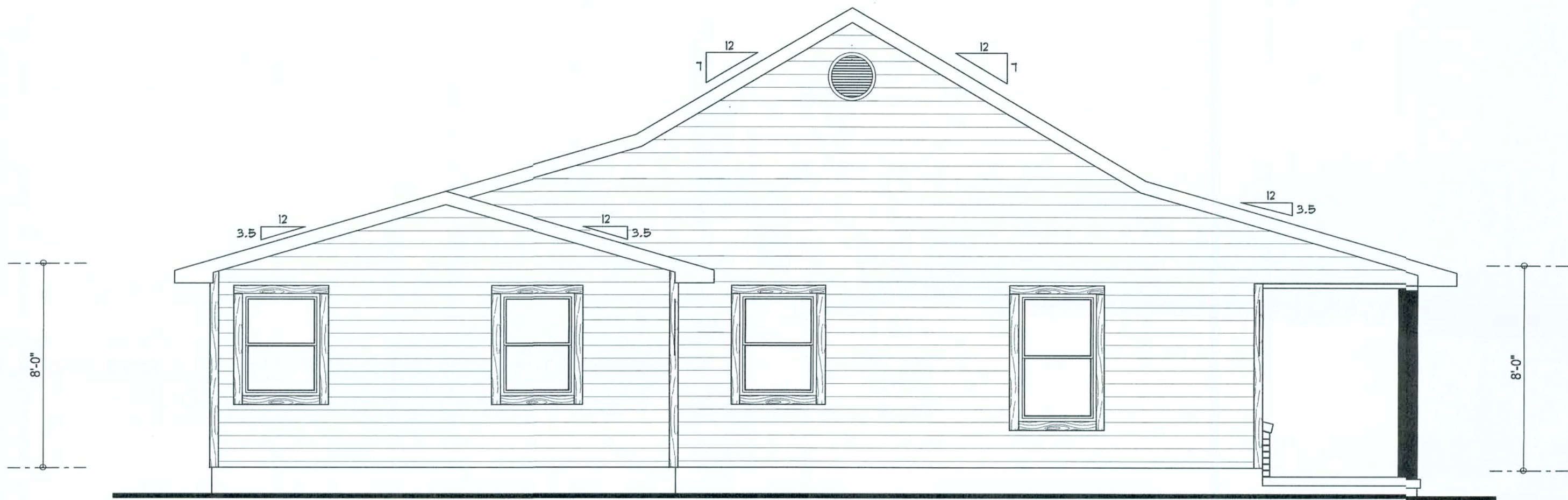


REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



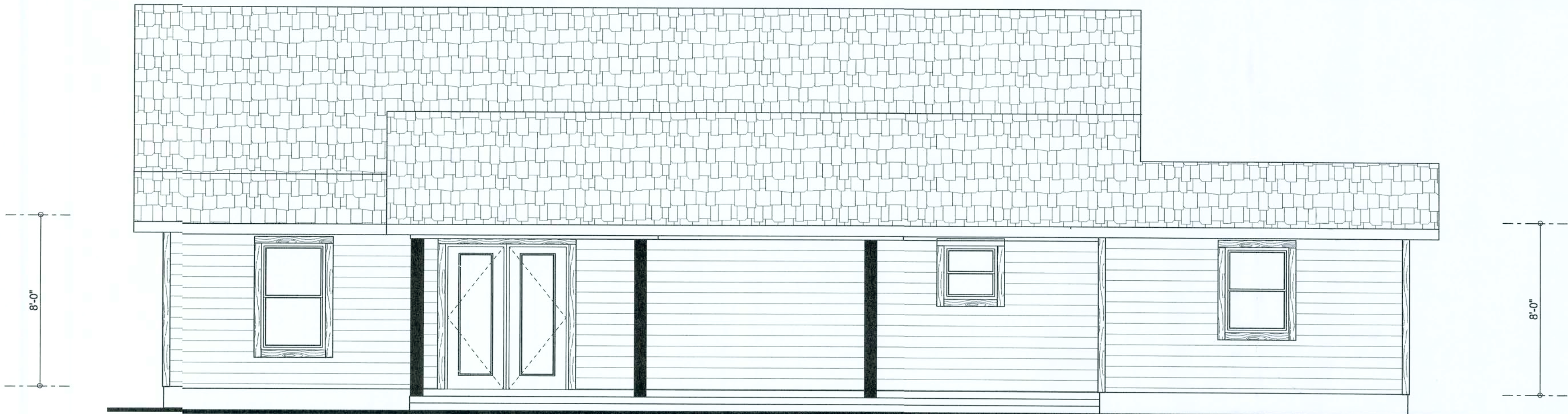
FRONT ELEVATION



LEFT ELEVATION



RIGHT ELEVATION



REAR ELEVATION



Erkinger Construction Group

John McClain Residence

ADDRESS:

Parcel# 09-4S-17-08301-009

Columbia County

PRINTED DATE:

February 01, 2017

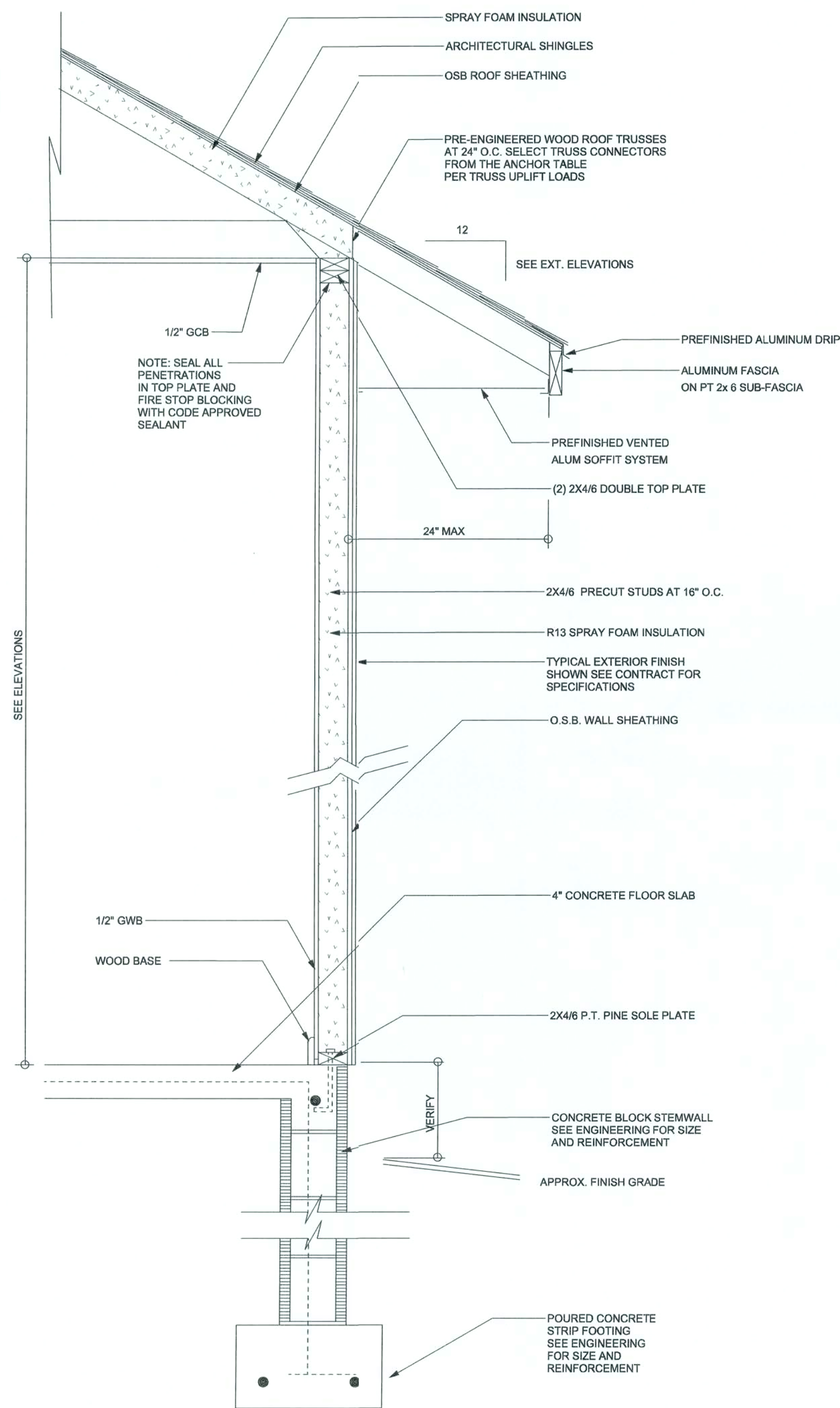
DRAWN BY:

Matthew A. Erkinger Sr.

