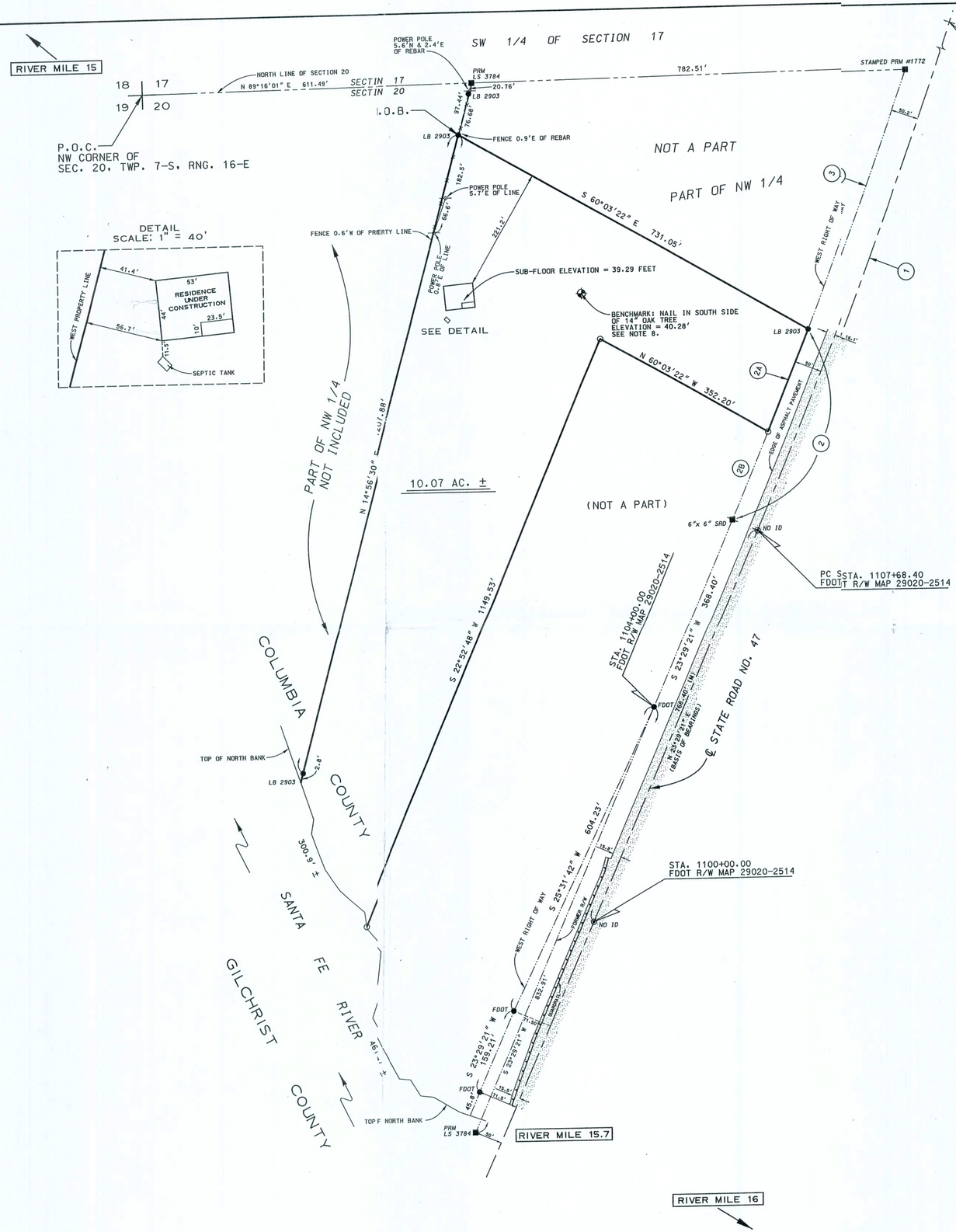
BOUNDARY SURVEY

IN

SECTION 20, TOWNSHIP 7 SOUTH, RANGE 16 EAST
COLUMBIA COUNTY, FLORIDA



DESCRIPTION:

