45064



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: zieger - Zieger

## Site Information:

Customer Info: SCCI Project Name: . Model: . Lot/Block: . Address: ., . City: Columbia County

Subdivision: .

State: FI

Name Address and License # of Structural Engineer of Record, If there is one, for the building. License #: Name: Address: City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 42 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

N 123456789011234567890	Seal# T27168000 T27168001 T27168002 T27168003 T27168005 T27168006 T27168006 T27168007 T27168007 T27168010 T27168010 T27168011 T27168013 T27168014 T27168015 T27168016 T27168016 T27168018 T27168018 T27168018	Truss Name A1 A2 A3 A4 A5 B1GIR B2 B3 B4 B5 B6 B7 B8 B6 B7 B8 B9 B10 B11 B12GIR C1GE C2 CJ01	Date 3/18/22	No. 234256789012334567890142	Seal# T27168022 T27168023 T27168024 T27168025 T27168026 T27168028 T27168029 T27168030 T27168030 T27168031 T27168033 T27168034 T27168035 T27168035 T27168036 T27168037 T27168038 T27168038 T27168041 T27168041	Truss Name D3 D4 D5 D6 E1GIR E2 E3 E4 E5GIR F1GE J1 J2 J2R J3 J4 M1 M2GIR M4 M5 M6	Date 3/18/22
19 20 21 22	T27168018	C2		41	T27168040	M5	

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Mayo Truss Company, Inc..

Truss Design Engineer's Name: Lee, Julius

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115

Lee, Julius

1 of 1



5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 2, 23 lb uplift at joint 10 and 114 lb uplift at joint 8.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

sters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 ray, 5/19/2020 BEFORE USE. A WARNING - Verify design p Design valid for use only with MTRK® connectors. This design is based only upon parameters and recardle reference reace nurvey and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses systems, see **ANSTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





capacity of bearing surface. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 1, 58 lb uplift at joint 10 and 310 lb uplift at joint 7.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



March 21,2022

🗛 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITeKe connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

ViiTek 6904 Parke East Blvd. Tampa FL 36610



TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

BOT CHORD WEBS

TOP CHORD

REACTIONS. (size) 1=0-3-8, 11=0-3-8, 8=0-3-8 Max Horz 1=-145(LC 10) Max Uplift 1=-7(LC 12), 11=-60(LC 12), 8=-323(LC 21) Max Grav 1=696(LC 1), 11=2220(LC 1), 8=126(LC 22)

2x4 SP No.2

2x4 SP No.2

2x4 SP No.2

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-2051/36, 2-3=-1014/0, 3-4=-291/64, 6-7=-8/1349, 7-8=0/1038

BOT CHORD 1-15=0/1878, 14-15=0/1874, 13-14=0/880, 11-12=-1255/152, 10-11=-893/0, 8-10=-893/0 WEBS 2-14=-955/124, 3-14=0/806, 3-13=-921/0, 5-13=0/741, 5-12=-973/50, 6-12=0/1339, 6-11=-1533/60, 7-11=-516/341

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=34ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 3-5-0, Interior(1) 3-5-0 to 15-2-8, Exterior(2E) 15-2-8 to 18-11-8, Exterior(2R) 18-11-8 to 23-11-5, Interior(1) 23-11-5 to 35-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 1, 60 lb uplift at joint 11 and 323 lb uplift at joint 8.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permament bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permament bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see <u>ANXITP1 Quality Criteria, DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 10 and 120 lb uplift at

joint 7. 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022



🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev; 5/19/2020 BEFORE USE. Design valid for use only with MTeKe connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **AVSE/THT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty Ply	1	Zieger	T27168005
ZIEGER	B1GIR	Hip Girder	1	2	Job Reference (optional)	127106005
Mayo Truss Company, In	c., Mayo, FL - 32066,				16 2021 MiTek Industries, Inc. Thu Mar 17 11:10:1 bbop2zce56-evrqT4tvF8roh7t5pETFo54iwAwbh7gK0	

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 3=-184(B) 5=-184(B) 10=-358(B) 8=-358(B) 17=-125(B) 19=-125(B) 20=-125(B) 22=-125(B) 23=-62(B) 24=-62(B) 25=-62(B) 26=-62(B) 26=-6