DRAWING NUMBER

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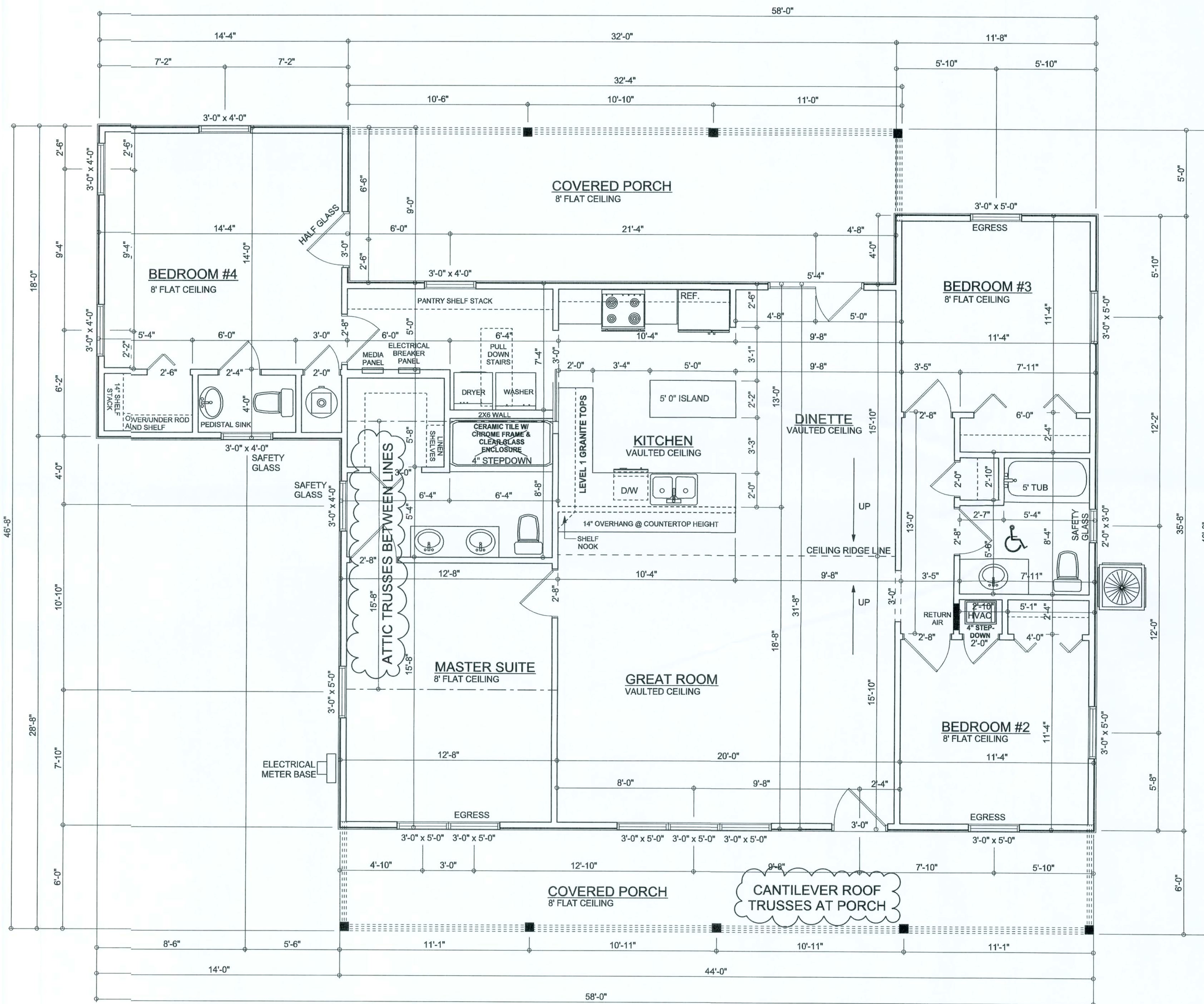
OF 3 SHEETS



TYPICAL DESIGN WALL SECTION

NON - STRUCTURAL DATA

SCALE: 1" = 1'-0"



FLOOR PLAN

SCALE: 1/4" = 1'-0"

ALL WINDOWS WITHIN 12" OF FINISH FLOOR HEIGHT ARE SAFETY GLASS

AREA SUMMARY

LIVING AREA	1,721	S . F .
COVERED PORCH AREA	548	S . F .
TOTAL AREA	2,269	S . F .

Erkinger Construction

Group

John McClain
Residence

ADDRESS:
Parcel#
09-4S-17-08301-009
Columbia County

PRINTED DATE:
February 01, 2017

DRAWN BY:
Matthew A. Erkinger Sr.

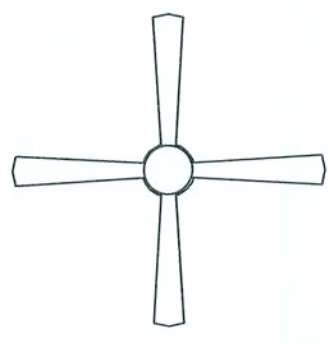
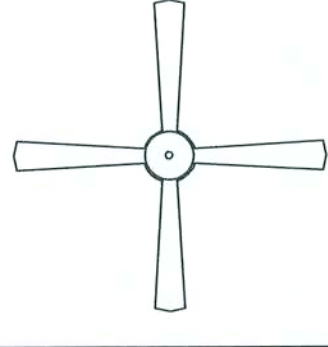
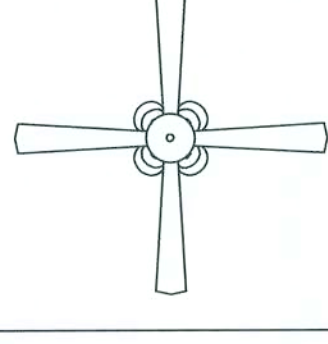
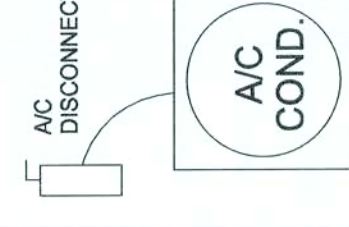
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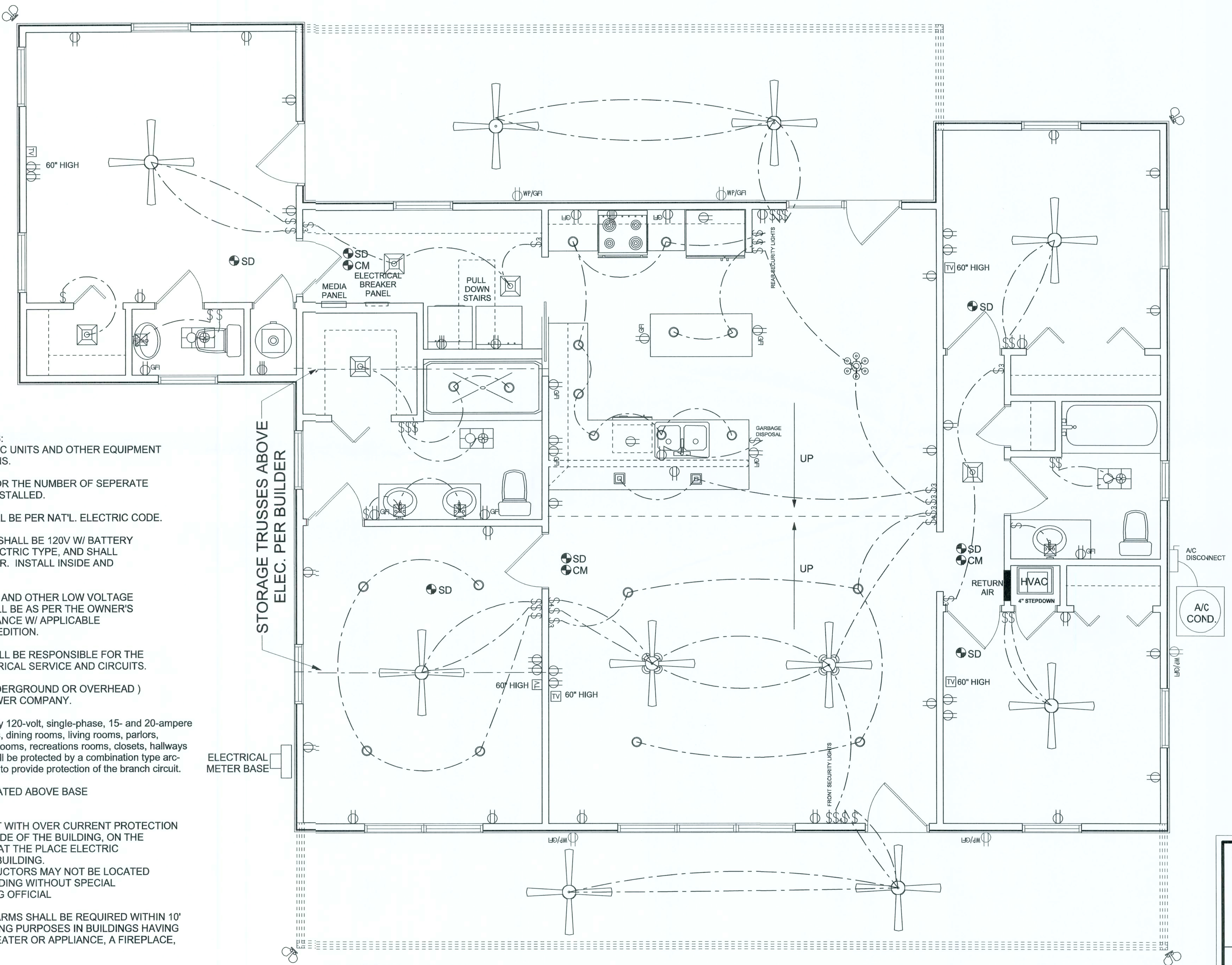
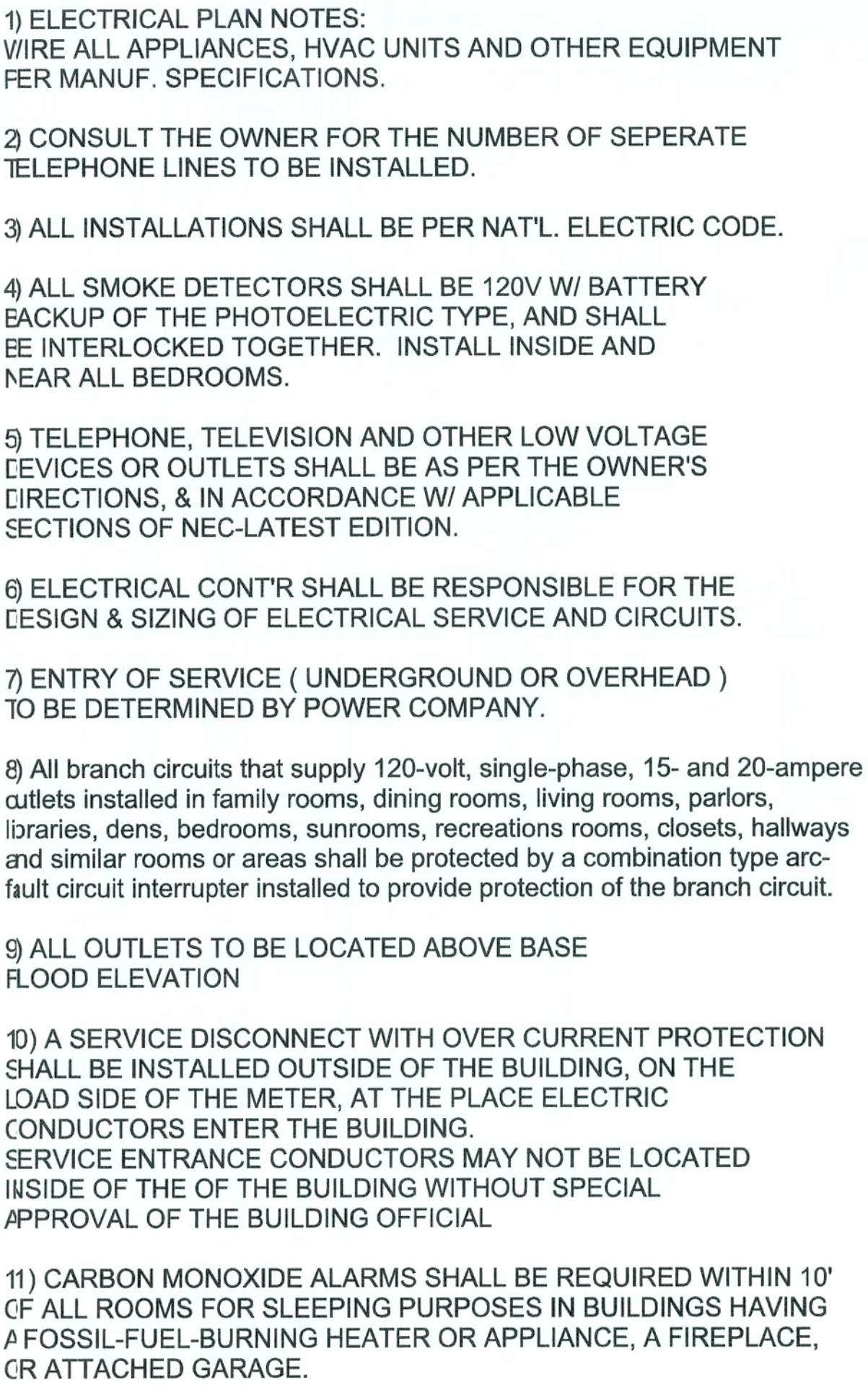
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OF 3 SHEETS

