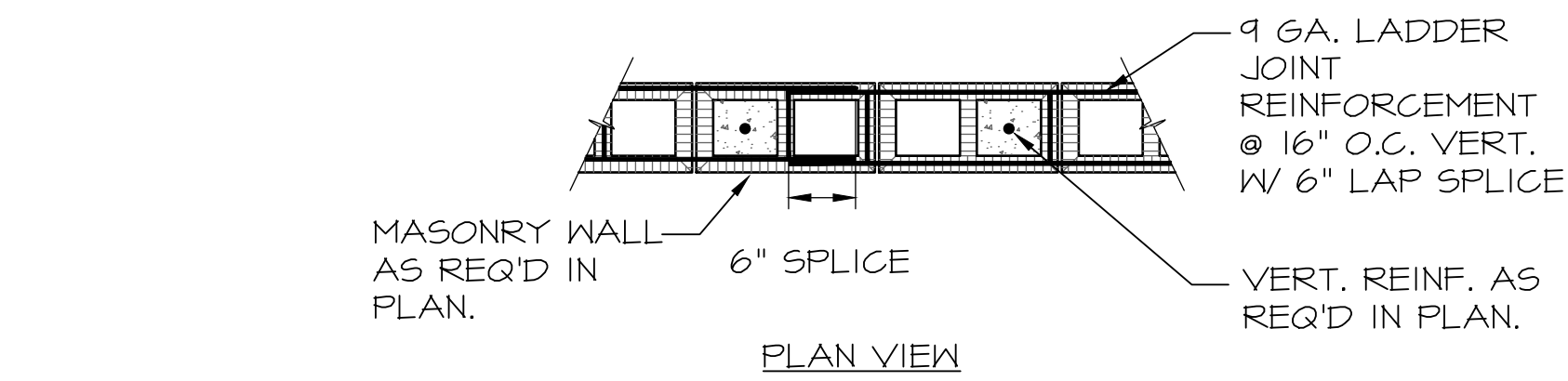
BEAM TO BEAM



3 MAS. JNT REINF. DETAIL

S8

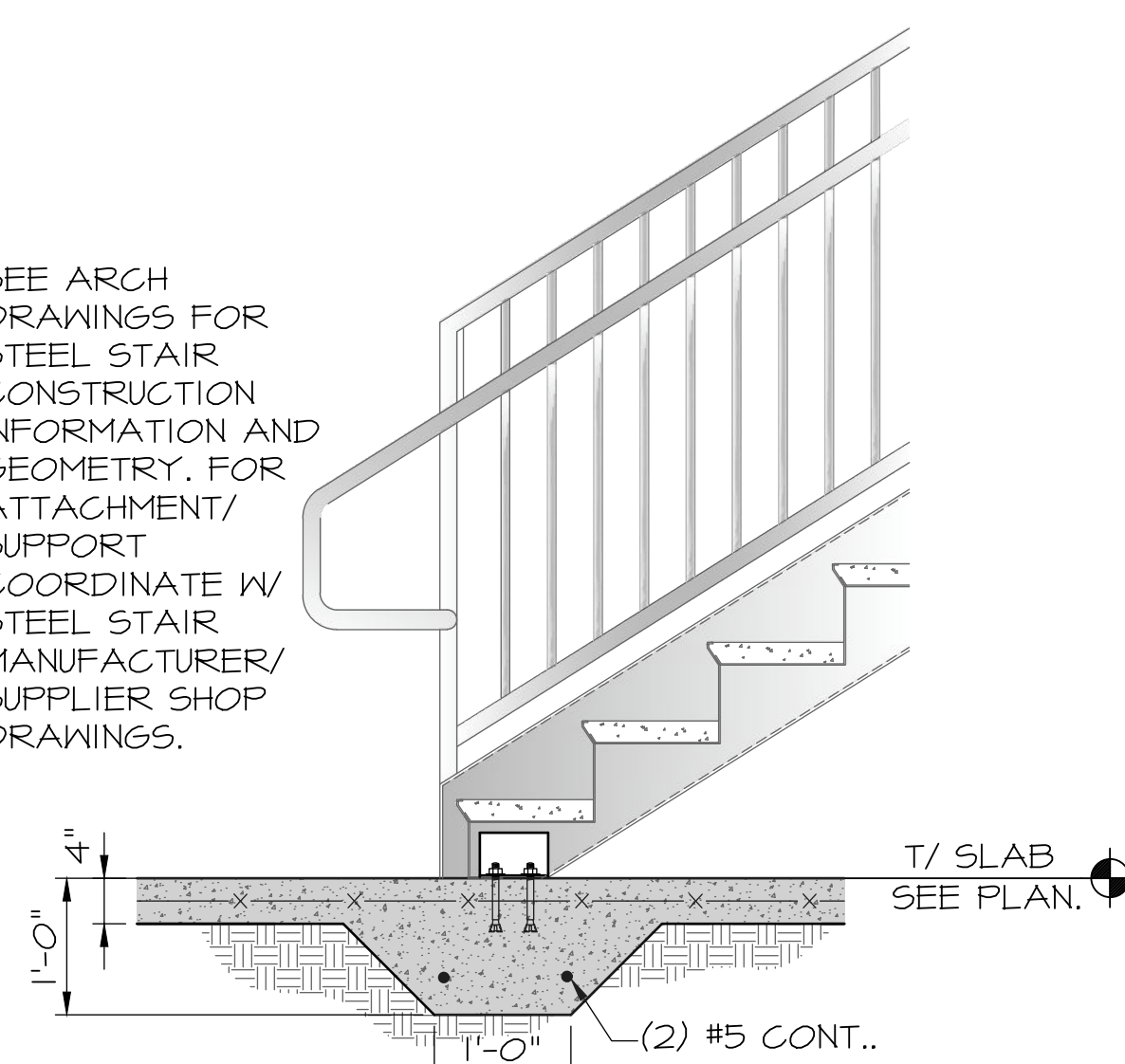
SCALE: 3/4"=1'-0"



6 STOREFRONT FTG DETAIL

S8

SCALE: 3/4"=1'-0"



8 STEEL STAIR @
SLAB ON GRADE

S8

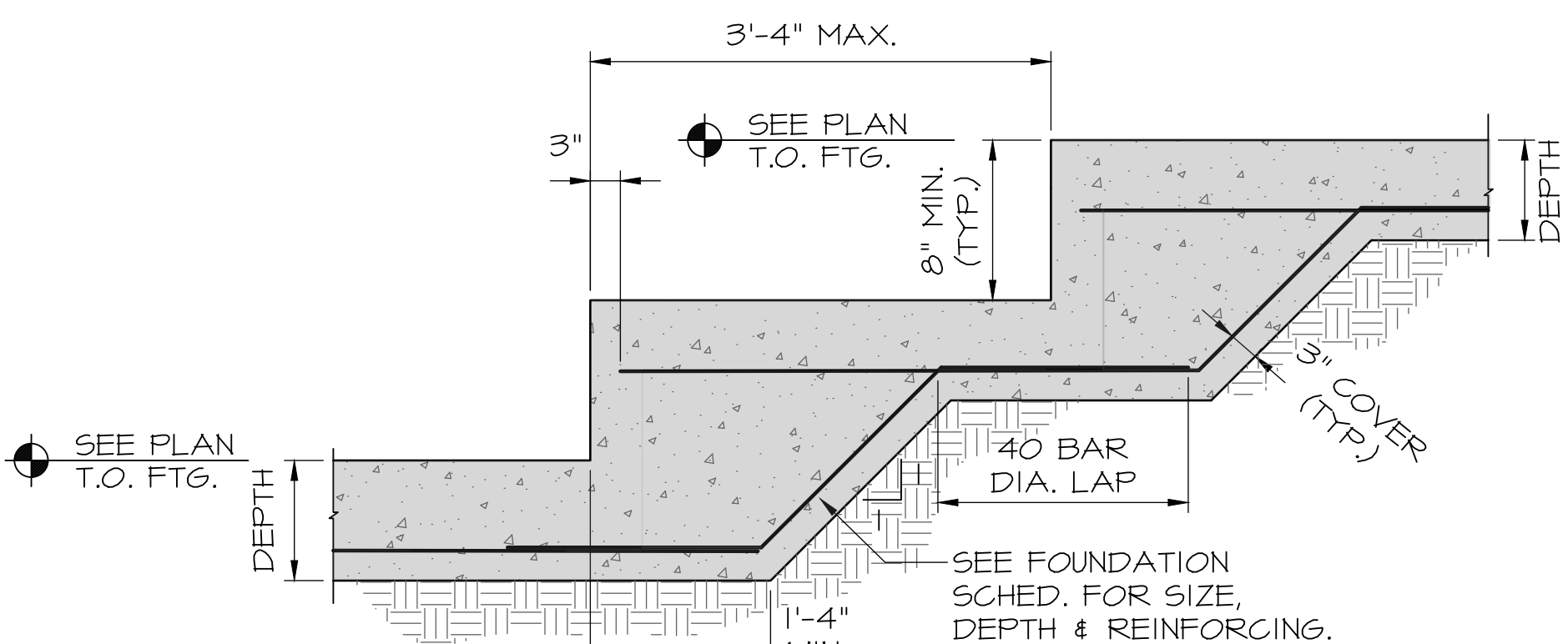
SCALE: 3/4"=1'-0"



9 STEPPED FOOT'G DETAIL

S8

SCALE: 3/4"=1'-0"



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SUITES
BY MARIOTT

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LAKE CITY HOTELS, INC.
NW COMMERCE DRIVE, LAKE CITY, FLORIDA

REVISION

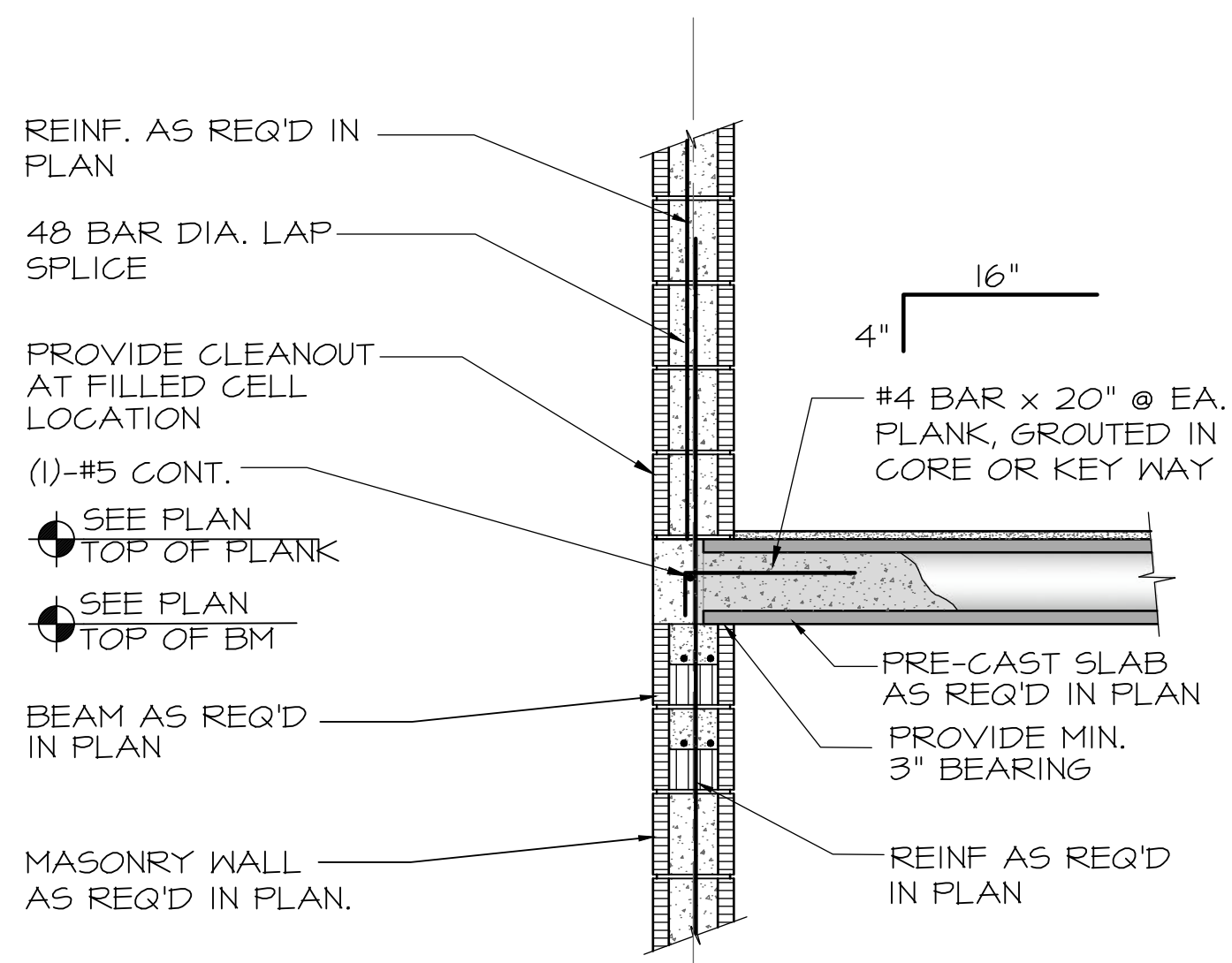
JOB NO.