WARNING - Vority design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rov. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANXITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





i i	4-9-4	9-0-0	15-0-0		19-2-12	24-0-0	)
	4-9-4	4-2-12	6-0-0		4-2-12	4-9-4	
Plate Offsets (X,Y)	[4:0-5-4,0-2-0], [10:0-2-8,0-3	3-0]					
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/TPI2	2-0-0 CSI. 1.25 TC 1.25 BC YES WB 014 Matrix	0.40 Vert(L 0.47 Vert(C 0.17 Horz(C x-AS	Ý) -0.17 9-10	>999 240	PLATES MT20 Weight: 121 lb	<b>GRIP</b> 244/190 FT = 20%
BOT CHORD 2x4 S	SP No.2 SP No.2 SP No.2		BRACI TOP CI BOT CI	IORD Structu	ural wood sheathing direc ceiling directly applied.	tly applied.	
REACTIONS. (si	ize) 7=0-3-8, 2=0-3-8						

Max Horz 2=84(LC 11) Max Uplift 2=-38(LC 12) Max Grav 7=957(LC 1), 2=1053(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-3=-1767/91, 3-4=-1398/116, 4-5=-1219/111, 5-6=-1409/107, 6-7=-1771/97
- BOT CHORD 2-11=-42/1532, 10-11=-42/1532, 9-10=-6/1201, 8-9=-42/1550, 7-8=-42/1550

WEBS 3-10=-383/42, 4-10=0/319, 5-9=0/357, 6-9=-391/61

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 9-0-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 15-0-0, Exterior(2R) 15-0-0 to 19-2-12, Interior(1) 19-2-12 to 24-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate arib DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permament bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		5-9-4		11-0-0	1	13-0-0		18	-2-12	The second se	24-0-0	
		5-9-4	1	5-2-12		2-0-0		5-	2-12		5-9-4	1
Plate Off	sets (X,Y)- [4	4:0-5-4,0-2-0], [10:0-2-8,	0-3-0]	-								
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.30	Vert(LL)	-0.06	10	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.47	Vert(CT)	-0.14	10-11	>999	180	V. 60 102200-00	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.35	Horz(CT)	0.05	7	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS	100000000000000000000000000000000000000					Weight: 127 lb	FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

LUMBER-

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2

WEBS 2x4 SP No.2

REACTIONS. (size) 7=0-3-8, 2=0-3-8 Max Horz 2=101(LC 11) Max Uplift 2=-38(LC 12) Max Grav 7=957(LC 1), 2=1053(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1733/80, 3-4=-1237/109, 4-5=-1042/113, 5-6=-1236/107, 6-7=-1732/87

- 2-11=-25/1496, 10-11=-25/1496, 9-10=0/1034, 8-9=-22/1511, 7-8=-22/1511 BOT CHORD

WEBS 3-11=0/251, 3-10=-530/55, 4-10=-4/300, 5-9=-2/326, 6-9=-547/66

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 11-0-0, Exterior(2E) 11-0-0 to 13-0-0, Exterior(2R) 13-0-0 to 17-2-15, Interior(1) 17-2-15 to 24-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

## DULIUS / EE ONAL //////

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

March 21,2022

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI Quality Criteria, DSB-89 and BCSI Building Comport Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	ŀ	6-3-4 6-3-4	+		12-0-0 5-8-12	1		17-8-1 5-8-12			24-0-0 6-3-4	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.36	Vert(LL)	-0.06	7-12	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.51	Vert(CT)	-0.15	8-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.05	6	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	k-AS	1 1 1					Weight: 114 lb	FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2

REACTIONS. (size) 6=0-3-8, 2=0-3-8 Max Horz 2=109(LC 11) Max Uplift 2=-38(LC 12) Max Grav 6=957(LC 1), 2=1053(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1709/123, 3-4=-1163/147, 4-5=-1164/152, 5-6=-1707/132

BOT CHORD 2-9=-58/1469, 8-9=-58/1469, 7-8=-56/1485, 6-7=-56/1485 WEBS

4-8=-20/652, 5-8=-607/83, 5-7=0/280, 3-8=-589/79, 3-9=0/277

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 12-0-0, Exterior(2R) 12-0-0 to 15-0-0, Interior(1) 15-0-0 to 24-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 2.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

