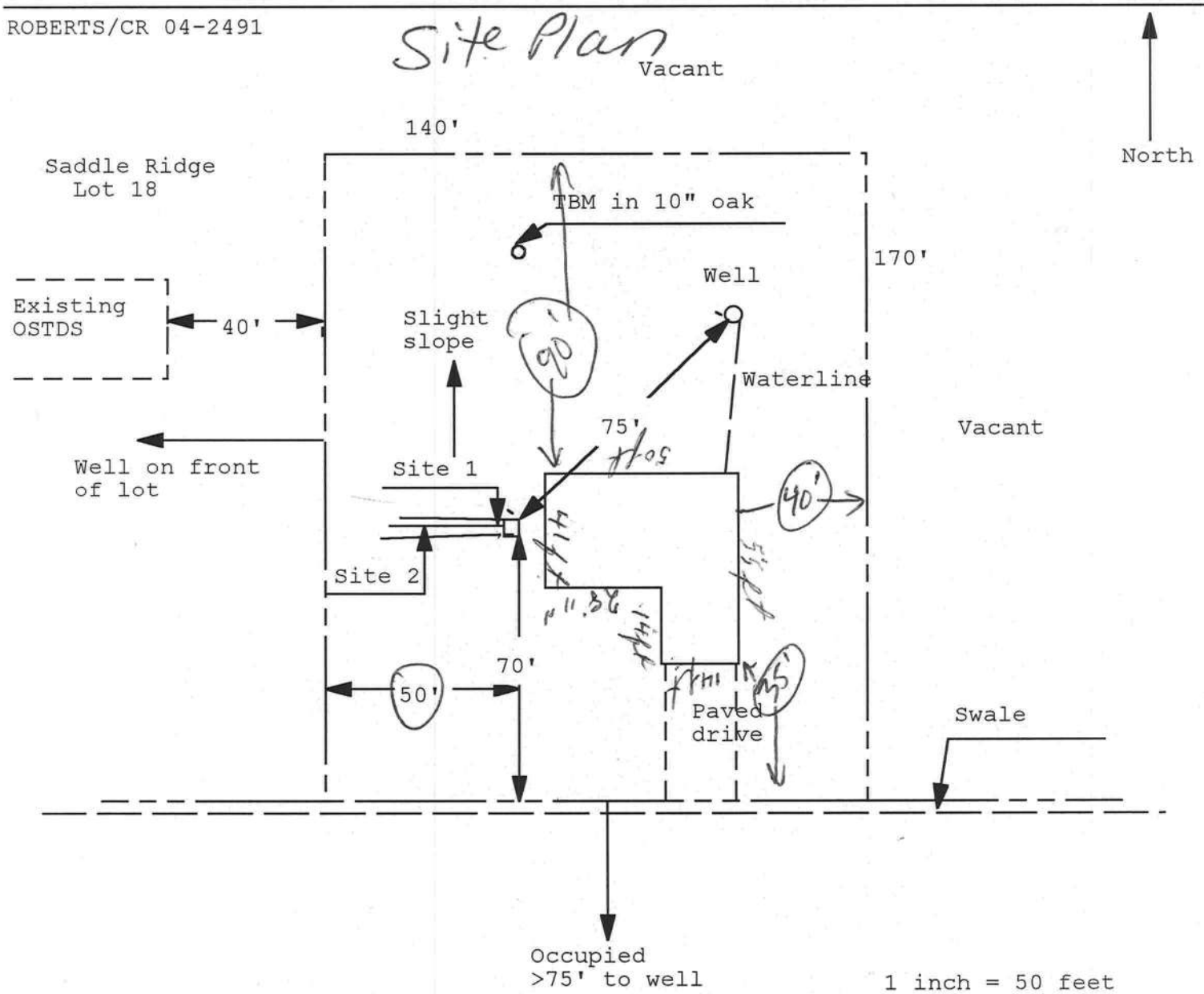
Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan

Permit Application Number: 05-0121N

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT

ROBERTS/CR 04-2491

Site Plan



Site Plan Submitted By Paul Lloyd Date 12/21/04

Plan Approved ✓ Not Approved ____ Date 12/21/04

By Paul Lloyd CPHU

Notes: _____

FORM 600A-2001

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**Mike Roberts**	Builder:	**Mike Roberts**
Address:	**Lot: , Sub: , Plat:**	Permitting Office:	Columbia
City, State:	**, FL 32025-**	Permit Number:	22831
Owner:	**Spec House**	Jurisdiction Number:	221000
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 29.0 kBtu/hr
3. Number of units, if multi-family	1		SEER: 11.00
4. Number of Bedrooms	3	b. N/A	
5. Is this a worst case?	No		
6. Conditioned floor area (ft²)	1495 ft²	c. N/A	
7. Glass area & type			
a. Clear - single pane	0.0 ft²	13. Heating systems	
b. Clear - double pane	156.0 ft²	a. Electric Heat Pump	Cap: 29.0 kBtu/hr
c. Tint/other SHGC - single pane	0.0 ft²		HSPF: 6.80
d. Tint/other SHGC - double pane	0.0 ft²	b. N/A	
8. Floor types			
a. Slab-On-Grade Edge Insulation	R=0.0, 184.0(p) ft	c. N/A	
b. N/A			
c. N/A		14. Hot water systems	
9. Wall types		a. Electric Resistance	Cap: 50.0 gallons
a. Frame, Wood, Exterior	R=13.0, 1052.0 ft²		EF: 0.90
b. Frame, Wood, Adjacent	R=13.0, 270.0 ft²	b. N/A	
c. N/A			
d. N/A		c. Conservation credits	
e. N/A		(HR-Heat recovery, Solar DHP-Dedicated heat pump)	
10. Ceiling types		15. HVAC credits	
a. Under Attic	R=30.0, 1575.0 ft²	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 40.0 ft		
b. N/A			

Glass/Floor Area: 0.10

Total as-built points: 22171
Total base points: 24047

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Will Myers

DATE: _____

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

DATE: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

GREAT SEAL OF THE STATE OF FLORIDA
IN GOD WE TRUST

BUILDING OFFICIAL: _____

DATE: _____

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , , FL, 32025-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1495.0	20.04	5392.8	Double, Clear	W	15.5	7.7	40.0	36.99	0.41	604.0
				Double, Clear	W	1.5	4.0	9.0	36.99	0.82	272.1
				Double, Clear	W	1.5	6.0	30.0	36.99	0.91	1013.5
				Double, Clear	N	1.5	4.0	6.0	19.22	0.88	101.6
				Double, Clear	E	7.5	7.7	22.0	40.22	0.52	463.0
				Double, Clear	E	1.5	6.0	45.0	40.22	0.91	1652.1
				Double, Clear	S	1.5	2.0	4.0	34.50	0.57	78.0
				As-Built Total:				**156.0**			**4184.4**
WALL TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Adjacent	270.0	0.70	189.0	Frame, Wood, Exterior			13.0	1052.0	1.50		1578.0
Exterior	1052.0	1.70	1788.4	Frame, Wood, Adjacent			13.0	270.0	0.60		162.0
Base Total:	**1322.0**		**1977.4**	**As-Built Total:**				**1322.0**			**1740.0**
DOOR TYPES	Area X	BSPM =	Points	Type				Area X	SPM =		Points
Adjacent	18.0	2.40	43.2	Exterior Insulated				20.0	4.10		82.0
Exterior	20.0	6.10	122.0	Adjacent Insulated				18.0	1.60		28.8
Base Total:	**38.0**		**165.2**	**As-Built Total:**				**38.0**			**110.8**
CEILING TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM X SCM =		Points
Under Attic	1495.0	1.73	2586.4	Under Attic			30.0	1575.0	1.73 X 1.00		2724.8
Base Total:	**1495.0**		**2586.4**	**As-Built Total:**				**1575.0**			**2724.8**
FLOOR TYPES	Area X	BSPM =	Points	Type			R-Value	Area X	SPM =		Points
Slab	184.0(p)	-37.0	-6808.0	Slab-On-Grade Edge Insulation			0.0	184.0(p	-41.20		-7580.8
Raised	0.0	0.00	0.0								
Base Total:			**-6808.0**	**As-Built Total:**				**184.0**			**-7580.8**
INFILTRATION	Area X	BSPM =	Points					Area X	SPM =		Points
	1495.0	10.21	15264.0					1495.0	10.21		15264.0

FORM 600A-2001

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , , FL, 32025-**	PERMIT #:

BASE			AS-BUILT					
Summer Base Points:		**18577.7**	**Summer As-Built Points:**					**16443.1**
Total Summer Points	System Multiplier	Cooling Points	Total Component	Cap Ratio	Duct Multiplier (DM x DSM x AHU)	System Multiplier	Credit Multiplier	Cooling Points
18577.7	**0.4266**	**7925.2**	16443.1 **16443.1**	1.000 **1.00**	(1.090 x 1.147 x 1.00) **1.250**	0.310 **0.310**	1.000 **1.000**	6378.5 **6378.5**

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , , FL, 32025-** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BWPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF	= Points
.18	1495.0	12.74	3428.3	Double, Clear	W	15.5	7.7	40.0	10.77	1.22	526.5
				Double, Clear	W	1.5	4.0	9.0	10.77	1.05	102.0
				Double, Clear	W	1.5	6.0	30.0	10.77	1.02	330.5
				Double, Clear	N	1.5	4.0	6.0	14.30	1.01	86.3
				Double, Clear	E	7.5	7.7	22.0	9.09	1.28	255.9
				Double, Clear	E	1.5	6.0	45.0	9.09	1.04	423.6
				Double, Clear	S	1.5	2.0	4.0	4.03	2.27	36.5
				As-Built Total:				**156.0**			**1761.5**
WALL TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Adjacent	270.0	3.60	972.0	Frame, Wood, Exterior			13.0	1052.0	3.40		3576.8
Exterior	1052.0	3.70	3892.4	Frame, Wood, Adjacent			13.0	270.0	3.30		891.0
Base Total:	**1322.0**		**4864.4**	**As-Built Total:**				**1322.0**			**4467.8**
DOOR TYPES	Area X	BWPM =	Points	Type				Area X	WPM =		Points
Adjacent	18.0	11.50	207.0	Exterior Insulated				20.0	8.40		168.0
Exterior	20.0	12.30	246.0	Adjacent Insulated				18.0	8.00		144.0
Base Total:	**38.0**		**453.0**	**As-Built Total:**				**38.0**			**312.0**
CEILING TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM X	WCM =	Points
Under Attic	1495.0	2.05	3064.8	Under Attic			30.0	1575.0	2.05 X	1.00	3228.8
Base Total:	**1495.0**		**3064.8**	**As-Built Total:**				**1575.0**			**3228.8**
FLOOR TYPES	Area X	BWPM =	Points	Type			R-Value	Area X	WPM =		Points
Slab	184.0(p)	8.9	1637.6	Slab-On-Grade Edge Insulation			0.0	184.0(p	18.80		3459.2
Raised	0.0	0.00	0.0								
Base Total:			**1637.6**	**As-Built Total:**				**184.0**			**3459.2**
INFILTRATION	Area X	BWPM =	Points					Area X	WPM =		Points
	1495.0	-0.59	-882.0					1495.0	-0.59		-882.0

FORM 600A-2001

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , , FL, 32025-**	PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**12566.0**	**Winter As-Built Points:**					**12347.2**
Total Winter Points	X System Multiplier	= Heating Points	Total Component	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
12566.0	**0.6274**	**7883.9**	12347.2 **12347.2**	1.000 **1.00**	(1.069 x 1.169 x 1.00) **1.250**	0.501 **0.501**	1.000 **1.000**	7737.6 **7737.6**

FORM 600A-2001

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , , FL, 32025-** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X Tank Ratio	X Multiplier	X Credit Multiplier	= Total	
3		2746.00	8238.0	50.0	0.90	3	1.00	2684.98	1.00	8054.9	
				As-Built Total:							**8054.9**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
7925	**7884**	**8238**	**24047**	**6378**	**7738**	**8055**	**22171**

PASS

FORM.600A-2001

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: , Sub: , Plat: , , FL, 32025-** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 6-12. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.4

The higher the score, the more efficient the home.

Spec House, Lot: , Sub: , Plat: , , FL, 32025-

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family	a.	Central Unit	Cap: 29.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 11.00
4.	Number of Bedrooms	3	b.	N/A	
5.	Is this a worst case?	No			
6.	Conditioned floor area (ft²)	1495 ft²	c.	N/A	
7.	Glass area & type				
a.	Clear - single pane	0.0 ft²	13.	Heating systems	
b.	Clear - double pane	156.0 ft²	a.	Electric Heat Pump	Cap: 29.0 kBtu/hr
c.	Tint/other SHGC - single pane	0.0 ft²			HSPF: 6.80
d.	Tint/other SHGC - double pane	0.0 ft²	b.	N/A	
8.	Floor types				
a.	Slab-On-Grade Edge Insulation	R=0.0, 184.0(p) ft	c.	N/A	
b.	N/A				
c.	N/A		14.	Hot water systems	
9.	Wall types		a.	Electric Resistance	Cap: 50.0 gallons
a.	Frame, Wood, Exterior	R=13.0, 1052.0 ft²			EF: 0.90
b.	Frame, Wood, Adjacent	R=13.0, 270.0 ft²	b.	N/A	
c.	N/A				
d.	N/A		c.	Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic	R=30.0, 1575.0 ft²	15.	HVAC credits	
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 40.0 ft		MZ-C-Multizone cooling,	
b.	N/A			MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

Address of New Home: _____ City/FL Zip: _____

**NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar™ designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/488-7824.*

This Instrument Prepared by & return to:
Name: ***JOYCE KIRPACH, an employee of TITLE OFFICES, LLC***
Address: ***1089 SW MAIN BLVD. LAKE CITY, FLORIDA 32025***
04Y-02021JK
Parcel I.D. #: ***03084-018***

Inst:2004004502 Date:02/27/2004 Time:16:54
Doc Stamp-Deed : 84.00
MK DC,P.DeWitt Cason,Columbia County B:1008 P:1175

SPACE ABOVE THIS LINE FOR PROCESSING DATA | *SPACE ABOVE THIS LINE FOR RECORDING DATA*

THIS WARRANTY DEED *Made the* ***24th*** *day of* ***February,*** *A.D.* ***2004,*** *by*

JANICE L. PARKS, Single *hereinafter called the grantor, to*

MICHAEL WARREN ROBERTS, Single *whose post office address is*
Rt 3, Box 148 B2, Lake City, FL. 32025
hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument, singular and plural, the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporations, wherever the context so admits or requires.)

Witnesseth: *That the grantor, for and in consideration of the sum of $10.00 and other valuable consideration, receipt whereof is hereby acknowledged, does hereby grant, bargain, sell, alien, remise, release, convey and confirm unto the grantee all that certain land situate in* ***Columbia County, State of FLORIDA,*** *viz:*

Lot 18, SADDLE RIDGE, according to the map or plat thereof as recorded in Plat Book 5, Page 67, of the Public Records of Columbia County, FLORIDA.

Subject to declaration of covenants, conditions and restrictions as recorded in Official Records Book 579 Page 609, amended in Official Records Book 606, Page 388, but omitting any covenant or restrictions as to race, color, religion, sex, handicap, familial status or national origin.

Easement, recorded in Official Records Book 572, Page 367, of the Public Records of Columbia County, FLORIDA.

Restrictions, conditions, reservations, easements, and other matters common to the subdivision or shown on the map or plat thereof recorded in Plat Book 5, Page 67, but omitting any covenant or restriction based on race, color, religion, sex, handicap, familial status or national origin.

Together *with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.*

To Have and to Hold *the same in fee simple forever.*

And *the grantor hereby covenants with said grantee that she is lawfully seized of said land in fee simple; that she has good right and lawful authority to sell and convey said land, and hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever, and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2003.*

In Witness Whereof, *the said grantor has signed and sealed these presents, the day and year first above written.*

Signed, sealed and delivered in the presence of:

Witness Signature
ENA JANE SIMMONS
Printed Name
Witness Signature
WILLIAM NEWCOMB
Printed Name

L.S.
JANICE L. PARKS
Address:
2536 HOOPERS ISLAND ROAD, FISHING CREEK, MD 21634

STATE OF Maryland
COUNTY OF Dorchester

The foregoing instrument was acknowledged before me this 24th *day of* **February, 2004**, *by* **JANICE L. PARKS**, *who is known to me or who has produced* Janice L Parks *as identification.*

Notary Public
My commission expires Oct 1 2006

Inst:2004004502 Date:02/27/2004 Time:16:54
Doc Stamp-Deed : 84.00
DC,P.DeWitt Cason,Columbia County B:1008 P:1176

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing
is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Rose Ann Wells
Deputy Clerk
Date Feb 7 2006

CLERK CIRCUIT COURT
COLUMBIA COUNTY, FLORIDA

Columbia County Building Permit Application

left message

Revised 9-23-04

For Office Use Only Application # 05-02-25 Date Received 2-8-05 By LH Permit # 546 / 23831

Application Approved by - Zoning Official BK Date _____ Plans Examiner JTH OK Date 2-9-05

Flood Zone X Development Permit NA Zoning RR Land Use Plan Map Category RLD

Comments Floor ~~height~~ height letter recieved / Have Noc

Applicants Name ~~Mike Roberts~~ Mack Robinson Phone 755-9476

Address ~~Rt.~~ 657 S.W. CAtherine Lane 32025

Owners Name ~~Owner~~ Michael Warren Roberts Phone _____

911 Address 4231 S.W. Co. Rd. 242 Lake City 32024

Contractors Name MACK Robinson Phone 755 2492

Address _____

Fee Simple Owner Name & Address _____

Bonding Co. Name & Address _____

Architect/Engineer Name & Address Will Myers MARK DISOSWAY

Mortgage Lenders Name & Address None

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progressive Energy

Property ID Number 03084-018 (21-4S-16) Estimated Cost of Construction _____

Subdivision Name Saddle Ridge Lot 18 Block ____ Unit ____ Phase ____

Driving Directions S. 47 turn on 242 Rt. go west .5 miles just before TRAFFIC Light on BRANFORD HYWAY Lot on the Right

Type of Construction Wood FRAME Number of Existing Dwellings on Property 0

Total Acreage .56 Ac Lot Size _____ Do you need a – Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 35 Side 40 Side 50 Rear 90

Total Building Height 18'2 Number of Stories 1 Heated Floor Area 1495 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Mike Roberts
Owner Builder or Agent (Including Contractor)

Mack Robinson
Contractor Signature
Contractors License Number _____
Competency Card Number _____

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this _____ day of _____ 20____.

Personally known____ or Produced Identification____

NOTARY STAMP/SEAL
LAURIE HOBSON
MY COMMISSION # DD 333503
EXPIRES: June 28, 2008
Bonded Thru Notary Public Underwriters

Notary Signature

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000000546

DATE 02/22/2005 PARCEL ID # 21-4S-16-03084-018

APPLICANT R. MACK ROBINSON PHONE 386.755.2492

ADDRESS 24262 US HWY 129 O'BRIEN FL 32071

OWNER MIKE ROBERTS PHONE 386.755.9476

ADDRESS 4231 SW CR 242 LAKE CITY FL 32024

CONTRACTOR R. MACK ROBINSON,JR. PHONE 386.755.2492

LOCATION OF PROPERTY 47-S TO C-242,GO W 5 MILES JUST BEFORE TRAFFIC LIGHT ON SR-247

LOT IS ON THE RIGHT. (TURN R C-242)

SUBDIVISION/LOT/BLOCK/PHASE/UNIT SADDLE RIDGE 18

SIGNATURE ✓ Mack Robinson

INSTALLATION REQUIREMENTS

[X] Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

[ ] Culvert installation shall conform to the approved site plan standards.

[ ] Department of Transportation Permit installation approved standards.

[ ] Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA

Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph 386-754-5419, Fax 386-754-6749

04 Dec 2004

Building and Zoning, Columbia County, Florida

Re: Site Evaluation, Mike Roberts, Spec House, Lot 18, Saddle Ridge Subdivision, 21-4S-16-03084-018, Columbia County, FL

Dear Building Inspector:

I have reviewed the Flood Insurance Rate Map and NGS topographic map and performed a site evaluation for the Mike Roberts, Spec House, Lot 18, Saddle Ridge Subdivision, 21-4S-16-03084-018, Columbia County, FL. The existing grade elevation within the house perimeter is less than one foot above the nearby county road that it fronts on CR 242. The lot is in Zone X on the FEMA rate map.

Based on my personal site inspection the grade at the building perimeter is higher than the land to the rear of the lot and beyond. Anecdotal evidence from an interview with a neighbor states that there were no flooding issues in all of Saddle Ridge during the 2004 storms but there was standing water in the low corner of the field beside Saddle Creek. Also, it is apparent from the topo map that the lot is higher than Rose Creek and it seems that storm water would run off to the rear onto lower land thru a narrows and on down to Rose Creek.

To avoid flood and water damage to the house I recommend the finished floor elevation be a minimum of 18" above the elevation everywhere along the path shown on the topo map. The builder must verify by his own grade shots or a topo survey that a continuous drainage path 18" below finished floor level exists to Rose Creek. The lot should be graded such that storm water will drain from all areas around the perimeter of the house to run to the rear. (This will not necessarily prevent ponding in the yard but it should keep the finished floor above the pond if one occurs.)

The finished floor elevation must be minimum 6" above finished grade per FBC2001. The finished grade should slope down from that elevation for another 6" within 12 feet away from the house in all directions so that all runoff drains away from the house. The owner must maintain the swales, slopes, and ditch to provide free drainage to the ditch and prevent any possibility of storm water backing up into the house.

Also, the owner should be aware that if free drainage is not maintained across neighboring lands in the path as shown on the topo map his house would be in a "bowl" surrounded by higher ground and would be more susceptible to flooding. Therefore if he sees any filling or grading taking place on that path it could potentially affect his own property.

Sincerely,

Mark Disosway
03DEC04

Mark Disosway, PE

FBC 2001, SECTION 1804 FOOTINGS AND FOUNDATIONS

§1804.1 General

§1804.1.1 Foundations shall be built on undisturbed soil or properly compacted fill material. Foundations shall be constructed of materials described in this chapter.

§1804.1.2 Pile foundations shall be designed and constructed in accordance with §1805.

§1804.1.3 The bottom of foundations shall extend no less than 12 inches (305 mm) below finish grade.

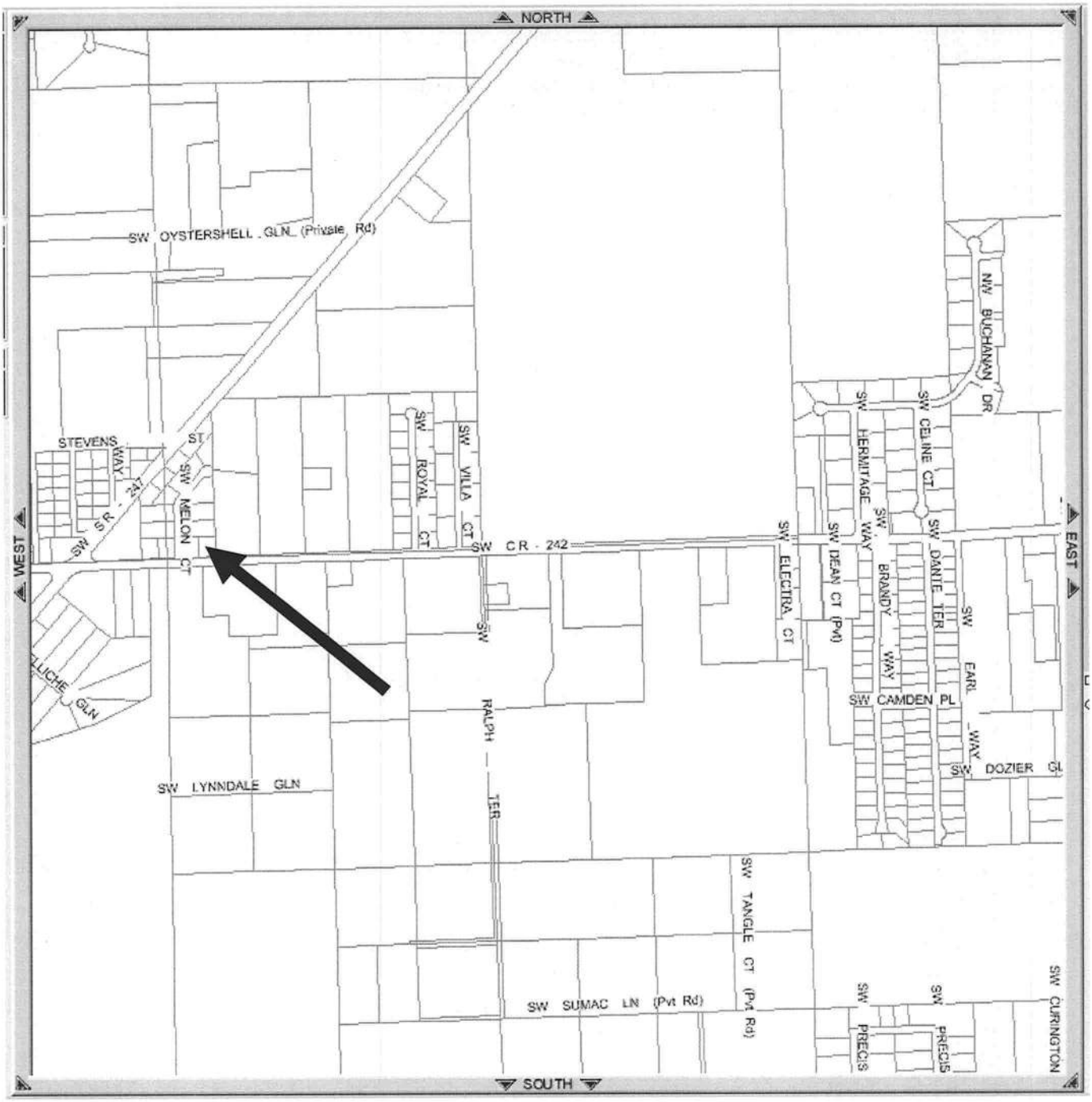
§1804.1.4 Temporary buildings and buildings not exceeding one story in height and 400 sq ft (37 m^2) in area shall be exempt from these requirements.

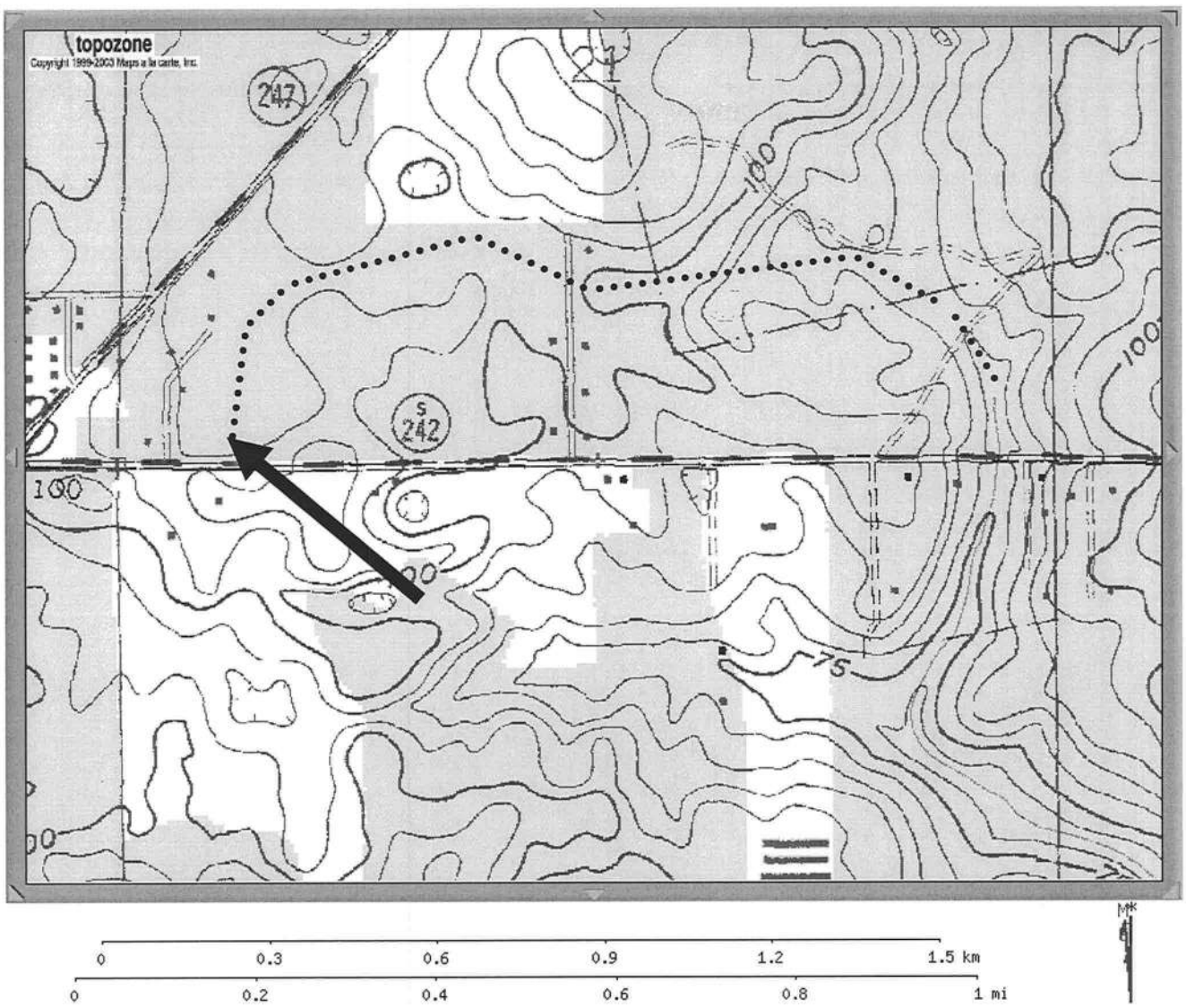
§1804.1.5 Excavations for foundations shall be backfilled with soil which is free of organic material, construction debris and large rocks.

§1804.1.6 Where water impacts the ground from a roof valley, downspout, scupper or other rain water collection or diversion device, provisions shall be made to prevent soil erosion and direct the water away from the foundation.

§1804.1.7 Finish grade shall be sloped away from the foundation for drainage.

§1804.1.8 The area under footings, foundations and concrete slabs on grade shall have all vegetation, stumps, roots and foreign materials removed prior to their construction. Fill material shall be free of vegetation and foreign material.


NORTH
SOUTH
EAST
WEST
SW OYSTERSHELL GLN (Private Rd)
STEVENS WAY
SW 247 ST
SW MELON CT
SW ROYAL CT
SW VILLA CT
SW CR - 242
SW ELECTRA CT
SW RALPH TER
SW LYNNDALE GLN
SW DEAN CT (Pvt)
SW HERMITAGE WAY
SW CELINE CT
NW BUCHANAN DR
SW DANTE TER
SW BRANDY WAY
SW CAMDEN PL
SW EARL WAY
SW DOZIER GL
SW TANGLE CT (Pvt Rd)
SW SUMAC LN (Pvt Rd)
SW PRECIS
SW CURINGTON


topozone
Copyright 1999-2003 Maps a la carte, Inc.
247
242
100
100
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75
0
0.3
0.6
0.9
1.2
1.5 km
0
0.2
0.4
0.6
0.8
1 mi


NORTH
WEST
EAST
SOUTH
SW STEVENS ST
SW DUNN WAY
SW GREMLIN WAY
SW DAYTONA WAY
SW MELON CT
SW ROYAL CT
SW VILLA CT
SW C R
456.27'
672.87'
672.67'
SW RALPH

FEAGLE & FEAGLE, ATTORNEYS, P.A.
ATTORNEYS AT LAW
153 NE MADISON STREET
POST OFFICE BOX 1653
LAKE CITY, FLORIDA 32056-1653
(386) 752-7191
Fax: (386) 758-0950

Marlin M. Feagle
e-mail: leagle@bellsouth.net

Mark E. Feagle
e-mail: mefeagle@bellsouth.net

Fax Cover Sheet

Name: JOHN KERCE
Organization: PLANNING & ZONING OFFICE
Fax: 758-2160
From: MARLIN M. FEAGLE
Date: MAY 9, 2005
Subject: LOT 18, SADDLE RIDGE SUBDIVISION
Pages: 2, INCLUDING COVER SHEET

☐ Urgent ☐ Reply ASAP ☐ Please Comment ☑ For Your Records

Comments:

From the desk of...
Diane S. Edenfield
Secretary
FEAGLE & FEAGLE, ATTORNEYS, P.A.
153 NE Madison Street
Post Office Box 1653
Lake City, Florida 32056-1653
386/752-7191

FEAGLE & FEAGLE, ATTORNEYS, P.A.
ATTORNEYS AT LAW
153 NE MADISON STREET
POST OFFICE BOX 1653
LAKE CITY, FLORIDA 32056-1653
(386) 752-7191
Fax: (386) 758-0950

Mark E. Feagle
e-mail: mefeagle@bellsouth.net

Marlin M. Feagle
e-mail: feagle@bellsouth.net

May 5, 2005

Post-it® Fax Note 7671	Date 5-6-05	# of pages ▶ 6
To John Colson	From M. Feagle	
Co./Dept.	Co.	
Phone #	Phone #	
Fax # 755-6167	Fax #	

Mr. John Colson
Donald F. Lee & Associates, Inc.
140 NW Ridgewood Avenue
Lake City, Florida 32055

Re: Lot 18, Saddle Ridge Subdivision

Dear John:

I'm enclosing for your review a copy of Columbia County Ordinance No. 2003-23, together with a letter from Mark Disosway, P.E. dated December 4, 2004.

