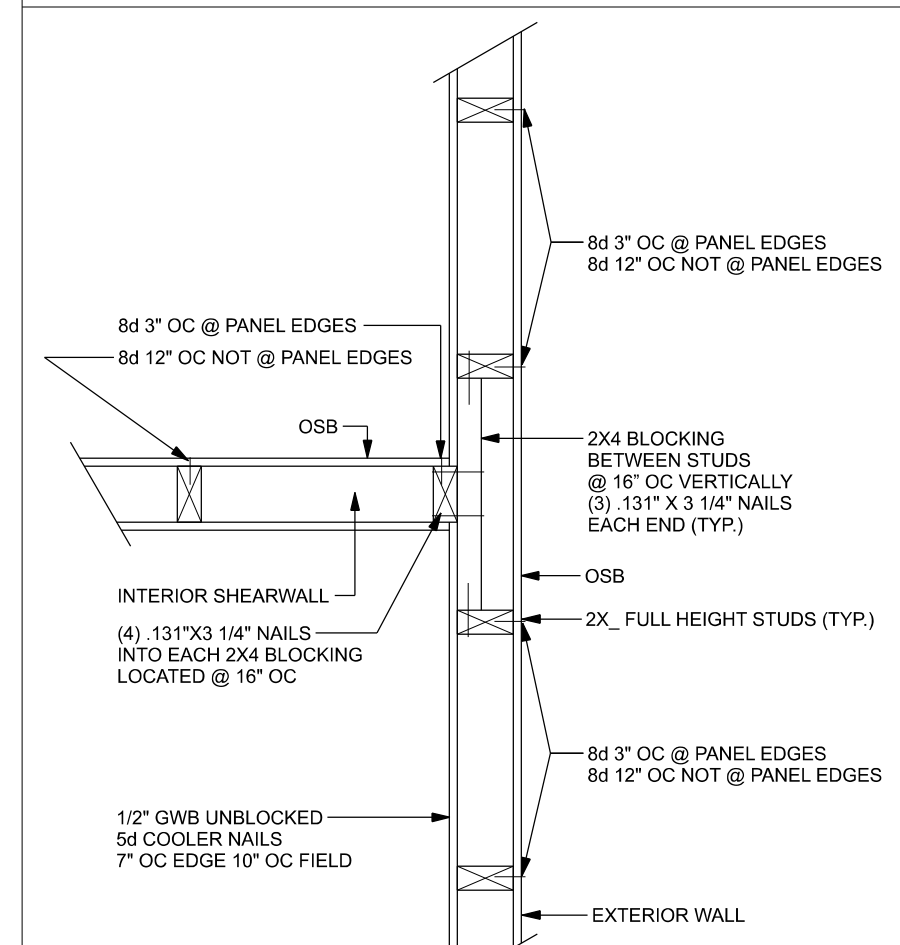
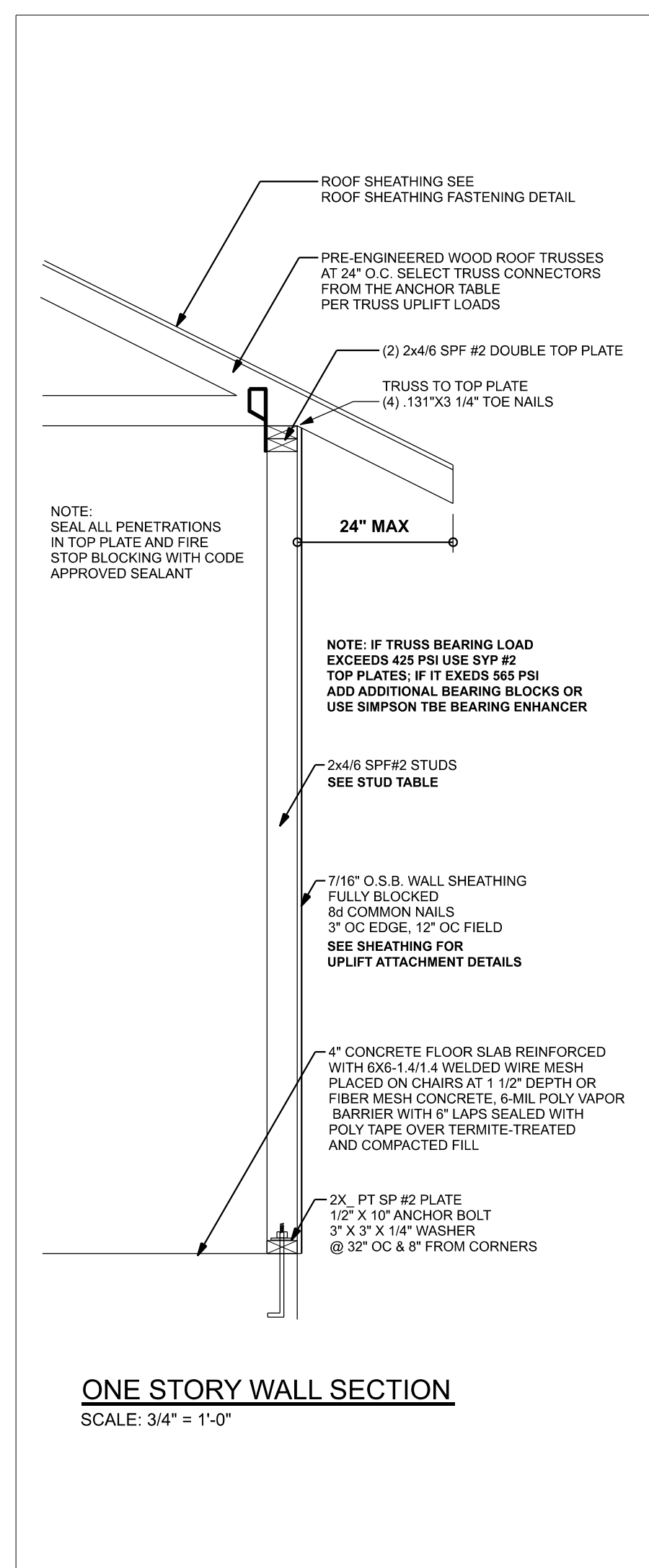
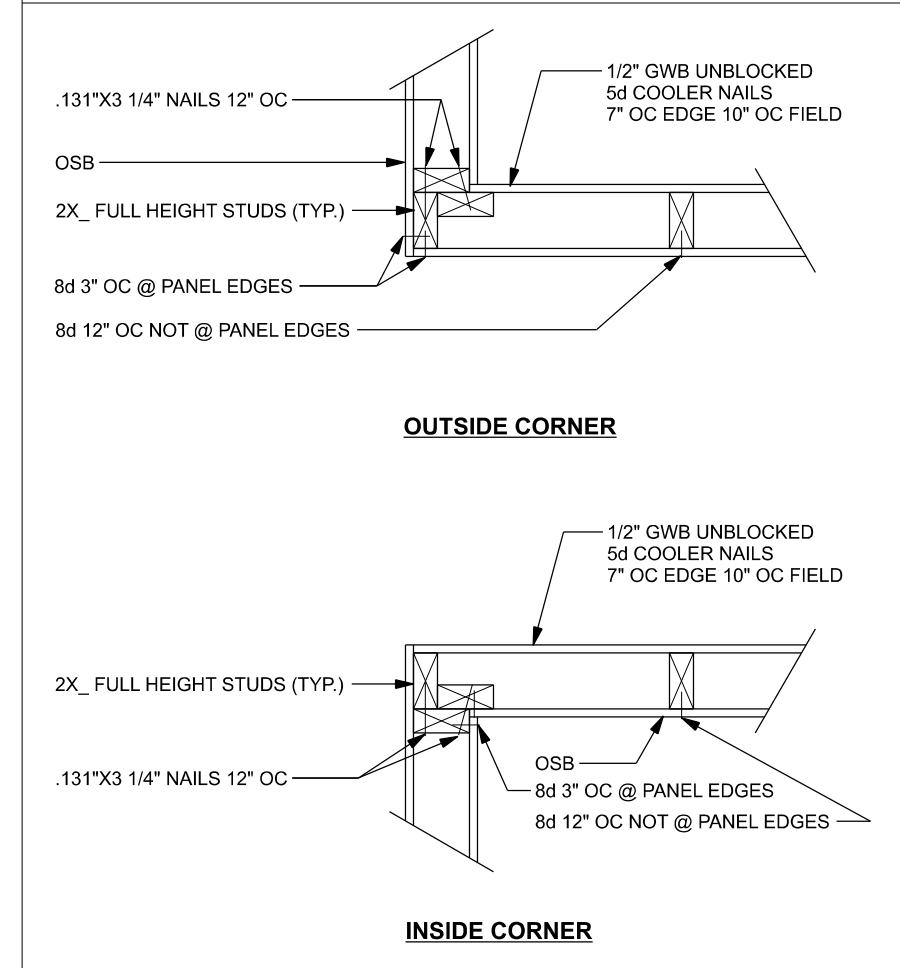