A WARNING - Verify design pu Ners and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTeKe connectors. This doubt HTS AND includeD bin HEREPECTIVE PAGE MILLING WITH AND THE SAUD MOLECULED WITH AND THE PECTIVE PAGE MILLING WITH AND THE SAUD MOLECULED WITH AND THE PECTIVE PAGE MILLING WITH AND THE SAUD MOLECULED WITH AND THE PECTIVE PAGE MILLING WITH AND THE SAUD MOLECULED WITH AND THE PECTIVE PAGE MILLING WITH AND THE SAUD MOLECULED WITH AND THE PECTIVE PAGE MILLING WITH AND THE PECTIVE PECTIVE PAGE MILLING WITH AND THE PECTIVE PECTIV fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Que Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

A WARNING - Visitly design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTRK⊕ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPH Quality Criteria**, DSB-89 and BCSI Building Compore **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES	CSI. TC 0.88 BC 0.73 WB 0.29	Vert(CT) -0.	in (loc) l/defl 12 10-13 >999 26 10-13 >999 04 7 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190 FT = 20%
BCDL 10.0 LUMBER- TOP CHORD 2x4 SP 1 BOT CHORD 2x4 SP 1 WEBS 2x4 SP 1	No.2	Matrix-AS	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood s Rigid ceiling dired	ctly applied.	Weight: 152 lb tly applied, except e 3-9. 4-8	

REACTIONS. (size) 7=0-3-8, 2=0-3-8 Max Horz 2=240(LC 11) Max Uplift 7=-18(LC 9), 2=-33(LC 12) Max Grav 7=951(LC 1), 2=1047(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1555/33, 3-4=-863/106, 4-5=-451/113, 5-6=-451/113, 6-7=-912/96

BOT CHORD 2-10=-182/1295, 9-10=-184/1291, 8-9=-149/713

WEBS 3-10=0/365, 3-9=-661/40, 4-9=0/445, 4-8=-523/101, 5-8=-285/76, 6-8=-83/910

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 15-2-0, Exterior(2R) 15-2-0 to 19-7-0, Interior(1) 19-7-0 to 23-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific

to the use of this truss component.

Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 7 and 33 lb uplift at

joint 2. 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum

sheatning be applied directly to the bottom chord.
a) This truss design requires that a minimum of 7/16" structural wood sheatning be applied directly to the top chord and 1/2" gypsum sheatnock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system, Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouckling of individual truss web and/or chord members only. Additional temporary and permanent bracing tabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANXTPT1 Quality Criteria, DSB-89 and BCSI Building Component</u> Safety information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 7 and 32 lb uplift at joint 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIP.7473 rev, 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see **ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

6904 Parke East Bivd, Tampa, FL 36610



- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 15-0-0, Exterior(2R) 15-0-0 to 19-6-0, Interior(1) 19-6-0 to 23-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 7 and 33 lb uplift at joint 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek@ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing Indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



1	6-9-4	
Plate Offsets (X Y)-	[3:0-2-8 0-3-0] [4:0-5-4 0-2-0] [9:0-2-8 0-3-0]	

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2020/TF	2-0-0 1.25 1.25 YES PI2014	CSI. TC BC WB Matrix	0.64 0.67 0.60 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.10 -0.18 0.04		l/defi >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 143 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP	No.2				BRACING-	D	Structur	al wood	sheathing dire	ectly applied, except e	nd verticals.

BOT CHORD

WEBS

Rigid ceiling directly applied.

1 Row at midpt

4-8

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2

REACTIONS. (size) 7=0-3-8, 2=0-3-8 Max Horz 2=207(LC 11) Max Uplift 7=-15(LC 9), 2=-34(LC 12)

Max Grav 7=1099(LC 17), 2=1160(LC 17)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-1822/35, 3-4=-1157/99, 4-5=-724/102, 5-6=-724/102, 6-7=-993/92

BOT CHORD 2-10=-179/1621, 9-10=-181/1614, 8-9=-149/1034

WEBS 3-10=0/310, 3-9=-666/36, 4-9=0/507, 4-8=-451/81, 5-8=-369/89, 6-8=-74/1130

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 13-0-0, Exterior(2R) 13-0-0 to 17-2-15, Interior(1) 17-2-15 to 23-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 15 lb uplift at joint 7 and 34 lb uplift at joint 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022



WRNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL7473 rev, 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rerotion and bracing of truss systems, see **ANSUTPI 10 Juliy Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suile 203 Waldorf, MD 20601



