Columbia County New Building Permit Application

Francisco State of the Part of
Zoning Official LW Date 9-18-19 Flood Zene V Date Received 9/18/19 By MG Permit # 3 67723
Zoning Official LW Date 9-18-19 Flood Zone X Land Use Aq Zoning A-3
Comments MEE 1' above good Accessry use to S.E. 0594 F. 30' Sides 25' Rear 25'
Dev Permit # In Floodway = Letter of A !! S.E. 0594 F. 30' 5ides 25' Rear 25'
Dev Permit # In Floodway Detter of Auth. from Contractor Dev Comp. letter
Owner Builder Disclosure Statement Dand Owner Affidavit DEllisville Water App Fee Paid Sub VF Form OR City Water For
Applicant (Who will sign/pickup the permit) Jeff Gutapfel Address 40 NW 1st St Williston FL 32696 Owners Name 1855168
Address 40 NN 15 St Williston FL 3210910
Confidures 10+L () ()
Contractor Email
Fee Simple Owner Name & Address
Bonding Co. Name & Address
Architect/Engineer Name & Address Wm Design - Associates, Inc 436 Sw Commerce Dr # 130 Mortgage Lenders Name & Address
Mortgage Lenders Name & Address
Circle the correct power company FL Power & Light Clay Elec. Suwannee Valley Elec. Duke Energy
Property ID Number 19 -/-S -1/03995
Property ID Number 19-65-16-03885-000 Estimated Construction Cost 370,000
Subdivision NameLotLotLotLotLotLotLotLotLotLotLotLotLotLotLotLot
Driving Directions from a Major Road 47 South Right on 27 Right on Cockers Si
to Address
Construction of 109 structure
Proposed Use/Occupancy () Lib House
Proposed Use/Occupancy Club House Number of Existing Dwellings on Property
or Explain
Circle Proposed Culvert Permit or Culvert Waiver or D.O.T. Permit or Have an Existing Drive
Actual Distance of Structure from Property Lines - Front 110 Side 115 Side 220 Rear 117
Number of Stories Heated Floor Area 3420 Total Floor Area 3700 Acreage
Zoning Applications applied for (5th a D
Zoning Applications applied for (Site & Development Plan, Special Exception, etc.)
Daniel J. C. Charles

Columbia County Building Permit Application

CODE: Florida Building Code 2014 and the 2011 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating

TIME LIMITATIONS OF APPLICATION: An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION, IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

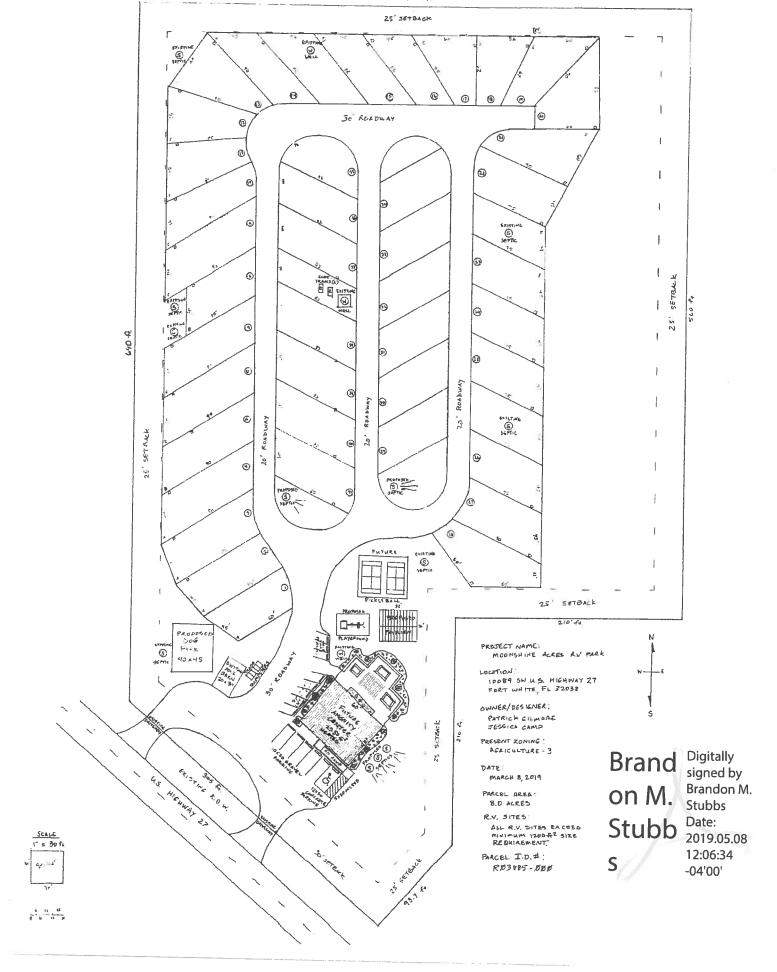
NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is

		You must verify if your property is
Sessica Count	Design Co	**Property owners <u>must sign</u> here <u>before</u> any permit will be issued.
"If this is an Owner Builder Domin 4		

**If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations

this building Permit including all application and perm	nit time limitations
Contractor's Signature Affirmed under penalty of perjury to by the Contractor and s	Contractor's License Number CBC060248 Columbia County Competency Card Number 2036
Personally known V or porduced Identification_	
Danne folsen s	
State of Florida Notary Signature (For the Contractor)	SEAL:



STATE OF FLORIDA DEPARTMENT OF HEALTH

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT Permit Application Number lmore ----- PART II - SITEPLAN -----Scale: 1 inch = 40 feet. 24117100 NT 30 (2) 371 Notes: Site Plan submitted by: MASTER CONTRACTOR Plan Approved Not Approved County Health Department ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

DH 4015, 08/09 (Obsoletes previous editions which may not be used) Incorporated: 64E-6.001, FAC (Stock Number: 5744-002-4015-6)



STATE OF FLORIDA DEPARTMENT OF HEALTH

19-0566 DATE PAID: RECEIPT #:

ONSITE SEWAGE TREATMENT AND DISPOSAL FEE PAID: SYSTEM APPLICATION FOR CONSTRUCTION PERMIT

APPLICATION FOR: [V] New System [] Existing System [] Holding Tank [] Innovative [] Repair [] Abandonment [] Temporary []
APPLICANT: Patrick Gilmore
AGENT: ROCKY FORD, A & B CONSTRUCTION TELEPHONE: 386-497-2311
MAILING ADDRESS: 546 SW Dortch Street, FT. WHITE, FL, 32038
TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTE BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.
PROPERTY INFORMATION LOT: NA BLOCK: NA SUB: NA DELLA LICE DE PLATTED:
PROPERTY ID #: 19-6S-16-03885-000 ZONING: I/M OR EQUIVALENT: [Y N
PROPERTY SIZE: 8 ACRES WATER SUPPLY: [K] PRIVATE PUBLIC []<=2000GPD []>2000GPD IS SEWER AVAILABLE AS PER 381.0065, FS? [Y / N] DISTANCE TO SEWER: NA FT
PROPERTY ADDRESS: 130 Cochran St Fort White DIRECTIONS TO PROPERTY: 47 South Right on 27 Right on Cochran St to address
BUILDING INFORMATION [] RESIDENTIAL [X] COMMERCIAL
Unit Type of No. of Building Commercial/Institutional System Design No Establishment Bedrooms Area Sqft Table 1, Chapter 64E-6, FAC
1 Clubhouse o 2420 3ath House 64, people
3
[] Floor/Equipment Drains [] Other (Specify)
DATE: 7/23/2019

vious editions which may not be used) Incorporated 64E-6.001, FAC

Page 1 of 4

SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT B

1909-56

WE HAVE GUTapfel/Gilmone

THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

NOTE: It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.

Use website to confirm licenses: http://www.columbiacountyffa.com/PermitSearch/ContractorSearch,aspx

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

*** #100 # ## ##3 P ##

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	LIECTRICAL	Prit Name Dale Mike Nadboralski	Signature	YE CO
a	4/19 12	Company Name Mike & Robble Electric		-Sic
	CL# 2040	UICETSE IT. EC13008183	Prone #: 352-250-8800	3. K.
	MECHANICAL/	Pohi Name Robert Cowart	Signarare 2005	Menul
	A/C	Company Name Cowart Air Conditionin	9	
	cc49010	License H CACO56733	Phone 11: 904-669-8608	2.0
	PLUMBING/	A:M Name George Degler	All Warrell Strong 2	Mor 1
9/2/19	GAS V	Company Name A Proud Plumber		- 12
9/1	ccr 1018	Lizense 9: CFC 1427133	Phone 9: 386-438-9135	we
	ROOFING	Principlane Gary Russell	Signature of far Stance!	W4.
	TU	Company Name Russell Roffing Inc	AND THE REAL PROPERTY AND ADMINISTRATION AND ADMINI	N
	rc. 2034	License 4: CCC1327203	Phone II 386-871-7239	= 15 - 10
	SHEFT METAL	Print Name	Signuture	jaje.
		Company Name:	morrows (i.e., a.g., m.g., ii)). — andrewskin administration and accompany	100
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,	FIRE SYSTEM!	Print Name	Signature	Read.
	SPRINKLER	Company Name	A y u o y To committed up de destination of the second section of	1 1 ነተ ኒኒ.፣
	CC#	ត្ លេខការខត្ត:	Prigne S.	" (4 ", (6)
	SOLAR	Print Name	Signature	Meg =
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	CC#	License #	Prone #:	
	STATE	Print Name	\$804110	Flore I
	SPECIALTY	Company Name		⊕ 93
	208311		Pliche P.	2 4

Electronic Articles of Organization For Florida Limited Liability Company

L19000038217 FILED 8:00 AM February 06, 2019 Sec. Of State tjschroeder

Article I

The name of the Limited Liability Company is: RJ INDUSTRIES LLC

Article II

The street address of the principal office of the Limited Liability Company is:

14991 NE JACKSONVILLE RD CITRA, FL. US 32113

The mailing address of the Limited Liability Company is:

14991 NE JACKSONVILLE RD CITRA, FL. US 32113

Article III

The name and Florida street address of the registered agent is:

CABRERA TAX ACCOUNTANTS LLC 8810 SW HWY 200 SUITE 103 OCALA, FL. 34481

Having been named as registered agent and to accept service of process for the above stated limited liability company at the place designated in this certificate, I hereby accept the appointment as registered agent and agree to act in this capacity. I further agree to comply with the provisions of all statutes relating to the proper and complete performance of my duties, and I am familiar with and accept the obligations of my position as registered agent.

Registered Agent Signature: STEVEN CABRERA

Article IV

The name and address of person(s) authorized to manage LLC:

Title: MGR

PATRICK R GILMORE

14991 NE JACKSONVILLE RD

CITRA, FL. 32113 US

Title: MGR JESSICA CAMP

14991 NE JACKSONVILLE RD

CITRA, FL. 32113 US

Article V

The effective date for this Limited Liability Company shall be:

02/01/2019

Signature of member or an authorized representative

Electronic Signature: STEVEN CABRERA

I am the member or authorized representative submitting these Articles of Organization and affirm that the facts stated herein are true. I am aware that false information submitted in a document to the Department of State constitutes a third degree felony as provided for in s.817.155, F.S. I understand the requirement to file an annual report between January 1st and May 1st in the calendar year following formation of the LLC and every year thereafter to maintain "active" status.

L19000038217 FILED 8:00 AM February 06, 2019 Sec. Of State tjschroeder District No. 1 - Ronald Williams District No. 2 - Rocky Ford District No. 3 - Bucky Nash District No. 4 - Toby Witt District No. 5 - Tim Murphy



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

Date/Time Issued:

10/10/2019 6:32:38 PM

Address:

10089 SW US HIGHWAY 27

City:

FORT WHITE

State:

FL

Zip Code

32038

Parcel ID

03885-000

REMARKS: Address Verification.

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.

Address Issued By:

Signed:/ Matt Crews

Columbia County GIS/911 Addressing Coordinator

COLUMBIA COUNTY
911 ADDRESSING / GIS DEPARTMENT

263 NW Lake City Ave., Lake City, FL 32055 Telephone: (386) 758-1125 Email: gis@columbiacountyfla.com



BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY

April 26, 2019

VIA ELECTRONIC MAIL

Patrick Gilmore & Jessica Camp RJ Industries, LLC 14991 NE Jacksonville Rd Citra, Fl 32113

Re: Special Exception 0594 – Moonshine Acres Board of Adjustment Determination Letter

Dear Mr. Gilmore,

At the April 25, 2019 Board of Adjustment ("Board") hearing, the Board approved your application for a Special Exception for a Recreational Vehicle Campground use as permitted in Section 4.5.7(8) of the County's Land Development Regulations ("LDRs") in accordance with Section 12.2 of the County's LDRs. Per Section 12.1.1 of the County's LDRs, there is a thirty (30) day appeal period for all Special Exceptions. If no appeal is filed within thirty (30) days, the decision of the Board shall become final. No permits shall be issued until the thirty (30) day appeal period has expired.

Attached for your records is a copy of Resolution BA SE 0594.

If you have any questions, please do not hesitate to contact me at bstubbs@columbiacountyfla.com or (386) 754-7119.

Sincerely,

Brandon M. Stubbs

County Planner/LDR Admin.

V. M. Sat

Columbia County Property Appraiser

Jeff Hampton

Parcel: << 19-6S-16-03885-000



1	>>
•	(

Owner & Pi	operty Info		
Owner	RJ INDUSTRIE 14991 NORTHE CITRA, FL 3211	AST JACKSO	VILLE RD
Site	130 COCHRAN	ST, FORT WH	HITE
Description*	COMM INTER O SE1/4 OF SE1/4, RUN W 210 FT, R/W OF US-47, N 304.93 FT, N API 560 FT TO POB, 1229-1907, WD 1	RUN N 532.89 5 210 FT, SW 93 NW ALONG R/M PROX 640 FT, E WD 1202-369,	FT FOR POB, 3.7 FT TO N APPROX 510 FT, S 1216-378, WD
Area	8 AC	S/T/R	19-6S-16
Use Code**	MISC RES (000700)	Tax District	3

^{*}The Description above is not to be used as the Legal Description for this

Property & Assessment Values 2018 Certified Values 2019 Preliminary Certified Mkt Land (2) \$39,693 Mkt Land (2) \$42,193 Ag Land (0) \$0 Ag Land (0) \$0 Building (0) \$0 Building (0) \$0 XFOB (5) \$5,600 XFOB (5) \$5,600 Just \$45,293 Just \$47,793 Class \$0 Class \$0 Appraised \$45,293 Appraised \$47,793 SOH Cap [?] \$0 SOH Cap [?] \$0 Assessed \$45,293 Assessed \$47,793 Exempt \$0 Exempt county:\$40,977 county:\$45,075 Total city:\$40,977 Total city:\$45,075 Taxable other:\$40,977 Taxable other:\$45,075 school:\$45,293 school:\$47,793

2019 Preliminary Certified Values

updated: 8/14/2019

Sale Date	Sale Price	Book/Page	Deed	V/I	Quality (Codes)	RCode
6/12/2019	\$80,800	1388/2064	QC	V	U	11
3/8/2019	\$160,000	1381/2573	WD	V	Q	01
2/6/2012	\$17,500	1229/1907	WD	1	Q	03
6/9/2011	\$10,000	1216/0378	WD	1	Q	01
6/9/2011	\$10,000	1216/0378	WD	1	Q	01
5/25/2011	\$100	1215/0476	PR	1	U	11
5/25/2011	\$100	1215/0476	PR	1	U	11
4/21/2011	\$100	1222/0469	WD	1	U	12
9/14/2010	\$25,000	1202/0369	WD	V	Q	01
2/25/2010	\$0	1189/1999	PB	V	U	11
		1116/1215				

parcel in any legal transaction.

The <u>Use Code</u> is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.



COLUMBIA COUNTY BUILDING DEPARTMENT RESIDENTIAL CHECK LIST

MINIMUM PLAN REOUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2017 EFFECTIVE 1 JANUARY 2018 AND THE NATIONAL ELECTRICAL 2014 EFFECTIVE 1 JANUARY 2018

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT FLORIDA BUILDING CODES RESIDENTIAL AND THE NATIONAL ELECTRICAL CODE. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS, FBC 1609.3.1 THRU 1609.3.3.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES **Revised 7/1/18**

Website: http://www.columbiacountyfla.com/BuildingandZoning.asp Each Box shall be **GENERAL REQUIREMENTS:** Circled as APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL Applicable Select From Drop down Two (2) complete sets of plans containing the following: All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void 3 Condition space (Sq. Ft.) Total (Sq. Ft.) under roof No NA

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL 107.1.

Site Plan information including:

4	Dimensions of lot or parcel of land	V		
5	Dimensions of all building set backs	1	1 to 17 days 1 de 1 d	
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed	~	************	or manufactures to a factor and adequate special speci
	well and septic tank and all utility easements.			1
7	Provide a full legal description of property.	V		

Wind-load Engineering Summary, calculations and any details are required.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL 8 Plans or specifications must show compliance with FRCR Chapter 3		Items to Include Each Box shall b Circled as Applicable		ll be
8	Plans or specifications must show compliance with FBCR Chapter 3	Yes	No	NA
		Select Fro	om Drop	down
9	Basic wind speed (3-second gust), miles per hour	- 6		
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	-V		
11	Wind importance factor and nature of occupancy		3 MARIE - 1800 -	
12	The applicable internal pressure coefficient, Components and Cladding	- V	n half tude on which collects of Collections . 3 pp/g	
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.			

Drawing including: All side views of the structure 0 15 Roof pitch ~ 16 Overhang dimensions and detail with attic ventilation 17 Location, size and height above roof of chimneys 18 Location and size of skylights with Florida Product Approval 19 V Number of stories 20 Building height from the established grade to the roofs highest peak

Items to Include-

Floor Plan Including: Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, 21 deck, balconies -V Raised floor surfaces located more than 30 inches above the floor or grade 22 --23 All exterior and interior shear walls indicated - V Shear wall opening shown (Windows, Doors and Garage doors) - 6 Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass. 26 Safety glazing of glass where needed Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth 27 (see chapter 10 and chapter 24 of FBCR) Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails 28 29 Identify accessibility of bathroom (see FBCR SECTION 320) All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form) **GENERAL REQUIREMENTS:** Items to Include-APPLICANT - PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL Each Box shall be Circled as Applicable FBCR 403: Foundation Plans Select From Drop down Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing. All posts and/or column footing including size and reinforcing - 0 32 Any special support required by soil analysis such as piling. ~ 33 Assumed load-bearing valve of soil - 0 Pound Per Square Foot Location of horizontal and vertical steel, for foundation or walls (include # size and type) For structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. - C Per the National Electrical Code article 250.52.3 FBCR 506: CONCRETE SLAB ON GRADE 35 Show Vapor retarder (6mil. Polyethylene with pints la en 6 inches and sealed) 36 Show control j oints, synthetic fiber reinforcement or welded fire fabric reinforcement and Sports FBCR 318: PROTECTION AGAINST TERMITES Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. Protection shall be provided by registered termiticides FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

38 Show all materials making up walls, wall height, and Block size, mortar type39 Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement

	Floor truss package shall including layout and details, signed and sealed by Florida Registered	/	I
10		- ~	
	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls,	- (-	
1	stem walls and/or priers	-0	
2		-4	
3		- 1	
4			
5	Show required under-floor crawl space	- V	
6	Show required amount of ventilation opening for under-floor spaces		
7	Show required covering of ventilation opening	- V	Ann - Annessan - Anne
8	Show the required access opening to access to under-floor spaces	-0	
^	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges &	- 0	1
9	intermediate of the areas structural panel sheathing		
0	Show Draftstopping, Fire caulking and Fire blocking	-0	
1	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	_ 2	
2	Provide live and dead load rating of floor framing systems (psf).	-0	
В	CR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION		
	Dres compare manter conventition and transport management to a distinguisher a sometime and the source and the	Items to	Include-
	GENERAL REQUIREMENTS:	Each Box	
	APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Circle	
		Appli	cable
	S	elect from l	Drop d o
3	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	- V	
4	Fastener schedule for structural members per table FBC-R602.3.2 are to be shown		
	Show wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural		
5	members, showing fastener schedule attachment on the edges & intermediate of the areas structural		
	panel sheathing	V	
	Show all required connectors with a max uplift rating and required number of connectors and		
6	oc spacing for continuous connection of structural walls to foundation and roof trusses or		
	rafter systems	- ~	
	Show sizes, type, span lengths and required number of support jack studs, king studs for	1	an
7	shear wall opening and girder or header per FBC-R602.7.	- ~	
3	Indicate where pressure treated wood will be placed	_ ~	
	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural		
)	panel sheathing edges & intermediate areas	-V	
)	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail		
	The state of the s		
-	CR :ROOF SYSTEMS:		
	Truss design drawing shall meet section FBC-R 802.10. I Wood trusses	- 0	
2	Include a layout and truss details, signed and sealed by Florida Professional Engineer	- 0	
3	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	- ~	
Ц	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	- v	
5	Provide dead load rating of trusses	- 0	
Ľ	CD 902. Conventional Doof Francis at Lauret		
	BCR 802:Conventional Roof Framing Layout		
	Rafter and ridge beams sizes, span, species and spacing	- 0	
	Connectors to wall assemblies' include assemblies' resistance to uplift rating	- V	
	Valley framing and support details	- 0	
	Provide dead load rating of rafter system		
	CD 903 DOOF SHEATHING	•	
B	CR 803 ROOF SHEATHING		
B	CR 803 ROOF SHEATHING Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness		

sheathing, grade, thickness

71 Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas

ROOF ASSEMBLIES FRC Chapter 9

72	Include all materials which will make up the roof assembles covering	T_	V	
73	Submit Florida Product Approval numbers for each component of the roof assembles covering	-	~	

FBCR Chapter 11 Energy Efficiency Code for Residential Building

Residential construction shall comply with this code by using the following compliance methods in the FBCR Chapter 11 Residential buildings compliance methods. Two of the required forms are to be submitted, N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.

Select from Dr. 74 Show the insulation R value for the following areas of the structure 75 Attic space 76 Exterior wall cavity 77 Crawl space 80 Submit two copies of a Manual J sizing equipment or equivalent computation study 79 Exhaust fans shown in bathrooms Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required 80 Show clothes dryer route and total run of exhaust duct Plumbing Fixture layout shown 81 All fixtures waste water lines shall be shown on the foundation and all fixtures waste water lines shall be shown on the foundation and and are repaired and total run of exhaust duct Private Potable Water 83 Pump motor horse power 84 Reservoir pressure tank gallon capacity 85 Rating of cycle stop valve if used 86 Show Switches, receptacles outlets, lighting fixtures and Ceiling fans 87 Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A 88 Show the location of smoke detectors & Carbon monoxide detectors	op Down
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88 Show the location of smoke detectors & Carbon monoxide detectors -	
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89 Show service panel, sub-panel, location(s) and total ampere ratings	Particular services of the American
billow service pariet, sae pariet, reservente) and rotal ampere ratings	-
On the electrical plans identify the electrical service overcurrent protection device for the main	
electrical service. This device shall be installed on the exterior of structures to serve as a	
90 disconnecting means for the utility company electrical service. Conductors used from the exterior	
disconnecting means to a panel or sub panel shall have four-wire conductors, of which one	
conductor shall be used as an equipment ground. Indicate if the utility company service entrance	
cable will be of the overhead or underground type.	
cable will be of the overhead or underground type.	
For structures with foundation which establish new electrical utility companies service	
connection a Concrete Encased Electrode will be required within the foundation to serve as an	
•	
Grounding electrode system. Per the National Electrical Code article 250.52.3	
91 Appliances and HVAC equipment and disconnects -	
92 Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed	
in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms,	
sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by	
a listed Combination arc-fault circuit interrupter, Protection device.	

Notice Of Commencement:

A notice of commencement form RECORDED in the Columbia County Clerk Office is required to be filed with the Building Department BEFORE ANY INSPECTIONS can be performed.

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as	
	Applicable	

ITEMS 95, 96, & 98 Are Required After APPROVAL from the ZONING DEPT. Select from Drop down Building Permit Application A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a \$15.00 application fee. The completed application with attached documents and application fee can be mailed. 94 Parcel Number The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com 95 Environmental Health Permit or Sewer Tap Approval A copy of a approved Columbia County Environmental Health (386) 758-1058 96 City of Lake City A City Water and/or Sewer letter. Call 386-752-2031 97 Toilet facilities shall be provided for all construction sites 98 Town of Fort White (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit. Flood Information: All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations (Municpde.cpm) CERTIFIED FINISHED FLOOR ELEVATIONS will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required. 101 A Flood development permit is also required for AE, Floodway & AH. Development permit cost is \$50.00 **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. County Public Works Dept, determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required. 911 Address: An application for a 911 address must be applied for and received through the Columbia 103 County Emergency Management Office of 911 Addressing Department (386) 758-1125.

Ordinance Sec. 90-75. - Construction debris. (e) It shall be unlawful for any person to dispose of or discard solid waste, including construction or demolition debris at any place within the county other than on an authorized disposal site or at the county's solid waste facilities. The temporary storage, not to exceed seven days of solid waste (excluding construction and demolition debris) on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance, shall not be deemed a violation of this section. The temporary storage of construction and demolition debris on the premises where generated or vegetative trash pending disposition as authorized by law or ordinance shall not be deemed in violation of this section; provided, however, such construction and demolition debris must be disposed of in accordance with this article prior to the county's issuance of a certificate of occupancy for the premises. The burning of lumber from a construction or demolition project or vegetative trash when done so with legal and proper permits from the authorized agencies and in accordance with such agencies' rules and regulations, shall not be deemed a violation of this section. No person shall bury, throw, place, or deposit, or cause to be buried, thrown, placed, or deposited, any solid waste, special waste, or debris of any kind into or on any of the public streets, road right-of-way, highways, bridges, alleys, lanes, thoroughfares, waters, canals, or vacant lots or lands within the county. No person shall bury any vegetative trash on any of the public streets, road right-of-way, highways, bridges, lanes, thoroughfares, waters, canals, or lots less than ten acres in size within the county.

Disclosure Statement for Owner Builders:

If you as the Applicant will be acting as your own contractor or owner/builder under section 489.103(7) Florida Statutes, you must submit the required notarized Owner Builder Disclosure Statement form.

**This form can be printed from the Columbia County Website on the Building and Zoning page under Documents. Web address is - http://www.columbiacountyfla.com/BuildingandZoning.asp

Section 105 of the Florida Building Code defines the:

Time limitation of application.

An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless such application has been pursued in good faith or a permit has been issued; except that the building official is authorized to grant one or more extensions of time for additional periods not exceeding 90 days each. The extension shall be requested in writing and justifiable cause demonstrated.

Single-family residential dwelling.

Section 105.3.4 A building permit for a single-family residential dwelling must be issued within 30 working days of application therefor unless unusual circumstances require a longer time for processing the application or unless the permit application fails to satisfy the Florida Building Code or the enforcing agency's laws or ordinances.

Permit intent.

Section 105.4.1: A permit issued shall be constructed to be a license to proceed with the work and not as authority to violate, cancel, alter or set aside any of the provisions of the technical codes, nor shall issuance of a permit prevent the building official from thereafter requiring a correction of errors in plans, construction or violations of this code. Every permit issued shall become invalid unless the work authorized by such permit is commenced within six months after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of six months after the time the work is commenced.

If work has commenced.

Section 105.4.1.1: If work has commenced and the permit is revoked, becomes null and void, or expires because of lack of progress or abandonment, a new permit covering the proposed construction shall be obtained before proceeding with the work.

New Permit.

Section 105.4.1.2: If a new permit is not obtained within 180 days from the date the initial permit became null and void, the building official is authorized to require that any work which has been commenced or completed be removed from the building site. Alternately, a new permit may be issued on application, providing the work in place and required to complete the structure meets all applicable regulations in effect at the time the initial permit became null and void and any regulations which may have become effective between the date of expiration and the date if issuance of the new permit.

Work Shall Be:

Section 105.4.1.3: Work shall be considered to be in active progress when the permit has received an approved inspection within 180 days. This provision shall not be applicable in case of civil commotion or strike or when the building work is halted due directly to judicial injunction, order or similar process.

The Fee:

Section 105.4.1.4: The fee for renewal reissuance and extension of a permit shall be set forth by the administrative authority.

Notification:

When the application is approved for permitting the applicant will be notified by phone as to the status by the Columbia County Building & Zoning Department.

Rec. Doc.