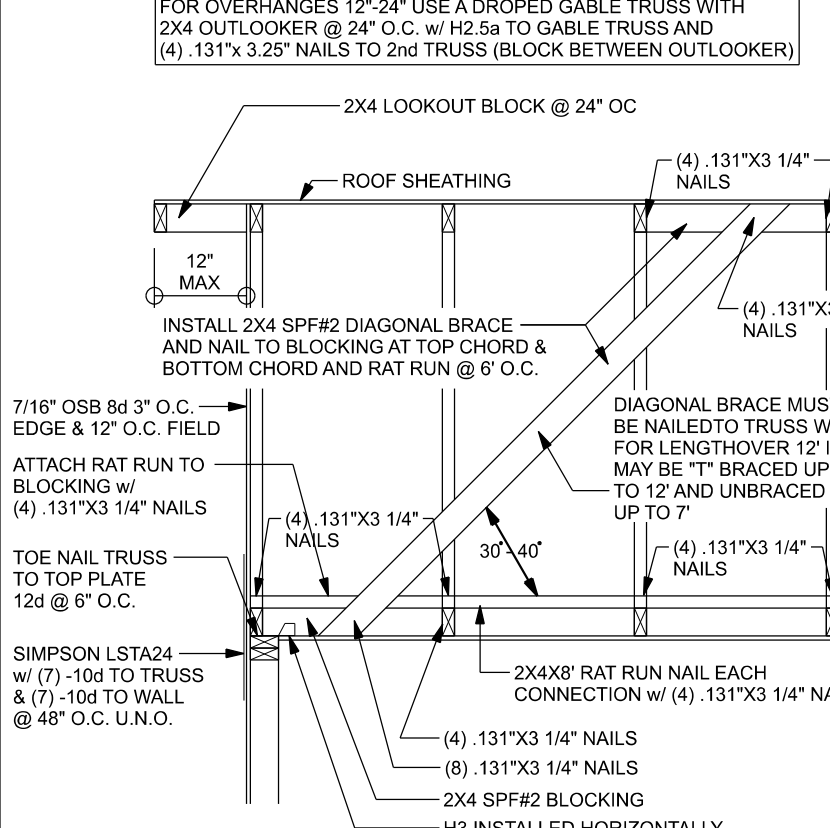
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