COMMENCE AT THE NORTHWEST CORNER OF SECTION 20, TOWNSHIP 7 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE N 89°16'01" E, ALONG THE NORTH LINE OF SAID SECTION 20, 611.49 FEET; THENCE S 14°56'30" W, 97.44 FEET TO THE POINT OF BEGINNING; THENCE S 60°03'22" E, 731.05 FEET TO THE WESTERLY RIGHT OF WAY OF STATE ROAD NO. 47 AND TO A POINT ON A CURVE; THENCE RUN SOUTHERLY, ALONG SAID WESTERLY RIGHT OF WAY, ALONG THE ARC OF SAID CURVE CONCAVE TO THE NORTHWEST, HAVING A RADIUS OF 11409.16 FEET, A DELTA OF 01°00'16", A CHORD BEARING AND DISTANCE OF S 22°06'29" W - 200.00 FEET, AN ARC DISTANCE OF 200.00 FEET; THENCE N 60°03'22" W, 352.20 FEET; THENCE S 22°52'48" W, 1149.53 FEET TO THE NORTH BANK OF THE SANTA FE RIVER AND TO A SET 5/8" REBAR & CAP (LB6685); THENCE RUN NORTHWESTERLY, ALONG SAID NORTH BANK OF THE SANTA FE RIVER, 300.9 FEET, MORE OR LESS; THENCE N 14°56'30" E, 2.8 FEET, MORE OR LESS, TO A 1/2" REBAR & CAP (LB2903); THENCE CONTINUE N 14°56'30" E, 1207.88 FEET TO THE POINT OF BEGINNING.

CONTAINING 10.07 ACRES, MORE OR LESS.

NOTES:

1. BEARINGS ARE BASED ON THE CENTERLINE OF STATE ROAD NO. 47, BEING N 23°29'21" E (PER FDOT R/W MAP 29020-2514).
2. SUBJECT PROPERTY LIES IN FLOOD ZONE "AE", AN AREA INSIDE OF THE 100-YEAR FLOOD PLAIN PER FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070 0255 B. LAST REVISION DATE JANUARY 6, 1988. FLOOD ZONE LINES, IF ANY, ARE SCALED FROM FLOOD INSURANCE RATE MAPS, PROVIDED BY FEMA.
3. RIVER MILE 15.7:
   100-YEAR FLOOD ELEVATION = 36 FEET
   10-YEAR FLOOD ELEVATION = 32 FEET
   2-YEAR FLOOD ELEVATION = 26 FEET
4. ONLY THOSE VISIBLE INTERIOR IMPROVEMENTS AND IMPROVEMENTS PERTINENT TO THE SUBJECT PROPERTY HAVE BEEN LOCATED AS SHOWN HEREON. EXCEPTION IS MADE HEREON TO UNDERGROUND FACILITIES AND OTHER IMPROVEMENTS NOT VISIBLE OR KNOWN AT DATE OF SURVEY.
5. THIS SURVEY WAS PREPARED WITHOUT THE BENEFIT OF AN ABSTRACT OR TITLE POLICY, THEREFORE, EXCEPTION IS MADE HEREIN REGARDING EASEMENTS, RESERVATIONS AND RESTRICTIONS OF RECORD NOT PROVIDED BY THE CLIENT.
6. CLOSURE EXCEEDS 1 : 10,000.
7. SCALE AND GRAPHIC LOCATION OF FENCES, GUARDRAIL, UTILITY POLES AND SOME MONUMENTS MAY BE EXAGGERATED FOR CLARITY.
8. THE BENCHMARK IS A NAIL IN THE SOUTH SIDE OF A 14" OAK TREE SET BY ALACHUA COUNTY LAND SURVEYORS INC. WITH ELEVATION OF 40.28 FEET. ELEVATIONS WERE BASED ON A FDOT BENCHMARK AT THE NORTHWEST CORNER OF THE BRIDGE OVER THE SANTA FE RIVER ON STATE ROAD NO. 47. ELEVATIONS WERE BASED ON THE NATIONAL GEODETIC VERTICAL DATUM.
9. SUB-FLOOR ELEVATION OF STRUCTURE IS 39.29 FEET.
10. THIS SURVEY WAS BASED ON PREVIOUS SURVEY BY ALACHUA COUNTY LAND SURVEYORS, INC., DATED FEBRUARY 11, 2002 (JOB NUMBER 202-029).
11. TRAIL ACCESS ROADWAY NOT LOCATED.
12. THE PURPOSE OF THIS SURVEY IS TO REVISE THE BOUNDARY FROM A PREVIOUS SURVEY. NO ATTEMPT WAS MADE TO VERIFY INTERIOR IMPROVEMENTS.