REVISIONS	

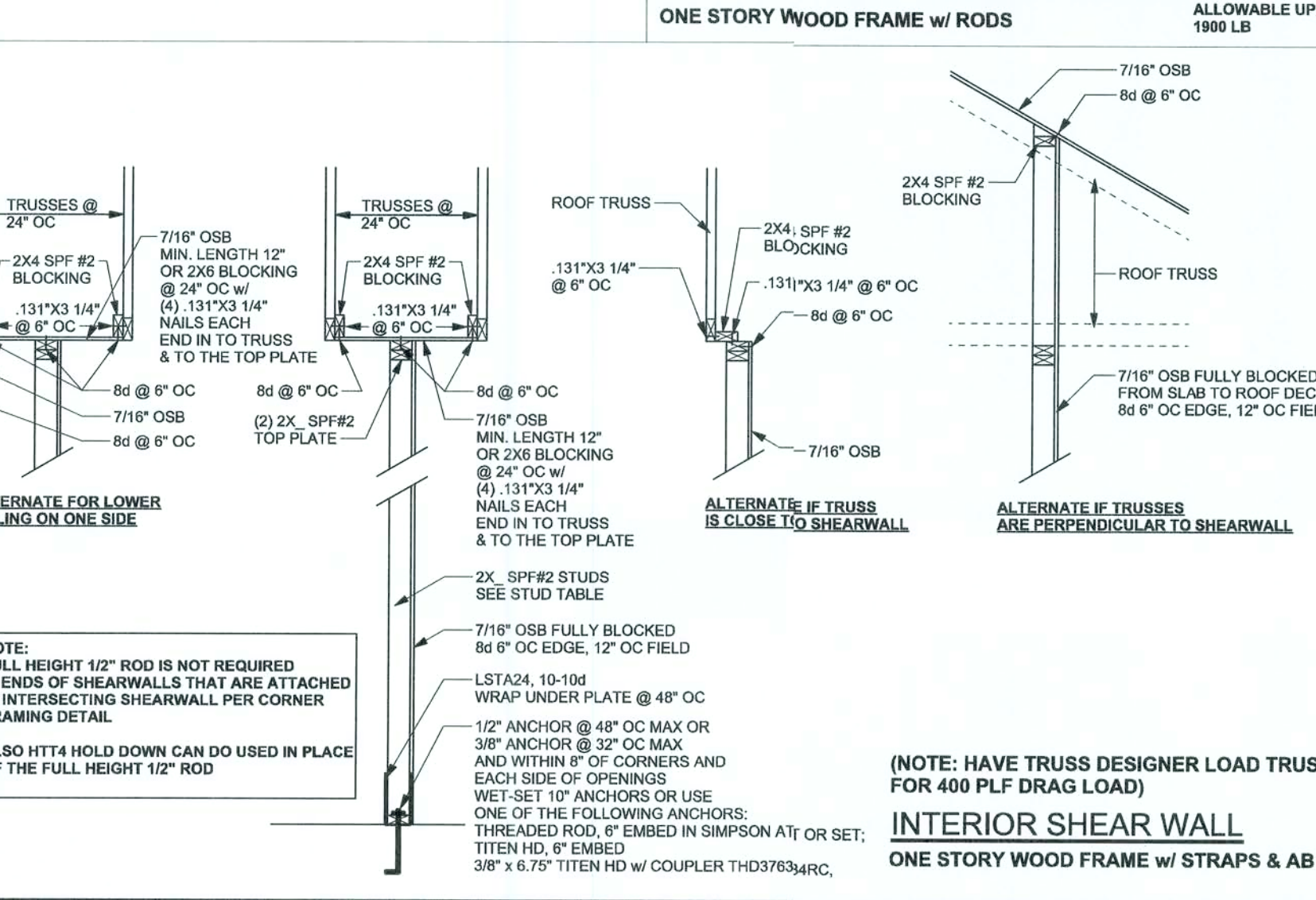
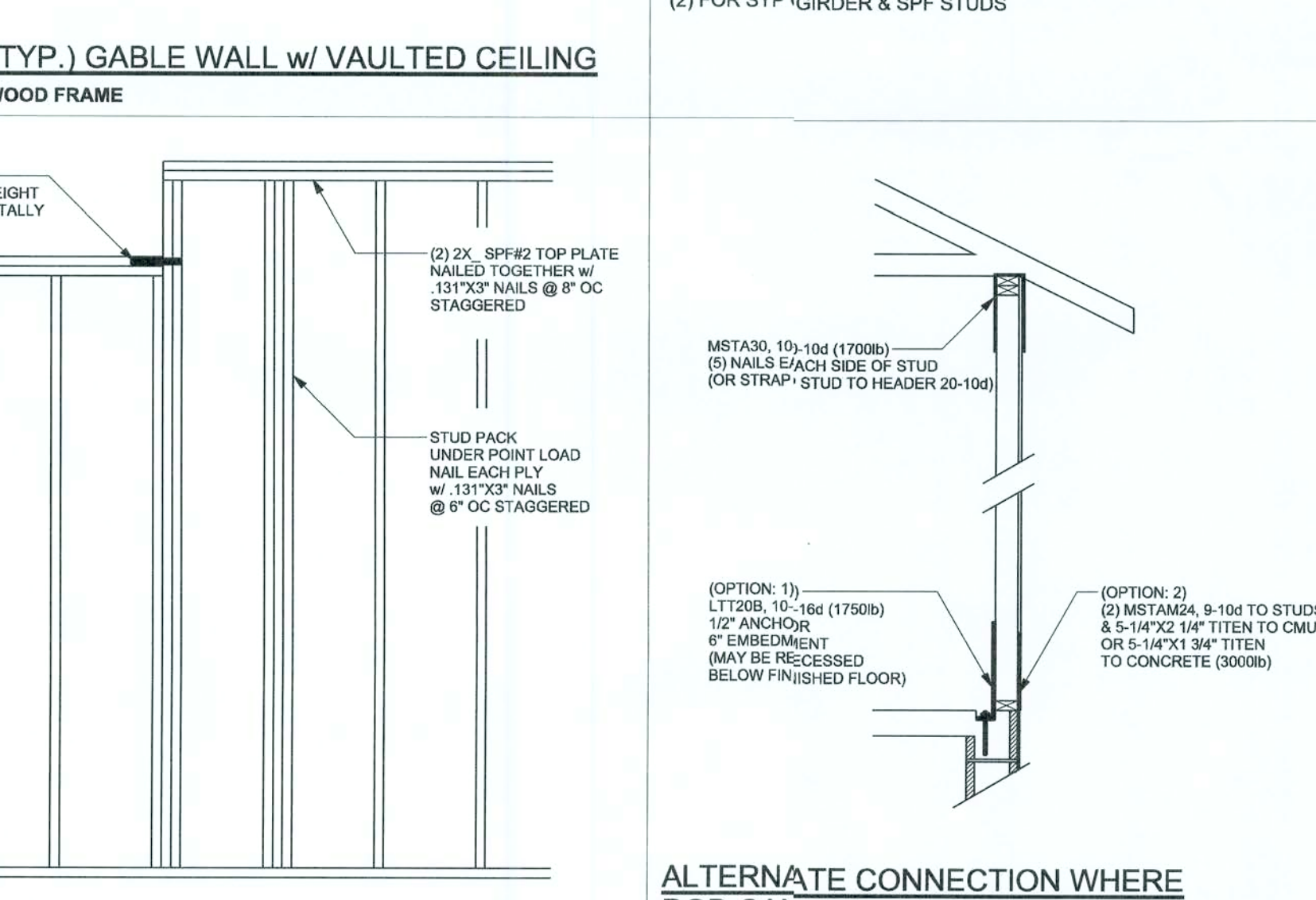
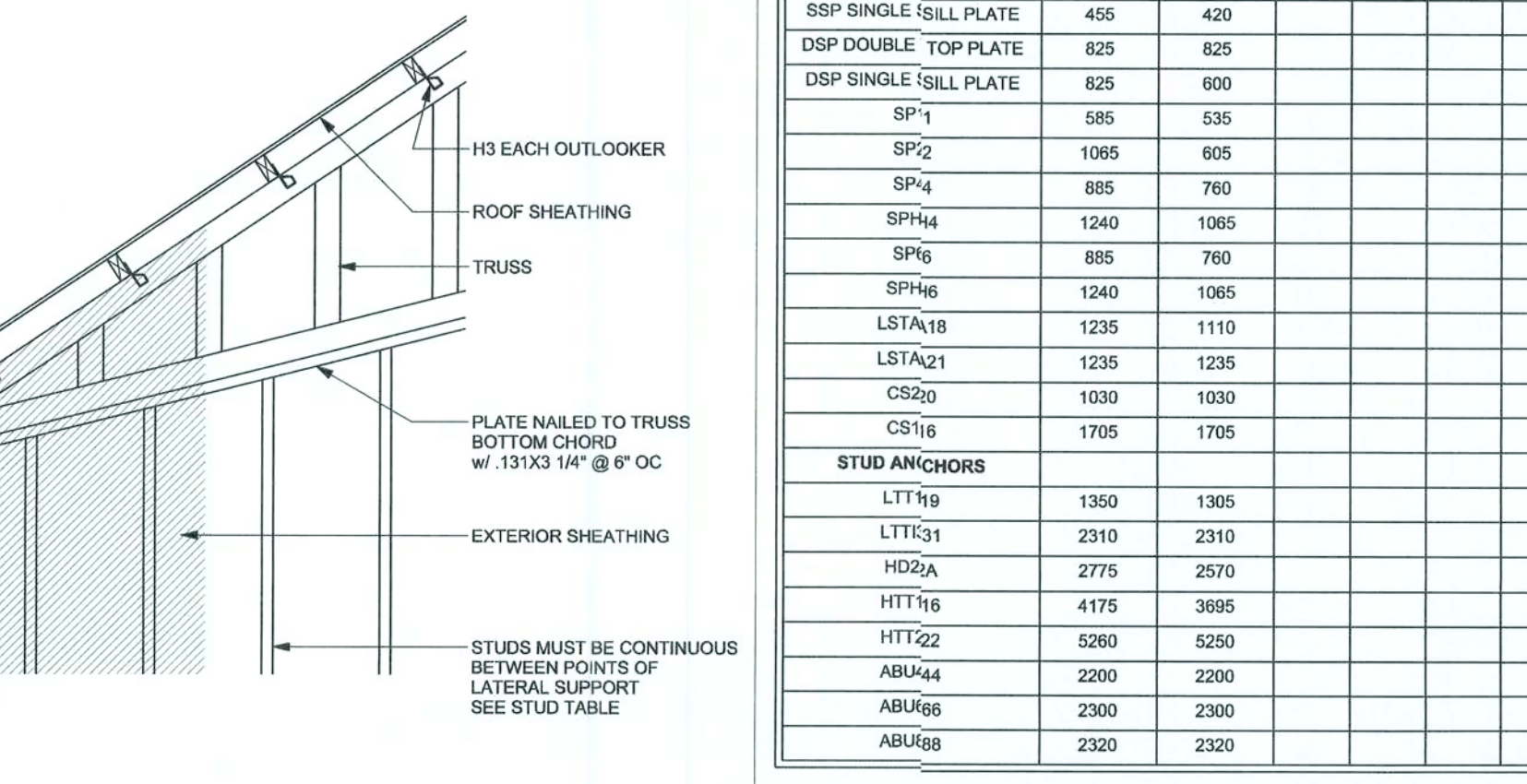
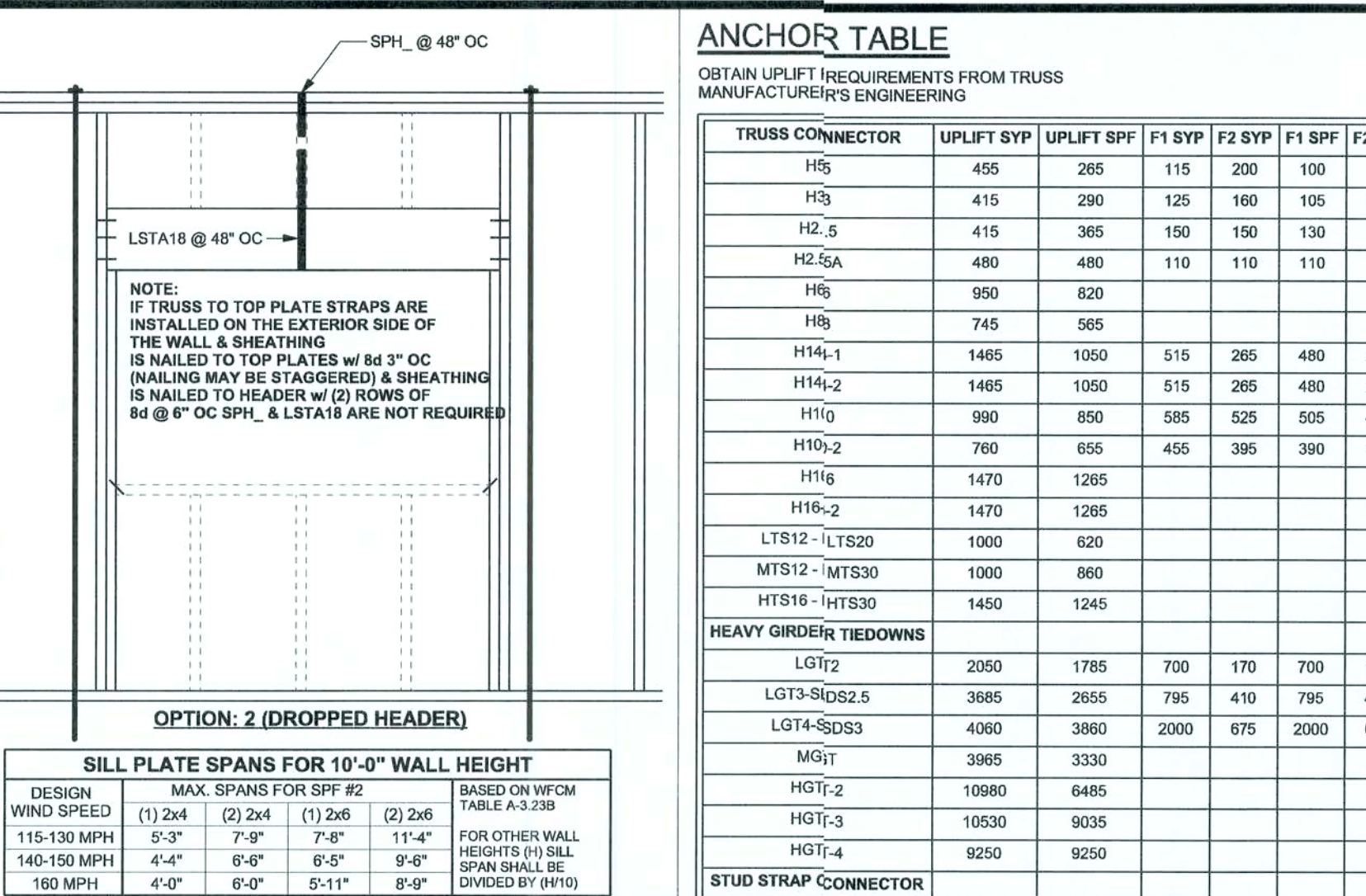
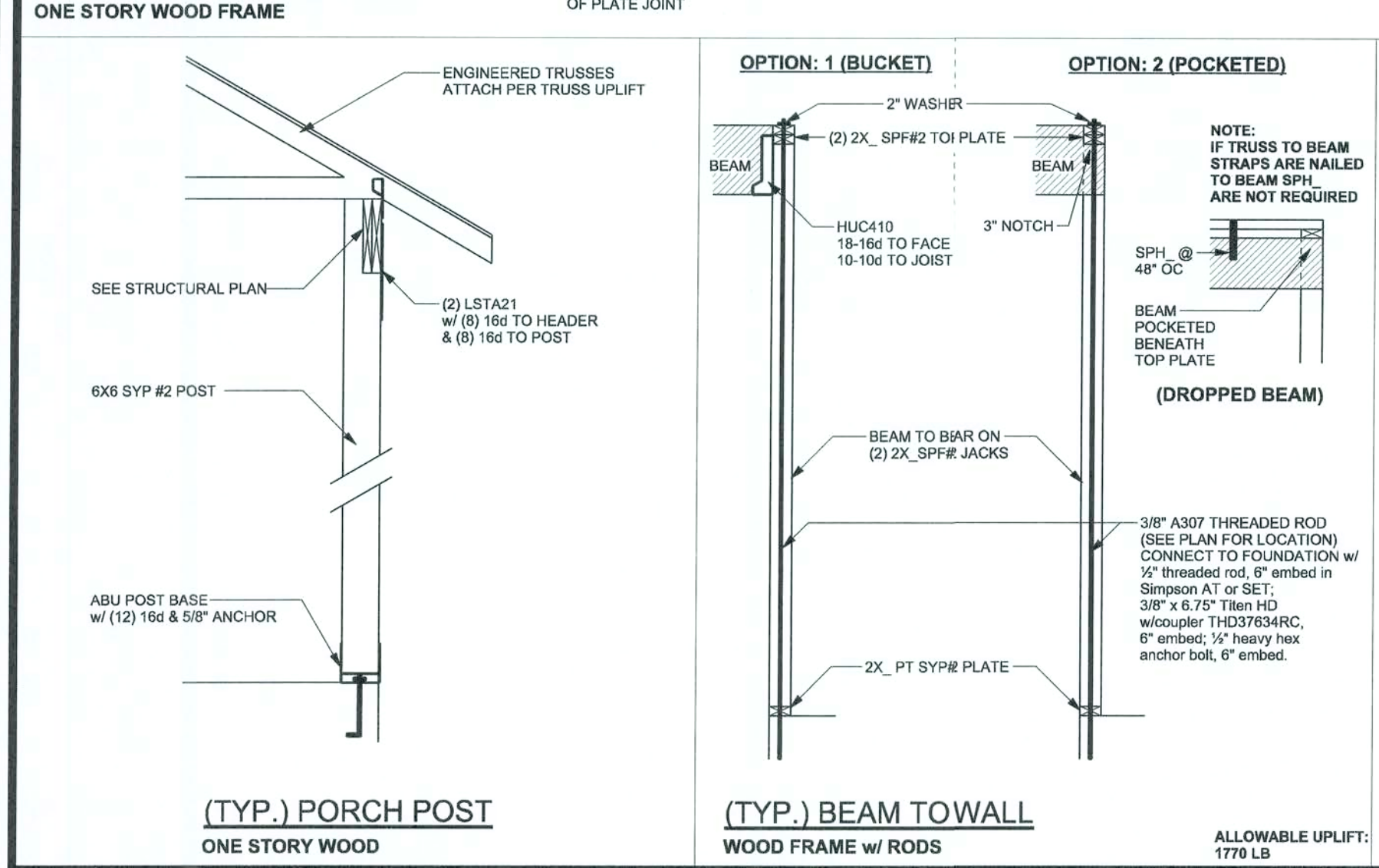
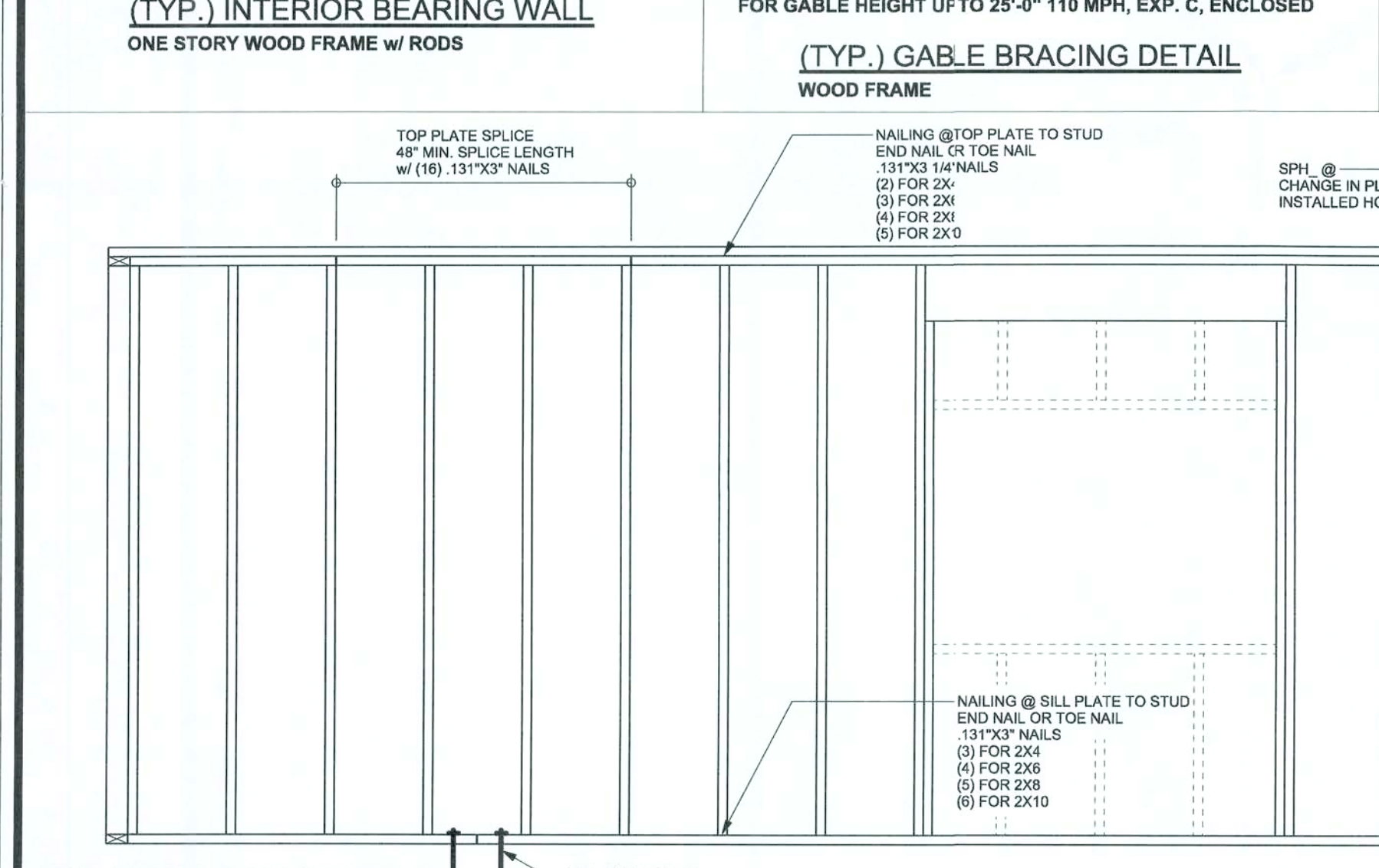