You will note in Section 1 of the Ordinance, Policy 4.2.40 requires new construction of any residential structure not located within a designated flood zone shall have the lowest finished floor, the bottom of the floor joist elevated no lower than one foot above adjacent paved or unpaved road subject to the following exemptions:

(1) Residential structures with certification by a Florida registered professional engineer as to the proper height or requirements for the protection of the structure against water damage.

Mike Roberts, who is apparently building a spec house on Lot 18, Saddle Ridge Subdivision, has requested a building permit with the finished floor elevation will, in fact, be lower than the adjacent paved road. Mr. Roberts has submitted a letter of opinion from engineer Mark Disosway which is enclosed. However, after reading Mr. Disosway's letter, it seems to raise more of an engineering question than a legal question as to whether the fact that the finished floor elevation of the residence below the adjacent roadway will cause the home to experience flooding problems in the future. I would appreciate your reviewing this information so you will be in a position to discuss it either with me or John Kerce at the Columbia County Building Department. One of us will give you a call within the next few days so the Building Department may make a final decision whether to issue the permit.

Thank you for your usual courtesies.

Very truly yours,

Marlin M. Feagle

MMF:dse
Enclosures
cc: Mr. John Kerce (w/o enclosures)

MARLIN - Not acceptable. Engineer required to establish minimum floor elevation. J. Colson

TOTAL P.02

DATE 02/22/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000022831

APPLICANT R. MACK ROBINSON PHONE 386.755.2492

ADDRESS 24262 US HWY 129 O'BRIEN FL 3071

OWNER MIKE ROBERTS PHONE 386.755.9476

ADDRESS 4231 SW CR 242 LAKE CIT FL 32024

CONTRACTOR R. MACK ROBINSON PHONE 386.744.2492

LOCATION OF PROPERTY 47-S TO C-242,TR GO W, 5 MILES JUST BEFORE TRAFFIC LIGHT ON SR 247 LOT IS ON THE RIGHT.

TYPE DEVELOPMENT SFD & UTILITY ESTIMATED COST OF CONSTRUCTION 74750.00

HEATED FLOOR AREA 1495.00 TOTAL AREA 2137.00 HEIGHT 18.20 STORIES 1

FOUNDATION CONC WALLS FRAMED ROOF PITCH 6'12 FLOOR CONC

LAND USE & ZONING RR MAX. HEIGHT 35

Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 21-4S-16-03084-018 SUBDIVISION SADDLE RIDGE

LOT 18 BLOCK PHASE UNIT 0 TOTAL ACRES 0.56

000000546		RB0054287		
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor	
18"X3'MITERED	05-0121-N	BLK		N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: 1 FOOT ABOVE ROAD

NOC ON FILE.

FLOOR HEIGHT LETTER ON FILE. Check # or Cash 1737

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power 03/30/2005 HD (date/app. by) Foundation (date/app. by) Monolithic (date/app. by)

Under slab rough-in plumbing 03/30/2005 HD (date/app. by) Slab 11/23/2005 HD (date/app. by) Sheathing/Nailing (date/app. by)

Framing (date/app. by) Rough-in plumbing above slab and below wood floor (date/app. by)

Electrical rough-in (date/app. by) Heat & Air Duct (date/app. by) Peri. beam (Lintel) (date/app. by)

Permanent power (date/app. by) C.O. Final (date/app. by) Culvert (date/app. by)

M/H tie downs, blocking, electricity and plumbing (date/app. by) Pool (date/app. by)

Reconnection (date/app. by) Pump pole (date/app. by) Utility Pole (date/app. by)

M/H Pole (date/app. by) Travel Trailer (date/app. by) Re-roof (date/app. by)

DATE 02/22/2005

Columbia County Building Permit

This Permit Expires One Year From the Date of Issue

PERMIT 000022831

APPLICANT R. MACK ROBINSON PHONE 386.755.2492

ADDRESS 24262 US HWY 129 O'BRIEN FL 3071

OWNER MIKE ROBERTS PHONE 386.755.9476

ADDRESS 4231 SW CR 242 LAKE CIT FL 32024

CONTRACTOR R. MACK ROBINSON PHONE 386.744.2492

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Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00

NO. EX.D.U. 0 FLOOD ZONE X DEVELOPMENT PERMIT NO.

PARCEL ID 21-4S-16-03084-018 SUBDIVISION SADDLE RIDGE

LOT 18 BLOCK PHASE UNIT 0 TOTAL ACRES 0.56

000000546		RB0054287		
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Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: 1 FOOT ABOVE ROAD
NOC ON FILE.
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(footer/Slab)

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Reconnection (date/app. by) Pump pole (date/app. by) Utility Pole (date/app. by)

M/H Pole (date/app. by) Travel Trailer (date/app. by) Re-roof (date/app. by)

Please provide the following.

1. Have your Structural Engineer revise and design your Building Plans for the new raised finished floor, in accord with recommendations from Cal-Tech Testing, Inc., (Elevation of the finished floor to be increased approximately 16 inches to an elevation of at least 107.0 feet), Provide 2 signed & sealed sets to the Building Department.

2. Get it ready for inspection.

3. Have your Surveyor do a certified finished floor elevation, (Provide the Building Department with a certified copy).

4. The Roof Trusses that have been in the weather at the job site, will not be allowed to be used, unless the Truss company that designed them gives me a engineered letter that they are structurally sound and in good condition.

Mike Roberts
755-9476

New Construction Subterranean Termite Soil Treatment Record

OMB Approval No. 2502-0525
(exp. 10/31/2005)

This form is completed by the licensed Pest Control Company.

Public reporting burden for this collection of information is estimated to average 15 minutes per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. This information is mandatory and is required to obtain benefits. HUD may not collect this information, and you are not required to complete this form, unless it displays a currently valid OMB control number.
Section 24 CFR 200.926d(b)(3) requires that the sites for HUD insured structures must be free of termite hazards. This information collection requires the builder to certify that an authorized Pest Control company performed all required treatment for termites, and that the builder guarantees the treated area against infestation for one year. Builders, pest control companies, mortgage lenders, homebuyers, and HUD as a record of treatment for specific homes will use the information collected. The information is not considered confidential.

This report is submitted for informational purposes to the builder on proposed (new) construction cases when soil treatment for prevention of subterranean termite infestation is specified by the builder, architect, or required by the lender, architect, FHA, or VA.

All contracts for services are between the Pest Control Operator and builder, unless stated otherwise. # 22831

Section 1: General Information (Treating Company Information)

Company Name: Aspen Pest Control, Inc.
Company Address: 301 NW Cole Terrace City Lake City State FL Zip 32055
Company Business License No. JB109476 Company Phone No. 386-755-3611
FHA/VA Case No. (if any)

Section 2: Builder Information

Company Name: Mike Roberts Company Phone No.

Section 3: Property Information

Location of Structure(s) Treated (Street Address or Legal Description, City, State and Zip) 4231 ZR 242 Lake City, FL

Type of Construction (More than one box may be checked) ☒ Slab ☐ Basement ☐ Crawl ☐ Other
Approximate Depth of Footing: Outside 0 Inside 0 Type of Fill Dirt

Section 4: Treatment Information

Date(s) of Treatment(s) 4-15-05
Brand Name of Product(s) Used Surrender
EPA Registration No. 70907-7-53663
Approximate Final Mix Solution % 0.5%
Approximate Size of Treatment Area: Sq. ft. 1905 Linear ft. 0 Linear ft. of Masonry Voids 0
Approximate Total Gallons of Solution Applied 190
Was treatment completed on exterior? ☐ Yes ☒ No
Service Agreement Available? ☒ Yes ☐ No

Note: Some state laws require service agreements to be issued. This form does not preempt state law.

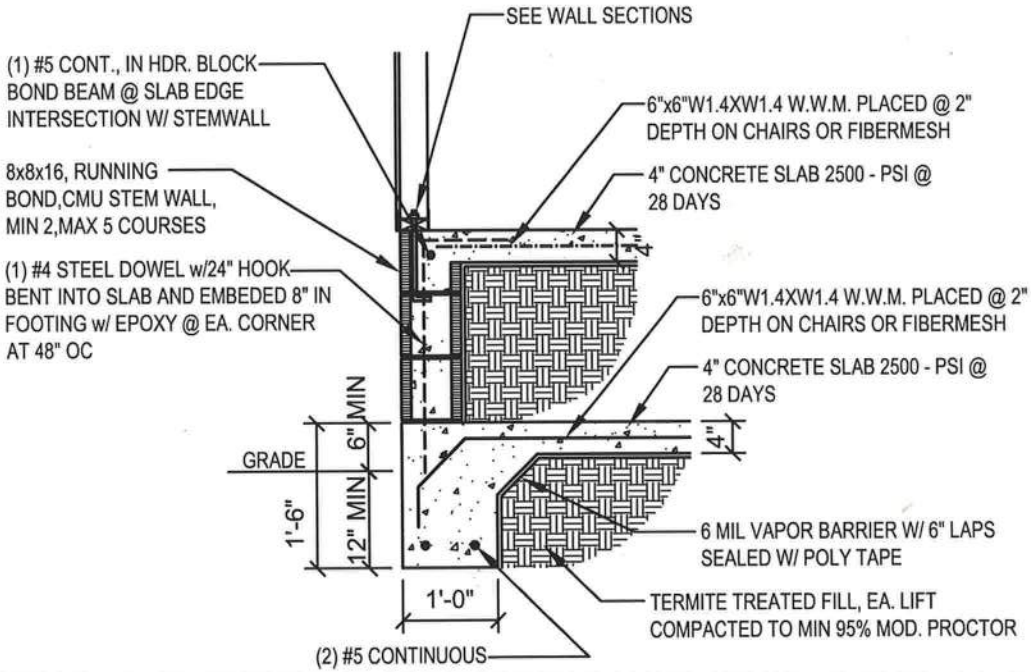
Attachments (List)
Comments

Name of Applicator(s) Steve Brannen Certification No. (if required by State law) JF104376

The applicator has used a product in accordance with the product label and state requirements. All treatment materials and methods used comply with state and federal regulations.

Authorized Signature [signature] Date 4-15-05

Warning: HUD will prosecute false claims and statements. Conviction may result in criminal and/or civil penalties. (18 U.S.C. 1001, 1010, 1012; 31 U.S.C. 3729, 3802)



F6 - MONOLITHIC / STEMWALL FOOTING

SCALE: 1/2"=1'-0"

MARK DISOSWAY P.E.
Ph 386-754-5419
POB868, Lake City, F 32056
PE No FL-53915 NC-26032

08sep05

BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

MEMORANDUM

Date: 10 August 2005

To: John Colson, P.E., County Engineer

From: Brian L. Kepner, County Planner BLK

Re: Saddle Ridge Lot 18

Please find attached a copy of the letter sent to the contractor concerning the problems with the floor elevation of the above referenced lot based on the recommendations you made to the Building Department. Also attached is a copy of the letter from the engineer (Cal-Tech Testing, Inc.) addressing those problems. Please review the letter and advise if it does address all the problems. Thank you.

xc: John D. Kerce, Building and Zoning Coordinator

Dale - I agree with John Dorman's analysis. Should the existing floor elevation not be raised the contractor/developer needs to advise the buyer/owner of this flood potential in writing.

J. Colson

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

JOB NO.: 05-200
DATE TESTED: 05/02/05
DATE REPORTED: 05/04/05

REPORT OF IN-PLACE DENSITY TEST

PROJECT:	Lot # 18, Spec House @ Saddleridge Subdivison
CLIENT:	Mack Robinson & Sons Construction, 24262 Us Hwy 129, O'Brien, FL 32071
GENERAL CONTRACTOR:	Mack Robinson & Sons Construction
EARTHWORK CONTRACTOR:	Mack Robinson & Sons Construction
INSPECTOR:	W. Hygema & T. Hygema

ASTM METHOD	**SOIL USE**
(D-2937) Drive Cylinder	BUILDING FILL

SPECIFICATION REQUIREMENTS: 95%

TEST NO.	TEST LOCATION	TEST DEPTH	WET DENSITY (lb/ft^3)	MOISTURE PERCENT	DRY DENSITY (lb/ft^3)	PROCTOR TEST NO.	PROCTOR VALUE	% MAXIMUM DENSITY
See Notes Below for Test 1 & 2								
1	North Side of Pad	0-12"	115.2	8.3	106.4	Pit	111.0	95.8%
2	West Side of Pad	0-12"	112.7	6.8	105.5	Pit	111.0	95.1%
3	15' North of East Side of Pad	0-12"	118.4	10.7	107.0	Pit	111.0	96.4%
4	Approx. Center of Pad	0-12"	120.0	11.3	107.8	Pit	111.0	97.1%

REMARKS: The Above Tests Meet Specification Requirements.

Notes: Test 1 & 2 where taken with a Nuc Gauge outside of slab.

PROCTORS

PROCTOR NO.	SOIL DESCRIPTION	MAXIMUM DRY UNIT WEIGHT (lb/ft^3)	OPT. MOIST.	TYPE
Pit	Light Brown Fine Sand (Dan Register Pit)	111.0	11.5	MODIFIED (ASTM D-1557)

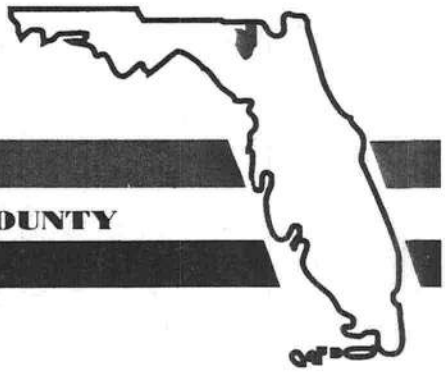
Respectfully Submitted,
CAL-TECH TESTING, INC.

Linda Creamer

Linda M. Creamer
President - CEO
ta

Reviewed By:

Date: 5/5/05
Florida Registration No: 52612



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

MEMORANDUM

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To: John Colson, P.E., County Engineer

From: Brian L. Kepner, County Planner BLK

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xc: John D. Kerce, Building and Zoning Coordinator

BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

May 12, 2005

Mack Robinson
24262 US Hwy 129
O`brien Fl. 32071

RE: Mike Roberts
Saddle Ridge Lot 18

Dear Mack:

As we have discussed the possible problems with the floor elevation of the home being constructed on Saddle Ridge Lot 18, I have had the County Engineer look at the home site.

After receiving his recommendations, the matter was discussed with the County Attorney. We were instructed to inform you that the following items must be addressed before any construction should continue on the home:

1. Have your engineer to establish an elevation as to the proper height of the finished floor for the protection of the home against water damage
2. Have a surveyor to certify the finished floor elevation height
3. This floor elevation must be at least as high as the engineer established
4. Have your engineer address any possible water runoff and its effect on the adjacent neighbors

Please contact me at (386) 758-1008 if you should have any questions.

Sincerely,

John D. Kerce
Building & Zoning Coor.
Columbia County

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

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June 16, 2005

Mike Roberts
657 S. W. Katherine Lane
Lake City, Florida 32025

Reference: Proposed Residence
Saddle Ridge Subdivision, Lot 18
C. R. 242
Columbia County, Florida
Cal-Tech Project No. 05-290

Dear Mr. Roberts,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lot 18 of Saddle Ridge Subdivision in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of a home to be constructed at the site and to provide recommendations as appropriate. Stem walls and floor slab for the residence were in place at the time of our investigation.

With reference to U. S. Geological Survey Benchmark "41 FPV" located near the site, the finished floor of the residence has elevation 105.6 feet, approximately. This elevation is approximately 2.4 feet below the centerline of the adjacent roadway, C. R. 242. Note that benchmark "41 FPV" has been disturbed; however, its elevation is believed to be in error by no more than 0.5 feet.

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown that such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the flood insurance rate map for Columbia County, Florida, two small zone "A" flood zones are located relatively near the home site; however, the flood elevation for these two flood zones is estimated to be approximately 100 feet, well below the finished floor elevation of the proposed residence, approximately 105.6 feet. Additionally, these two flood zones are topographically isolated from the home site, and flooding within these flood zones would not affect the site, lot 18 of Saddle Ridge.

The home site is located within a topographically lower area that with sufficient rainfall could have local flooding. However, this topographically lower area is not identified on the flood insurance rate map as a flood zone. Therefore, we believe flooding within this area to the finished floor elevation of approximately 105.6 feet is unlikely.

Based upon elevations determined within the topographically lower area that includes the home site, the maximum flood elevation that could occur is about 106.5 feet, or approximately 0.9 feet above the finished floor elevation. Floodwater higher than this elevation would leave the area through overland flow to topographically lower areas to the northeast and southeast. This estimated maximum flood elevation of approximately 106.5 feet assumes that a dam or other obstruction is not placed on adjacent properties that would alter the natural flow of floodwater.

It is our opinion that flooding to the existing finished flood elevation of approximately 105.6 feet is highly unlikely; however, to remove any possibility of flooding, we recommend the elevation of the finished floor be increased approximately 16 inches to an elevation of approximately 107.0 feet.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer 8/8/05
52612

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

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June 16, 2005

Mike Roberts
657 S. W. Katherine Lane
Lake City, Florida 32025

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Columbia County, Florida
Cal-Tech Project No. 05-290

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Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown that such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the flood insurance rate map for Columbia County, Florida, two small zone "A" flood zones are located relatively near the home site; however, the flood elevation for these two flood zones is estimated to be approximately 100 feet, well below the finished floor elevation of the proposed residence, approximately 105.6 feet. Additionally, these two flood zones are topographically isolated from the home site, and flooding within these flood zones would not affect the site, lot 18 of Saddle Ridge.

The home site is located within a topographically lower area that with sufficient rainfall could have local flooding. However, this topographically lower area is not identified on the flood insurance rate map as a flood zone. Therefore, we believe flooding within this area to the finished floor elevation of approximately 105.6 feet is unlikely.

Based upon elevations determined within the topographically lower area that includes the home site, the maximum flood elevation that could occur is about 106.5 feet, or approximately 0.9 feet above the finished floor elevation. Floodwater higher than this elevation would leave the area through overland flow to topographically lower areas to the northeast and southeast. This estimated maximum flood elevation of approximately 106.5 feet assumes that a dam or other obstruction is not placed on adjacent properties that would alter the natural flow of floodwater.

It is our opinion that flooding to the existing finished flood elevation of approximately 105.6 feet is highly unlikely; however, to remove any possibility of flooding, we recommend the elevation of the finished floor be increased approximately 16 inches to an elevation of approximately 107.0 feet.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer 8/8/05
5261Z

22831 755-7416 - Mike Roberts

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

June 16, 2005
Revised: August 19, 2005

Mike Roberts
657 S. W. Katherine Lane
Lake City, Florida 32025

Reference: Proposed Residence
Saddle Ridge Subdivision, Lot 18, C. R. 242
Columbia County, Florida
Cal-Tech Project No. 05-290

Dear Mr. Roberts,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lot 18 of Saddle Ridge Subdivision in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of a home to be constructed at the site and to provide recommendations as appropriate. The monolithic foundation for the residence was in place at the time of our investigation.

With reference to U. S. Geological Survey Benchmark "41 FPV" located near the site, the finished floor of the residence has an elevation of approximately 105.6 feet. This elevation is about 2.4 feet below the centerline of the adjacent roadway, C. R. 242. Note that benchmark "41 FPV" has been disturbed; therefore, its elevation is believed to be in error by as much as 0.5 feet. However, the recommendations provided within this report are based upon relative site topography and within reason are independent of whether or not there is some small difference between the actual and published elevations of benchmark "41 FPV". This benchmark has been used since its actual elevation is believed to be reasonably close to the published elevation of 100.00 feet.

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown that such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the flood insurance rate map for Columbia County, Florida, two small zone "A" flood zones are located relatively near the home site; however, the flood elevation for these two flood zones is estimated to be approximately 100 feet, well below the finished floor elevation of the proposed residence, approximately 105.6 feet. Additionally, these two flood zones are topographically isolated from the home site, and flooding within these flood zones would not affect lot 18 of Saddle Ridge.

Lot 18 of Saddle Ridge is however located within a topographically lower area that with sufficient rainfall could have local flooding. This topographically lower area is

not identified on the flood insurance rate map as a flood zone; therefore, we believe flooding within this area and especially to the finished floor elevation of approximately 105.6 feet is highly unlikely.

Based upon elevations determined within the topographically lower area that includes the home site, the maximum flood elevation that could occur is about 106.5 feet, or approximately 0.9 feet above the finished floor elevation. Floodwater higher than this elevation would leave the area through overland flow to topographically lower areas to the northeast and southeast. This estimated maximum flood elevation of approximately 106.5 feet assumes that a dam or other obstruction is not placed on adjacent properties that would alter the natural flow of floodwater.

It is our opinion that flooding to the existing finished flood elevation of approximately 105.6 feet is highly unlikely; however, to essentially remove any possibility of flooding, we recommend the elevation of the finished floor be increased approximately 16 inches to an elevation of approximately 107.0 feet. To assist in establishing this elevation on site, you may use a temporary benchmark set by others. This temporary benchmark consists of a large nail set in the base of a power pole located at the southeast corner of the site. Relative to other elevations used for this evaluation, the elevation of the nail has been determined to be 106.85 feet. Therefore, we recommend the finished floor of the residence be set approximately 2 inches above the nail used as a temporary benchmark.

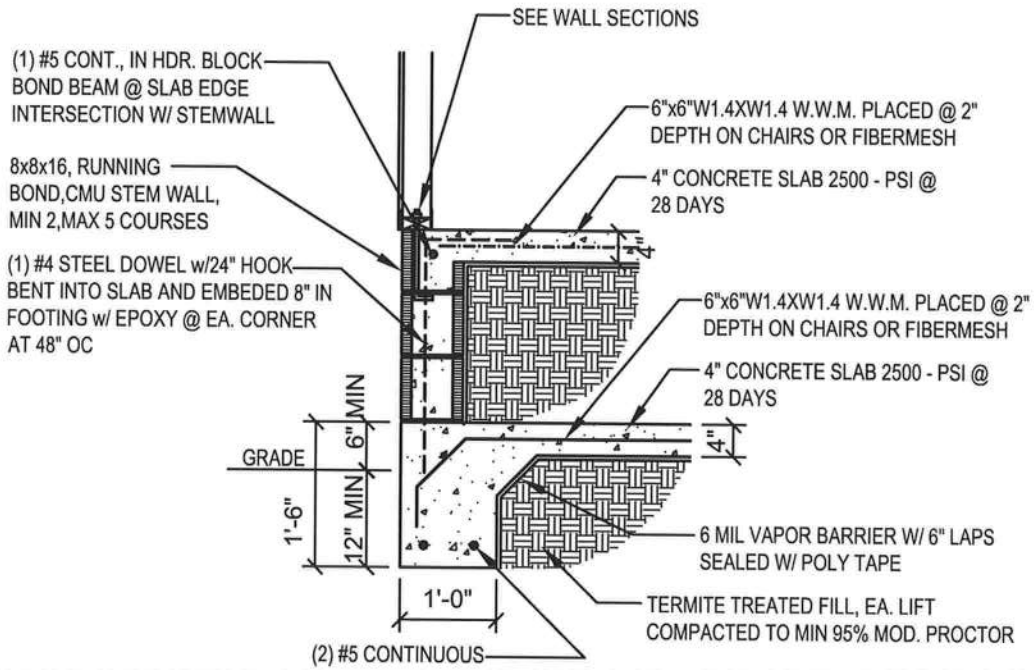
The site is located at a higher elevation within the topographically lower area; therefore, changes in storm water runoff due to the presence of the residence will generally be limited to an increase in runoff that is approximately equal to the decrease in surface infiltration below the residence. Due to the relatively small size of the residence this decrease is surface infiltration and the resulting increase in runoff will be very small and essentially negligible. In the event that flooding does occur and floor water rises to the level of the residence, the residence would then occupy volume that otherwise would be occupied by floodwater. Based upon the size of the residence and the size of the basin, floodwater elevations within the basin would increase by a factor of about 1/500th of the increase in flood depth. In other words, the presence of the residence would increase floodwater depth by less than the thickness of a dime for every 12-inch increase in floodwater depth. This increase is also negligible.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer 8/19/05
52612



F6 - MONOLITHIC / STEMWALL FOOTING

SCALE: 1/2"=1'-0"

MARK DISOSWAY P.E.
Ph 386-754-5419
POB868, Lake City, FL 32056
PE No FL-53915 NC-26032

08SEP05

William A. Baker P.E.
Florida License No. 30565
109 SE Jenese Way, Lake City, Florida 32025-1837
Telephone No. / Fax (386) 752-8523

Inspection Of Wood In Trusses Resulting From Outdoor Storage
(Owner - Mike Roberts' - Spec House)

A Review of the state of the wood in trusses that have been stored on the ground and exposed to the weather for close to a year.

The owner, Mike Roberts, took delivery of trusses, for a house he was about to build, in early 2005. The trusses were laid out in the yard with one of the larger trusses directly on the ground and the rest stacked on top of it, or otherwise off the ground. Now, one year later, the roof is about to be constructed and the County Building Inspection Department is questioning the state of the wood in the trusses after that length of exposure.

On Saturday, January 14, with Mike Roberts and a helper, we examined each truss by turning them over in order to get a close look at both sides. Mr. Roberts had previously washed the trusses with Clorox to get rid of any mildew that had started to form. The wood generally looked quite good with a slight graying on some sides due to weathering. The bottom side of the truss, #**TO9**, that lay directly on the ground, showed obvious deterioration. The next truss, #**TO10**, that lay on top of truss, #**TO9**, showed evidence of termite intrusion in the center of the bottom chord and in the top chord close to the peak. These two trusses are to be replaced. There was no evidence of deterioration of the wood in the remaining trusses and truss parts.

There have been two minor changes in the roof design since the trusses were first delivered. Two hip roofs have been changed to gable ends. Also Mr. Roberts does not expect to use the valley trusses supplied with the original design. It is requested that the truss fabricator supply an up-to-date set of certified truss drawings with plan.

It is recommended that the designer/fabricator review the trusses carefully as there are splice plates that need to be reattached, or re-secured, and there is at least one strut that needs to be spliced.

Signed - W. A. Baker Jan. 18, 2006

William A. Baker

Cc: Mike Roberts, 657 SW Catherine Lane, Lake City, Florida 32025
Columbia County Building Dept., 135 NE Hernando Ave. B-21, Lake City 32055
Mr. Chris Riser, Builders First Source, 6550 Roosevelt Blvd., Jacksonville, Fl 32244
Thomas E. Miller PE, 16105 North Florida Ave., Suite B, Lutz, Fl 33549
Mark Disosway P.E., POB 868, Lake City, Florida 32056

Project Information for: L101301

Builder:	Mike Roberts (owner builder)	Date:	2/2/2005
Lot:	N/A	Start Number:	001
Subdivision:	4231 SW County Rd 242		
County or City:	Columbia County		
Truss Page Count:	40		

Truss Design Load Information (UNO) Design Program: MiTek 5.2

Gravity		Wind		Building Code:	FBC2001
Roof (psf):	42	Wind Standard:	ASE 7-98		
Floor (psf):	55	Wind Speed (mph):	110		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

Owner Builder
Address: N/A
N/A
Designer: 39

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549

Notes:
1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI 1-1995 Section 2.2
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	020205001	2/2/2005				
2	CJ3	020205002	2/2/2005				
3	CJ3A	020205003	2/2/2005				
4	CJ5	020205004	2/2/2005				
5	EJ3	020205005	2/2/2005				
6	EJ5	020205006	2/2/2005				
7	EJ7	020205007	2/2/2005				
8	EJ7A	020205008	2/2/2005				
9	HJ3	020205009	2/2/2005				
10	HJ5	020205010	2/2/2005				
11	HJ7	020205011	2/2/2005				
12	T01	020205012	2/2/2005				
13	T02	020205013	2/2/2005				
14	T03	020205014	2/2/2005				
15	T04	020205015	2/2/2005				
16	T05	020205016	2/2/2005				
17	T06	020205017	2/2/2005				
18	T06TRY	020205018	2/2/2005				
19	T07	020205019	2/2/2005				
20	T07TRY	020205020	2/2/2005				
21	T08	020205021	2/2/2005				
22	T09	020205022	2/2/2005				
23	T10	020205023	2/2/2005				
24	T11	020205024	2/2/2005				
25	T12	020205025	2/2/2005				
26	T13	020205026	2/2/2005				
27	T14	020205027	2/2/2005				
28	T15	020205028	2/2/2005				
29	T16	020205029	2/2/2005				
30	T17	020205030	2/2/2005				
31	T18	020205031	2/2/2005				
32	T19	020205032	2/2/2005				
33	T20	020205033	2/2/2005				
34	V01	020205034	2/2/2005				
35	V02	020205035	2/2/2005				
36	V03	020205036	2/2/2005				
37	V04	020205037	2/2/2005				
38	V05	020205038	2/2/2005				
39	V06	020205039	2/2/2005				
40	V07	020205040	2/2/2005				

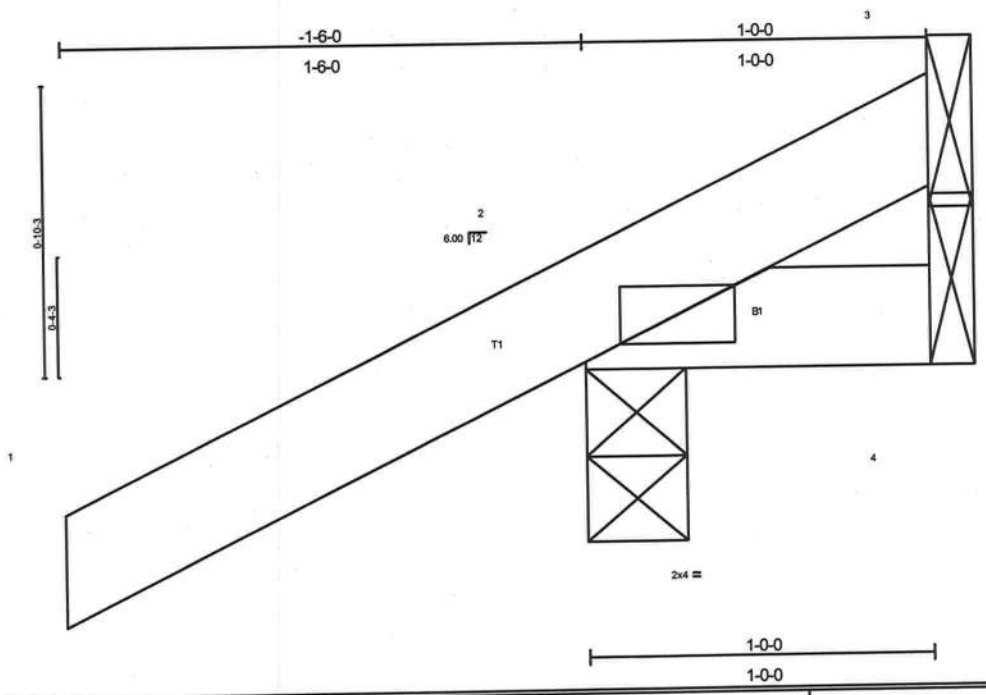
FEB 0 2 2005

Thomas E. Miller, FL PE 56877, EB #9196

Dwg.#020205001

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	CJ1	MONO TRUSS	16	1	Job Reference (optional)

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:43 2005 Page 1



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TCLL 20.0	Plates Increase 1.25	TC 0.14	Vert(LL) -0.00 2 >999 240	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL) 0.02 1 >802 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 3 n/a n/a	Weight: 6 lb
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=-40/Mechanical, 2=189/0-3-8, 4=14/Mechanical
Max Horz 2=70(load case 5)
Max Uplift 3=-40(load case 1), 2=-192(load case 5), 4=-9(load case 3)
Max Grav 3=61(load case 5), 2=189(load case 1), 4=14(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-45/29
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3, 192 lb uplift at joint 2 and 9 lb uplift at joint 4.