CURVE DATA

CURVE NO.	RADIUS	DELTA	ARC LENGTH	CHORD DIST.	CHORD BEARING
1	11,459.16'	08°33'00"	1,710.00'	1,708.41'	S 19°12'51" W
2	11,409.16'	01°51'48"	371.05'	371.03'	S 22°32'15" W
2A	11,409.16'	01°00'16"	200.00'	200.00'	S 22°06'29" W
2B	11,409.16'	00°51'32"	171.05'	171.04'	S 23°02'23" W
3	11,409.16'	02°30'46"	500.33'	500.33'	S 20°21'52" W

LEGEND

○ DENOTES 5/8" IRON ROD & CAP SET (LB6685)	N – NORTH
● DENOTES IRON PIPE OR REBAR FOUND	E – EAST
□ DENOTES 4"x4" CONCRETE MONUMENT SET (LB6685)	S – SOUTH
■ DENOTES 4"x4" CONCRETE MONUMENT FOUND	W – WEST
◉ DENOTES NAIL & DISK FOUND	℄ – CENTERLINE
x⟶x DENOTES FENCE	(P) – PLAT
OE⟶OE DENOTES OVERHEAD ELECTRIC	(D) – DEED
⟶◇⟶ DENOTES POWER POLE	(C) – CALCULATED
⟶ DENOTES GUY ANCHOR	(M) – MEASURED
CONCRETE	O/S – OFFSET
FND – FOUND	NO ID – NO IDENTIFICATION
± – MORE OR LESS	CM – CONCRETE MONUMENT
PC – POINT OF CURVATURE	IP – IRON PIPE
PT – POINT OF TANGENCY	IPC – IRON PIPE & CAP
PI – POINT OF INTERSECTION	RB – REBAR
PRC – POINT OF REVERSE CURVATURE	RBC – REBAR & CAP
PCC – POINT OF COMPOUND CURVATURE	IR – IRON ROD
R – RADIUS	IRC – IRON ROD & CAP
T – TANGENT	NL – NAIL
L – ARC LENGTH	NL+D – NAIL & DISK
Δ – CENTRAL ANGLE	ORB – OFFICIAL RECORDS BOOK
CH – CHORD BEARING & DISTANCE	PG – PAGE(S)
PCP – PERMANENT CONTROL POINT	POC – POINT OF COMMENCEMENT
PRM – PERMANENT REFERENCE MONUMENT	POB – POINT OF BEGINNING
R/W – RIGHT OF WAY	SEC – SECTION
FDOT – FLORIDA DEPARTMENT OF TRANSPORTATION	TWP – TOWNSHIP
	RNG – RANGE

SURVEY FOR: BARBARA FERGUSON

4-21-06
DATE OF SIGNATURE:

04-11-06
DATE OF FIELD SURVEY:

BRIAN SCOTT DANIEL, PSM
PROFESSIONAL SURVEYOR AND MAPPER
FLORIDA CERTIFICATE NO. 6449

SURVEY VALID ONLY TO THE DATE OF FIELD SURVEY SHOWN HEREON. NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. FLORIDA CERTIFICATE OF AUTHORIZATION NO. LB6685

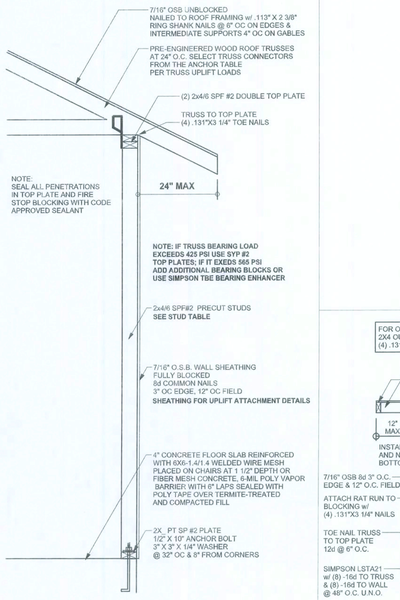
BAILEY BISHOP & LANE, INC.
191 SW COMMERCE DRIVE, SUITE 135
P. O. BOX 3717
LAKE CITY, FL 32056-3717
PH. (386) 752-5640 FAX (386) 755-7771
Eng. Lic. 7362 Survey Lic. LB-0006685

BARBARA FERGUSON

FEVISIONS

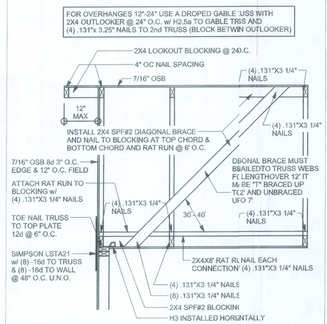
04/21/06 – REVISED BOUNDARY.