	5-9-4 5-9-4		11-0-0 5-2-12		1	17-6-0	) 		- 1	24-0-0 6-6-0	
Plate Offsets (X,Y)-	[4:0-5-4,0-2-0], [9:0-2-8,0	-3-0]	5-2-12			0-0-0				5-0-0	
LOADING (psf) TCLL 20.0 TCDL 10.0	SPACING- Plate Grip DOL Lumber DOL	2-0-0 1.25 1.25	CSI. TC BC	0.43 0.52	DEFL. Vert(LL) Vert(CT)	in -0.07 -0.17	(loc) 8-9 8-9	l/defl >999 >999	L/d 240 180	PLATES MT20	GRIP 244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr Code FBC2020/TI	YES PI2014	WB Matri	0.33 x-AS	Horz(CT)	0.04	7	n/a	n/a	Weight: 136 lb	FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied.
WEBS	2x4 SP No.2	WEBS	1 Row at midpt 4-8

REACTIONS. (size) 7=0-3-8, 2=0-3-8 Max Horz 2=177(LC 11) Max Uplift 7=-13(LC 9), 2=-35(LC 12) Max Grav 7=951(LC 1), 2=1047(LC 1)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-1717/49, 3-4=-1229/95, 4-5=-858/94, 5-6=-858/94, 6-7=-895/88

BOT CHORD 2-10=-191/1480, 9-10=-191/1480, 8-9=-144/1032

WEBS 3-10=0/252, 3-9=-513/54, 4-9=0/381, 5-8=-442/102, 6-8=-69/1113

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 11-0-0, Exterior(2R) 11-0-0 to 15-2-15, Interior(1) 15-2-15 to 23-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for the term and for the term of term of
- members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific

to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 13 lb uplift at joint 7 and 35 lb uplift at joint 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MITek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permament bracing is atways required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANXITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



	4-9-4	9-0-6	00		16-6-0			1		24-0-0	
10	4-9-4	4-2-1	2		7-6-0					7-6-0	
Plate Offsets (X,Y)	[4:0-5-4,0-2-0], [9:0-2-8,0	-3-0]			1			_			
LOADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC	0.55	Vert(LL)	-0.12	8-9	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.25	BC	0.68	Vert(CT)	-0.26	8-9	>999	180		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.30	Horz(CT)	0.04	7	n/a	n/a		
BCDL 10.0	Code FBC2020/T	PI2014	Matri	x-AS						Weight: 129 lb	FT = 20%

LUNDER-		BRACING			
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheat	hing directly applied, exce	pt end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly a	pplied.	
WEBS	2x4 SP No.2	WEBS	1 Row at midpt	4-8	

REACTIONS. (size) 7=0-3-8, 2=0-3-8 Max Horz 2=147(LC 11) Max Uplift 7=-11(LC 9), 2=-36(LC 12) Max Grav 7=951(LC 1), 2=1047(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1752/44, 3-4=-1387/89, 4-5=-1160/86, 5-6=-1160/86, 6-7=-886/84

2-10=-175/1519, 9-10=-175/1519, 8-9=-138/1194 BOT CHORD

3-9=-374/44, 4-9=0/339, 5-8=-508/115, 6-8=-64/1322 WEBS

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II: Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 9-0-0, Exterior(2R) 9-0-0 to 13-2-15, Interior(1) 13-2-15 to 23-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 11 lb uplift at joint 7 and 36 lb uplift at ioint 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a fuss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlings with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses safets, see **ANSTPH Quality Criteria**, DSB-89 and BCSI Building Compos Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