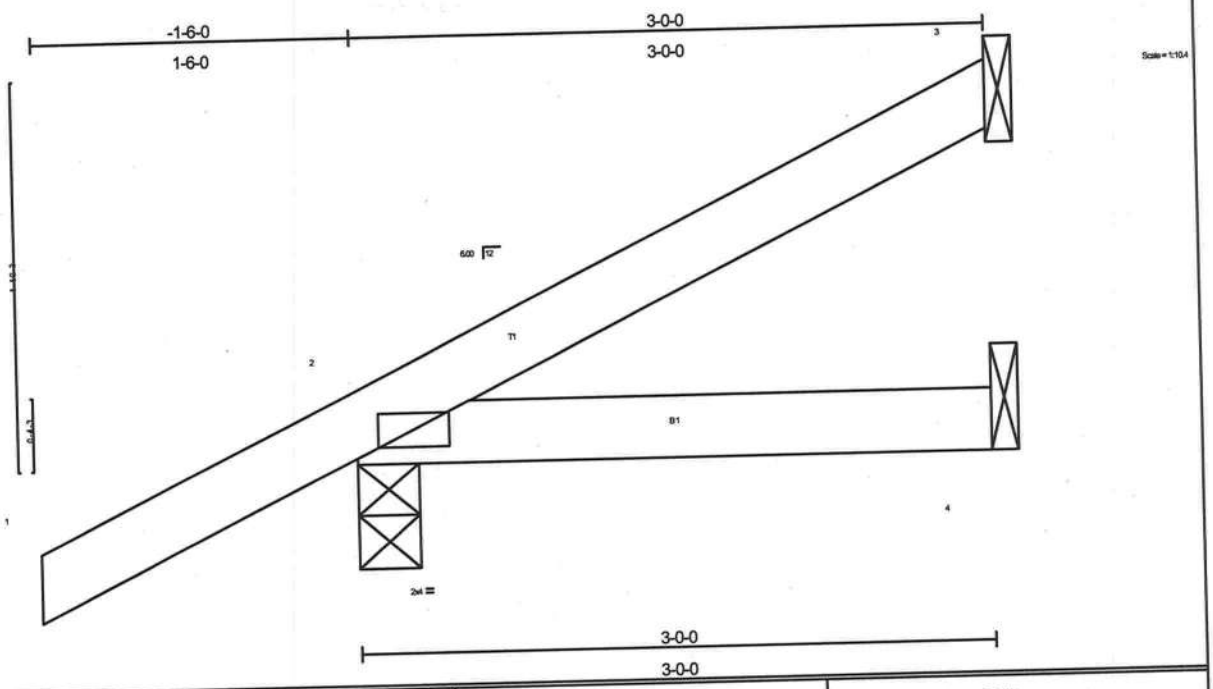
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

~~Dwg.#020205002~~

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	CJ3	MONO TRUSS	11	1	Job Reference (optional)

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:44 2005 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.15	Vert(LL)	0.01	2-4	>999	240	MII20	249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.07	Vert(TL)	0.03	1	>734	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a	Weight: 12 lb	
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)							

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-38(load case 5), 2=-186(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-57/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3, 186 lb uplift at joint 2 and 27 lb uplift at joint 4.

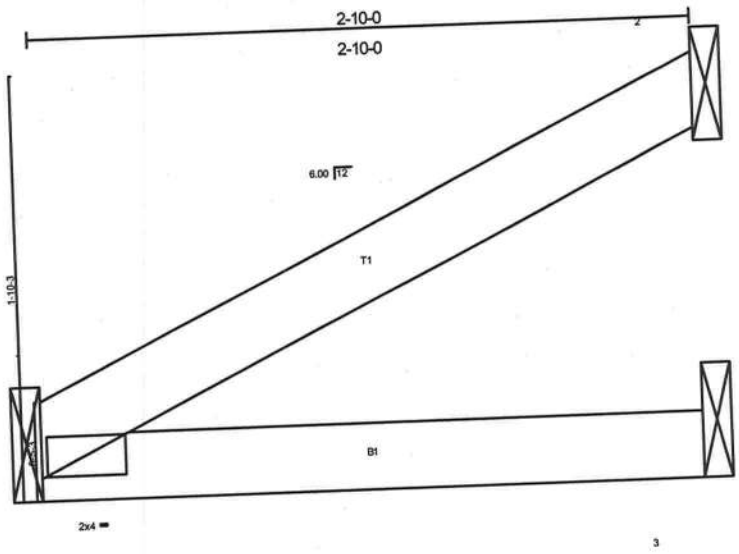
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

~~Dwg.#020205003~~

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	CJ3A	MONO TRUSS	1	1	Job Reference (optional)

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:44 2005 Page 1



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TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.00	1-3	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 9 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 2-10-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=114/Mechanical, 2=73/Mechanical, 3=41/Mechanical
Max Horz 1=69(load case 5)
Max Uplift1=-24(load case 5), 2=-70(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-77/28
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 1 and 70 lb uplift at joint 2.

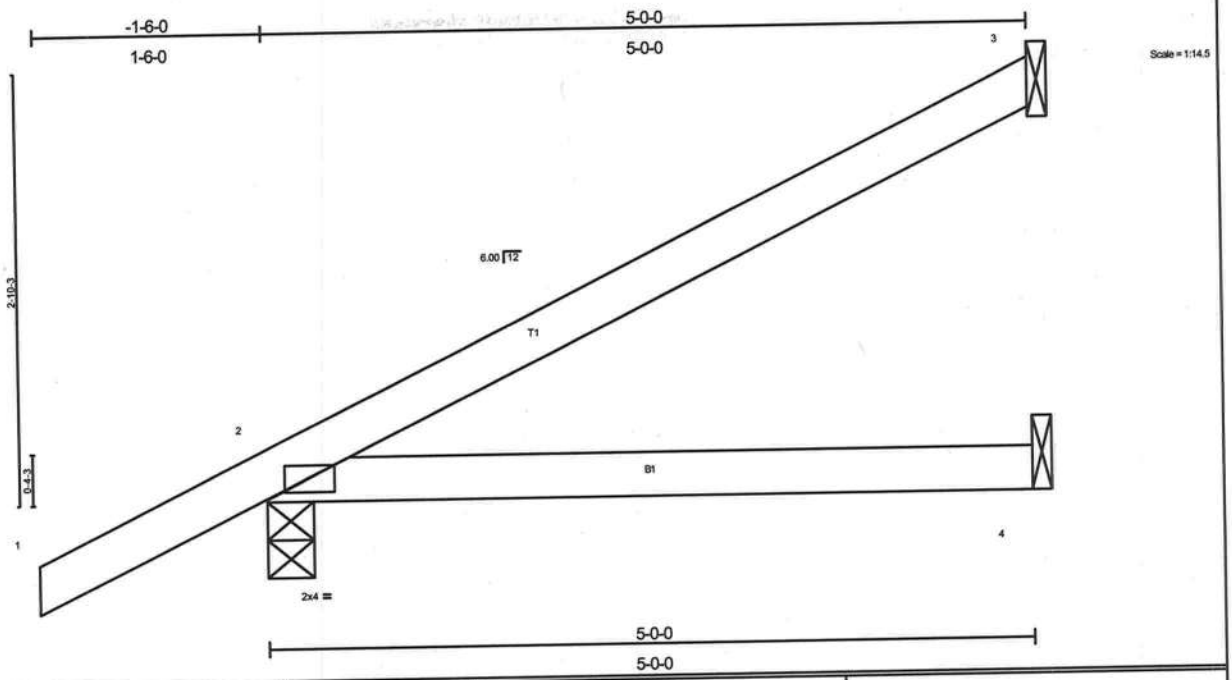
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#020205004

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	CJ5	MONO TRUSS	8	1	Job Reference (optional)

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:44 2005 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.29	Vert(LL) 0.08 2-4 >704 240	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.20	Vert(TL) -0.08 1 >260 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a	Weight: 18 lb
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift 3=-101(load case 5), 2=-218(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-117/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3, 218 lb uplift at joint 2 and 46 lb uplift at joint 4.

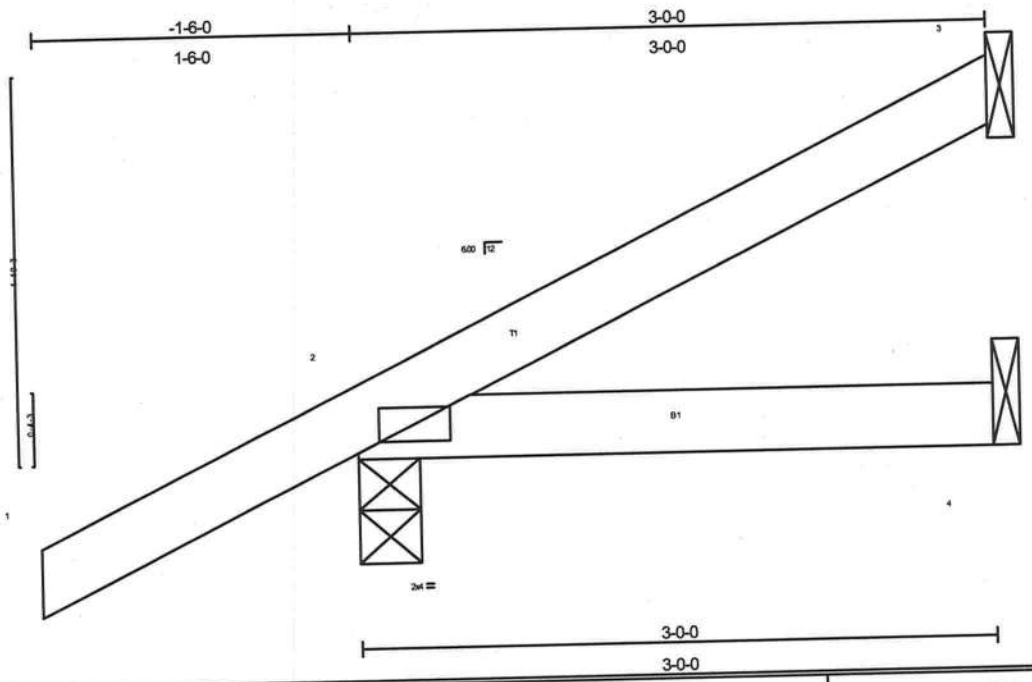
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#020205005

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	EJ3	MONO TRUSS	8	1	Job Reference (optional)

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:45 2005 Page 1



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TCLL 20.0	Plates Increase 1.25	TC 0.15	Vert(LL) -0.00 2-4 >999 240	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.05	Vert(TL) 0.03 1 >732 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a	Weight: 12 lb
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=49/Mechanical, 2=232/0-3-8, 4=42/Mechanical
Max Horz 2=115(load case 5)
Max Uplift3=-38(load case 5), 2=-151(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-57/16
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3 and 151 lb uplift at joint 2.

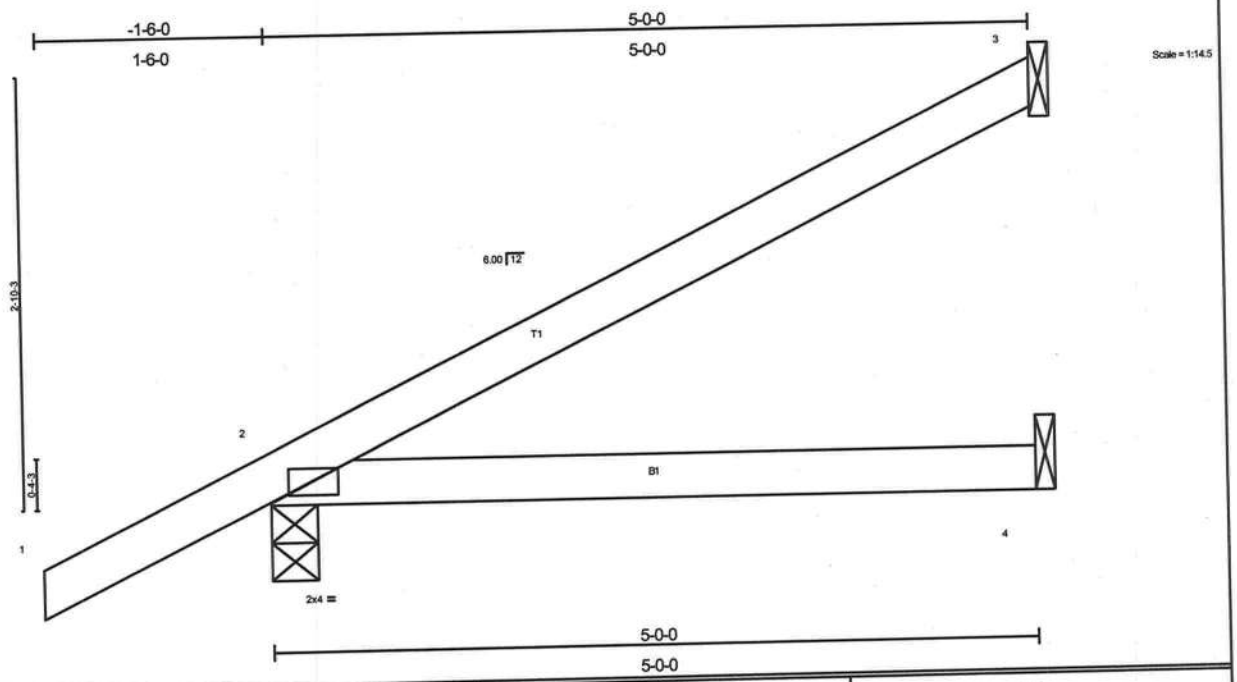
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	EJ5	MONO TRUSS	3	1	Job Reference (optional)

Dwg.#020205006

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:45 2005 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.29	Vert(LL)	-0.03	2-4	>999	240	MII20	249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.14	Vert(TL)	-0.08	1	>260	180		
BCLL 10.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)						Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=114/Mechanical, 2=305/0-3-8, 4=72/Mechanical
Max Horz 2=162(load case 5)
Max Uplift3=-101(load case 5), 2=-157(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-117/41
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 157 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	EJ7	MONO TRUSS	29	1	Job Reference (optional)

Dwg.#020205007

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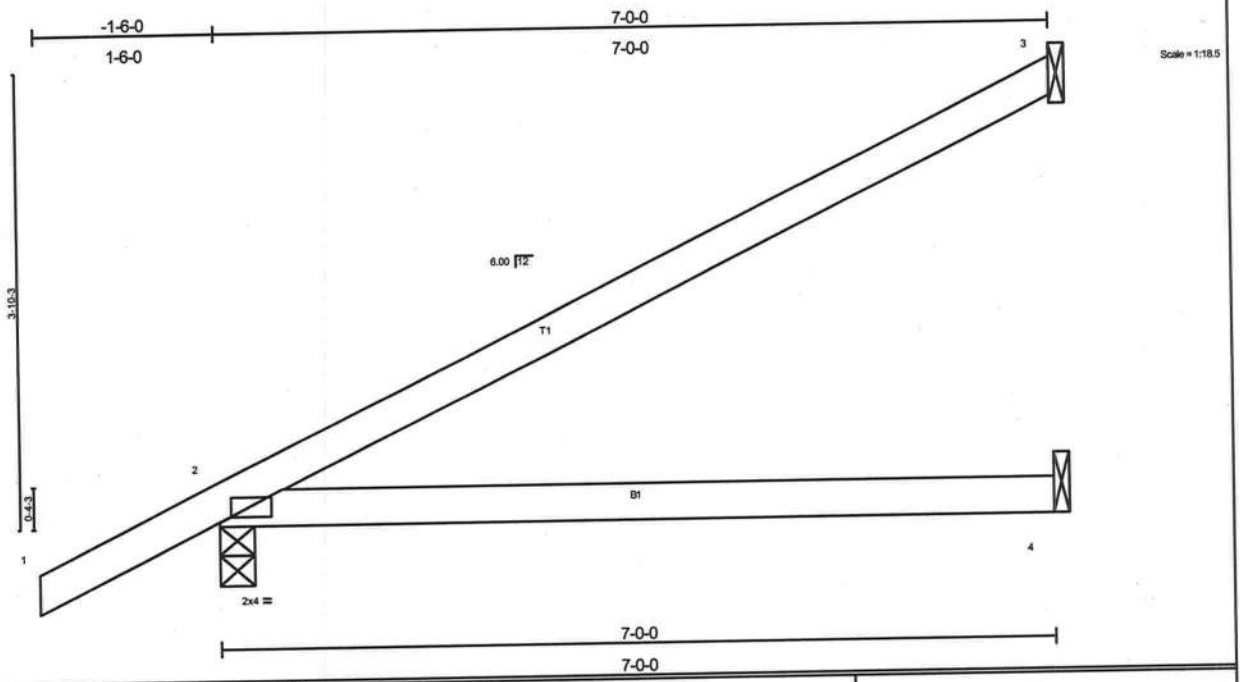

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	0.33	2-4	>250	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.40	Vert(TL)	0.18	1	>110	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=173/Mechanical, 2=385/0-3-8, 4=102/Mechanical
Max Horz 2=149(load case 5)
Max Uplift 3=-99(load case 5), 2=-196(load case 5), 4=-65(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-89/62
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS interior zone and C-C Interior(1) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 3, 196 lb uplift at joint 2 and 65 lb uplift at joint 4.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	EJ7A	MONO HIP	1	1	Job Reference (optional)

Dwg.#020205008

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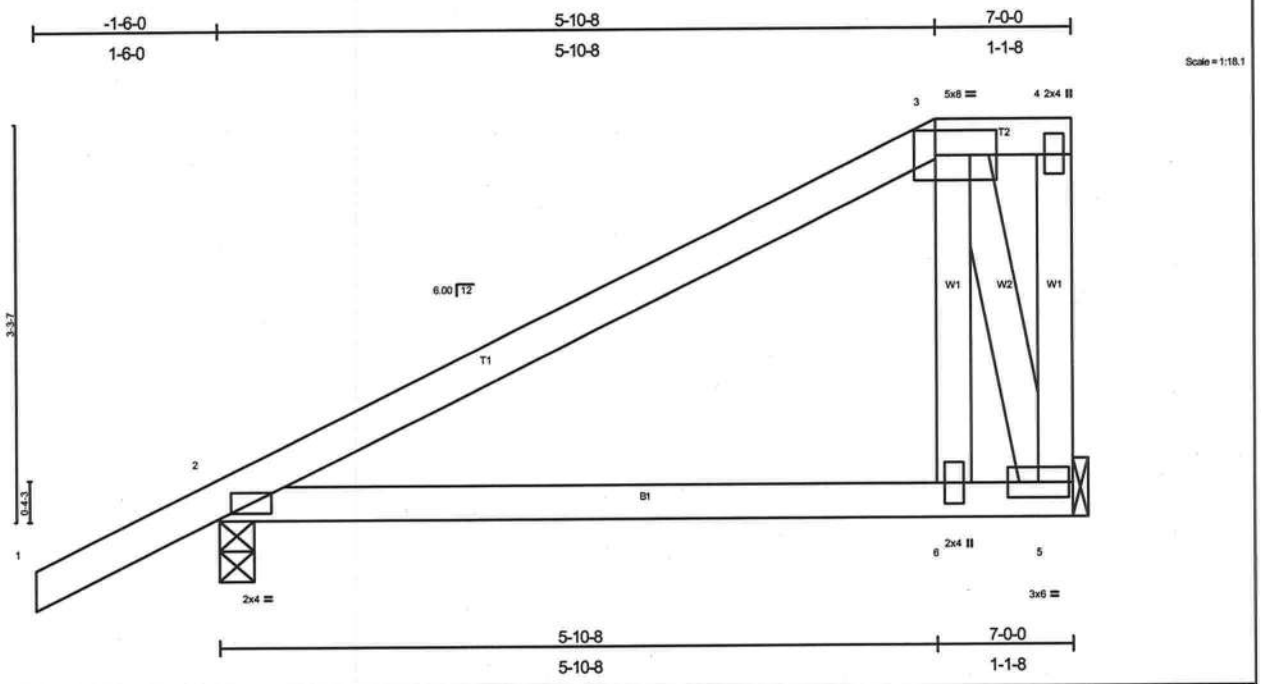

Plate Offsets (X,Y): [3:0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.03	2-6	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.16	1	>127	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=271/Mechanical, 2=382/0-3-8
Max Horz 2=184(load case 5)
Max Uplift 5=-105(load case 5), 2=-180(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-195/0, 3-4=-0/0, 4-5=-26/29
BOT CHORD 2-6=-63/105, 5-6=-60/116
WEBS 3-6=0/241, 3-5=-373/193

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 5 and 180 lb uplift at joint 2.

LOAD CASE(S) Standard

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	HJ3	MONO TRUSS	2	1	Job Reference (optional)

Dwg.#020205009

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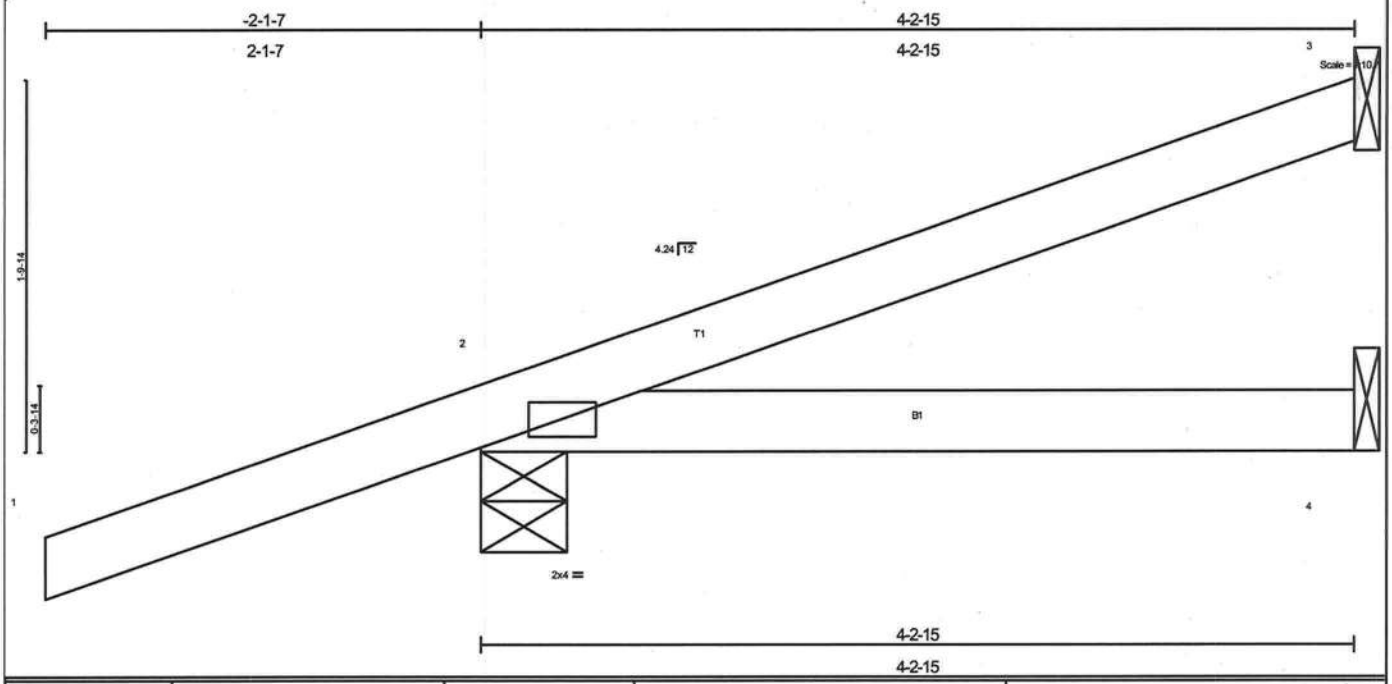

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.01	2-4	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	0.13	1	>221	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 16 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=41/Mechanical, 2=225/0-4-15, 4=42/Mechanical
Max Horz 2=82(load case 2)
Max Uplift 3=-18(load case 2), 2=-182(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-28/7
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 3 and 182 lb uplift at joint 2.
3) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-57(F=-2, B=-2), 2=-0(F=15, B=15)-to-4=-32(F=-1, B=-1)

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	HJ5	MONO TRUSS	2	1	Job Reference (optional)

Dwg.#020205010

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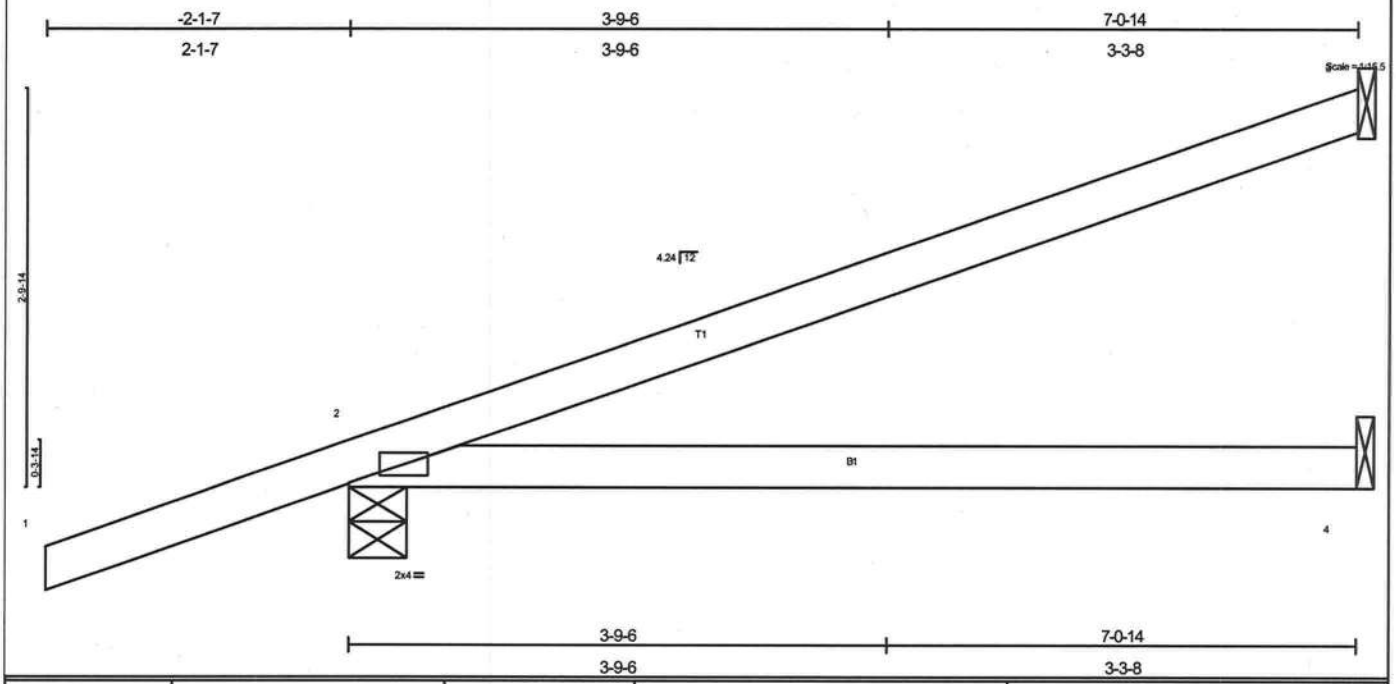

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.43	Vert(LL) -0.09 2-4 >871 240	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.29	Vert(TL) -0.06 1 >499 180	
BCLL 10.0	Rep Stress Incr NO	WB 0.00	Horz(TL) -0.00 3 n/a n/a	
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		Weight: 25 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=198/Mechanical, 2=322/0-4-15, 4=120/Mechanical
Max Horz 2=151(load case 2)
Max Uplift 3=-158(load case 2), 2=-190(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-73/47
BOT CHORD 2-4=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 3 and 190 lb uplift at joint 2.
3) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-3=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-4=-53(F=-12, B=-12)

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205011
L101301	HJ7	MONO TRUSS	4	1	Job Reference (optional)	

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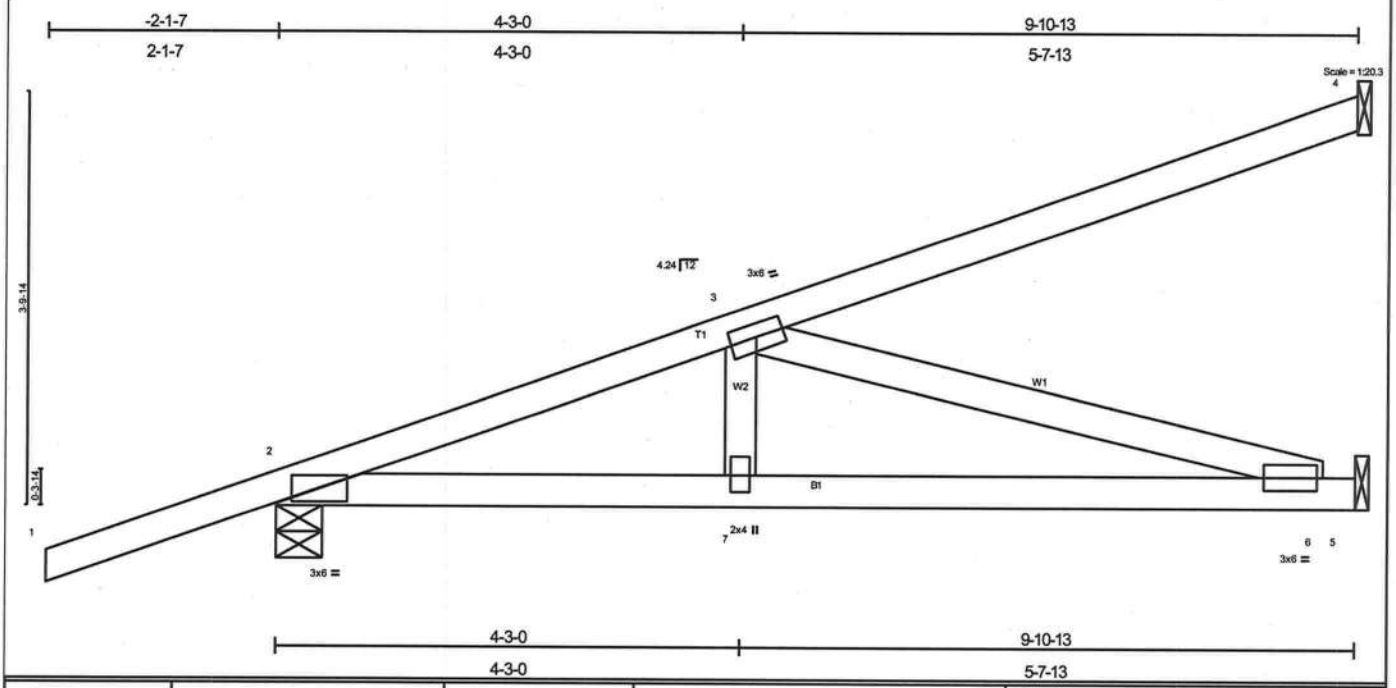

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.53	Vert(LL) -0.09 6-7 >999 240	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.52	Vert(TL) 0.10 1 >286 180	
BCLL 10.0	Rep Stress Incr NO	WB 0.52	Horz(TL) 0.01 5 n/a n/a	Weight: 44 lb
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=484/0-4-15, 5=389/Mechanical
Max Horz 2=253(load case 2)
Max Uplift 4=-230(load case 2), 2=-226(load case 2), 5=-77(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=-937/179, 3-4=-104/65
BOT CHORD 2-7=-370/874, 6-7=-370/874, 5-6=0/0
WEBS 3-7=0/198, 3-6=-909/384

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 230 lb uplift at joint 4, 226 lb uplift at joint 2 and 77 lb uplift at joint 5.
3) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=26, B=26)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T01	HIP	1	1	Job Reference (optional)

Dwg.#020205012

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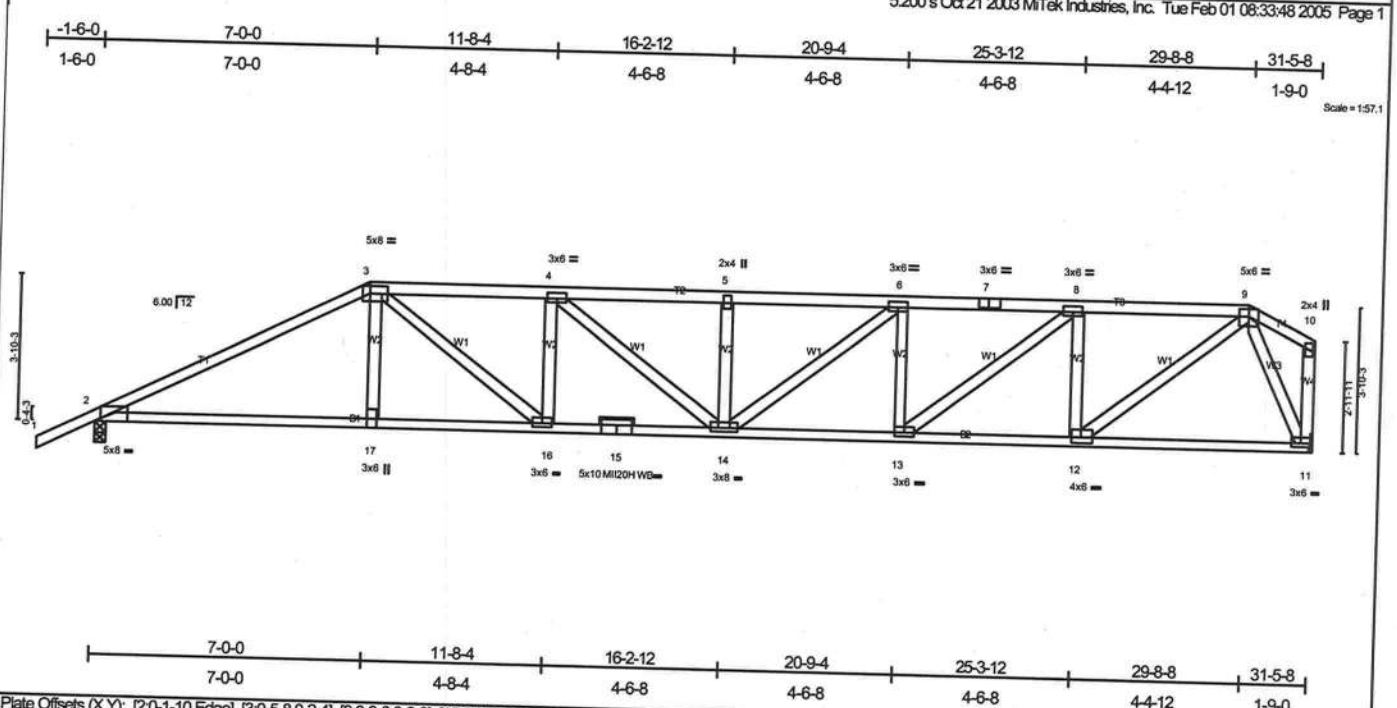

Plate Offsets (X,Y): [2:0-1-10,Edge], [3:0-5-8,0-2-4], [9:0-3-0,0-2-0], [12:0-2-12,0-2-0], [15:0-5-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.46	14-16	>810	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-0.65	14-16	>578	180	MII20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	0.81	Horz(TL)	0.17	11	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 171 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D
W1 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 2-2-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-1-3 oc bracing.