DATE

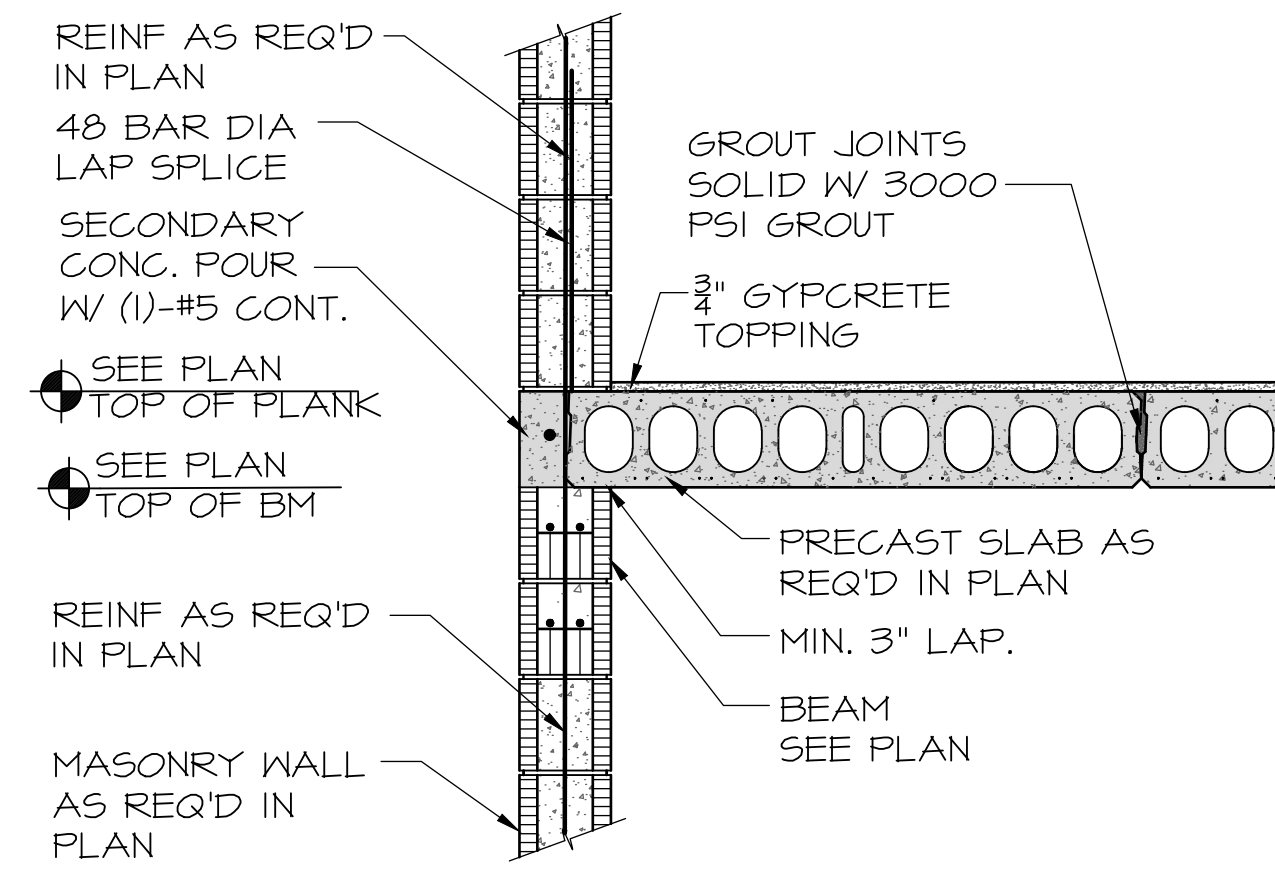
SHEET

S8

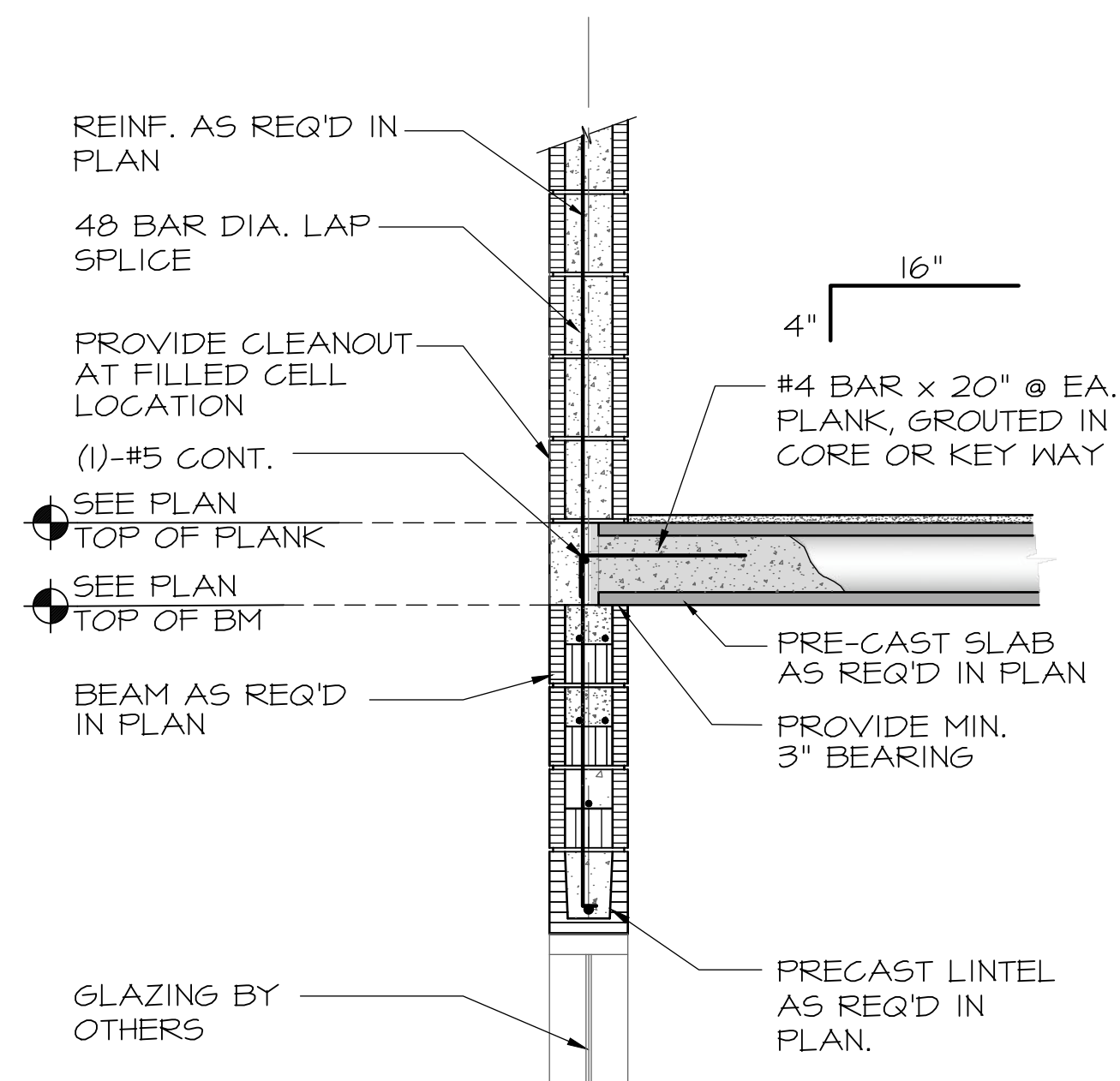
13 OF



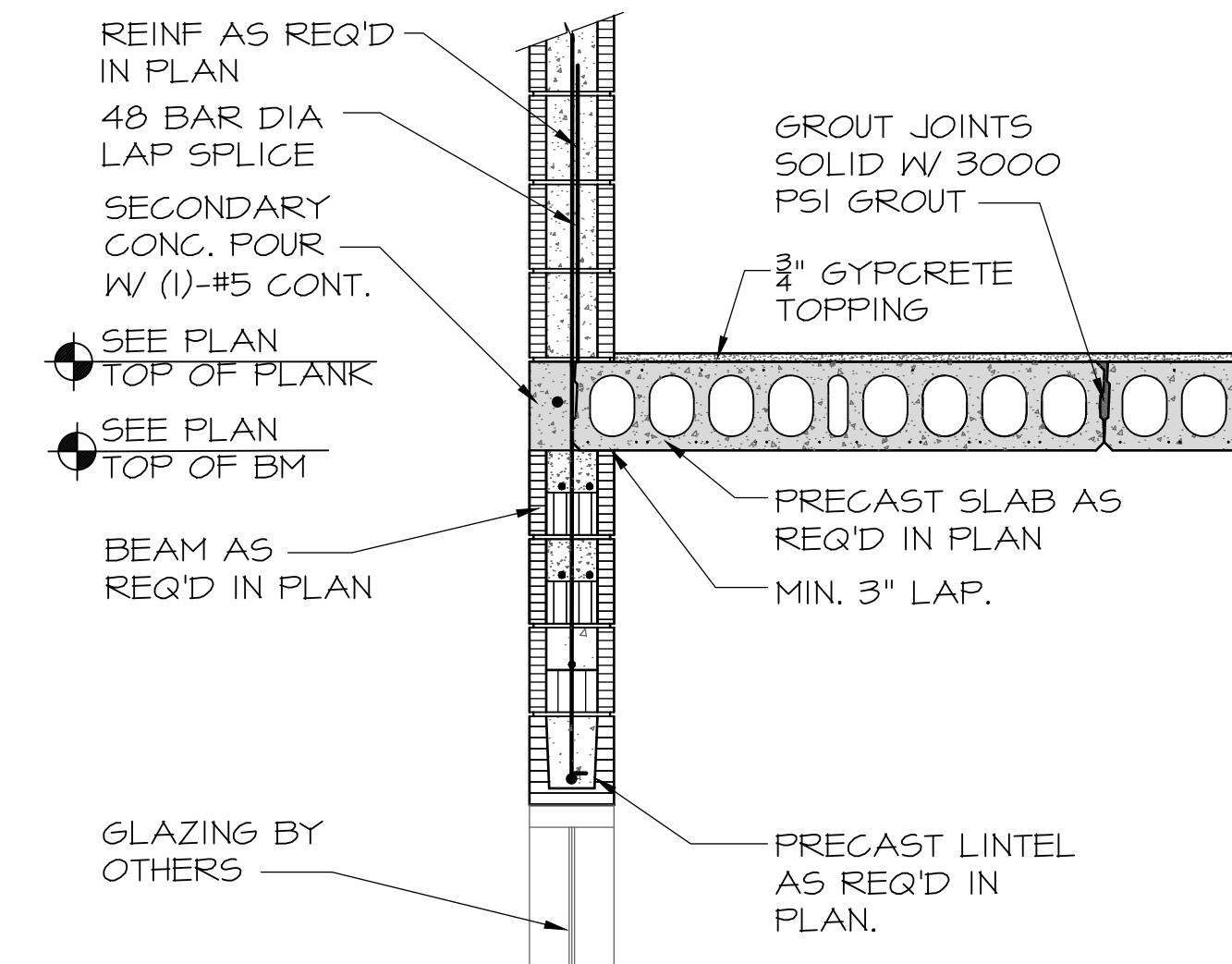
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S9 **HOLLOWCORE PLANK**
EXTER. BRG DETAIL
SCALE: 3/4"=1'-0"



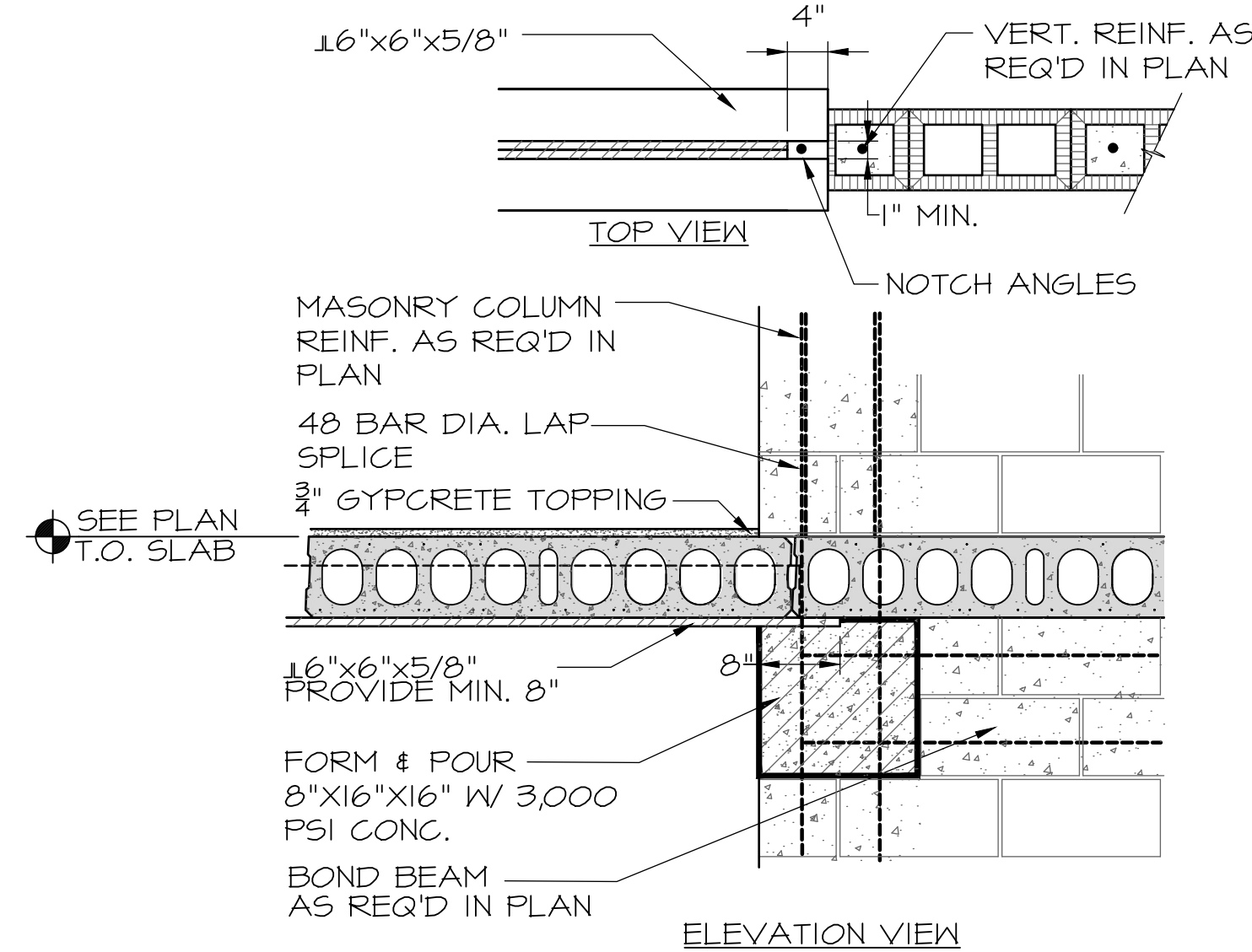
2
S9 **HOLLOWCORE PLANK**
BRG. WALL DETAIL
SCALE: 3/4"=1'-0"