DRAWN BY: JD
JOB NUMBER 030804FER
FIELD BOOK EFB
SHEET NO. 1 OF 1



ONE STORY WALL SECTION

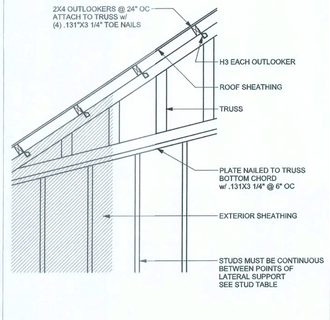
SCALE: 3/4" = 1'-0"



SPACE RAT RUN & DIAGONAL BRACE 6'-0" O.C.
FOR GABLE HEIGHT UP TO 25'-0" 110 MPH, EXP. C, &CLOSED

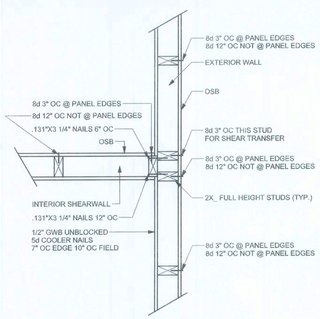
(TYP.) GABLE BRACING DETIL

WOOD FRAME



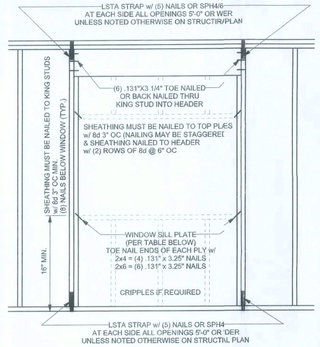
(TYP.) GABLE WALL w/ VAULTED CEILING

WOOD FRAME



(TYP.) INTERSECTING WALL FRAMING

WOOD FRAME

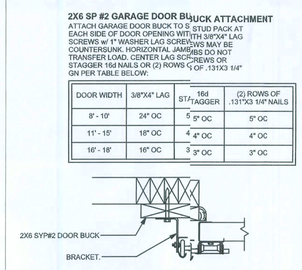


TYPICAL HEADER STRAPING DETAIL

ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

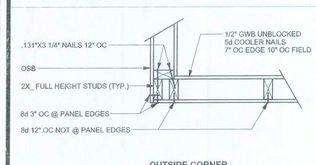
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT

DESIGN WIND SPEED	MAX. SPANS FOR SPF #2 (1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
115-130 MPH	5'-3"	7'-9"	7'-8"	11'-4"	BASED ON WFCM TABLE A-3.2(8)
140-150 MPH	4'-4"	6'-6"	6'-5"	9'-6"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)
160 MPH	4'-0"	6'-0"	5'-11"	8'-9"	