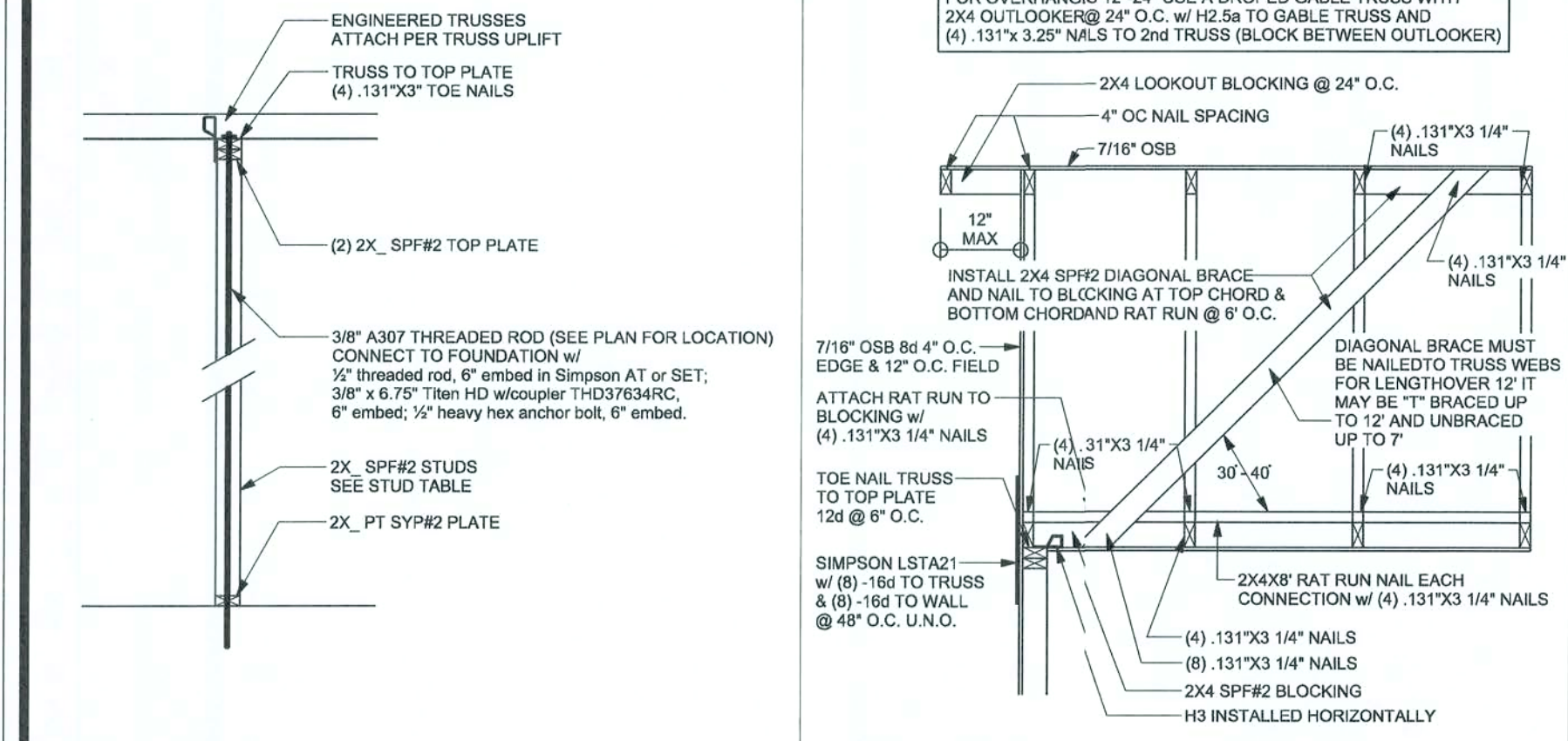
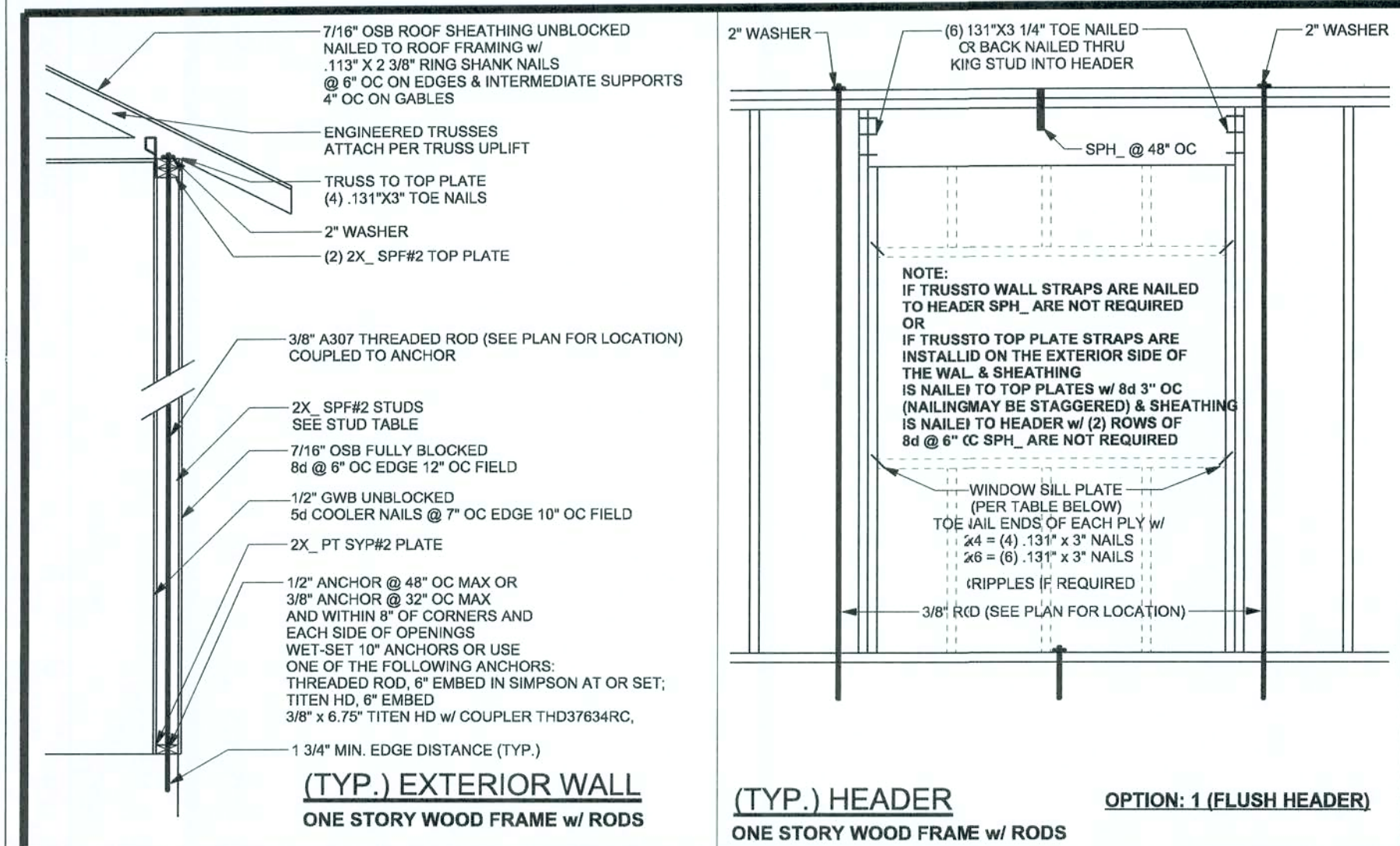
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL	SYMBOL
electrical panel	
ceiling fan globe 1	
ceiling fan globe 2	
ceiling fan spotlights 1	
ceiling shade square	
exterior light 2	
chandelier	
pendant large	
AC Disconnect	
Fan Light - bathroom	
Light - Can	
Light - Security	
cable tv outlet	
outlet	
outlet 220v	
outlet gfi	
outlet wp gfi	
switch	
switch 3 way	
switch 4 way	