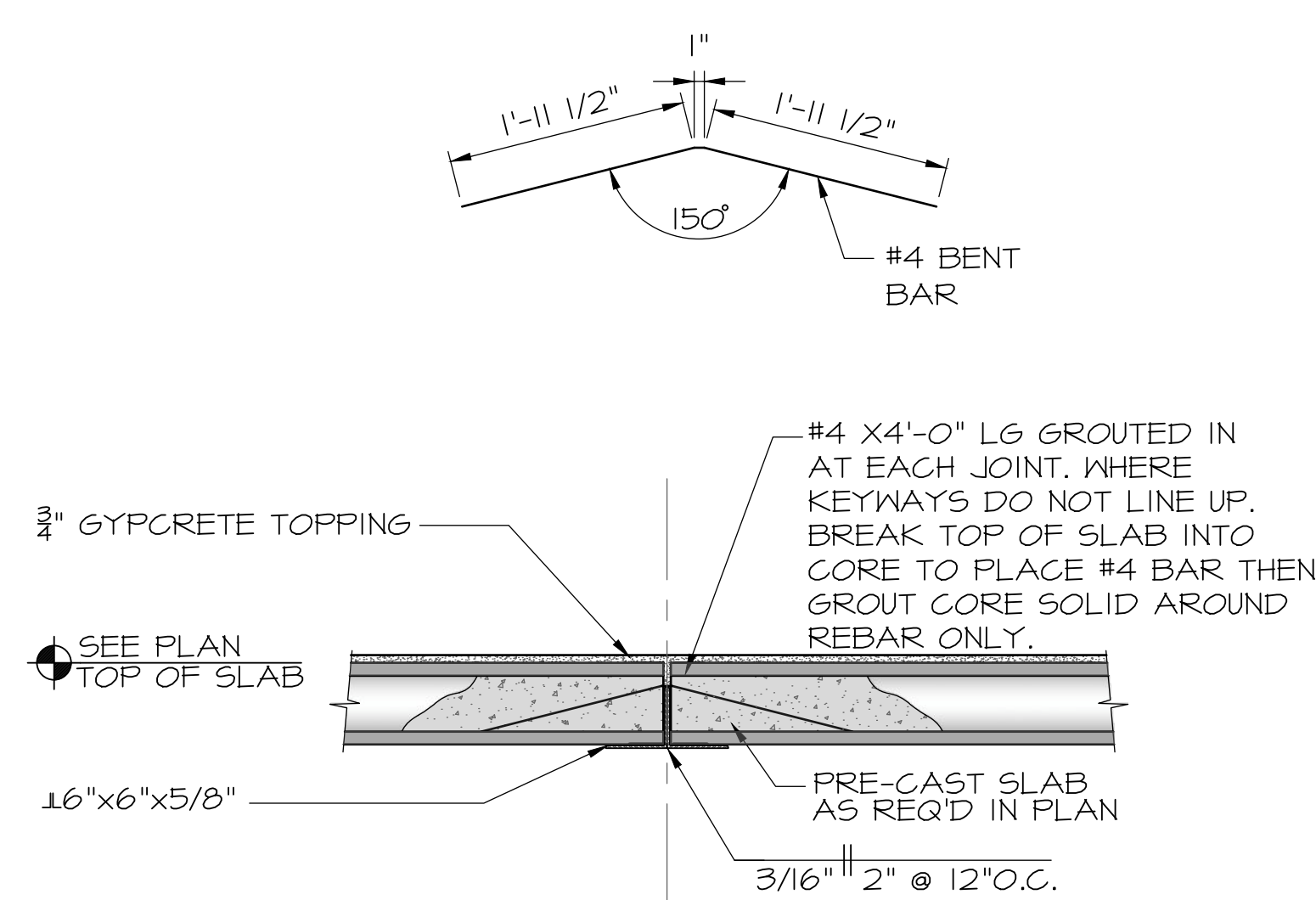
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S9 **HOLLOWCORE PLANK**
EXTER. BRG DETAIL
SCALE: 3/4"=1'-0"



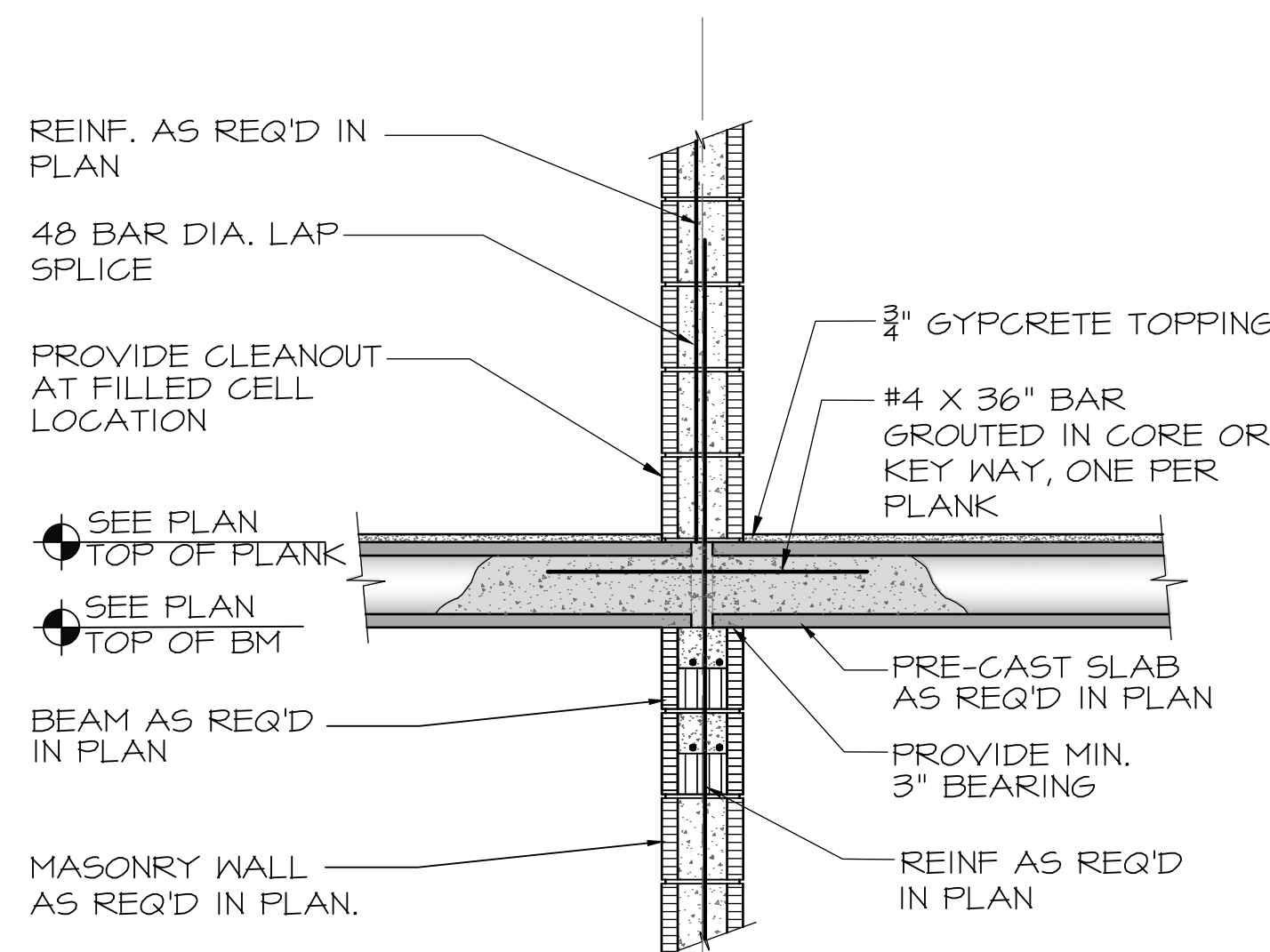
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S9 **HOLLOWCORE PLANK**
BRG. WALL DETAIL
SCALE: 3/4"=1'-0"



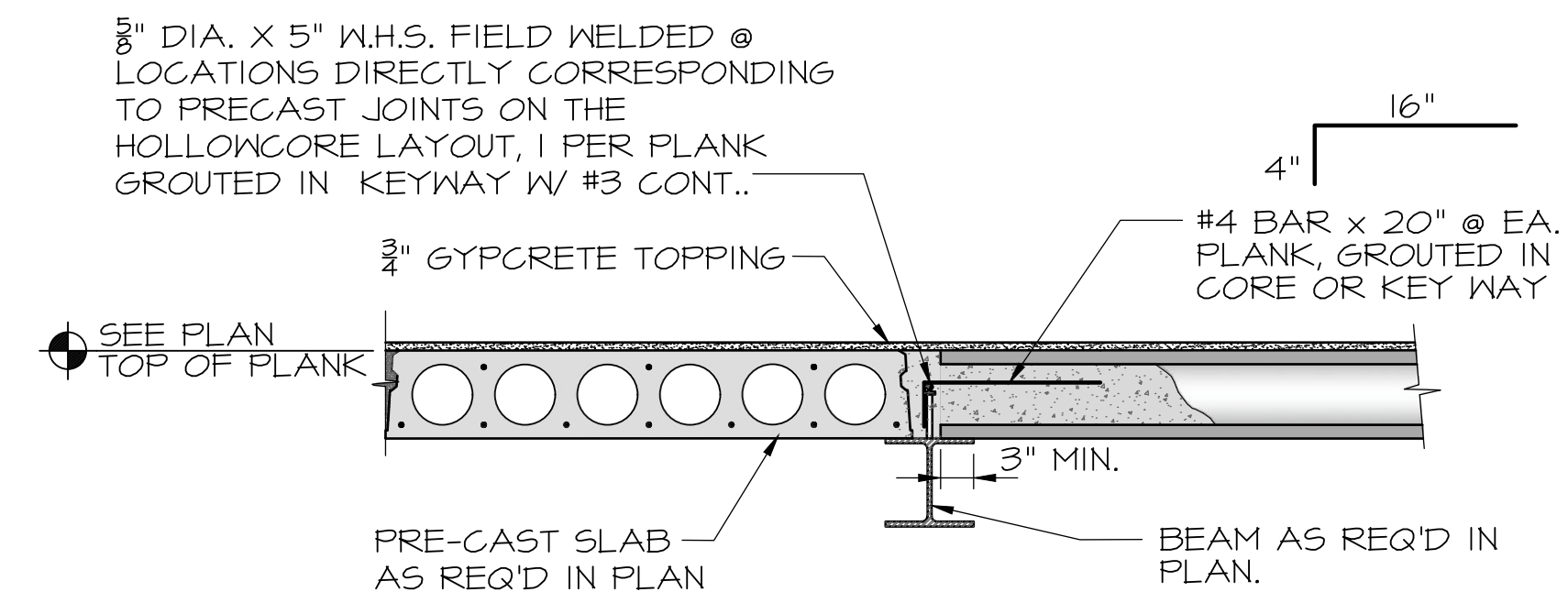
5
S9 **STL ANGLE LINTEL**
@ CORRIDOR.
SCALE: 3/4"=1'-0"