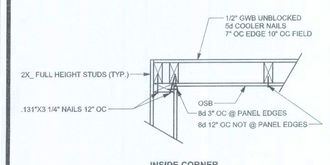


(TYP.) GARAGE DOOR BUCK INSTALLATION

WOOD FRAME



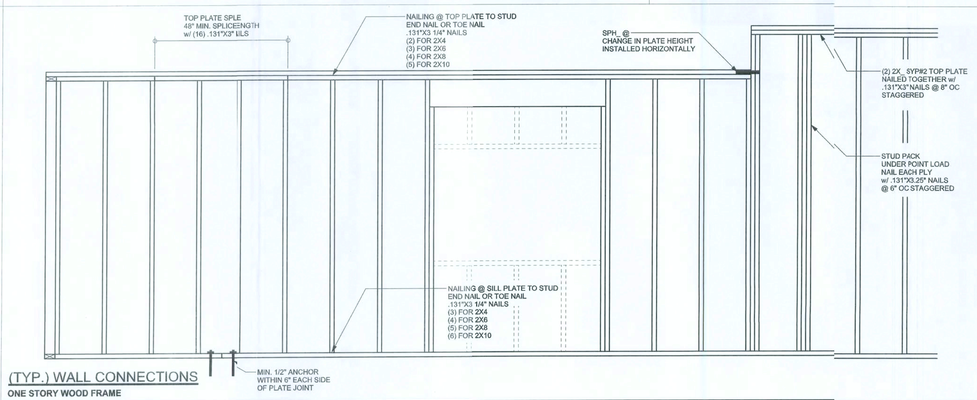
OUTSIDE CORNER



INSIDE CORNER

(TYP.) CORNER FRAMING

WOOD FRAME



(TYP.) WALL CONNECTIONS

ONE STORY WOOD FRAME

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES	
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"	
H3	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"	
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"	
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"	
H6	950	820					8-8d	8-8d	
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"	
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d	
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d	
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"	
H10-2	760	655	455	395	390	340	5-10d	6-10d	
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"	
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"	
LTS12 - LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"	
MTS12 - MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"	
HTS16 - HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"	
HEAVY GIRDER TIEDOWNS									**TO FOUNDATION**
LGT2	2050	1785	700	170	700	170	14-16d	14-16d	
LGT3-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16dS	
LGT4-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS	
MGT	3965	3330					22 -10d		5/8" ANCHOR
HGT-2	10980	6485					16 -10d		2-5/8" ANCHOR
HGT-3	10530	9035					16 -10d		2-5/8" ANCHOR
HGT-4	9250	9250					16 -10d		2-5/8" ANCHOR
STUD STRAP CONNECTOR									**TO STUDS**
SSP DOUBLE TOP PLATE	435	435						3-10d	4 -10d
SSP SINGLE SILL PLATE	455	420						1-10d	4 -10d
DSP DOUBLE TOP PLATE	825	825						6 -10d	8 -10d
DSP SINGLE SILL PLATE	825	600						2 -10d	8 -10d
SP1	585	535						4 -10d	6 -10d
SP2	1065	605						6 -10d	6 -10d
SP4	885	760							6-10d x 1 1/2"
SPH4	1240	1065							10-10d x 1 1/2"
SP6	885	760							6-10d x 1 1/2"
SPH6	1240	1065							10-10d x 1 1/2"
LSTA18	1235	1110							14-10d
LSTA21	1235	1235							16-10d
CS20	1030	1030							14-10d
CS16	1705	1705							22-10d
STUD ANCHORS							**TO STUDS**		**TO FOUNDATION**
LTT19	1350	1305					8-16d		1/2" ANCHOR
LTT31	2310	2310					18-10d x 1 1/2"		5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS		5/8" ANCHOR
HTT16	4175	3695					18-16d		5/8" ANCHOR
HTT22	5260	5250					32-16d		5/8" ANCHOR
ABU44	2200	2200					12-16d		5/8" ANCHOR
ABU66	2300	2300					12-16d		5/8" ANCHOR
ABU88	2320	2320					18-16d		2-5/8" ANCHOR

(1) w/ INSTALLATION OF 4-16dS OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

MASONRY NOTES:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B4,
EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS
FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR
RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C,
STUD DEFLECTION LIMIT H/240 (NOT OK FOR SOME BRITTLE FINISH?).
STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING
LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING.
(END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SP #2	925	1.6
2x10	SP #2	800	1.6
2x12	SP #2	750	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2010. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 65KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DIA (25" FOR #5 BARS) UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-99, U.N.O.

GLULAM BEAMS: GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH .113" X 2 3/8" RING SHANK NAILS @ 6" OC ON EDGES & INTERMEDIATE SUPPORTS 4" OC ON GABLES

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

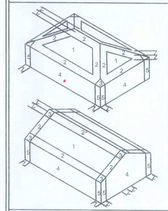
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH 2010 FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH 2010 FBCR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF 2010 FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER 2010 FLORIDA BUILDING CODE RESIDENTIAL, SECTION R301.2.1	
ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT	
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.) BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)	
2.) WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY	
3.) TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY	
4.) RISK CATEGORY = II, (MRI = 700 YR)	
5.) ROOF ANGLE = 7-45 DEGREES	
6.) MEAN ROOF HEIGHT = <30 FT	
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))	



Zone	Effective Wind Area (ft2) 10		
1	39	-43	
2	39	-66	
3	39	-100	
4	43	-46	
5	43	-57	

Garage Door 2010 FBCR, Table R301.2(4)		
8x7 Garage Door	37	-42
16x7 Garage Door	36	-40

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)
SOIL BEARING CAPACITY	1500 PSF
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

COLUMBIA COUNTY BUILDING DEPARTMENT
Received for FILE COPY
Code Compliance
PLANS EXAMINER

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENNINEER: Mark Disoway, PE No.53915, PIB 868, Lake City, FL 32056, 386-754-419

DIMENSIONS:
Stated dimensios supercede scaled dimensions. Refr all questions to Mark Disoway,*.E. for resolution. Do not proceed ithout clarification.