(TYP.) CORNER FRAMING
WOOD FRAME

ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. C	15/32"	ASTM F1667 RSR-01 (2 3/8" x 0.113") or ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.113")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc	

Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



(TYP.) GABLE BRACING DETAIL
WOOD FRAME

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

(TYP.) PORCH POST
ONE STORY WOOD

HEADER STRAP TABLE			
Uplift	Top Connection	Bottom Connection	Anchor Bolt Spacing
227 pif	SP2 @ 32" OC	SP1 @ 32" OC	48" OC
454 pif	SP2 @ 16" OC	SP1 @ 16" OC	32" OC
309 pif	LSTA24, 14-10d @ 48" OC	LSTA24, 14-10d @ 48" OC	48" OC
465 pif	LSTA24, 14-10d @ 32" OC	LSTA24, 14-10d @ 32" OC	32" OC

(2) MS 1A24, 18-100 header to jacks	HT 14
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SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.2.8 FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	

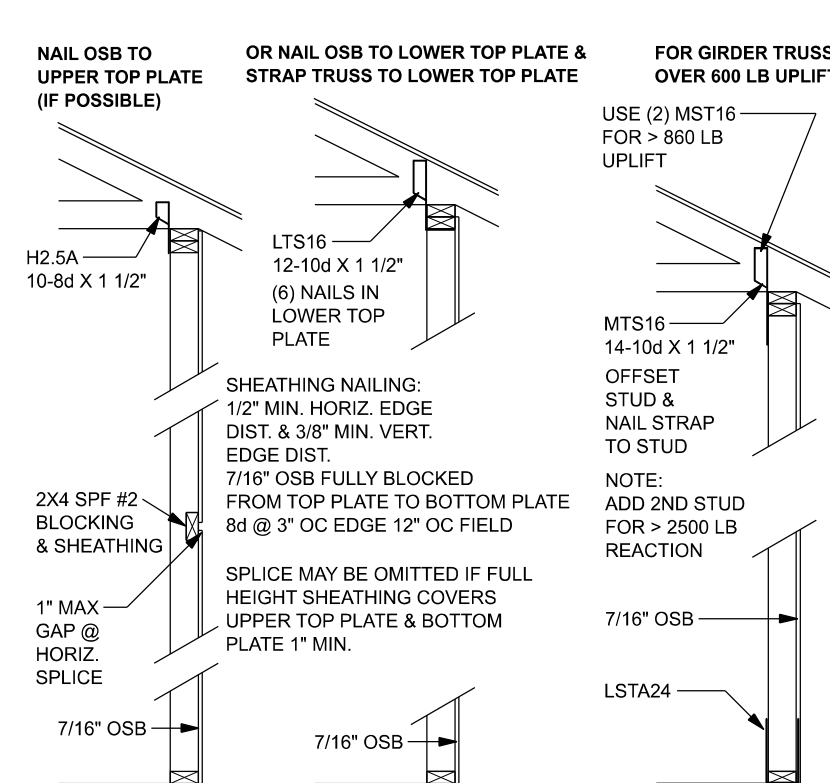
SILL PLATE SPANS FOR 10'-0" WALL HEIGHT			
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.8
	(1) 2x4	(2) 2x4	(1) 2x6 (2) 2x6
130 MPH EXP. C	5'-3"	7'-9"	7'-7" 11'-3"

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.8	
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-3"	7'-9"	7'-7"	11'-3"

SILL PLATE SPANS FOR 10'-0" WALL HEIGHT			
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2.8
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130 MPH EXP. C	5'-3"	7'-9"	7'-7"