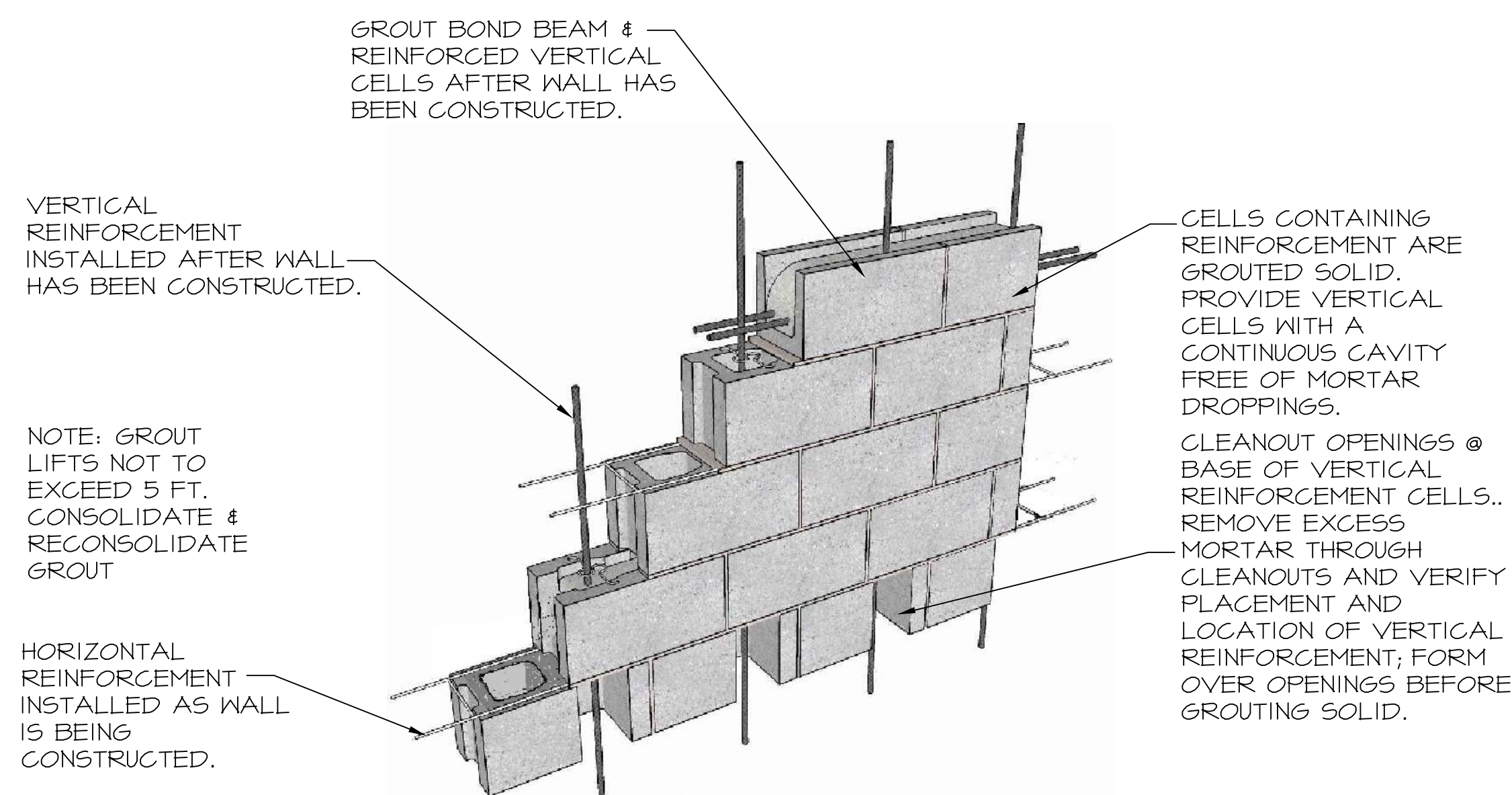
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S9 **STL. LINTEL SECTION**
SCALE: 3/4"=1'-0"