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CERTIFICATION I hereby certify that I have examined this pln, and that the applicatio portions of the pln, relating to wind engineering comply with sectn R301.2.1, 2010 Florida Building Code Residential to the best of mynowledge.

LIMITATION: Ths design is valid for one building, at speciied location.

MARK DISOWAY P.E. 53915
No. 53915 STATE OF FLORIDA PROFESSIONAL ENGINEER
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PRINTED DATE:
Tuesday, September 2, 2014

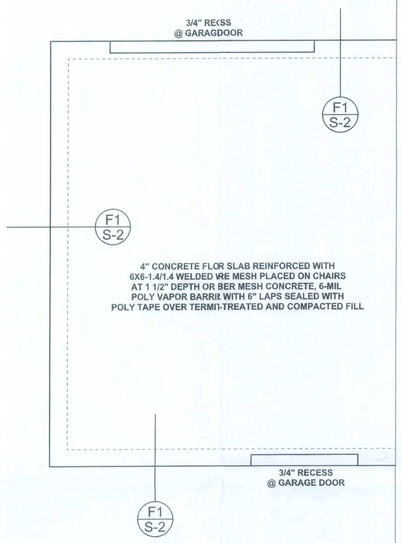
DRAWN BY: | STRUCTURAL BY:

FINALS DATE
2Sep14

JOBNUMBER:
140723

DRAVING NUMBER
S-1

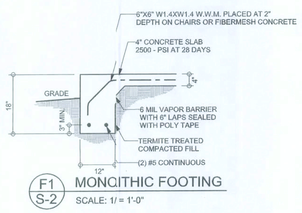
OF 2 SHEETS



FOUNDATION PLAN

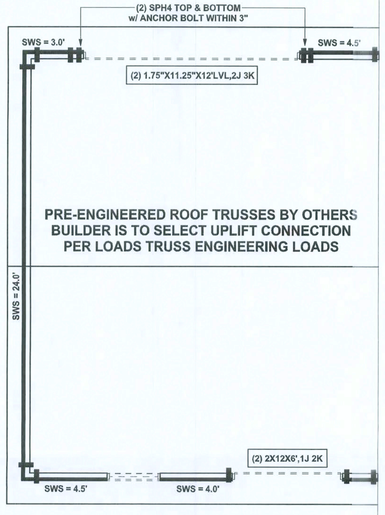
SCALE: 1/4" = 1'-0"

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTRAL FLOOR PLAN FOR ACTUAL DIMENSIONS



F1 S-2 MONCITHIC FOOTING

SCALE: 1/ = 1'-0"



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SP #2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03. BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

HEADER LEGEND

(2) 2X12X0',1J 1K
- HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

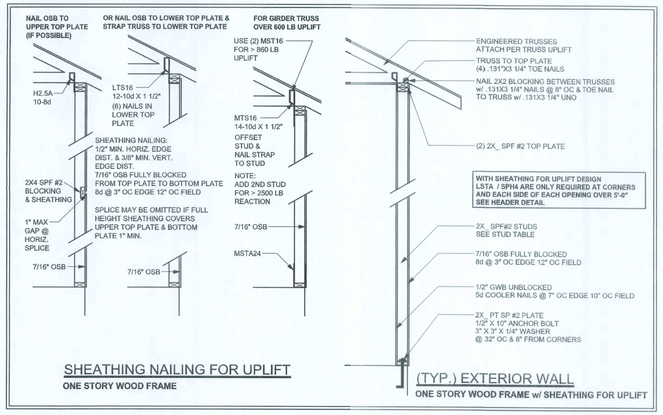
TOTAL SHEAR WALL SEGMENTS

INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	15.7'	24.0'
LONGITUDINAL	16.0'	16.0'

= REQUIRED LOCATION OF LSTA OR SPH @ TOP & BOTTOM

SHEATHING FOR UPLIFT ATTACHMENT DETAILS



SHEATHING NAILING FOR UPLIFT
ONE STORY WOOD FRAME

(TYP.) EXTERIOR WALL
ONE STORY WOOD FRAME w/ SHEATHING FOR UPLIFT

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 98, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. or resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, ad that the applicable portions of the plan, relating to wind engineering comply with section R01.2.1, 2010 Florida Building Code Residential to the best of my knowledge.

LIMITATION: This deign is valid for one building, at specified lcation.

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OF 2 iHEETS