	Truss	Truss Type			Qty	Ply	Zieger			T27168016
GER	B12GIR	Half Hip Gire	der		1	2	Joh Data	nce (optional)		12/100010
layo Truss Company, Ir	nc., Mayo, FL - 32066,					8.430 s Aug	16 2021 Mi	Tek Industries,		1:10:09 2022 Page 1
-1-6-0	7-0-0		12-8-9	ID:	:jEMSs5LWG	18-3-7		bJdiqOBvKNC\	24-0-0	ZxIEXkVKq8gpza2SS
1-6-0	7-0-0		5-8-9		1	5-6-13			5-8-9	- HO
										Scale = 1:42
		Special 4x8 =			NAILED			NAILED		NAILED
			NAILED I	NAILED	1.5x4	NAILED	NAILED	6x8 =	NAILED	3x4
		3	14 D D	15	4 16 17	18	19 20	5 21	22	23 6
1	6.00 12		$\sim$	110	19 0.0	0.0			0.0	<u> </u>
	//						//			
			~		11	//				
2										
143		181	nn	nn	Prin	0.0	ແມ	1 <u>8</u> 1 1.11	0.0	
		10	24	25	9 26	27	28	8 29	30	31
3x4 =		1.5x4	NAILED	NAILED	6x8 =	NAILED	NAILED	1.5x4	NAILED	3x4 =
		Special			NAILED			NAILED		NAILED
<u> </u>	7-0-0		12-8-9		1	18-3-7		1	24-0-0	
ate Offsets (X,Y)-	7-0-0 [3:0-5-4,0-2-0]		5-8-9			5-6-13	K		5-8-9	
DADING (psf)	SPACING-	2-0-0	CSI.	DE	EFL.	in (loc)	l/defl	L/d	PLATES	GRIP
LL 20.0	Plate Grip DOL	1.25	TC 0.58	Ve	ert(LL) -C	0.08 9-10	>999	240	MT20	244/190
	Lumber DOL	1.25	BC 0.56	Ve	ert(CT) -0	0.19 9-10	>999	100		
								180 n/a		
CDL 10.0 CLL 0.0 * CDL 10.0	Rep Stress Incr Code FBC2020/TPI	NO	WB 0.49 Matrix-MS			0.06 7		n/a	Weight: 245 I	b FT = 20%
CLL 0.0 * CDL 10.0	Rep Stress Incr	NO	WB 0.49	Ho				1.5-525	Weight: 245 I	b FT = 20%
CLL 0.0 * CDL 10.0 JMBER- DP CHORD 2x4 SP	Rep Stress Incr Code FBC2020/TPI	NO	WB 0.49	BR	orz(CT) (	0.06 7 Structura	n/a al wood she	n/a	Weight: 245 I	
CLL 0.0 * CDL 10.0 JMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP	Rep Stress Incr Code FBC2020/TPI: No.2 No.2	NO	WB 0.49	Ho BR. TO	ACING-	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.		
CLL 0.0 * CDL 10.0 DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2	NO	WB 0.49	Ho BR. TO	ACING-	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max He	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 ) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7)	NO 2014	WB 0.49	Ho BR. TO	ACING-	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Uj	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 e) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8	NO 2014 3)	WB 0.49	Ho BR. TO	ACING-	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Uj Max Gi	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919	NO 2014 3) (LC 1)	WB 0.49 Matrix-MS	BR. TOI BO	ACING-	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP TC CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max U Max G DRCES. (lb) - Max.	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force	NO 2014 3) (LC 1) bs 250 (lb) or less	WB 0.49 Matrix-MS	BR. TOI BO	ACING-	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Uj Max GO ORCES. (lb) - Max. DP CHORD 2-3=- DT CHORD 2-10=	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 e) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919 Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 60/3104, 9-10=0/3129, 8-9=	NO 2014 3) (LC 1) ss 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25	WB 0.49 Matrix-MS except when show 94/92 85	BR, TOI BO'	ACING- P CHORD	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max U Max G DRCES. (lb) - Max. DP CHORD 2-3=- DT CHORD 2-10= EBS 3-10=	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 e) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919 Comp./Max. Ten, - All force 3561/0, 3-4=-3682/34, 4-5=	NO 2014 3) (LC 1) ss 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25	WB 0.49 Matrix-MS except when show 94/92 85	BR, TOI BO'	ACING- P CHORD	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 JMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max G DRCES. (lb) - Max. DP CHORD 2-3=- DT CHORD 2-10= EBS 3-10= DTES-	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 0/730, 3-9=-112/734, 4-9=-	NO 2014 3) (LC 1) 95 250 (Ib) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25 -829/187, 5-9=-37	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544,	BR, TOI BO'	ACING- P CHORD	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max H: Max U; Max GO DRCES. (lb) - Max. DP CHORD 2-10= EBS 3-10= DT CHORD 2-10= EBS 3-10= DT CHORD 2-10= CHORD 2	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919 Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row al	NO 2014 3) (LC 1) rs 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25 -829/187, 5-9=-37 0/2585, 7-8=0/25 -829/187, 5-9=-37	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544,	BR, TOI BO'	ACING- P CHORD	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s.	applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max U Max G DRCES. (lb) - Max. DP CHORD 2-3=- DT CHORD 2-10= EBS 3-10= DT CHORD 3-1	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 e) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten, - All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0	NO 2014 3) (LC 1) xs 250 (lb) or less 3882/34, 6-7=-3 0/2585, 7-8=0/25 829/187, 5-9=-37 1.131"x3") nails at 10-9-0 oc. w at 0-9-0 oc.	