7
S9 **HOLLOWCORE PLANK**
INTER. BRG DETAIL
SCALE: 3/4"=1'-0"



8
S9 **HOLLOWCORE PLANK**
STEEL BM. BRG DETAIL
SCALE: 3/4"=1'-0"

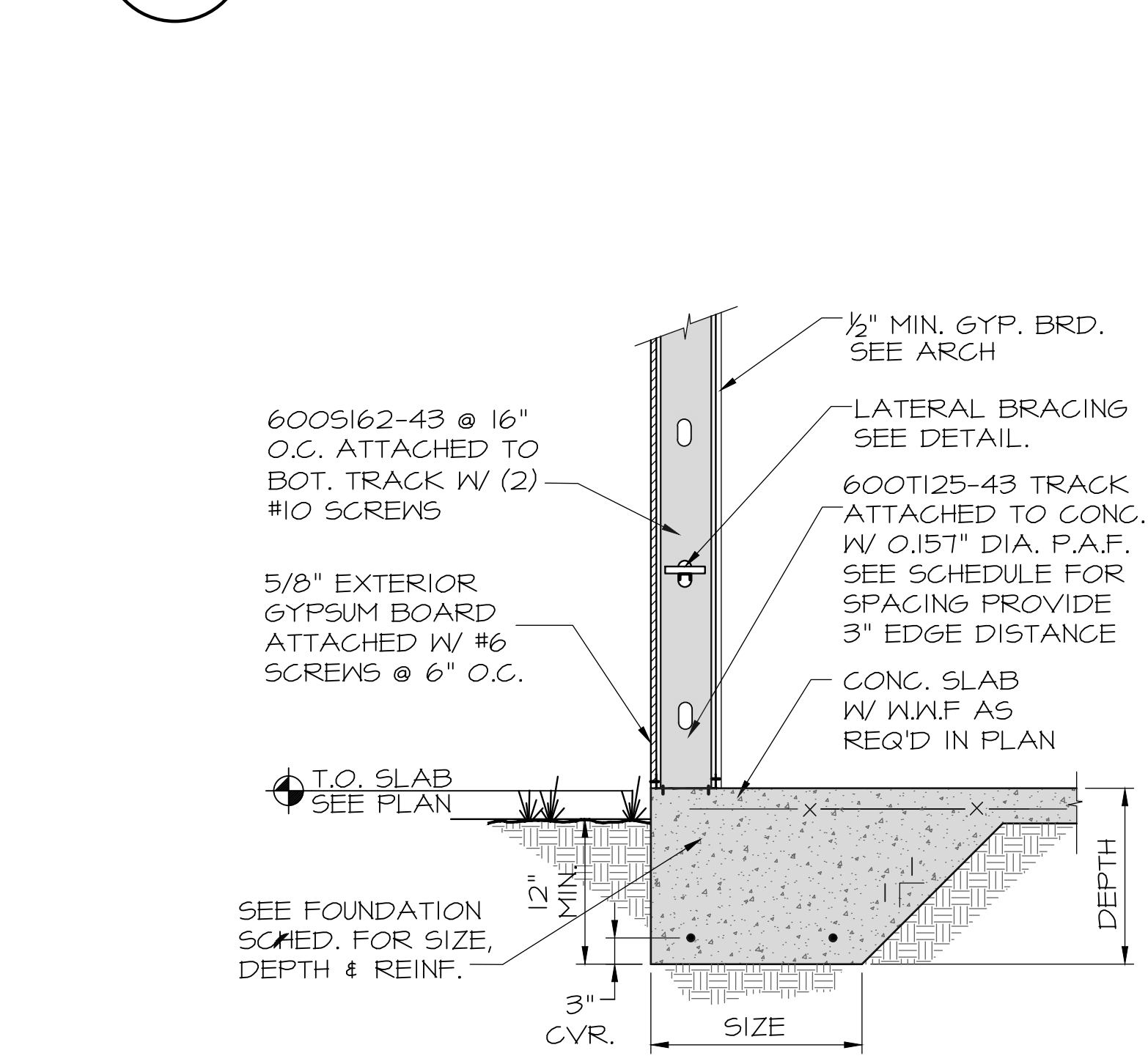
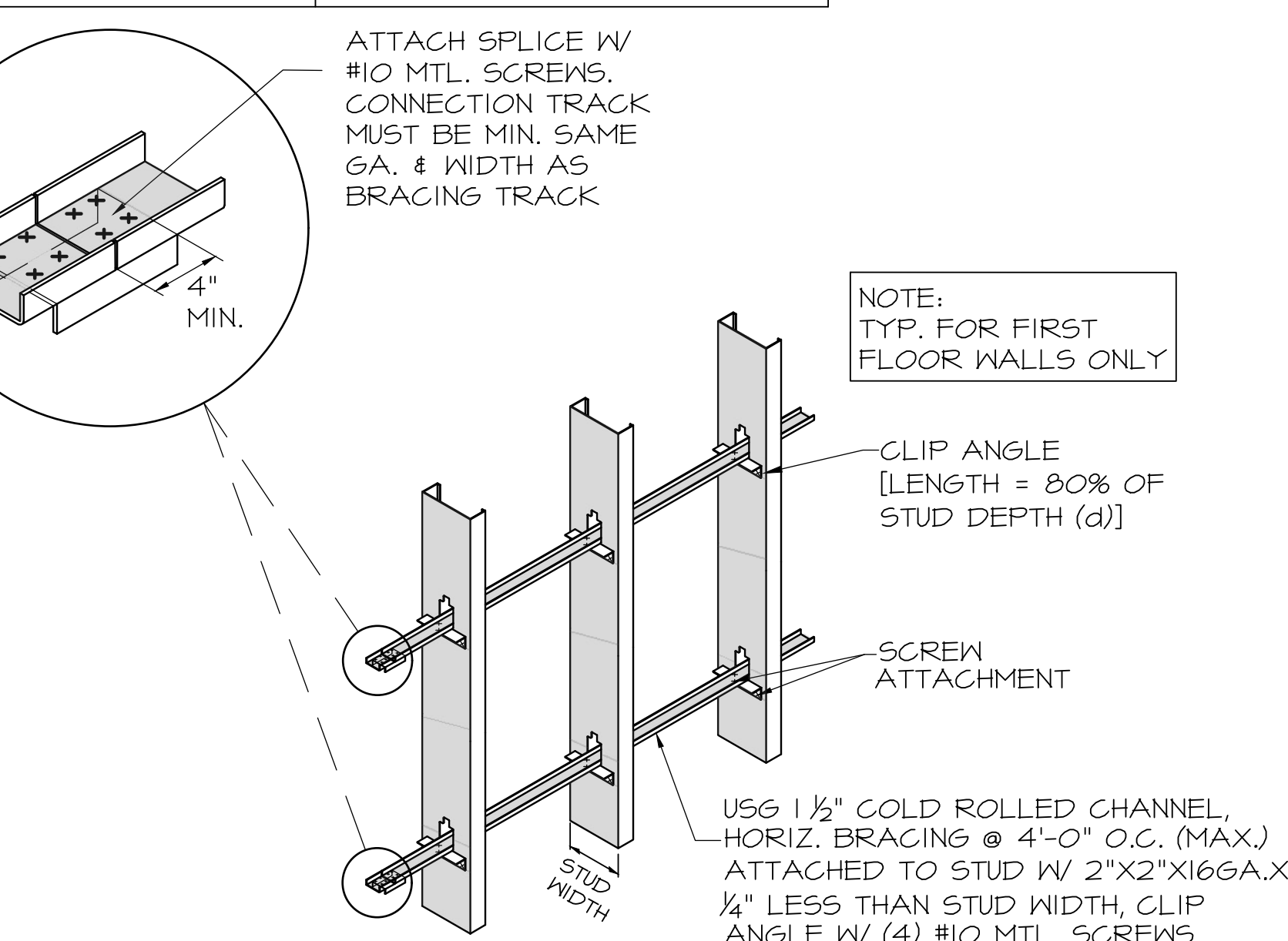
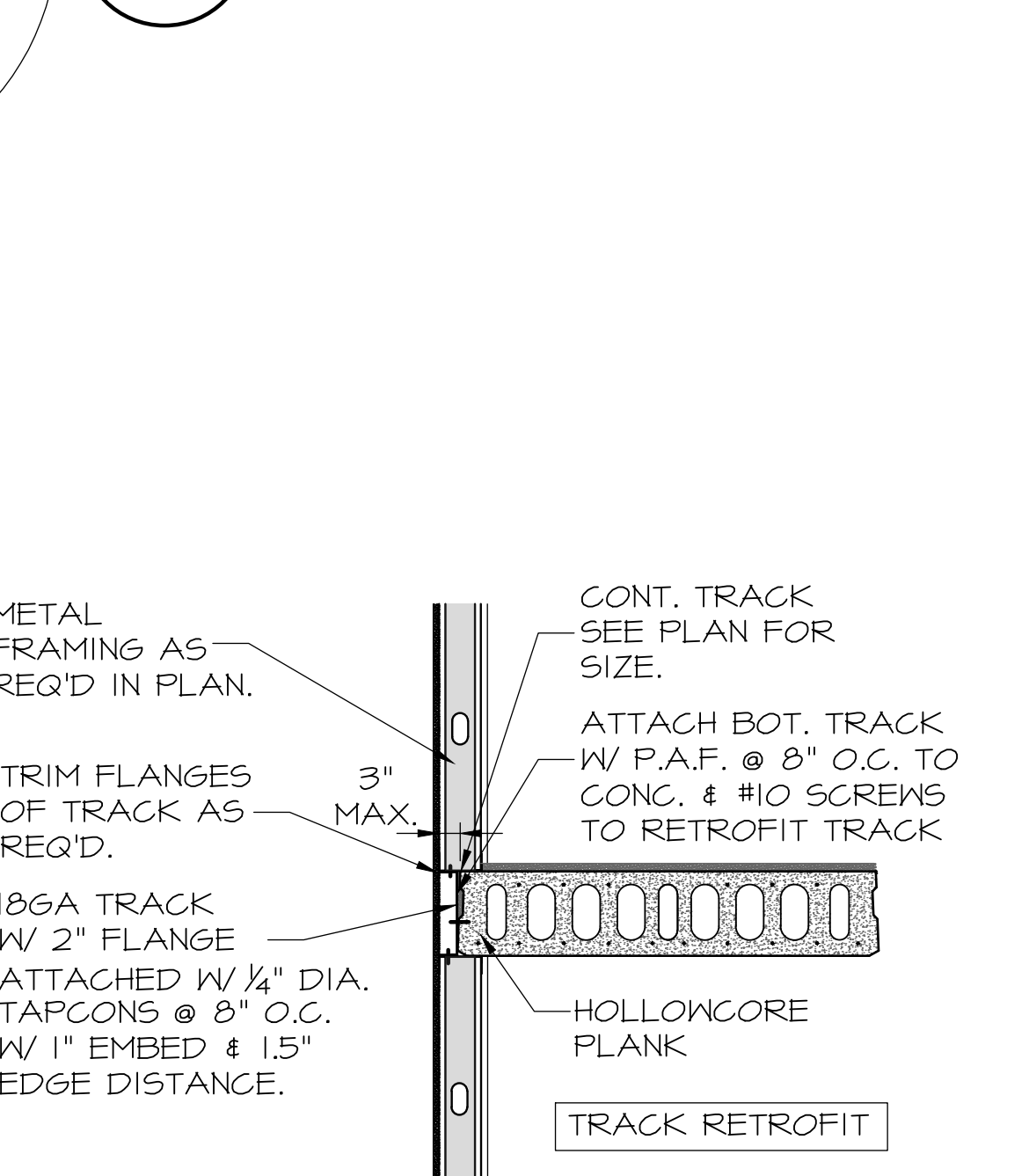
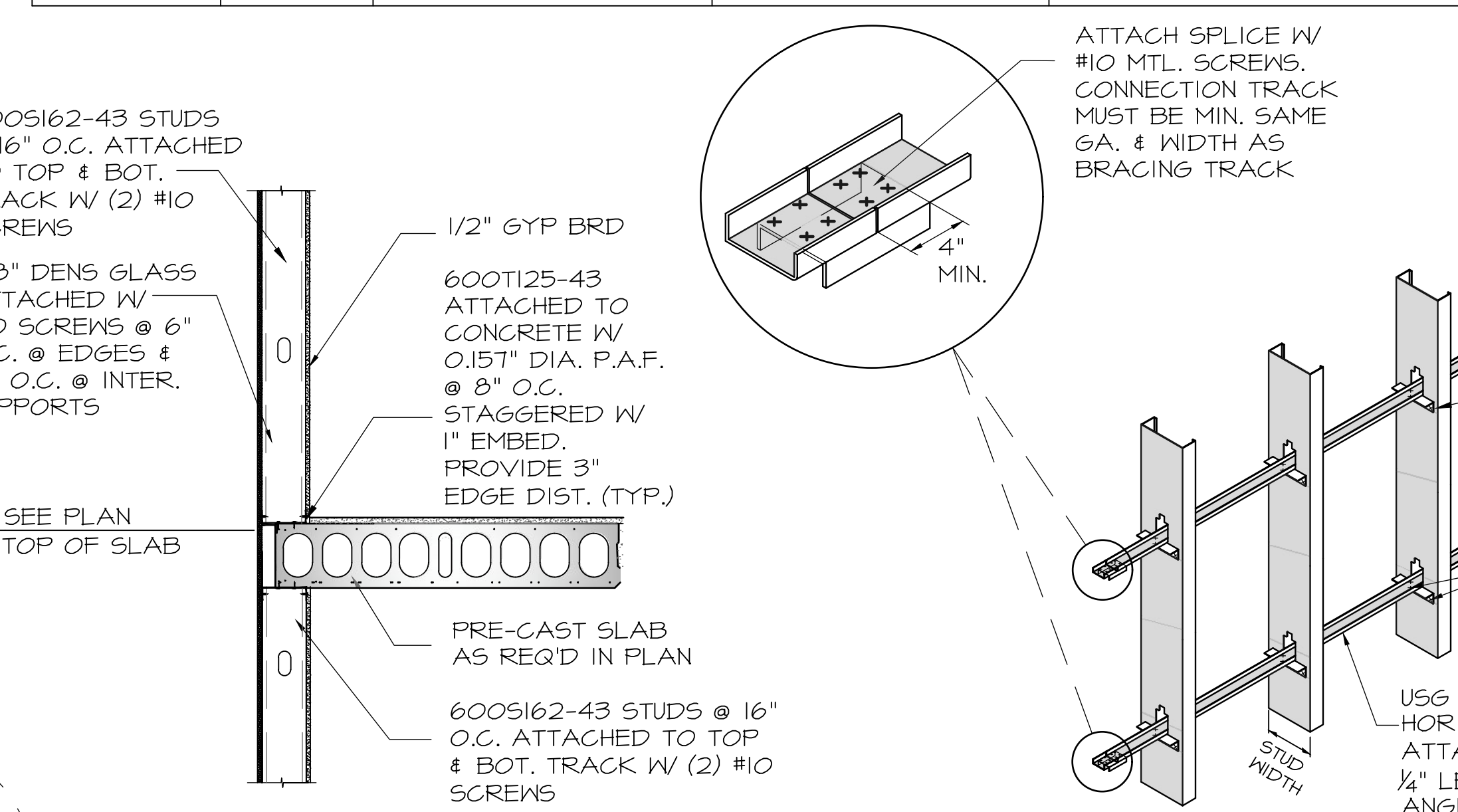
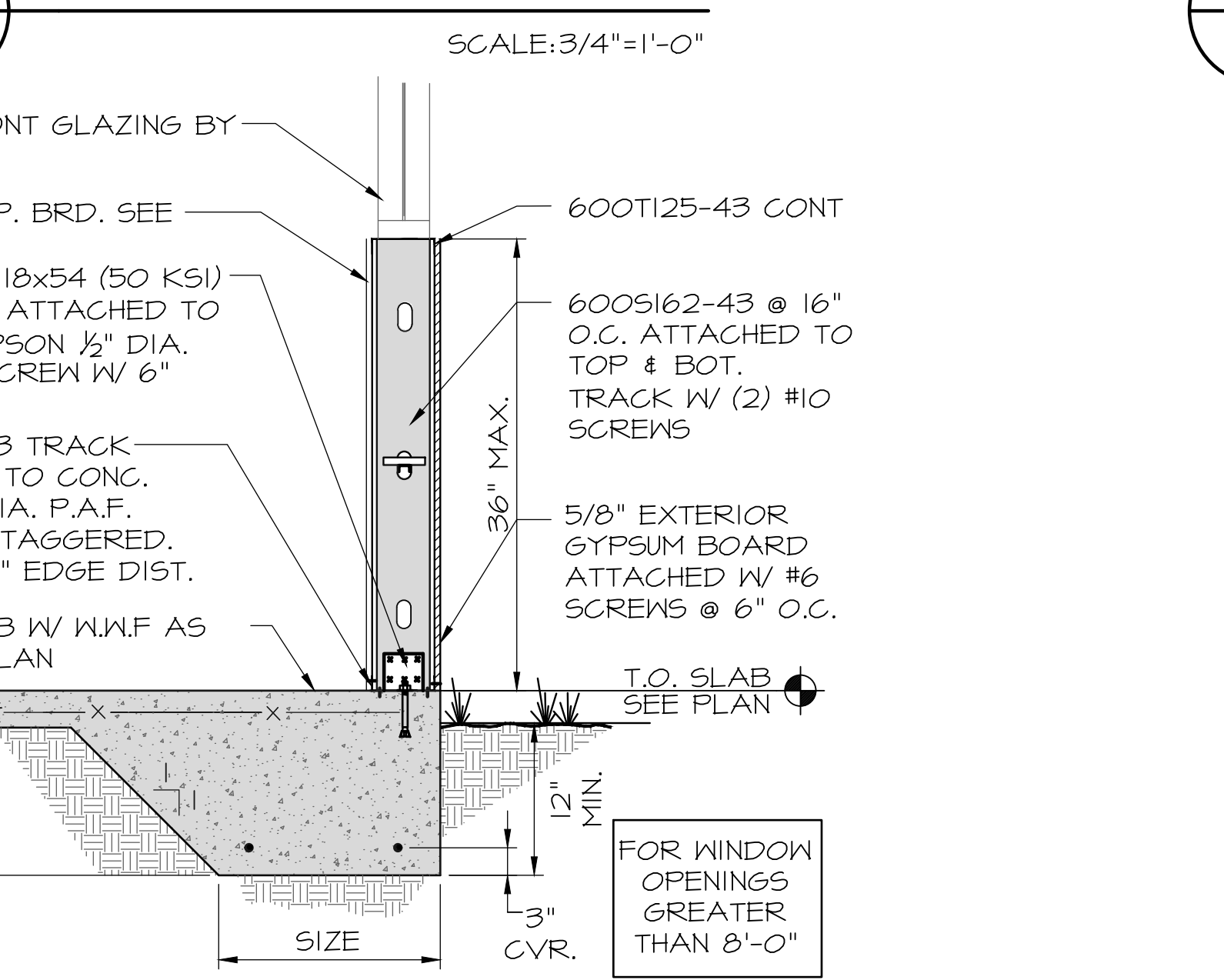
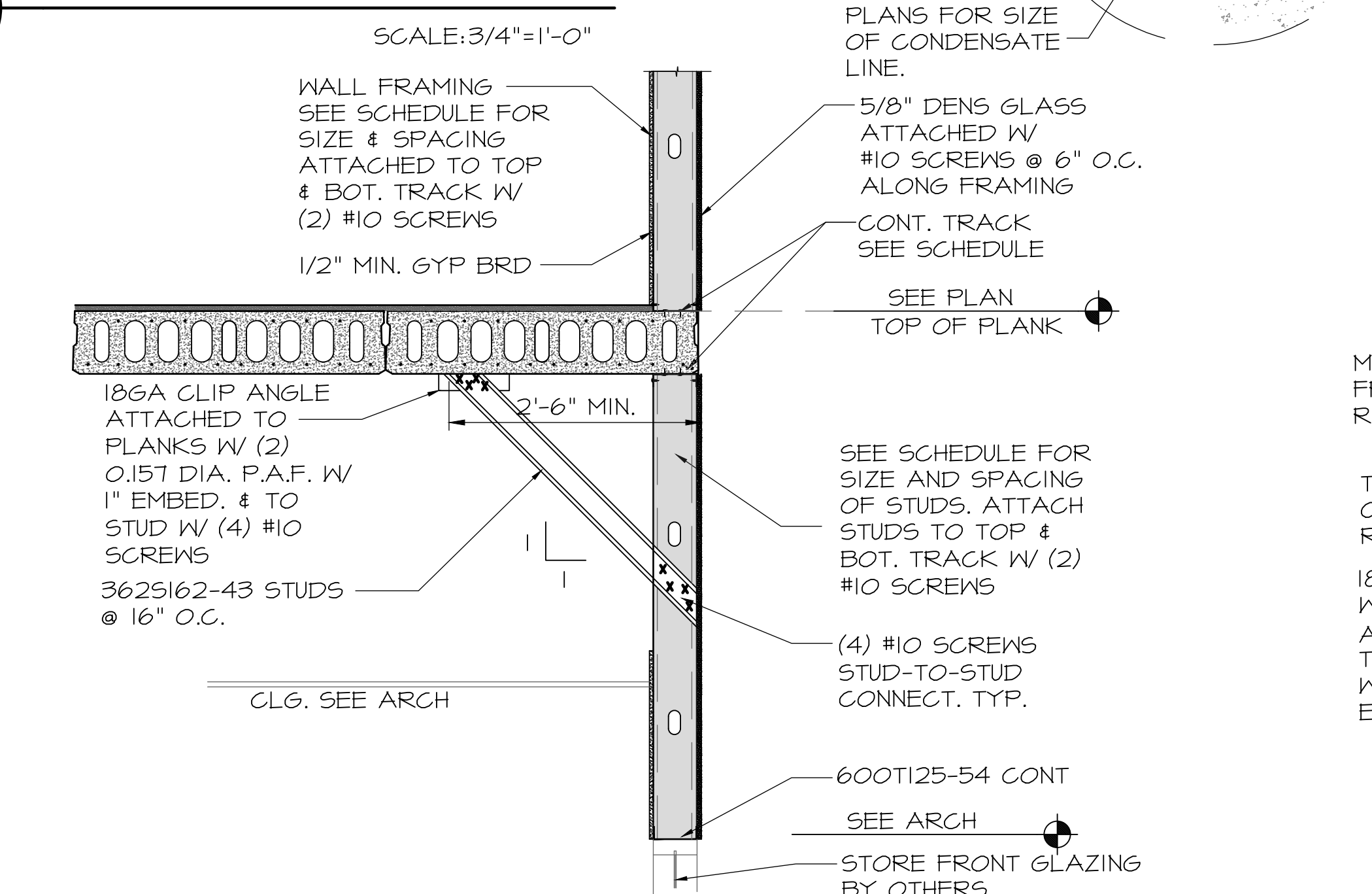
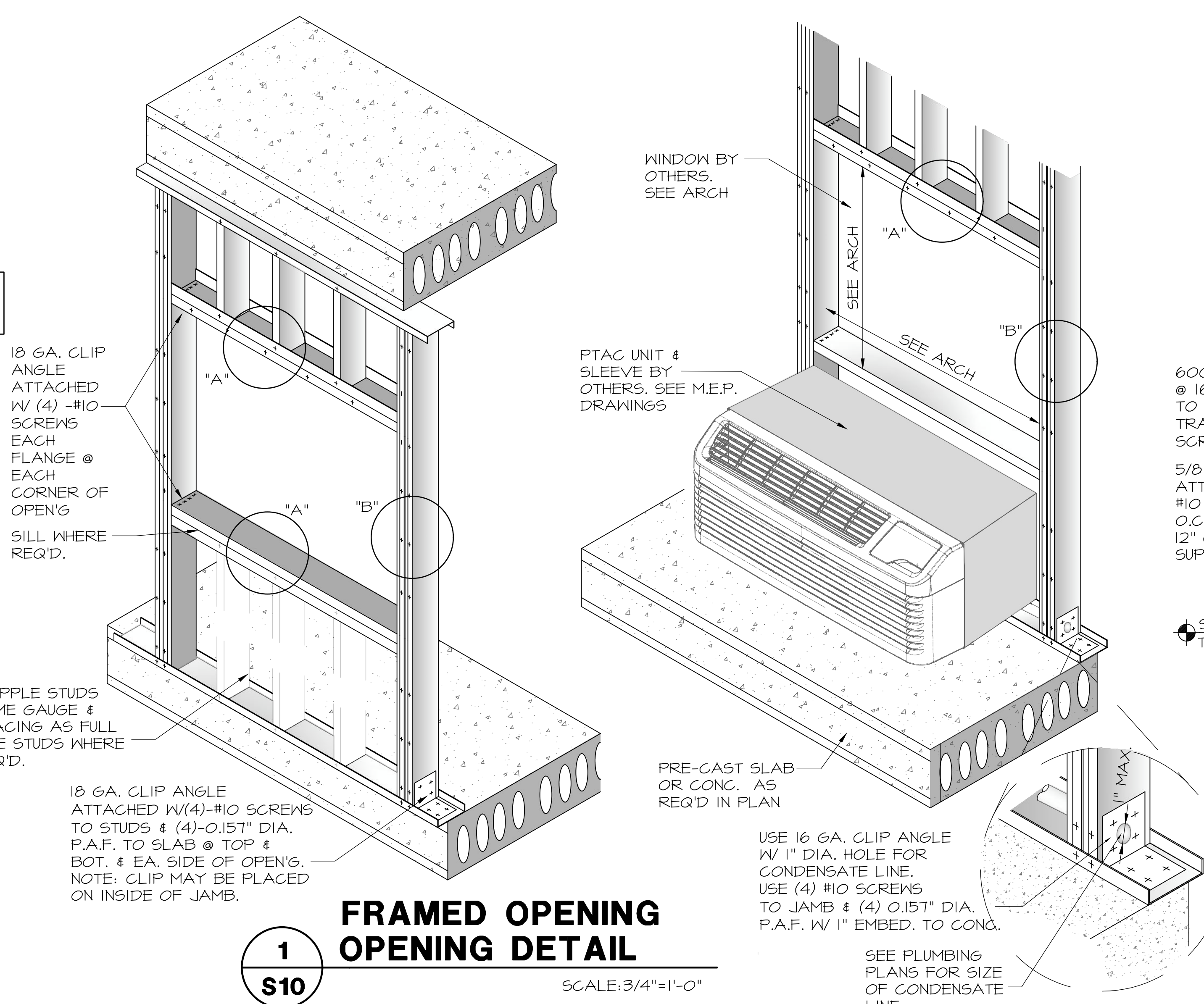
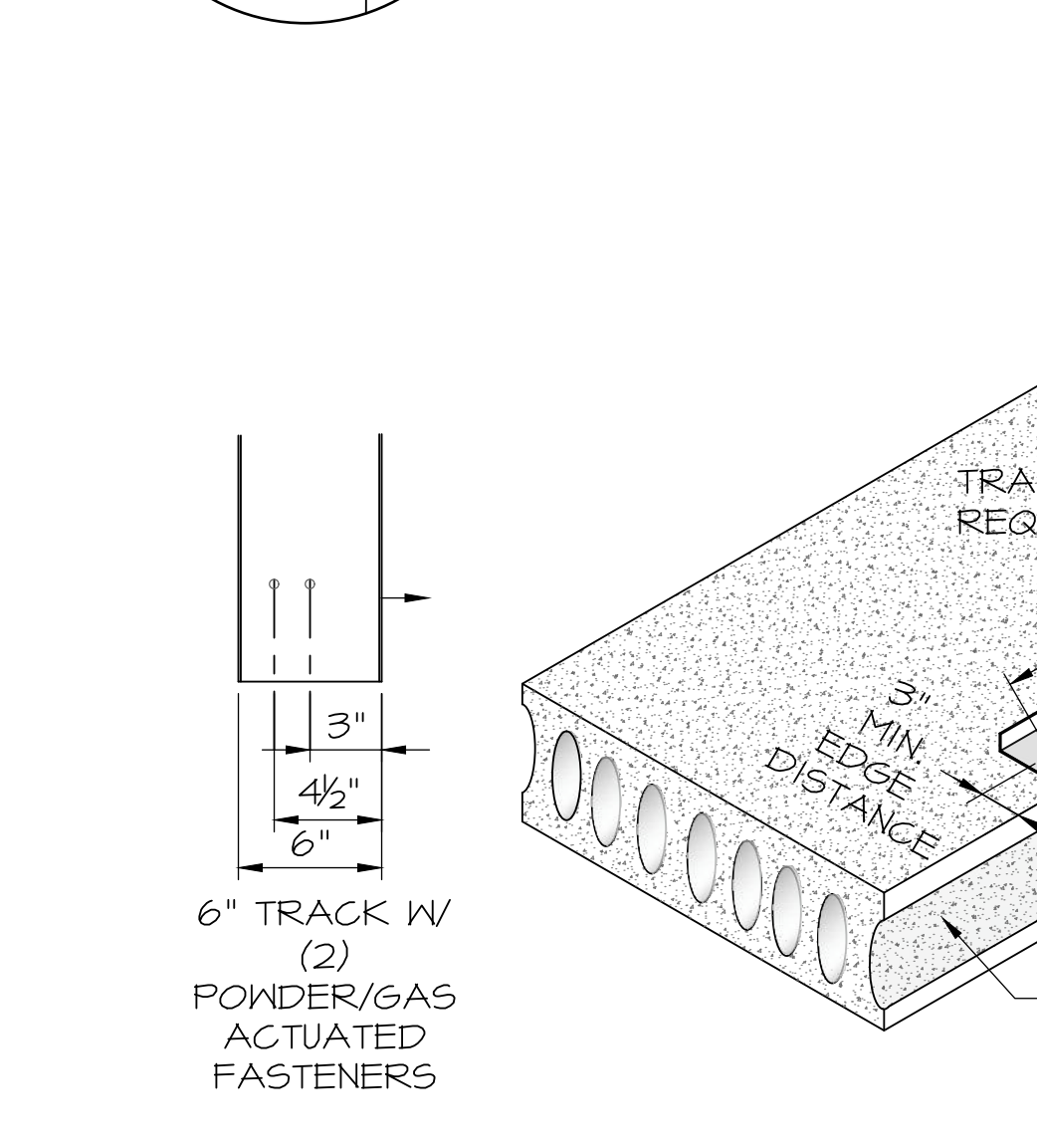
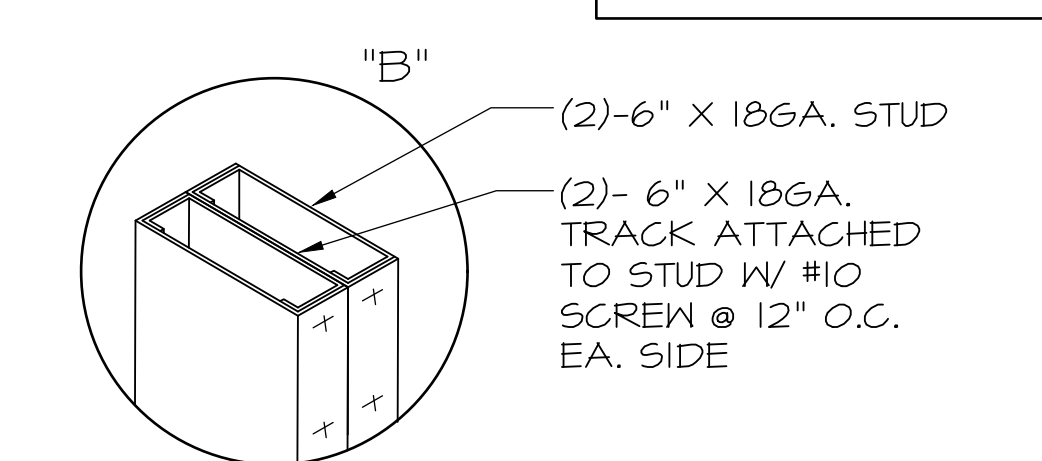