REACTIONS (lb/size) 2=2752/0-3-8, 11=2893/Mechanical
Max Horz 2=133(load case 4)
Max Uplift2=-1106(load case 3), 11=-1253(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-5322/2288, 3-4=-6130/2772, 4-5=-6488/2937, 5-6=-6488/2937, 6-7=-5784/2637, 7-8=-5784/2637, 8-9=-4049/1849, 9-10=-145/36, 10-11=-125/51
BOT CHORD 2-17=-2008/4659, 16-17=-2017/4693, 15-16=-2705/6130, 14-15=-2705/6130, 13-14=-2570/5784, 12-13=-1782/4048, 11-12=-576/1260
WEBS 3-17=-235/820, 9-11=-2818/1396, 9-12=-1551/3588, 3-16=-923/1909, 8-12=-1896/1054, 4-16=-858/608, 8-13=-1001/2206, 4-14=-227/486, 6-13=-1085/670, 5-14=-533/428, 6-14=-381/898

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MII20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1106 lb uplift at joint 2 and 1253 lb uplift at joint 11.
6) Girder carries hip end with 7-0-0 end setback
7) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539.0lb down and 277.5lb up at 7-0-0 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-9=-118(F=-64), 9-10=-118(F=-64), 2-17=-30, 11-17=-65(F=-35)
Concentrated Loads (lb)
Vert: 17=-539(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205013
L101301	T02	HIP	1	1	Job Reference (optional)	

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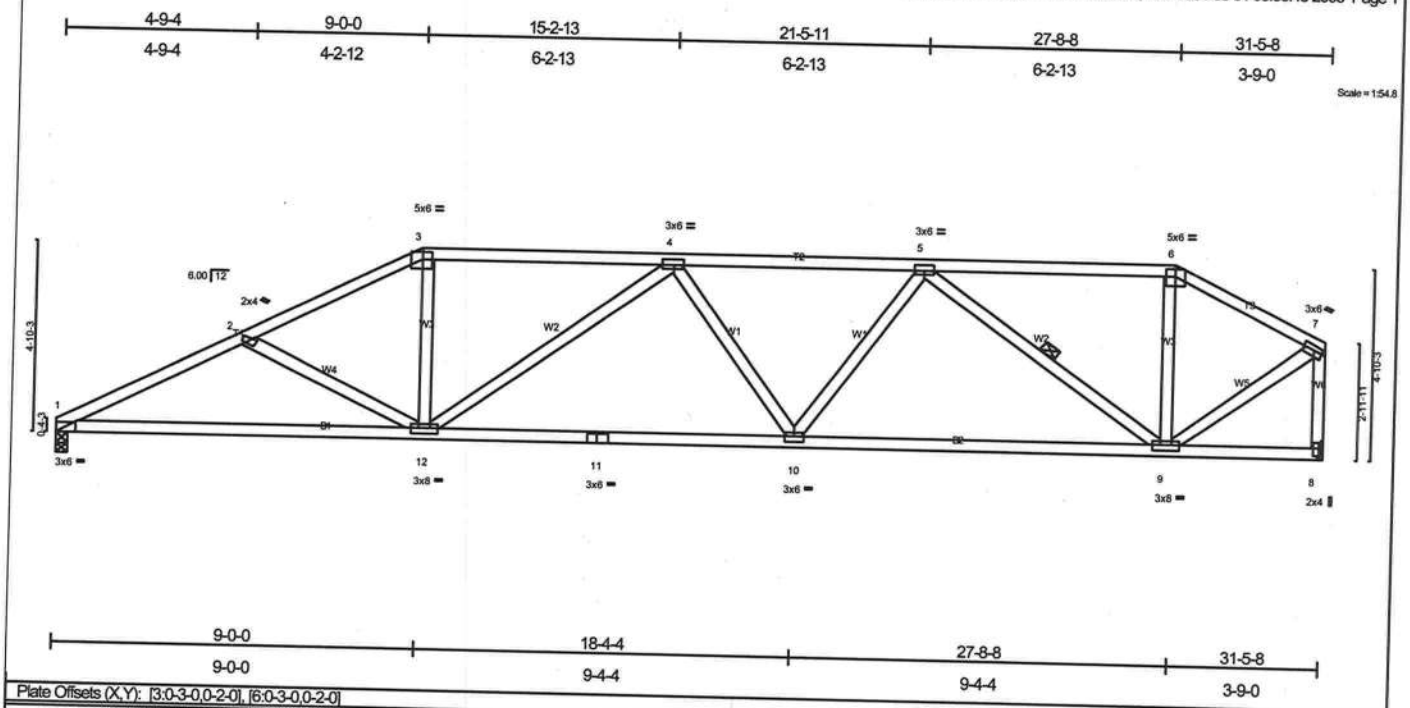

Plate Offsets (X,Y): [3:0-3-0,0-2-0], [6:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.21	9-10	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.30	9-10	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.08	8	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 164 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-11-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-9

REACTIONS (lb/size) 1=1309/0-3-8, 8=1309/Mechanical
Max Horz 1=162(load case 5)
Max Uplift1=-345(load case 4), 8=-391(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2409/1115, 2-3=-2169/962, 3-4=-1916/925, 4-5=-2202/978, 5-6=-1010/500, 6-7=-1151/497, 7-8=-1294/567
BOT CHORD 1-12=-1073/2109, 11-12=-963/2259, 10-11=-963/2259, 9-10=-821/1952, 8-9=-19/8
WEBS 2-12=-242/293, 3-12=-193/658, 4-12=-523/283, 4-10=-110/161, 5-10=-91/450, 5-9=-1198/533, 6-9=-15/254, 7-9=-465/1232

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 1 and 391 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205014
L101301	T03	HIP	1	1	Job Reference (optional)	

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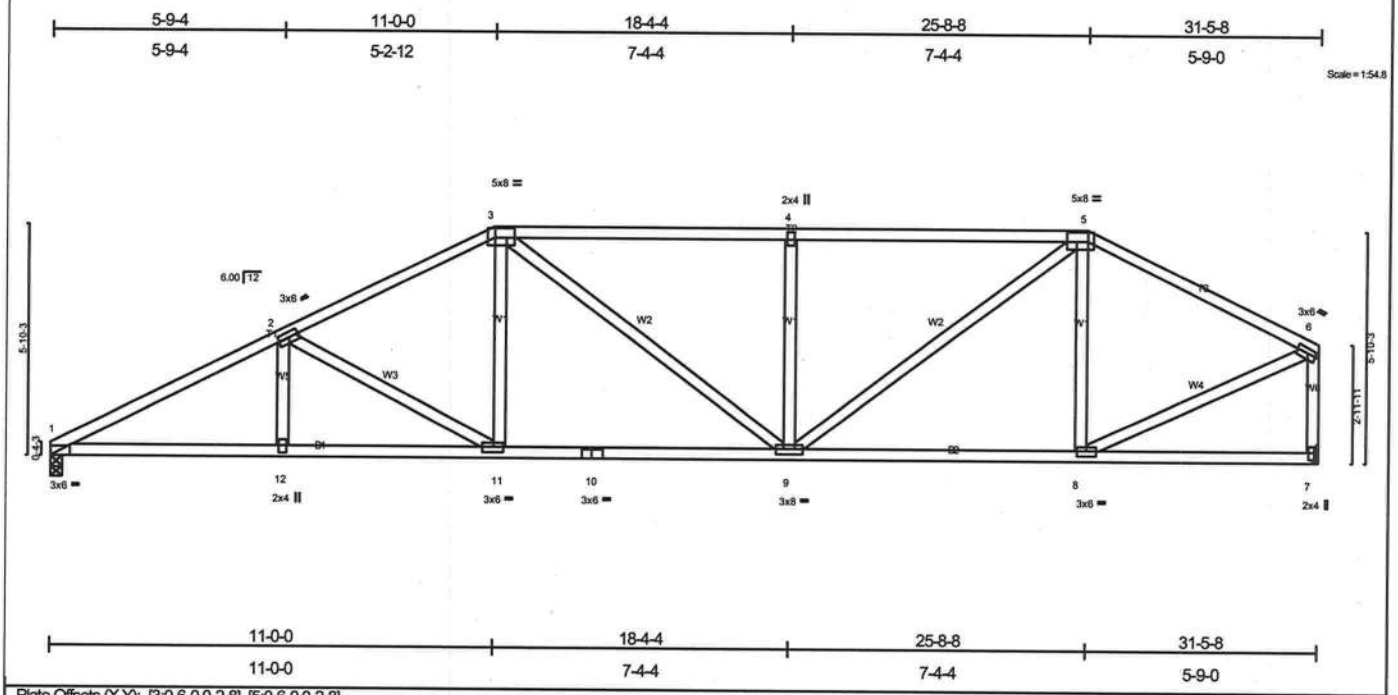

Plate Offsets (X,Y): [3:0-6-0,0-2-8], [5:0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	-0.16	9-11	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.22	9-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.06	7	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 171 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-11-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-2-3 oc bracing.

REACTIONS (lb/size) 1=1309/0-3-8, 7=1309/Mechanical
Max Horz 1=182(load case 5)
Max Uplift1=-349(load case 5), 7=-338(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-2462/1120, 2-3=-1973/940, 3-4=-1854/928, 4-5=-1854/928, 5-6=-1347/622, 6-7=-1229/605
BOT CHORD 1-12=-1063/2128, 11-12=-1063/2128, 10-11=-748/1723, 9-10=-748/1723, 8-9=-464/1143, 7-8=-47/61
WEBS 3-11=-133/417, 3-9=-166/300, 4-9=-423/299, 5-9=-376/939, 5-8=-323/249, 6-8=-468/1208, 2-12=0/179, 2-11=-476/363

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 349 lb uplift at joint 1 and 338 lb uplift at joint 7.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205015
L101301	T04	HIP	1	2	Job Reference (optional)	

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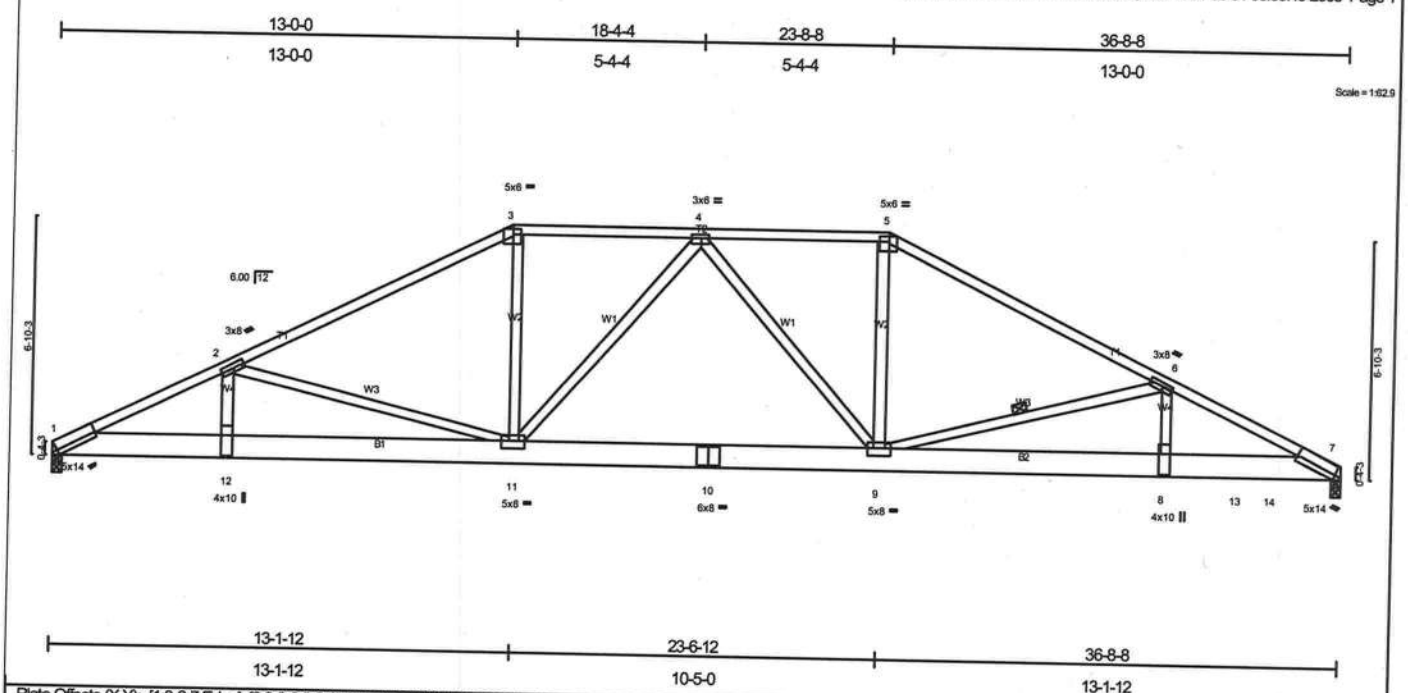

Plate Offsets (X,Y): [1:0-2-7,Edge], [3:0-3-0,0-2-0], [5:0-3-0,0-2-0], [7:0-2-7,Edge], [8:0-6-12,0-2-0], [12:0-6-12,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.23	8-9	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.49	Vert(TL)	-0.32	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.96	Horz(TL)	0.07	7	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 487 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-7-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-9

REACTIONS (lb/size) 1=2201/0-3-8, 7=6278/0-3-8
Max Horz 1=-127(load case 5)
Max Uplift 1=-668(load case 4), 7=-2207(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-4550/1367, 2-3=-3897/1142, 3-4=-3430/1106, 4-5=-4588/1543, 5-6=-5171/1623, 6-7=-12427/4347
BOT CHORD 1-12=-1307/4048, 11-12=-1307/4048, 10-11=-1220/4082, 9-10=-1220/4082, 8-9=-3855/11129, 8-13=-3855/11129, 13-14=-3855/11129, 7-14=-3855/11129
WEBS 3-11=-341/1297, 5-9=-549/1848, 2-11=-680/390, 4-11=-1199/526, 4-9=-418/907, 6-9=-6865/2728, 2-12=0/123, 6-8=-1702/4775

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, Except member 10-7 2 X 8 - 4 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 6-8 2 X 4 - 2 rows at 0-6-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 668 lb uplift at joint 1 and 2207 lb uplift at joint 7.
7) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 4522.0lb down and 1707.4lb up at 31-8-8, and 865.0lb down and 326.6lb up at 33-8-8 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 5-7=-54, 1-14=-30
Concentrated Loads (lb)
Vert: 8=-4522(F) 13=-865(F)
Trapezoidal Loads (plf)
Vert: 14=-66(F=-36)-to-7=-30

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T05	SPECIAL	1	1	Job Reference (optional)

Dwg.#020205016

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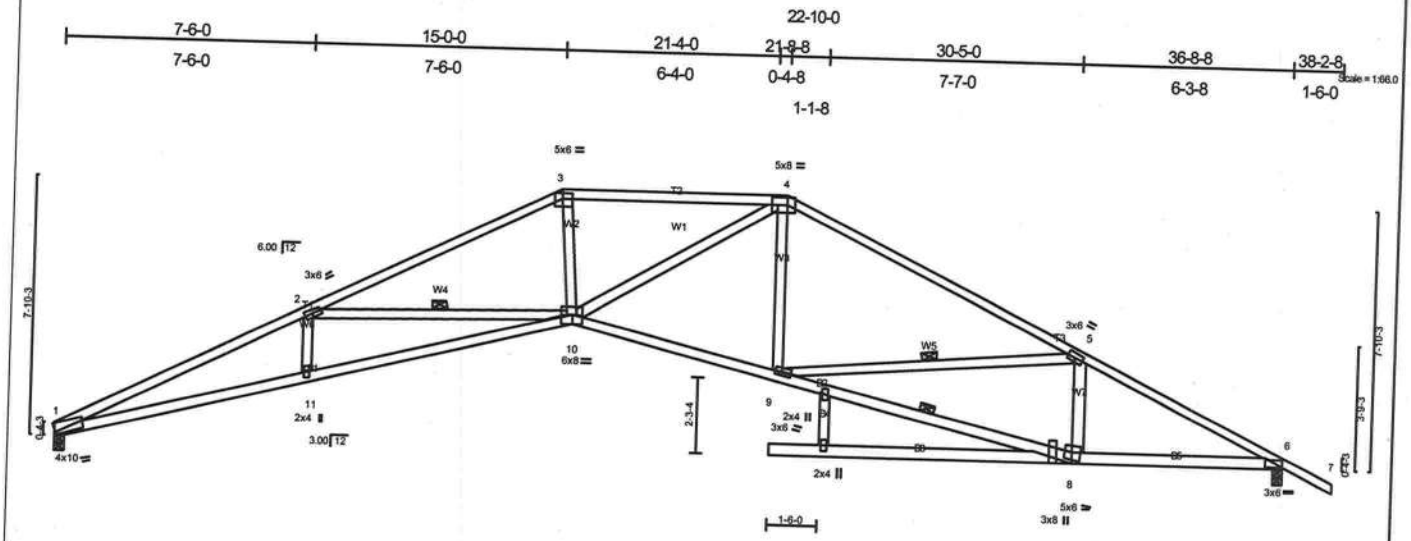

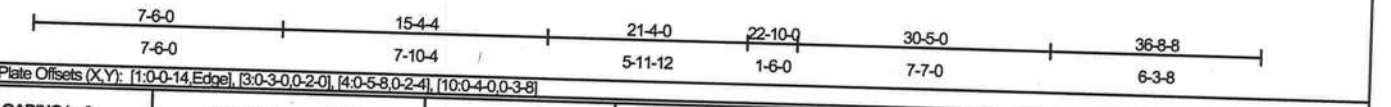

Plate Offsets (X,Y): [1:0-0-14,Edge], [3:0-3-0,0-2-0], [4:0-5-8,0-2-4], [10:0-4-0,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	-0.59	10-11	>738	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.83	10-11	>524	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.45	6	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 189 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B1 2 X 4 SYP No.1D, B4 2 X 4 SYP No.3, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed.
BOT CHORD Rigid ceiling directly applied or 4-5-8 oc bracing. Except:
1 Row at midpt 8-9
WEBS 1 Row at midpt 2-10, 5-9

REACTIONS (lb/size) 1=1527/0-3-8, 6=1620/0-3-8
Max Horz 1=-195(load case 6)
Max Uplift1=-434(load case 5), 6=-530(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5366/2486, 2-3=-4020/1733, 3-4=-3643/1685, 4-5=-2994/1388, 5-6=-2966/1442, 6-7=0/35
BOT CHORD 1-11=-2108/4866, 10-11=-2105/4862, 9-10=-887/2688, 8-9=-1164/2676, 6-8=-1138/2583
WEBS 2-11=0/256, 2-10=-1185/894, 3-10=-490/1401, 4-10=-385/1272, 5-8=-394/337, 4-9=-65/262, 5-9=-106/315

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 434 lb uplift at joint 1 and 530 lb uplift at joint 6.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T06	SPECIAL	1	1	Job Reference (optional)

Dwg.#020205017

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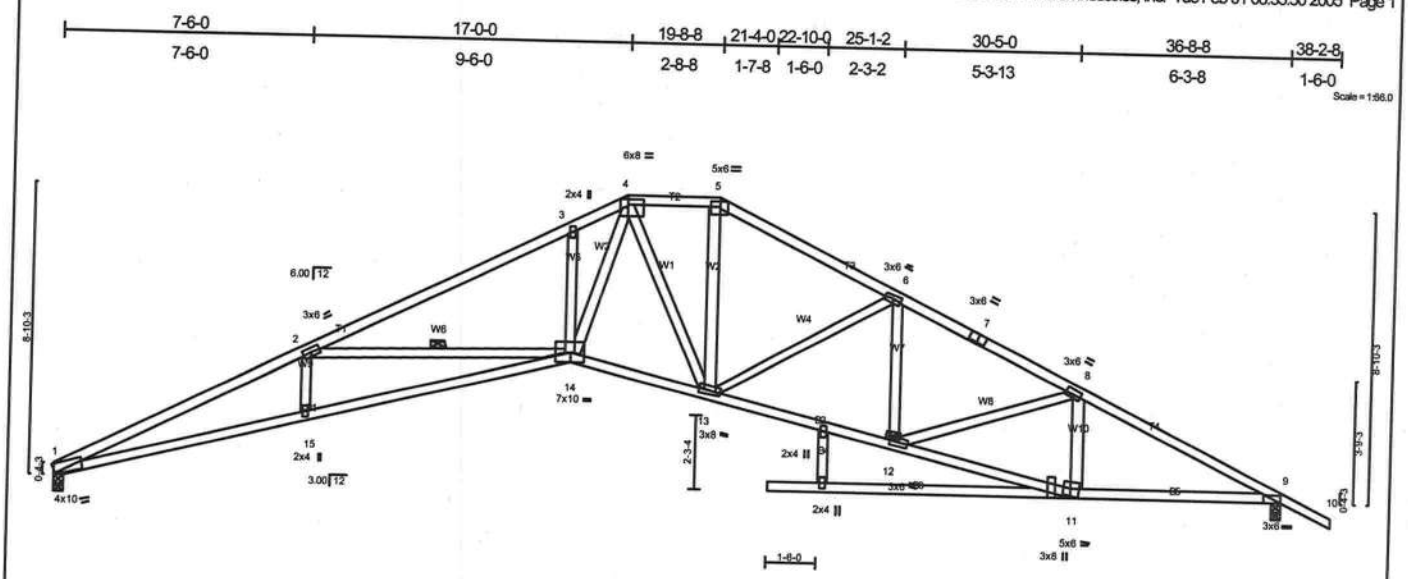

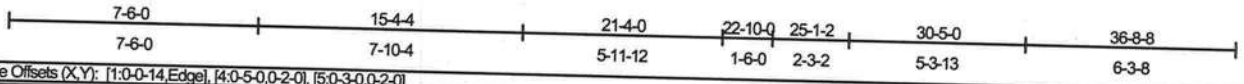

Plate Offsets (X,Y): [1:0-0-14,Edge], [4:0-5-0,0-2-0], [5:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.60	14-15	>732	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.84	14-15	>519	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.46	9	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 207 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D *Except*
T3 2 X 4 SYP No.1D, T4 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2D *Except*
B1 2 X 4 SYP No.1D, B4 2 X 4 SYP No.3, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Sheathed.
BOT CHORD Rigid ceiling directly applied or 4-4-8 oc bracing. Except:
1 Row at midpt 11-13
WEBS 1 Row at midpt 2-14

REACTIONS (lb/size) 1=1527/0-3-8, 9=1620/0-3-8
Max Horz 1=-215(load case 6)
Max Uplift1=-452(load case 5), 9=-548(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5372/2578, 2-3=-4009/1831, 3-4=-4000/2056, 4-5=-2399/1312, 5-6=-2722/1382, 6-7=-3004/1563, 7-8=-3077/1544, 8-9=-2902/1450, 9-10=0/35
BOT CHORD 1-15=-2191/4873, 14-15=-2188/4868, 13-14=-847/2761, 12-13=-1106/2793, 11-12=-1152/2590, 9-11=-1129/2515
WEBS 2-15=0/256, 8-11=-434/306, 5-13=-371/922, 8-12=-43/241, 4-13=-721/220, 3-14=-402/489, 2-14=-1228/889, 4-14=-1252/2571, 6-12=-31/104, 6-13=-426/369

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 452 lb uplift at joint 1 and 548 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T06TRY	SPECIAL	1	1	Job Reference (optional)

Dwg.#020205018

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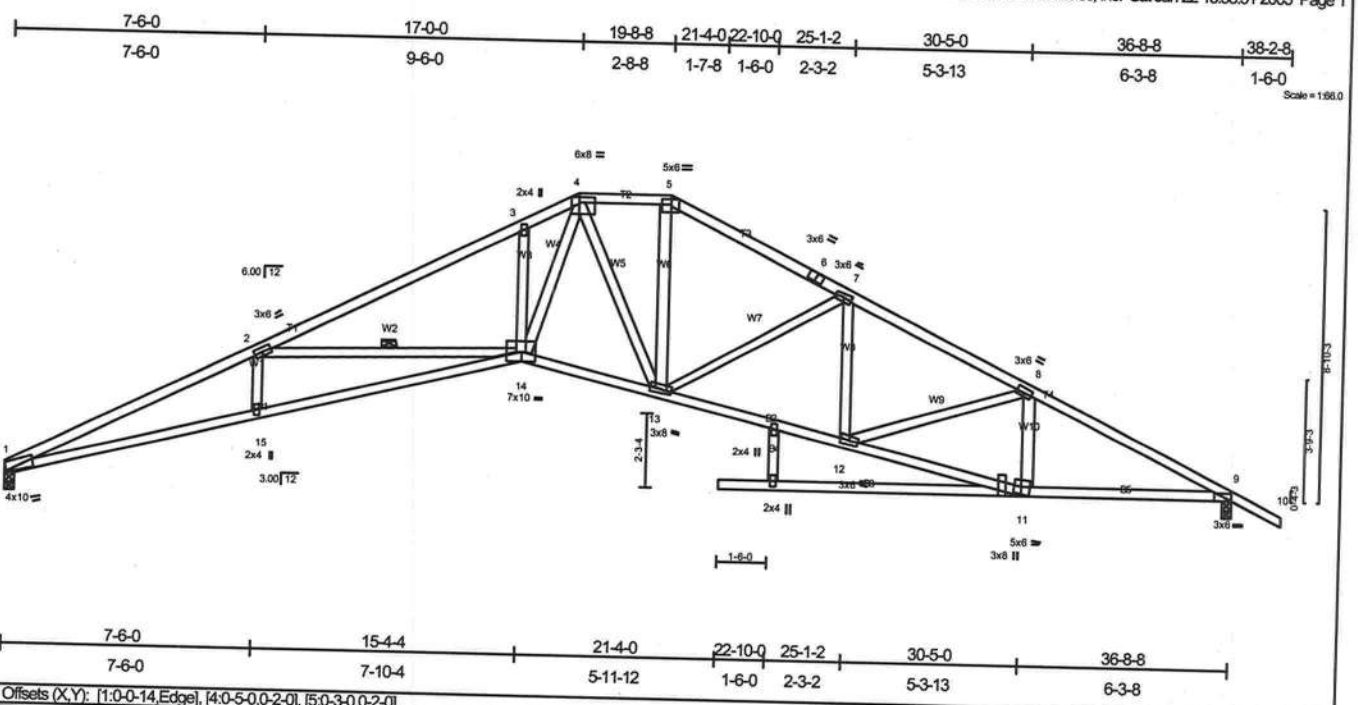

Plate Offsets (X,Y): [1:0-0-14,Edge], [4:0-5-0,0-2-0], [5:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.95	Vert(LL)	-0.60	14-15	>732	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.84	14-15	>519	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.82	Horz(TL)	0.46	9	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 207 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D *Except* T3 2 X 4 SYP No.1D, T4 2 X 4 SYP No.1D
BOT CHORD 2 X 4 SYP No.2D *Except* B1 2 X 4 SYP No.1D, B4 2 X 4 SYP No.3, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Sheathed.
BOT CHORD Rigid ceiling directly applied or 4-4-8 oc bracing. Except: 1 Row at midpt 11-13
WEBS 1 Row at midpt 2-14

REACTIONS (lb/size) 1=1527/0-3-8, 9=1620/0-3-8
Max Horz 1=-215(load case 6)
Max Uplift 1=-452(load case 5), 9=-548(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5372/2578, 2-3=-4009/1831, 3-4=-4000/2056, 4-5=-2399/1312, 5-6=-2658/1382, 6-7=-2722/1363, 7-8=-3077/1563, 8-9=-2902/1450, 9-10=0/35
BOT CHORD 1-15=-2191/4873, 14-15=-2188/4868, 13-14=-847/2761, 12-13=-1106/2793, 11-12=-1152/2590, 9-11=-1129/2515
WEBS 2-15=0/256, 8-11=-434/306, 5-13=-371/922, 8-12=-43/242, 4-13=-720/220, 3-14=-403/489, 2-14=-1228/889, 4-14=-1252/2571, 7-12=-31/104, 7-13=-426/370