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544,	BR, TOI BO'	ACING- P CHORD	0.06 7 Structura except e	n/a al wood she	n/a eathing directly s. y applied or 10	e applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- IP CHORD 2x4 SP CHORD 2x4 SP CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Uj Max G PCCES. (lb) - Max. ) PCHORD 2-3=-, DT CHORD 2-10= EBS 3-10= DTES- 2-ply truss to be cont Top chords connected as All loads are conside	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 prz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919 Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row at 0-9-0 red equally applied to all pl	NO 2014 3) (LC 1) ss 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25 -829/187, 5-9=-37 0.131"x3") nails as t 0-9-0 oc. w at 0-9-0 oc. 0 oc. lies, except if note	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba	Hc BR TOI BO //n. 5-7=-2979/0 ack (B) face	ACING- P CHORD T CHORD	0.06 7 Structura except e Rigid cei	n/a al wood she nd verticals iling directly	n/a eathing directly s. y applied or 10	e applied or 5-10-	
CLL 0.0 * CDL 10.0 MBER- OP CHORD 2x4 SP TCHORD 2x4 SP CHORD 2x4 SP CHORD 2x4 SP CACTIONS. (size Max Hi Max Uj Max GO CRCES. (lb) - Max. OP CHORD 2-10= CHORD 2-1	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 90/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row at ceted as follows: 2x4 - 1 row at follows: 2x4 - 1 row at 0-9-0	NO 2014 3) (LC 1) ss 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25 -829/187, 5-9=-37 0.131*x3") nails at t 0-9-0 oc. w at 0-9-0 oc. o oc. iies, except if notte e only loads note	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle	Hc BR TOI BO //n. 5-7=-2979/0 ack (B) face	ACING- P CHORD T CHORD	0.06 7 Structura except e Rigid cei	n/a al wood she nd verticals iling directly	n/a eathing directly s. y applied or 10	e applied or 5-10-	
LL 0.0 * DL 10.0 MBER- PCHORD 2x4 SP DT CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max G MAX. U Max G PCHORD 2-3=-: DT CHORD 2-10= EBS 3-10= DTES- 2-ply truss to be con TO CHORD 2-10= EBS 3-10= DTES- 2-ply truss to be conside ply connections have Unbalanced roe live Wind: ASCE 7-16; V	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 90/730, 3-9=-112/734, 4-9=- nected together with 10d (0 de as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at follows: 2x4 - 1 row at 0-9-0 red equally applied to all pl e been provided to distribute loads have been considere ult=130mph (3-second gus)	NO 2014 3) (LC 1) bs 250 (lb) or less -3682/34, 6-7=-3 0/2585, 7-8=0/25 829/187, 5-9=-37 1.131"x3") nails at to-9-0 oc. w at 0-9-0 oc. b oc. ies, except if notk e only loads note e of for this design. t) Vasd=101mph;	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI	/n. 5-7=-2979/( ack (B) face css otherwis DL=6.0psf; t	orz(CŤ) ( ACING- P CHORD T CHORD T CHORD 0 0 in the LOAI se indicated. h=15ft; B=43	0.06 7 Structura except e Rigid cei	n/a al wood she ind verticals iling directly ection. Ply ave=4ft; Ca	n/a eathing directly s. y applied or 10	e applied or 5-10-	S LEE NSE
CLL 0.0 * CDL 10.0 IMBER- IP CHORD 2x4 SP CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max G PCCES. (lb) - Max. IP CHORD 2-3=- CHORD 2-10= EBS 3-10= DTES- 2-ply truss to be cont Top chords contected as: All loads are conside ply connections have Unbalanced roof live Wind: ASCE 7-16; V II; Exp B; Encl., GCp	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row follows: 2x4 - 1 row	NO 2014 3) (LC 1) bs 250 (lb) or less -3682/34, 6-7=-3 0/2585, 7-8=0/25 829/187, 5-9=-37 1.131"x3") nails at to-9-0 oc. w at 0-9-0 oc. b oc. ies, except if notk e only loads note e of for this design. t) Vasd=101mph;	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI	/n. 5-7=-2979/( ack (B) face css otherwis DL=6.0psf; t	orz(CŤ) ( ACING- P CHORD T CHORD T CHORD 0 0 in the LOAI se indicated. h=15ft; B=43	0.06 7 Structura except e Rigid cei	n/a al wood she ind verticals iling directly ection. Ply ave=4ft; Ca	n/a eathing directly s. y applied or 10	e applied or 5-10-	
