

VSE Project Number: U3607.0073.221

April 29, 2022

Pure Energy Solar ATTENTION: Wayne Irwin 2121 NW 2nd St. Gainesville, FL 32609

#### REFERENCE: Madero Residence: 449 Southwest Madison Court, Lake City, FL 32024 Solar Array Installation

To Whom It May Concern:

We have reviewed the existing structure at the above referenced site. The purpose of our review was to determine the adequacy of the existing structure to support the proposed installation of solar panels on the roof as shown on the panel layout plan.

Based upon our review, we conclude that the existing structure is adequate to support the proposed solar panel installation.

#### **Design Parameters**

Code: 2020 Florida Building Code - Residential, 7th Edition (2018 IRC)

Risk Category: II Design wind speed, Vult: 118 mph (3-sec gust)

Wind exposure category: C

#### **Connection to Roof**

Mounting connection: (1) 5/16" lag screw w/ min. 2.5" threaded embedment into framing at max. 48" o.c. along rails Install (2) rails per row of panels, evenly spaced; panel length perpendicular to the rails shall not exceed 70 in Rail cantilever shall not exceed 33% of connection spacing Connections shall be staggered to not overload any existing structural member

#### Conclusions

Based upon our review, we conclude that the existing structure is adequate to support the proposed solar panel installation. In the area of the solar array, other live loads will not be present or will be greatly reduced (2020 FBC - Residential, Section R324.4.1.1). The gravity loads, and thus the stresses of the structural elements, in the area of the solar array are either decreased or increased by no more than 5%. Therefore, the requirements of Section 807.5 of the 2020 FBC - Existing Building are met and the structure is permitted to remain unaltered.

The solar array will be flush-mounted (no more than 10" above the roof surface) and parallel to the roof surface. Thus, we conclude that any additional wind loading on the structure related to the addition of the proposed solar array is negligible. The attached calculations verify the capacity of the connections of the solar array to the existing roof against wind (uplift), the governing load case. Increases in lateral forces less than 10% are considered acceptable. Thus the existing lateral force resisting system is permitted to remain unaltered.



#### **Limitations**

Installation of the solar panels must be performed in accordance with manufacturer recommendations. All work performed must be in accordance with accepted industry-wide methods and applicable safety standards. The contractor must notify Vector Structural Engineering, LLC should any damage, deterioration or discrepancies between the as-built condition of the structure and the condition described in this letter be found. The use of solar panel support span tables provided by others is allowed only where the building type, site conditions, site-specific design parameters, and solar panel configuration match the description of the span tables. The design of the solar panels, solar racking (mounts, rails, etc.) and electrical engineering is the responsibility of others. Waterproofing around the roof penetrations is the responsibility of others. Vector Structural Engineering assumes no responsibility for improper installation of the solar array. Vector Structural Engineering shall be notified of any changes from the approved layout prior to installation.

VECTOR STRUCTURAL ENGINEERING, LLC

FL Firm License: COA 26626



04/29/2022

Jacob Proctor, P.E. FL License: 74277 - Expires: 02/28/2023 Project Engineer

Enclosures

JSP/bac

This item has been digitally signed and sealed by Jacob Proctor on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



#### **PROJECT:** Madero Residence Components and Cladding Wind Calculations Label: Solar Panel Array Note: Calculations per ASCE 7-16 SITE-SPECIFIC WIND PARAMETERS: Basic Wind Speed [mph]: 118 Notes: Exposure Category: С **Risk Category:** Ш ADDITIONAL INPUT & CALCULATIONS: Height of Roof, h [ft]: 15 (Approximate) Comp/Cladding Location: Gable Roofs $20^{\circ} < \theta \le 27^{\circ}$ Enclosure Classification: Enclosed Buildings Zone 1, 2e GCp: 1.50 0.77 Figure 30.3-2C Zone 1, 2e $\gamma_a$ : Fig. (negative coeff.) 29.4-8 Zone 2n, 2r, 3e GCp: 2.43 Zone 2n, 2r, 3e $\gamma_a$ : 0.77 Zone 3r $\gamma_a$ : Zone 3r GCp: 2.84 0.77 9.5 Table 26.11-1 α: z<sub>a</sub> [ft]: 900 Table 26.11-1 K<sub>h</sub>: 0.85 Table 26.10-1 K<sub>e</sub>: 1.00 Table 26.9-1 K<sub>zt</sub>: 1 Equation 26.8-1 K<sub>d</sub>: 0.85 Table 26.6-1 Velocity Pressure, q<sub>h</sub> [psf]: 25.6 Equation 26.10-1 $\gamma_{E}$ : 1.50 Section 29.4.4 $p = q_h (GC_p)(\gamma_E)(\gamma_a)$ WIND PRESSURES: Equation 29.4-7 Zone 1, 2e, p [psf]: 44.5 psf (1.0 W) Zone 2n, 2r, 3e, p [psf]: 72.0 psf (1.0 W) Zone 3r, p [psf]: psf (1.0 W) 84.2