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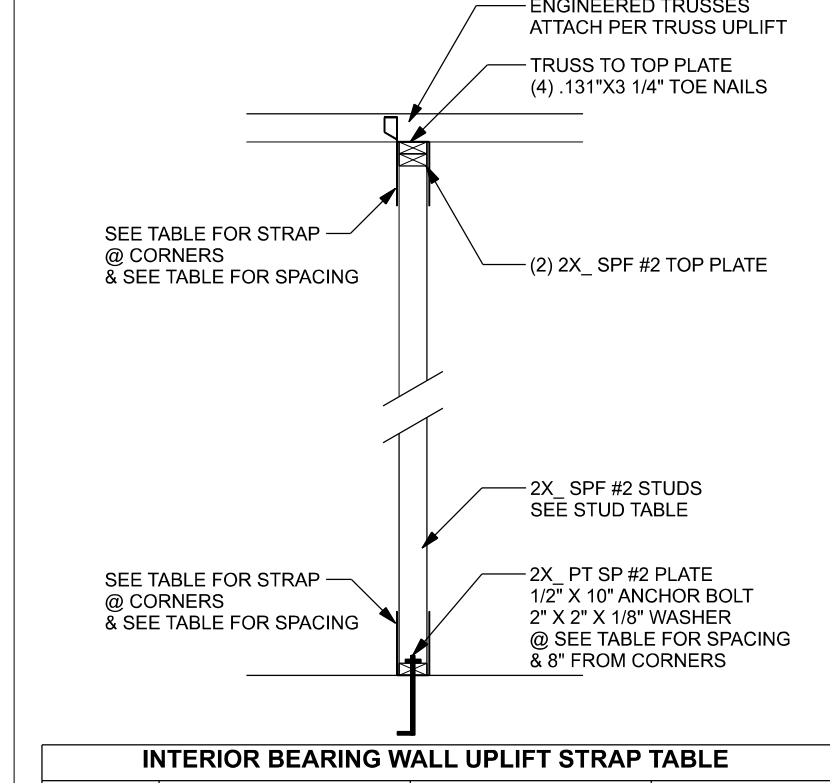
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CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	-	-	
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"	
575	496	H2.5A	4-8d x 1 1/2"	4-8d x 1 1/2"	
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"	
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"	
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	6-10d	6-10d	
1640	1455	MSTA24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
585	535	SP1	6-10d	4-10d	
1065	605	SP2	6-10d	6-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	16-16d x 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD	
4235	3640	HTT4	16-16d x 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor	
2200	ABU44	12-16d	5/8"x12" Drill & Epoxy		
2300	ABU66	12-16d	5/8"x12" Drill & Epoxy		
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200	ABU44	12-16d	5/8"x7" Drill & Epoxy		
2300	ABU66	12-16d	5/8"x7" Drill & Epoxy		

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

(TYP.) GABLE WALL w/ VAULTED CEILING
WOOD FRAME

(TYP.) PORCH POST
ONE STORY WOOD

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(TYP.) PORCH POST
ONE STORY WOOD

GENERAL NOTES:
TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER HAS SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2x8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1600 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVIDES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F_y = 80ksi, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C-1116. SUPPLIER TO PROVIDE ASTM C-1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR APPROVALS. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 315-86, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

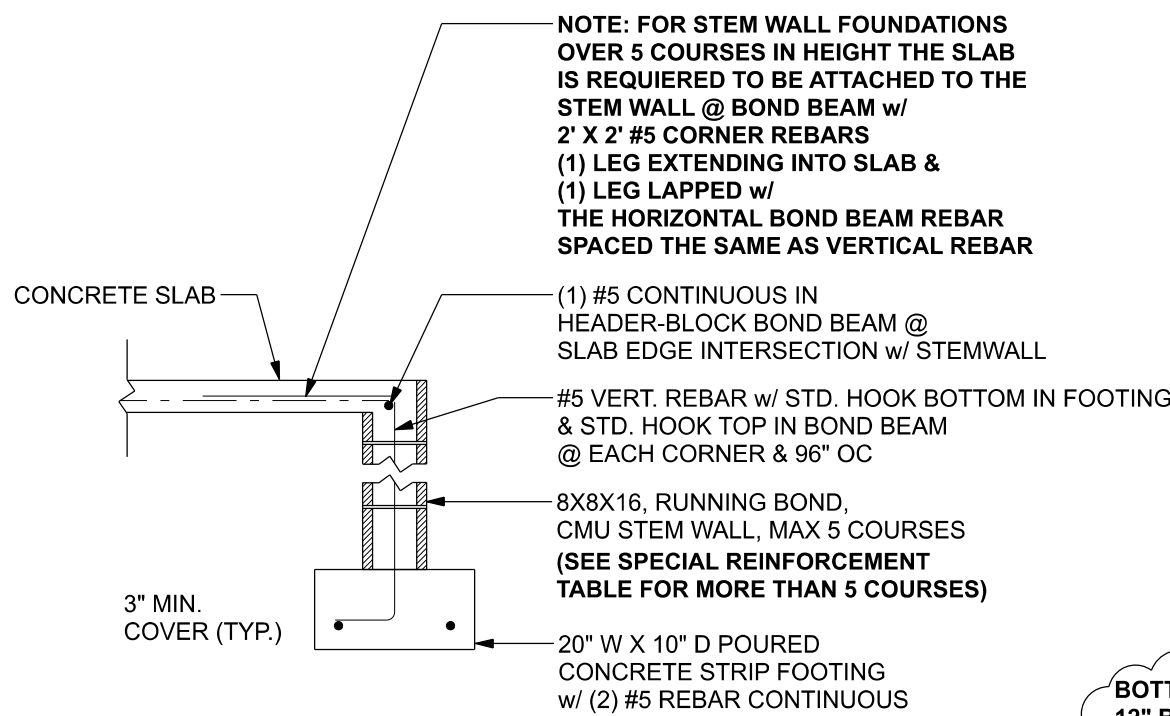
ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 10" IN GROUDED CMU.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