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP TCHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Uj Max Gi DRCES. (lb) - Max. PCHORD 2-3=- DT CHORD 2-10= EBS 3-10= DT CHORD 2-10= DT CHORD 2-	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919 Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at 0-9-0 red equally applied to all pl a been provided to distribute loads have been considered utl=130mph (3-second gusl i=0.18; MWFRS (directiona DOL=1.60 Project engineer responsible	NO 2014 3) (LC 1) ss 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25 -829/187, 5-9=-37 0.131*x3") nails at t 0-9-0 oc. w at 0-9-0 oc. 0 oc. iies, except if notte e only loads note ed for this design. t) Vasd=101mph; ii); cantilever left i	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ;	Ho BR TOI BO m. 5-7=-2979/0 ack (B) face ss otherwis DL=6.0psf; h end vertica	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD 0 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1	0.06 7 Structura except e Rigid cei D CASE(S) s 5ft; L=24ft; ea ht exposed; 1	n/a al wood she nd verticals iling directly ection. Ply ave=4ft; Ca Lumber	n/a eathing directly s. y applied or 10	e applied or 5-10-	S LEE NSE
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP TC CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max U Max G DRCES. (lb) - Max. DP CHORD 2-3=- DT CHORD 2-10= EBS 3-10= DTES- 2-ply truss to be com Top chords connected BS 3-10= DTES- 2-ply truss to be com Webs connected as All loads are conside ply connections have Unbalanced roof live Wind: ASCE 7-16; V II; Exp B; Encl., GCp DOL=1.60 plate grip Building Designer / F to the use of this trus	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919 Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at 0-9-0 red equally applied to all pl a been provided to distribute loads have been considered utl=130mph (3-second gusl i=0.18; MWFRS (directiona DOL=1.60 Project engineer responsible	NO 2014 3) (LC 1) es 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25 -829/187, 5-9=-37 0.131"x3") nails at 10-9-0 oc. w at 0-9-0 oc. 0 oc. lies, except if notte e only loads note ed for this design. t) Vasd=101mph; a); cantilever left e for verifying app	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ;	Ho BR TOI BO m. 5-7=-2979/0 ack (B) face ss otherwis DL=6.0psf; h end vertica	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD 0 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1	0.06 7 Structura except e Rigid cei D CASE(S) s 5ft; L=24ft; ea ht exposed; 1	n/a al wood she nd verticals iling directly ection. Ply ave=4ft; Ca Lumber	n/a eathing directly s, y applied or 10	e applied or 5-10-	S LEE NSE
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Up Max GO DRCES. (lb) - Max. (size Max Hi Max Up Max GO DRCES. (lb) - Max. (size Max Hi Max Up Max GO DRCES. (lb) - Max. (size Max Hi Max GO DRCES. (lb) - Max. (size Size GO CRCES. (lb) - Max. (size GO	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/730, 3-9=-112/734, 4-9=- to/730, 3-9=-112/734, 4-9=- to/7	NO 2014 3) (LC 1) ss 250 (lb) or less 	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ; lied roof live load s d nonconcurrent w	Ho BR TOI BO M. 5-7=-2979/0 ack (B) face ess otherwis DL=6.0psf; h end vertica shown cover ith any othe	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD T CHORD 0 0 0 0 0 0 0 1 15ft; B=4 al left and rig rs rain loadir er live loads.	0.06 7 Structura except e Rigid cei D CASE(S) so 5ft; L=24ft; ea ht exposed; i ng requireme	n/a al wood she nd verticals ling directly ection. Ply ave=4ft; Ca Lumber nts specific	n/a eathing directly s, y applied or 10 t.	e applied or 5-10-	S LEE NSE