ELECTRICAL PLAN

Erkinger Construction Group John McClain Residence ADDRESS: Parcel# 09-4S-17-08301-009 Columbia County
PRINTED DATE: February 01, 2017
DRAWN BY: Matthew A. Erkinger Sr.
DRAWING NUMBER #3 OF 3 SHEETS



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT GYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	285	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H5	415	290	125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H8	950	620					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	980	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1285					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1285					2-10d x 1 1/2"	10-10d x 1 1/2"
LT812 - LT820	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MT512 - MT530	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HT516 - HT530	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"

HEAVY GIRDER TIEDOWNS

LG12	2050	1785	700	170	700	170	14-16d	14-16d
LG13-SIDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 2 1/2"	26-16ds
LG14-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16ds
MSG1	3965	3330					22-10d	5/8" ANCHOR
HGT1-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT1-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT1-4	9250	9250					16-10d	2-5/8" ANCHOR

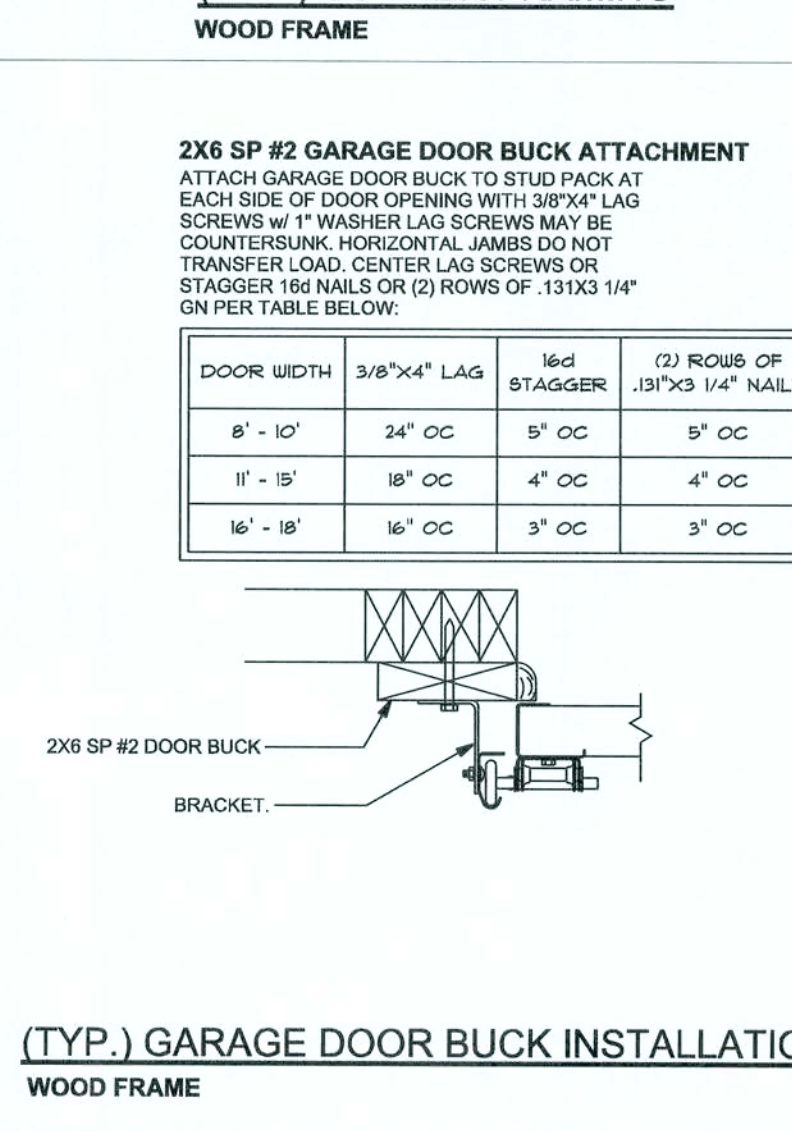
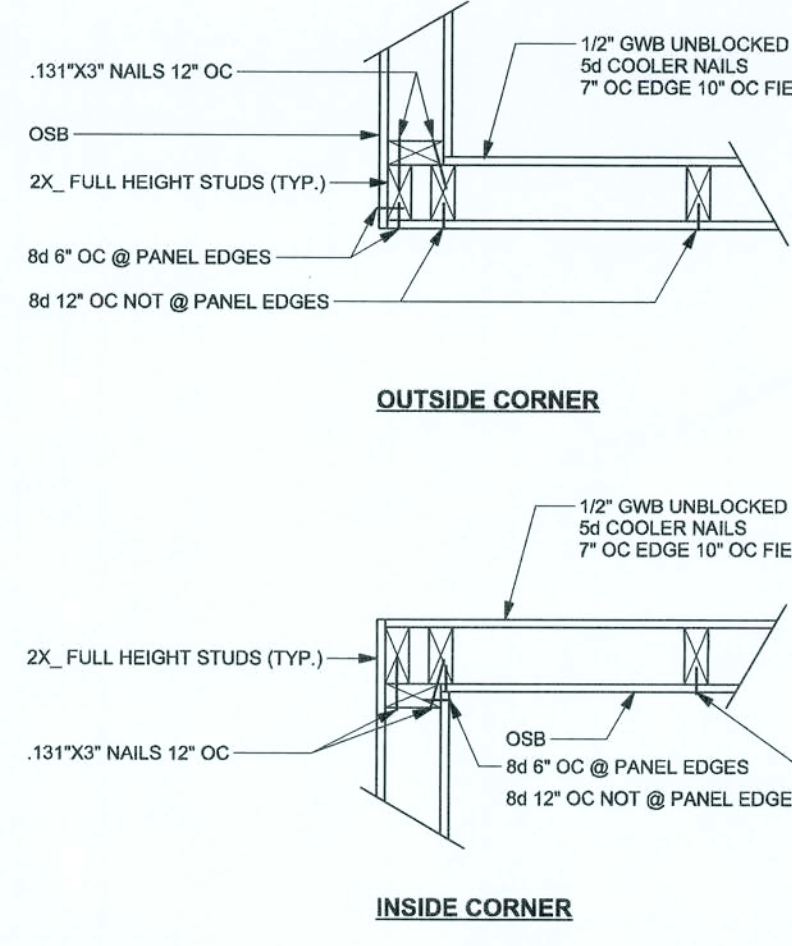
STUD STRAP CONNECTOR

SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE ISILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	6-10d
DSP SINGLE ISILL PLATE	825	800					2-10d	6-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	
SPH4	1240	1065					10-10d x 1 1/2"	
SP6	885	760					6-10d x 1 1/2"	
SPH6	1240	1065					10-10d x 1 1/2"	
LSTA18	1235	1110					14-10d	
LSTA21	1235	1235					16-10d	
CS20	1030	1030					14-10d	
CS16	1705	1705					22-10d	

STUD ANCHORS

LT176	1350	1365					8-16d	1/2" ANCHOR
LT1731	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HD2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HT176	4175	3695					18-16d	5/8" ANCHOR
HT122	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR

(1) W/ INSTALLATION OF 4-16ds OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