This instrument was prepared by, record and return to:
Jon I. McGraw, Esq.
Schatt & Hesser, McGraw
328 N.E. 1st Avenue, Suite 100
Ocala, FL 34470
352-789-6520

QUITCLAIM DEED

THIS INDENTURE, made effective the day of June, 2019, between PATRICK R. GILMORE and JESSICA CAMP, husband and wife, whose address is 14991 NE Jacksonville Road, Citra, Florida 32113, Grantor, and R.J. INDUSTRIES LLC, a Florida limited liability company, whose address is 14991 NE Jacksonville Road, Citra, Florida 32113, Grantee. (Wherever used herein the terms "Grantor" and "Grantee" include all the parties to the instrument and the heirs, legal representatives and assigns of the individuals, and the successors and assigns of corporations).

WITNESSETH, that said Grantor, for and in consideration of the sum of Ten and No/100 Dollars (\$10.00) and other good and valuable considerations, receipt of which is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys and confirms unto Grantees all that certain land situate in Columbia County, Florida, to wit:

SEE ATTACHED EXHIBIT "A"

Property Appraiser's Parcel I.D. Number: 19-6S-16-03885-000

SUBJECT TO:

- 1. Ad valorem taxes for 2019 and subsequent years;
- 2. Any and all governmental zoning laws, rules and regulations applicable to the property;
- 3. Easements, reservations, declaration of covenants, conditions and restrictions and riparian rights of record, if any

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD the same together with all and singular the appurtenances thereunto belonging or in anywise appertaining, and all the estate, right, title, encumbrances, interest, lien, equity and claim whatsoever of Grantor, either in law or equity, to the only proper use, benefit and behoof of the Grantee forever.

Grantor and Grantee are used for singular or plural, as context requires,

IN WITNESS WHEREOF, Grantor has hereunto set Grantor's hand and seal the day and year first above written.

And

Signed, sealed and delivered in our presence as to all:

Witness

Witness

(Print Name)

(Drint Morna)

Jessica Cam

Inst. Number: 201912016058 Book: 1388 Page: 2065 Page 2 of 3 Date: 7/12/2019 Time: 3:36 PM P.DeWitt Cason Clerk of Courts, Columbia County, Florida Doc Deed: 565.60 Doc Mort: 0.00 Int Tax: 0.00

STATE OF FLORIDA COUNTY OF MARION

I HEREBY CERTIFY that on this day before me, an officer duly qualified to take acknowledgments, personally appeared Patrick R Gilmore and Jessica Camp, husband and wife, known to me (YES NOX) to be the persons described in and who executed the foregoing instrument, OR who have produced as identification and acknowledged before me that they executed same for the purposes expressed herein.

WITNESS my hand and official scal in the County and State last aforesaid this 12 day of June, 2019.

Notary Public, State of Florida My Commission Expires:





Commence at the Intersection of the East line of the SE 1/4 of the SE 1/4 of Section 19, Township 6 South, Range 16 East, and the North right of way line of US Highway No. 27 and run North 1 degrees 45' West, along said East line, 532.89 feet, thence South 88 degrees 15' West, 210 feet to the Point of Beginning; thence continue South 88 degrees 15' West, 210.00 feet; thence North 1 degrees 45' West, 130.00 feet; thence North 88 degrees 15' East, 210.00 feet; thence South 1 degrees 45' East, 130.00 feet to the Point of Beginning, Columbia County, Florida.

Together with a 15 foot easement: Commence at the Intersection of the East line of the SE 1/4 of the SE 1/4 of Section 19, Township 6 South, Range 16 East, and the North right of way line of US Highway No. 27 and run North 48 degrees 52' 16" West, along the North right of way line of said U.S. Highway No. 27, a distance of 614.09 feet to the East line of a 15 foot easement and the Point of Beginning; thence North 1 degrees 45' West, 244.74 feet; thence South 88 degrees 15' West, 15.00 feet; thence South 1 degrees 45' East, 230.81 feet to the north right of way line of said U.S. Highway No. 27; thence South 48 degrees 52' 16" East, along said North right of way line, 20.47 feet to the Point of Beginning.

Together with a 30 foot easement: Commence at the intersection of the East line of the SE 1/4 of the SE 1/4 of Section 19, Township 6 South, Range 16 East, and the North right of way line of U.S. Highway No. 27 and run North 48 degrees 52' 16" West, along the North right of way line of said U.S. Highway No. 27, a distance of 573.15 feet to the East line of a 30 foot easement and the Point of Beginning; thence North 1 degrees 45' West, along the East line of said 30 foot easement, 272.60 feet; thence South 88 degrees 15' West, 30.00 feet; thence South 1 degrees 45' East, 244.74 feet to the North right of way line of said U.S. Highway No. 27, thence South 48 degrees 52' 16" East, along said North right of way line 40.94 feet to the Point of Beginning

And

Commence at the intersection of the East line of the Southeast 1/4 of the Southeast 1/4 of Section 19, Township 6 South, Range 16 East, Columbia County, Florida, and the North right of way line of U.S. Highway No. 27 and run North 01 degrees 45' West, along said East line 532.89 feet to the Point of Beginning; thence South 88 degrees 15' West, 210 feet; thence North 01 degrees 45' West, 130 feet; thence South 88 degrees 15' West, 300 feet, thence North 01 degrees 45' West, 430 feet; thence North 88 degrees 15' East, 510 feet to said East line; thence South 1 degrees 45' East, along said East line 560 feet to the Point of Beginning.

Together with:

15 foot easement: commence at the intersection of the East line of the Southeast 1/4 of the Southeast 1/4 of Section 19, Township 6 South, Range 16 East, Columbia County, Florida and the North right of way line of U.S. Highway No. 27 and run North 48 degrees 52' 16" West, along the North right of way line of said U.S. Highway No. 27, a distance of 614.09 feet to the East line of a 15.00 foot easement and the Point of Beginning; thence North 1 degrees 45' East, 244.74 feet; thence South 88 degrees 15' West, 15.00 feet; thence South 1 degrees 45' East, 230.81 feet to the North right of way line of said U.S. High No. 27; thence South 48 degrees 52' 16" East, along said North right of way line, 20.47 feet to the Point of Beginning.

Also:

30 foot easement: Commence at the intersection of the East line of the Southeast 1/4 of the Southeast 1/4 of Section 19, Township 6 South, Range 16 East, Columbia County, Florida and the North right of way line of U.S. Highway No. 27, and run North 48 degrees 52' 16" West, along the North right of way line of U.S. Highway No. 27, a distance of 573.15 feet to the East line of a 30 foot easement and the Point of Beginning: thence North 1 degrees 45' West, along the East line of said 30 foot easement, 272.60 feet; thence South 88 degrees 15' West, 30.00 feet; thence South 1 degrees 45' East, 244.74 feet to the North right of way line of said U.S. Highway No. 27; thence South 48 degrees 52' 16" East, along said North right of way line, 40.94 feet to the Point of Beginning.

AND

Commence at the point of intersection of the North Right-of-Way line of U.S. Highway No. 27 and the East line of the Southeast 1/4 of the Southeast 1/4 of Section 19, Township 6 South, Range 16 East, Columbia County, Florida, and run North 48 deg. 51' West along the right-of-way line 377 feet to the Point of Beginning; thence continue North 48 deg. 51' West, 196.00 feet; thence North 1 deg. 45' West, 142.6 feet; thence North 88 deg. 15' East, 210.00 feet; thence South 1 deg. 45' East, 210.00 feet, thence South 43 deg. 15' West, 93.7 feet to the Point of Beginning, Being a part of the Southeast 1/4 of the Southeast 1/4.

ALSO

Approximately the West 90 feet of the following property:
Begin at the intersection of the East line of the Southeast 1/4 of Section 19, Township 6 South, Range 16 East, Columbia
County, Florida, and the North right-of-way line of U.S. Highway No. 27 and run North 1 deg. 45' West, along said East line
532.70 feet; thence South 88 deg. 15' West, 210.00 feet; thence North 1 deg. 45' West, 130.00 feet; thence South 88 deg. 15'
West, 300.00 feet; thence South 1 deg. 45' East, to the North right-of-way line of U.S. Highway No. 27; thence continue in a
Southeast direction along the North line of U.S. Highway No. 27 to the Point of Beginning.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			Approval realiser(s)
A. SWINGING	Therm-Try.		EL SIEGOF O
B. SLIDING	The Harman		FL-15225.2
C. SECTIONAL/ROLL UP			
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	Pella Windows		FL-12952,2
B. HORIZONTAL SLIDER	1214 221121000		FL 18750KVA
C. CASEMENT			
D. FIXED			
E. MULLION	Pella windows		FL-13815,1
F. SKYLIGHTS			1 6 1381371
G. OTHER			
3. PANEL WALL			
A. SIDING			
B. SOFFITS			
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES			
B. NON-STRUCTURAL METAL	Gulf Coast	26 Ga PBR	FL-11651
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCTURAL COMPONENTS			
A. WOOD CONNECTORS	Simpson		FL-10456
8. WOOD ANCHORS			
C. TRUSS PLATES	Alpine		
D. INSULATION FORMS			=L-1999
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
The products listed below did not de		autau tuaduasaan akas at thu titi	

The products listed below did not demonstrate product approval at plan review. Lunderstand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.

Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

Contractor OR Agent Signature	Date	NOTES:									



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: S1220 - GILMORE RES

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info: GADCO Project Name: GILMORE Model: 000

Lot/Block: 000

Subdivision: 000

Address: 000, 000

State: FL

City: 000

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special **Loading Conditions):**

Design Code: FRC2017/TPI2014

Wind Code: ASCE 7-10

Wind Speed: 140 mph

Design Program: MiTek 20/20 8.2

Roof Load: 37.0 psf

Floor Load: N/A psf

This package includes 9 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
1	T18043898	CAP1	9/5/19
2	T18043899	CAP2	9/5/19
2 3 4	T18043900	CAP3	9/5/19
4	T18043901	CAP4	9/5/19
5	T18043902	T1	9/5/19
6	T18043903	T2	9/5/19
7	T18043904	<u>T</u> 3	9/5/19
8	T18043905	<u>T4</u>	9/5/19
9	T18043906	T5	9/5/19



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Duley Truss.

Truss Design Engineer's Name: Albani, Thomas

My license renewal date for the state of Florida is February 28, 2021.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Job Truss Truss Type Qty Ply GILMORE RES T18043898 S1220 CAP1 GABLE 24 1 Job Reference (optional) 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:43 2019 Page 1 Duley Truss, Dunnellon, FL - 34430, ID.AMnGBqM9o3FaPdlsuvOqloyggBQ-hdzNrALyTmuK94d41_unzDGk2578N5jVT5cTEyygfvQ 4-6-0 4-6-0 Scale = 1:30.0 4x4 = 3 12.00 12 10 54 3x4 == 3x4 == 1.5x4 9-0-0 9-0-0 [2:0-1-6,0-1-8], [4:0-1-6,0-1-8] Plate Offsets (X,Y)-**PLATES** GRIP LOADING (psf) SPACING-2-0-0 DEFL. in l/defl L/d (loc) MT20 244/190 **TCLL** 20.0 Plate Grip DOL 1.25 TC 0.34 Vert(LL) n/a n/a 999 **TCDL** 7.0 Lumber DOL 1.25 BC 0.14 Vert(CT) n/a n/a 999 **BCLL** 0.0 Rep Stress Incr YES WB 0.04 Horz(CT) 0.00 n/a n/a

LUMBER-

BCDI

TOP CHORD 2x4 SP No.2D **BOT CHORD OTHERS**

10.0

2x4 SP No.2D 2x4 SP No.3

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 36 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. All bearings 9-0-0.

Max Horz 1=-159(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) except 1=-450(LC 17), 5=-358(LC 18), 2=-556(LC 12), 4=-556(LC

12)

All reactions 250 lb or less at joint(s) 6 except 1=425(LC 12), 5=425(LC 12), 2=647(LC 17), 4=574(LC Max Grav

Matrix-P

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Code FRC2017/TPI2014

TOP CHORD 1-2=-349/385, 4-5=-348/349

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-2-8 to 3-2-8, Interior(1) 3-2-8 to 4-6-0, Exterior(2) 4-6-0 to 7-6-0, Interior(1) 7-6-0 to 8-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 4-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 5, 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 1, 358 lb uplift at joint 5, 556 lb uplift at joint 2 and 556 lb uplift at joint 4.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

September 5,2019



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Job Truss Type Qty Ply GILMORE RES Truss T18043899 S1220 CAP2 2 Piggyback Job Reference (optional) 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:44 2019 Page 1 Dunnellon, FL - 34430. Duley Truss, ID:AMnGBqM9o3FaPdlsuvOqloyggBQ-ApXl2WMaE40BnEBHaiP0WRoztVNy6XSfilL0mPygfvP 5-0-0 5-0-0 5-0-0 Scale = 1:27.7 4x4 = 10.00 12 1.5x4 || 1.5x4 0-1-10 · 10 9 8 2x4 = 2x4 = 1.5x4 || 1.5x4 || 1.5x4 10-0-0 CSI. DEFL. l/defi L/d **PLATES** GRIP LOADING (psf) SPACING-2-0-0 in (loc) 244/190 999 MT20 Plate Grip DOL TC 0.08 Vert(LL) **TCLL** 20.0 1.25 n/a n/a BC 0.04 Vert(CT) n/a n/a 999 TCDL 7.0 Lumber DOL 1.25 6 WB 0.07 0.00 n/a **BCLL** 0.0 Rep Stress Incr YES Horz(CT) n/a Code FRC2017/TPI2014 Weight: 43 lb FT = 20% BCDL 10.0 Matrix-P

LUMBER-

TOP CHORD 2x4 SP No.2D BOT CHORD 2x4 SP No.2D **OTHERS** 2x4 SP No.3

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-0-0.

(lb) - Max Horz 1=-139(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2, 6 except 1=-126(LC 10), 10=-146(LC 12), 8=-146(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 2, 6, 9, 10, 8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-2-12 to 3-0-0, Interior(1) 3-0-0 to 5-0-0, Exterior(2) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 9-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2, 6 except (it=lb) 1=126, 10=146, 8=146,
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



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September 5,2019



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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Job Truss Type Qty Ply GILMORE RES Truss T18043900 S1220 CAP3 GABLE 14 Job Reference (optional) Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:45 2019 Page 1 Duley Truss, ID:AMnGBqM9o3FaPdlsuvOqloyggBQ-e?57GsNC?N82OOmT8PxF2eL4Uuh6r?9owP5airygfvO 5-0-0 5-0-0 Scale = 1:27.5 4x4 = 3 10.00 12 9 10 0-1-10 6 3x5 = 3x5 = 1.5x4 10-0-0 Plate Offsets (X,Y)-[2:0-2-4,0-1-8], [4:0-2-4,0-1-8] **PLATES** GRIP SPACING-CSI. DEFL. I/defi L/d LOADING (psf) 2-0-0 in (loc) TC 999 MT20 244/190 TCLL 20.0 Plate Grip DOL 1.25 0.34 Vert(LL) n/a n/a BC 999 TCDL 7.0 Lumber DOL 1.25 0.17 Vert(CT) n/a n/a 0.00 **BCLL** 0.0 Rep Stress Incr YES WB 0.04 Horz(CT) n/a n/a Weight: 37 lb FT = 20% BCDL 10.0 Code FRC2017/TPI2014 Matrix-P

LUMBER-

TOP CHORD 2x4 SP No.2D **BOT CHORD**

2x4 SP No.2D **OTHERS** 2x4 SP No.3

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-0-0.

(lb) -Max Horz 1=-139(LC 10)

Max Uplift All uplift 100 b or less at joint(s) except 1=-411(LC 17), 5=-341(LC 18), 2=-513(LC 12), 4=-513(LC 12)

All reactions 250 lb or less at joint(s) except 1=375(LC 12), 5=375(LC 12), 2=628(LC 17), 4=577(LC 18), 6=253(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-273/332, 4-5=-272/291

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 5-0-0, Exterior(2) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 9-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 4-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 411 lb uplift at joint 1, 341 lb uplift at joint 5, 513 lb uplift at joint 2 and 513 lb uplift at joint 4.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



6904 Parke East Blvd. Tampa FL 33610 Date

September 5,2019



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ANSITEPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information

available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Ply GILMORE RES Job Truss Qtv Truss Type T18043901 CAP4 GABLE 2 S1220 Job Reference (optional) 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:46 2019 Page 1 Dunnellon, FL - 34430, Duley Truss, ID:AMnGBqM9o3FaPdlsuvOqloyggBQ-6CfVTBOrmhGv0XLfi6SUbsuJLl3baRoy93q7qHygfvN 4-6-0 4-6-0 Scale = 1:30.2 13 12.00 12 1.5x4 1.5x4 5 3 14 76 0-1-10 10 2x4 = 2x4 = 1.5x4 || 1.5x4 1.5x4 PLATES GRIP LOADING (psf) SPACING-2-0-0 CSI. DEFL in (loc) I/defl I/d 244/190 **TCLL** 20.0 Plate Grip DOL 1.25 TC 0.08 Vert(LL) n/a n/a 999 MT20 TCDL 7.0 Lumber DOL 1.25 BC 0.03 Vert(CT) n/a n/a 999 **BCLL** 0.0 Rep Stress Incr YES WB 0.08 Horz(CT) 0.00 6 n/a n/a

LUMBER-

BCDL

TOP CHORD 2x4 SP No.2D **BOT CHORD** 2x4 SP No.2D OTHERS 2x4 SP No.3

10.0

BRACING-

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Weight: 41 lb

FT = 20%

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 9-0-0.

(lb) - Max Horz 1=-159(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2, 6 except 1=-149(LC 10), 10=-169(LC 12), 8=-169(LC 12)

Matrix-P

Max Grav All reactions 250 lb or less at joint(s) 1, 7, 2, 6, 9, 10, 8

FORCES. (ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

Code FRC2017/TPI2014

3-10=-261/266, 5-8=-261/266 **WEBS**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-2-8 to 3-2-8, Interior(1) 3-2-8 to 4-6-0, Exterior(2) 4-6-0 to 7-6-0, Interior(1) 7-6-0 to 8-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members
- 8) Bearing at joint(s) 7, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2, 6 except (it=lb) 1=149, 10=169, 8=169,
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.



6904 Parke East Blvd. Tampa FL 33610 Date:

September 5,2019



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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not



Job Truss Type Qty Ply GILMORE RES Truss T18043902 S1220 T1 Piggyback Base Supported Gable 2 Job Reference (optional) Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:48 2019 Page 1 **Duley Truss**, ID:AMnGBqM9o3FaPdlsuvOqloyggBQ-2anGutP5IIWcFrV2pXUygHzW66gF2KaFcNJEvAygfvL 33-8-0 11-10-0 10-0-0 11-10-0 Scale = 1:69.8 4x4 4x4 = 7 38 39 12 13 10.00 12 14 15 X 16 17 3x5 || 3x5 18 2-0-0 28 27 26 24 23 22 21 19 37 35 34 33 32 31 30 29 25 20 36 11 4×4 4x4 4x4 33-8-0 33-8-0 [7:0-2-0,0-1-13], [12:0-2-0,0-1-13], [19:Edge,0-3-8], [27:0-1-12,0-0-0], [28:0-2-0,0-1-4], [28:0-0-0,0-1-12] Plate Offsets (X,Y)-LOADING (psf) DEFL. L/d **PLATES** GRIP SPACING-2-0-0 CSI. in I/defl (loc) MT20 244/190 **TCLL** 20.0 Plate Grip DOL 1.25 TC 0.64 Vert(LL) n/a n/a 999 999 TCDL 7.0 Lumber DOL 1,25 BC 0.34 Vert(CT) n/a n/a BCLL 0.0 Rep Stress Incr YES WB 0.19 Horz(CT) -0.01 19 n/a n/a FT = 20%Weight: 311 lb BCDL 10.0 Code FRC2017/TPI2014 Matrix-R LUMBER-**BRACING-**TOP CHORD 2x4 SP No.2D TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, **BOT CHORD** 2x4 SP No.2D except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 7-12. 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. **WEBS** 12-25, 11-26, 10-27, 9-29, 8-30, 7-31, 6-32 **OTHERS** 2x4 SP No.3 WEBS 1 Row at midpt , 5-33, 13-24, 14-23

REACTIONS. All bearings 33-8-0.

Max Horz 37=-457(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 26, 27, 29, 30 except 37=-377(LC 10), 19=-345(LC 11), 32=-109(LC 12), 33=-129(LC 12), 34=-120(LC 12), 35=-113(LC 12), 36=-315(LC 11), 24=-109(LC 12), 23=-129(LC 12), 22=-120(LC 12),

21=-113(LC 12), 20=-292(LC 10)

Max Grav All reactions 250 lb or less at joint(s) 26, 27, 29, 30, 32, 33, 34, 35, 24, 23, 22, 21 except 37=448(LC 11), 19=415(LC 10), 25=284(LC 12), 31=284(LC 12), 36=473(LC 10), 20=450(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-37=-285/238, 1-2=-337/302, 3-4=-236/281, 4-5=-338/403, 5-6=-446/534,

6-7=-545/650, 7-8=-453/547, 8-9=-452/547, 9-10=-452/547, 10-11=-452/547,

11-12=-453/547, 12-13=-545/650, 13-14=-446/534, 14-15=-338/403, 15-16=-235/280,

17-18=-312/276, 18-19=-264/216

12-25=-285/228, 7-31=-285/228, 2-36=-251/216

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf, BCDL=6.0psf; h=25ft; B=45ft; L=34ft, eave=2ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3) 0-1-12 to 3-6-2, Exterior(2) 3-6-2 to 11-10-0, Corner(3) 11-10-0 to 15-2-6, Exterior(2) 15-2-6 to 21-10-0, Corner(3) 21-10-0 to 25-2-6, Exterior(2) 25-2-6 to 33-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 1.5x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26, 27, 29, 30

Continues ob file Bib 32=377, 19=345, 32=109, 33=129, 34=120, 35=113, 36=315, 24=109, 23=129, 22=120, 21=113, 20=292.



Thomas A. Albani PE No.39380

6904 Parke East Blvd. Tampa FL 33610

September 5,2019

MiTek USA, Inc. FL Cert 6634

No 39380

No 39380

Thomas A. Albani Pe No.39380

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rov. 10/03/2015 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guildance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

Job	Truss	Truss Type	Qty	Ply	GILMORE RES
S1220	T1	Piggyback Base Supported Gable	2	1	T18043902
5,125		1387			Job Reference (optional)

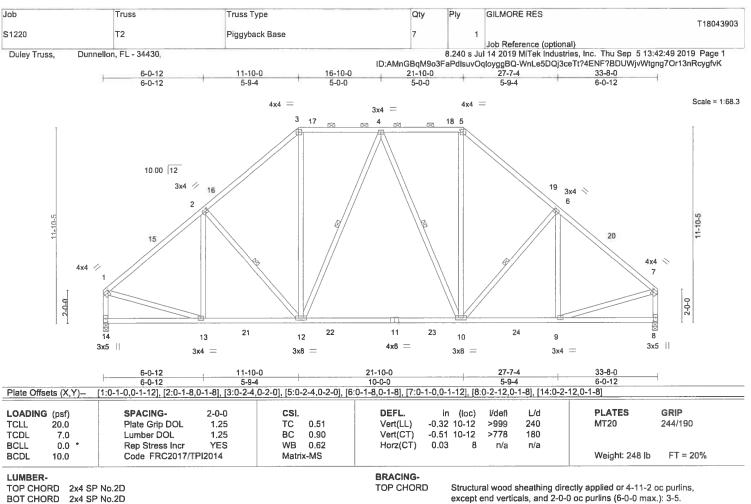
Duley Truss, Dunnellon, FL - 34430,

8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:48 2019 Page 2 ID:AMnGBqM9o3FaPdlsuvOqloyggBQ-2anGutP5IIWcFrV2pXUygHzW66gF2KaFcNJEvAygfvL

NOTES-

12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





BOT CHORD

WEBS

Rigid ceiling directly applied or 9-4-3 oc bracing.

2-12, 4-12, 4-10, 6-10

1 Row at midpt

2x4 SP No.3 WEBS

REACTIONS. (lb/size) 14=1235/0-4-0, 8=1235/0-4-0

Max Horz 14=-457(LC 10)

Max Uplift 14=-453(LC 12), 8=-453(LC 12) Max Grav 14=1313(LC 17), 8=1313(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1378/605, 2-3=-1288/732, 3-4=-1037/663, 4-5=-1037/663, 5-6=-1288/732,

6-7=-1379/605, 1-14=-1252/588, 7-8=-1252/589

BOT CHORD 13-14=-433/471, 12-13=-409/1257, 10-12=-264/1088, 9-10=-378/986

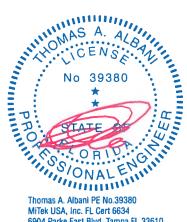
WEBS 2-12=-406/291, 3-12=-238/549, 4-12=-256/191, 4-10=-256/191, 5-10=-238/549,

6-10=-406/291, 1-13=-313/975, 7-9=-314/977

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf, BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-12 to 3-6-2, Interior(1) 3-6-2 to 11-10-0, Exterior(2) 11-10-0 to 16-10-0, Interior(1) 16-10-0 to 21-10-0, Exterior(2) 21-10-0 to 26-7-2, Interior(1) 26-7-2 to 33-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=453, 8=453.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



6904 Parke East Blvd. Tampa FL 33610 Date:

September 5,2019



\Lambda WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual trus web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



Qty Ply GILMORE RES Job Truss Type Truss T18043904 S1220 ТЗ Attic Job Reference (optional) 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:50 2019 Page 1 Duley Truss, Dunnellon, FL - 34430, | D:AMnGBqM9o3FaPdlsuvOqloyggBQ-_zv0JZRLqwmKV9fQxyWQli2vZvBfW2vX4hoLz2ygfvJ 21-10-0 22-11-12 28-2-2 33-8-0 16-10-0 11-10-0 10-8-4 5-2-6 5-0-0 5-5-14 5-2-6 Scale = 1:69.7 4x8 = 4x8 = 1.5x4 5 24 3x4 10.00 12 18 20 19 17 25 22 3x4 16 1.5x4 3x4 \ -11 4 v R = 4×4 3x5 \ 3x5 / 2-0-0 2-0-0 10 12 11 13 14 2x4 || 2x4 6x10 MT20HS = 6x10 MT20HS = 3x4 = 3x4 = 10-8-4 22-11-12 12-3-8 5-5-14 Plate Offsets (X,Y)--[1:0-1-12,0-1-8], [2:0-1-8,0-1-8], [3:0-1-8,0-1-8], [4:0-6-4,0-2-0], [6:0-6-4,0-2-0], [7:0-1-8,0-1-8], [8:0-1-8,0-1-8], [9:0-1-12,0-1-8], [10:Edge,0-3-8], 12:0-4-12,0-3-0], [13:0-4-12,0-3-0] **PLATES** GRIP LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) Vdefl L/d 244/190 Plate Grip DOL 1.25 TC 0.45 Vert(LL) -0.62 13-14 >650 240 MT20 TCLL 20.0 Lumber DOL 1.25 вс 0.98 Vert(CT) -0.64 13-14 >624 180 MT20HS 187/143 TCDL 7.0 YES WB 0.90 Horz(CT) 0.03 10 n/a n/a **BCLL** 0.0 Rep Stress Incr Attic FT = 20%BCDL 10.0 Code FRC2017/TPI2014 Matrix-MS -0.31 12-13 459 360 Weight: 274 lb **BRACING-**LUMBER-TOP CHORD Structural wood sheathing directly applied or 4-4-8 oc purlins,

BOT CHORD

JOINTS

TOP CHORD 2x4 SP No.2D **BOT CHORD**

2x4 SP No.1 *Except* 12-13: 2x8 SP No.1D

WEBS 2x4 SP No.3

REACTIONS.

(lb/size) 15=1443/0-4-0, 10=1443/0-4-0 Max Horz 15=-457(LC 10)

Max Uplift 15=-293(LC 12), 10=-293(LC 12)

Max Grav 15=1622(LC 18), 10=1622(LC 19)

FORCES. (jb) - Max. Comp./Max, Ten. - All forces 250 (jb) or less except when shown. TOP CHORD

1-2=-1712/415, 2-3=-1723/520, 3-4=-1287/566, 4-5=-1203/627, 5-6=-1203/627

6-7=-1287/566, 7-8=-1723/520, 8-9=-1713/415, 1-15=-1576/424, 9-10=-1575/424 14-15=-410/442, 13-14=-275/1526, 12-13=-92/1411, 11-12=-240/1227

BOT CHORD **WEBS** 2-14=-431/130, 2-13=-295/299, 13-16=-39/645, 3-16=0/633, 18-20=-434/127,

19-20=-434/126, 12-17=-39/645, 7-17=0/633, 8-12=-295/299, 8-11=-431/130, 1-14=-177/1223, 9-11=-178/1221, 4-18=-176/653, 6-19=-176/653, 4-20=-252/434,

5-20=-304/279, 6-20=-252/434, 3-18=-724/235, 7-19=-724/235

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-12 to 3-6-2, Interior(1) 3-6-2 to 11-10-0, Exterior(2) 11-10-0 to 16-10-0, Interior(1) 16-10-0 to 21-10-0, Exterior(2) 21-10-0 to 26-7-2, Interior(1) 26-7-2 to 33-6-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- All plates are MT20 plates unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Ceiling dead load (5.0 psf) on member(s). 16-18, 18-20, 19-20, 17-19; Wall dead load (5.0 psf) on member(s). 13-16, 12-17
- 8) Bottom chord live load (30.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 12-13
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=293, 10=293
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) Attic room checked for L/360 deflection.



except end verticals, and 2-0-0 oc purlins (5-7-4 max.): 4-6.