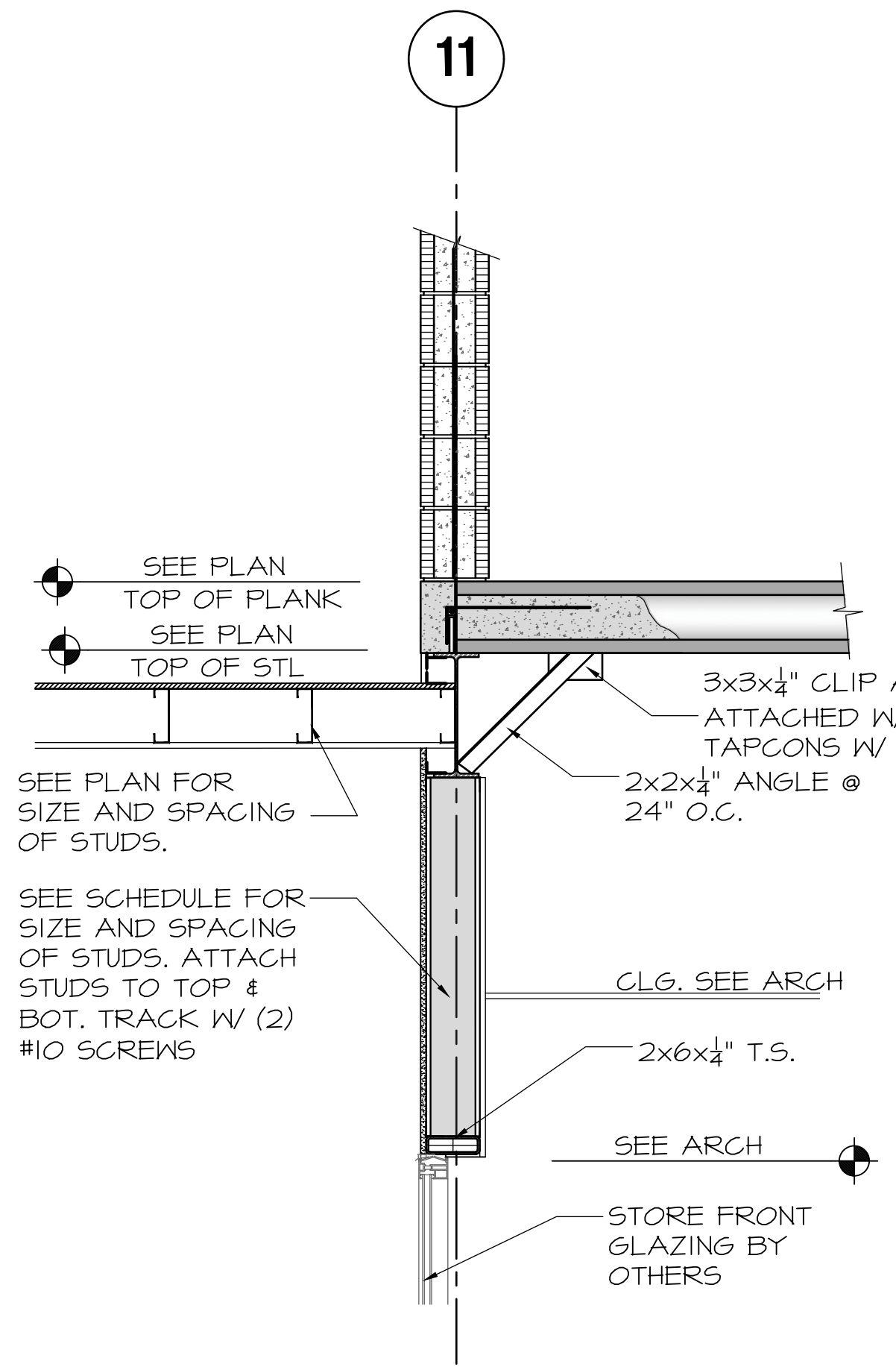
9
S9 **HIGH LIFT GROUTING PROCEDURES**
SCALE: N.T.S.

"A"
 6"X 18GA. TRACK
 ATTACHED W/ #10
 SCREW @ 12" O.C.
 STAGGERED
 6"X 18GA. STUD
 6"X 18GA. TRACK
 ATTACHED TO STUD
 W/ #10 SCREW @ 12"
 O.C. EA. SIDE

MAXIMUM OPENING
 WIDTH OF 8'-0"

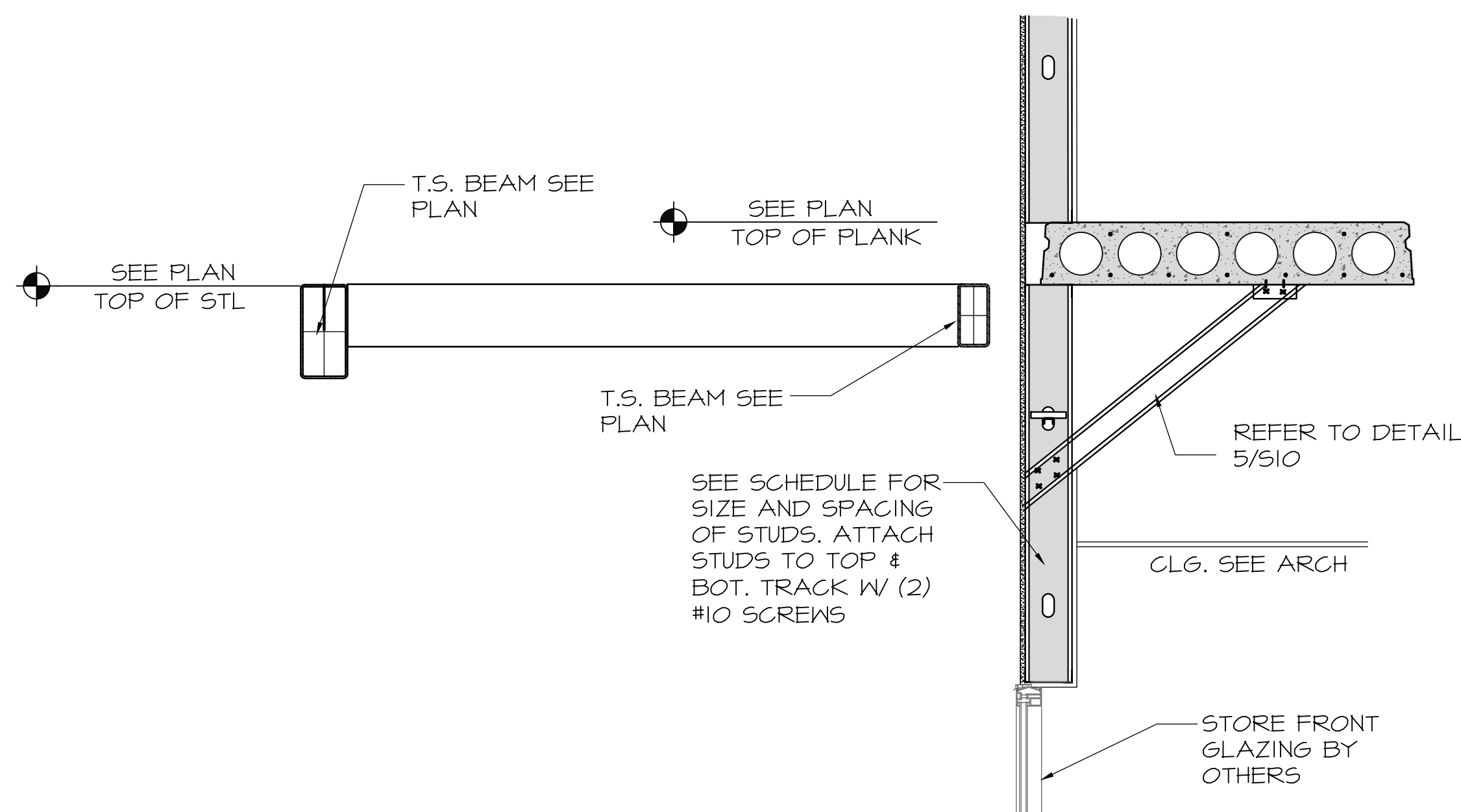
FOR OPENING
 GREATER THAN 8'-0"
 USE DETAIL 8/S11.