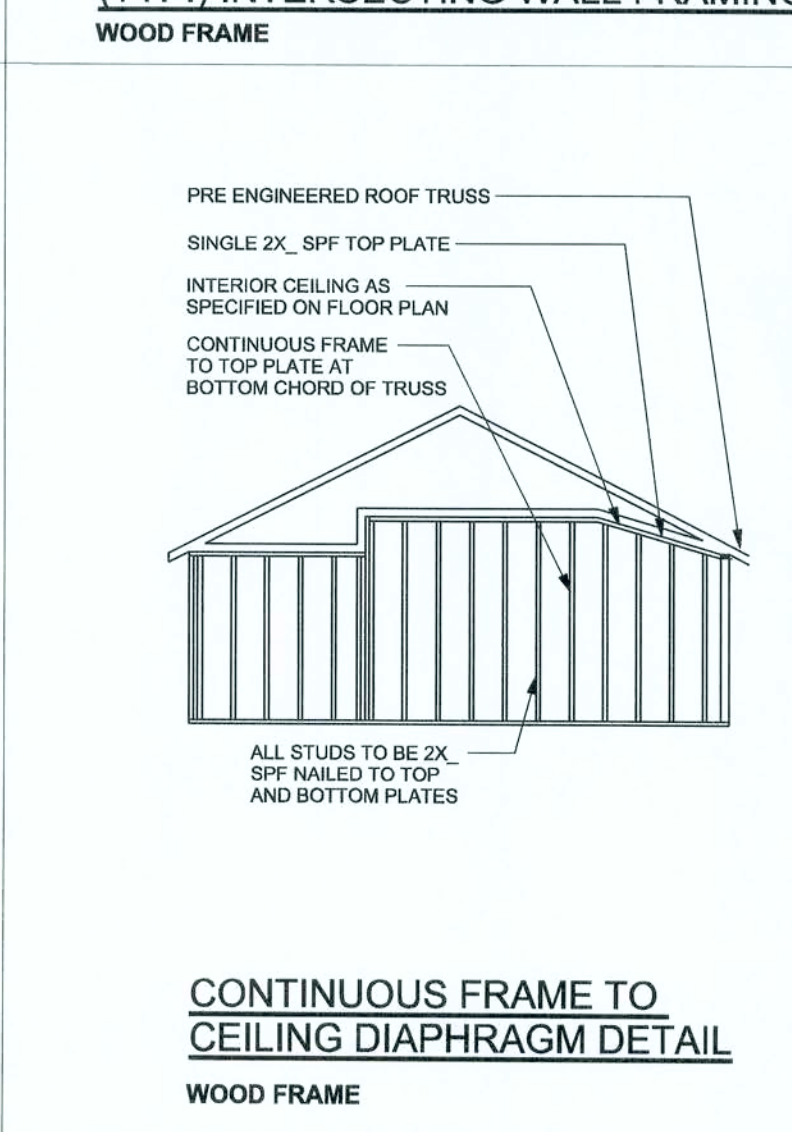
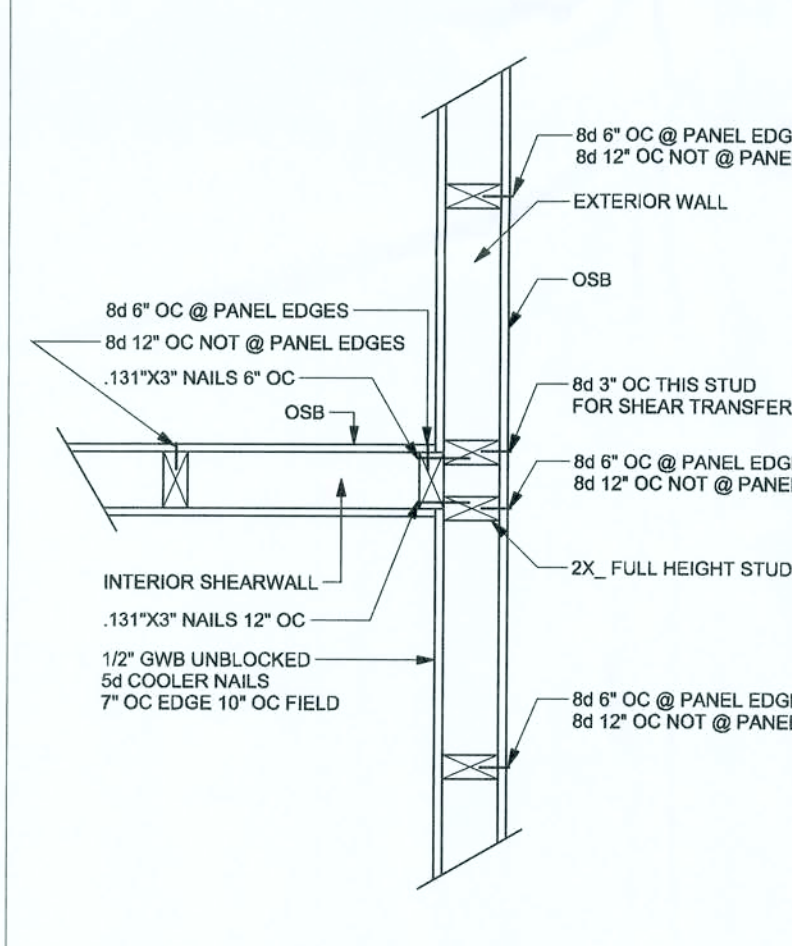


EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP2	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2400	2.0
PSL	PARALAM	2900	2.0



GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBOR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: $6" \times 6" \times W14 \times 4$, $F_y = 60$ KSI. WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 16 FT. DO NOT CUT WHEN OR BEFORE FORMING (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A615, GRADE 60, DEFORMED BARS, $F_y = 60$ KSI. ALL LAP SPICES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-05, U.N.O.

GULAM BEAMS: GLB, 24F-V3SP, $F_b = 2400$, $E = 180000$ UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZED CALCULATIONS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 RAILING MEMBERS, WITH PANEL EDGES STAGGERED. FASTENED WITH 8d COMMON NAILS (131) 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTOR ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAMOR 16" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16" UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBOR TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK:

- CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
- PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBOR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
- PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION; IF YOU BELIEVE THE PLAN OMIT A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
- VERIFY THE TRUSS MANUFACTURER'S SEPALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBOR, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBOR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER 5th EDITION FLORIDA BUILDING CODE RESIDENTIAL (2014) R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS, MEAN ROOF HEIGHT)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

- BASIC WIND SPEED = 130 MPH, (3 SEC GUST, 33 FT, EXP. C)
- WIND EXPOSURE = C, BUILDER MUST FIELD VERIFY
- TOPOGRAPHIC FACTOR = 1.0, BUILDER MUST FIELD VERIFY
- RISK CATEGORY = II, (MRI = 700 YR)
- ROOF ANGLE = 7-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2.2)

Zone	Effective Wind Area (ft ²)	
	10	
1	39	-43
2	39	-68
3	39	-100
4	43	-46
5	43	-57

Garage Door Table R301.2.4			
8x7 Garage Door	37	42	
16x7 Garage Door	36	-40	

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12)

ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12)

12 PSF (12:12 AND GREATER)

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)

SOIL BEARING CAPACITY 1000PSF

NOT IN FLOOD ZONE (BUILDER TO VERIFY)

REVISIONS

NO.	DESCRIPTION

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, P.O. Box 888, Lake City, FL 32065, 386-754-6416

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 5th Edition Florida Building Code Residential (2014) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

Erkinger Construction Group

John McClain Residence

ADDRESS: Parcel# 09-45-17-08301-009 Columbia County

Mark Disoway P.E. 163 SW Midtown Place, #103 Lake City, Florida 32025 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: Wednesday, February 1, 2017