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Brace at Jt(s): 18, 19, 20

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September 5,2019



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not



Ply GILMORE RES Job Qty Truss Truss Type T18043905 S1220 **T4** GABLE 1 Job Reference (optional)

Duley Truss, Dunnellon, FL - 34430,

8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:52 2019 Page 1

Structural wood sheathing directly applied or 4-2-9 oc purlins,

except end verticals, and 2-0-0 oc purlins (2-7-2 max.): 4-6.

Rigid ceiling directly applied or 7-1-7 oc bracing.

13-10-0 18-4-0 27-8-0 22-10-4 28-8-0 4-6-0 4-6-0

Scale = 1:82.4

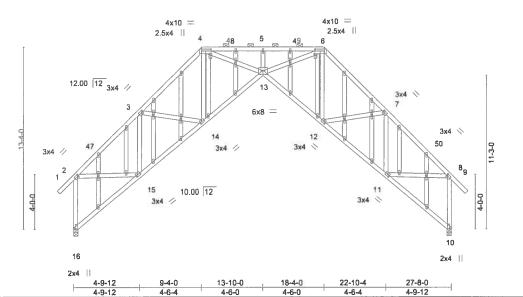


Plate Offsets (X,Y)-[11:0-1-8,0-1-8], [12:0-1-8,0-1-8], [13:0-4-0,0-3-8], [14:0-1-8,0-1-8], [15:0-1-8,0-1-8], [16:0-0-13,0-1-8], [17:0-1-12,0-0-12], [21:0-1-9,0-0-12], [24:0-1-9 0-0-12], [29:0-1-8,0-0-12], [34:0-1-12,0-0-12], [38:0-1-9,0-0-12], [41:0-1-9,0-0-12], [46:0-1-8,0-0-12]

LOADING (psf) TCLL 20.0 TCDL 7.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Page Stress lang VES	CSI. TC 0.54 BC 0.39	DEFL. in (loc) I/defl L/d Vert(LL) 0.29 13 >999 240 Vert(CT) -0.45 13 >736 180 Horz(CT) 0.80 10 n/a n/a	PLATES GRIP MT20 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code FRC2017/TPI2014	WB 0.61 Matrix-MS	Horz(CT) 0.80 10 n/a n/a	Weight: 265 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2D 2x4 SP No.2D **BOT CHORD**

2x4 SP No.3 *Except* **WEBS**

4-13,6-13: 2x4 SP No.2D

OTHERS 2x4 SP No.3

REACTIONS. (lb/size) 16=1080/0-4-0, 10=1080/0-4-0

Max Horz 16=-606(LC 10)

Max Uplift 16=-451(LC 12), 10=-451(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1558/733, 3-4=-2038/838, 4-5=-4697/1531, 5-6=-4697/1531, 6-7=-1798/790, TOP CHORD

7-8=-1320/636, 2-16=-1258/629, 8-10=-1046/638

BOT CHORD 15-16=-780/735, 14-15=-786/1772, 13-14=-711/2061, 12-13=-468/1756, 11-12=-438/1345 3-15=-681/269, 3-14=-174/343, 4-14=-273/265, 4-13=-1040/3529, 6-13=-1240/3780, WEBS

6-12=-333/326, 7-12=-240/460, 7-11=-679/290, 2-15=-252/928, 8-11=-265/913

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=28ft; eave=4ft; Cat. II: Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -1-1-4 to 1-10-12, Interior(1) 1-10-12 to 9-4-0, Exterior(2) 9-4-0 to 13-10-0, Interior(1) 13-10-0 to 18-4-0, Exterior(2) 18-4-0 to 22-10-4, Interior(1) 22-10-4 to 28-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Provide adequate drainage to prevent water ponding.
- 5) All plates are 1.5x4 MT20 unless otherwise indicated
- 6) Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 16, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 16=451, 10=451.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Thomas A. Albani PE No.39380 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

September 5,2019

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 rev 10/03/2015 BEFORE USE

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not



GILMORE RES Joh Truss Type Ply Truss Qty T18043906 S1220 T5 PIGGYBACK BASE 24 Job Reference (optional) Duley Truss,

Dunnellon, FL - 34430,

8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Sep 5 13:42:53 2019 Page 1 ID:AMnGBqM9o3FaPdlsuvOqloyggBQ-PYa9xbTE7r8vMcN?c447NKgOH7MXjUF_me1?aNygfvG

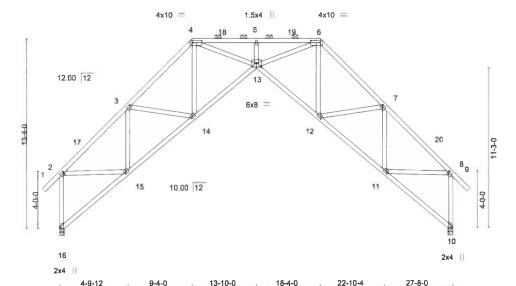
Structural wood sheathing directly applied or 4-2-9 oc purlins,

except end verticals, and 2-0-0 oc purlins (2-7-2 max.): 4-6.

Rigid celling directly applied or 7-1-7 oc bracing.

27-8-0 9-4-0 13-10-0 22-10-4 4-9-12 18-4-0 4-6-4 4-6-0

Scale = 1:79.2



4-6-4 4-6-0 4-6-4 [2:0-1-0,0-1-8], [3:0-1-4,0-1-8], [4:0-8-4,0-1-12], [6:0-8-4,0-1-12], [7:0-1-4,0-1-8], [8:0-1-0,0-1-8], [10:0-0-13,0-1-8], [11:0-1-8,0-1-8], [12:0-1-8,0-8], [12:0-1-8,0-8Plate Offsets (X.Y)-[13:0-4-0,0-3-8], [14:0-1-8,0-1-8], [15:0-1-8,0-1-8], [16:0-0-13,0-1-8]

LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.54	Vert(LL)	0.29	13	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.39	Vert(CT)	-0.45	13	>736	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.61	Horz(CT)	0.80	10	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2	2014	Matri	x-MS						Weight: 193 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.2D 2x4 SP No.2D **BOT CHORD**

2x4 SP No.3 *Except* **WEBS**

4-13.6-13: 2x4 SP No.2D

(lb/size) 16=1080/0-4-0, 10=1080/0-4-0

Max Horz 16=-606(LC 10)

Max Uplift 16=-451(LC 12), 10=-451(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1558/733, 3-4=-2038/838, 4-5=-4697/1531, 5-6=-4697/1531, 6-7=-1798/790,

7-8=-1320/636, 2-16=-1258/629, 8-10=-1046/638

15-16=-780/735, 14-15=-786/1772, 13-14=-711/2061, 12-13=-468/1756, 11-12=-438/1345 **BOT CHORD** 3-15=-681/269, 3-14=-174/343, 4-14=-273/265, 4-13=-1040/3529, 6-13=-1240/3780, WEBS

6-12=-333/326, 7-12=-240/460, 7-11=-679/290, 2-15=-252/928, 8-11=-265/913

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=28ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -1-1-4 to 1-10-12, Interior(1) 1-10-12 to 9-4-0, Exterior(2) 9-4-0 to 13-10-0, Interior(1) 13-10-0 to 18-4-0, Exterior(2) 18-4-0 to 22-10-4, Interior(1) 22-10-4 to 28-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) All plates are 3x4 MT20 unless otherwise indicated.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 16, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb)
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

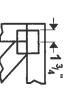
September 5,2019

Design value for use only with Art level connectors. This design is based only upon parameters shown, and is for an individual building computer, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of trusses and truss systems, see
ANSITPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

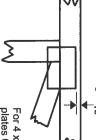


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- ¹/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

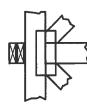
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Min size shown is for crushing only

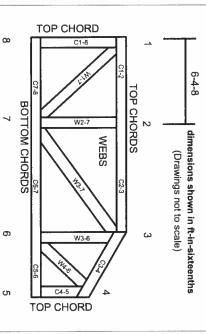
Industry Standards:

ANSI/TP11: National Design Specification for Metal
Plate Connected Wood Truss Construction.

DSB-89: BCSI:

Design Standard for Bracing.
Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

4.

- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.

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- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

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- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or after truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

Quote # S1220 Order#



(352) 465-0964

Duley Truss, Inc. P.O. Box 340 Dunnellon, FL 34430 Office: (352) 465-0964 Fax: (352) 465-0463 duleytruss@bellsouth.net

Mailing	Add	ress:_					Job Delivery Address:								
GADCO				Contact:				Name:							
				Phone:				GILMORE RES							
108 NW 1ST STREE								Addres	s:						
WILLISTON, FL 326	+														
Phone: (352) 426-25 Fax: () -		Email:				,									
			<u></u>			<u></u>									
P.O. Number:															
Designer: Ryan Sherman															
Quote # S1220			Order #									Printed	1: 09/05/19		
Bldg Code: FRC2	017/ TP	12014		Wind	Des M	lethod			Expo	sure Cat	Occupancy Ca	t Velocity	/ TC Dead / f	BC Dead	
Bldg Cat: Resid	ential		MWFRS(Di	rectional)	C-C hy	/brid Win	d ASC	E 7-10		С	ll l	140.0	00 / 4.200 /	6.000	
ROOF TRU	SSE		ADING ORMATION	TCLL-TCDL-B0		STRESS IN	CR.	ROOF	TRUS	S SPACIN	IG: 24.0 IN. O.C	. (TYP.)			
PROFILE	QTY	ТОР	ID	BASE	TOP	LEFT OH		REACTIONS							
	PLY	вот	1	O/A	вот	RIGHT OH	[
		12.00		07-10-06	2 X 4		Jt High	and the same of th	1	2		4	5 425,3		
	24	0.00	CAP1	07-10-06	2 X 4		High Low		25.3 19.6	647.4 -556.0	0.0	574.0 -556.0	-357.5	1. to 1. Val. 1 1 40	
							Loc-X Loc-Y	00-02		00-06-13 00-03-06	04-06-00 00-03-06	08-05-03 00-03-06	08-09-08 00-00-00		
		10.00		08-08-09	08-08-09 2 X 4		Jt		1	2		9	8	6	
	2 0.00		CAP2				High Low		26.1	208.5 -87.6	219,6 -146.0	97.6 0.0	218.4 -146.0	162.1 -87.6	
		0.00	OAFZ	08-08-09	2 X 4		Loc-X	00-02	2-12	00-07-11	03-00-00	05-00-00	07-00-00	09-04-05	
							Loc-Y	00-00	0-00	00-03-06	00-03-06	00-03-06	00-03-06	00-03-06	
		10.00			2 X 4	Jt High		37	74.6	628,4		576.7	374.6		
	14	0.00	CAP3		09 2 X 4		Low Loc-X	00-02	10.7	-513.2 00-07-11	0.0	-513.2 09-04-05	-340.5 09-09-04		
							Loc-X	00-00		00-03-06	00-03-06	00-03-06	00-00-00		
		12.00		07-10-06	2 X 4		Jt		1	2		9	8	6	
	2	2	0.00	CAP4	07-10-06	2 X 4		High Low		38.3 49.2	183.6 -67.1	217.1 -169.3	96.7 0.0	215.4 -169.3	130,1 -45.8
	_	0.00					Loc-X Loc-Y	00-02 00-00		00-06-13 00-03-06	02-06-00 00-03-06	04-06-00 00-03-06	06-06-00 00-03-06	08-05-03 00-03-06	
		10.00		33-08-00	2 X 4		Jt	00 00	37	36	35	34	33	32	
			T4				High		17.5	472.9	149.2	193.4	177.9	189.1 -109.0	
	2	0.00	T1	33-08-00	2 X 4		Loc-X	00-0	77.1 I-12	-315.0 01-10-00	-113.3 03-10-00	-120.3 05-10-00	-128.9 07-10-00	09-10-00	
							Loc-Y	00-01	I-12	00-01-12	00-01-12	00-01-12	00-01-12	00-01-12	
		10.00		33-08-00	2 X 4		Jt High	1.3	14	1,313.3		101017010		EDILAYENEA	
	7	0.00	T2	33-08-00	2 X 4		Low	-4!	52.9	-452.9					
							Loc-X	00-01		33-06-04 00-01-12		MAN CONTRACTOR			
		10.00		33-08-00	2 X 4		Jt		15	10					
	7		Т3	33-08-00			High Low		93.4	1,622.3 -293.4				A STATE OF THE STA	
	,	0.00	0.00		33-08-00	U 2 A 4		Loc-X	00-01	1-12	33-06-04				
							Loc-Y	00-0		00-01-12					
THE OWNER OF THE OWNER OWNER OF THE OWNER		12.00	_	27-08-00			Jt High	1,08	16 80.3	1,080.3					
-	2	10.00	T4	27-08-00	2 X 4	01-00-00	Low Loc-Y		50.6	-450.6 27-06-04			CONTRACTOR LA		
							Loc-X Loc-Y	00-0		27-06-04 00-00-06		5-6-34		CONTRACTOR CONTRACTOR	

Jt High

Low

Loc-X Loc-Y

27-08-00 2 X 4 01-00-00

27-08-00 2 X 4 01-00-00

12.00

10.00

24

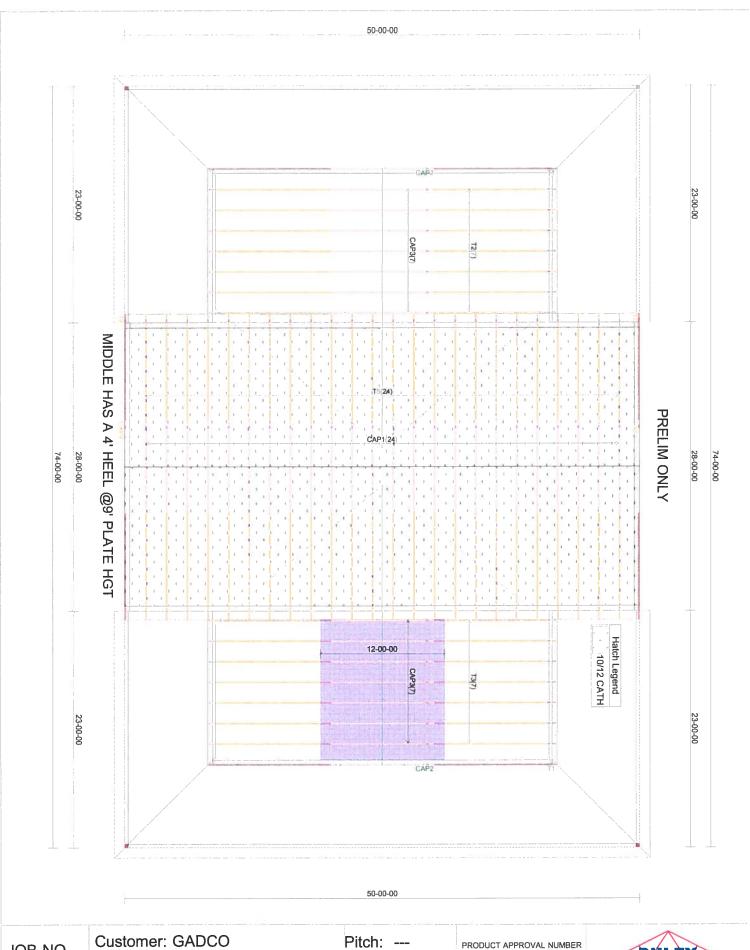
T5

16 1,080.3 -450.6 00-01-12

00-00-06

10 1,080.3 -450.6 27-06-04

00-00-06



JOB NO. **S1220**

Description: GILMORE RES Designer: Ryan Sherman

Overhang: ---

PRODUCT APPROVAL NUMBER FL 2197.4 MT20 PLATES MITEK INDUSTRIES, INC.



PROJECT SUMMARY

Short Desc: MSN Description: Moonshine

Owner:

Address1: 10089 US Hwy 27 City: Ft White

Address2: State: FL

Zip: 32113

Type: Convention Center Class: New Finished building

Jurisdiction: ALACHUA COUNTY, ALACHUA COUNTY, FL (111000)

Conditioned Area: 2420 SF Conditioned & UnConditioned Area: 2420 SF

No of Stories: 1 Area entered from Plans 2420 SF

Permit No: 0 Max Tonnage 4.7

If different, write in:

Building Rotation: 91 Deg Clockwise. Walls & windows will be rotated accordingly



Compliance Summary								
Component	Design	Criteria	Result					
Gross Energy Cost (in \$)	1,854.0	2,683.0	PASSED					
LIGHTING CONTROLS			PASSES					
EXTERNAL LIGHTING			No Entry					
HVAC SYSTEM			PASSES					
PLANT			No Entry					
WATER HEATING SYSTEMS			No Entry					
PIPING SYSTEMS			No Entry					
Met all required compliance from Check List?			Yes/No/NA					
IMPORTANT MESSAGE								
Info 5009 An input report of this design bui	iding must be subm	litted along wi	th this					

Compliance Report

Florida Building Code, Sixth Edition (2017) - Energy Conservation

EnergyGauge Summit® Fla/Com-2017, Effective Date: Dec 31, 2017 ASHRAE 90.1-2013 - Energy Cost Budget Option

	Check List								
	Applications for compliance with the Florida Building Code, Energy Conservation shall include:								
	This Checklist								
	The full compliance report generated by the software that contains the project summary, compliance summary, certifications and detailed component compliance reports.								
	The compliance report must include the full input report generated by the software as contigous part of the compliance report.								
	Boxes appropriately checked in the Mandatory Section of the complaince report.								
To incl	IING: INPUT REPORT NOT GENERATED. Jude input report in final submission, go to the Project Form, Settings Tab and check x - "Append Input Report to Compliance Output Report" Terun your calculation								

CERTIFICATIONS I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code Prepared By: David Marra Building Official: Date: 8/22/19 Date: I certify that this building is in compliance with the FLorida Energy Efficiency Code Owner Agent: Date: If Required by Florida law, I hereby certify (*) that the system design is in compliance with the Florida Energy **Efficiency Code** Architect: Reg No: Electrical Designer: Reg No: Lighting Designer: Reg No: Mechanical Designer: David Marra Reg No: CAC1818215

Reg No:

(*) Signature is required where Florida Law requires design to be performed by registered design professionals. Typed names and registration numbers may be used where all relevant information is contained on signed/sealed plans.

Plumbing Designer:

Project: MSN Title: Moonshine Type: Convention Center (WEA File: FL_GAINESVILLE_REGIONAL_AP.tm3) **Building End Uses** 1) Proposed 2) Baseline Total 119.00 172.40 \$1,854 \$2,683 ELECTRICITY(MBtu/kW 119.00 172.40 h/\$)34859 50532 \$1,854 \$2,683 **AREA LIGHTS** 13.90 45.10 4063 13208 \$216 \$701 MISC EQUIPMT 35.90 35.90 10528 10528 \$560 \$559 **PUMPS & MISC** 0.10 0.10 34 37 \$2 \$2 SPACE COOL 34.50 43,40 10107 12702 \$538 \$674 **SPACE HEAT** 3.90 5.40 1145 1594 \$61 \$85 **VENT FANS** 30.70 42.50 8982 12463 \$478 \$662 Credits Applied: None **PASSES** Passing Criteria = 2683 Design (including any credits) = 1854 Passing requires Proposed Building cost to be at most 100% of

Baseline cost. This Proposed Building is at 69.1%

		External Lig	hting C	omplianc	e		
Description	C	ategory	Tradable?	Allowance (W/Unit)	Area or Length or No. of Units (Sqft or ft)	ELPA (W)	CLP (W)
							None
	1000						
Project: MSN Fitle: Moonshine Type: Convention WEA File: FL_G	Center AINESVILLI	E_REGIONAL_AP.tm3) Lighting Cont	rols Con	npliance			
itle: Moonshine voe: Convention	Center AINESVILLI Ashrae ID	E_REGIONAL_AP.tm3) Lighting Conti	rols Con	Area (sq.ft)	Design CP	Min CP	Compli- ance
itle: Moonshine ype: Convention WEA File: FL_G Acronym	AINESVILLI Ashrae ID	Lighting Cont		Area		CP	-
Title: Moonshine Type: Convention WEA File: FL_G	Ashrae ID	Lighting Cont		Area (sq.ft)	CP	CP 1	апсе

	Systen	n Report	Compli	ance				
Pr0Sy1	System 1	•		ant Volume System < 65		No. of Unit		
Component	Category	Capacity	Design Eff	Eff Criteria	Design IPLV	IPLV Criteria	Comp- liance	
Cooling System	Air Conditioners Air Cooled Split System < 65000 Btu/h Cooling Capacity	56500	14.00	13.00	8.00		PASSE	
Heating System	Heat Pumps Air Cooled (Heating Mode) Split System < 65000 Btu/h Cooling Capacity	59000	8.20	8.20			PASSES	
Air Handling System -Supply	Air Handler (Supply) -	2000	0.80	0.82			PASSE	
_	Air Handler (Supply) -	2000	Const	0.82 ant Volume System < 65		N	PASSES	
System -Supply	Air Handler (Supply) - Constant Volume	2000 Capacity	Const	ant Volume		IPLV Criteria	o. of Unit	
System -Supply Pr0Sy2 Component	Air Handler (Supply) - Constant Volume System 2 Category Air Conditioners Air Cooled Split System < 65000 Btu/h		Const Split S	ant Volume System < 65 Eff	000 Btu/hr Design	IPLV	Comp-	
System -Supply Pr0Sy2	Air Handler (Supply) - Constant Volume System 2 Category Air Conditioners Air Cooled	Capacity	Const Split S Design Eff	ant Volume System < 65 Eff Criteria	000 Btu/hr Design IPLV	IPLV	o. of Unit	

No	Design Min Eff Eff ter Heater (Design IPLV	Min IPLV	Catego	ory	Non	ne	Comp
		Complian	nce			Non	ne .	
		Complian	nce					*
		Complian	ice					
Description Type								
	Category				Design Loss	Max Loss	Comp liance	
							None	
	Dining 6	System C						
	Pining S	System C	omnlia	nce				

None

Mandatory Re	Mandatory requirements compiled of Energy and Pacific Northwest No Adopted with permission	quirements compiled by US Department d Pacific Northwest National Laboratory. permission				
Topic	Section	Component	Description	Yes	N/A	Exempt
	1. T	o be checked	by Designer or Engineer			
Insulation	5.8.1.2	Envelope	Below-grade wall insulation installed per manufacturer's instructions.			
Insulation	5.8.1.2	Envelope	Slab edge insulation installed per manufacturerà \mathbb{C}^{TM} s instructions.			
Insulation	5.5.3.5	Envelope	Slab edge insulation depth/length.			
Insulation	6.4.4.1.5	Envelope	Bottom surface of floor structures incorporating radiant heating insulated to >=R-3.5.			
Fenestration	5.5.3.6	Envelope	U-factor of opaque doors associated with the building thermal envelope meets requirements.			
SYSTEM_SPECIFIC	6.5.1.3, 6.5.1.4	Mechanical	Air economizers provided where required (and not exempted), meet the requirements for design capacity, control signal, ventilation controls, high-limit shut-off, integrated economizer control, and provide a means to relieve excess outside air during operation.			
SYSTEM_SPECIFIC	6,5,1, 6,5,1,2, 6,5,1,3	Mechanical	Water economizers provided where required, meet the requirements for design capacity, maximum pressure drop and integrated economizer control.			
SYSTEM_SPECIFIC	6.5.1.5	Mechanical	Economizer operation will not increase heating energy use during normal operation.			
SYSTEM_SPECIFIC	6,5,2,2,1	Mechanical	Three-pipe hydronic systems using a common return for hot and chilled water are not used.			
SYSTEM_SPECIFIC	6.5.2.2.3	Mechanical	Hydronic heat pump systems connected to a common water loop meet heat rejection and heat addition requirements.			
SYSTEM_SPECIFIC	6,5.1,6	Mechanical	Water economizer specified on hydronic cooling and humidification systems designed to maintain inside humidity at >35 ŰF dewpoint if an economizer is required.			
SYSTEM_SPECIFIC	6,5,3,1,1	Mechanical	HVAC fan systems at design conditions do not exceed allowable fan system motor nameplate hp or fan system bhp.			
SYSTEM_SPECIFIC	6.5.3.1.2	Mechanical	HVAC fan motors not larger than the first available motor size greater than the bhp.			
HVAC	6,5,6,1	Mechanical	Exhaust air energy recovery on systems meeting Tables 6.5.6.1-1, and 6.5.6.1-2.			
SYSTEM_SPECIFIC	7.4.2	Mechanical	Service water heating equipment meets efficiency requirements.			
SYSTEM_SPECIFIC	7.5.2	Mechanical	Service water heating equipment used for space heating complies with the service water heating equipment requirements.		Ц	Ц
Insulation	5.8,1.2	Envelope	Above-grade wall insulation installed per manufacturer's instructions.			
Insulation	5.8.1.2	Envelope	Floor insulation installed per manufacturer's instructions.			
Controls	10.4.3	Mechanical	Elevators are designed with the proper lighting, ventilation power, and standby mode.			
SYSTEM_SPECIFIC	6,4.1.1, 6.8.1-7a	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7			
SYSTEM_SPECIFIC	6.4.1.1, 6.8.1-7b	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7			
SYSTEM_SPECIFIC	6.4.1.1, 6.8.1-7c	Mechanical	Heat Rejection Equipment: Minimum Efficiency RequirementTable 6.8.1-7			

SYSTEM_SPECIFIC	6.4.1.1, 6.8.1-7d	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7	
SYSTEM_SPECIFIC	6.5.5.3	Mechanical	Centrifugal fan open-circuit cooling towers having combined rated capacity >= 1100 gpm meets minimum efficiency requirement; Table 6.8.1-7	
SYSTEM_SPECIFIC	6.4.1.1, 6.8.1-7e	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7	
SYSTEM_SPECIFIC	6.4.1.1, 6.8.1-7f	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7	
SYSTEM_SPECIFIC	6,4,1,1,6,8,1-7g	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7.	
SYSTEM_SPECIFIC	6.4.1.1, 6.8.1-7h	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7	
SYSTEM_SPECIFIC	6,4,1,1, 6,8,1-7	Mechanical	Heat Rejection Equipment: Minimum Efficiency Requirement Table 6.8.1-7	
SYSTEM_SPECIFIC	7.5.3	Mechanical	Gas-fired water-heating equipment installed in new buildings: where a singular piece of water-heating equipment >= 1,000 kBtu/h serves the entire building, thermal efficiency must be >= 90 Et Where multiple pieces of water-heating equipment serve the building with combined rating is >= 1,000 kBtu/h, the combined input-capacity-weighted-average thermal efficiency , thermal efficiency must be >= 90 Et. Exclude input rating of equipment in individual dwelling units and equipment <= 100 kBtu/h.	
	2	2. To be chec	ked by Plan Reviewer	
Plan Review	4.2.2, 5.4.3.1.1, 5.7	Envelope	Plans and/or specifications provide all information with which compliance can be determined for the building envelope and document where exceptions to the standard are claimed.	
Plan Review	4.2.2, 6.4.4.2.1, 6.7.2	Mechanical	Plans, specifications, and/or calculations provide all information with which compliance can be determined for the mechanical systems and equipment and document where exceptions to the standard are claimed. Load calculations per acceptable engineering standards and	
Plan Review	4,2,2, 7,7,1, 10,4,2	Mechanical	handbooks. Plans, specifications, and/or calculations provide all information with which compliance can be determined for the service water heating systems and equipment and document where exceptions to the standard are claimed. Hot water system	
Plan Review	4.2.2, 8.4.1.1, 8.4.1.2, 8.7	Project	sized per manufacturer's sizing guide. Plans, specifications, and/or calculations provide all information with which compliance can be determined for the electrical systems and equipment and document where exceptions are claimed. Feeder connectors sized in accordance with approved plans and branch circuits sized for	
Plan Review	4.2.2, 9.4.3, 9.7	Interior Lighting	maximum drop of 3%. Plans, specifications, and/or calculations provide all information with which compliance can be determined for the interior lighting and electrical systems and equipment and document where exceptions to the standard are claimed. Information provided should include interior lighting power calculations, wattage of bulbs and	
Plan Review	9.7	Exterior Lighting	ballasts, transformers and control devices. Plans, specifications, and/or calculations provide all information with which compliance can be determined for the exterior lighting and electrical systems and equipment and document where exceptions to the standard are claimed. Information provided should include exterior lighting power calculations, wattage of bulbs and ballasts, transformers and control devices.	