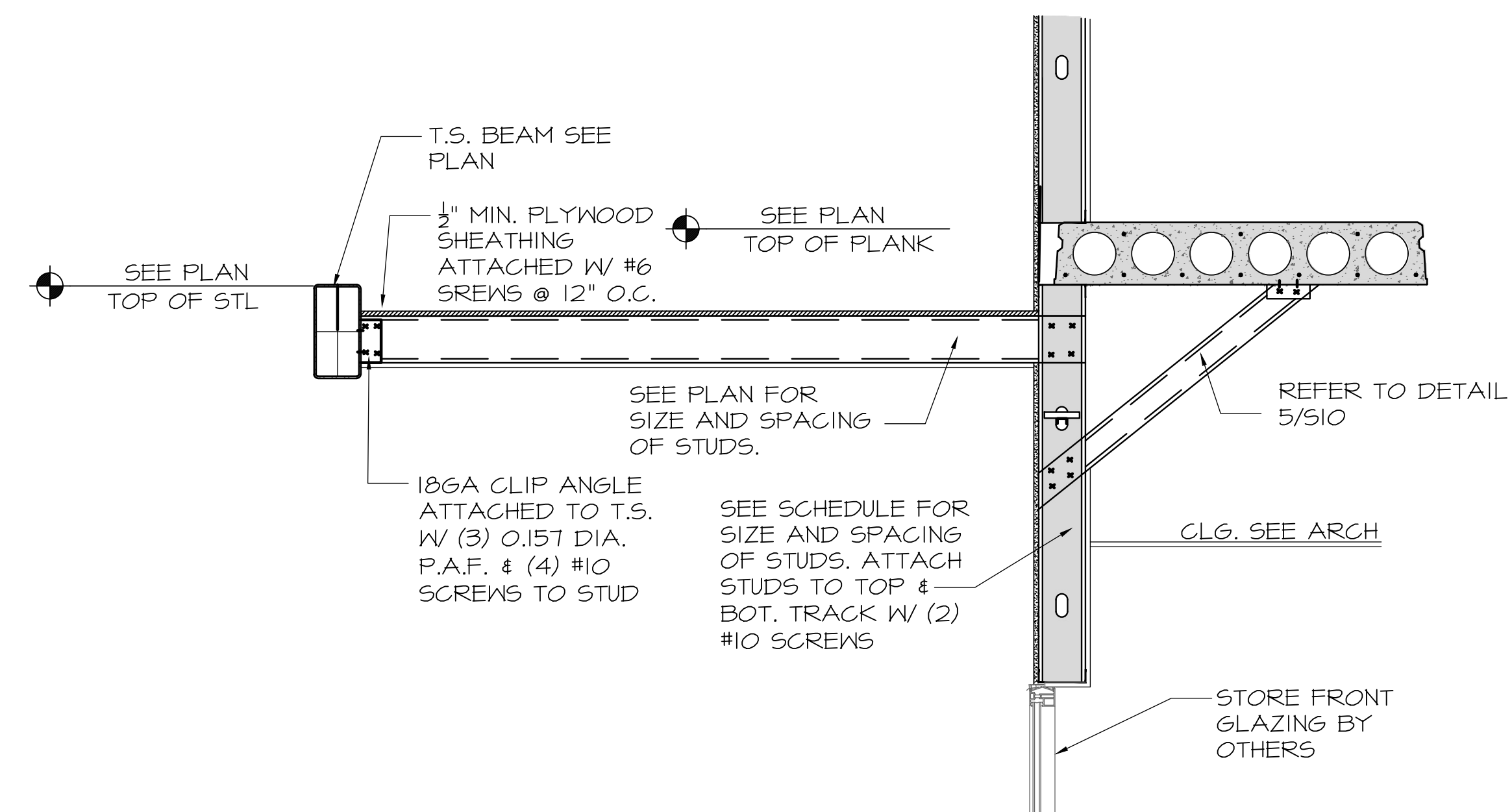
1 SECTION
S11

SCALE: 3/4"=1'-0"



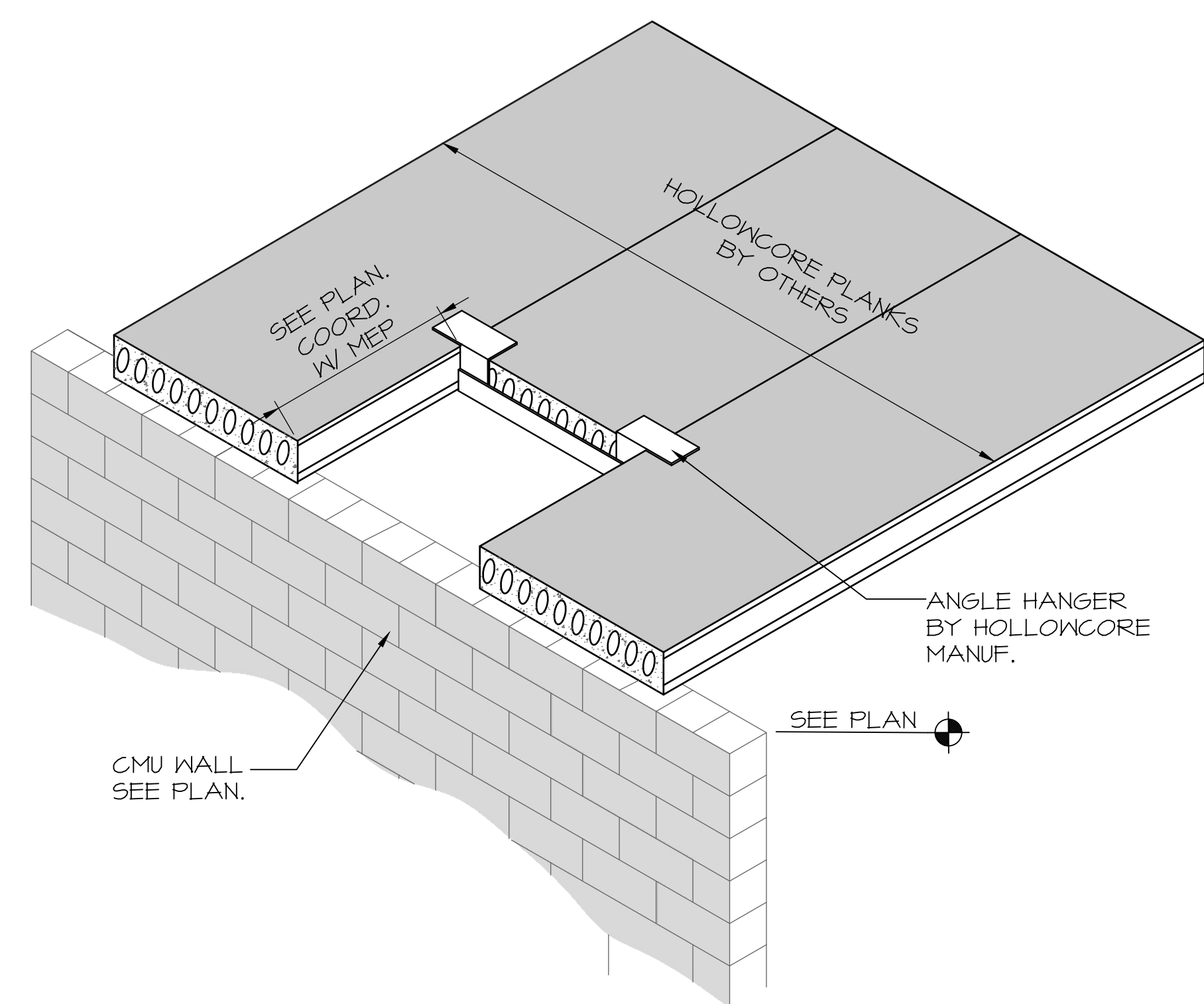
2 SECTION
S11

SCALE: 3/4"=1'-0"



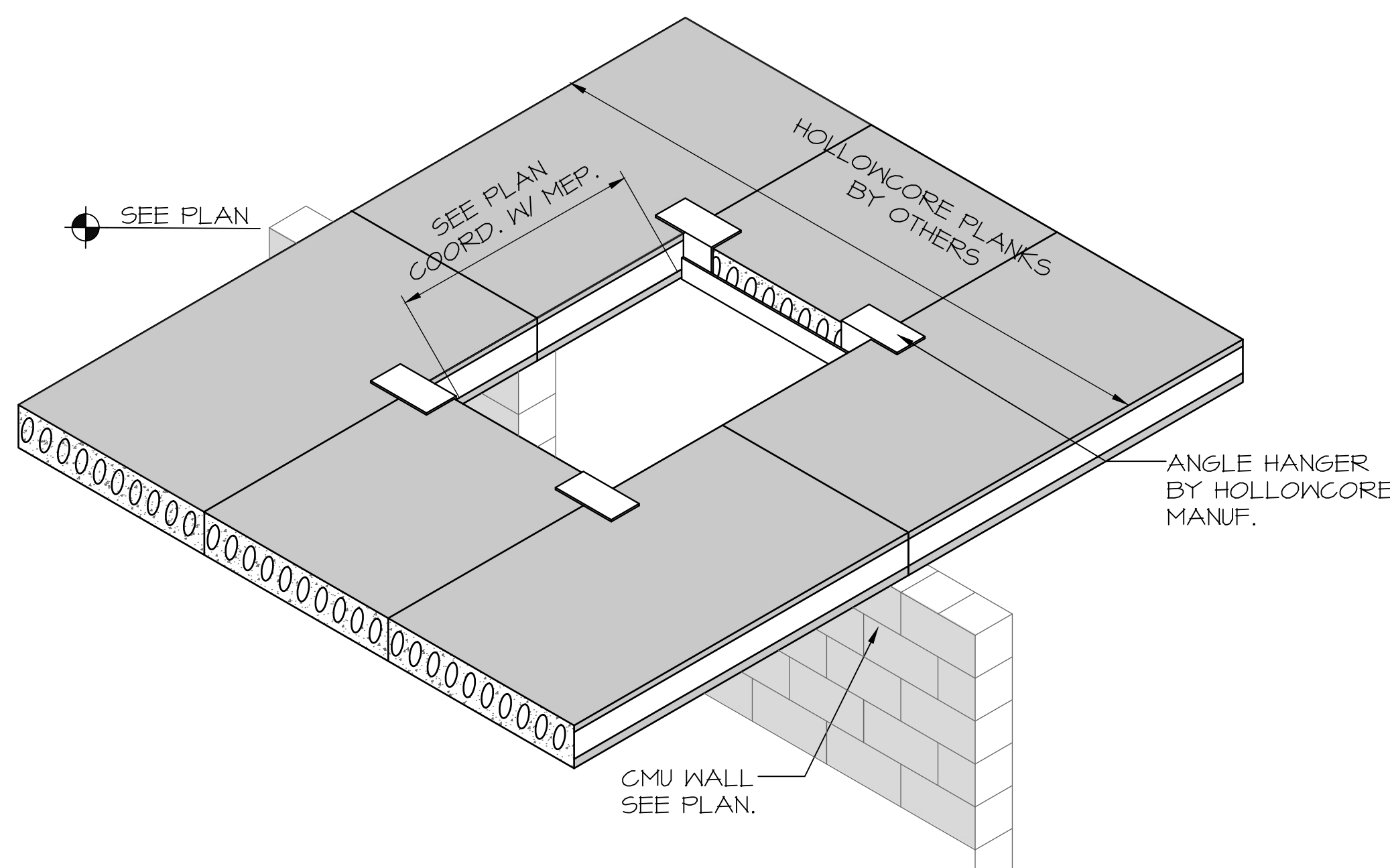
3 SECTION
S11

SCALE: 3/4"=1'-0"