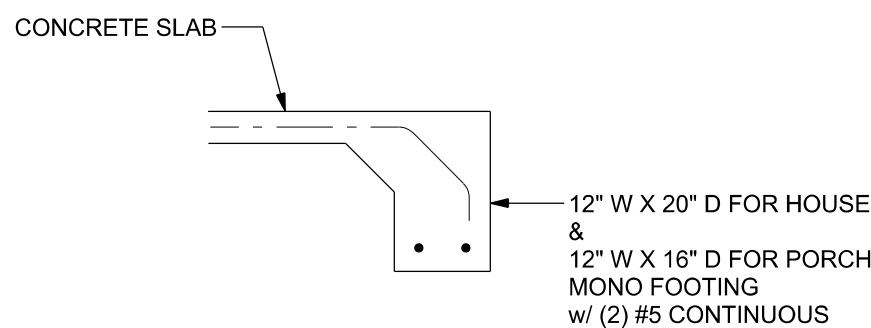
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL TRUSS SHEETS.



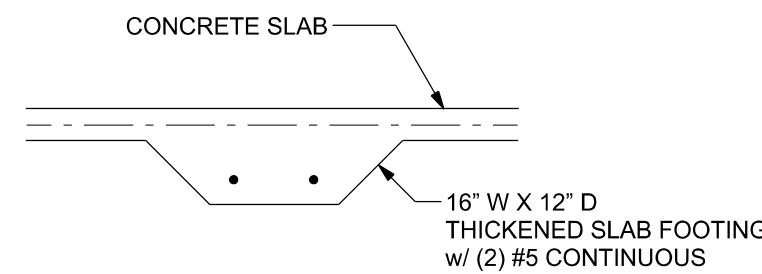
F1 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE:
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

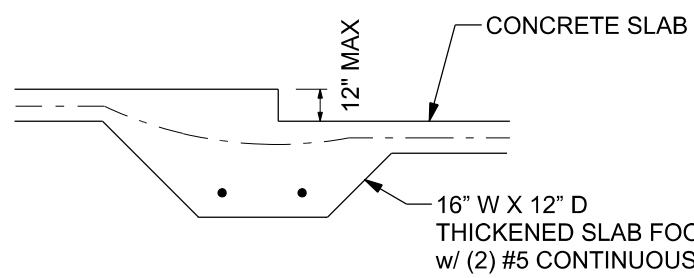
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F1 S-2 OPTIONAL MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



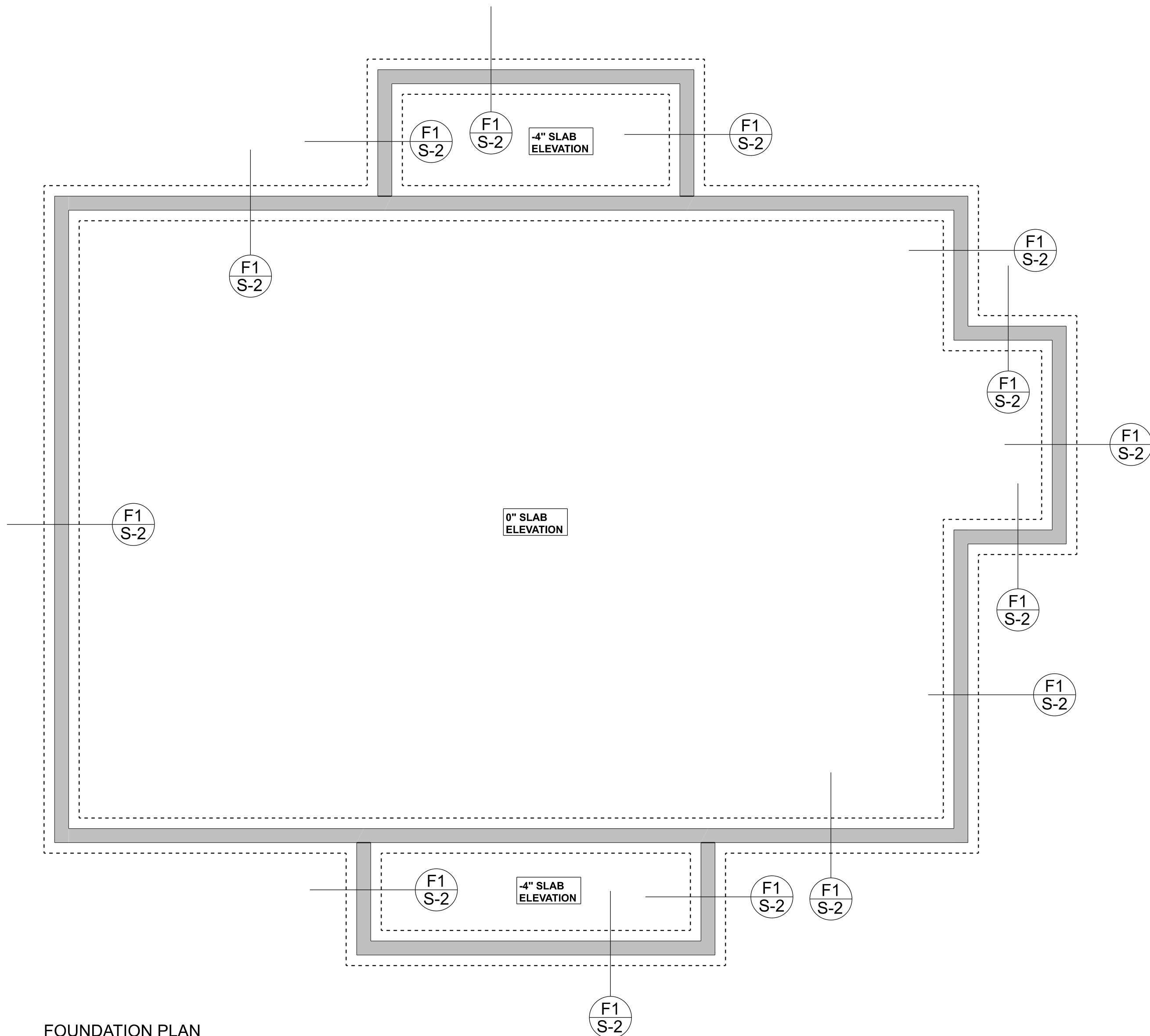
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls Fm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade 40, Type SW, Type FBS, 8"x2 1/2"x11 1/2"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A225, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- FN - 1) DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN - 2) CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN - 3) THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X8-14/14 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 5-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES TREATMENT METHOD CAN BE USED INSTEAD).