Insulation	5,8,1,7,3	Envelope	Insulation in contact with the ground has <=0.3% water absorption rate per ASTM C272.	
Air Leakage	5.4,3.4	Envelope	Vestibules are installed where building entrances separate conditioned space from the exterior, and meet exterior envelope requirements. Doors have self-closing devices, and are >=7 ft apart (>= 16 ft apart for adjoinging floor area >= 40000 sq.ft.). Vestibule floor area <=7 50 sq.ft. or 2 percent of the adjoining conditioned floor area.	
HVAC	6.4.3.4.4	Mechanical	Ventilation fans >0.75 hp have automatic controls to shut off fan when not required.	
HVAC	6.4.3.8	Mechanical	Demand control ventilation provided for spaces >500 ft2 and >25 people/1000 ft2 occupant density and served by systems with air side economizer, auto modulating outside air damper control, or design airflow >3,000 cfm.	
HVAC	6.4.4.1.4	Mechanical	Thermally ineffective panel surfaces of sensible heating panels have insulation >= R-3,5.	
HVAC	6.5.2.3	Mechanical	Dehumidification controls provided to prevent reheating, recooling, mixing of hot and cold airstreams or concurrent heating and cooling of the same airstream.	
SYSTEM_SPECIFIC	6.5.3.1.3	Mechanical	Fans have efficiency grade (FEG) >= 67. The total efficiency of the fan at the design point of operation <= 15% of maximum total efficiency of the fan.	
SYSTEM_SPECIFIC	6,5,3,5	Mechanical	Motors for fans >= 1/12 hp and < 1 hp are electronically-commutated motors or have a minimum motor efficiency of 70%. These motors are also speed adjustable for either balancing or remote control.	
SYSTEM_SPECIFIC	6.4.3.10	Mechanical	DDC system installed and capable of providing control logic including monitoring zone and system demand for fan pressure, pump pressure, heating, and cooling; transferring zone and system demand information from zones to air distribution system controllers and from air distribution systems to heating and cooling plant controllers; automatically detecting and alerting system operator when zones and systems excessively drive the reset logic; allow operator removal of zone(s) from the reset algorithm; AND capable of trending and graphically displaying input and output points.	
SYSTEM_SPECIFIC	6,5.3.2.3	Mechanical	Reset static pressure setpoint for DDC controlled VAV boxes reporting to central controller based on the zones requiring the most pressure. Controls provide: zone damper monitoring or indicator of static pressure need, autodetection, alarm, and operator override of zones excessively triggering reset logic.	
SYSTEM_SPECIFIC	6.5.3.3	Mechanical	Multiple zone VAV systems with DDC of individual zone boxes have static pressure setpoint reset controls.	
SYSTEM_SPECIFIC	6.5.3.4	Mechanical	Multiple zone HVAC systems have supply air temperature reset controls.	
SYSTEM_SPECIFIC	6.5.4.1	Mechanical	System turndown requirement met through multiple single-input boilers, one or more modulating boilers, or a combination of single-input and modulating boilers Boiler input between 1.0 MBtu/h and 5 MBtu/h has 3.1 turndown ratio, boiler input between 5.0 MBtu/h and 10 MBtu/h has 4.1 turndown ratio, boiler input > 10.0 MBtu/h has 5.1 turndown ratio.	
HVAC	6542	Mechanical	HVAC pumping systems >10 hp designed for variable fluid flow.	
SYSTEM_SPECIFIC	6.5.4 3, 6.5.4.3.1, 6.5.4 3.2	Mechanical	Fluid flow shutdown in pumping systems to multiple chillers or boilers when systems are shut down.	

Ī		3. To be ch	ecked by Inspector		Mark!
Wattage	9.4.2	Exterior Lighting	Exterior lighting power is consistent with what is shown on the approved lighting plans, demonstrating proposed watts are less than or equal to allowed watts.	Ш	
SYSTEM_SPECIFIC	6.4.3.3.4	Mechanical	considers mass radiant slab floor temperature. Zone isolation devices and controls.		
SYSTEM_SPECIFIC	6.4.3.3.3	Mechanical	temporary operation as required for maintenance. Systems with setback controls and DDC include optimum start controls. Optimum start algorithm		
HVAC	6.4.3.3.2	Mechanical	applicable. Setback controls allow automatic restart and		
Other Equipment	10,4.1	Mechanical	device. Electric motors meet requirements where		
Controls	8,4,2	Project	allowed unless standby loss less than calculated maximum. AHJ has approved or combined connected load <150 kBtu/h. At least 50% of all 125 volt 15- and 20-Amp receptacles are controlled by an automatic control		
HVAC SYSTEM_SPECIFIC	7.5.1	Mechanical	radiant heat Combined space and water heating system not		
	6.5.8.1	Mechanical	VAV hood exhaust and supply systems, direct make-up air or heat recovery. Unenclosed spaces that are heated use only		П
HVAC	6.5.7.2	Mechanical	>5000 cfm meet replacement air, ventilation system, or energy recovery requirements. Fume hoods exhaust systems >=5,000 cfm have		
SYSTEM_SPECIFIC	6,5.7.1.4	Mechanical	system, or energy recovery requirements shown in Table 6.5.7.1.3. Kitchen hoods with a total exhaust airflow rate		
SYSTEM_SPECIFIC	6,5.7.1.3	Mechanical	flow required to meet space heating or cooling, or b) hood exhaust flow minus the available air transfer from available spaces. Kitchen hoods with a total exhaust airflow rate >5000 cfm meet replacement air, ventilation		
SYSTEM_SPECIFIC	6,5.7.1.2	Mechanical	Conditioned supply air to space with a kitchen hood shall not exceed the greater of a) supply		
HVAC	6,5.7.1.1	Mechanical	>=50% of exhaust air volume. Kitchen hoods >5,000 cfm have make up air >=50% of exhaust air volume.		
HVAC	6.5.7.1.1	Mechanical	Kitchen hoods >5,000 cfm have make up air		
SYSTEM_SPECIFIC	6.5 5.2 3	Mechanical	with manufacturers specs and control all fans to the same fan speed required for the instantaneous cooling duty. NA		
SYSTEM_SPECIFIC	6.5.5.2.2	Mechanical	controls to control the leaving fluid temperature or condensing temp/pressure of heat rejection device. Multicell heat rejection equipment with variable-speed fan drives installed that operate the maximum number of fans allowed that comply	_	
SYSTEM_SPECIFIC	6,5,5,2,1	Mechanical	hours of operation (Table 6.5.4.6). Fan systems with motors >=7.5 hp associated with heat rejection equipment to have capability to operate at 2/3 of full-speed and auto speed		
SYSTEM_SPECIFIC	6.5.4.6	Mechanical	controls or devices to reduce pump motor demand. Chilled-water and condenser water piping sized according to design flow rate and total annual		
SYSTEM_SPECIFIC	6,5,4,5,2	Mechanical	boiler systems >300,000 Btu/h. Hydronic heat pumps and water-cooled unitary air conditioners with pump systems >5 hp have		
SYSTEM_SPECIFIC	6,5,4,4	Mechanical	Temperature reset by representative building loads in pumping systems >10 hp for chiller and		

Insulation	5.8.1.7	Envelope	Exterior inculation protected assistant	
	3.0.1.7	Envelope	Exterior insulation protected against damage, sunlight, moisture, wind, landscaping and equipment maintenance activities.	
HVAC	6.4.3.7	Mechanical	Freeze protection and snow/ice melting system sensors for future connection to controls	
Air Leakage	5.4.3.1	Envelope	Continuous air barrier is wrapped, sealed, caulked, gasketed, and/or taped in an approved manner, except in semiheated spaces in climate zones 1-6.	
Air Leakage	5,4.3.2	Envelope	Factory-built and site-assembled fenestration and doors are labeled or certified as meeting air leakage requirements.	
Fenestration	5.8.2.1, 5.8.2.3, 5.8.2.4, 5.8.2.5	Envelope	Fenestration products rated (U-factor, SHGC, and VT) in accordance with NFRC or energy code defaults are used.	
Fenestration	5.8.2.2	Envelope	Fenestration and door products are labeled, or a signed and dated certificate listing the U-factor, SHGC, VT, and air leakage rate has been provided by the manufacturer.	
SYSTEM_SPECIFIC	7.4.4.1	Mechanical	Temperature controls installed on service water heating systems (<=120°F to maximum temperature for intended use).	
SYSTEM_SPECIFIC	7.4.4.2	Mechanical	Automatic time switches installed to automatically switch off the recirculating hot-water system or heat trace.	
SYSTEM_SPECIFIC	7.4.6	Mechanical	Heat traps installed on non-circulating storage water tanks.	
HVAC	6.4.1.4, 6.4.1.5	Mechanical	HVAC equipment efficiency verified. Non-NAECA HVAC equipment labeled as meeting 90.1.	
SYSTEM_SPECIFIC	6.4.1.5.2	Mechanical	PTAC and PTHP with sleeves 16 in. by 42 in. labeled for replacement only.	
HVAC	6.4.3.4.1	Mechanical	Stair and elevator shaft vents have motorized dampers that automatically close.	
HVAC	6.4.3.4.2, 6.4.3.4.3	Mechanical	Outdoor air and exhaust systems have motorized dampers that automatically shut when not in use and meet maximum leakage rates. Check gravity dampers where allowed.	
HVAC	6.4.3.4.5	Mechanical	Enclosed parking garage ventilation has automatic contaminant detection and capacity to stage or modulate fans to 50% or less of design capacity.	
HVAC	6.5.3.2.1	Mechanical	DX cooling systems >= 75 kBtu/h (>= 65 kBtu/h effective 1/2016) and chilled-water and evaporative cooling fan motor hp >= ż designed to vary indoor fan airflow as a function of load and	
HVAC	6.4.4.1.1	Mechanical	comply with operational requirements. Insulation exposed to weather protected from damage. Insulation outside of the conditioned space and associated with cooling systems is	
HVAC	6.4.4.1.2	Mechanical	vapor retardant. HVAC ducts and plenums insulated. Where ducts or plenums are installed in or under a slab, verification may need to occur during Foundation Inspection.	
HVAC	6.4.4.1.3	Mechanical	HVAC piping insulation thickness. Where piping is installed in or under a slab, verification may	
HVAC	6.4.4.2.1	Mechanical	need to occur during Foundation Inspection. Ducts and plenums sealed based on static pressure and location.	
SYSTEM_SPECIFIC	6.4.4.2.2	Mechanical	Ductwork operating >3 in, water column requires air leakage testing.	
SYSTEM_SPECIFIC	6.5.2.1	Mechanical	Zone controls can limit simultaneous heating and cooling and sequence heating and cooling to each	
SYSTEM_SPECIFIC	6.5 2.2.2	Mechanical	zone. Two-pipe hydronic systems using a common distribution system have controls to allow a deadband >=15 ŰF, allow operation in one mode for at least 4 hrs before changeover, and have rest controls to limit heating and cooling supply temperature to <=30 ŰF.	

HVAC	6,5,2,4,1	Mechanical	Humidifiers with airstream mounted preheating jackets have preheat auto-shutoff value set to activate when humidification is not required.	
HVAC	6.5.2.4.2	Mechanical	Humidification system dispersion tube hot surfaces in the airstreams of ducts or air-handling	
SYSTEM_SPECIFIC	6.5,3.2.2	Mechanical	units insulated >= R-0.5. VAV fans have static pressure sensors positioned so setpoint <=1,2 in. w.c. design pressure.	
SYSTEM_SPECIFIC	6.5.4.5.1	Mechanical	Two-position automatic valve interlocked to shut off water flow when hydronic heat pump with	
SYSTEM_SPECIFIC	6 5 6 2	Mechanical	pumping system > 10 hp is off. Condenser heat recovery system that can heat water to 85 ŰF or provide 60% of peak heat rejection is installed for preheating of service hot water.	
HVAC	6.5.7.1.5	Mechanical	Approved field test used to evaluate design air flow rates and demonstrate proper capture and	
SYSTEM_SPECIFIC	6.5.9	Mechanical	containment of kitchen exhaust systems. Hot gas bypass limited to: <=240 kBtu/h é€* 15%	
HVAC	6.4.3.9	Mechanical	>240 kBtu/h å€* 10% Heating for vestibules and air curtains include automatic controls that shut off the heating system when outdoor air temperatures > 45F. Vestibule heating systems controlled by a	
Controls	6,5,10	Mechanical	thermostat in the vestibule with setpoint <= 60F. Doors separating conditioned space from the outdoors have controls that disable/reset heating	
Controls	9.4.1.1	Interior Lighting	and cooling system when open. Automatic control requirements prescribed in Table 9.6.1, for the appropriate space type, are installed. Mandatory lighting controls (labeled as 'REQ') and optional choice controls (labeled as	
Controls	9.4.1.1	Interior Lighting	'ADD1' and 'ADD2') are implemented. Independent lighting controls installed per approved lighting plans and all manual controls	
Controls	9.4.1.2	Interior Lighting	readily accessible and visible to occupants. Parking garage lighting is equipped with required lighting controls and daylight transition zone	
Controls	9.4.1.1f	Interior Lighting	lighting. Daylight areas under skylights and roof monitors that have more than 150 W combined input power for general lighting are controlled by photocontrols.	
Controls	9.4.1.4	Exterior Lighting	Automatic lighting controls for exterior lighting installed.	
Controls	9.4.1.3	Interior Lighting	Separate lighting control devices for specific uses installed per approved lighting plans	
Wattage	9.6.2	Interior Lighting	Additional interior lighting power allowed for special functions per the approved lighting plans and is automatically controlled and separated from general lighting.	
Wattage	9.6.4	Interior Lighting	Where space LPD requirements are adjusted based on room cavity ratios, dimensions are	
Insulation	5.5.3.1	Envelope	consistent with approved plans. Roof R-value. For some ceiling systems, verification may need to occur during Framing	
Insulation	5.8.1.2, 5.8.1.3	Envelope	Inspection. Roof insulation installed per manufacturera€™s instructions. Blown or poured loose-fill insulation	
Insulation	5.8.1.1	Envelope	is installed only where the roof slope is <=3 in 12. Building envelope insulation is labeled with R-value or insulation certificate has been provided	
Insulation	5.8.1.9	Envelope	listing R-value and other relevant data Building envelope insulation extends over the full area of the component at the proposed rated R or	
Insulation	5.8.1.4	Envelope	U value. Eaves are baffled to deflect air to above the insulation.	
Insulation	5.8.1.5	Envelope	Insulation is installed in substantial contact with the inside surface separating conditioned space from unconditional space.	

Insulation	5.8.1.6	Envelope	Recessed equipment installed in building envelope assemblies does not compress the				
Insulation	5.8.1.7.1	Envelope	adjacent insulation. Attics and mechanical rooms have insulation protected where adjacent to attic or equipment access.				
Insulation	5.8.1.7.2	Envelope	Foundation vents do not interfere with insulation.				
Insulation	5,8,1,8	Envelope	Insulation intended to meet the roof insulation requirements cannot be installed on top of a suspended ceiling. Mark this requirement compliant if insulation is installed accordingly.				
SYSTEM_SPECIFIC	6.4.3.1.1	Mechanical	Heating and cooling to each zone is controlled by a thermostat control.				
HVAC	6.4.3.1.2	Mechanical	Thermostatic controls have a 5 ŰF deadband.				
HVAC	6.4.3.2	Mechanical	Temperature controls have setpoint overlap restrictions				
HVAC	6.4,3,3.1	Mechanical	HVAC systems equipped with at least one automatic shutdown control.				
SYSTEM_SPECIFIC	6,4,3,5	Mechanical	Heat pump controls prevent supplemental electric resistance heat from coming on when not needed.				
HVAC	6.4 3.6	Mechanical	When humidification and dehumidification are provided to a zone, simultaneous operation is prohibited. Humidity control prohibits the use of fossil fuel or electricity to produce RH > 30% in the warmest zone humidified and RH < 60% in				
HVAC	6,4.3.6	Mechanical	the coldest zone dehumidified. When humidification and dehumidification are provided to a zone, simultaneous operation is prohibited. Humidity control prohibits the use of fossil fuel or electricity to produce RH > 30% in the warmest zone humidified and RH < 60% in the coldest zone dehumidified.				
SYSTEM_SPECIFIC	7.4.4.3	Mechanical	Public lavatory faucet water temperature <= 110Å °F.				
SYSTEM_SPECIFIC	7.4.4.4	Mechanical	Controls are installed that limit the operation of a recirculation pump installed to maintain				
SYSTEM_SPECIFIC	7.4.5.1	Mechanical	temperature of a storage tank. Pool heaters are equipped with on/off switch and no continuously burning pilot light.				
SYSTEM_SPECIFIC	7.4.5.2	Mechanical	Pool covers are provided for heated pools and pools heated to >90ŰF have a cover >=R-12.				
SYSTEM_SPECIFIC	7.4.5.3	Mechanical	Time switches are installed on all pool heaters and pumps.				
Wattage	9,2,2,3	Interior Lighting	Interior installed lamp and fixture lighting power is consistent with what is shown on the approved lighting plans, demonstrating proposed watts are				
SYSTEM_SPECIFIC	7.4.3	Mechanical	less than or equal to allowed watts. All piping in circulating system insulated				
SYSTEM_SPECIFIC	7.4.3	Mechanical	First 8 ft of outlet piping is insulated				
SYSTEM_SPECIFIC	7.4.3	Mechanical	All heat traced or externally heated piping insulated				
4. To b	4. To be checked by Inspector at Project Completion and Prior to Issuance of						
Plan Review	6.7.2.4	Certifica Mechanical	Detailed instructions for HVAC systems commissioning included on the plans or				
Plan Review	6.7.2.4	Mechanical	specifications for projects >=50,000 ft2. Detailed instructions for HVAC systems commissioning included on the plans or				
Post Construction	6.7.2.1	Mechanical	specifications for projects >=50,000 ft2. Furnished HVAC as-built drawings submitted within 90 days of system acceptance.				

			within 90 days of system acceptance.		
Post Construction	6.7.2.3	Mechanical	An air and/or hydronic system balancing report is provided for HVAC systems serving zones >5,000		
HVAC	67.24	Mechanical	ft2 of conditioned area. HVAC control systems have been tested to ensure proper operation, calibration and adjustment of controls.		
Post Construction	8.7.1	Interior Lighting	Furnished as-built drawings for electric power systems within 30 days of system acceptance.		
Post Construction	8.7.2	Interior Lighting	Furnished O&M instructions for systems and equipment to the building owner or designated representative.		



Right-Suite® Universal 2019 Short Form Unit 1

Job: Moonshine Date: Aug 22, 2019

DJM

Project Information

For:

Moonshine

		Htg	Clg			Htg	Clg
Outside db	(°F)	33	92	Inside db	(°F)	70	72
Outside RH	(%)	-	51	Inside RH	(%)	-	50
Outside wb	(°F)	_	77	inside wb	(°F)	_	60
Daily range	(°F)	_	18	Design TD	(°F)	37	20
	V ' /			200.9	` '		
Moisture diff.	(gr/lb)	-	59				

Heating Equipment

Cooling Equipment

Unit 1

0

0

in H2O

Make Model Type Efficiency Heating Input Heating Output Humidifier Leaving Air Temp	Goodman Mfg. GSZ140601K Split ASHP 8.5 HSPF 59.0 13.2 98.7	gpd °F	Make Model Type COP / EER / SEER Sensible Cooling Latent Cooling Total Cooling Leaving Air Temp	Goodman Mfg. GSZ140601K Split ASHP 14.0 39.5 16.9 56.5 55.0	MBtuh MBtuh MBtuh °F	
Actual Heating Fan	1883	cfm	Actual Cooling Fan	1883	cfm	

Equipment Location System Type Fan Motor Heat Type

PEAKCV PACKAGE Fan & Motor Combined Efficiency Static Pressure Across Fan

NAME Area		Heat Loss	Sensible Gain	Latent Gain	Htg cfm	Clg cfm	Time	
Gathering rm	1400	30447	36226	16043	1883	1883	Aug 1800 LDT	
Unit 1	1400	30447	36226	16043	1883	1883	Aug 1800 LDT	

Job: Moonshine Date: Aug 22, 2019 By: DJM

Project Information

For:

Moonshine

Zone:	Unit 1

COOLING LOAD

1.	DESIGN CONDITION		t Aug 1800 L	DΤ	Peak load	at Aug 1800) LDT	
	inside: 72 °F	Outs		°F	TD:	20 °F	Inc wh	60 °F
	RH: 51 %	MOIS	stDiff: 58.8	gr/lb	Mult:	1.0	Ins.wb	
						Sensible	_	Latent
2.	SOLAR RADIATIO					94		-
3.	TRANSMISSION G	AINS	Sensible			94	46	-
	Walls:		268				-	-
	Glass:		32	51				-
	Doors:			0			-	-
	Partitions:			0			-	•
	Floors:			0			-	-
	Ceilings:		354	15			-	-
4.	INTERNAL HEAT	GAIN	Sensible		Latent	124	21	7140
	Occupants:		714		7140		-	-
	Lights:		528	31	_		-	-
	Motors:			0	-		-	-
	Appliances:			0	0		-	-
5.	INFILTRATION:	Ou	tside air cfm:		0		0	0
6.		ace load	Sensible	9	Latent	313	09	7140
•	Envelope		3130		7140		-	-
	Less external			0	_		-	-
	Redistribution			0	0		-	-
7.	SUPPLY DUCT						0	-
8.		Space load	+ supply due	:t		313	09	-
0.	Actual cfm:	18			17		_	-
9.	VENTILATION:		ke-up air cfm		224	49	17	8903
10.	RETURN AIR LOA		ighting + pler				0	-
11.	RETURN DUCT			(,		0	-
12.	TOTAL LOADS OF	NEOUIPA	IENT			362	26	16043
	TO THE POST OF							

HEATING LOAD

13.	DESIGN CONDITIONS			Mult:	1.0
	Inside: 70 °F	Outside:	33 °F	TD:	37 °F
14.	TRANSMISSION LOSSE	ES			13749
	Walls:		2484		
	Glass:		6329		-
	Doors:		0		-
	Partitions:		0		-
	Floors:		3185		-
	Ceilings:		1751		-
15.	INFILTRATION:	Outside air	cfm:	70	2802
16.	SUBTOTAL: Space k	oad			16551
	Envelope		16551		-
	Less external		0		-
	Less transfer		0		-
	Redistribution		0		
17.	SUPPLY DUCT:				0
18.	VENTILATION:	Make-up a	ir cfm:	224	8965
19.	HUMIDIFICATION				4931
	Piping				0
20.	RETURN DUCT		8		0
21.	TOTAL HEATING LOA	D ON EQUIE	PMENT		30447



Job: Moonshine Date: Aug 22, 2019

By: DJM

Project Information

For:

Moonshine

External static pressure
Pressure losses
Available static pressure
Supply / return available pressure
Lowest friction rate
Actual air flow
Total effective length (TEL)

Heating
0.50 in H2O
0 in H2O
0.50 in H2O
0.50 in H2O
0.378 / 0.122 in H2O
0.310 in/100ft
1883 cfm

Cooling 0.50 in H2O 0 in H2O 0.50 in H2O 0.378 / 0.122 in H2O 0.310 in/100ft 1883 cfm

162 ft

Supply Branch Detail Table

Name	2.5	esign (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	H x W (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
Gathering rm	С	4473	269	269	0.311	10.0	0x 0	VIFx	31.4	90.0	st1
Gathering rm-A	c	4473	269	269	0.320	10.0	0×0	VIFx	28.2	90.0	st1
Gathering rm-B	c	4473	269	269	0.310	10.0	0x 0	VIFx	32.0	90.0	st1
Gathering rm-D	C	4473	269	269	0.322	10.0	0x 0	VIFx	27.3	90.0	st1
Gathering rm-E	C	4473	269	269	0.331	10.0	0x 0	VIFx	24.2	90.0	st2
Gathering rm-F	C	4473	269	269	0.337	10.0	0x 0	VIFx	22.0	90.0	st2
Gathering rm-G	c	4473	269	269	0.325	10.0	0x 0	VIFx	26.2	90.0	st2

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
st1	Peak AVF	1076	1076	0.310	493	20.0	0 x 0	VinIFIx	
st2	Peak AVF	807	807	0.325	578	16.0	0 x 0	VinIFIx	

Return Branch Detail Table

Name	Grille Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	H x V (in)	V	Stud/Joist Opening (in)	Duct Matl	Trunk
rb2 rb5	0x 0 0x 0	807 1076	807 1076	39.5 39.1	0.310 0.313		16.0 20.0	0x 0x	0 0		VIFx VIFx	

Job: Moonshine Date: Aug 22, 2019

By: DJM

Project Information

For:

Moonshine

Zone:	Unit 2
ZULIC.	UIIILZ

COOLING LOAD

1.		at Aug 1800 LDT tside: 92 °F	Peak load TD:	at Sep 1600 LDT 20 °F	
		istDiff: 58.8 gr/lb	Mult:	1.0 Ins.wb	60 °F
		_		Sensible	Latent
2.	SOLAR RADIATION THRO	UGH GLASS		5516	-
3.	TRANSMISSION GAINS	Sensible		8713	3-0
	Walls:	3753		-	-
	Glass:	2177		-	-
	Doors:	201		_	_
	Partitions:	0		-	-
	Floors:	0		-	-
	Ceilings:	2583		•	-
4.	INTERNAL HEAT GAIN	Sensible	Latent	9049	5202
	Occupants:	5202	5202	-	-
	Lights:	3847	-	-	-
	Motors:	0	-	-	-
	Appliances:	0	0	-	-
5.	INFILTRATION: O	utside air cfm:	0	0	0
6.	SUBTOTAL: Space load	Sensible	Latent	23279	5202
	Envelope	23279	5202	-	-
	Less external	0	-		-
	Redistribution	0	0	-	-
7.	SUPPLY DUCT			0	-
8.		d + supply duct		23279	-
		00 at supply TD:	17	-	-
9.	VENTILATION: Ma	ake-up air cfm:	163	3582	6487
10.		Lighting + plenum (net)		0	-
11.	RETURN DUCT			0	-
12.	TOTAL LOADS ON EQUIPM	MENT		26861	11689

HEATING LOAD

13.	DESIGN CONDITIONS			Mult:	1.0
	Inside: 70 °F	Outside:	33 °F	TD:	37 °F
14.	TRANSMISSION LOSSE	ES			14413
	Walls:		3875		
	Glass:		4237		•
	Doors:		391		-
	Partitions:		0		_
	Floors:		4633		_
	Ceilings:		1276		-
15.	INFILTRATION:	Outside air		51	2041
16.	SUBTOTAL: Space k		OIIII.	01	16454
	Envelope	J	16454		70707
	Less external		0		_
	Less transfer		ñ		_
	Redistribution		ň		-
17.	SUPPLY DUCT:		o .		_
18.	VENTILATION:	Make-up air	cfm:	163	6532
19.	HUMIDIFICATION	wake-up all	Citti.	103	3593
	Piping				2093
20.	RETURN DUCT				0
21.	TOTAL HEATING LOA	D ON FOLLID	MENT		26570
ÆΙ.	TOTAL HEATING LOA	D ON EQUIP	ATE 14 I		26578

Job: Moonshine Date: Aug 22, 2019

By: DJM

Project Information

For:

Moonshine

External static pressure
Pressure losses
Available static pressure
Supply / return available pressure
Lowest friction rate
Actual air flow
Total effective length (TEL)

Heating
0.50 in H2O
0 in H2O
0.50 in H2O
0.50 in H2O
0.312 / 0.188 in H2O
0.222 in/100ft
1300 cfm

Cooling 0.50 in H2O 0 in H2O 0.50 in H2O 0.312 / 0.188 in H2O 0.222 in/100ft 1300 cfm

225 ft

Supply Branch Detail Table

Name	1	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	H x W (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
HC RR	С	1262	65	67	0.279	5.0	0x 0	VIFx	21.7	90.0	st3
Laundry	h	1484	117	100	0.305	6.0	0×0	VIFx	12.2	90.0	st3
Mech	С	633	14	33	0.313	4.0	0×0	VIFx	9.6	90.0	st3
Office	С	4909	224	259	0.222	9.0	0x 0	VIFx	50.6	90.0	st4
RR 1	С	1310	63	69	0.300	5.0	0x 0	VIFx	14.0	90.0	st3
RR 2	h	1786	141	96	0.298	7.0	0x 0	VIFx	14.5	90.0	st3
RR 3	h	1885	149	94	0.279	7.0	0x 0	VIFx	21.9	90.0	st3
Store	С	5524	263	291	0.237	10.0	0x 0	VIFx	41.8	90.0	st4
Store-B	С	5524	263	291	0.242	10.0	0x0	VIFx	38.7	90.0	st4

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
st3	Peak AVF	550	459	0.279	514	14.0	0 x 0	VinlFlx	
st4	Peak AVF	750	841	0.222	476	18.0	0 x 0	VinlFlx	

Return Branch Detail Table

Name	Grille Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	H x V (in)	٧	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1 rb3	0x 0 0x 0	998 302	939 361	84.2 84.8	0.223 0.222		18.0 12.0		0		VIFx VIFx	rt1 rt1

Return Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
rt1	Peak AVF	1300	1300	0.222	596	20.0	0 x 0	VinlFlx	

Page 3

Stats & Facts

Submit Surcharge

siness & Professional Regulation

BCIS Home







Product Approval Menu > Product or Application Search > Application List > Application Detail

FL15225-R4 Application Type Revision Code Version 2017 **Application Status** Approved

Log In User Registration Hot Topics

*Approved by DBPR. Approvals by DBPR shall be reviewed and ratified by

the POC and/or the Commission if necessary.