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 452 lb uplift at joint 1 and 548 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE Dwg.#020205019
L101301	T07	SPECIAL	3	1	Job Reference (optional)

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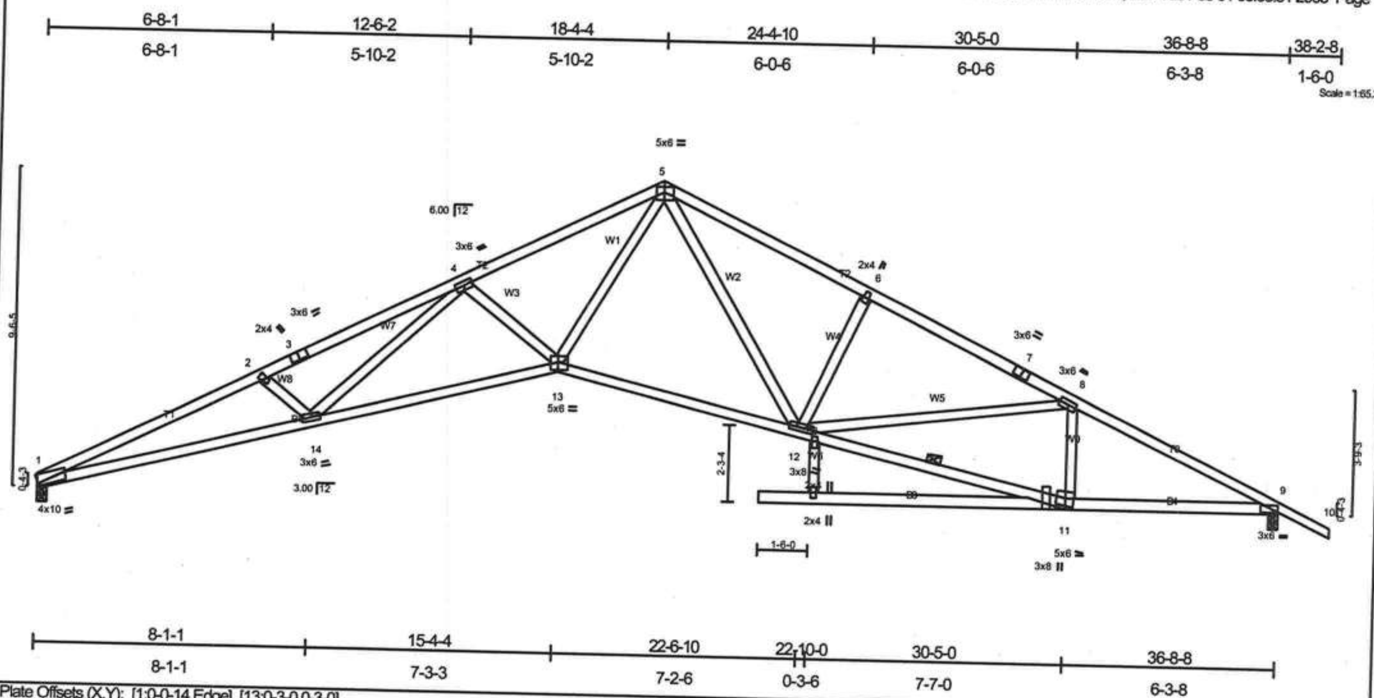

Plate Offsets (X,Y): [1:0-0-14,Edge], [13:0-3-0,0-3-0]

LOADING (psf)		SPACING		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.71	Vert(LL)	-0.56	13-14	>785	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.93	Vert(TL)	-0.78	13-14	>559	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.78	Horz(TL)	0.44	9	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 196 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B1 2 X 4 SYP No.1D, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Sheathed or 2-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-3-14 oc bracing. Except:
1 Row at midpt 11-12

REACTIONS (lb/size) 1=1527/0-3-8, 9=1620/0-3-8
Max Horz 1=-229(load case 6)
Max Uplift1=-463(load case 5), 9=-558(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-5307/2664, 2-3=-5075/2538, 3-4=-5003/2558, 4-5=-3902/1917, 5-6=-2940/1621, 6-7=-3005/1589, 7-8=-3080/1569, 8-9=-2923/1484, 9-10=0/35
BOT CHORD 1-14=-2276/4829, 13-14=-1642/4011, 12-13=-739/2352, 11-12=-1186/2622, 9-11=-1160/2534
WEBS 2-14=-251/347, 4-14=-492/881, 4-13=-608/522, 5-13=-1006/2430, 5-12=-448/711, 6-12=-304/351, 8-12=-46/208, 8-11=-397/326

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 463 lb uplift at joint 1 and 558 lb uplift at joint 9.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T07TRY	SPECIAL	3	1	Job Reference (optional)

Dwg.#020205020

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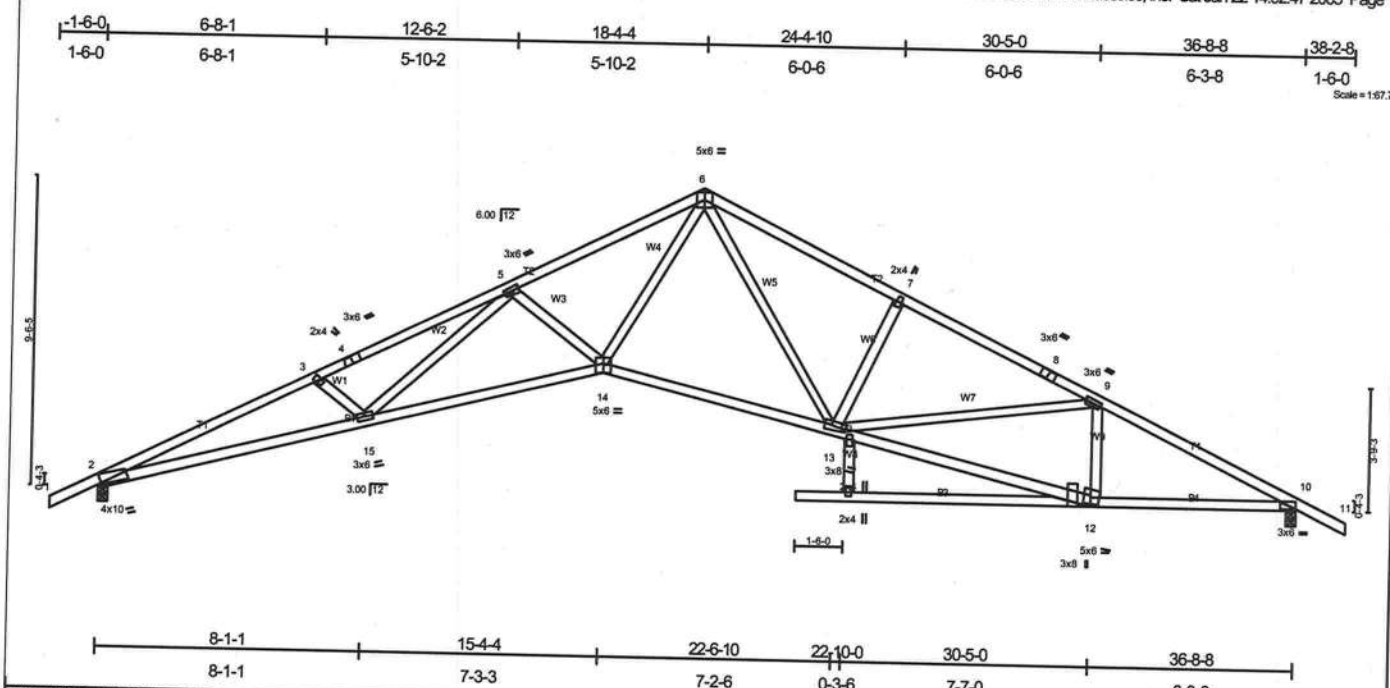


Plate Offsets (X,Y): [2:0-0-14,Edge], [14:0-3-0,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.67	Vert(LL) -0.56 14-15 >785 240	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.83	Vert(TL) -0.78 14-15 >559 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.78	Horz(TL) 0.44 10 n/a n/a	Weight: 199 lb
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D *Except*
B1 2 X 4 SYP No.1D, B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-4-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-5-13 oc bracing. Except:
1 Row at midpt 12-13

REACTIONS (lb/size) 2=1618/0-3-8, 10=1618/0-3-8
Max Horz 2=-215(load case 6)
Max Uplift2=-558(load case 5), 10=-558(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-5281/2596, 3-4=-5036/2475, 4-5=-4965/2495, 5-6=-3891/1900, 6-7=-2934/1611, 7-8=-2999/1579, 8-9=-3074/1559, 9-10=-2918/1477, 10-11=0/35
BOT CHORD 2-15=-2208/4787, 14-15=-1619/3996, 13-14=-730/2346, 12-13=-1180/2618, 10-12=-1154/2530
WEBS 3-15=-243/334, 5-15=-449/854, 5-14=-602/512, 6-14=-993/2421, 6-13=-449/711, 7-13=-304/351, 9-13=-44/206, 9-12=-396/324

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 558 lb uplift at joint 2 and 558 lb uplift at joint 10.

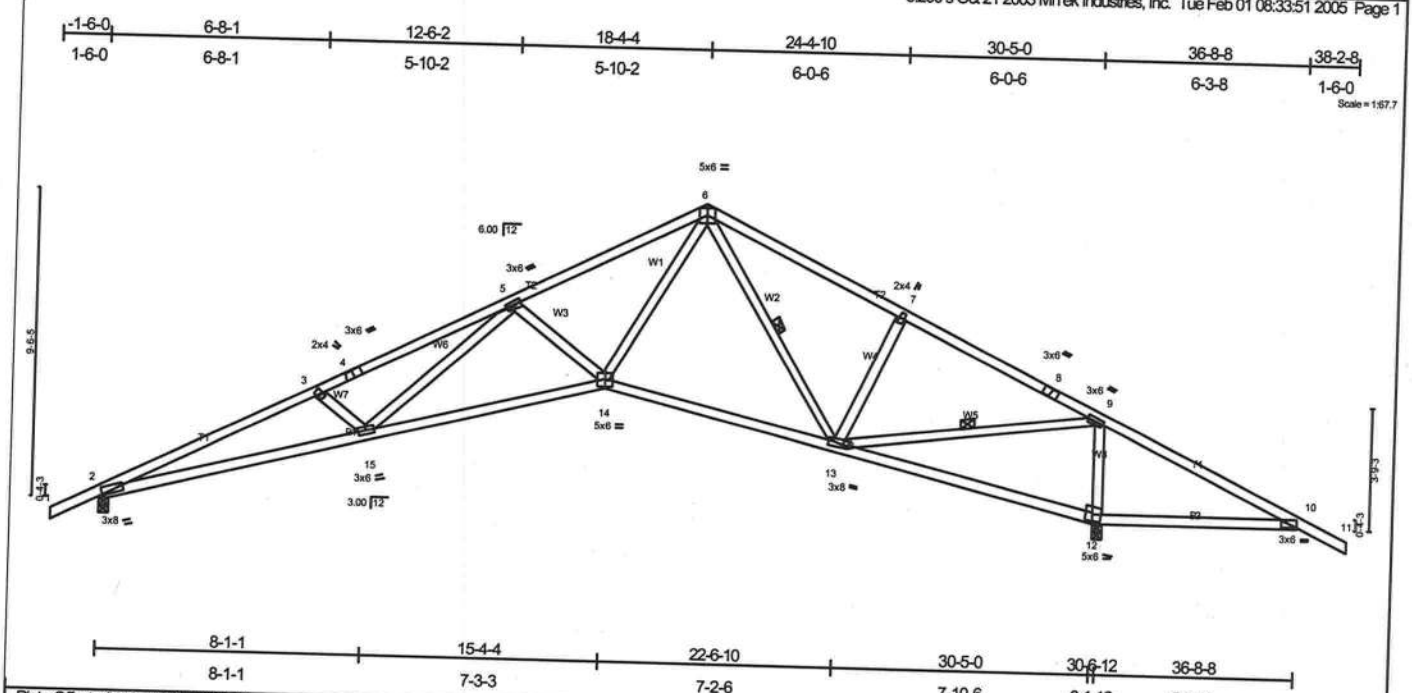
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T08	SPECIAL	4	1	Job Reference (optional)

Dwg.#020205021

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:51 2005 Page 1



8-1-1	15-4-4	22-6-10	30-5-0	30-6-12	36-8-8	
8-1-1	7-3-3	7-2-6	7-10-6	0-1-12	6-1-12	

Plate Offsets (X,Y): [13:0-2-7,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.35	14-15	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.81	Vert(TL)	-0.50	14-15	>731	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.61	Horz(TL)	0.28	12	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 183 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-11-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-5-2 oc bracing.
WEBS 1 Row at midpt 6-13, 9-13

REACTIONS (lb/size) 2=1289/0-3-8, 12=1952/0-3-8
Max Horz 2=-215(load case 6)
Max Uplift 2=-488(load case 5), 12=-841(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/34, 2-3=-3941/1729, 3-4=-3684/1600, 4-5=-3612/1621, 5-6=-2475/986, 6-7=-1335/572, 7-8=-1396/540, 8-9=-1476/519, 9-10=-774/724, 10-11=0/35
BOT CHORD 2-15=-1417/3565, 14-15=-860/2704, 13-14=-268/1349, 12-13=-690/903, 10-12=-570/817
WEBS 3-15=-264/355, 5-15=-487/892, 5-14=-613/523, 6-14=-596/1801, 6-13=-309/160, 7-13=-295/339, 9-13=-1016/1907, 9-12=-1542/1061

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; cantilever right exposed ; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 488 lb uplift at joint 2 and 841 lb uplift at joint 12.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T09	HIP	1	1	Job Reference (optional)

Dwg.#020205022

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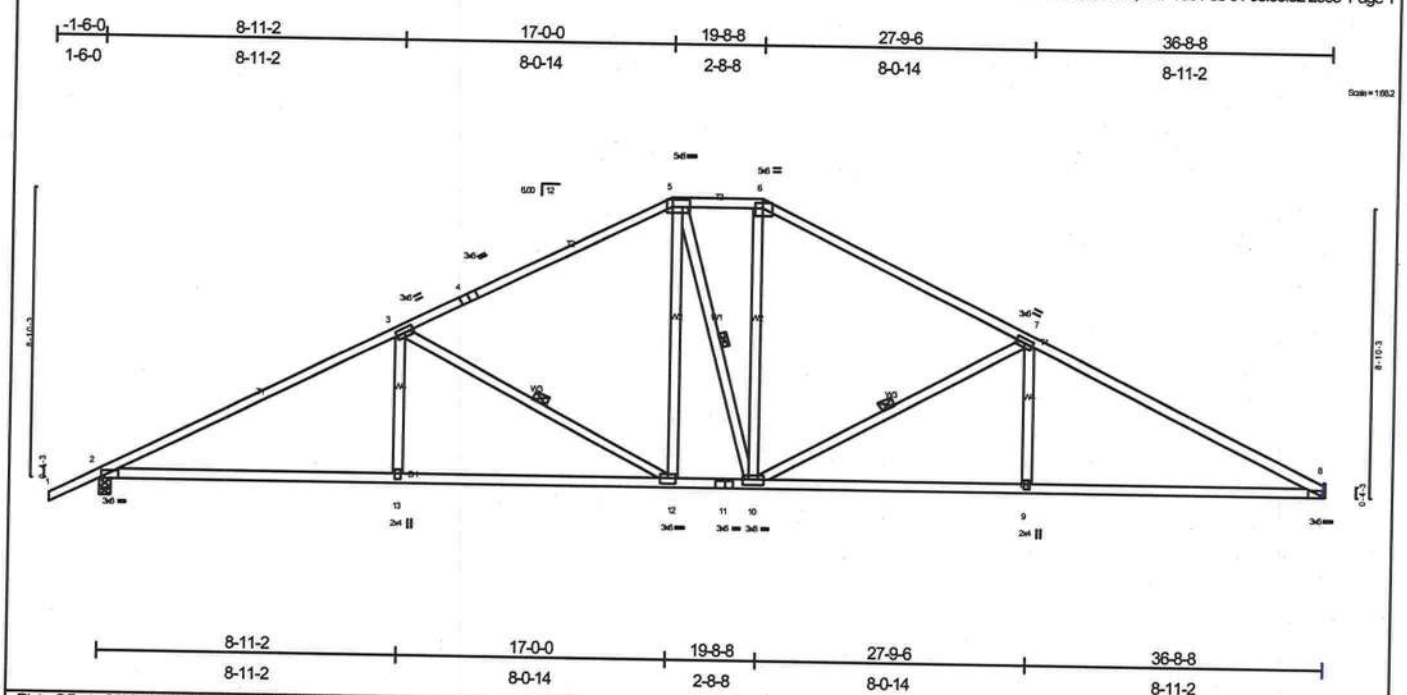

Plate Offsets (X,Y): [5:0-6-0,0-2-8], [6:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.75	Vert(LL)	-0.30	8-9	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.43	8-9	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.56	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 195 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-7-9 oc bracing.
WEBS 1 Row at midpt 3-12, 5-10, 7-10

REACTIONS (lb/size) 8=1531/Mechanical, 2=1624/0-3-8
Max Horz 2=216(load case 5)
Max Uplift 8=-454(load case 6), 2=-549(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2790/1394, 3-4=-1949/1075, 4-5=-1840/1103, 5-6=-1657/1084, 6-7=-1952/1105, 7-8=-2814/1421
BOT CHORD 2-13=-1101/2398, 12-13=-1101/2398, 11-12=-585/1654, 10-11=-585/1654, 9-10=-1130/2427, 8-9=-1130/2427
WEBS 3-13=0/309, 3-12=-861/593, 5-12=-251/552, 5-10=-208/223, 6-10=-260/577, 7-10=-890/626, 7-9=0/316

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 8 and 549 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T10	HIP	1	1	Job Reference (optional)

Dwg.#020205023

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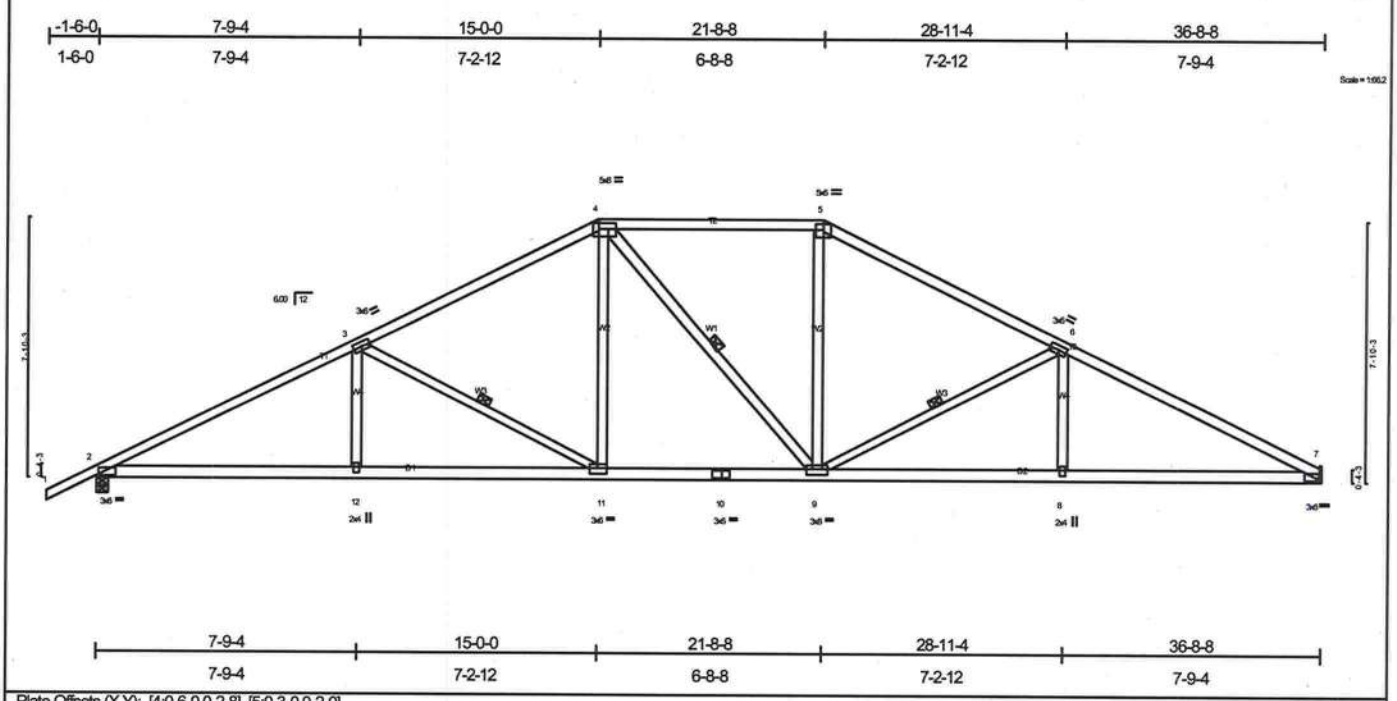

Plate Offsets (X,Y): [4:0-6-0,0-2-8], [5:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.21	7-8	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.30	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.12	7	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 188 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-4-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-9-8 oc bracing.
WEBS 1 Row at midpt 3-11, 4-9, 6-9

REACTIONS (lb/size) 7=1531/Mechanical, 2=1624/0-3-8
Max Horz 2=196(load case 5)
Max Uplift 7=-436(load case 6), 2=-530(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2844/1375, 3-4=-2124/1124, 4-5=-1831/1097, 5-6=-2129/1128, 6-7=-2876/1408
BOT CHORD 2-12=-1100/2454, 11-12=-1100/2454, 10-11=-655/1827, 9-10=-655/1827, 8-9=-1136/2490, 7-8=-1136/2490
WEBS 3-12=0/261, 3-11=-723/509, 4-11=-198/538, 4-9=-174/185, 5-9=-207/548, 6-9=-759/547, 6-8=0/270

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 436 lb uplift at joint 7 and 530 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE Dwg.#020205024
L101301	T11	HIP	1	1	Job Reference (optional)

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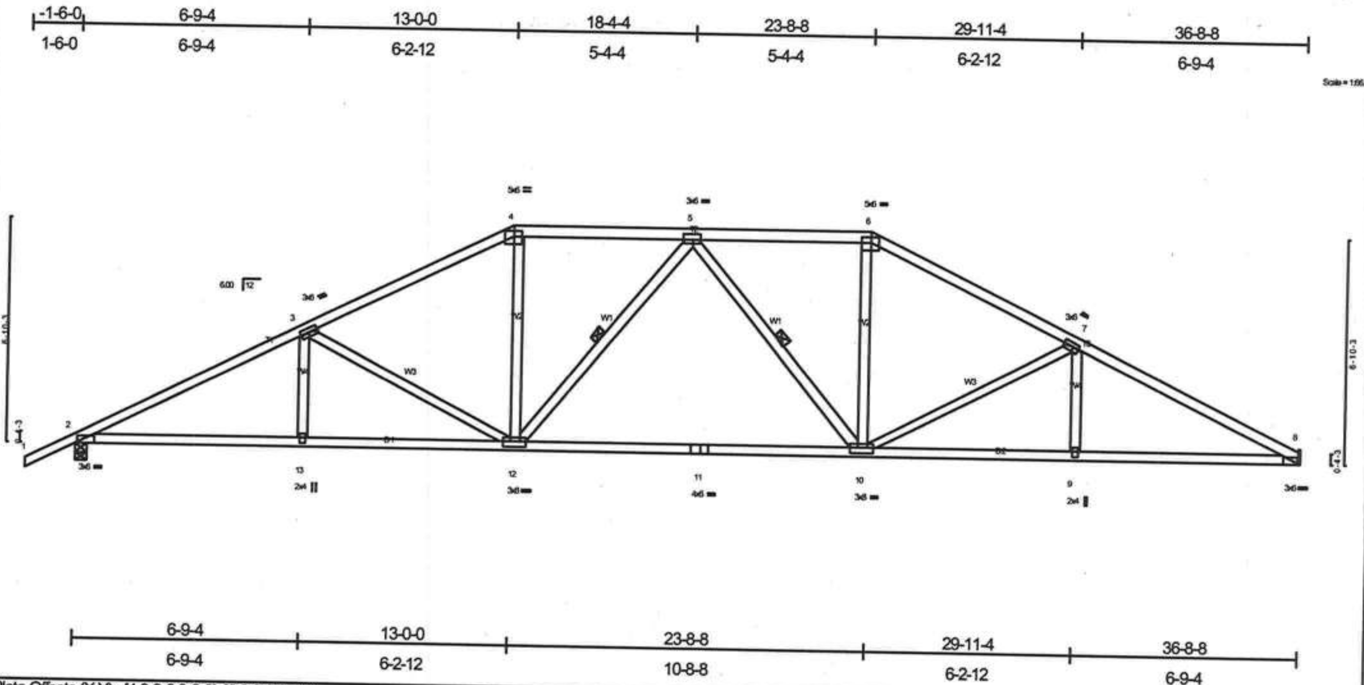

Plate Offsets (X,Y): [4:0-3-0,0-2-0], [6:0-3-0,0-2-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.42	Vert(LL) -0.39 10-12 >999 240	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.79	Vert(TL) -0.57 10-12 >773 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.56	Horz(TL) 0.12 8 n/a n/a	
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		Weight: 189 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-6-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 5-10-14 oc bracing.
WEBS 1 Row at midpt 5-12, 5-10

REACTIONS (lb/size) 8=1531/Mechanical, 2=1624/0-3-8
Max Horz 2=176(load case 5)
Max Uplift 8=-415(load case 6), 2=-509(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2877/1349, 3-4=-2306/1140, 4-5=-2007/1100, 5-6=-2015/1106, 6-7=-2316/1148, 7-8=-2900/1390
BOT CHORD 2-13=-1090/2490, 12-13=-1090/2490, 11-12=-788/2116, 10-11=-788/2116, 9-10=-1134/2535, 8-9=-1134/2535
WEBS 3-13=0/185, 3-12=-572/428, 4-12=-235/680, 5-12=-311/192, 5-10=-301/189, 6-10=-243/687, 7-10=-613/471, 7-9=0/196

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 415 lb uplift at joint 8 and 509 lb uplift at joint 2.

LOAD CASE(S) Standard

February 2, 2005, Truss Design Engineer:
Thomas E. Miller PE 56877, Byron K. Anderson PE 60987
Structural Engineering and Inspections, Inc. EB 9196
16105 N. Florida Ave. Ste B, Lutz, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205025
L101301	T12	HIP	1	1	Job Reference (optional)	

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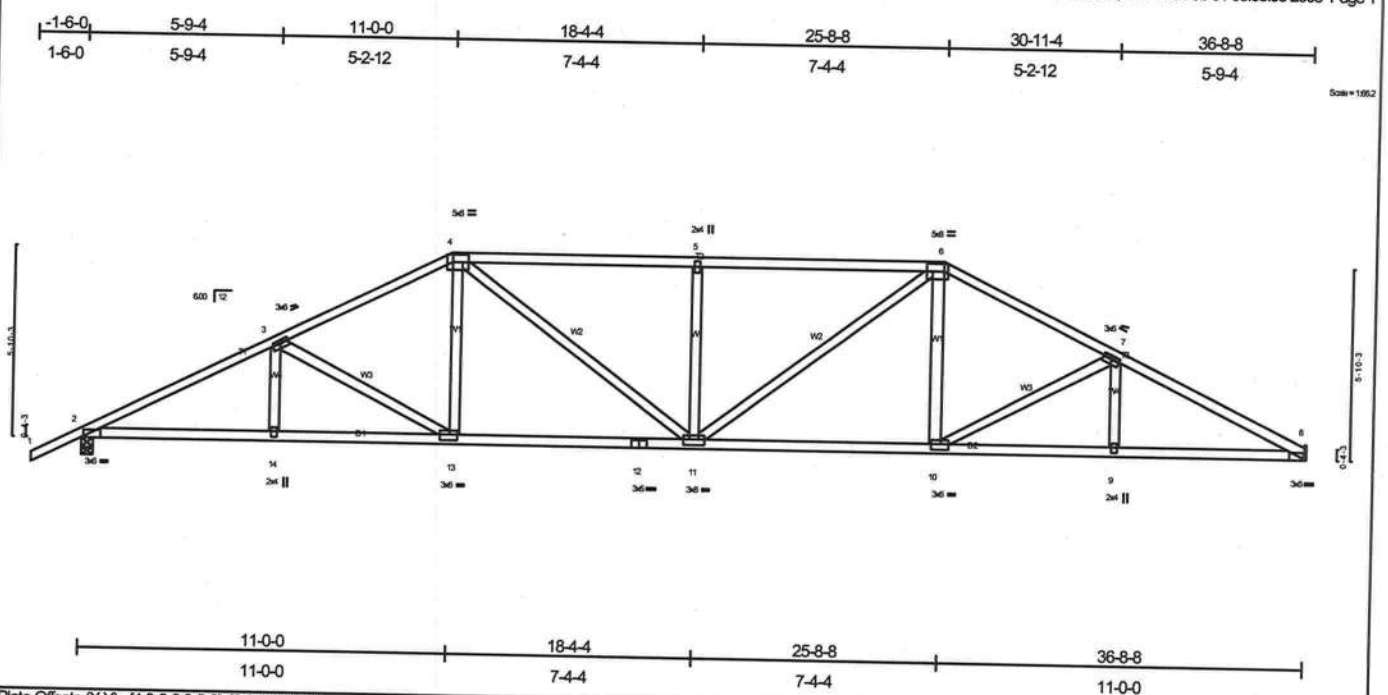

Plate Offsets (X,Y): [4:0-6-0,0-2-8], [6:0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.24	10-11	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.34	10-11	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(TL)	0.12	8	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-7-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-3 oc bracing.