Amira Builders

Welsh Res.

PROJECT ADDRESS:
648 SW Lonicata Loop
FL White, FL 32038

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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Thursday, February 3, 2022

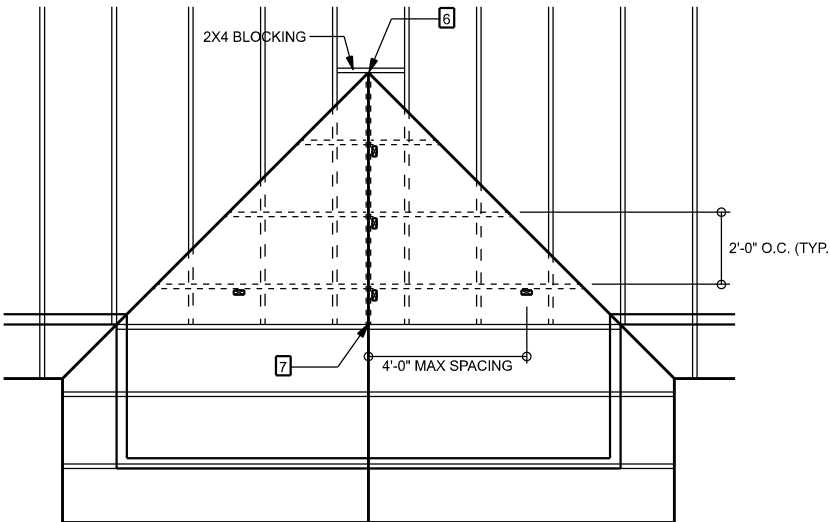
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JOB NUMBER:
220120