Comments Archived

Product Manufacturer Address/Phone/Email

Therma-Tru Corporation 118 Industrial Drive Edgerton, OH 43517 (419) 298-1740 rickw@rwbldgconsultants.com

Authorized Signature

Vivian Wright rickw@rwbldgconsultants.com

Technical Representative Address/Phone/Email

Quality Assurance Representative Address/Phone/Email

Category Subcategory Exterior Doors

Swinging Exterior Door Assemblies

Compliance Method

Certification Mark or Listing

Certification Agency Validated By

National Accreditation & Management Institute

Ryan J. King, P.E.

Validation Checklist - Hardcopy Received

Referenced Standard and Year (of Standard)

Standard Year 101/I.S.2 1997 **ASTM E1886** 2002 **ASTM E1996** 2002 ASTM E330 2002 TAS 201, 202, 203 1994

Equivalence of Product Standards Certified By

Florida Licensed Professional Engineer or Architect FL15225 R4 Equiv (a) Equivalency of Standards.pdf

Product Approval Method

Method 1 Option A

Date Submitted Date Validated Date Pending FBC Approval Date Approved

09/21/2017 12/20/2017

12/22/2017

FL#	Model, Number or Name	Description			
15225.1	a. "Classic-Craft" and "Benchmark by Therma-Tru"	6/8 and 8/0 Opaque and Glazed Fiberglass Door with and without Sidelites. Inswing and Outswing			
Impact Resistar Design Pressure Other: See INST 15225.1-80 (8'0 Door Products Dire	se outside HVHZ: Yes nt: No	Certification Agency Certificate FL15225 R4 C CAC 15225.1 NAMI certs.pdf Quality Assurance Contract Expiration Date 12/31/2021 Installation Instructions FL15225 R4 II (a) Inst 15225.1-68.pdf FL15225 R4 II (a) Inst 15225.1-80.pdf FL15225 R4 II (a) Inst 15225-68M.pdf FL15225 R4 II FL-15225.1 Glazing Detpdf Verified By: Lyndon F. Schmidt, P.E. 43409 Created by Independent Third Party: Yes Evaluation Reports FL15225 R4 AE (a) Eval 15225.1-80.pdf Created by Independent Third Party: Yes			
15225.2	b. "Construction Series" and "Benchmark by Therma-Tru"	6/8 Opaque Steel Door with and without Sidelites. Inswing and Outswing			
Impact Resistar Design Pressure Other: See INST 15225-68M (6'8 D	se outside HVHZ: Yes nt: Yes	Certification Agency Certificate FL15225 R4 C CAC 15225.2 NAMI certs.pdf Quality Assurance Contract Expiration Date 12/31/2021 Installation Instructions FL15225 R4 II (a) Inst 15225.2-68.pdf FL15225 R4 II (a) Inst FL-15225-68MA.pdf FL15225 R4 II FL-15225.2 Glazing Detpdf Verified By: Lyndon F. Schmidt, P.E. 43409 Created by Independent Third Party: Yes Evaluation Reports FL15225 R4 AE (a) Eval 15225.2-68.pdf FL15225 R4 AE (a) FL-15225-68MA.pdf Created by Independent Third Party: Yes			
15225.3	c. "Fiber-Classic" and "Benchmark by Therma-Tru"	6/8 and 8/0 Opaque and Glazed Fiberglass Door with and without Sidelites. Inswing and Outswing			
Impact Resistar Design Pressure Other: See INST 15225.3-80 (8'0 D Door Products Dire Note - Glazing Sha approval requires sidelites, which ha	e outside HVHZ: Yes nt: No	Certification Agency Certificate FL15225 R4 C CAC 15225.3 NAMI certs.pdf Quality Assurance Contract Expiration Date 12/31/2021 Installation Instructions FL15225 R4 II (a) Inst 15225.3-68.pdf FL15225 R4 II (a) Inst 15225.3-80.pdf FL15225 R4 II (a) Inst 15225-68M.pdf FL15225 R4 II (a) Inst 15225-68M.pdf FL15225 R4 II FL-15225.3 Glazing Detpdf Verified By: Lyndon F. Schmidt, P.E. 43409 Created by Independent Third Party: Yes Evaluation Reports FL15225 R4 AE (a) Eval 15225.3-68.pdf FL15225 R4 AE (a) Eval 15225.3-80.pdf FL15225 R4 AE (a) Eval 15225-68M.pdf Created by Independent Third Party: Yes			
15225.4	d. "Premium Series" and "Benchmark by Therma-Tru"	k 6/8 and 8/0 Opaque Steel Door with and without Sidelites. Inswing and Outswing			
Impact Resistan Design Pressure Other: See INST 15225.4-80 (8'0 D Door Products Dire	e outside HVHZ: Yes nt: Yes	Certification Agency Certificate FL15225 R4 C CAC 15225.4 NAMI certs.pdf Quality Assurance Contract Expiration Date 12/31/2021 Installation Instructions FL15225 R4 II (a) Inst FL-15225-68MA.pdf FL15225 R4 II (b) Inst 15225.4-68.pdf FL15225 R4 II (b) Inst 15225.4-80.pdf FL15225 R4 II (b) Inst 15225.4-80.pdf FL15225 R4 II FL-15225.4 Glazing Detpdf Verified By: Lyndon F. Schmidt, P.E. 43409 Created by Independent Third Party: Yes Evaluation Reports FL15225 R4 AE (a) FL-15225-68MA.pdf			

15225.5	e. "Smooth-Star" and "Benchmark by			
Impact Resistant: Design Pressure: Other: See INST 1: 15225.5-80 (8'0 Do Door Products Direct Note - Glazing Shall approval requires the sidelites, which have	outside HVHZ: Yes : No	without Sidelites. Inswing and Outswing Certification Agency Certificate FL15225 R4 C CAC 15225.5 NAMI certs.pdf Quality Assurance Contract Expiration Date 12/31/2021 Installation Instructions FL15225 R4 II (a) Inst 15225.5-68.pdf FL15225 R4 II (a) Inst 15225.5-80.pdf FL15225 R4 II (a) Inst 15225-68M.pdf		
15225.6	f. "Premium Series", "Construction Series" and "Benchmark by Therma- Tru" with Transoms	6/8 and 8/0 Opaque and Glazed Steel Door with Transoms. With and without Sidelites. Inswing and Outswing		
Impact Resistant Design Pressure: Other: See INST 1 15225.6-80 (8'0 Do	outside HVHZ: Yes : No	Certification Agency Certificate FL15225 R4 C CAC 15225.6 NAMI certs.pdf Quality Assurance Contract Expiration Date 12/31/2021 Installation Instructions FL15225 R4 II (a) Inst 15225.6-68.pdf FL15225 R4 II (a) Inst 15225.6-80.pdf FL15225 R4 II FL-15225.6 Glazing Detpdf Verified By: Lyndon F. Schmidt, P.E. 43409 Created by Independent Third Party: Yes Evaluation Reports FL15225 R4 AE (a) Eval 15225.6-68.pdf FL15225 R4 AE (a) Eval 15225.6-80.pdf Created by Independent Third Party: Yes		



Contact Us:: 2601 Blair Stone Road, Tallahassee FL 32399 Phone: 850-487-1824

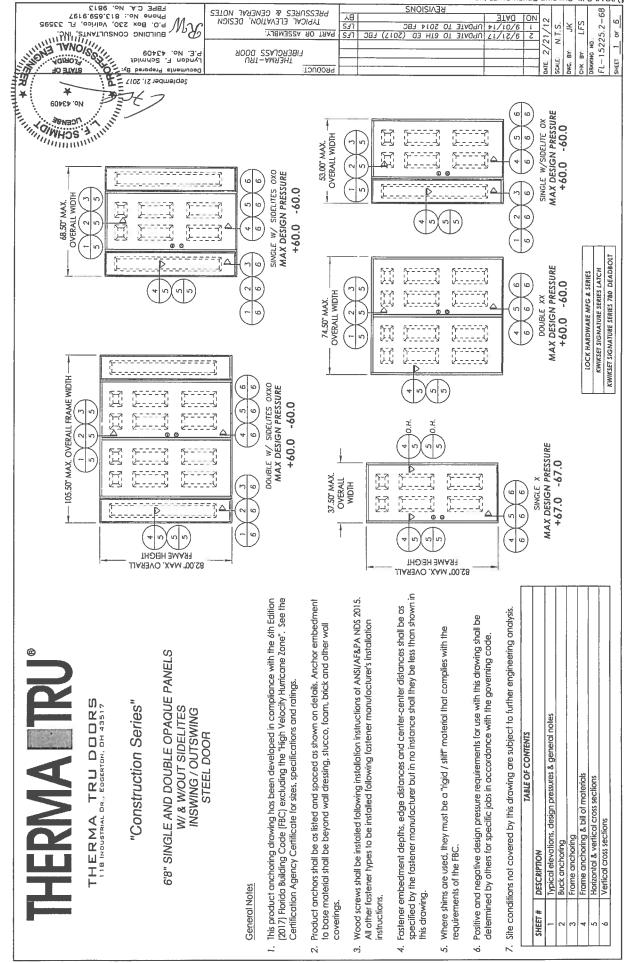
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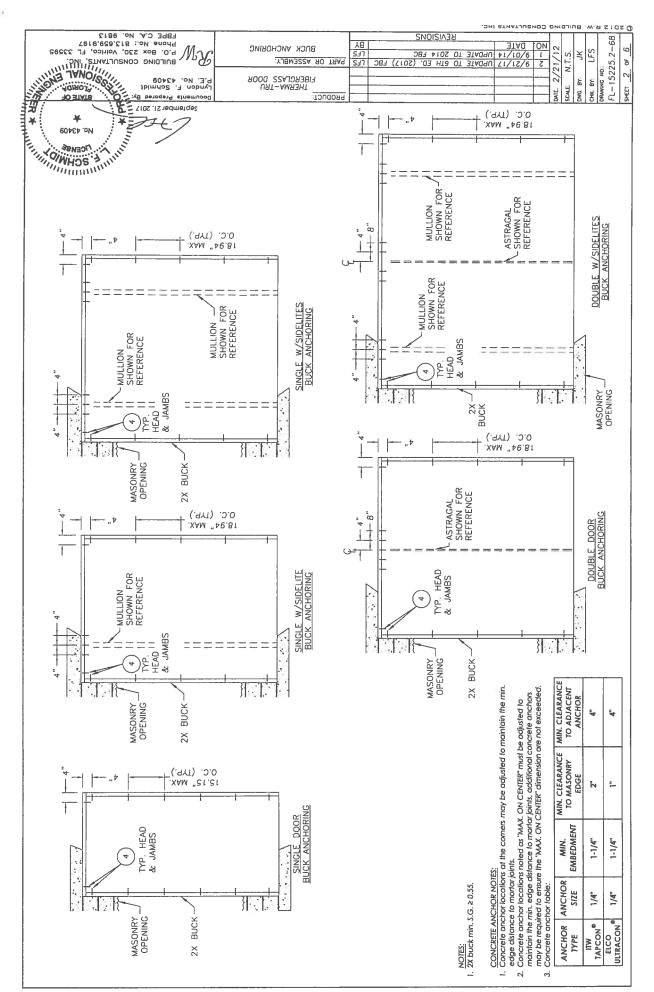
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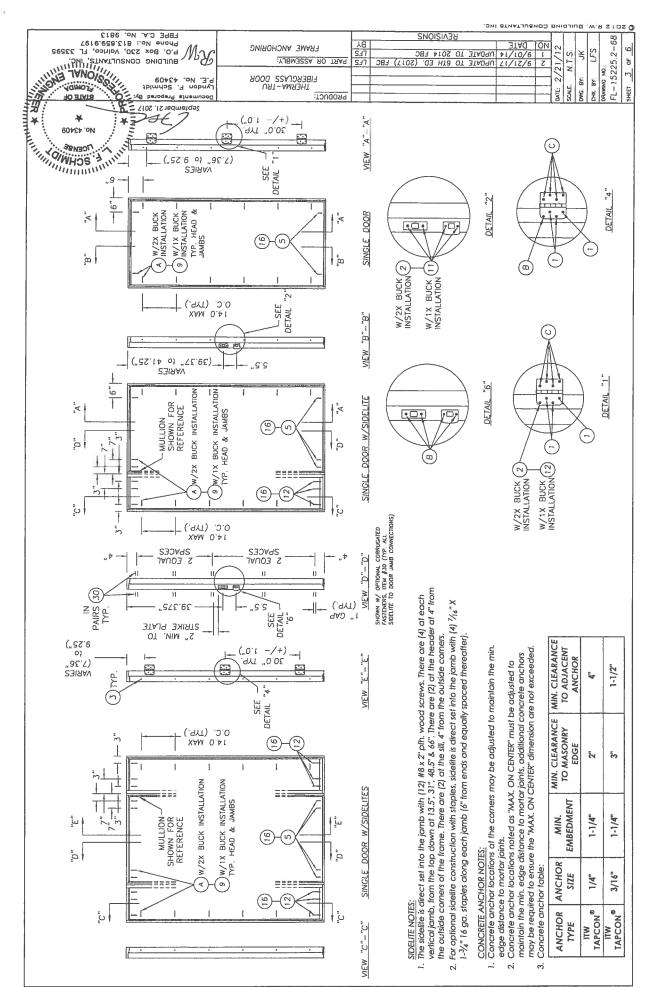


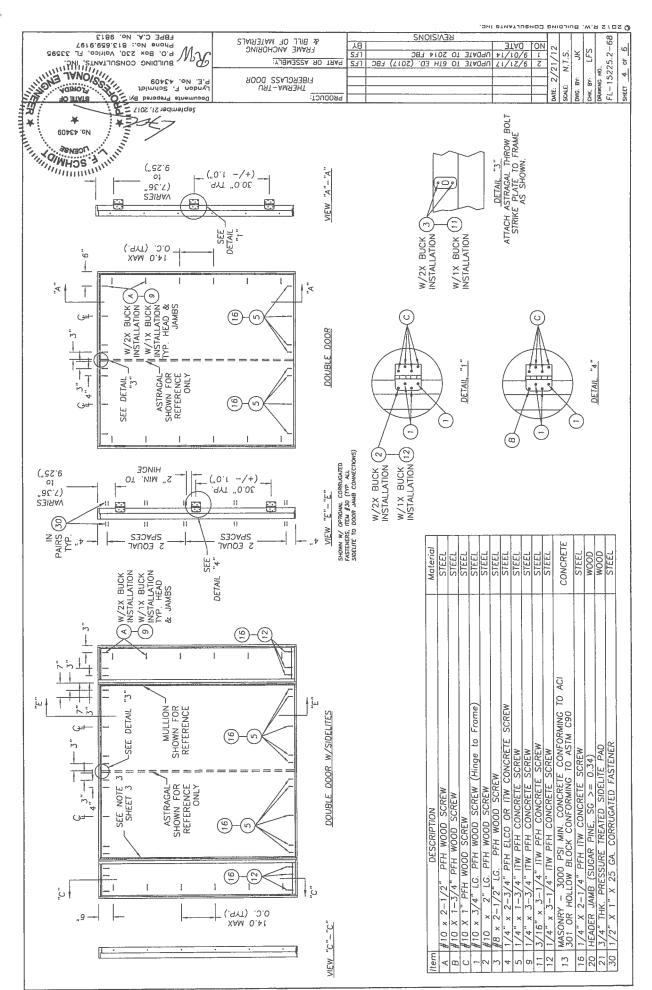
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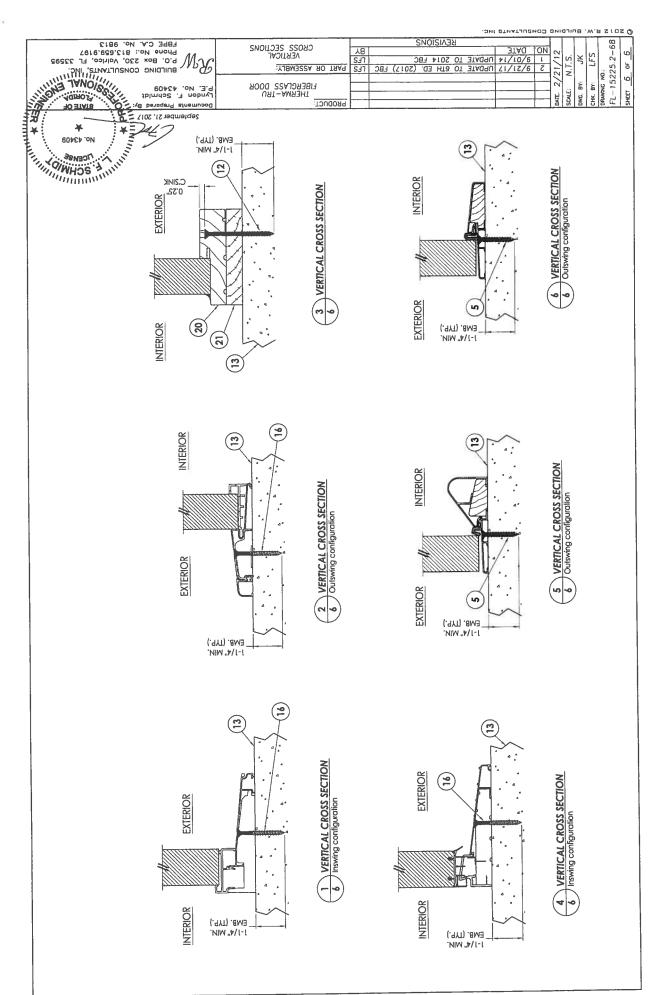
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Stats & Facts

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Product Approval Menu > Product or Application Search > Application List > Application Detail

FL# FL12952-R5 Application Type Affirmation Code Version 2017 **Application Status** Approved

Comments Archived

Product Manufacturer Pella Corporation Address/Phone/Email 102 Main St. Pella, IA 50219

(641) 621-6096

pellaproductapproval@pella.com

Beth Phelps Authorized Signature

phelpsba@pella.com

Technical Representative Joseph Hayden Address/Phone/Email 102 Main Street Pella, IA 50219

(641) 621-6096 jahayden@pella.com

Pat Bortscheller Quality Assurance Representative Address/Phone/Email 102 Main Street Pella, IA 50219

(641) 621-1000 PJBortscheller@pella.com

Category Windows Subcategory Double Hung

Compliance Method Certification Mark or Listing

Window and Door Manufacturer Association Certification Agency

Terrence E. Lunn, PE Validated By

Validation Checklist - Hardcopy Received

Referenced Standard and Year (of Standard) Standard Year 101/I.S.2/A440-08 2008

Equivalence of Product Standards

Certified By

I affirm that there are no changes in the new Florida Building Code which affect my product(s) and my product(s) are in compliance with the new Florida Building Code.

Documentation from approved Evaluation or Validation Entity

Yes No N/A Product Approval Method

Method 1 Option A

Date Submitted

12/04/2017

Date Validated

12/04/2017

Date Pending FBC Approval

Date Approved

12/07/2017

Summary of Products

FL#	Model, Number or Name	Description			
12952.1	ProLine Clad Double Hung Window	ProLine Series Non-Impact Double Hung Window (41" x 77")			
Impact Resistar Design Pressure	se outside HVHZ: Yes nt: No	Certification Agency Certificate FL12952 R5 C CAC CCL PLCDH 4577.pdf Quality Assurance Contract Expiration Date 05/05/2023 Installation Instructions FL12952 R5 II 1142.pdf Verified By: Warren W. Shaefer, P.E. 44135 Created by Independent Third Party: Yes Evaluation Reports FL12952 R5 AE 1142.pdf Created by Independent Third Party: Yes			
12952.2	ProLine Clad Double Hung Window	ProLine Series Non-Impact Double Hung Window (37" x 65")			
Impact Resistar Design Pressure	se outside HVHZ: Yes nt: No	Certification Agency Certificate FL12952 R5 C CAC CCL PLCDH 3765.pdf Quality Assurance Contract Expiration Date 05/05/2023 Installation Instructions FL12952 R5 II 1142.pdf Verified By: Warren W. Schaefer, P.E. 44135 Created by Independent Third Party: Yes Evaluation Reports FL12952 R5 AE 1142.pdf Created by Independent Third Party: Yes			
12952.3	ProLine Clad Double Hung Window	ProLine Series Non-Impact Double Hung Window (33" x 59")			
Impact Resistar Design Pressure	se outside HVHZ: Yes nt: No	FL12952 R5 C CAC CCL PLCDH 3359.pdf Quality Assurance Contract Expiration Date 05/05/2023 Installation Instructions FL12952 R5 II 1142.pdf Verified By: Warren W. Schaefer, P.E. 44135 Created by Independent Third Party: Yes Evaluation Reports FL12952 R5 AE 1142.pdf Created by Independent Third Party: Yes			
12952.4	ProLine Clad Double Hung Window	ProLine Series Non-Impact Double Hung Window (33" x 47")			
Impact Resistar Design Pressure	se outside HVHZ: Yes nt: No	Certification Agency Certificate FL12952 R5 C CAC CCL PLCDH 3347.pdf Quality Assurance Contract Expiration Date 05/05/2023 Installation Instructions FL12952 R5 II 1142.pdf Verified By: Warren W. Schaefer, P.E. 44135 Created by Independent Third Party: Yes Evaluation Reports FL12952 R5 AE 1142.pdf Created by Independent Third Party: Yes			
12952.5	ProLine Clad Double Hung Window	ProLine Series Non-Impact Double Hung Window (29" x 53")			
Impact Resistar	se outside HVHZ: Yes nt: No	Certification Agency Certificate FL12952 R5 C CAC CCL PLCDH 2953.pdf Quality Assurance Contract Expiration Date 05/05/2023 Installation Instructions FL12952 R5 II 1142.pdf Verified By: Warren W. Schaefer, P.E. 44135 Created by Independent Third Party: Yes Evaluation Reports FL12952 R5 AE 1142.pdf Created by Independent Third Party: Yes			

12952.6	ProLine Clad Double Hung Window	ProLine Series Non-Impact Double Hung Window (29" x 65")			
Impact Resistan Design Pressure	e outside HVHZ: Yes it: No	Certification Agency Certificate FL12952 R5 C CAC CCL PLCDH 2965.pdf Quality Assurance Contract Expiration Date 05/05/2023 Installation Instructions FL12952 R5 II 1142.pdf Verified By: Warren W. Schaefer, P.E. 44135 Created by Independent Third Party: Yes Evaluation Reports FL12952 R5 AE 1142.pdf Created by Independent Third Party: Yes			

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Product Approval Accepts:





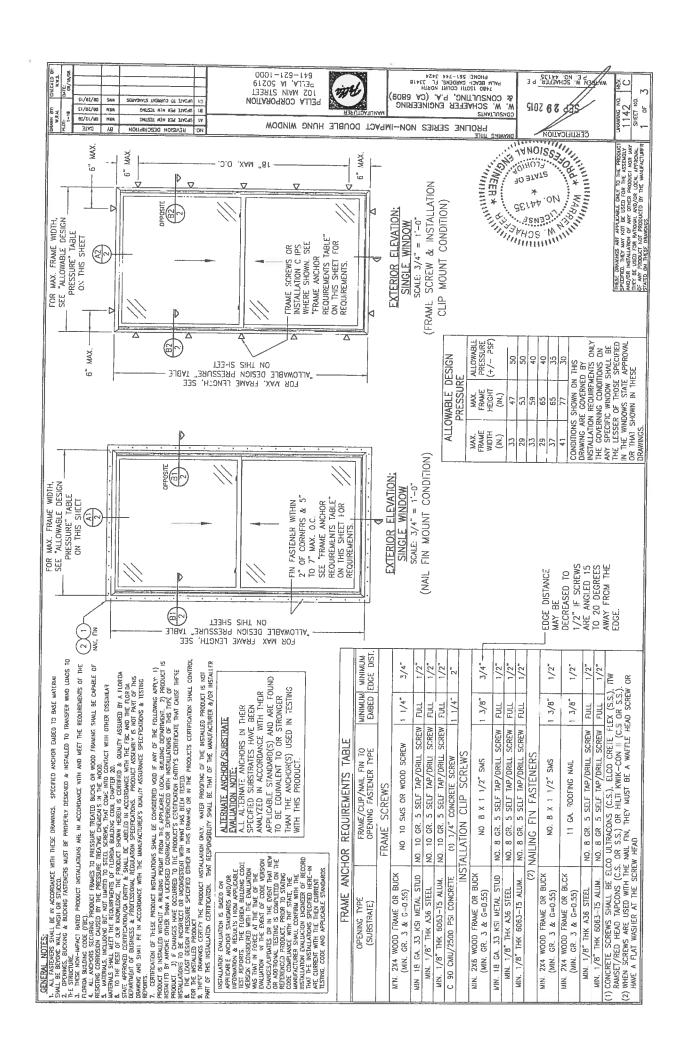


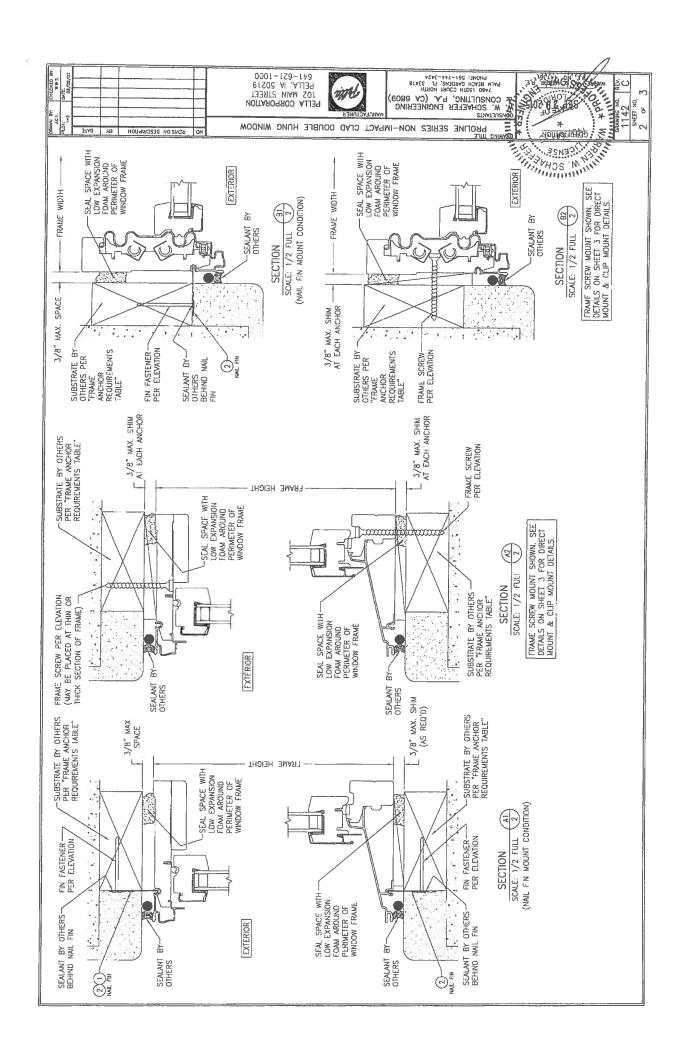


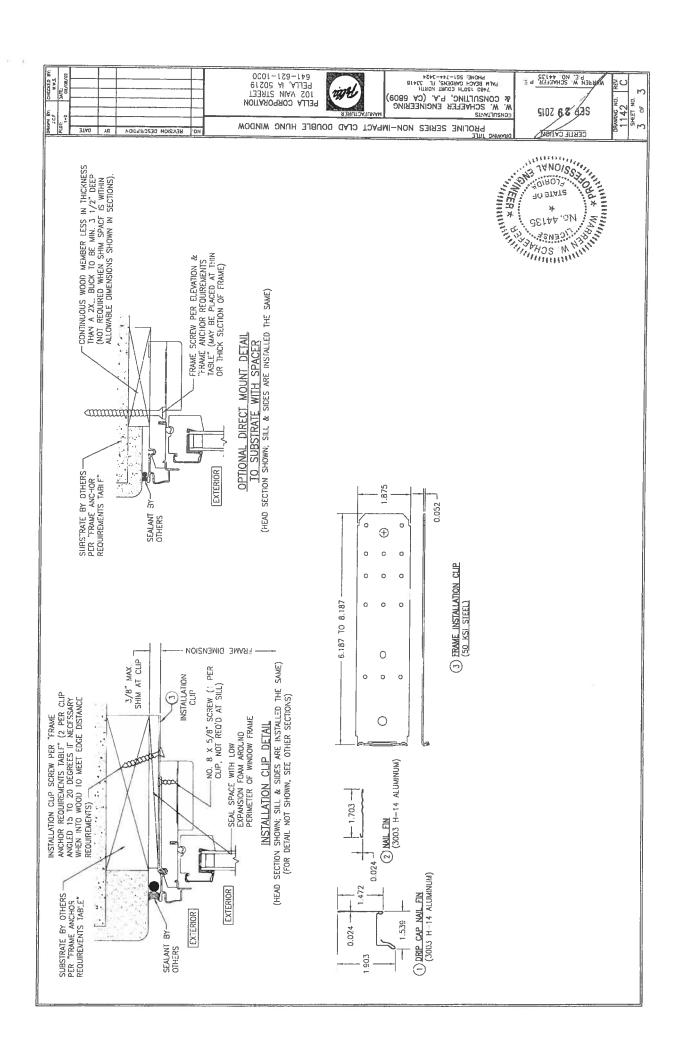
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Application Type Code Version Application Status FI 13815-R3