REACTIONS (lb/size) 8=1531/Mechanical, 2=1624/0-3-8
Max Horz 2=156(load case 5)
Max Uplift 8=-392(load case 6), 2=-486(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2917/1313, 3-4=-2454/1165, 4-5=-2582/1278, 5-6=-2582/1278, 6-7=-2469/1179, 7-8=-2967/1364
BOT CHORD 2-14=-1069/2528, 13-14=-1069/2528, 12-13=-790/2153, 11-12=-790/2153, 10-11=-802/2167, 9-10=-1123/2584, 8-9=-1123/2584
WEBS 4-13=-119/405, 4-11=-263/652, 5-11=-420/299, 6-11=-258/638, 6-10=-138/424, 7-10=-491/370, 3-14=0/167, 3-13=-443/321, 7-9=0/180

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 392 lb uplift at joint 8 and 486 lb uplift at joint 2.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T13	HIP	1	1	Job Reference (optional)

Dwg.#020205026

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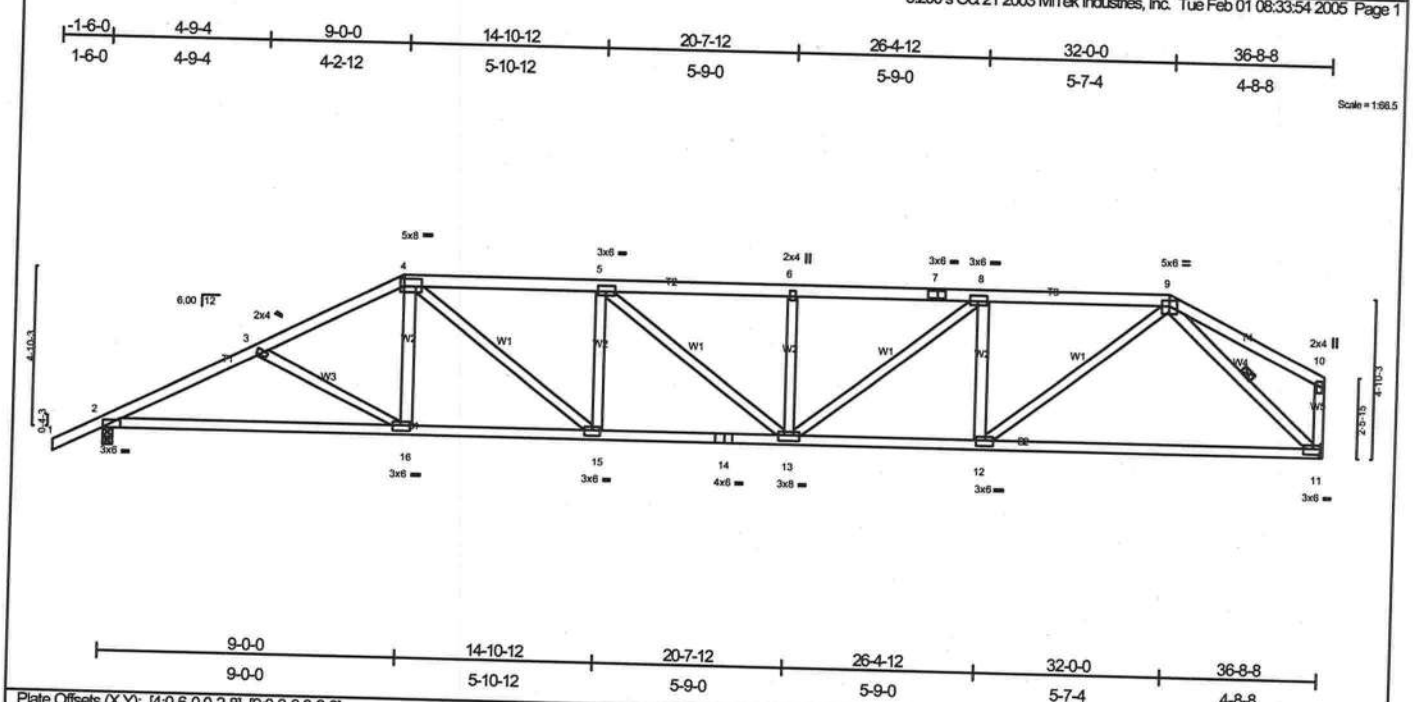

Plate Offsets (X,Y): [4:0-6-0,0-2-8], [9:0-3-0,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.82	Vert(LL)	-0.27	11-12	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.40	11-12	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.11	11	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 200 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-7-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-11-15 oc bracing.
WEBS 1 Row at midpt 9-11

REACTIONS (lb/size) 2=1620/0-3-8, 11=1527/Mechanical
Max Horz 2=196(load case 5)
Max Uplift2=-457(load case 5), 11=-463(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2849/1272, 3-4=-2621/1138, 4-5=-2982/1341, 5-6=-3040/1353, 6-7=-3040/1353, 7-8=-3040/1353, 8-9=-2529/1118, 9-10=-241/103, 10-11=-208/149
BOT CHORD 2-16=-1175/2494, 15-16=-946/2310, 14-15=-1207/2981, 13-14=-1207/2981, 12-13=-984/2529, 11-12=-559/1329
WEBS 3-16=-221/259, 4-16=-70/347, 9-12=-547/1578, 4-15=-402/926, 8-12=-734/401, 5-15=-411/276, 8-13=-300/666, 5-13=-78/139, 6-13=-307/224, 9-11=-1644/760

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 457 lb uplift at joint 2 and 463 lb uplift at joint 11.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T14	HIP	1	1	Job Reference (optional)

Dwg.#020205027

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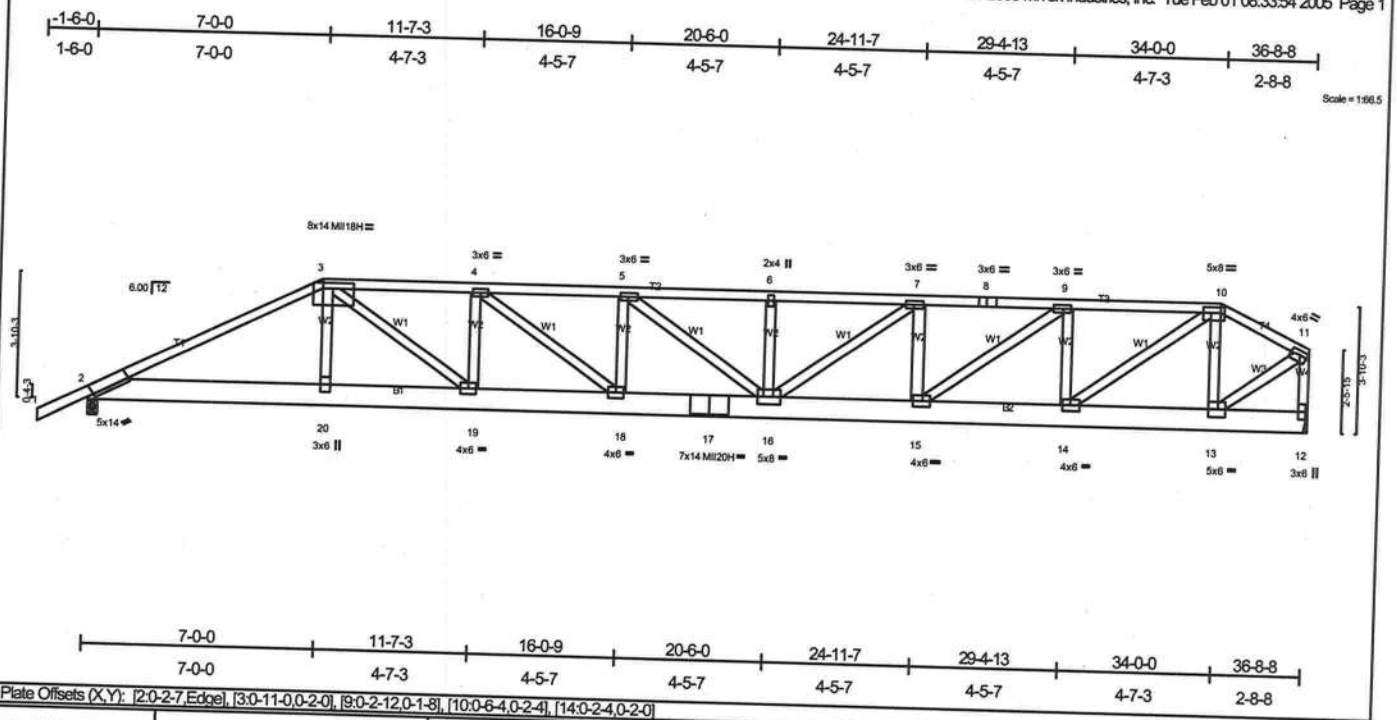

Plate Offsets (X,Y): [2:0-2-7,Edge], [3:0-11-0,0-2-0], [9:0-2-12,0-1-8], [10:0-6-4,0-2-4], [14:0-2-4,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.93	Vert(LL)	-0.70	16-18	>627	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.98	16-18	>448	180	MII20H	187/143
BCLL	10.0	Rep Stress Incr	NO	WB	1.00	Horz(TL)	0.13	12	n/a	n/a	MII18H	195/188
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 255 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W4 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D
W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D, W1 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 1-6-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-2-14 oc bracing.

REACTIONS (lb/size) 2=3239/0-3-8, 12=3375/Mechanical
Max Horz 2=179(load case 4)
Max Uplift 2=-1094(load case 3), 12=-1208(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-6533/2316, 3-4=-7972/2956, 4-5=-8998/3348, 5-6=-8905/3320, 6-7=-8905/3320, 7-8=-7888/2943, 8-9=-7888/2943, 9-10=-5730/2136, 10-11=-2856/1044, 11-12=-3217/1166
BOT CHORD 2-20=-2096/5784, 19-20=-2104/5818, 18-19=-2938/7971, 17-18=-3330/8998, 16-17=-3330/8998, 15-16=-2925/7888, 14-15=-2118/5730, 13-14=-894/2473, 12-13=-30/78
WEBS 3-20=-195/790, 11-13=-1138/3113, 10-13=-1503/607, 10-14=-1521/4054, 3-19=-1092/2770, 9-14=-2192/903, 4-19=-1341/597, 9-15=-1016/2721, 4-18=-510/1318, 7-15=-1300/559, 5-18=-476/266, 7-16=-473/1286, 5-16=-122/67, 6-16=-517/273

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MII20 plates unless otherwise indicated.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1094 lb uplift at joint 2 and 1208 lb uplift at joint 12.
6) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539.0lb down and 203.5lb up at 7-0-0 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-10=-118(F=-64), 10-11=-54, 2-20=-30, 13-20=-65(F=-35), 12-13=-129(F=-99)
Concentrated Loads (lb)
Vert: 20=-539(F)

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T15	HIP	1	1	Job Reference (optional)

Dwg.#020205028

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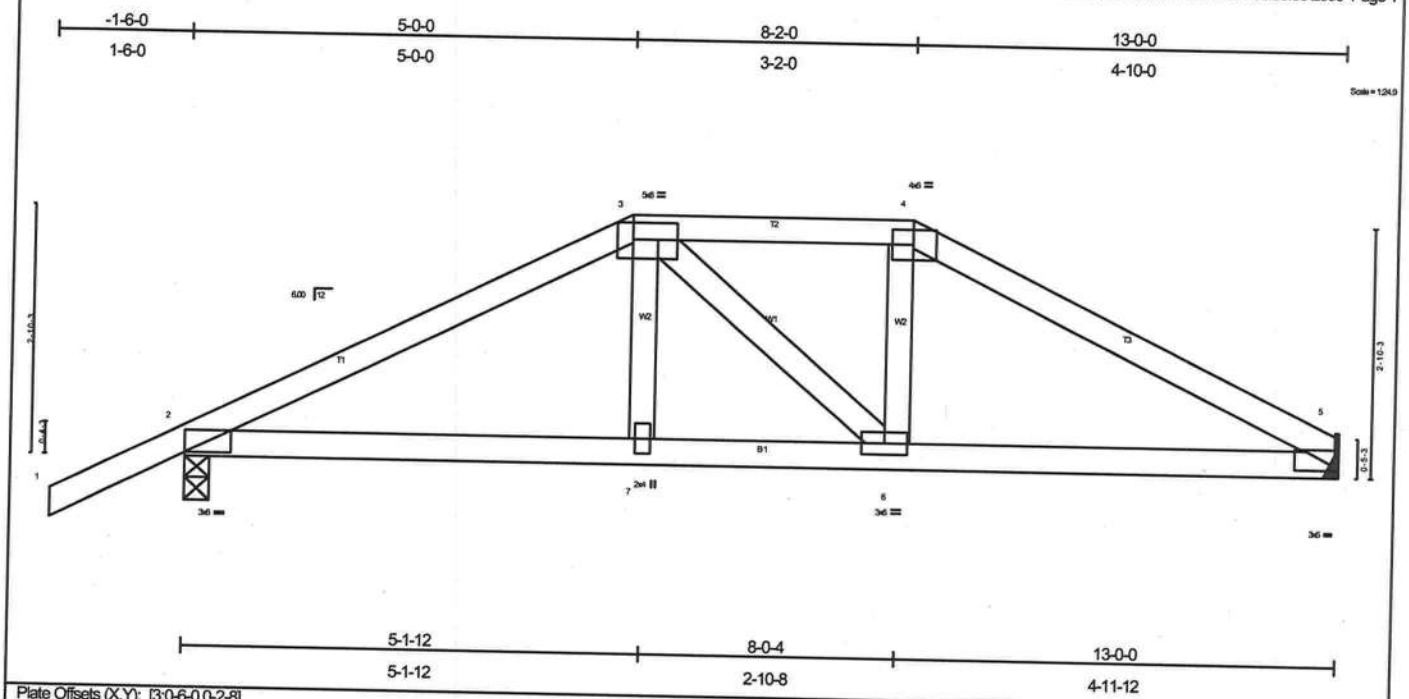

Plate Offsets (X,Y): [3:0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	-0.04	5-6	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.06	5-6	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.14	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 55 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 5-1-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-6-0 oc bracing.

REACTIONS (lb/size) 5=865/Mechanical, 2=960/0-3-8
Max Horz 2=97(load case 4)
Max Uplift 5=-320(load case 5), 2=-420(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1514/535, 3-4=-1320/551, 4-5=-1512/561
BOT CHORD 2-7=-467/1289, 6-7=-472/1312, 5-6=-434/1297
WEBS 3-7=-99/399, 4-6=-130/450, 3-6=-85/93

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 320 lb uplift at joint 5 and 420 lb uplift at joint 2.
5) Girder carries hip end with 5-0-0 end setback
6) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245.0lb down and 126.1lb up at 8-2-0, and 245.0lb down and 126.1lb up at 5-0-0 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-91(F=-37), 4-5=-54, 2-7=-30, 6-7=-50(F=-20), 5-6=-30
Concentrated Loads (lb)
Vert: 7=-245(F) 6=-245(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205029
L101301	T16	COMMON	1	2	Job Reference (optional)	

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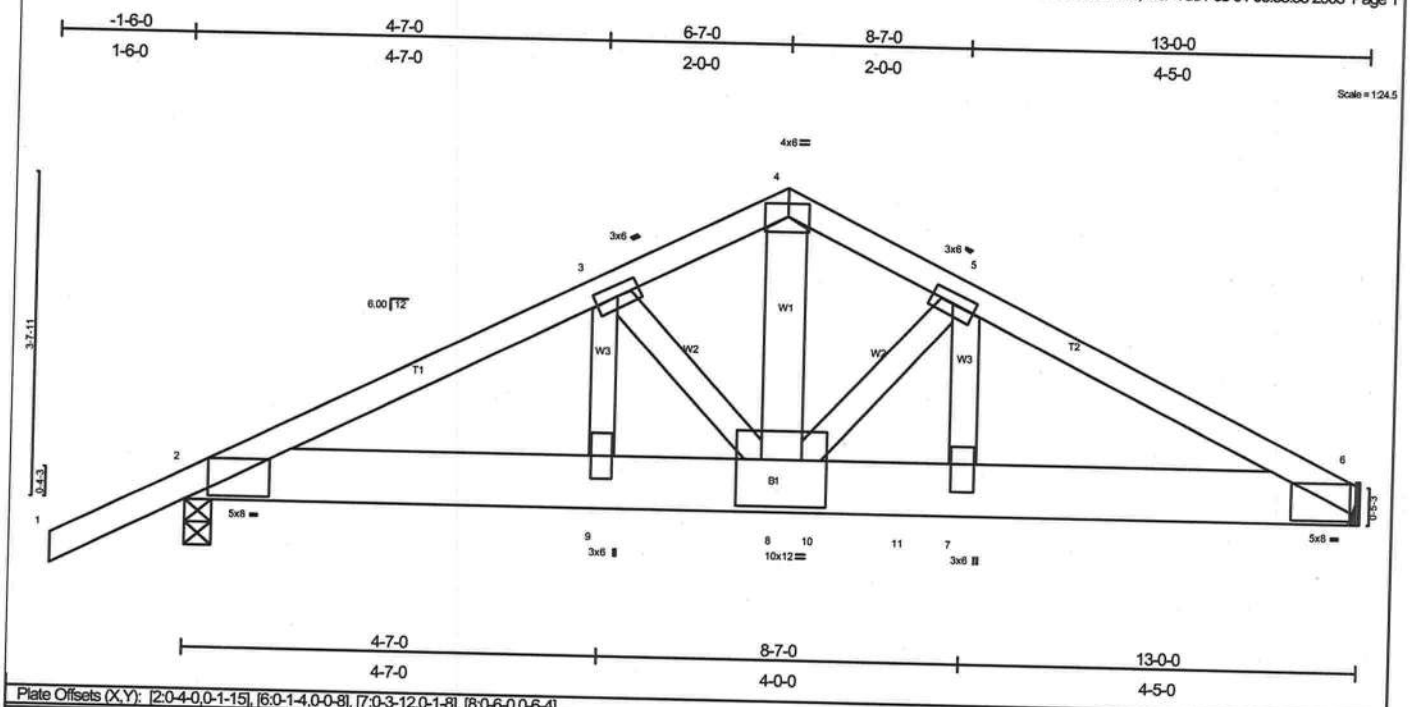

Plate Offsets (X,Y): [2:0-4-0,0-1-15], [6:0-1-4,0-0-8], [7:0-3-12,0-1-8], [8:0-6-0,0-6-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.23	Vert(LL)	-0.06	7	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.09	7	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.35	Horz(TL)	0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 167 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W1 2 X 6 SYP No.1D

BRACING
TOP CHORD Sheathed or 5-2-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=4522/Mechanical, 2=2558/0-3-8
Max Horz 2=119(load case 4)
Max Uplift 6=-1668(load case 5), 2=-991(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/41, 2-3=-4934/1753, 3-4=-5097/1897, 4-5=-5097/1893, 5-6=-6780/2481
BOT CHORD 2-9=-1569/4375, 8-9=-1569/4375, 8-10=-2161/6029, 10-11=-2161/6029, 7-11=-2161/6029, 6-7=-2161/6029
WEBS 4-8=-1635/4384, 3-9=-449/241, 5-7=-786/2204, 3-8=-134/384, 5-8=-2191/882

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 6 - 2 rows at 0-7-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1668 lb uplift at joint 6 and 991 lb uplift at joint 2.
6) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2893.0lb down and 1092.3lb up at 7-0-0 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-11=-30, 6-11=-642(F=-612)
Concentrated Loads (lb)
Vert: 10=-2893(F)

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THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T17	HIP	1	1	Job Reference (optional)

Dwg.#020205030

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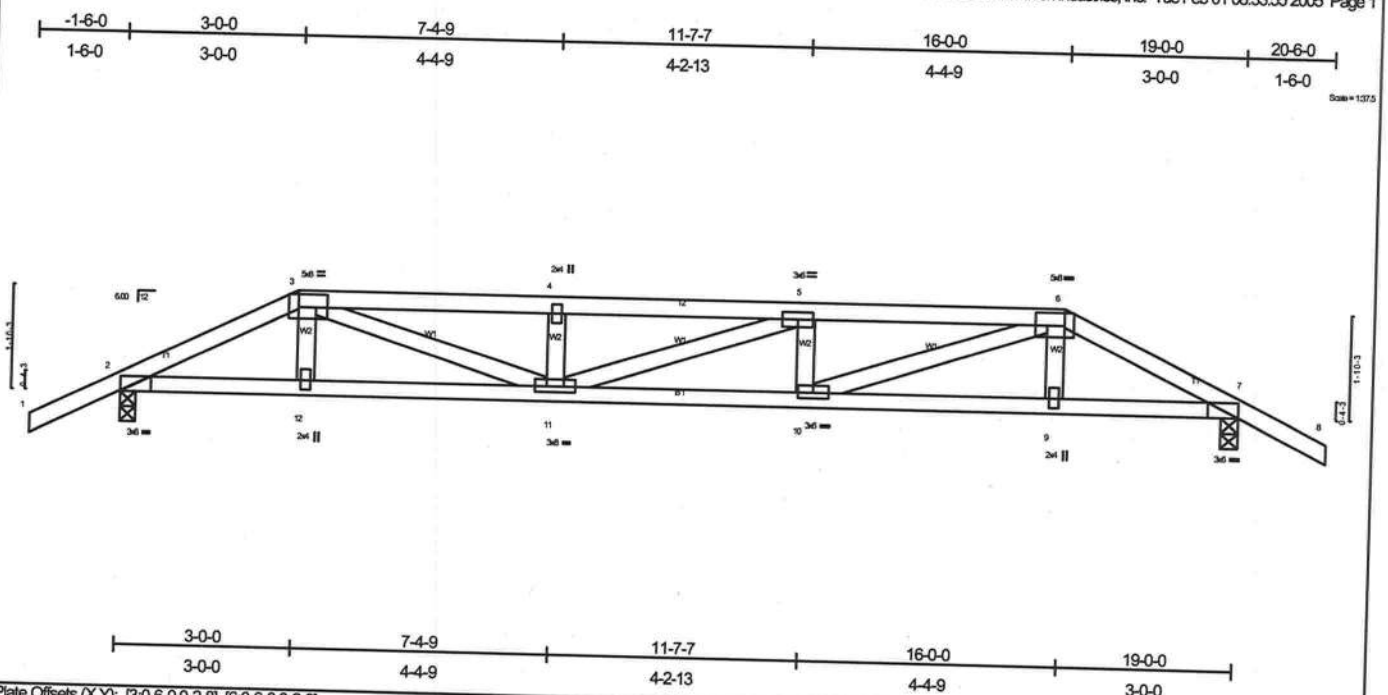

Plate Offsets (X,Y): [3:0-6-0,0-2-8], [6:0-6-0,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.21	Vert(LL)	-0.17	10-11	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.56	Vert(TL)	-0.24	10-11	>938	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.41	Horz(TL)	0.05	7	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 89 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-10-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-6-9 oc bracing.

REACTIONS (lb/size) 2=1034/0-3-8, 7=1034/0-3-8
Max Horz 2=63(load case 4)
Max Uplift2=-345(load case 3), 7=-345(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-1776/649, 3-4=-2726/1052, 4-5=-2725/1052, 5-6=-2727/1053, 6-7=-1776/649, 7-8=0/35
BOT CHORD 2-12=-549/1535, 11-12=-549/1552, 10-11=-997/2726, 9-10=-534/1551, 7-9=-534/1534
WEBS 3-12=-6/188, 3-11=-525/1266, 4-11=-270/202, 5-11=-17/15, 5-10=-276/207, 6-10=-527/1267, 6-9=-5/188

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 345 lb uplift at joint 2 and 345 lb uplift at joint 7.
5) Girder carries hip end with 3-0-0 end setback.
6) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63.0lb down and 32.4lb up at 16-0-0, and 63.0lb down and 32.4lb up at 3-0-0 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-6=-64(F=-10), 6-8=-54, 2-12=-30, 9-12=-35(F=-5), 7-9=-30
Concentrated Loads (lb)
Vert: 12=-63(F) 9=-63(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#02020503I
L101301	T18	HIP	1	2	Job Reference (optional)	

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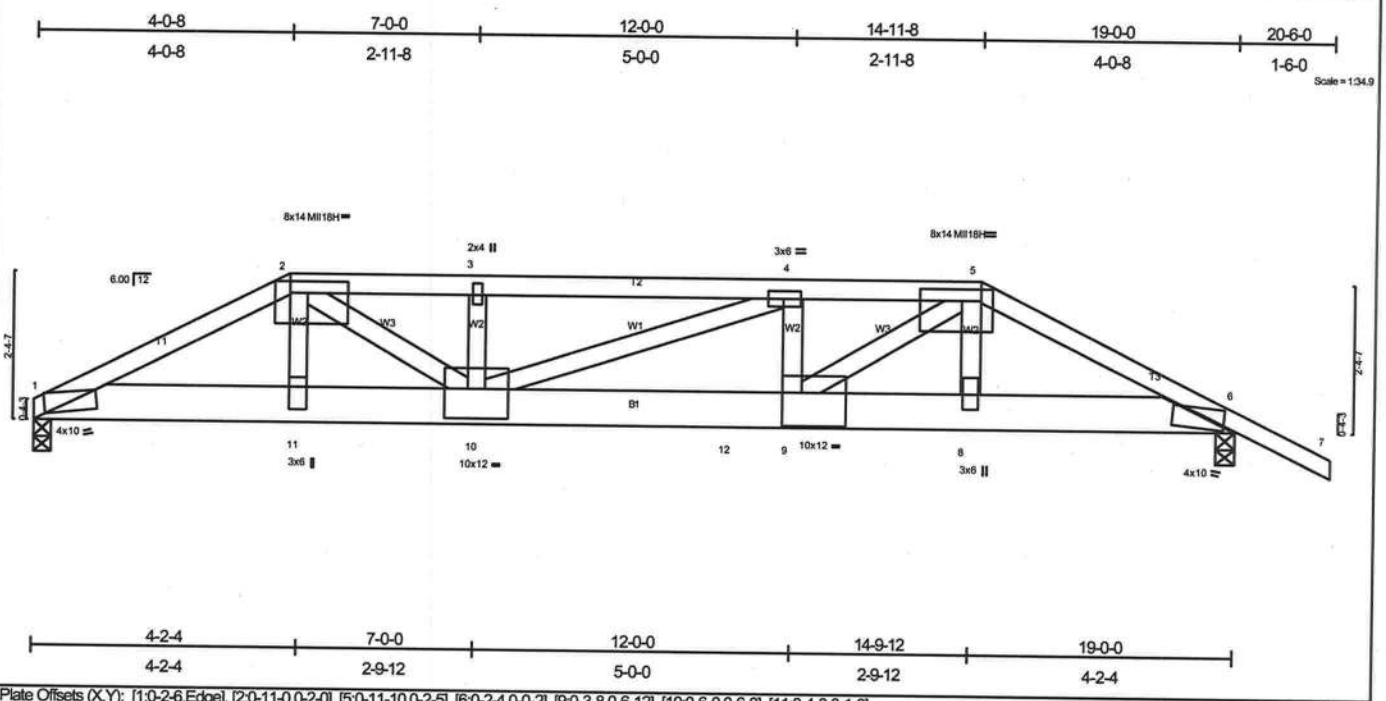

Plate Offsets (X,Y): [1:0-2-6,Edge], [2:0-11-0,0-2-0], [5:0-11-10,0-2-5], [6:0-2-4,0-0-2], [9:0-3-8,0-6-12], [10:0-6-0,0-6-0], [11:0-4-8,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.98	Vert(LL)	-0.34	9-10	>651	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.72	Vert(TL)	-0.48	9-10	>465	180	MII18H	195/188
BCLL	10.0	Rep Stress Incr	NO	WB	0.77	Horz(TL)	0.07	6	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 232 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3 2 X 4 SYP No.2D, W3 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=7587/0-3-8, 6=5293/0-3-8
Max Horz 1=-94(load case 5)
Max Uplift1=-2787(load case 3), 6=-1942(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-13522/5005, 2-3=-16918/6316, 3-4=-16918/6316, 4-5=-17315/6470, 5-6=-11124/4100, 6-7=0/41
BOT CHORD 1-11=-4463/12115, 10-11=-4536/12319, 10-12=-6397/17315, 9-12=-6397/17315, 8-9=-3619/9931, 6-8=-3627/9942
WEBS 2-11=-966/2684, 5-8=-142/120, 2-10=-2096/5532, 3-10=-130/127, 4-9=-30/92, 4-10=-460/178, 5-9=-3353/8832

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 3 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc, Except member 4-9 2 X 4 - 2 rows at 0-4-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MII20 plates unless otherwise indicated.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2787 lb uplift at joint 1 and 1942 lb uplift at joint 6.
8) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 3375.0lb down and 1274.3lb up at 12-0-0 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54, 2-5=-54, 5-7=-54, 1-12=-753(F=-723), 6-12=-30
Concentrated Loads (lb)
Vert: 9=-3375(F)

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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T19	HIP	1	1	Job Reference (optional)

Dwg.#020205032

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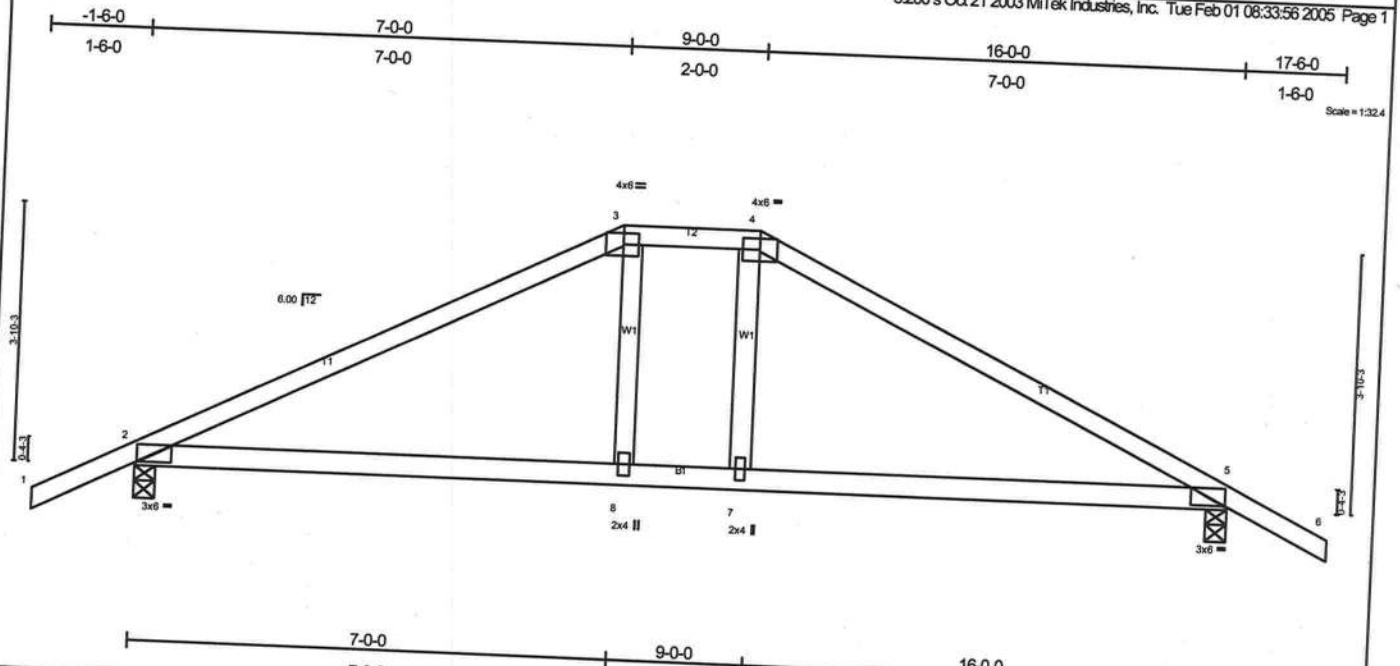

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.39	Vert(LL)	-0.17	5-7	>999	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.65	Vert(TL)	-0.22	5-7	>846	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.04	5	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 65 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 3-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-2-6 oc bracing.

REACTIONS (lb/size) 2=1381/0-3-8, 5=1381/0-3-8
Max Horz 2=-103(load case 5)
Max Uplift 2=-815(load case 4), 5=-815(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-2333/1216, 3-4=-2031/1137, 4-5=-2333/1216, 5-6=0/35
BOT CHORD 2-8=-1017/2002, 7-8=-1030/2031, 5-7=-1013/2002
WEBS 3-8=-419/796, 4-7=-419/796

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 815 lb uplift at joint 2 and 815 lb uplift at joint 5.
5) Girder carries hip end with 7-0-0 end setback
6) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539.0lb down and 277.5lb up at 9-0-0, and 539.0lb down and 277.5lb up at 7-0-0 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-8=-30, 7-8=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-539(F) 7=-539(F)

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Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	T20	COMMON	2	1	Job Reference (optional)

Dwg.#020205033

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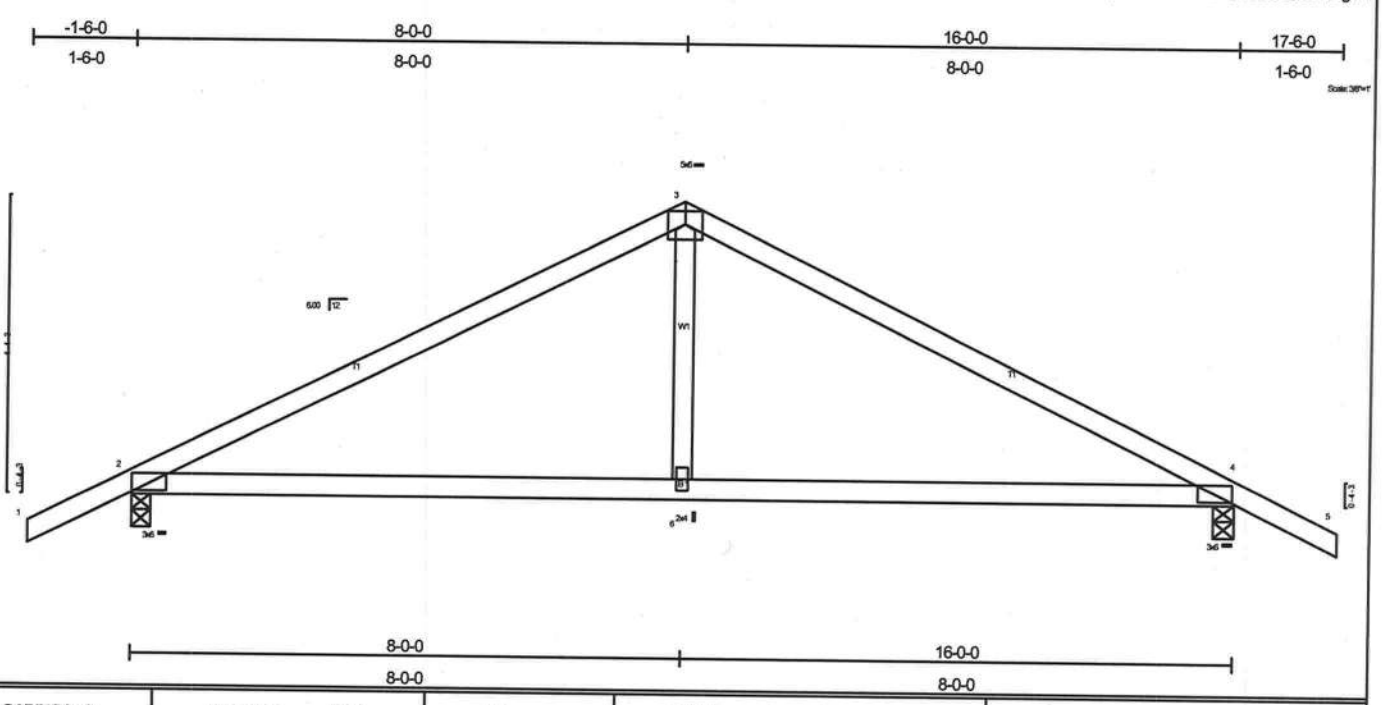


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	0.26	2-6	>720	240	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.52	Vert(TL)	0.23	2-6	>808	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.02	4	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 61 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-9-10 oc bracing.