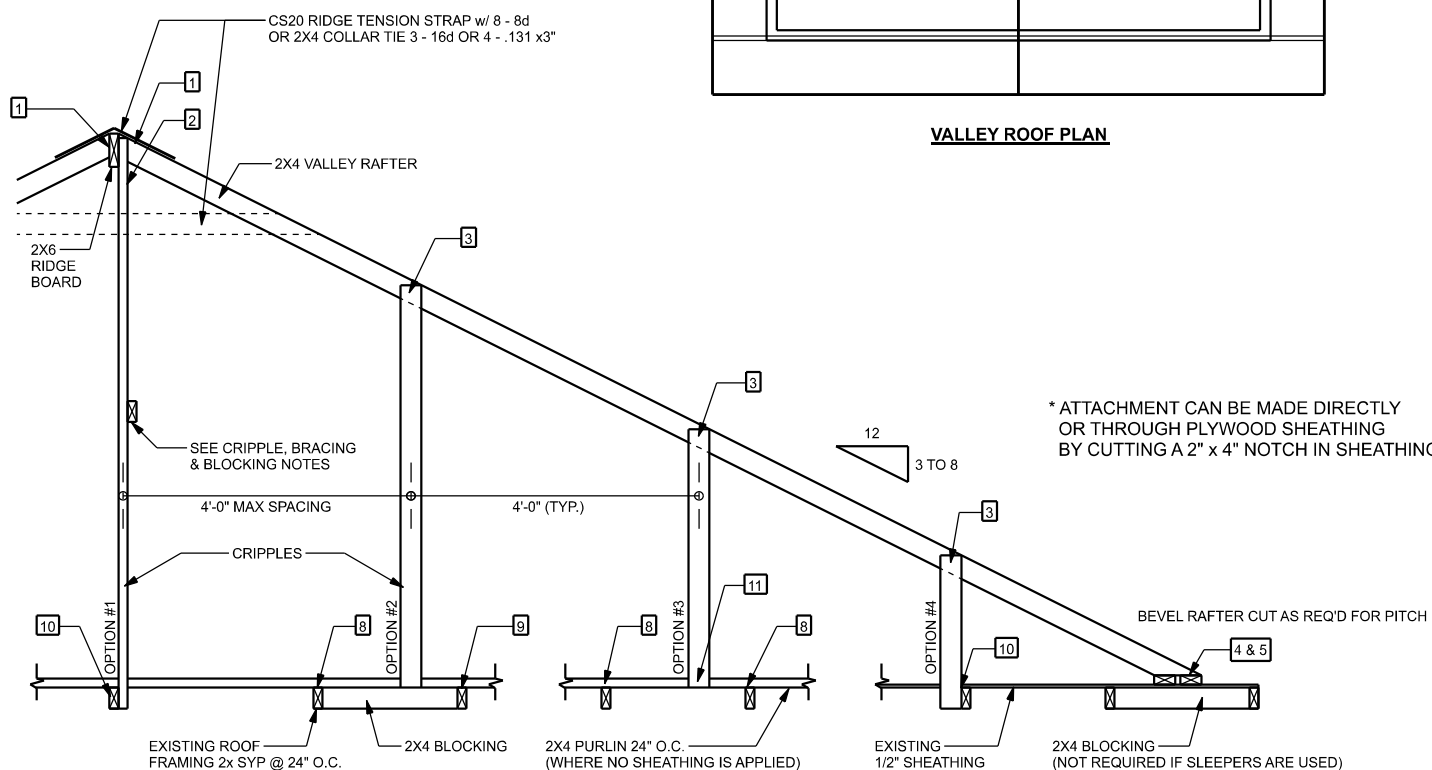
S-2
OF 3 SHEETS

LUMBER SIZE & GRADE MINIMUM REQUIREMENTS

RISE BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
FURLIN/LATERAL BRACING	2X4 SYP #2
SLEEPERS	2X (WIDTH OF RAFTER SEAT CUT) SPF #3 OR 2 PARALLEL 2X4 SPF #3
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



VALLEY ROOF PLAN



SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL

SCALE: 1/4" = 1'-0"

VALLEY ROOF PLAN MEMBER LEGEND

— TRUSS
 — TRUSS UNDER VALLEY FRAMING
 — VALLEY RAFTER OR RIDGE
 — CRIPPLE

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

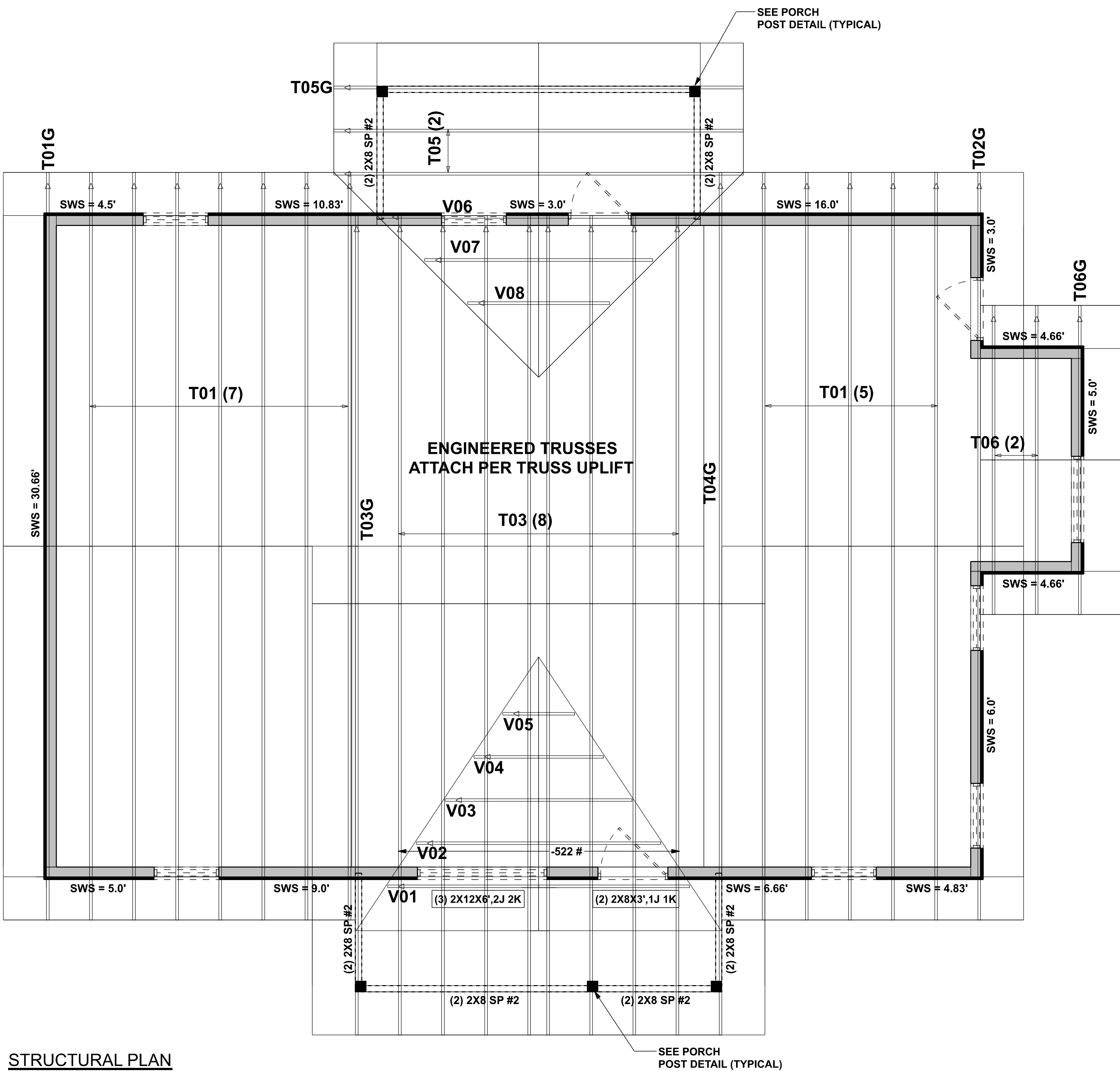
1. 2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3" TOE NAILS
2. CRIPPLE TO RIDGE	3-16d OR 6-131 x 3" FACE NAILS
3. CRIPPLE TO RAFTERS	3-16d OR 6-131 x 3" FACE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12-131 x 3" TOE NAILS
5. SLEEPER TO TRUSS	4-16d OR 6-131 x 3" FACE NAILS EACH TRUSS
6. RIDGE BOARD TO ROOF BLOCK	3-16d OR 6-131 x 3" TOE NAILS
7. RIDGE BOARD TO TRUSS	3-16d OR 6-131 x 3" TOE NAILS
8. FURLIN TO TRUSS (TYP.)	3-16d OR 6-131 x 3" NAILS
9. FURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO FURLIN)	4-16d OR 6-131 x 3" NAILS
10. TRUSS TO BLOCKING	3-16d OR 6-131 x 3" END NAILS
11. CRIPPLE TO TRUSS	3-16d OR 6-131 x 3" FACE NAILS
12. CRIPPLE TO FURLIN	3-16d OR 6-131 x 3" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPAN: 8'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2.
 MAXIMUM ROOF AREA PER SUPPORT: 1600 IN ZONE 2 & 3, 2400 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 1600 OR 7'-0" X 3'-0" SPAN = 1600)
 FURLIN REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED.
 FURLIN SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6" AND NAIL UPWARDS THROUGH SHEATHING INTO FURLIN WITH A MINIMUM OF 8-16d COMMON WIRE NAILS.
 THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
 - SPANS (DISTANCE BETWEEN HEELS) 40'-0" OR LESS
 - MAXIMUM VALLEY HEIGHT: 16'-0" OR LESS
 - MAXIMUM WIND SPEED: 100 MPH
 - MAXIMUM MEAN ROOF HEIGHT: 30 FEET
 - MEETS IRC / ASCE 7-10 WIND REQUIREMENTS
 - EXPOSURE CATEGORY "C", 1 = 1.0, Kd = 1.0
 - ENCLOSED BUILDING