Revision 2017 Approved

Comments Archived

Product Manufacturer Address/Phone/Email

Pella Corporation 102 Main St. Pella, IA 50219 (641) 621-6096

pellaproductapproval@pella.com

Authorized Signature

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Quality Assurance Representative

Address/Phone/Email

Pat Bortscheller 102 Main Street Pella, IA 50219 (641) 621-1000 PJBortscheller@pella.com

Category Subcategory Windows Mullions

Compliance Method

Evaluation Report from a Florida Registered Architect or a Licensed Florida

Professional Engineer

Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed the Robert J. Amoruso

Evaluation Report

Florida License

PF-49752

Quality Assurance Entity

Architectural Testing, Inc., an Intertek Company

Quality Assurance Contract Expiration Date

12/31/2020

Validated By

Charles Pagen, Fenestration Technology, LLC

Validation Checklist - Hardcopy Received

Certificate of Independence

FL13815 R3 COI CI 1678 Rev1 415-0309 Mullion ss.pdf

Referenced Standard and Year (of Standard)

Standard AAMA 450 Year 2010

Equivalence of Product Standards

Certified By

Sections from the Code

Product Approval Method

Method 1 Option D

Date Submitted01/26/2018Date Validated01/29/2018Date Pending FBC Approval02/04/2018Date Approved04/10/2018

Summary of Products

FL#	Model, Number or Name	Description		
13815.1	Pella Clad and Wood Windows	Joining Mullion with End Anchors - Window to Window Combinations		
Approved for use Impact Resistate Design Pressur Other: Compliant	re: N/A nce limited to Wind Zone 3 or less where basic not exceed 140 mph. See page 2 of report for	Installation Instructions F13815 R3 II Pella Report No. 0507-04 060110 ss.pdf Verified By: Robert J. Amoruso PE-49752 Created by Independent Third Party: Yes Evaluation Reports F13815 R3 AE PER 1673 Pella Rev2 ss.pdf Created by Independent Third Party: Yes		
13815.2 Pella Clad and Wood Windows and Doors		Joining Mullion with End Anchors - Door to Window and Door to Door Combinations		
2301312		Installation Instructions FL13815 R3 II Pella Report No. 0506-06 060710 ss.pdf Verified By: Robert J. Amoruso PE-49752 Created by Independent Third Party: Yes Evaluation Reports FL13815 R3 AE PER 1678 Pella Rev2 ss.pdf		





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Pella Corporation

102 Main Street Pella, Iowa 50219

PROJECT: **MULLION CALCULATIONS** FOR VERTICAL AND HORIZONTAL MULLION

SERIES: Joining Mull for Doors with Transoms and/or Sidelights (Tight Mulls)

PROJECT NO.: 415-0309 REPORT NO.: 0506-6 DATE: 5/26/2015

ROBERT J. AMORUSO, P.E. FL. NO. 49752 F.B.P.E. CERTIFICATE OF AUTHORIZATION NO. 25935

Digitally signed by Robert

DN: cn=Robert J. Amoruso, o, ou=PTC Product Design Group, email=robert@ptc-

Date: 2015.05.26 10:57:06

J. Amoruso

-04'00'

corp.com, c=US

Longwood, Florida 32752 Phone: (321) 690-1788 Fax: (321-) 690-1789

PTC, PRODUCT DESIGN GROUP

Prepared by:

P.O. Box 520775

FL. NO. 49752



P.O. Box 520775 Longwood, Florida 32752 Client: Pella Corporation

Project #: 415-0309 Report #: 0506-6

Date: 5/26/2015

Page 2 of 13

Prepared by: RJA

SCOPE PTC, PRODUCT DESIGN GROUP was contracted by Pella Corporation to perform engineering evaluations of various mulled door and window combinations. This report will be based on joining the properties of the weakest wood frame (Inswing Door) jambs as demonstrated in the section property section of this report, by evaluation of possible groupings of mullions based on section property comparisons in accordance with the AAMA 450-06/09/10. Structural analysis of the mullions is based on the current edition of the Florida Building Code – Building and Residential Volumes, Chapter 17 and Chapter 6 respectively using the product grouping method and tributary loading of AAMA 450.

PRODUCT APPLICABILITY: This report is applicable to the following Pella Corporation product lines and types only.

1. Product Lines

- a. Architect Series (including HurricaneShield, 850 Series), exterior clad and wood.
- Designer Series (750 Series), exterior clad
- c. Proline Series (450 Series), exterior clad
- d. Pella Support Products, exterior clad
- e. Pella Entry Systems, exterior clad and wood

2. Product Types

- a. Casement
- b. Awning
- c. Picture
- d. Direct Set Geometric and Radius
- e. Sliding Patio Door
- f. In-swing Patio Door
- g. Out-swing Patio Door
- h. In-swing Entry Door
- i. Out-swing Entry Door

3. Product Materials

- a. Extruded aluminum for aluminum portions of door and windows.
- b. Pine for wood portions of door and window frames.

The tributary areas are identified and calculated. These results are used to calculate the maximum pressure that the product can withstand without exceeding the allowable stresses or deflection. The minimum of these results is selected and defined as the design pressure for the product. Mullion stress and deflection is calculated using the AAMA 450 load distribution method.

For each mullion configuration, spreadsheet programs are used to calculate the apparent EI and equivelent I, a design pressure verification using equivelent I, and alternate spans from the design pressure calculation.



P.O. Box 520775 Longwood, Florida 32752 Client: Pella Corporation Project #: 415-0309 Report #: 0506-6 Prepared by: RJA

Date: 5/26/2015

Page 3 of 13

ANALYSIS METHODOLOGY:

The following methodology was used to develop the Load width span tables presented in this report.

- 1. Determine the allowable design pressure (D.P.) based on the actual shear stress, bending stress, deflection and mullion reactions as compared to their respect design allowables choosing the worst case.
- 2.. Worst case section properties determined by a comparison of the individual component sections of the applicable products, the cross sections exhibiting the lowest Moment of Inertia (I) and section Modulus (Z) will be used. Area as it is related to shear stress does not govern design so it is not a deciding factor of the worst case section.
- 3. Using a ratio of the wood window jambs Modulus of Elasticity (I) compute the overall apparent section properties of the worst case cross section.
- 4. Use the properties determined above in determining the load width/span tables.
- 5. Analysis based on:
 - a. Horizontal Shear Stress = P/A (psi)
 - b. Bending Stress = Mc/I = M/Z (psi)
 - i. M = bending moment and is equal to M/Kf
 - ii. Kf is moment coefficient based on trapezoidal load distribution.
 - c. Deflection = WL 3 / KdEI
 - i. Kd is deflection coefficient based on trapezoidal distribution.
 - ii. W is load on mullion in Lbs. = wL where w is distributed load in Lbs/in and L in mullion span inches.
 - d. Mullion End Load Reactions
 - Based on trapezoidal load on mullion resolved into total load distributed to each end of mullion equally.
- 6. Load Width/Span Tables based on:
 - a. Solving for the allowable design pressure (D.P.) considering various window sizes and mullion spans.



P.O. Box 520775 Longwood, Florida 32752 Client: Pella Corporation Project #: 415-0309

Report #: 0506-6 Prepared by: RJA

Date: 5/26/2015

Page 4 of 13

MULLION END ANCHORAGE:

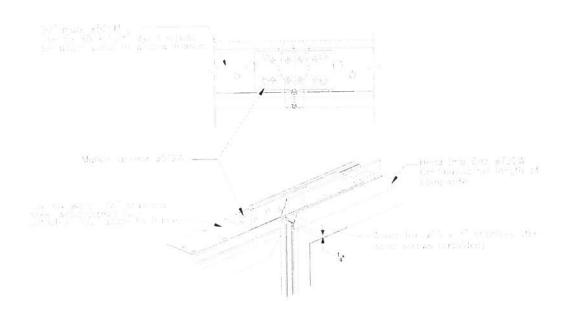
1. Calculated performance is based on the following:

Mullions anchor plates

- a. 512-A Wood mullion anchor clip. (316 stainless steel)
- b. 5071 installation type clip (A-653 Hot dipped Galvanized steel)
- c. 500 P steel plate and through frame alternate end anchorage.

MULLION ATTACHEMENT:

- 2. a. Mullion End Clip 512-A (Fin mull anchor clip)
 - i. The end clip is secured to each end of window jambs using eight #10 x 5/8" wood screws below, and requires a minimum of 1-1/2" wood buck thickness (See Detail A).
 - ii For 20" plate attachment to both window frames use six (6) #6 x 5/8" stainless steel screws.
 - iii. For clip attachment to wood frame substrates use five (5) #10 x 2" long or longer wood screws per clip leg into the installation bucking or frame with a minimum of 1 1/2" embedment into substrate and maintaining at least 3/4" minimum edge distance is required based on the analysis used in this report for development of the load width and span.



Mullion End Clip 512-A (Fin mull anchor clip) Detail A



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Longwood, Florida 32752

Client: Pella Corporation Project #: 415-0309

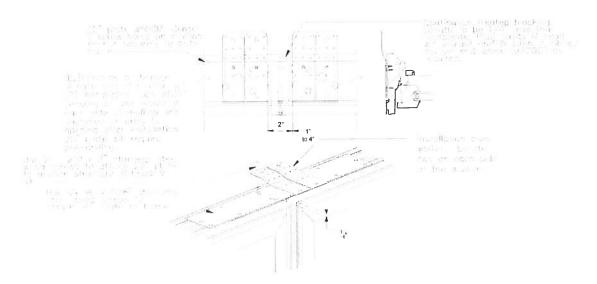
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Date: 5/26/2015

Page 5 of 13

f. Mullion End Clip 5071

- i. The end clips are secured to each end of joined widow frames using a 6-1/2" steel plate #500M installed directely over both frames, then install two (2) 5071 mull clips two at each side of the joined window frames, (See Detail B)
- ii For 6-1/2" plate attachment to both window frames use six (6) #6 x 5/8" stainless steel screws.
- iii. For clip attachment to wood frame substrates use a total of four (4) #10 x 2" long or longer wood screws one per each clip leg into the installation bucking or frame with a minimum of 1 1/2" embedment into substrate and maintaining at least 3/4" minimum edge distance, for clip attachment to window frames through plate use two (2) #5/8" stainless steel screws per each clip is required based on the analysis used in this report for development of the load width span.
- iv For clip attachment to CMU or concrete substrates use a total of four (4) 3/16" dia. masonry anchors one (1) per each clip leg into the installation bucking or frame with a minimum of 1 1/4" embedment into substrate and maintaining at least 2 5/8" minimum edge distance each clip anchor hole spacing must be located at least on 2 1/4" spacing between anchors, for clip attachment to window frames through plate use two (2) #10 x 5/8" stainless steel screws per each individual clip is required based on the analysis of this report for development of the load width span.



Detail B Mullion End Clip 5071



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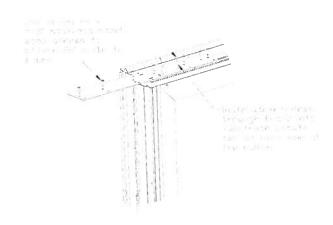
Report #: 0506-6 Prepared by: RJA

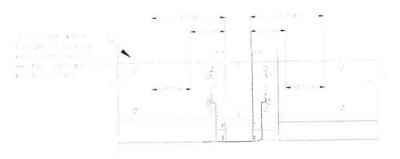
Date: 5/26/2015

Page 6 of 13

- g. Alternate, Through 500 P steel plate and main frame at mullion sill end.
 - i. For 20" plate attachment to both main frame members use seven (7) #6 x 5/8" stainless steel wood screws and two (2) #10 x 3" stainless steel wood screws to attach the 500 P plate directly to mullion structure at sill end (See detail C).
 - For anchor attachment to wood frame substrates use a total of four (4) #10 x 2" long or longer wood screws located 2" and 4-1/4" from each mullion end into the installation bucking or frame with a minimum embedment of 1-1/2"and maintaining at least 3/4" minimum edge distances is required based on the analysis used on this report for the development of the load width and span tables.
 - iii. For anchor attachment to cmu or concrete substrates use a total of four (4) 3/16" diam. Masonry anchors located 2" and 4-1/4" from each mullion end into the installation bucking or frame with a minimum embedment of 1-1/4" and maintaining at least 2-5/8" minimum edge distances is required based on the analysis used on this report for the development of the load width and span tables.

Detail C Through Plate and Frame anchorage





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Longwood, Florida 32752

Client: Pella Corporation

Project #: 415-0309 Report #: 0506-6 Prepared by: RJA

Date: 5/26/2015

Page 7 of 13

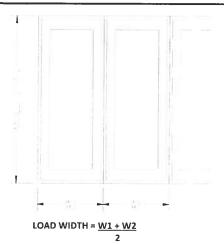
REFERENCED DOCUMENTS:

- 1. AAMA 450-06/09/10
- 2. Florida Building Code
 - Current Edition of the Florida Building Code (FBC), Building Volume
 - Chapter 17 Mullions
 - Current Edition of the Florida Building Code (FBC), Residential volume b.
 - Chapter 6 Mullions



P.O. Box 520775 Longwood, Florida 32752 Client: Pella Corporation Project #: 415-0309 Report #: 0506-6 Prepared by: RJA Date: 5/26/2015

Page 8 of 13



THIS CHART APPLIES TO MULTIPLE WIDE 1 HIGH, VERTICALLY.

LOAD WIDTH MUST BE CALCULATED USING LARGEST ADJACENT (SIDE BY SIDE) DOOR HEIGHTS OR WIDTHS.

MULL SPANS ARE RESTRICTED BY THE LONGEST SPAN, (All TABLES).

WINDOWS MAY BE INTERMIXED COMBINATIONS OF CASEMENT, AWNING, PICTURE, IN-SASH AND DIRECT SET GEOMETRIC OR RADIUS, (ALL TABLES).

ALLOWABLE PRESSURES ON THE MULLED ASSEMBLY SHALL BE CONTROLLED BY THE LESSER OF MULLION OR THE INDIVIDULE WINDOW UNIT.

* Use this table for intermediate 2-way ver	cical mullions.
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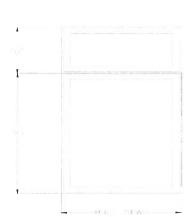
^{*} For anchor clip attachment to substrates, see notes on sheets 4 & 5 on this report.

	MULLION TABLE 1								
	DOOR TO DOOR OR DOOR TO WINDOW								
(VERTICAL SPAN—JOINING Mull)									
MULL SPAN	LOAD WIDTH	CLIP TYPE 512A (PSF)	CLIP TYPE 5071 (PSF)	MULL SPAN	LOAD WIDTH	CLIP TYPE 512A (PSF)	CLIP TYPE 5071 (PSF)		
	30	82	82		30	30	30		
60	36	71	71	96	36	26	26		
"	42	64	64	50	42	22	22		
	48	60	60		48	20	20		
	30	67	67		30	26	26		
66	36	58	58	102	36	_ 22	22		
00	42	51	51	102	42	19	19		
	48	47	47		48	17	17		
	30	56	56	108	30	22	22		
72	36	48	48		36	19	19		
/2	42	42	42		42	16	16		
	48	38	38		48	14	14		
	30	47	47		30	19	19		
78	36	40	40	114	36	16	16		
′°	42	35	35	114	42	14	14		
	48	32	32		48	12	12		
	30	40	40		30	16	16		
84	36	34	34	120	36	13	13		
04	42	30	30	120	42	12	12		
	48	27	27		48	10	10		
	30	35	35						
90	36	29	29						
טפ	42	26	26						
	48	23	23						

^{*} where installation applications do not permit the use of clip type 5071 an alternate through frame mullion anchorage may be substituted see detail "C" on sheet 6 of this report.



P.O. Box 520775 Longwood, Florida 32752 Client: Pella Corporation Project #: 415-0309 Report #: 0506-6 Prepared by: RIA Date: 5/26/2015 Page 9 of 13



LOAD WIDTH = $\frac{W1 + W2}{2}$

THIS CHART APPLIES TO 1 WIDE 1 HIGH, HORIZONTALLY.

LOAD WIDTH MUST BE CALCULATED USING LARGEST ADJACENT (SIDE BY SIDE) DOOR AND WINDOW HEIGHTS OR WIDTHS.

MULL SPANS ARE RESTRICTED BY THE LONGEST SPAN, (All TABLES).

WINDOWS MAY BE INTERMIXED COMBINATIONS OF CASEMENT, AWNING, PICTURE, IN-SASH AND DIRECT SET GEOMETRIC OR RADIUS, (ALL TABLES).

ALLOWABLE PRESSURES ON THE MULLED ASSEMBLY SHALL BE CONTROLLED BY THE LESSER OF MULLION OR THE INDIVIDULE DOOR UNIT.

- * Use this table for intermediate 2-way horizontal mullions.
- * For anchor clip attachment to substrates, see notes on sheets 4 & 5 on this report.
- * Where installation applications do not permit the use of clip type 5071 an alternate through frame mullion anchorage may be substituted, see detail "C" on sheet 6 on this report.

	MULLION TABLE 2										
	DOOR TO WINDOW (HORIZONTAL SPAN—JOINING MULLION FOR DOOR TO WINDOW UNITS)										
		CLIP	CLIP			CLIP	CLIP				CLIP TYPE
MULL	LOAD	TYPE	TYPE	MULL	LOAD	TYPE	TYPE	MULL	LOAD	512A	5071
SPAN	WIDTH	512A	5071	SPAN	WIDTH	512A	5071	SPAN	WIDTH	(PSF)	(PSF)
\vdash	30	(PSF) 85	(PSF) 85		30	(PSF) 85	(PSF) 85		30	31	31
	36	85	85		36	77	77		36	27	27
	42	85	85		42	69	69		42	23	23
	48	85	85		48	63	63		48	21	21
30	54	85	85	66	54	59	59	96	54	19	19
	60	85	85	1	60	57	57		60	18	18
	66	85	85		66	57	57		66	17	17
	72	85	85		72	57	57		72	16	16
	30	85	85		30	84	84		30	26	26
	36	85	85		36	72	72		36	22	22
	42	85	85		42	64	64		42	19	19
36	48	85	85	68	48	59	59	102	48	17	17
50	54	85	85	"	54	55	55	-01	54	16	16
	60	85	85		60	53	53		60	15	15
	66	85	85		66	52	52		66	14	14
	72	85	85	<u> </u>	72	52	52	\vdash	72	13	13
	30	85	85		30	74	74		30	22	22
	36		85 85		36	63	63	108	36	19	19
42	42	85	85	72	42	56	56 51		42 48	16 14	16 14
	48 54	85 85	85 85		48 54	51 48	48		54	13	13
	60	85	85		60	45	45		60	12	12
	66	85	85		66	44	44		66	11	11
	72	85	85		72	44	44		72	11	11
	30	85	85	\vdash	30	60	60		30	19	19
	36	85	85	1	36	51	51		36	16	16
	42	85	85	1	42	45	45		42	14	14
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48	54	85	85	78	54	38	38	114	54	11	11
	60	85	85		60	36	36		60	10	10
	66	85	85		66	35	35		66	9	9
	72	85	85	$oxed{oxed}$	72	34	34		72	9	9
	30	85	85		30	48	48		30	16	16
	36	85	85		36	41	41		36	13	13
	42	85	85		42	36	36		42	12	12
54	48	85	85	84	48	32	32	120	48	10	10
	54	85	85		54	30	30		54	9	9
	60	85	85		60	28	28		60	9	9
	66	85	85		66	27	27		66	8	8
	72	85	85		72	26	26		72	7	7
	30	85	85		30	38	38				
	36	85	85		36	33	33				
	42	85	85		42	29	29				
60	48	80	80	90	48	26	26				
	54	76 75	76 75		54	24	24				
	60 66	75 75	75		60 66	21	21				
	72	75	75		72	20	20				
	/ 4	/3	/3		,,	20	20	I			



Client: Pella Corporation Project #: 415-0309 Report #: 0506-6 Prepared by: RJA Date: 5/26/2015

Page 10 of 13

Longwood, Florida 32752

Product Grouping:

- Architect Clad HurricaneShield (Windzone 3 only), Architect, Designer and Proline Clad combination assemblies 1. with Joining mullion in intermixed configurations of casement, Awning, Picture, In-sash or Direct set Geometric or Radius.
- 2. Product type grouping: operable vs. fixed window load distribution was considered in the determination of the theoretical stress and deflection analysis due to mullion bending load and how concentrated loads produce higher bending stresses and deflections.

Section Property Grouping:

- The casement and inswing door type frame jambs combined produce the lowest overall section properties as joined section properties The wood jambs joined mechanically with anchors act as one member.
- The illistration below shows the individule component section properties for the casement and inswing door jamb. 2.
- The intermixed combinations of Casement, Awning, Picture, In-Sash, and Direct Set geometric or Radius were 3. examined, the combined section properties calculated using AutoCad (independent of material stiffnesses), and based on comparison of these section properties the combination of units that produce the weakest section determined. This section, combined was used in the analysis.

JOINED COMPOSITE SECTION PROPERTIES

Door to Window

In-swing door jamb with casement jamb used in door to window or door to door mull analysis is shown at left; Only the wood jambs, wood side stops are considered in the calculation of section properties. The wood covers are not considered.

The combined section properties for the above, considering the material differences were found to be the following.

ı			
	l =	10.579 in4	
	S ==	4.904 in3	
ı	۸	0.114 in 2	

(In terms of wood)

The individule wood jambs, wood side stops section properties used to determine the combined section properties are given on the following page.



P.O. Box 520775 Longwood, Florida 32752 Client: Pella
Project #: 415-0309
Report #: 0506-6
Prepared by: RJA
Date: 5/26/2015
Page 11 of 13

Joining Mulf, 2-Way (VT or HZ)

Wood Side Stop (VT or HZ)

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There is a second of the contract of the contr
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wood Jamb (VT or HZ)

The setter 13 Mole 2 Start 1 S

Wood Door Jamb and Stop (VT or HZ)
Composite Properties

The property of the property o



Client: Pella Corporation Project #: 415-0309 Report #: 0506-6 Prepared by: RJA Date: 5/26/2015 Page 12 of 13

SECTION PROPERTY COMPARISON CHART (VT. & HZ.)

	Minimum of Compared Section Properties						
Brand	<u>Product</u>	<u>Section</u>	Exterior	MOI_Wood			
	Window Mullion Charts	All	Clad	4.1239			
	Door Mullion Charts	All	Clad	6.4792			
Architect Series	Clad Frame (Direct Set)	IIA	Clad	4.3409			
Architect Series	Casement	All	Wood	5.8819			
Architect Series	In-swing Patio Door	All	Wood	10.5581			
Architect Series	Out-swing Patio Door	All	Wood	8.7762			
Architect Series	Casement	All	Clad	4.1239			
Architect Series	In-swing Patio Door	All	Clad	6.4792			
Architect Series	Out-swing Patio Door	All	Clad	9.2629			
Architect Series	Sliding Patio Door	Jamb	Clad	6.5248			
Architect Series	Sliding Patio Door	Head	Clad	6.5248			
Designer Series	Casement	Ali	Clad	4.1239			
Designer Series	In-swing Patio Door	All	Clad	6.4792			
Designer Series	Out-swing Patio Door	All	Clad	9.2629			
Designer Series	Sliding Patio Door	Jamb	Clad	6.5248			
Designer Series	Sliding Patio Door	Head	Clad	6.5248			
Proline	Casement	All	Clad	4.1239			
Proline	In-swing Patio Door	All	Clad	8.0055			
Proline	Sliding Patio Door	Jamb	Clad	7.5874			
Proline	Sliding Patio Door	Head	Clad	6.6263			
Pella	In-swing Entry Door	All	Clad	6.7557			
Pella	Out-swing Entry Door	All	Clad	9.2338			
Pella	In-swing Entry Door	All	Wood	6.7557			

MOE Wood (x10^6 psi)

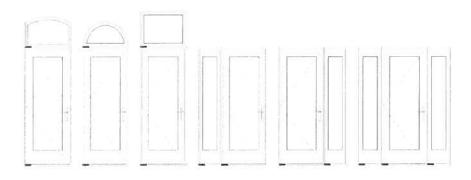
Discussion:

- 1). Frame Section Properties above provided by Pella Corporation.
- 2). Ixx is moment of Inertia about axis of bending of the mulled configuration, Ymax is the maximum distance from "X" axis to outer fibers of the section in the "y" direction. Zx is section modulus = Ixx / y max. A is area.
- 3). The configuration with the lowest 1 and Z will envelope all other configurations. A is relatively the same for all, except the DS radius, therefore will not be deciding factor.
- 4). Based on the above comparison of sections the casement jamb is the worst case.
- 5). Comparison above based on section properties of the combined jamb sections only.



P O Box 520775 Longwood, Florida 32752 Client: Pella Project #: 415-0309 Report #: 0506-6 Prepared by: RJA Date: 5/26/2015 Page 13 of 13

THE FOLLOWING CONFIGURATIONS ARE EXAMPLES IN THIS REPORT 2-Way





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Product Approval Menu > Product or Application Search > Application List > Application Detail

FL# FL11651-R3 Application Type Revision 2017 Code Version Application Status Approved

Comments Archived

Product Manufacturer Gulf Coast Supply & Manufacturing, LLC.

Address/Phone/Email 4020 S. W. 449th Street

Horseshoe Beach, FL 32648 (352) 498-0778 info@gulfcoastsupply.com

Eng David Authorized Signature

info@gulfcoastsupply.com

Technical Representative David Eng 14429 SW 2nd Pl Address/Phone/Email

Ste G30

Newberry, FL 32669 (352) 498-0778 info@gulfcoastsupply.com

David Eng Quality Assurance Representative

14429 SW 2nd Pl Address/Phone/Email

Ste G30

Newberry, FL 32669 (352) 498-0778

david.eng@gulfcoastsupply.com

Category Roofing Subcategory Metal Roofing

Evaluation Report from a Florida Registered Architect or a Licensed Florida Compliance Method

Professional Engineer

Evaluation Report - Hardcopy Received

Florida Engineer or Architect Name who developed the Daniel S. Kuhn

Evaluation Report

PE-75519 Florida License

Quality Assurance Entity Keystone Certifications, Inc.

Quality Assurance Contract Expiration Date 04/12/2024 Locke Bowden P.E. Validated By

✓ Validation Checklist - Hardcopy Received

FL11651 R3 COI Cert of Independence 2017-10-09.pdf Certificate of Independence

Referenced Standard and Year (of Standard) Year Standard 1992 FM 4471 TAS 100 1995 TAS 110 2000 2003 TAS 125 2012 UL 1897 2006 **UL 580**

Equivalence of Product Standards Certified By

Sections from the Code

Product Approval Method

Method 1 Option D

10/10/2017 Date Submitted 10/12/2017 Date Validated 10/17/2017 Date Pending FBC Approval 12/12/2017 Date Approved

Summary of Products

Go to Page



do to rage				
L # Model, Number or Name		Description		
1651.1 0.032" Aluminum 5V Crimp		Min 0.032" 5 V Crimp Aluminum Roof Panel over 15/32" Plywood		
Approved for u Impact Resista Design Pressu	re: +N/A/-108.5PSF osf @ 12" o.c. Install per manufacturers deta	Installation Instructions FL11651 R3 II FL11651 1 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 1 R3 0325vCrimp 15-32Plywood NonHVHZ-signed.pdf Created by Independent Third Party: Yes		
11651.2	0.032" Aluminum Gulf Lok	Min 0.032" Aluminum Gulf Lok 16" Wide Roof Panel over		

15/32" Plywood **Installation Instructions** Limits of Use FL11651 R3 II FL11651 2 R3 Metal Roof Panel Detail-

signed.pdf

Evaluation Reports

Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes

Approved for use in HVHZ: No Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: +N/A/-116.0PSF

Other: -52.5 psf @ 5 3/16" o.c. fastener spacing. -116.0 psf @ 5 3/16" o.c. fastener spacing. Install per manufacturers details. Not for use in HVHZ Zones.