DRAWN BY: STRUCTURAL BY: Even Beamley

FINAL DATE: 2017-02-01

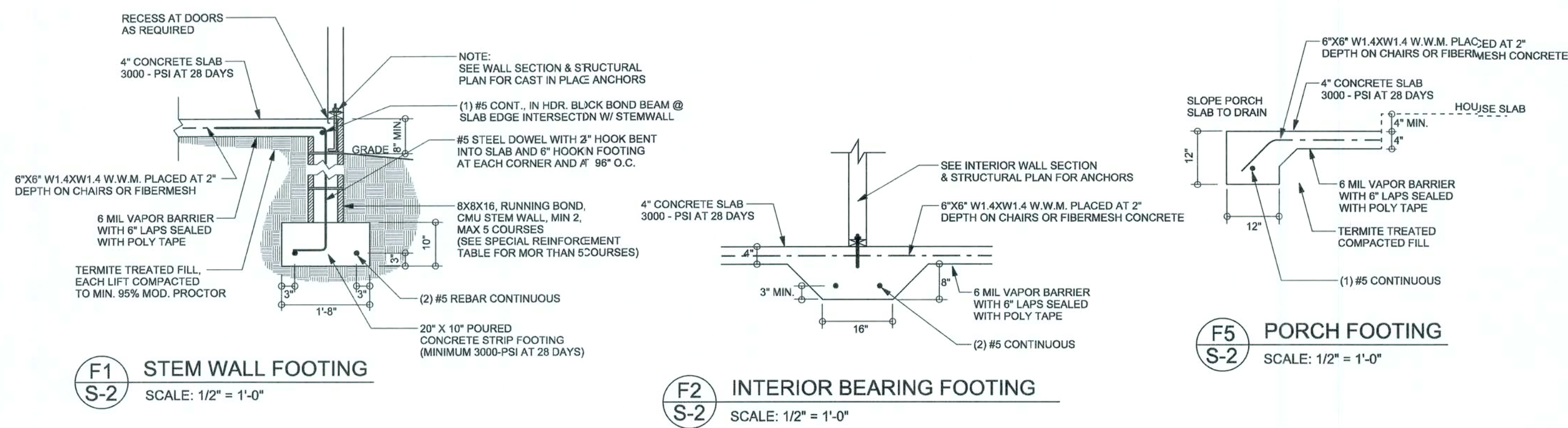
JOB NUMBER: 170000

DRAWING NUMBER S-1

OF 2 SHEETS

REVISIONS	

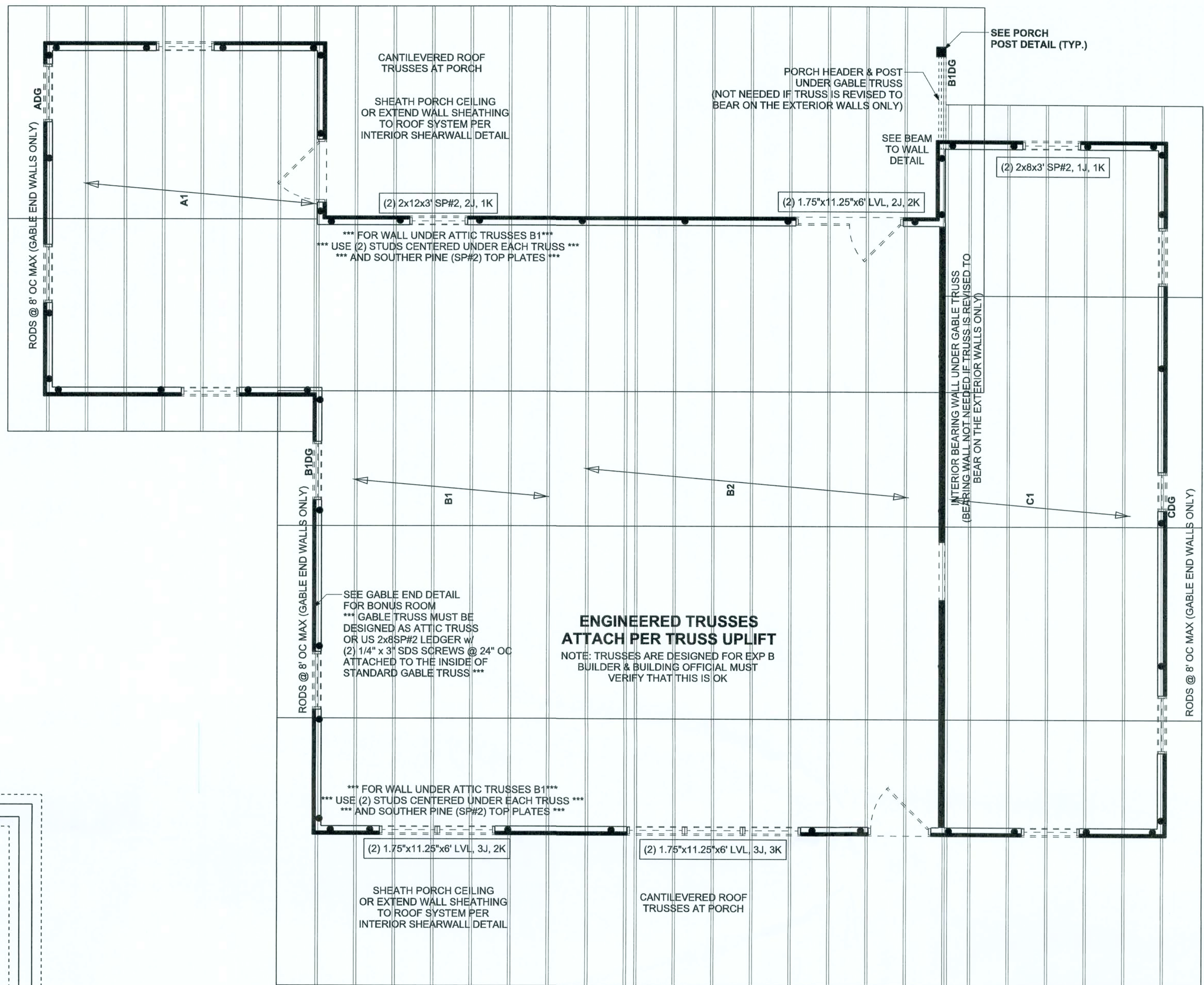
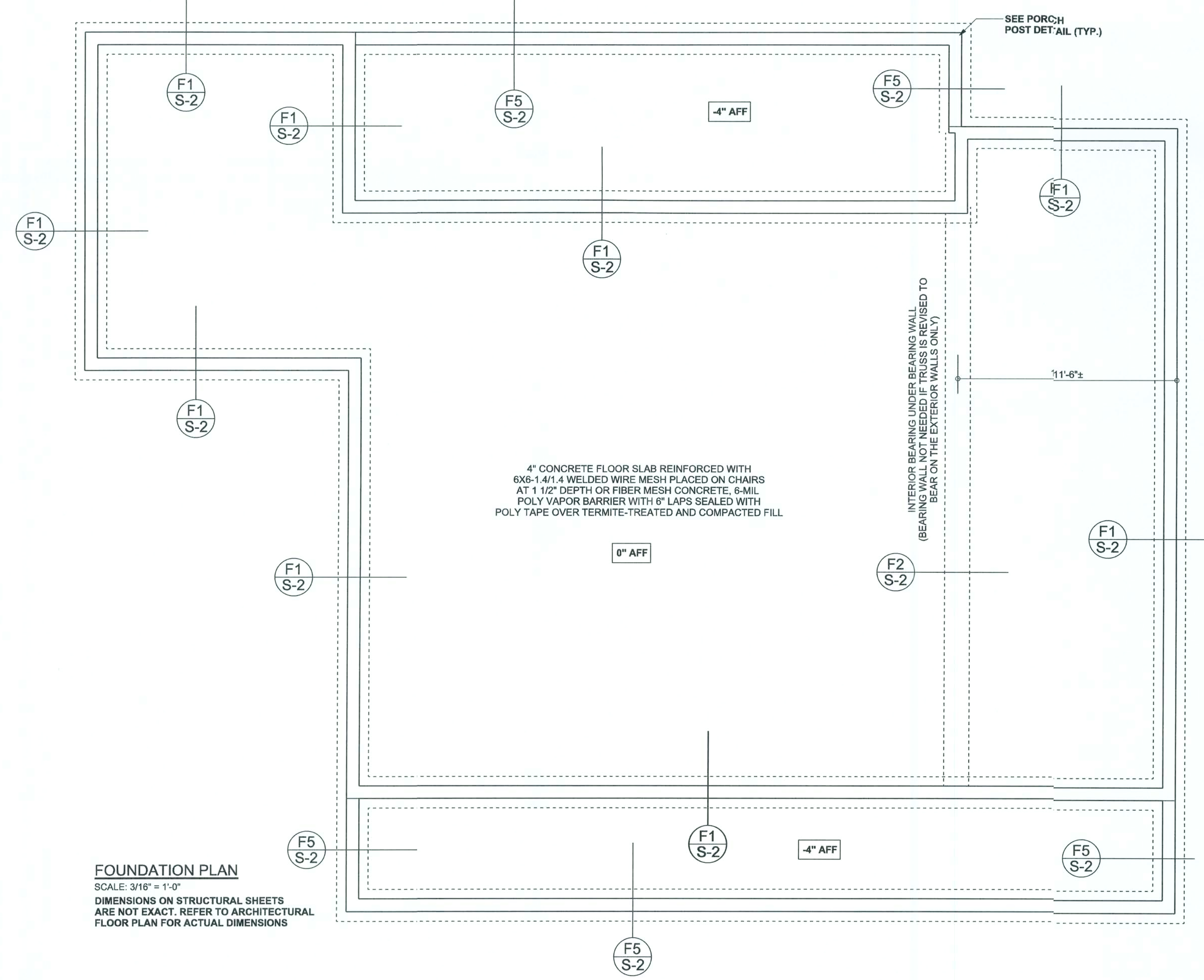
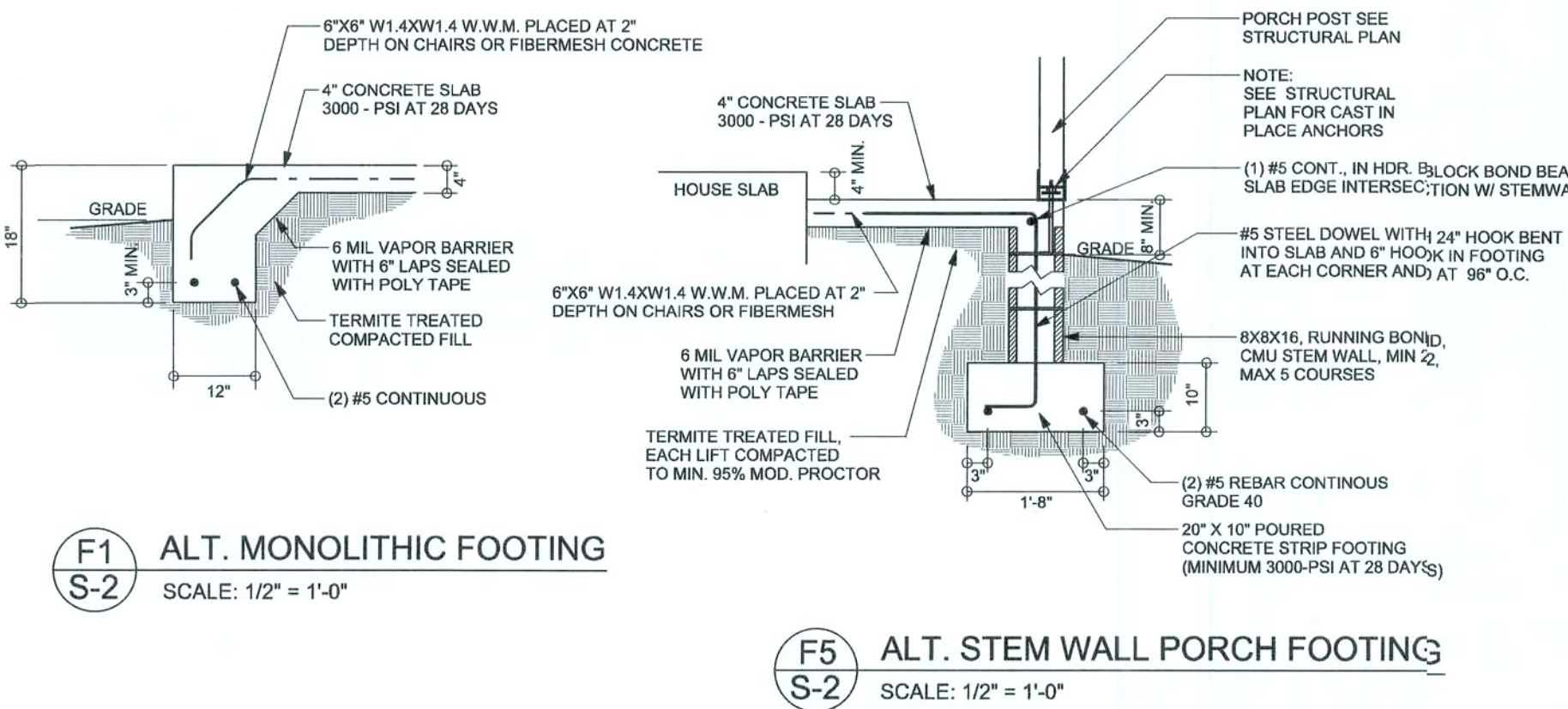
SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Dupont ladder reinforcement at 16" O.C. vertically or a horizontal bond beam with 165 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 3/8" A307 ALL THREADED ROD

STRUCTURAL PLAN NOTES

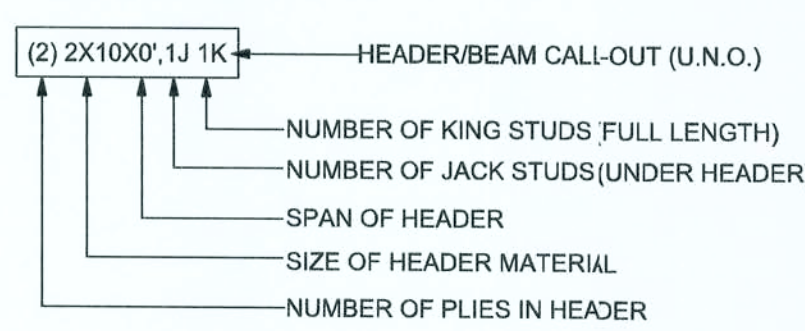
- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X6 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS1-03, BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	15871 LBS	27992 LBS
LONGITUDINAL	11191 LBS	32395 LBS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS JOB #16-126A

HEADER LEGEND



BONUS ROOM / GABLE END BRACING

SCALE: 1/2" = 1'-0"

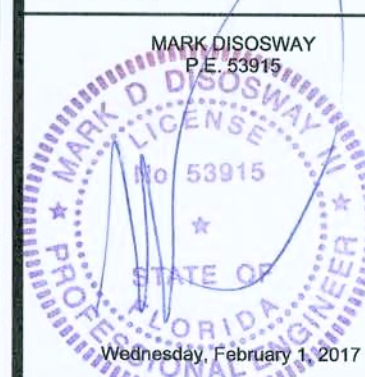
WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, PCB 698, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, 5th Edition Florida Building Code Residential (2014) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.



Erkinger Construction Group

John McClain Residence

ADDRESS:
Parcel#
09-4S-17-06301-009
Columbia County

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163 SW Midtown Place, #103
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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE:
Wednesday, February 1, 2017

DRAWN BY: STRUCTURAL BY:
Evan Beamsley

FINALS DATE:
2017-02-01

JOB NUMBER:
170000

DRAWING NUMBER
S-2
OF 2 SHEETS