(a = 3 ft)



## Calculate Uplift Forces on Connection

	Pressure (0.6 Dead -0.6 Wind) (psf)	Max Trib. Width <sup>1</sup> (ft)	Max Trib. Area <sup>2</sup> (ft <sup>2</sup> )	Max Uplift Force (lbs)
Zone 1, 2e	24.9	4.0	11.7	291
Zone 2n, 2r, 3e	41.4	4.0	11.7	483
Zone 3r	48.7	4.0	11.7	569

## **Calculate Connection Capacity**

Lag Screw Size [in].	5/16	
	5/10	
C <sub>d</sub> :	1.6	NDS Table 2.3.2
Embedment <sup>3</sup> [in]:	2.5	
Grade:	SPF (G = 0.42)	
Nominal Capacity [lbs/in]:	205	NDS Table 12.2A
Number of Screws:	1	
Prying Coefficient:	1.4	
Total Capacity [lbs]:	586	

## **Determine Result**

Maximum Demand [lbs]:	569 586	
Result:	Capacity > Dema	and, Connection is adequate.

## <u>Notes</u>

1. 'Max Trib. Width' is the width along the rails tributary to the connection.

2. 'Max Trib Area' is the product of the 'Max. Trib Width' and 1/2 the panel width/height perpendicular to the rails. (2) rails per row of panels. Length of panels perpendicular to the rails shall not exceed 70".

3. Embedment is measured from the top of the framing member to the beginning of the tapered tip of the lag screw. Embedment in sheathing or other material is not effective. The length of the tapered tip is not part of the embedment length.



GRAVITY LOADS		Roof Pitch:	6.0 :12
ROOF DEAD LOAD (D)	Design material weight [psf]	Increase due to pitch	Material weight [psf]
Metal Corrugated	3.4	1.12	3.0
1/2" Plywood	1.1	1.12	1.0
Framing	3.0		3.0
Insulation	0.5		0.5
1/2" Gypsum Clg.	2.2	1.12	2.0
M, E & Misc	6.5		6.5
Total Existing Roof DL	16.7		
PV Array DL	3.4	1.12	3

## ROOF LIVE LOAD (Lr)

Existing Design Roof Live Load [psf] Roof Live Load With PV Array [psf]

20	ASCE 7-16 Table 4.3-1
0	2020 FBC - Residential, Section R324.4.1.1



Summary of Loads Existing With PV Array D [psf] 17 20 Lr [psf] 20 0 S [psf] 0 0 Maximum Gravity Loads: Existing With PV Array (D + Lr) / Cd [psf] 29 ASCE 7-16, Section 2.4.1 22 (D + S) / Cd [psf] 19 22 ASCE 7-16, Section 2.4.1 (Cd = Load Duration Factor = 0.9 for D, 1.15 for S, and 1.25 for Lr) Maximum Gravity Load [psf]: 29 22 Ratio Proposed Loading to Current Loading: 76% ОК The gravity loads, and thus the stresses of the structural elements, in the area of the solar array are either decreased or increased by no more than 5%. Therefore, the requirements of Section 807.5 of the 2020 FBC - Existing Building are met and the structure is permitted to remain unaltered.





Madero Layout 449 SW Madison Ct. Lake City, FL 32024