CRIPPLE, BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 9'-0" TO 10'-0" LONG NAILED w/ 2-10d NAILS OR 2X4 1" OR SCAB BRACE NAILS TO FLAT EDGE OF CRIPPLE WITH 6 NAILS @ 6" O.C. 1" OR SCAB MUST BE 80% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB's OR BOTH FACES w/ 1" OR SCAB. USE STRESS GRADED LUMBER & BOB OR COMMON NAILS.
 - NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD.
 - INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
 - INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
 - APPLY ALL NAILING IN ACCORDANCE TO NDS-1907 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



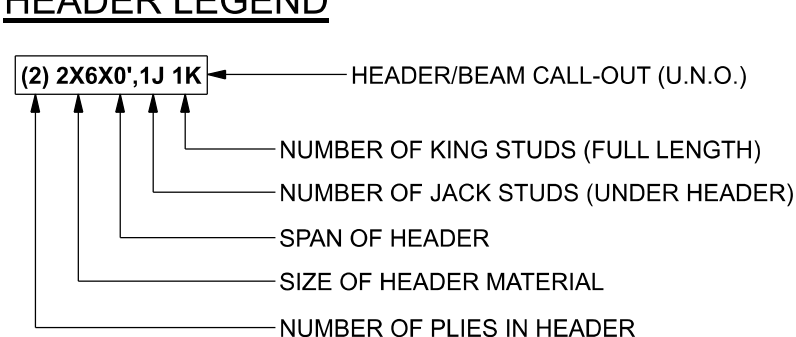
STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 ALL HEADERS w/ UPLIFT TO BE STRAPPED DOWN @ EACH SIDE WITH (1) LSTA24, 14-10d @ TOP & BOTTOM OF WALL WRAP UNDER BOTTOM PLATE & OVER TOP PLATE 1/2" X 10" ANCHOR BOLT w/ 3" X 3" X 1/4" WASHER MUST BE LOCATED WITHIN 6" OF KING STUD @ ALL DOOR LOCATIONS (U.N.O.)
- SN-4 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-5 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-6 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	15631 LBF	16593 LBF
REQUIRED	11506 LBF	7785 LBF

Amira Builders

Weist Res.

PROJECT ADDRESS:
 163 SW Midtown Place
 Suite 103
 Lake City, FL 32025

DIMENSIONS:
 Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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Thursday, February 3, 2022

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 386.754.5419
 disoswaydesign@gmail.com

JOB NUMBER:
 220120

S-3
 OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2530630