FL11651 R3 AE FL11651 2 R3 032GulfLok 16 15-

32Plywood NonHVHZ-signed.pdf Created by Independent Third Party: Yes

O O Page 1 / 2 O O

1" Gulf Snap 24 Ga. 17" wide Roof Panel over 15/32" Plywood 1" 24ga. Gulf Snap 11651.3 Installation Instructions Limits of Use

Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A

Design Pressure: +N/A/--131.0PS Other: -67.3 psf @ 24" o.c. clip spacing. -131 psf @ 6" o.c.

clip spacing. Install per manufacturers details. For use in HVHZ Zones.

FL11651 R3 II FL11651 3 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519

Created by Independent Third Party: Yes **Evaluation Reports** FL11651 R3 AE FL11651 3 R3 1 24GulfSnap 17 15-32Plywood HVHZ-signed.pdf

Created by Independent Third Party: Yes

1" VersaLoc 24ga 16.5" Wide Roof Panel over 15/32" Plywood 1" 24ga. VersaLoc 11651.4

Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A

Design Pressure: +N/A/-138.5PSF Other: -71 psf @ 24" o.c. clip spacing. -138.5 psf @ 6" o.c. clip spacing. Install per manufacturers details. For use in HVHZ Zones.

Installation Instructions FL11651 R3 II FL11651 4 R3 Metal Roof Panel Detail-

signed.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes **Evaluation Reports**

FL11651 R3 AE FL11651 4 R3 24VersaLoc SR 16.5 15-32Plywood HVHZ-signed.pdf Created by Independent Third Party: Yes

1" VersaLoc min .032" Aluminum over 15/32" Plywood 1" VersaLoc .032" Aluminum 11651.5

Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A

Design Pressure: +N/A/-101.0PSF Other: -63.3 psf @ 24" o.c. clip spacing. -101.0 psf @ 6" o.c. **Installation Instructions** FL11651 R3 II FL11651 5 R3 Metal Roof Panel Detailsigned.pdf

Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes

Evaluation Reports

	manufacturers details. For use in HVHZ	32Plywood HVHZ-signed.pdf		
		Created by Independent Third Party: Yes		
11651.6	1.5" 24ga. Gulf Snap	1.5" Gulf Snap 24ga. 16" Wide Roof Panel over 15/32" Plywoo		
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: +N/A/-116.0PSF Other: -86.0 psf @ 24" o.c. clip spacing108.5 psf @ 12" o.c. clip spacing116.0 psf @ 6" o.c. clip spacing. Install per manufacturers detals. For use in HVHZ Zones.		Installation Instructions FL11651 R3 II FL11651 6 R3 Metal Roof Panel Detaisigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 6 R3 1.5 24GulfSnap 16 15 32Plywood HVHZ-signed.pdf Created by Independent Third Party: Yes		
11651.7 1.5" 24ga. Versal oc		1.5" VersaLoc min 24ga. 16" wide Roof Panel over 15/32" Plywood		
	tside HVHZ: Yes /A	Installation Instructions FL11651 R3 II FL11651 7 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 7 R3 1.5 24VersaLoc 16 15-32Plywood HVHZ-signed.pdf Created by Independent Third Party: Yes		
11651.8	2" 24ga. MegaLoc	Min 24 Ga. 2" MegaLoc 18" wide Roof Panel over 15/32" Plywood		
	tside HVHZ: Yes 'A	Installation Instructions FL11651 R3 II FL11651 8 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 8 R3 24VersaLoc 18 15-32Plywood NonHVHZ-signed.pdf Created by Independent Third Party: Yes		
11651.9	24 Ga. Gulf Lok	Min 24 Ga. Gulf Lok 16" Wide over 15/32" Plywood		
@ 5-1/8" o.c. fastener s	tside HVHZ: Yes /A /A/-96.7PSF .0-1/4" o.c. fastener spacing96.76 psf spacing121.75 psf @ 5-1/8" fastener 1/8" fastener spacing. Install per	Installation Instructions FL11651 R3 II FL11651 9 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 9 R3 24GulfLok 16 15-32Plywood NonHVHZ-signed.pdf		
	Not for use in HVHZ Zones.	Created by Independent Third Party: Yes		
manufacturers details. I	Not for use in HVHZ Zones. 24 Ga. Gulf Seam	Min 24 Ga. Gulf Seam 16" Wide Roof Panel over 1x4 Wood Purlins over 15/32" Plywood		
manufacturers details. In the second	24 Ga. Gulf Seam HVHZ: No tside HVHZ: Yes /A I/A/-93.5PSF " o.c. Install per manufacturers details.	Min 24 Ga. Gulf Seam 16" Wide Roof Panel over 1x4 Wood Purlins over 15/32" Plywood Installation Instructions FL11651 R3 II FL11651 10 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports		
manufacturers details. In the second	24 Ga. Gulf Seam HVHZ: No tside HVHZ: Yes /A I/A/-93.5PSF " o.c. Install per manufacturers details.	Min 24 Ga. Gulf Seam 16" Wide Roof Panel over 1x4 Wood Purlins over 15/32" Plywood Installation Instructions FL11651 R3 II FL11651 10 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 10 R3 24GulfSeam 16 1x4 15-32Plywood NonHVHZ-signed.pdf		
manufacturers details. I 11651.10 Limits of Use Approved for use in I Approved for use out Impact Resistant: N/ Design Pressure: +N, Other: -93.5 psf @ 18 Not for use in HVHZ Zot 11651.11 Limits of Use Approved for use in I Approved for use out Impact Resistant: N/ Design Pressure: +N, Other: -69.25 psf @ 2	HVHZ: No tside HVHZ: Yes /A I/A/-93.5PSF " o.c. Install per manufacturers details. nes. 24 Ga. Gulf Seam HVHZ: No tside HVHZ: Yes /A	Min 24 Ga. Gulf Seam 16" Wide Roof Panel over 1x4 Wood Purlins over 15/32" Plywood Installation Instructions FL11651 R3 II FL11651 10 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 10 R3 24GulfSeam 16 1x4 15-32Plywood NonHVHZ-signed.pdf Created by Independent Third Party: Yes Min 24 Ga. Gulf Seam, 1 3/4" Snap Lock, 18" Wide Roof Panel		
manufacturers details. I 11651.10 Limits of Use Approved for use in I Approved for use out Impact Resistant: N/ Design Pressure: +N, Other: -93.5 psf @ 18 Not for use in HVHZ Zot 11651.11 Limits of Use Approved for use in I Approved for use out Impact Resistant: N/ Design Pressure: +N, Other: -69.25 psf @ 2 o.c. clip spacing. Install	HVHZ: No tside HVHZ: Yes /A //A/-93.5PSF " o.c. Install per manufacturers details. nes. 24 Ga. Gulf Seam HVHZ: No tside HVHZ: Yes /A //A/-108.5PSF -4" o.c. clip spacing108.5 psf @ 12"	Min 24 Ga. Gulf Seam 16" Wide Roof Panel over 1x4 Wood Purlins over 15/32" Plywood Installation Instructions FL11651 R3 II FL11651 10 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 10 R3 24GulfSeam 16 1x4 15-32Plywood NonHVHZ-signed.pdf Created by Independent Third Party: Yes Min 24 Ga. Gulf Seam, 1 3/4" Snap Lock, 18" Wide Roof Panel over 15/32" Plywood Installation Instructions FL11651 R3 II FL11651 11 R3 Metal Roof Panel Detailsigned.pdf Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Evaluation Reports FL11651 R3 AE FL11651 11 R3 24GulfSeam 18 15-32Plywood NonHVHZ-signed.pdf		

Florida Building Code Online

Approved for use outside HVHZ: Yes signed.pdf Impact Resistant: N/A Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Design Pressure: +N/A/-156.5PSF Other: -108.5 psf @ 12" o.c. fastener spacing. -156.5 psf @ **Evaluation Reports** 6" o.c. fastener spacing. Install per manufacturers details. For FL11651 R3 AE FL11651 12 R3 265vCrimp 15-32Plywood HVHZ-signed.pdf use in HVHZ Zones. Created by Independent Third Party: Yes 11651.13 26 Ga. 5V Crimp Min 26 Ga. 5V Crimp over 15/32" Plywood Limits of Use **Installation Instructions** Approved for use in HVHZ: No FL11651 R3 II FL11651 13 R3 Metal Roof Panel Detail-Approved for use outside HVHZ: Yes signed.pdf Impact Resistant: N/A Verified By: Dan Kuhn, P.E. 75519 Design Pressure: +N/A/-131.PSF Created by Independent Third Party: Yes Other: -94.25 psf @ 16" o.c. fastener pattern. -131.0 psf @ **Evaluation Reports** FL11651 R3 AE FL11651 13 R3 265vCrimp 15-16" o.c. fastener pattern. Install per manufacturers details. Not for use in HVHZ Zones. 32Plywood NonHVHZ-signed.pdf Created by Independent Third Party: Yes 11651.14 26 Ga. 5V Crimp Min 26 Ga. 5V Crimp Roof Panel over 1x4 Wood Purlins over 15/32" Plywood Limits of Use **Installation Instructions** Approved for use in HVHZ: No FL11651 R3 II FL11651 14 R3 Metal Roof Panel Detail-Approved for use outside HVHZ: Yes signed.pdf Impact Resistant: N/A Verified By: Dan Kuhn, P.E. 75519 Design Pressure: +N/A/-156.5PSF Created by Independent Third Party: Yes Other: -108.5 psf @ 12" o.c. fastener spacing. -156.5 psf @ **Evaluation Reports** 6" o.c. fastener spacing. Install per manufacturers details. For FL11651 R3 AE FL11651 14 R3 265vCrimp 1x4 15-32Plywood NonHVHZ-signed.pdf use in HVHZ Zones. Created by Independent Third Party: Yes 11651.15 26 Ga. Gulf Lok Min 26 Ga. Gulf Lok 16" Wide Roof Panel over 15/32" Plywood Limits of Use **Installation Instructions** Approved for use in HVHZ: Yes FL11651 R3 II FL11651 15 R3 Metal Roof Panel Detail-Approved for use outside HVHZ: Yes <u>signed,pdf</u> Impact Resistant: N/A Verified By: Dan Kuhn, P.E. 75519 Design Pressure: +N/A/-161.0PSF Created by Independent Third Party: Yes **Evaluation Reports** Other: -63.5 psf @ 5 3/16" o.c. fastener spacing. -121.75 psf @ 5 3/16" spacing. -161.0 psf @ 5 3/16" o.c. fastener spacing. FL11651 R3 AE FL11651 15 R3 26GulfLok 16 15-Install per manufacturers details. For use in HVHZ Zones. 32Plywood HVHZ-signed.pdf Created by Independent Third Party: Yes 11651.16 26 Ga. Gulf Lok Min 26 Ga. Gulf Lok 16" Wide Roof Panel over 15/32" Plywood Limits of Use **Installation Instructions** FL11651 R3 II FL11651 16 R3 Metal Roof Panel Detail-Approved for use in HVHZ: No Approved for use outside HVHZ: Yes signed.pdf Verified By: Dan Kuhn, P.E. 75519 Impact Resistant: N/A Design Pressure: +N/A/-63.5PSF Other: -63.5 psf @ 5 3/16" o.c. fastener spacing. -121.75 psf Created by Independent Third Party: Yes **Evaluation Reports** FL11651 R3 AE FL11651 16 R3 26GulfLok 16 15-@ 5 3/16" fastener spacing. -161 psf @ 5 3/16" fastener spacing. Install per manufacturers details. Not for use in HVHZ 32Plywood NonHVHZ-signed.pdf Created by Independent Third Party: Yes Zones. 11651.17 26 Ga. GulfPBR Min 26 Ga. Gulf PBR Roof Panel over 15/32" Plywood **Installation Instructions** Limits of Use Approved for use in HVHZ: No FL11651 R3 II FL11651 17 R3 Metal Roof Panel Detail-Approved for use outside HVHZ: Yes signed.pdf Verified By: Dan Kuhn, P.E. 75519 Impact Resistant: N/A Created by Independent Third Party: Yes Design Pressure: +N/A/-154.7PSF Other: -59.25 psf @ 24" o.c. fastener pattern. -154.75 psf @ **Evaluation Reports** FL11651 R3 AE FL11651 17 R3 26GulfPBR 15-12" o.c. fastener pattern. Install per manufacturer details. Not 32Plywood NonHVHZ-signed.pdf for use in HVHZ Created by Independent Third Party: Yes Min 26 Ga. GulfPBR Roof Panel over 1x4 Wood Purlins over 11651.18 26 Ga. GulfPBR 15/32" Plywood Installation Instructions Limits of Use FL11651 R3 II FL11651 18 R3 Metal Roof Panel Detail-Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes signed.pdf Impact Resistant: N/A Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes Design Pressure: +N/A/-151.7PSF **Evaluation Reports** Other: -100.5 psf @ 24" o.c. fastener spacing. -151.75 psf @ 12" fastener spacing. Install per manufacturer's details. For use FL11651 R3 AE FL11651 18 R3 26GulfPBR 1x4 15-32Plywood HVHZ-signed.pdf in HVHZ Zones Created by Independent Third Party: Yes Min 26 Ga. GulfPBR over 15/32" Plywood 11651.19 26 Ga. GulfPBR

Florida Building Code Online

Limits of Use

Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes

Impact Resistant: N/A

Design Pressure: +N/A/-154.7PSF

Other: -60.5 psf @ 24" o.c. fastener spacing. -154.75 psf @ 12" o.c. fastener spacing. Install per manufacturer's details. For

use in HVHZ Zones.

Installation Instructions

FL11651 R3 II FL11651 19 R3 Metal Roof Panel Detail-

signed.pdf

Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes

Evaluation Reports

FL11651 R3 AE FL11651 19 R3 26GulfPBR 15-

32Plywood HVHZ-signed.pdf

Created by Independent Third Party: Yes

11651.20

26 Ga. GulfPBR

Min 26 Ga. GulfPBR over 1x4 Wood Purlins over 15/32"

Plywood

Limits of Use

Approved for use in HVHZ: No Approved for use outside HVHZ: Yes

Impact Resistant: N/A

Design Pressure: +N/A/-151.7PSF

Other: -94.25 psf @ 24" o.c. fastener spacing. -151.75 psf @ 12" fastener spacing. Install per manufacturer's details. Not for

use in HVHZ Zones

Installation Instructions

FL11651 R3 II FL11651 20 R3 Metal Roof Panel Detail-

<u>signed.pdf</u>

Verified By: Dan Kuhn, P.E. 75519 Created by Independent Third Party: Yes

Evaluation Reports

FL11651 R3 AE FL11651 20 R3 26GulfPBR 1x4 15-

O Page 1 / 2 O

32Plywood NonHVHZ-signed.pdf

Created by Independent Third Party: Yes

Go to Page





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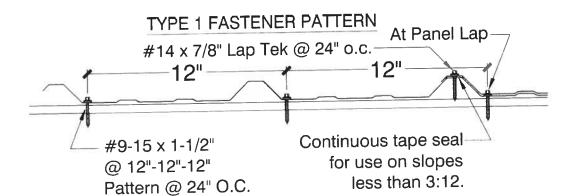
METAL ROOF PANEL DETAIL



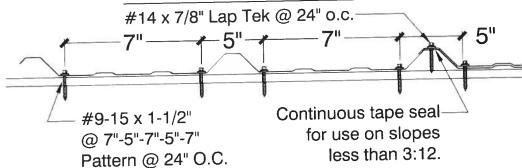


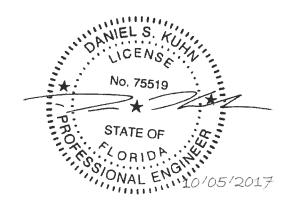
GULFPBR™

26 GUAGE GULFPBR™ PANEL OVER 15/32" PLYWOOD FLORIDA PRODUCT APPROVAL NO. 11651.19 R3



TYPE 2 FASTENER PATTERN





FL# 11651.19 R3 • OCTOBER 5, 2017

METAL ROOF PANEL DETAIL



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FL# FL10456-R4 **Application Type** Revision Code Version 2017

Approved

*Approved by DBPR. Approvals by DBPR shall be reviewed and ratified by the POC and/or the Commission if necessary.

Comments Archived

Product Manufacturer Address/Phone/Email

Application Status

Simpson Strong-Tie Co. 2221 Country Lane McKinney, TX 75070 (800) 999-5099 Ext 3027 kcullum@strongtie.com

Authorized Signature

Keith Cullum kcullum@strongtie.com

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Quality Assurance Representative

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Shelby Short

5956 W. Las Positas Boulevard

Pleasanton, CA 94588 (800) 999-5099 sshort@strongtie.com

Category Subcategory Structural Components Wood Connectors

Compliance Method

Evaluation Report from a Product Evaluation Entity

Evaluation Entity Quality Assurance Entity

Quality Assurance Contract Expiration Date

Validated By

ICC Evaluation Service, LLC Benchmark Holdings, L.L.C.

12/31/2019 Ted Berman, PE

Validation Checklist - Hardcopy Received

Certificate of Independence

FL10456 R4 COI ICC-ES Cert of Independence 2017-12-06.pdf

Referenced Standard and Year (of Standard)

Year **Standard** 2012 **AISI S100** 2015 ANSI/AWC NDS **ASTM D7147** 2005

Equivalence of Product Standards Certified By

Sections from the Code

Product Approval Method

Method 1 Option C

Date Submitted Date Validated

12/13/2017 12/17/2017

Date Pending FBC Approval

Date Approved

12/19/2017

FL#	Model, Number or Name	Description		
10456.1	DSP, SSP	Stud to Plate tie		
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other: In HVHZ, if using DSP with 10dx1.5" nails to sill plate, DSP with 10d common nails in SPF/HF lumber, or SSP to resist roof uplift provide supplemental connection to achieve minimum 700 lbs. uplift		Installation Instructions FL10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.2	H1, H10A, H10-2, H2.5A, H3, H4, H5, H6, H7Z	Hurricane Ties		
Approved for u Impact Resista Design Pressur Other: In HVHZ, roof uplift, 2 conr		Installation Instructions FL10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.3	HGT-2, HGT-3, HGT-4	Heavy Girder Tiedown		
		Installation Instructions Fi10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.4	HS24	Hurricane Tie		
Impact Resista Design Pressur Other: In HVHZ	se outside HVHZ: Yes nt: N/A	Installation Instructions F110456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.5	HTS16, HTS20, HTS24, HTS28, HTS30, HTS30C	Heavy Twist Strap		
		Installation Instructions F110456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.6	LFTA	Light Floor Tie Anchor		
	·	Installation Instructions FL10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.7	LTS12, LTS16, LTS18, LTS20	Light Twist Strap		
Limits of Use		Installation Instructions		

FL10456 R4 II ESR-2613.pdf

Approved for use in HVHZ: Yes

Florida Building Code Online

	r long Bulluli	ig Code Offline		
Impact Resistan Design Pressure Other: In HVHZ, i		Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.8	MTS12, MTS16, MTS18, MTS20, MTS30	Medium Twist Strap		
Limits of Use Approved for use in HVHZ: Yes Approved for use outside HVHZ: Yes Impact Resistant: N/A Design Pressure: N/A Other:		Installation Instructions FL10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.9	RSP4	Stud to Plate Connector		
Impact Resistan Design Pressure Other: In HVHZ, i	e outside HVHZ: Yes t: N/A	Installation Instructions FL10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.10	SP1, SP2, SP4, SP6, SP8	Stud Plate Tie		
Impact Resistant Design Pressure: Other: In HVHZ, II	e outside HVHZ: Yes t: N/A	Installation Instructions FL10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		
10456.11	SPH4, SPH6, SPH8	Heavy Stud Plate Tie		
Limits of Use Approved for use Approved for use Impact Resistant Design Pressure: Other:	e outside HVHZ: Yes :: N/A	Installation Instructions FL10456 R4 II ESR-2613.pdf Verified By: ICC Evaluation Service, LLC Created by Independent Third Party: Evaluation Reports FL10456 R4 AE ESR-2613.pdf		

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ESR-2613

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DIVISION: 06 00 00—WOOD, PLASTICS AND COMPOSITES SECTION: 06 05 23—WOOD, PLASTIC, AND COMPOSITE FASTENINGS

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC.

5956 WEST LAS POSITAS BOULEVARD PLEASANTON, CALIFORNIA 94588

EVALUATION SUBJECT:

SIMPSON STRONG-TIE® HURRICANE AND SEISMIC STRAPS AND TIES FOR WOOD FRAMING



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ICC-ES Evaluation Report

ESR-2613

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DIVISION: 06 00 00—WOOD, PLASTICS, AND COMPOSITES
Section: 06 05 23—Wood, Plastic, and Composite Fastenings

REPORT HOLDER:

SIMPSON STRONG-TIE COMPANY INC. 5956 WEST LAS POSITAS BOULEVARD PLEASANTON, CALIFORNIA 94588 (800) 925-5099 www.strongtie.com

EVALUATION SUBJECT:

SIMPSON STRONG-TIE® HURRICANE AND SEISMIC STRAPS AND TIES FOR WOOD FRAMING

1.0 EVALUATION SCOPE

Compliance with the following codes:

- 2015, 2012, 2009 and 2006 International Building Code® (IBC)
- 2015, 2012, 2009 and 2006 International Residential Code[®] (IRC)

Property evaluated:

Structural

2.0 USES

The Simpson Strong-Tie® hurricane and seismic straps and ties described in this report are used as wood framing connectors in accordance with Section 2304.10.3 of the 2015 IBC and Section 2304.9.3 of the 2012, 2009 and 2006 IBC. The products may also be used in structures regulated under the IRC when an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

3.0 DESCRIPTION

3.1 General:

The Simpson Strong-Tie hurricane and seismic straps and ties recognized in this report are installed to resist design forces on wood-frame construction resulting from the application of the most critical effects of the load combinations prescribed by code that include wind or seismic loads.

3.1.1 Hurricane Ties: Hurricane ties are used to anchor wood rafters or joists to wood wall plates or studs or to anchor wood studs to wood sill plates. The H6 and H7Z

ties are formed from No. 16 gage galvanized steel; the H1, H2.5A, H3, H5, H10A, and H10-2 ties are formed from No. 18 gage galvanized steel; and the H4 tie is formed from No. 20 gage galvanized steel. See <u>Table 1</u> for tie model numbers, tie dimensions, fastener schedules, and allowable loads. See <u>Figures 1a</u> and <u>1b</u> for illustrations of the hurricane ties recognized in this report, and <u>Figure 1c</u> for illustrations of installation configurations with designated allowable load directions.

- **3.1.2 HS24 Hurricane Tie:** The HS24 hurricane tie anchors wood rafters or trusses to wood wall top plates. The HS24 connector is formed from No. 18 gage galvanized steel. See <u>Table 2</u> for required fasteners and allowable loads. See <u>Figure 2</u> for a drawing of the HS24 tie and a typical installation detail.
- 3.1.3 LTS, MTS, and HTS Series Twist Straps: The LTS, MTS, and HTS series twist straps are used to anchor wood trusses or rafters to wood wall double top plates, wood studs, wood beams, or wood rim boards. The LTS, MTS, and HTS series twist straps are formed from No. 18, No. 16, and No. 14 gage galvanized steel, respectively. See Table 3 for strap model numbers, overall strap lengths, required fasteners, and allowable uplift loads when installed with different fastener schedules. See Figure 3 for a drawing of an LTS12 twist strap and two typical MTS strap installations.
- **3.1.4 LFTA Light Floor Tie Anchor:** The LFTA light floor tie anchor is used as a floor-to-floor tension tie and is formed from No. 16 gage galvanized steel. See <u>Table 4</u> for anchor tie dimensions, required fasteners, and the assigned allowable uplift load. See <u>Figure 4</u> for a drawing of the LFTA connector.
- 3.1.5 SP and SPH Series Stud Plate Connectors: The SP1 connector fastens one edge of a wood stud to the contiguous edge of a wood sill plate, and the SP2 connector fastens to one side of a wood double top plate and to the contiguous edge of a wood stud. The SP4, SP6, SP8, SPH4, SPH6, and SPH8 are 11/4-inch-wide (32 mm) U-shaped straps with a horizontal portion that bears against the wood wall top plates or sill plates and two vertical legs that are nailed to the edges of a wood stud. The SP and SPH connectors are fabricated from No. 20 and No. 18 gage galvanized steel, respectively. See Table 5 for SP and SPH models, connector dimensions, required fasteners, and allowable uplift loads. See Figure 5 for drawings of the SP1 and SP2 connector, and of typical stud-to-plate connection details for the SP1, SP2, SP4, and SPH4 connectors.

- 3.1.6 RSP4 Reversible Stud Plate Ties: The RSP4 tie plates are used to connect a nominally 2-inch-wide wood stud to either a top or sill plate of a wood framed wall. The RSP4 tie connector is fabricated from No. 20 gage galvanized steel. See <u>Table 6</u> for required fasteners and allowable loads. See <u>Figure 6a</u> for a drawing of the RSP4 connector showing overall dimensions; <u>Figure 6b</u> for a drawing of a typical RSP4 installation connecting a wood double top plate to a wood stud; and <u>Figure 6c</u> for a typical RSP4 installation connecting a wood sill plate.
- 3.1.7 SSP and DSP Stud Plate Connectors: The SSP stud-to-plate connector is used to provide a positive connection between a single wood stud and the top or sill plate of the same wood wall, and the DSP stud-to-plate connector is used to provide a positive connection between a double wood stud and the wood wall top or sill plate of the same wood wall. The SSP and DSP connectors are fabricated from No. 18 gage galvanized steel. See Table 7 for required fasteners and allowable uplift loads. See Figure 7 for drawings of the SSP and DSP connectors showing overall dimensions; a drawing of an SSP installation connecting a stud to a sill plate; and a drawing of a DSP installation connecting a double wood stud assembly to a top plate.
- 3.1.8 HGT Heavy Girder Tiedown Brackets: The HGT heavy girder tiedown brackets are used to provide a positive connection between wood roof beams or multi-ply wood roof trusses and wood posts vertically aligned to support the end reaction of the beam or truss member. The HGT tiedown connector is a U-shaped bracket that is installed over the top chord of the roof truss having a slope from 3:12 (14 degrees) to 8:12 (34 degrees). Other components required for the connection, such as the anchor rods and hold-down or tie-down devices, that must be used to form a complete load path to resist design uplift forces from their point of origin to the load-resisting elements, that is, the vertically aligned supporting wood post, must be designed and specified by the registered design professional. The HGT tiedown brackets are fabricated from No. 7 gage steel, and are supplied with insert plates and crescent washers. See Table 8 for tiedown connector models, connector dimensions, fastener schedules, and allowable uplift loads. See Figure 8 for a drawing of the HGT-2 tiedown connector.

3.2 Materials:

3.2.1 Steel: Unless otherwise noted, the connectors described in this report are fabricated from ASTM A653, SS designation, Grade 33, galvanized steel with a minimum yield strength, Fy, of 33,000 psi (227 MPa) and a minimum tensile strength, Fu, of 45,000 psi (310 MPa). The HTS twist straps, the SSP and DSP stud-to-plate ties, and the H2.5A hurricane tie are fabricated from ASTM A653, SS designation, Grade 40, steel with a minimum yield strength of 40,000 psi (275 MPa) and a minimum tensile strength of 55,000 psi (379 MPa).

The body of the HGT heavy girder tiedown bracket is fabricated from ASTM A1011, SS designation, Grade 33, hot rolled steel with a minimum yield strength of 33,000 psi (227 MPa) and a minimum tensile strength of 52,000 psi (358 MPa), and the crescent washers of the HGT bracket are fabricated from ASTM A36 steel with a minimum yield strength of 36,000 psi (248 MPa) and a minimum tensile strength of 58,000 psi (399 MPa).

Base-metal thicknesses for the connectors in this report are as follows:

NOMINAL THICKNESS (gage)	MINIMUM BASE-METAL THICKNESS (inch)
No. 3	0.2285
No. 7	0.1705
No. 10	0.1275
No. 14	0.0685
No. 16	0.0555
No. 18	0.0445
No. 20	0.0335

For SI: 1 inch = 25.4 mm.