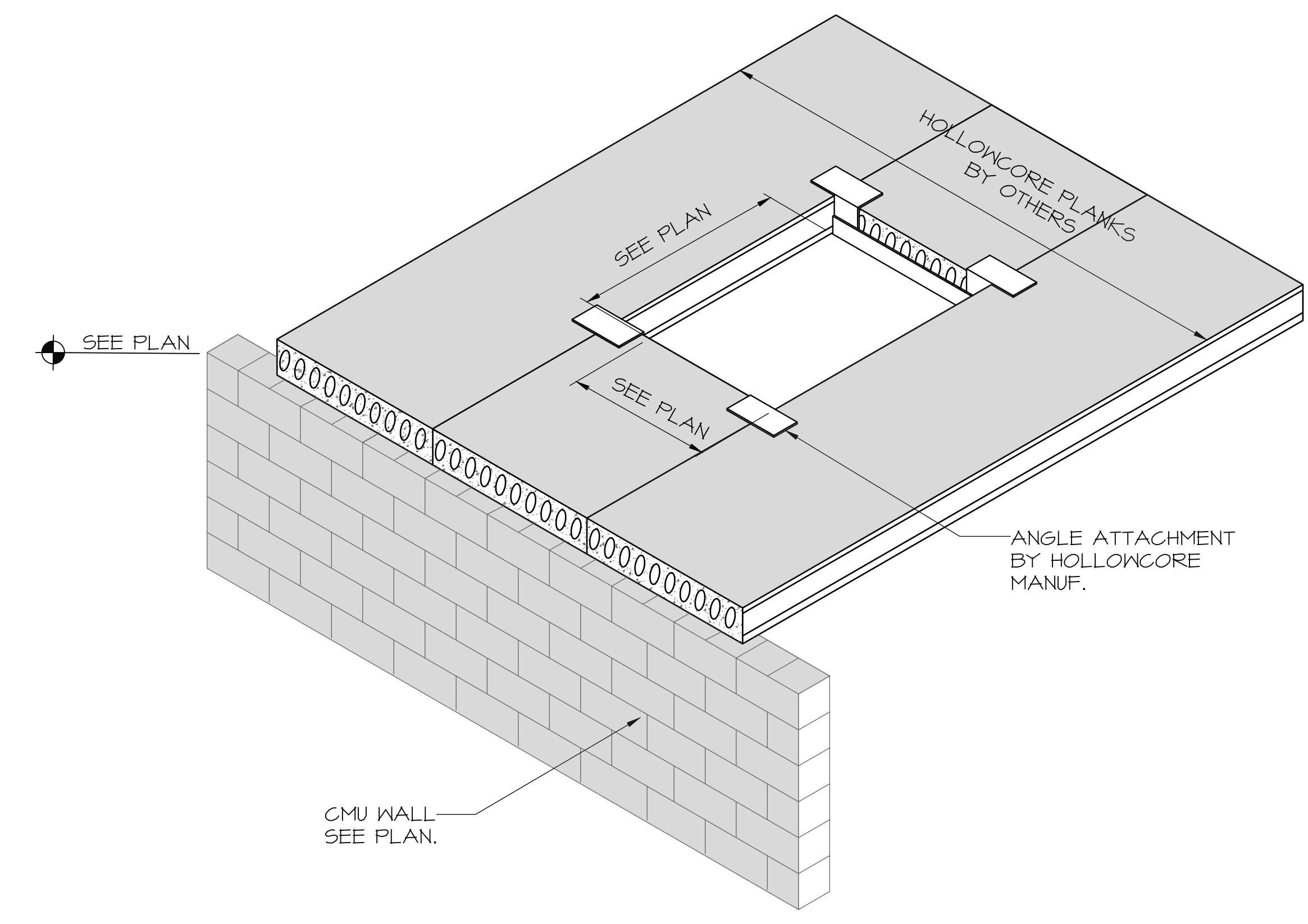
4 HOLLOWCORE PLANK OPEN'G SUPPORT DETAIL
S11

SCALE: 1/2"=1'-0"



5 HOLLOWCORE PLANK OPEN'G SUPPORT DETAIL
S11

SCALE: 1/2"=1'-0"



6 HOLLOWCORE PLANK OPEN'G SUPPORT DETAIL
S11

SCALE: 1/2"=1'-0"

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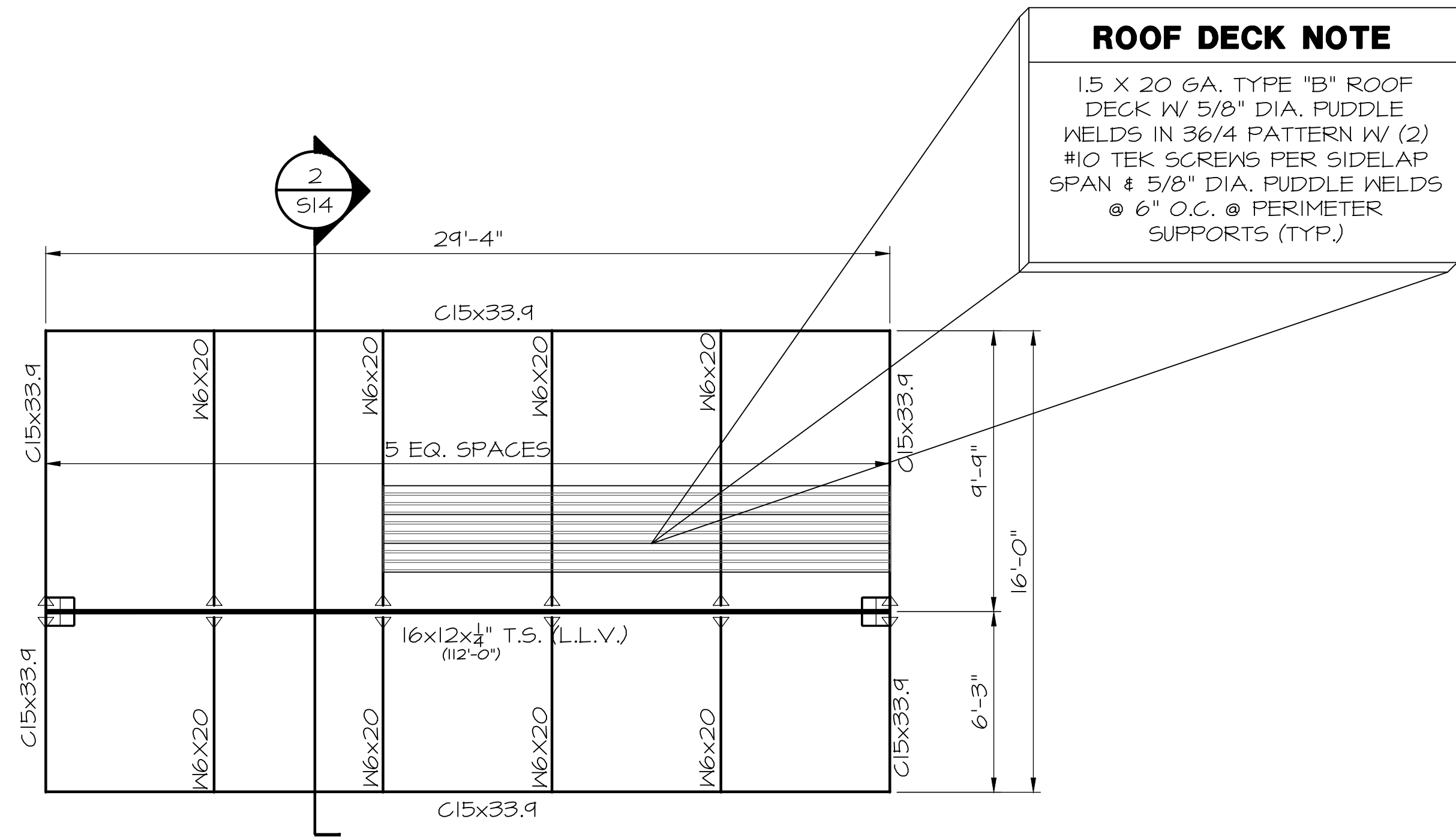
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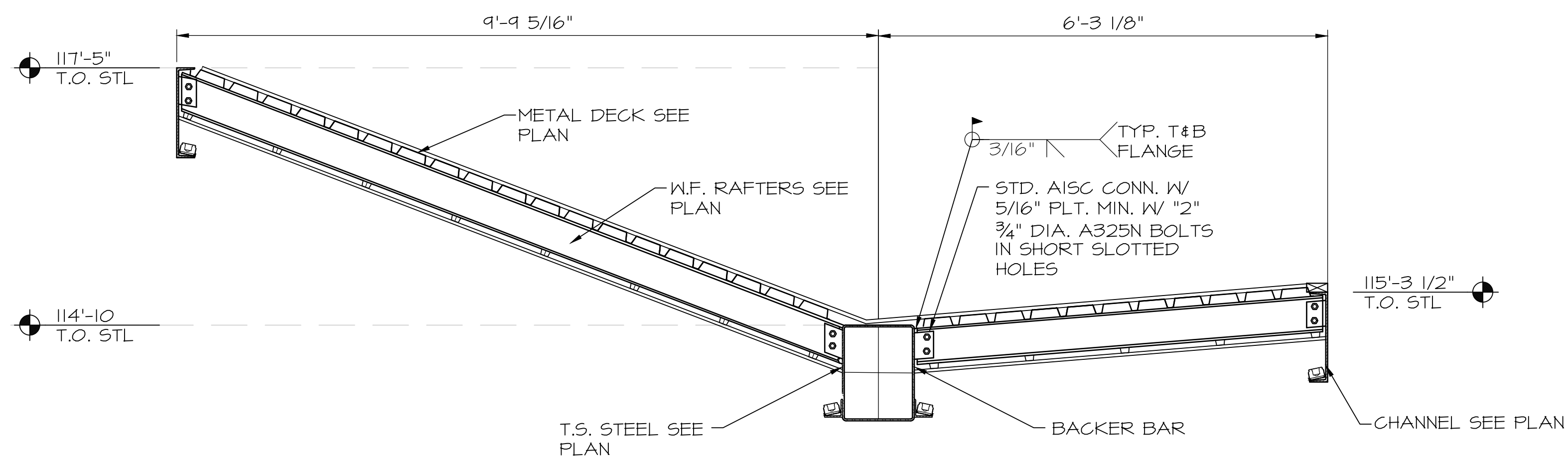
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S11

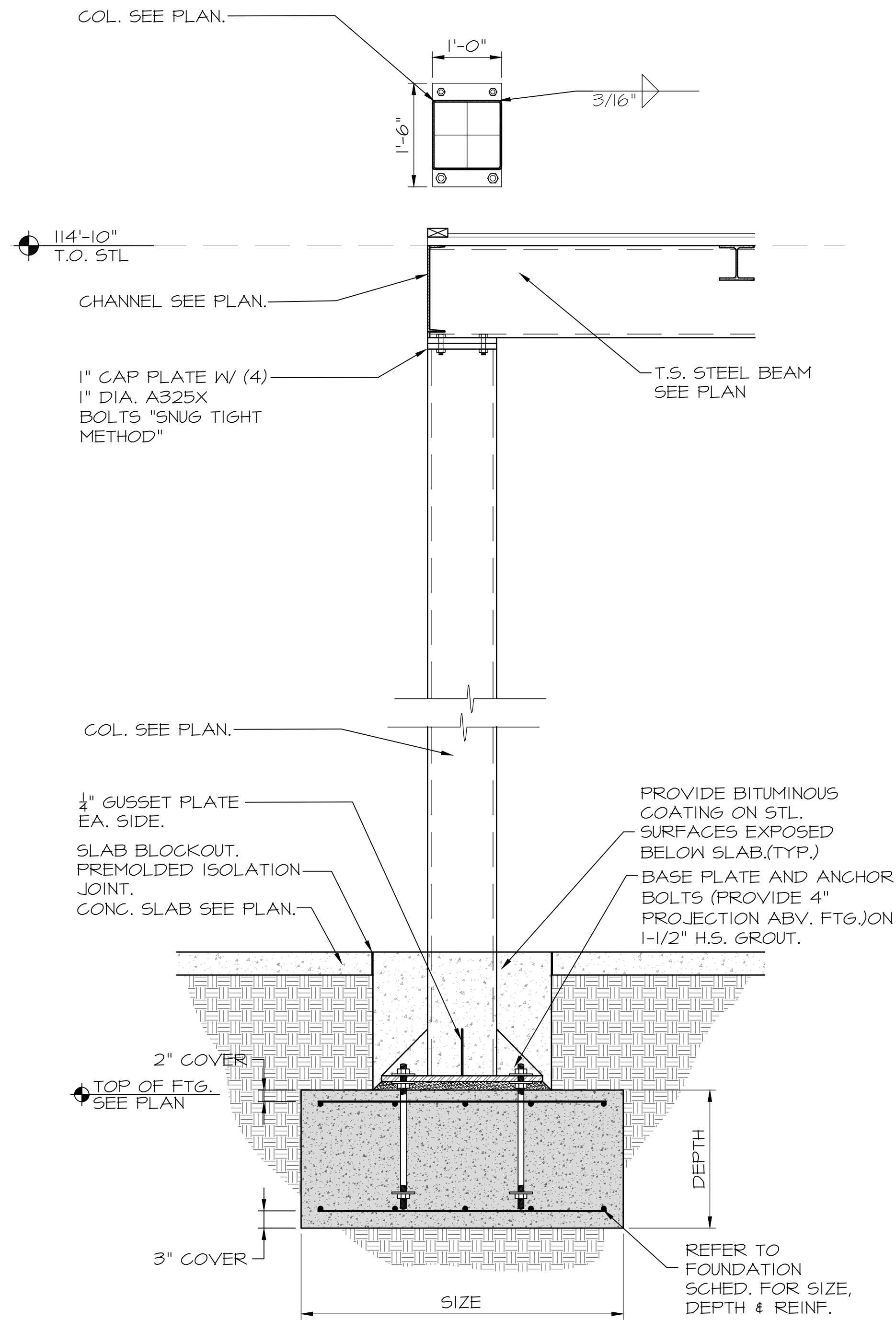
16 OF



1 CANOPY FRAMING PLAN
SCALE: 1/4"=1'-0"



2 CANOPY SECTION
SCALE: 3/4"=1'-0"



3 CANOPY COL. SECTION
SCALE: 3/4"=1'-0"

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S12

17 OF