REACTIONS (lb/size) 2=749/0-3-8, 4=749/0-3-8
Max Horz2=-112(load case 6)
Max Uplift2=-494(load case 5), 4=-494(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=-949/1100, 3-4=-949/1100, 4-5=0/35
BOT CHORD 2-6=-804/770, 4-6=-804/770
WEBS 3-6=-516/305

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 494 lb uplift at joint 2 and 494 lb uplift at joint 4.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	V01	VALLEY	1	1	Job Reference (optional)

Dwg.#020205034

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:57 2005 Page 1

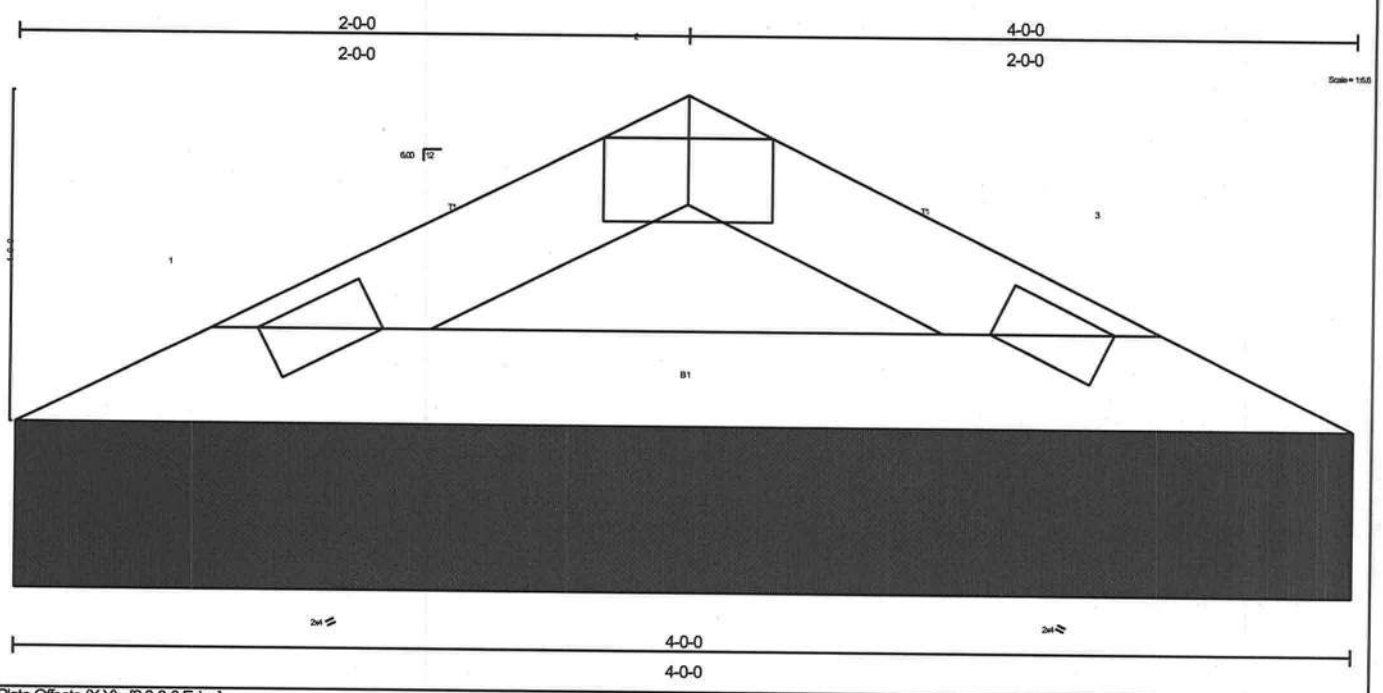


Plate Offsets (X,Y): [2:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.04	Vert(LL)	n/a	-	n/a	999	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 11 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D

BRACING
TOP CHORD Sheathed or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=116/4-0-0, 3=116/4-0-0
Max Horz 1=-14(load case 6)
Max Uplift1=-35(load case 5), 3=-35(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-97/119, 2-3=-97/119
BOT CHORD 1-3=-76/75

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1 and 35 lb uplift at joint 3.

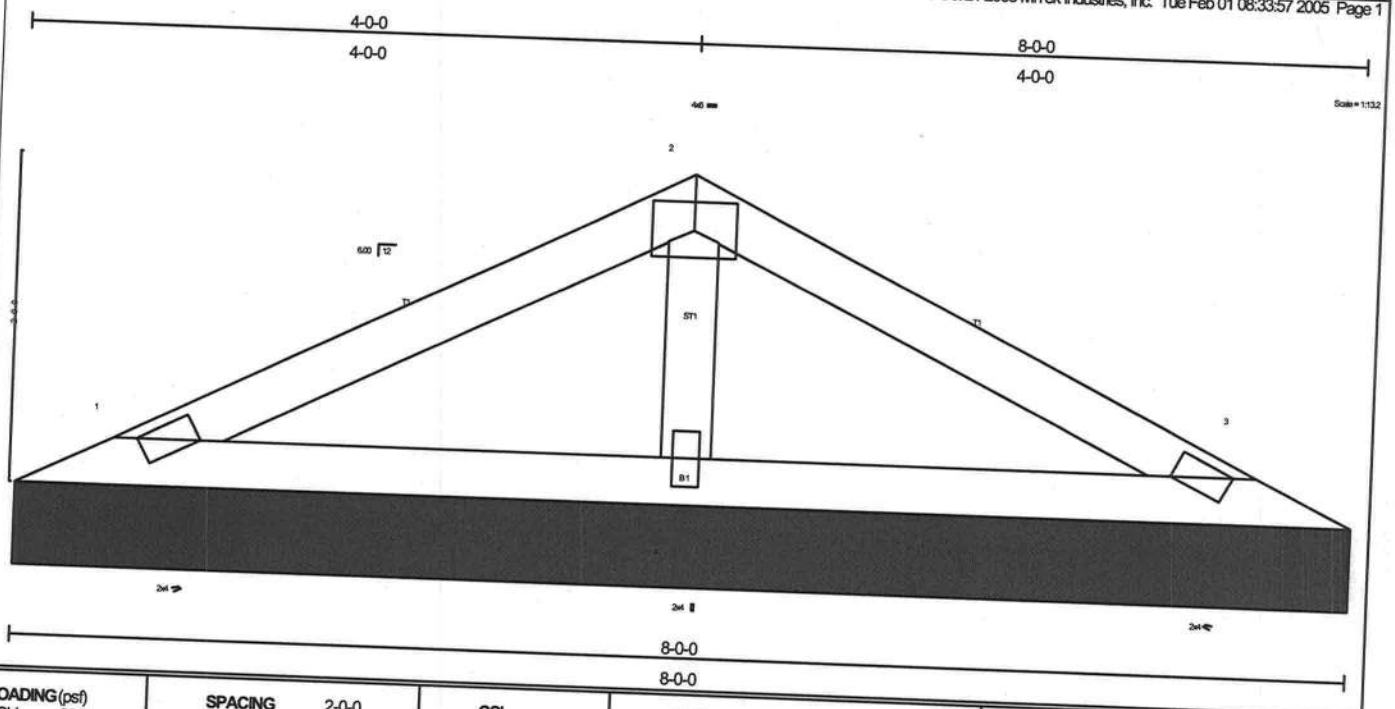
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	V02	VALLEY	1	1	Job Reference (optional)

Dwg.#020205035

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:57 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	n/a	-	n/a	999	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 25 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=140/8-0-0, 3=140/8-0-0, 4=289/8-0-0
Max Horz 1=34(load case 5)
Max Uplift 1=-60(load case 5), 3=-66(load case 6), 4=-53(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-56/60, 2-3=-56/60
BOT CHORD 1-4=-2/21, 3-4=-2/21
WEBS 2-4=-162/178

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 60 lb uplift at joint 1, 66 lb uplift at joint 3 and 53 lb uplift at joint 4.

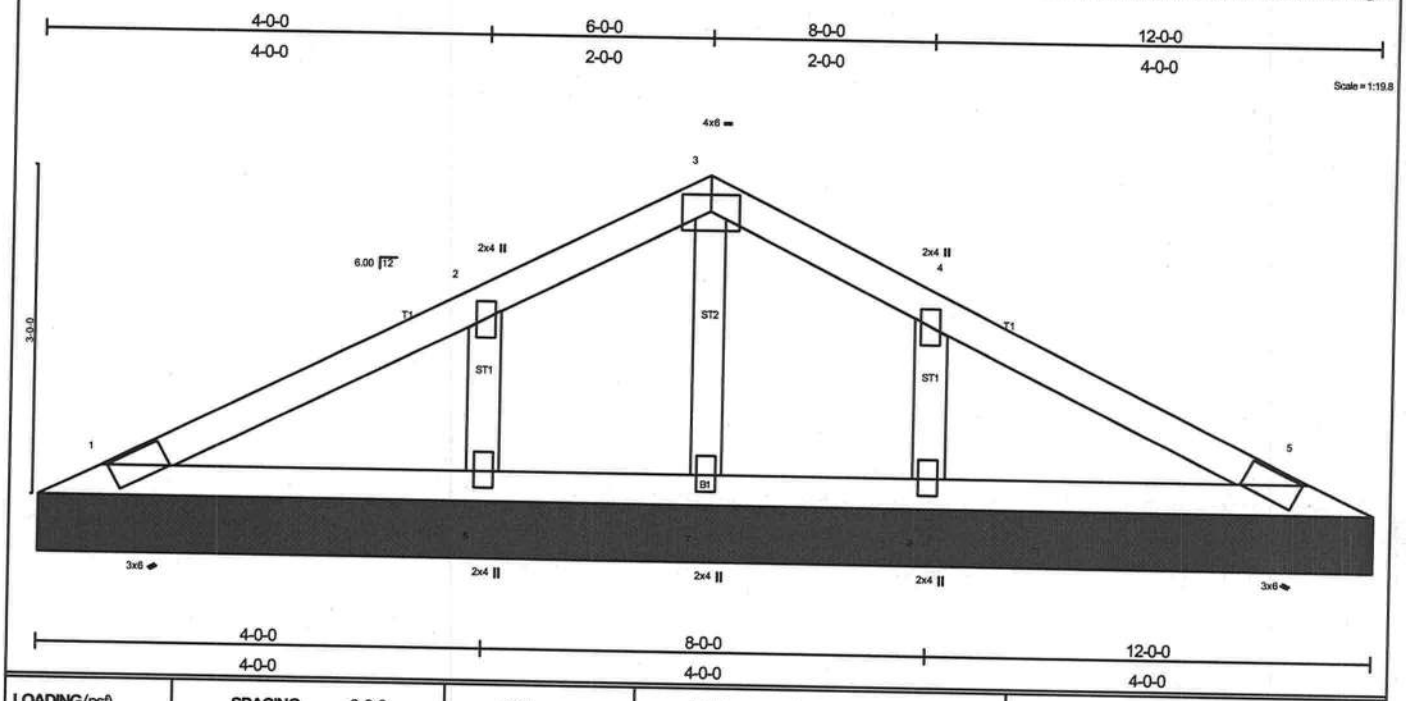
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	V03	VALLEY	1	1	Job Reference (optional)

Dwg.#020205036

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:58 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	n/a	-	n/a	999	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 44 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
OTHERS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=118/12-0-0, 5=118/12-0-0, 7=69/12-0-0, 6=299/12-0-0, 8=299/12-0-0
Max Horz 1=-54(load case 6)
Max Uplift 1=-28(load case 6), 5=-36(load case 6), 6=-152(load case 6), 8=-152(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-43/42, 2-3=-42/115, 3-4=-42/115, 4-5=-43/42
BOT CHORD 1-8=0/57, 7-8=0/57, 6-7=0/57, 5-6=0/57
WEBS 3-7=-52/0, 4-6=-182/233, 2-8=-182/233

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 1, 36 lb uplift at joint 5, 152 lb uplift at joint 6 and 152 lb uplift at joint 8.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	V04	VALLEY	1	1	Job Reference (optional)

Dwg.#020205037

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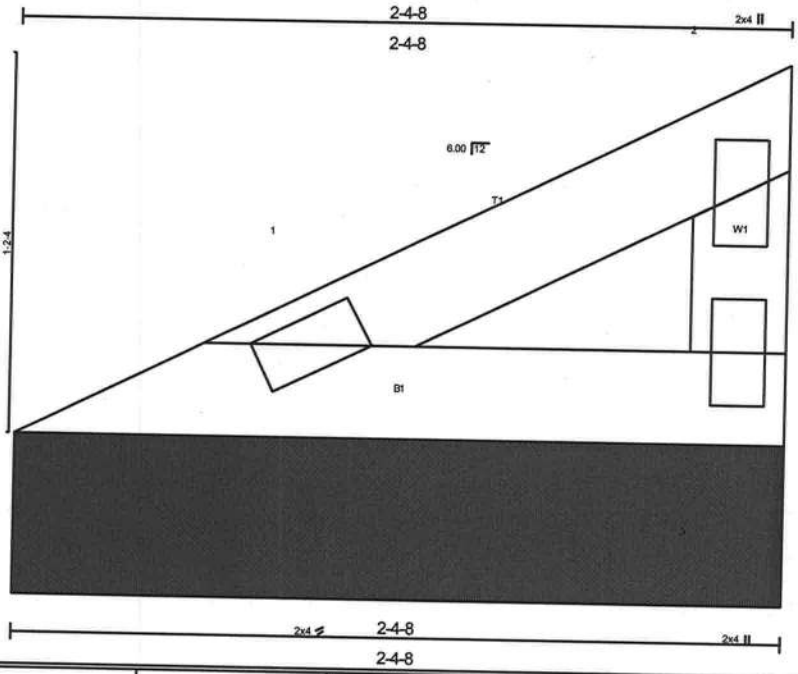


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.03	Vert(LL)	n/a	-	n/a	999	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 2-4-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=54/2-4-8, 3=54/2-4-8
Max Horz 1=37(load case 5)
Max Uplift1=-9(load case 5), 3=-32(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-42/15, 2-3=-35/70
BOT CHORD 1-3=0/0

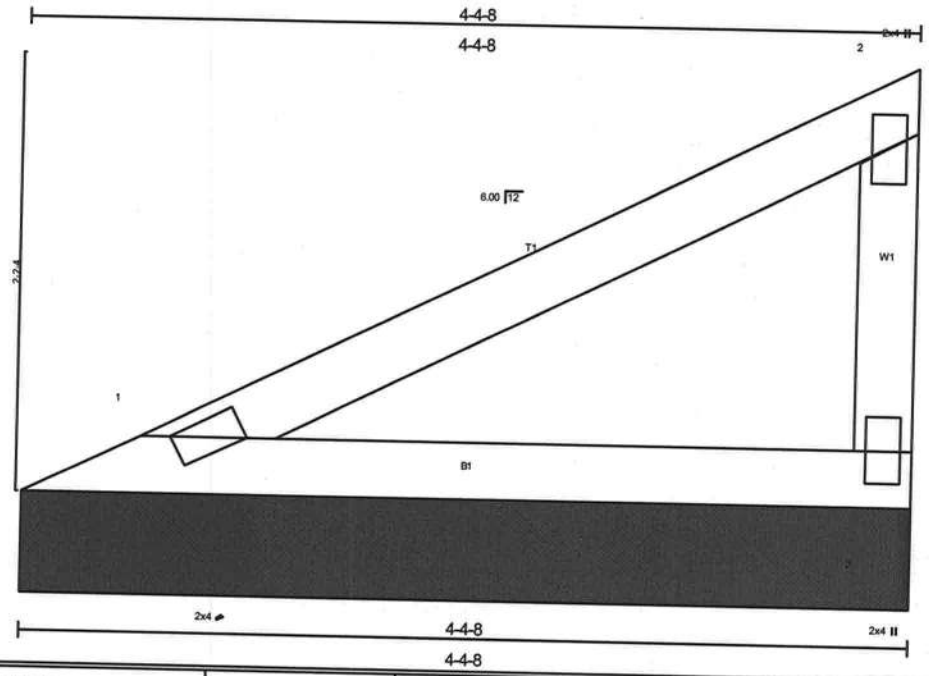
NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Gable requires continuous bottom chord bearing.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1 and 32 lb uplift at joint 3.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205038
L101301	V05	VALLEY	1	1	Job Reference (optional)	

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:59 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.20	Vert(LL)	n/a	-	n/a	999	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00		n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 4-4-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=152/4-4-8, 3=152/4-4-8
Max Horz 1=83(load case 5)
Max Uplift1=-36(load case 5), 3=-78(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-90/35, 2-3=-98/175
BOT CHORD 1-3=0/0

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Gable requires continuous bottom chord bearing.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 1 and 78 lb uplift at joint 3.

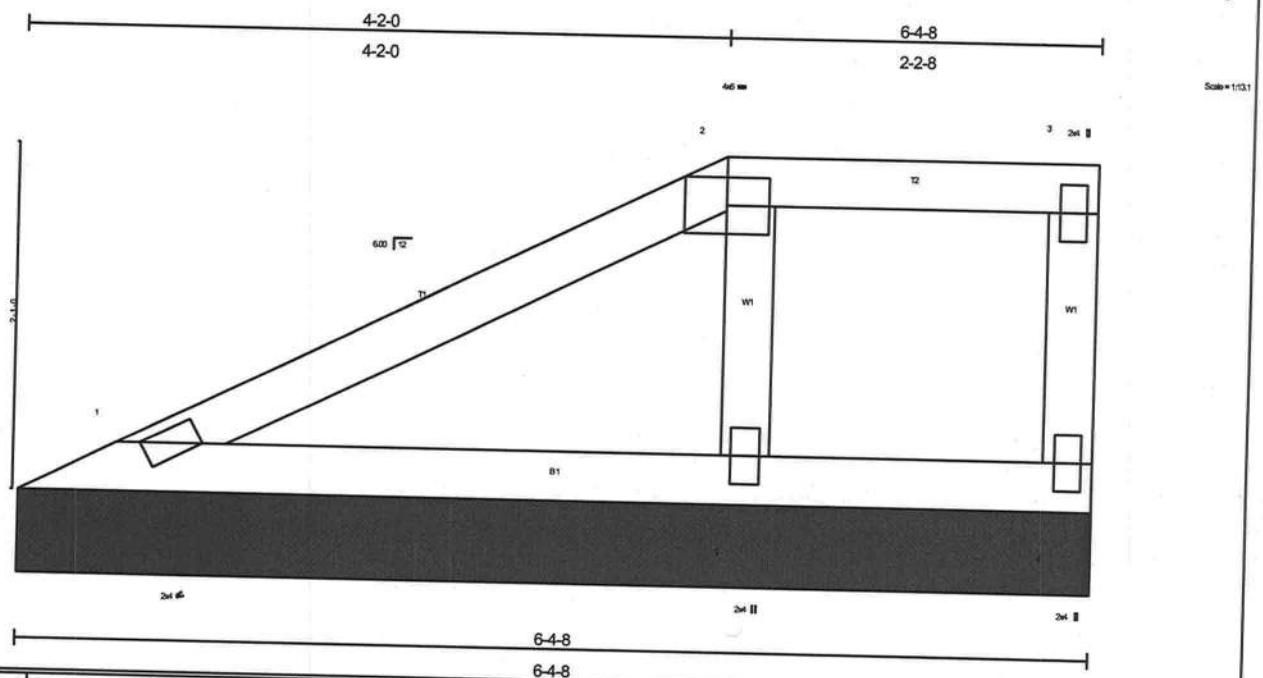
LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE
L101301	V06	VALLEY	1	1	Job Reference (optional)

Dwg.#020205039

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:59 2005 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.18	Vert(LL)	n/a	-	n/a	999	MII20	249/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	n/a	-	n/a	999		
BCLL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	5.0	Code FBC2001/ANSI95		(Matrix)							Weight: 23 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=147/6-4-8, 4=64/6-4-8, 5=260/6-4-8
Max Horz 1=82(load case 5)
Max Uplift1=-41(load case 5), 4=-37(load case 3), 5=-79(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-67/21, 2-3=-0/0, 3-4=-56/63
BOT CHORD 1-5=-18/12, 4-5=0/0
WEBS 2-5=-146/220

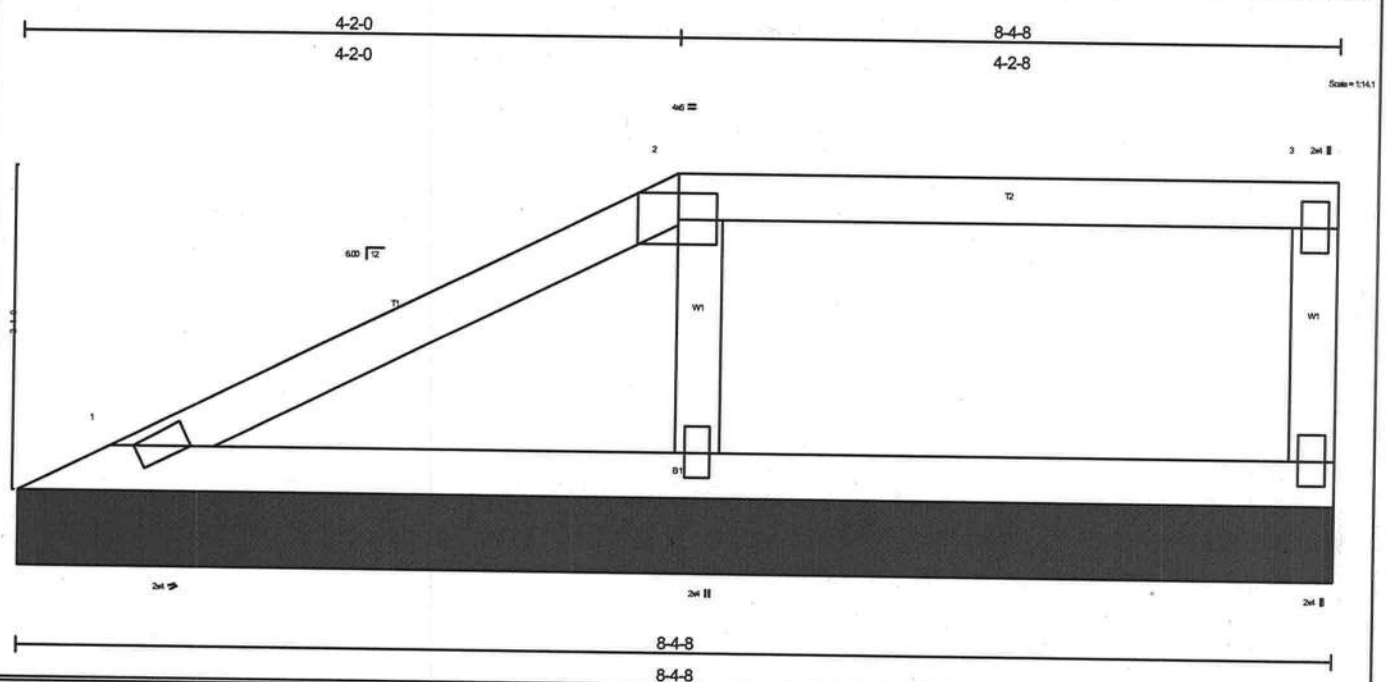
NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 1, 37 lb uplift at joint 4 and 79 lb uplift at joint 5.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Job	Truss	Truss Type	Qty	Ply	MIKE ROBERTS - SPEC HOUSE	Dwg.#020205040
L101301	V07	VALLEY	1	1	Job Reference (optional)	

5.200 s Oct 21 2003 MiTek Industries, Inc. Tue Feb 01 08:33:59 2005 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.18	Vert(LL) n/a - n/a 999	MII20 249/190
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(TL) n/a - n/a 999	
BCLL 10.0	Rep Stress Incr YES	WB 0.06	Horz(TL) -0.00 4 n/a n/a	Weight: 29 lb
BCDL 5.0	Code FBC2001/ANSI95	(Matrix)		

LUMBER
TOP CHORD 2 X 4 SYP No.2D
BOT CHORD 2 X 4 SYP No.2D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=145/8-4-8, 4=155/8-4-8, 5=340/8-4-8
Max Horz 1=82(load case 5)
Max Uplift1=-43(load case 5), 4=-68(load case 3), 5=-91(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-57/16, 2-3=-0/0, 3-4=-110/112
BOT CHORD 1-5=-21/16, 4-5=0/0
WEBS 2-5=-198/254

NOTES
1) Wind: ASCE 7-98; 110mph (3-second gust); h=14ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) Gable requires continuous bottom chord bearing.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 1, 68 lb uplift at joint 4 and 91 lb uplift at joint 5.

LOAD CASE(S) Standard

FEBRUARY 2, 2005, TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Mack Robinson/Mike Roberts

1. New Truss Package for Gable Trusses.
2. Truss Company needs to supply an up to date Certified Truss Package, including any changes in the design for gable ends.
3. Truss Company needs to state in writing they have reviewed the trusses and that they are in compliance with the Florida Building Code.
4. Need updated elevation plans, to show gables.
5. Need updated wind Load plan.

William A. Baker P.E.
Florida License No. 30565
109 SE Jenese Way, Lake City, Florida 32025-1837
Telephone No. / Fax (386) 752-8523

Inspection Of Wood In Trusses Resulting From Outdoor Storage
(Owner - Mike Roberts' - Spec House)

A Review of the state of the wood in trusses that have been stored on the ground and exposed to the weather for close to a year.

The owner, Mike Roberts, took delivery of trusses, for a house he was about to build, in early 2005. The trusses were laid out in the yard with one of the larger trusses directly on the ground and the rest stacked on top of it, or otherwise off the ground. Now, one year later, the roof is about to be constructed and the County Building Inspection Department is questioning the state of the wood in the trusses after that length of exposure.

On Saturday, January 14, with Mike Roberts and a helper, we examined each truss by turning them over in order to get a close look at both sides. Mr. Roberts had previously washed the trusses with Clorox to get rid of any mildew that had started to form. The wood generally looked quite good with a slight graying on some sides due to weathering. The bottom side of the truss, #**TO9**, that lay directly on the ground, showed obvious deterioration. The next truss, #**TO10**, that lay on top of truss, #**TO9**, showed evidence of termite intrusion in the center of the bottom chord and in the top chord close to the peak. These two trusses are to be replaced. There was no evidence of deterioration of the wood in the remaining trusses and truss parts.

There have been two minor changes in the roof design since the trusses were first delivered. Two hip roofs have been changed to gable ends. Also Mr. Roberts does not expect to use the valley trusses supplied with the original design. It is requested that the truss fabricator supply an up-to-date set of certified truss drawings with plan.

It is recommended that the designer/fabricator review the trusses carefully as there are splice plates that need to be reattached, or re-secured, and there is at least one strut that needs to be spliced.

Signed - W. A. Baker Jan. 18, 2006

William A. Baker

Cc: Mike Roberts, 657 SW Catherine Lane, Lake City, Florida 32025
Columbia County Building Dept., 135 NE Hernando Ave. B-21, Lake City 32055
Mr. Chris Riser, Builders First Source, 6550 Roosevelt Blvd., Jacksonville, Fl 32244
Thomas E. Miller PE, 16105 North Florida Ave., Suite B, Lutz, Fl 33549
Mark Disosway P.E., POB 868, Lake City, Florida 32056

22831

BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

Doyle Crews

Property Apprasier

Dear Mr. Crews:

The house located on Lot 18, Saddle Ridge Subdivision was not completed as of January 1, 2007. If you have any questions, please do not hesitate to contact me.