The galvanized connectors have a minimum G90 zinc coating specification in accordance with ASTM A653. Some models (designated with a model number ending with Z) are available with a G185 zinc coating specification in accordance with ASTM A653. Some models (designated with a model number ending with HDG) are available with a hot-dip galvanization, also known as "batch" galvanization, in accordance with ASTM A123, with a minimum specified coating weight of 2.0 ounces of zinc per square foot of surface area (600 g/m2), total for both sides. Model numbers in this report do not include the Z or HDG ending, but the information shown applies.

The HGT Heavy Girder Tiedown Brackets have a painted finish and may also be available with the HDG finish.

The lumber treater or holder of this report (Simpson Strong-Tie Company) should be contacted for recommendations on minimum corrosion resistance of steel connectors in contact with the specific proprietary preservative treated or fire retardant treated lumber.

3.2.2 Wood: Supporting wood members to which these connectors are fastened must be solid sawn lumber, gluedlaminated lumber, or engineered lumber [such as Laminated Veneer Lumber (LVL), Parallel Strand Lumber (PSL), and Laminated Strand Lumber (LSL)] having dimensions consistent with the connector dimensions shown in this report. Unless otherwise noted, supporting wood members and supported members must have an assigned minimum specific gravity of 0.50 (minimum equivalent specific gravity of 0.50 for engineered lumber), except as noted in Table 5 for the SPH stud plate tie connectors, which provides values for which lumber members having assigned minimum specific gravities of 0.50 and 0.55 are required; and Table 7 for the SSP and DSP stud-to-plate tie connectors, which permits lumber having assigned minimum specific gravities of 0.50 and 0.43. The lumber used with the connectors described in this report must have a maximum moisture content of 19 percent (16 percent for engineered lumber) except as noted in Section 4.1.

The thickness of the wood members must be equal to or greater than the length of the fasteners specified in the tables in this report, except if noted otherwise in the tables and accompanying footnotes in this report, or as required by wood member design, whichever controls.

3.2.3 Fasteners: Bolts, at a minimum, must comply with ASTM A36 or A307. Nails used for connectors, straps, and ties described in this report must comply with ASTM F1667 and have the following minimum dimensions and bending yield strengths (Fyb):

FASTENERS	NAIL DIAMETER (inch)	NAIL LENGTH (inches)	F _{yb} (psi)
8d × 1 ¹ / ₂	0.131	11/2	100,000
8d	0.131	21/2	100,000
10d × 1 ¹ / ₂	0.148	11/2	90,000
10d	0.148	3	90,000

For SI: 1 inch = 25.4 mm.

Fasteners used in contact with preservative treated or fire retardant treated lumber must comply with Section 2304.10.5 of the 2015 IBC, Section 2304.9.5 of the 2012, 2009 and 2006 IBC, Section R317.3 of the 2015, 2012 and 2009 IRC or Section R319.3 of the 2006 IRC, as applicable. The lumber treater or this report holder (Simpson Strong-Tie Company) should be contacted for recommendations on minimum corrosion resistance of fasteners and connection capacities of fasteners used with the specific proprietary preservative treated or fire retardant treated lumber.

4.0 DESIGN AND INSTALLATION

4.1 Design:

The tabulated allowable loads shown in this report are based on allowable stress design (ASD) and include the load duration factor, C_D , corresponding with the applicable loads in accordance with the National Design Specification for Wood Construction (NDS).

Tabulated allowable loads apply to products connected to wood used under dry conditions and where sustained temperatures are 100°F (37.8°C) or less. When products are installed to wood having a moisture content greater than 19 percent (16 percent for engineered wood products), or when wet service is expected, the allowable loads must be adjusted by the wet service factor, CM, specified in the NDS. When connectors are installed in wood that will experience sustained exposure to temperatures exceeding 100°F (37.8°C), the allowable loads in this report must be adjusted by the temperature factor, Ct, specified in the NDS.

Connected wood members must be analyzed for loadcarrying capacity at the connection in accordance with the NDS.

4.2 Installation:

Installation of the connectors must be in accordance with this evaluation report and the manufacturer's published installation instructions. In the event of a conflict between this report and the manufacturer's published installation instructions, this report governs.

4.3 Special Inspection:

4.3.1 Main Wind-force-resisting Systems under the IBC: Periodic special inspection must be conducted for components within the main wind-force-resisting system, where required in accordance with Sections <u>1704.2</u> and <u>1705.11</u> of the 2015 IBC, Sections <u>1704.2</u> and <u>1705.10</u> of the 2012 IBC, Sections <u>1704</u> and <u>1706</u> of the 2009 IBC, and Section <u>1704</u> of the 2006 IBC.

- **4.3.2** Seismic-force-resisting Systems under the IBC: Periodic special inspection must be conducted for components within the seismic-force-resisting system, in accordance with Sections 1704.2 and 1704.12 of the 2015 IBC, Sections 1704.2 and 1705.11 of the 2012 IBC, and Sections 1704 and 1707 of the 2009 and 2006 IBC.
- **4.3.3 Installations under the IRC:** Special inspections are normally not required for connectors used in structures regulated under the IRC. However, for components and systems requiring an engineered design in accordance with IRC Section R301, periodic special inspection must be in accordance with Sections 4.3.1 and 4.3.2 of this report.

5.0 CONDITIONS OF USE

The Simpson Strong-Tie Hurricane and Seismic Straps and Ties described in this report comply with, or are suitable alternatives to what is specified in, those codes listed in Section 1.0 of this report, subject to the following conditions:

- 5.1 The connectors must be manufactured, identified and installed in accordance with this report and the manufacturer's published installation instructions. A copy of the instructions must be available at the jobsite at all times during installation.
- 5.2 Calculations showing compliance with this report must be submitted to the code official. The calculations must be prepared by a registered design professional where required by the statues of the jurisdiction in which the project is to be constructed.
- 5.3 Adjustment factors noted in Section 4.1 and the applicable codes must be considered, where applicable.
- 5.4 Connected wood members and fasteners must comply, respectively, with Sections 3.2.2 and 3.2.3 of this report.
- 5.5 Use of connectors with preservative or fire retardant treated lumber must be in accordance with Section 3.2.1 of this report. Use of fasteners with preservative or fire retardant treated lumber must be in accordance with Section 3.2.3 of this report.

6.0 EVIDENCE SUBMITTED

Data in accordance with the ICC-ES Acceptance Criteria for Joist Hangers and Similar Devices (AC13), dated February 2017.

7.0 IDENTIFICATION

The products described in this report are identified with a die-stamped label or adhesive label indicating the name of the manufacturer (Simpson Strong-Tie), the model number, and the number of an index evaluation report (ESR-2523) that is used as an identifier for the products recognized in this report.

TABLE 1—HURRICANE TIES

MODEL NO.	FASTENERS (Quantity-Type)			ALLOWABLE LOADS ^{1,2} (lbs)				
	To Rafter	To Plates	To Stud	Connection	Uplift ^{4,5} C _D =1.6	Lateral ^{6,7} C _p =1.6		
				Configurations ³		F ₁	F ₂	
H1	6-8d x 1 ¹ / ₂	4–8d	_		545	510	190	
H10A	9-10d x 1 ¹ / ₂	9-10d x 1 ¹ / ₂		1	1,040	565	285	
H10-2	610d	6–10d	_		655	430	355	
H2.5A	5-8d	5-8d		9	565	110	110	
H3	4–8d	4–8d		2	435	210	170	
H5	4-8d	4-8d			440	100	225	
H4	_	48d	4–8d	3	330	_	_	
	48d	48d	_	4	330	140	175	
H6	N - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	8-8d	8-8d	5	1,230		20 MIE :	
H7Z	4-8d	2-8d	88d	6	830	410		

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

Design Uplift / Allowable Uplift +

Design Lateral Parallel to Plate / Allowable Lateral Parallel to Plate +

Design Lateral Perpendicular to Plate / Allowable Lateral Perpendicular to Plate ≤ 1.0.

The three terms in the unity equation consider all possible forces that the hurricane tie may be designed and installed to resist. The number of terms that must be considered for simultaneous loading is determined by the registered design professional and is dependent on the method of calculating wind forces and the assumed load path that the connector is designed to resist.

of calculating wind forces and the assumed load path that the connector is designed to resist.

3"Connection Configurations" shown in Figure 1c (next page) indicate the load directions F₁ and F₂, and are details showing connector installations on the outside of the wall for clarity. Installation on the inside of the wall is acceptable to achieve the tabulated allowable loads.

*Connections in the same area (i.e. truss to plate connector and plate to stud connector) must be on installed on the same side of the wall to achieve the tabulated allowable uplift loads and ensure a continuous load path.

⁵Allowable uplift loads have been increased for wind or earthquake loading, and no further increase is allowed. Allowable loads must be reduced when other load durations govern.

⁶Allowable lateral loads in the F₁ direction must not be used to replace diaphragm boundary members or nailing or replace solid blocking required by code to laterally support the ends of joists/rafters.

Additional shear transfer elements must be considered the connector installation induces cross grain bending or tension of the truss or rafter members.

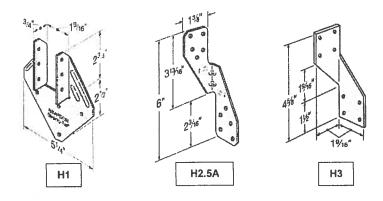


FIGURE 1a-H1, H2.5A, AND H3 HURRICANE TIES

¹Allowable loads are for one anchor installed to a minimum nominal 2x supported and minimum nominal 2x supporting wood member. A rafter minimum actual thickness of 2¹/₂ inches must be used when framing anchors are installed on each side of the rafter and on the same side of the plate.

²Allowable simultaneous loads in more than one direction on a single connector must be evaluated as follows:

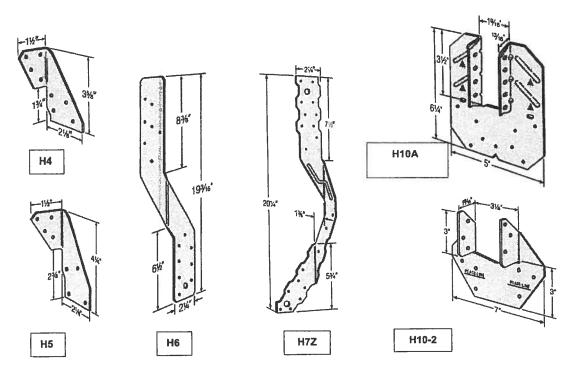


FIGURE 1b—H4, H5, H6, H7Z, H10A AND H10-2 HURRICANE TIES

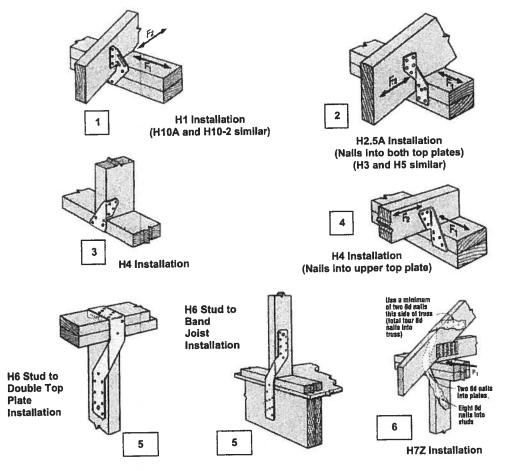


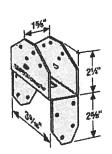
FIGURE 1c—CONNECTION CONFIGURATIONS OF HURRICANE TIE INSTALLATIONS SPECIFIED IN TABLE 1

TABLE 2—HS24 HURRICANE TIE

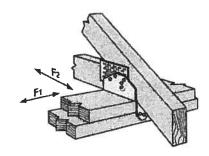
	FASTENERS1 (C	luantity-Type)	ALLOWABLE LOADS (lbs) Cp=1.6			
MODEL NO.	To Rafter or Truss	To Double Top Plate	Uplift ²	Lateral ^{3,4,5}		
				F ₁	F ₂	
	8-8d x 11/2 & 2-8d (slant)	8–8d	605	645	1,100	
HS24	8-8d x 1 ¹ / ₂	8–8d	625	625	680	

For SI: 1 inch = 25.4 mm, 1 lbs = 4.5 N.

member. 5F_1 load direction is parallel to plate, and F_2 load direction is perpendicular to plate.



HS24 Dimensions U.S. Patents 5,603,580



HS24 Installation and Allowable Load Directions

FIGURE 2—HS24 HURRICANE TIE

¹"Slant" nailing refers to 8d common nails installed as toenails on each side of the connector. The nails must be driven through the connector at an angle approximately 30° with the rafter/truss member with the nail penetrating through the rafter/truss member into the wood double top

plate.

The uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when

other load durations govern.

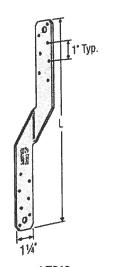
³Allowable lateral loads in the F₁ direction must not be used to replace diaphragm boundary members or nailing or replace solid blocking required by code to laterally support the ends of joists/rafters.

Additional shear transfer elements must be considered the connector installation induces cross grain bending or tension of the truss or rafter

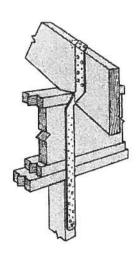
TABLE 3-LTS, MTS, AND HTS TWIST STRAPS

			TOTAL QUANTITY	OF FASTENERS1	ALLOWABLE UP	LIFT LOADS ^{2,3} (lbs)	
TWIST STRAP SERIES MODEL NO.	MODEL NO.	NO LENGIH	When Installed with 10d Common	When Installed with 10dx1 ¹ / ₂ Common Nails	When Installed with 10d Common Nails	When installed with 10dx11/2 Common Nails	
SERIES		(in)	Nails		C ₀ = 1.6	C ₀ = 1.6	
	LTS12	12					
LTS⁴	LTS16	16	12	12	660	600	
LIS	LTS18	18] '2	12	000	000	
	LTS20	20					
	MTS12	12					
	MTS16	16		14	990	990	
MTS4	MTS18	18	14				
	MTS20	20					
	MTS30	30					
	HTS16	16	16	16	1,310	1,310	
	HTS20	20					
UTO ⁵	HTS24	24]				
HTS⁵	HTS28	28	20	24	1,310	1,310	
	HTS30	30					
	HTS30C	30	7				

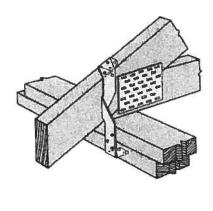
For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N



LTS12 (MTS and HTS Similar)



Typical MTS Installation -Rafter to Stud



Typical MTS Installation -Truss to Double Top Plate

FIGURE 3-TWIST STRAPS

¹Half of the fasteners must be installed on each end of the strap to achieve the allowable uplift load.
²Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code.
³Tabulated allowable uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be

reduced when other load durations govern.

Each model of the LTS and MTS twist strap series (except for the MTS30) has more nail holes than the minimum quantity of nails specified in the table.
⁵HTS30C has the twist in the center of the strap length.

TABLE 4-LFTA LIGHT FLOOR TIE ANCHOR

LETA ANCHOR DIMENSIONS (in)		SIONS (in)	FASTENERS ²	ALLOWABLE TENSION LOAD34 (Ibs	
MODEL NO.	Strap Width (W)	Clear Span	Overall Length (L)	(Quantity-Type)	C ₀ = 1.6
LFTA	21/4	17	38 ³ / ₈	16-10d Common	1,325

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

maximum nominal depth of 12 inches.

2Half of the fasteners must be installed on each end of the strap to achieve the allowable uplift load.

3Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code.

4Tabulated allowable uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.



FIGURE 4-LFTA LIGHT FLOOR TIE ANCHOR

¹The LFTA anchor is used to transfer tension forces between vertically aligned wood studs across floor framing with floor joists having a

TABLE 5—SP AND SPH STUD PLATE TIES

CONNECTOR	MODEL		ECTOR IONS (in)	FÁSTEN (Quantity			PLIFT LOADS ^{2,3,4} JS)	
SERIES	NO.	(W)	(L)	To Stud	To Plate	C ₀ = 1.6		
			(10)	10 3000	TO Flate	S.G.=0.50	S.G.=0.55	
	SP1	_		6–10d	410d	555	555	
	SP2	_	_	6–10d	6–10d	1,010	1,010	
	SP4	3 ⁹ / ₁₆	71/4	6-10d x 1 ¹ / ₂	_ [825	825	
	SP4	3 /16		6-16d x 2 ¹ / ₂		850	850	
SP	SP6	5 ⁹ / ₁₆	73/4	6-10d x 1 ¹ / ₂		825	825	
				6-16d x 2 ¹ / ₂		850	850	
	SP8	SP8 7 ⁵ / ₁₆	8 ⁵ / ₁₆	6-10d x 1 ¹ / ₂	_	825	825	
				6-16d x 2 ¹ / ₂		850	850	
		CDUA	081	43,	10-10d x 1 ¹ / ₂			1,040
	SPH4	3/18	39/10 83/4	12-10d x 1 ¹ / ₂		1,280	1,175	
SPH	CDUE	59/16	01/	10-10d x 1 ¹ / ₂			1,040	
SPH	SPH6	16 57/16	91/4	12-10d x 1 ¹ / ₂	_	1,280	1,415	
	enue	75 }	03,	10-10d x 1 ¹ / ₂			1,175	
	SPH8	/ /16	7 ⁵ / ₁₆ 8 ³ / ₈	12-10d x 1 ¹ / ₂	<u> </u>	1,280	1,415	

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

Profile.")

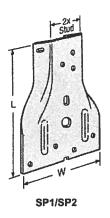
2 Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code.

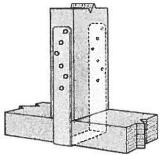
2 Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code.

3 Tabulated allowable uplift loads must be selected based on duration of load as permitted by the applicable building code. ³Tabulated allowable uplift loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be

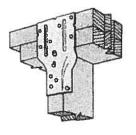
reduced when other load durations govern.

Allowable uplift loads are given for wood assemblies consisting of lumber having an assigned specific gravity (S.G.) of 0.50, such as Douglas fir-larch, and 0.55, such as southern pine.

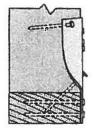




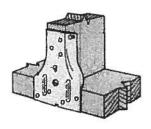
Typical SPH4 Installation: Stud to Wood Sill Plate (SP4 Similar)



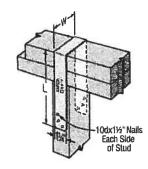
SP2 Installation: **Stud to Double Top Plate**



SP1 Nailing Profile



SP1 Installation: Stud to Sill Plate



Typical SP4 Installation: Double Top Plate to Stud (SPH Similar)

¹For Models SP1 and SP2, one 10d common stud nail must be installed as a toenail. It must be driven through the connector at an angle approximately 30° with the stud with the nail penetrating through the stud into the wood sill plate. (See detail on this page entitled "SP1 Nailing

TABLE 6-RSP4 REVERSIBLE STUD PLATE CONNECTOR¹

MODEL	FASTENERS (Quantity-Type)		ALLOWABLE LOADS ^{2,3} (lbs) C _D = 1.6				
NO. To Stud		To Plate	Connection Configuration ⁴	Uplift	Lateral ⁶		
	10 Otau	Enter the terms	Connection Conniguration	Opint	F ₁	F ₂	
RSP4 4-8d x 1 ¹ / ₂	1 04 × 41/ 4 04 × 41/	Stud to Double Top Plate	390	165	245		
	$4-8d \times 1^{1}/_{2}$ $4-8d \times 1^{1}/_{2}$		Stud to Sill Plate	245	165	225	

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

¹Refer to Figure 6a for overall dimensions of the RSP4 plate connector.

²Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

³Tabulated allowable loads have been increased for wind or earthquake loading. No further increase is allowed. Allowable loads must be reduced when other load durations govern.

⁴Refer to Figure 6b and 6c for connection configurations.

⁵F₁ load direction is parallel to plate, and F₂ load direction is perpendicular to plate.

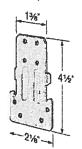


FIGURE 6a—RSP4 STUD PLATE CONNECTOR DIMENSIONS

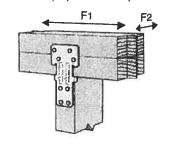


FIGURE 6b—RSP4 INSTALLATION: STUD TO DOUBLE TOP PLATE

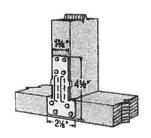


FIGURE 6c—RSP4 INSTALLATION: STUD TO SILL PLATE U.S. PATENT 5,697,725

TABLE 7—SSP AND DSP STUD PLATE TIE CONNECTORS

MODEL NO.		FASTENERS (Quantity-Type)		ALLOWABLE UPLIFT LOADS ^{1,2,3,4} (ibs) C ₀ =1.6			
				Double Top Plate	Sill I	Plate	
	Studs	Double Top Plate	Sili Plate	S,G. = 0.50	S.G. = 0,50	\$.G. = 0.43	
4-10d x 1 ¹ / ₂ SSP 4-10d	3-10d x 1 ¹ / ₂		330	-	_		
	4-10d x 11/2	_	1-10d x 1 ¹ / ₂	_	395	310	
		3–10d	_	410		_	
	4–10d	_	1–10d		430	400	
		6-10d x 1 ¹ / ₂		730			
DSP	8-10d x 1 ¹ / ₂		2-10d x 1 ¹ / ₂		620	515	
		6-10d		780		=	
	8–10d		2-10d		780	565	

For SI: 1 inch = 25.4 mm, 1 lbs = 4.45 N.

¹Tabulated allowable uplift loads have been increased for wind or earthquake loading no further increase allowed. Reduce loads when other load durations govern.

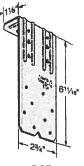
²When cross-grain bending or cross-grain tension cannot be avoided, mechanical reinforcement to resist such forces should be considered.

³For Sill Plate allowable uplift loads, all round nail holes in the connector must be filled with the specified quantity and type of nails.

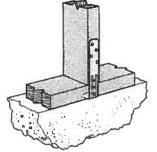
For Double Top Plate allowable uplift loads, all round and triangular nail holes the tie connectors must be filled with the specified quantity and type of nails.



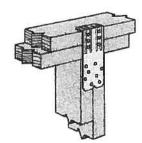
SSP U.S. Patent 7,065,932 7,356,973



DSP U.S. Patent 7,065,932 7,356,973



SSP Installation: Single Stud to SIII Plate



DSP Installation:
Double Stud to Double Top Plate

FIGURE 7—SSP/DSP STUD PLATE TIES

TABLE 8-HGT HEAVY GIRDER TIEDOWN CONNECTORS^{1,2}

MODEL3,4	HGT CONNECTOR WIDTH	DISTANCE BETWEEN THREADED RODS	FÄST (Quant	ALLOWABLE UPLIFT	
NO.	(W) (In.)	(on center) (inches)	Threaded Rod ^{7,8}	To Multi-ply Truss	C _b ≡1,6 (lbs)
HGT-2	3 ⁵ / ₁₆	5 ³ / ₄	2-5/8" Dia.	16-10d	10,345
HGT-3	415/18	7 ³ / ₆	2 ⁶ / ₈ " Dia.	16-10d	10,440
HGT-4	6 ⁹ / ₁₆	9	2–⁵/ ₈ " Dia.	16–10d	11,395

For SI: 1 inch = 25.4mm, 1 lbs = 4.45 N.

¹The HGT connector can accommodate top chord slopes from minimum 3:12 (14°) to maximum 8:12 (34°) and are provided with crescent washers for sloped top chord installations.

²All elements of the tie-down assembly (multi-ply trusses, vertically aligned wood studs/posts, and the full-height threaded rods) must be

designed to resist applied loads.

The HGT-2, HGT-3, and HGT-4 connector attaches to the heel joint of a two-ply, three-ply, and four-ply wood truss, respectively, where each

ply thickness is nominal 2 inches.

When the HGT-3 is used with a two-ply truss, shimming is required, and the shimming material must be similar (thickness and grade of lumber) as the truss member material. Additionally, the entire assembly must be designed by a registered design professional to act as one

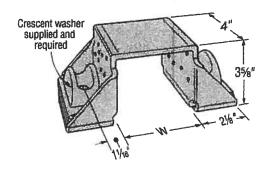
⁵Tabulated allowable loads must be selected based on duration of load as permitted by the applicable building code.

⁶The uplift loads have been increased for wind or earthquake loading with no further increase is allowed. Reduce loads when other load

durations govern.

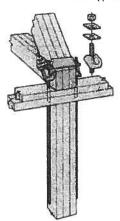
7Full-height threaded rods are shown in Figure 8 for illustration purposes only, as one method of transferring the design load from the HGT

connector to the foundation. The threaded rod material specifications must be specified by the registered design professional.
⁸Two LBP ⁵/₈-inch washers must be installed on top of each crescent washer. LBP washers and crescent washers are required. Crescent washers are supplied with the connector. LBP5/8 washers are available from Simpson Strong-Tie Company, and are 2-inch square by 9/64-inch thick galvanized steel washers with a center bolt hole to accommodate a 5/8-inch diameter threaded bolt/rod.



HGT-2 (HGT-3 and HGT-4 similar)

Install two LBP% washers on top of each crescent washer (total four %" washers) for wood installation. All washers and crescent washers are required. Crescent washers are supplied.



Typical HGT-3 Installation with full height threaded rod. The design of the threaded rod, including any necessary hardware or shrinkage compensating devices, is outside the scope of this report.

FIGURE 8-HGT HEAVY GIRDER TIEDOWN CONNECTOR

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FL# FL1999-R6 Application Type Revision Code Version 2017 **Application Status** Approved

> *Approved by DBPR. Approvals by DBPR shall be reviewed and ratified by the POC and/or the Commission if necessary.

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Comments Archived

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Category Subcategory Structural Components

Truss Plates

Compliance Method

Evaluation Report from a Product Evaluation Entity

Evaluation Entity Quality Assurance Entity

Benchmark Holdings, L.L.C. Quality Assurance Contract Expiration Date 12/31/2019

Validated By

ICC Evaluation Service, LLC

William M. Ranieri, PE

Validation Checklist - Hardcopy Received

Certificate of Independence

FL1999 R6 COI CERTIFICATION OF INDEPENDENCE - William M

Ranieri.pdf

Referenced Standard and Year (of Standard)

Standard TPI 1

Year 2014

Equivalence of Product Standards Certified By

Sections from the Code

Product Approval Method

Method 1 Option C

Date Submitted
Date Validated

09/07/2017 09/19/2017

Date Pending FBC Approval

Date Approved

09/21/2017

Summary of Products

FL#	Model, Number or Name	Description				
1999.1	ITW BCG/Alpine Higher Strength (H & S)	Metal Connector Plate				
Limits of Use Approved for us Approved for us Impact Resistar Design Pressure Other: per TPI 1	se outside HVHZ: Yes nt: N/A	Installation Instructions FL1999 R6 II Installation Instructions.pdf Verified By: Stuart Lewis PE 45927 Created by Independent Third Party: No Evaluation Reports FL1999 R6 AE ESR-1118.pdf				
1999.2	ITW BCG/Alpine Hinge Plate	Metal Connector Plate				
Limits of Use Approved for us Approved for us Impact Resistar Design Pressure Other: Per TPI 1	se outside HVHZ: Yes nt: N/A	Installation Instructions FL1999 R6 II Installation Instructions.pdf Verified By: Stuart Lewis PE 45927 Created by Independent Third Party: No Evaluation Reports FL1999 R6 AE ESR-1118.pdf				
1999.3	ITW BCG/Alpine Trulox	Metal Connector Plate				
Limits of Use Approved for us Approved for us Impact Resistan Design Pressure Other: per TPI 1	e outside HVHZ: Yes at: N/A	Installation Instructions FL1999 R6 II Installation Instructions.pdf Verified By: Stuart Lewis PE 45927 Created by Independent Third Party: No Evaluation Reports FL1999 R6 AE ESR-1118.pdf				
1999.4	ITW BCG/Alpine Wave	Metal Connector Plate				
Limits of Use Approved for us Approved for us Impact Resistan Design Pressure Other: Per TPI 1	e outside HVHZ: Yes it: N/A	Installation Instructions FL1999 R6 II Installation Instructions.pdf Verified By: Stuart Lewis PE 45927 Created by Independent Third Party: No Evaluation Reports FL1999 R6 AE ESR-1118.pdf				



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INSTALLATION INFORMATION FOR TRUSS PLATES

Installation instructions for these truss plate products are fully described by provisions of the Florida Building Code, including reference to the TPI 1 standard, and design drawings prescribed by those documents. The Florida Building Code and the TPI 1 standard require a truss design drawing specifying relevant parameters for the trusses in which these truss plate products are permitted to be used, including details on the truss plates (size, positioning, embedment, and limitations thereon). All uses of these truss plate products require such a truss design drawing, fabrication of the truss in accordance with the TPI 1 standard, and application of the truss within a structure in accordance with the Florida Building Code. Trusses are assembled using these truss plates pressed into the wood within the tolerances provided by TPI's Quality Criteria for Metal Plate Connected Wood Trusses, as shown in Section 3 of TPI 1.