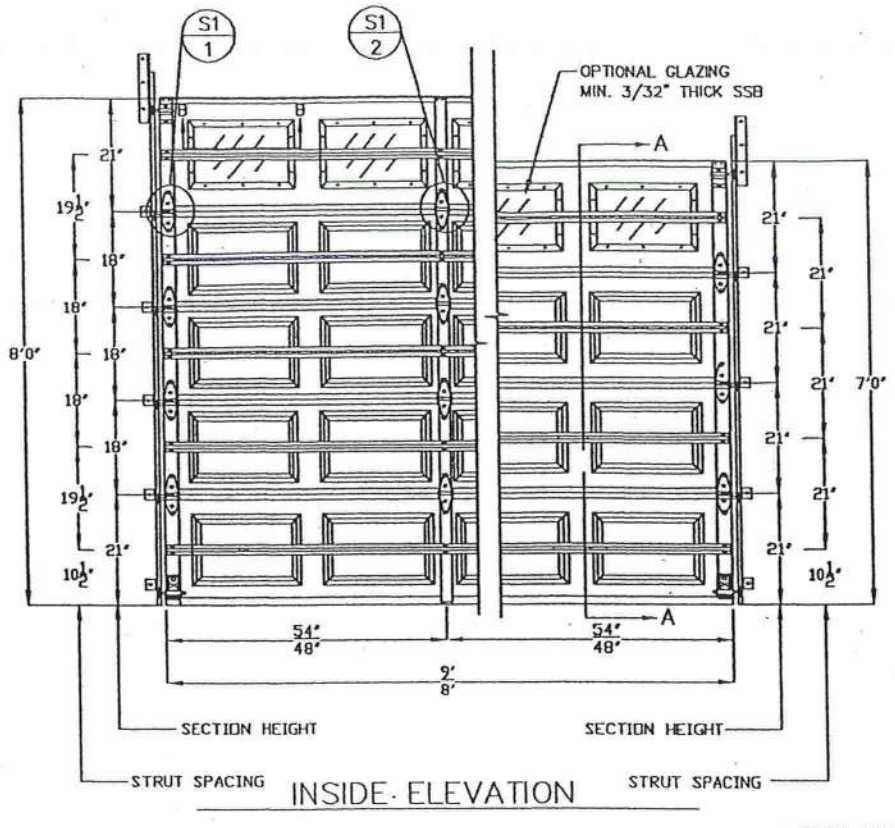
Harry Dicks

Harry Dicks

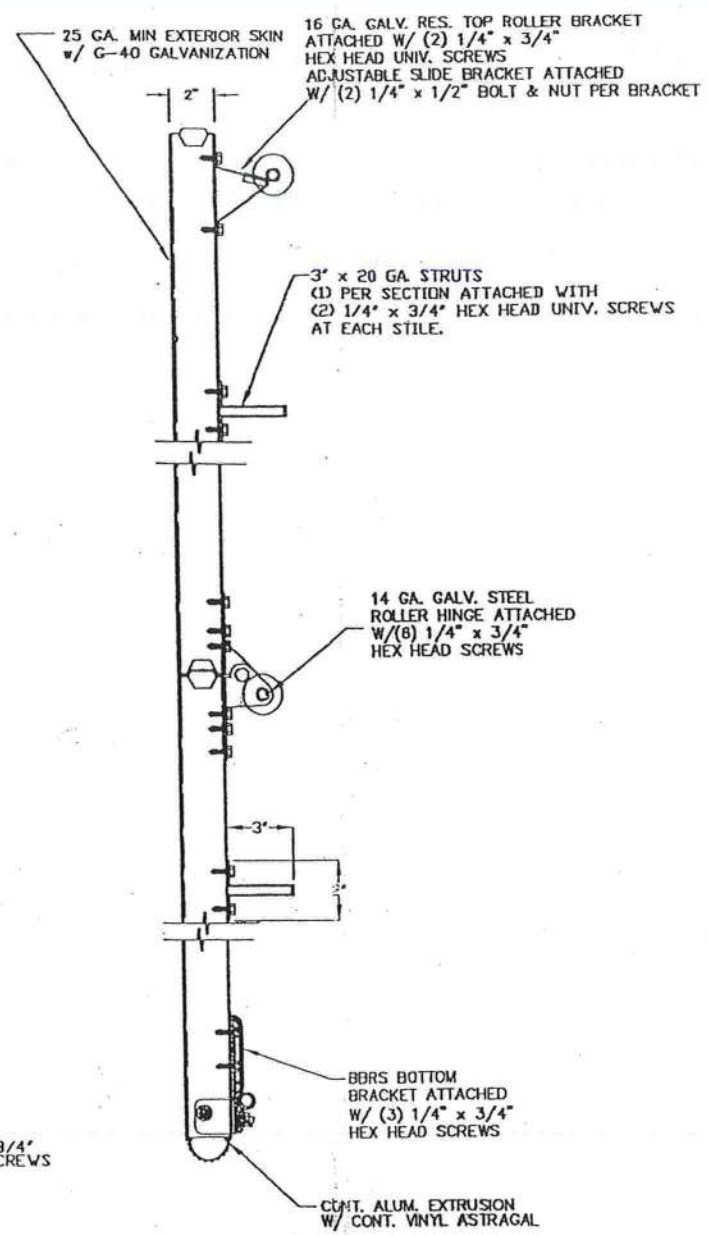
Building Inspector

HD/gt

MASTER 2



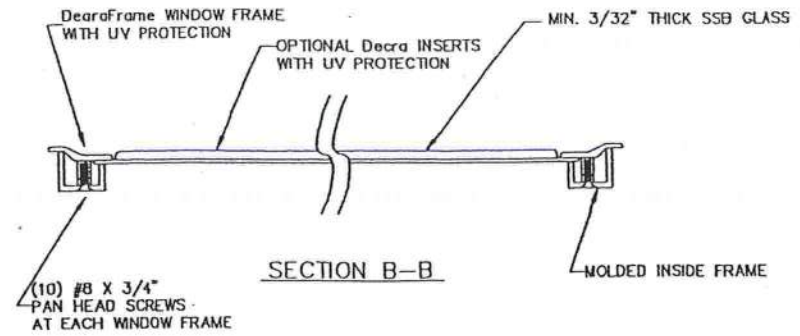
INSIDE ELEVATION



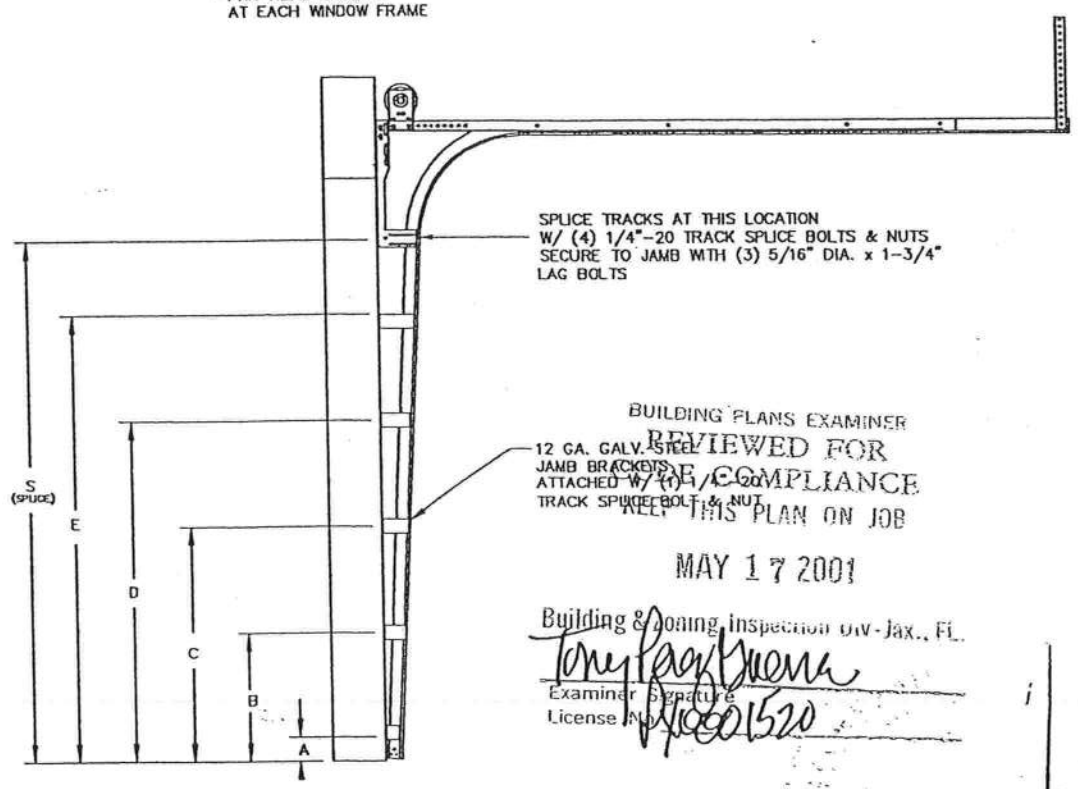
SECTION A–A (SIDE VIEW)

GLAZING OPTION CROSS SECTION

TEST No. SBC-580-011 ON OCTOBER 12, 1995 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +48.4 PSF AND -54.7 PSF. BY COMPARISION, FOUR (4) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 9' X 7' AND 9' X 8' MODEL 600 AND 950 DOORS.



SECTION B–B



TRACK CONFIGURATION FOR 6'6" UP TO 8' TALL DOORS

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"

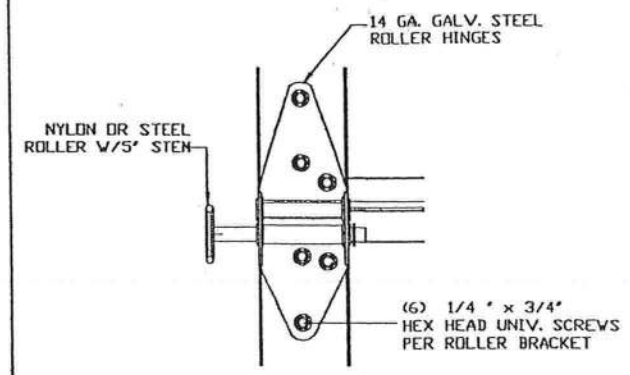
SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 25 GA. (.019") MIN. ROLL FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION.
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   - A. BASIC WIND SPEED OF 110 MPH
   - B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   - C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   - D. USE FACTOR OF 1.0
   - E. EXPOSURE RATING OF C

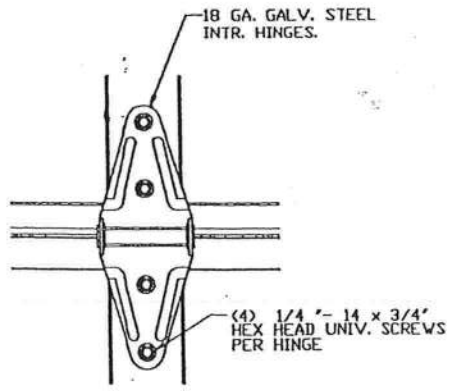
BUILDING PLANS EXAMINER
REVIEWED FOR
CODE COMPLIANCE
ALL THIS PLAN ON JOB

MAY 17 2001

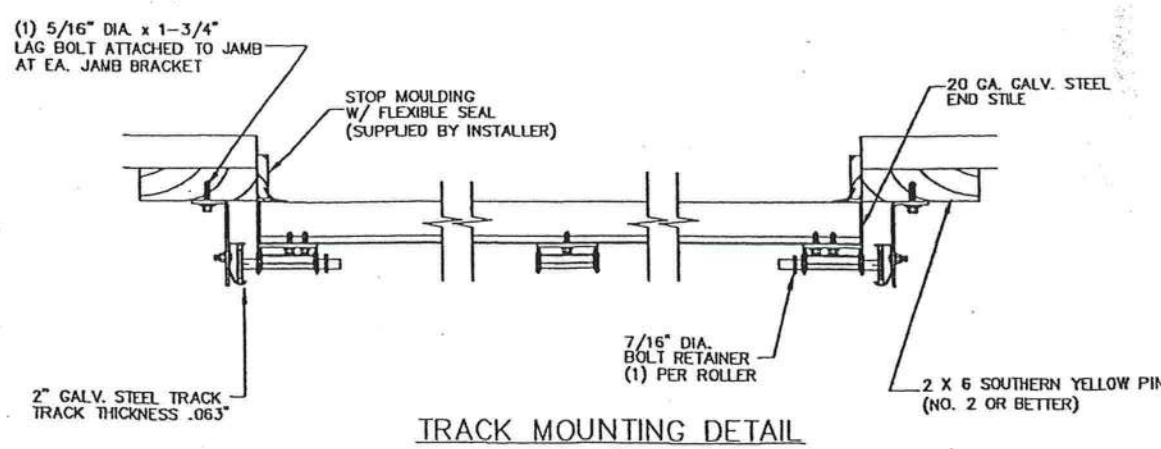
Building & Zoning Inspection Div - Jax., FL.
Examiner Signature & License No. 0001520



TYP. ROLLER BRACKET (S1 1)
N.T.S.



TYP. HINGE CONNECTION (S1 2)
N.T.S.



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 24" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

MAX SIZE	9' x 8'
DESIGN LOADS	+28.7 PSF -31.3 PSF
TEST LOADS	+43.0 PSF -46.9 PSF

REV	DESCRIPTION OF REVISIONS	DATE	BY

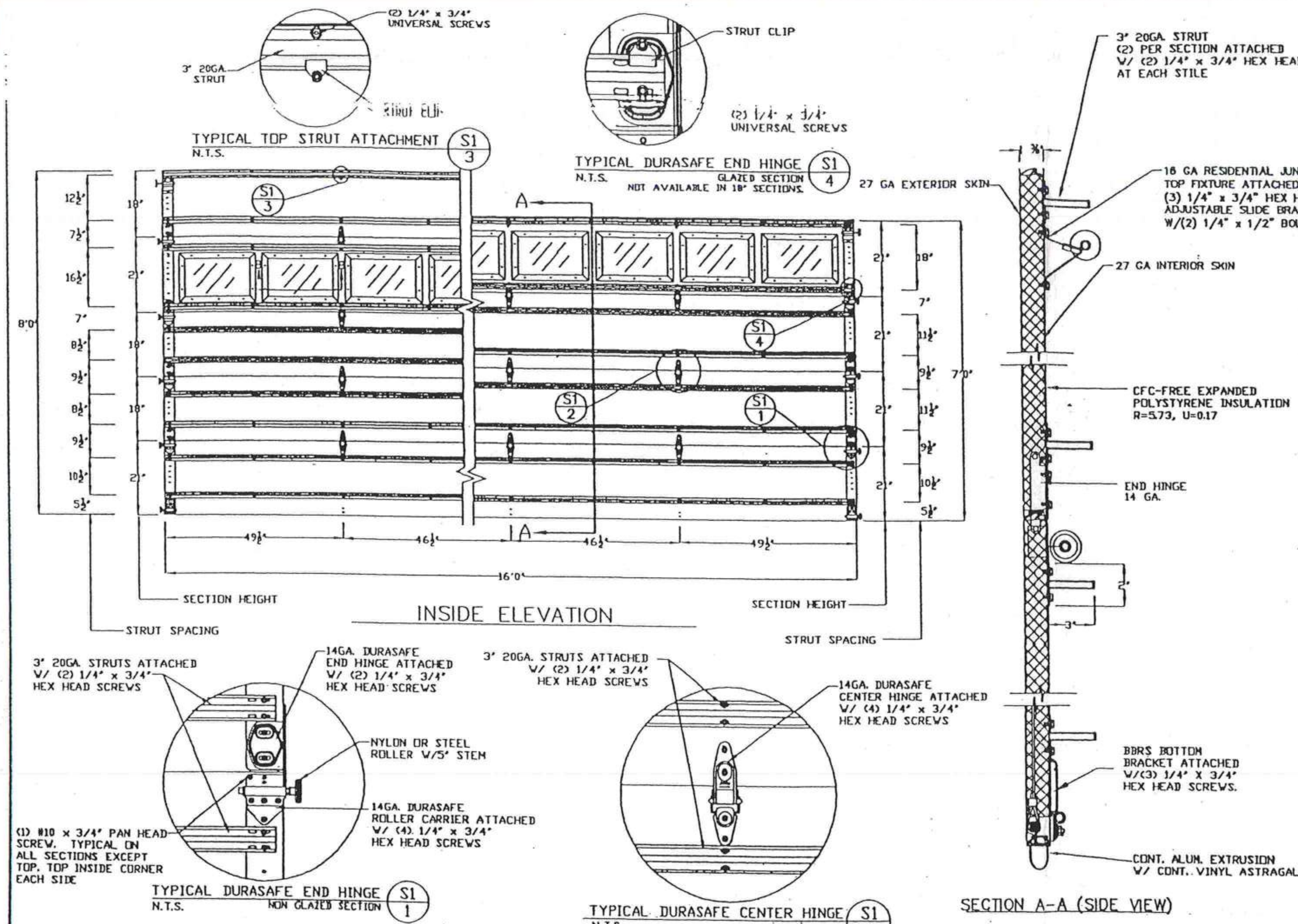
Amarr GARAGE DOORS
5931 GRASSY CREEK BLVD. WINSTON-SALEM, N.C. 27105

MODEL #600 STRATFORD
MODEL #950 HERITAGE III

SIZE	DRAWN BY	TLJ	DATE	04/9/01	DRAWING NUMBER
B	CHECKED BY		DATE		SBC-580-007-J
SCALE: NOT TO SCALE					SHEET 1 OF 1

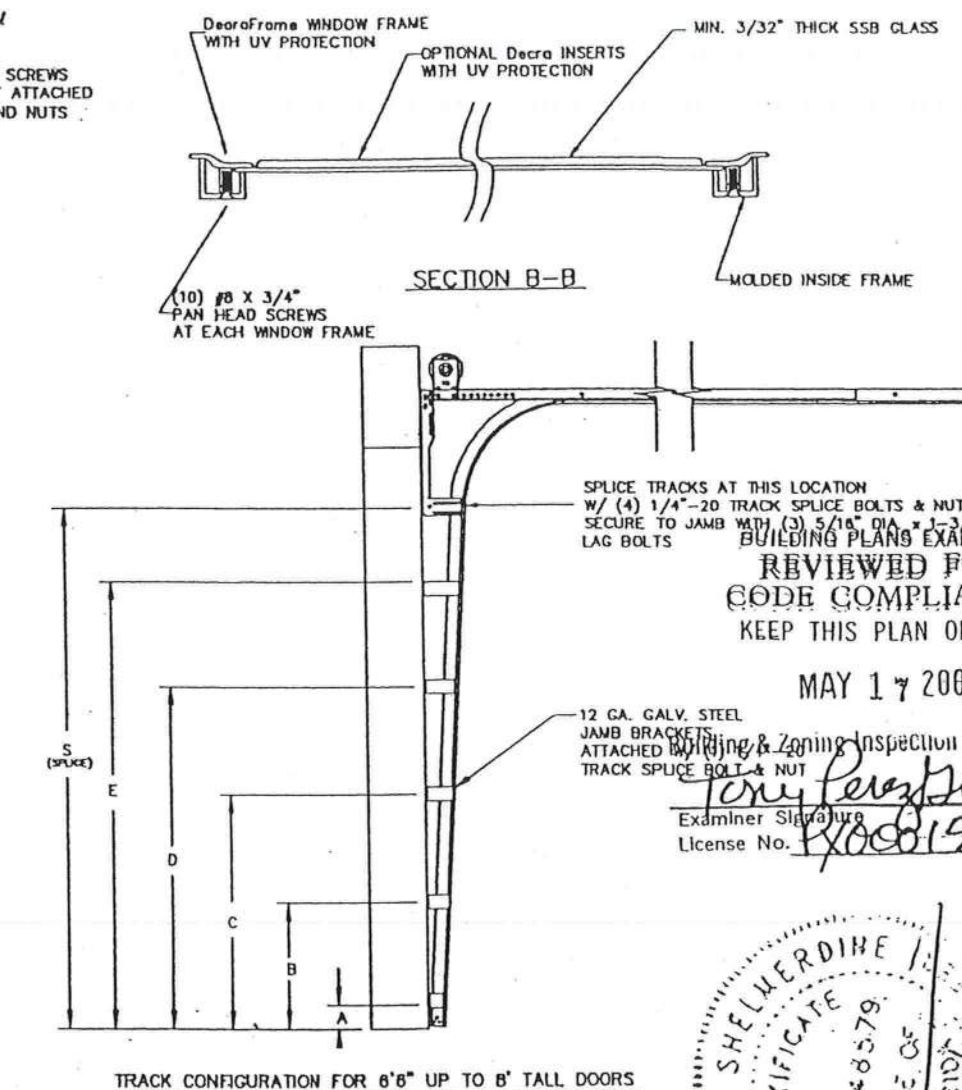
4/19/01

MASTER 1



GLAZING OPTION CROSS SECTION

TEST No. SBC 580 020 ON MAY 31, 2000 INCLUDED GLASS WINDOWS IN THE DOOR BEING USED. THE TEST PRESSURES WERE +49.5 PSF AND -51.9 PSF. BY COMPARISON, EIGHT (8) WINDOWS MAY BE INSTALLED IN (1) ONE SECTION OF THE 16' X 7' AND 16' X 8' MODEL 1500-D DOORS.

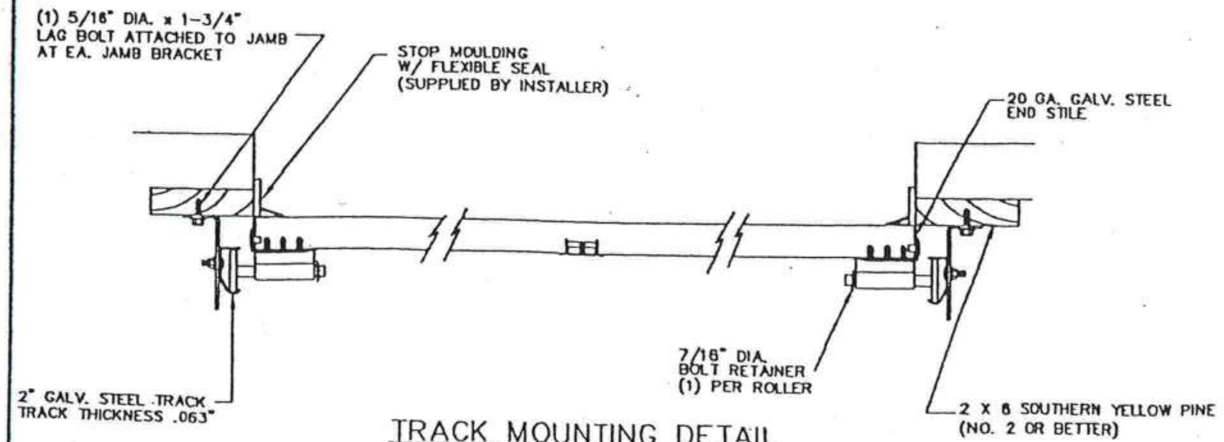


BUILDING PLANS EXAMINER
REVIEWED FOR CODE COMPLIANCE
KEEP THIS PLAN ON JOB
MAY 17 2001
Building & Zoning Inspection Div.-Jax., FL.
Examiner Signature
License No. PX0001520

THOMAS L. SHELMERDINE
CERTIFICATE NO. 48579
STATE OF FLORIDA

JAMB BRACKET LOCATIONS

	A	B	C	D	E	S
6'-6"	4"	21-1/2"	39"	57"		70"
7'-0"	4"	21-1/2"	42"	63"		76"
7'-6"	4"	18-1/2"	36"	54"	72"	82"
8'-0"	4"	21-1/2"	39"	57"	75"	88"



TRACK MOUNTING DETAIL

WOOD JAMB ATTACHMENT TO STRUCTURE

RATED FOR 110 MPH FASTEST-MILE BASIC WIND SPEEDS

VERTICAL JAMB ATTACHMENT TO WOOD FRAME STRUCTURE
5/16" X 3" LAG SCREWS STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO 2,000 PSI CONCRETE
HILTI KWIK BOLT 3/8" X 4" STARTING 6" FROM ENDS THEN 24" O.C.
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET RED HEAD 3/8" X 3" STARTING 6" FROM ENDS THEN 24" O.C.

VERTICAL JAMB ATTACHMENT TO C-90 BLOCK
HILTI SLEEVE ANCHOR 3/8" X 2-3/4" STARTING 6" FROM ENDS THEN 24" O.C.
ITW/RAMSET TAPCON 1/4" X 2-3/4" STARTING 6" FROM ENDS, USE PAIRS OF FASTENERS (3" APART) AT 16" O.C.

*LAGS AND BOLTS CAN BE COUNTERSUNK TO PROVIDE A FLUSH MOUNTING SURFACE.
*PREPARATION OF WOOD JAMBS BY OTHERS

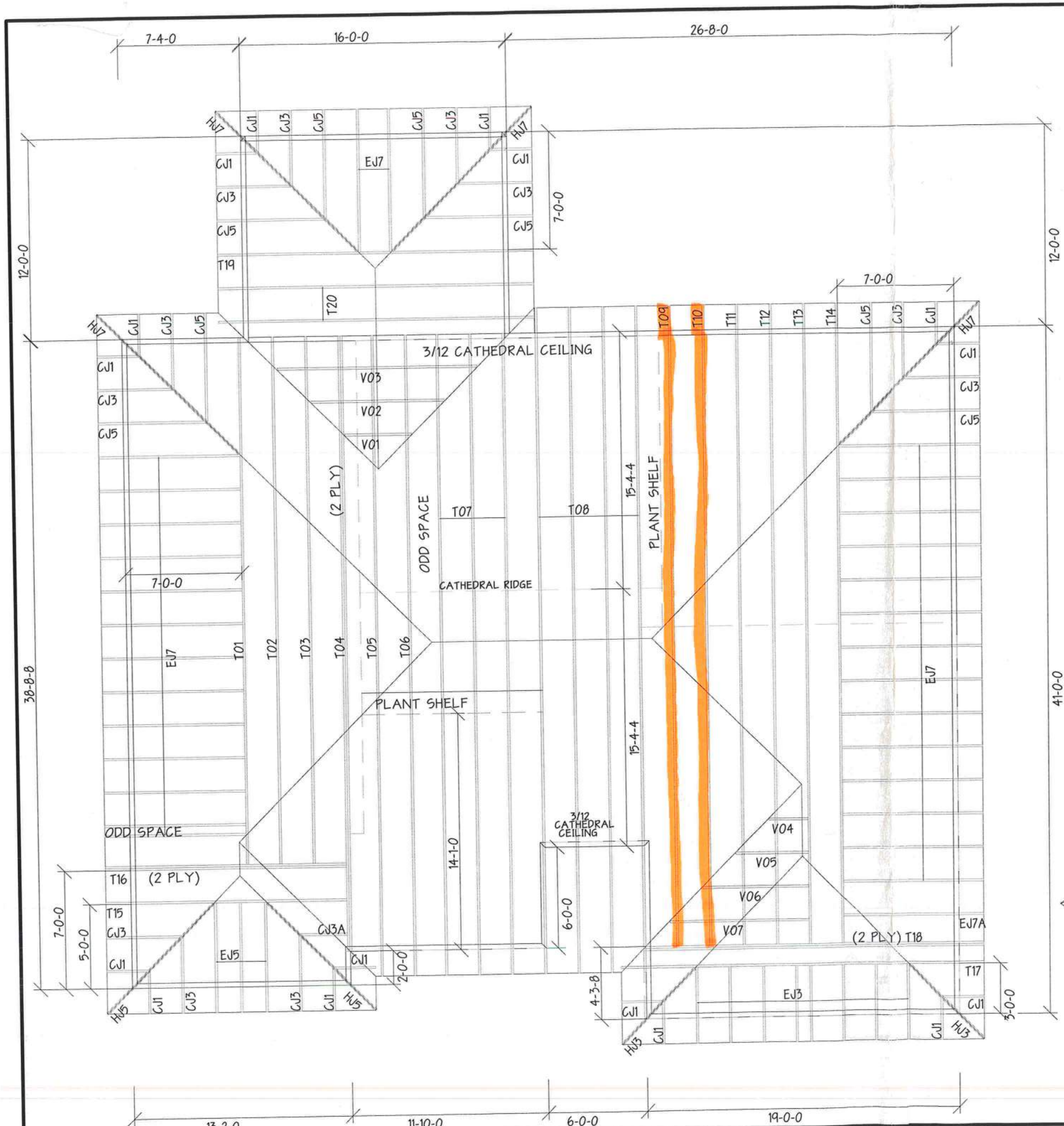
SPECIFICATIONS AND NOTES

1. DOORS AND HARDWARE WILL BE DESIGNED, MANUFACTURED AND INSTALLED WITH STANDARDS AS SET FORTH BY DASMA.
2. DOOR SECTIONS SHALL BE 27 GA. MIN. (.018") INTERIOR AND EXTERIOR ROLLED FORMED LIGHT COMMERCIAL QUALITY, G-40 GALVANIZATION
3. DOORS UPTO 7'0" HIGH CONSIST OF (4) SECTIONS AS SHOWN.
4. DOORS UPTO 8'0" HIGH CONSIST OF (5) SECTIONS AS SHOWN.
5. SUPPORTING STRUCTURAL ELEMENTS SHALL BE DESIGNED BY A REGISTRED PROFESSIONAL ENGINEER FOR WIND LOADS INDICATED ON THIS DRAWING IN ADDITION TO OTHER LOADINGS.
6. THE METHOD OF TESTING WAS IN SUBSTANTIAL CONFORMANCE WITH THE PROCEDURE DESCRIBED IN ASTM E330-90, AND THE SOUTHERN BUILDING CODE SECTION 1606 WIND LOAD DESIGN CRITERIA THE PRESSURES SHOWN ON THE DRAWINGS WERE CALCULATED USING THE FOLLOWING PARAMETERS:
   A. BASIC WIND SPEED OF 110 MPH
   B. DOOR CAN BE INSTALLED WITH 5 FEET OF DOORS WIDTH INSIDE THE EDGE STRIP.
   C. 15' MEAN ROOF HEIGHT AT ANY SLOPE
   D. USE FACTOR OF 1.0
   E. EXPOSURE RATING OF C

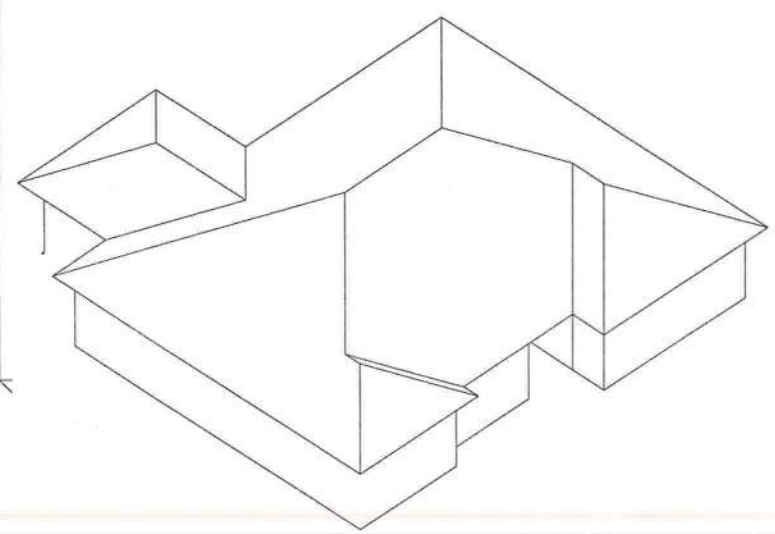
MAX SIZE 16' x 8'
DESIGN LOADS +29.5 PSF -29.5 PSF
TEST LOADS +45.3 PSF -45.3 PSF

REV | DESCRIPTION OF REVISIONS | DATE | BY

Amarr
MODEL #1500 WeatherGuard
SIZE B | DRAWN BY DJ | DATE 04/18/00 | DRAWING NUMBER SBC-580-019-J
CHECKED BY | DATE
SCALE: NOT TO SCALE | SHEET 1 OF 1



HANGER SCHEDULE
8 HUS26
1 HGUS28-2 (T16)
2 HGUS26 (T01, T14)



BEARING HEIGHT SCHEDULE

9" PLATE

6/12
18" OH

NOTES:

1) REFER TO HIB 91 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING.) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HUS26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
8) BEAM/HEADER/LINTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date :_____

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville
PHONE: 904-772-6100 FAX: 904-772-1973

Lake City
PHONE: 904-755-6894 FAX: 904-755-7973

Sanford
PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER: MIKE ROBERTS
LEGAL ADDRESS: SPEC HOUSE
MODEL: REVISION:
SCALE: NTS
DATE: 1/22/05 DRAWN BY: J.P. JOB #: L101301

William A. Baker P.E.
Florida License No. 30565
109 SE Jenese Way, Lake City, Florida 32025-1837
Telephone No. / Fax (386) 752-8523

Inspection Of Wood In Trusses Resulting From Outdoor Storage
(Owner - Mike Roberts' - Spec House)

A Review of the state of the wood in trusses that have been stored on the ground and exposed to the weather for close to a year.

The owner, Mike Roberts, took delivery of trusses, for a house he was about to build, in early 2005. The trusses were laid out in the yard with one of the larger trusses directly on the ground and the rest stacked on top of it, or otherwise off the ground. Now, one year later, the roof is about to be constructed and the County Building Inspection Department is questioning the state of the wood in the trusses after that length of exposure.

On Saturday, January 14, with Mike Roberts and a helper, we examined each truss by turning them over in order to get a close look at both sides. Mr. Roberts had previously washed the trusses with Clorox to get rid of any mildew that had started to form. The wood generally looked quite good with a slight graying on some sides due to weathering. The bottom side of the truss, #**TO9**, that lay directly on the ground, showed obvious deterioration. The next truss, #**TO10**, that lay on top of truss, #**TO9**, showed evidence of termite intrusion in the center of the bottom chord and in the top chord close to the peak. These two trusses are to be replaced. There was no evidence of deterioration of the wood in the remaining trusses and truss parts.

There have been two minor changes in the roof design since the trusses were first delivered. Two hip roofs have been changed to gable ends. Also Mr. Roberts does not expect to use the valley trusses supplied with the original design. It is requested that the truss fabricator supply an up-to-date set of certified truss drawings with plan.

It is recommended that the designer/fabricator review the trusses carefully as there are splice plates that need to be reattached, or re-secured, and there is at least one strut that needs to be spliced.

Signed - W. A. Baker Jan. 18, 2006

William A. Baker

Cc: Mike Roberts, 657 SW Catherine Lane, Lake City, Florida 32025
Columbia County Building Dept., 135 NE Hernando Ave. B-21, Lake City 32055
Mr. Chris Riser, Builders First Source, 6550 Roosevelt Blvd., Jacksonville, Fl 32244
Thomas E. Miller PE, 16105 North Florida Ave., Suite B, Lutz, Fl 33549
Mark Disosway P.E., POB 868, Lake City, Florida 32056

TRUSS PROJECT
L 101301