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Uj Max Gi DRCES. (lb) - Max. P CHORD 2-3=-; DT CHORD 2-10= EBS 3-10= DT CHORD 2-10= EBS 3-10= DT CHORD 2-3=-; DT CHORD 2-10= EBS 3-10= DT CHORD 2-10= EBS 3-10= DT CHORD 2-10= EBS 3-10= DT CHORD 2-3=-; DT CHORD	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 90/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at ofcllows: 2x4 - 1 row at 0-9-0 red equally applied to all pl b been provided to distribute loads have been considered ult=130mph (3-second gust i=0.18; MWFRS (directiona DOL=1.60 Project engineer responsible is component. ainage to prevent water por	NO 2014 3) (LC 1) ss 250 (lb) or less s-3682/34, 6-7=-3 0/2585, 7-8=0/25 .829/187, 5-9=-37 0.131"x3") nails as t 0-9-0 oc. w at 0-9-0 oc. b oc. iies, except if not e only loads note ad for this design. 1) Vasd=101mph; ii); cantilever left is e for verifying app nding. tom chord live loa '20.0psf on the b	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ; lied roof live load s d nonconcurrent w	Ho BR TOI BO M. 5-7=-2979/0 ack (B) face ess otherwis DL=6.0psf; h end vertica shown cover ith any othe	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD T CHORD 0 0 0 0 0 0 0 1 15ft; B=4 al left and rig rs rain loadir er live loads.	0.06 7 Structura except e Rigid cei D CASE(S) so 5ft; L=24ft; ea ht exposed; i ng requireme	n/a al wood she nd verticals ling directly ection. Ply ave=4ft; Ca Lumber nts specific	n/a eathing directly s, y applied or 10 t.	e applied or 5-10-	S LEE NSE
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP TC CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max G DRCES. (lb) - Max. P CHORD 2-3=- DT CHORD 2-10= EBS 3-10= DTES- 2-ply truss to be con Top chords connected BS 3-10= DTES- 2-ply truss to be conside ply connections have Unbalanced roof live Wind: ASCE 7-16; V II; Exp B; Encl., GCp DOL=1.60 plate grip Building Designer / F to the use of this trus Provide adequate dr. This truss has been * This truss has been	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919 Comp./Max. Ten, - All force 3561/0, 3-4=-3682/34, 4-5= 0/3104, 9-10=0/3129, 8-9= 0/730, 3-9=-112/734, 4-9=- Nor30, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row at accted as follows: 2x4 - 1 row at been provided to distribute loads have been considered ut=130mph (3-second gus) i=0.18; MWFRS (directiona DOL=1.60 Project engineer responsible is component. ainage to prevent water por designed for a 10.0 psf bott in designed for a live load of	NO 2014 3) (LC 1) ss 250 (lb) or less 3682/34, 6-7=-3 0/2585, 7-8=0/25 -829/187, 5-9=-37 0.131"x3") nails at to-9-0 oc. w at 0-9-0 oc. 0 oc. lies, except if notte e only loads note ed for this design. t) Vasd=101mph; a); cantilever left e for verifying app nding. tom chord live load '20.0psf on the b members.	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, a follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ; lied roof live load s d nonconcurrent wi ottom chord in all a	Ho BR TOI BO no. 5-7=-2979/0 ack (B) face sss otherwis DL=6.0psf; H end vertica shown cover ith any othe reas where	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD T CHORD 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1	0.06 7 Structura except e Rigid cei D CASE(S) s 5ft; L=24ft; ea ht exposed; i ng requireme 3-6-0 tall by	n/a al wood she ind verticals iling directly ection. Ply ave=4ft; Ca Lumber ints specific 2-0-0 wide	n/a eathing directly s, y applied or 10 t.	e applied or 5-10-	S LEE NSE
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hi Max Up Max GO DRCES. (lb) - Max. DP CHORD 2-10= EBS 3-10= DT CHORD 2-10= EBS 3-10= DTES- 2-ply truss to be conn Top chords connecte Bottom chords connected Bottom chords connected as All loads are conside ply connections have Unbalanced roof live Wind: ASCE 7-16; VI II; Exp B; Encl., GCp DOL=1.60 plate grip Building Designer / F to the use of his trus Provide adequate dr This truss has been * This truss has been * This truss has been	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= :0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ad as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at ofollows: 2x4 - 1 row at 0-9- follows: 2x4 - 1 row at 0-9- follows: 2x4 - 1 row at 0-9- cred equally applied to all pl a been provided to distribute loads have been considere ult=130mph (3-second gust i=0.18; MWFRS (directiona DOL=1.60 roject engineer responsible ss component. ainage to prevent water por designed for a 10.0 psf bott n designed for a live load of ottom chord and any other of	NO 2014 3) (LC 1) ss 250 (lb) or less 	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, s follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ; lied roof live load s d nonconcurrent w ottom chord in all a ste capable of withs	Ho BR. TOI BO M. 5-7=-2979/0 ack (B) face ess otherwis DL=6.0psf; h end vertica shown cover ith any othe reas where standing 11 l	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD T CHORD 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1	0.06 7 Structura except e Rigid cei D CASE(S) s 5ft; L=24ft; ea ht exposed; i ng requireme 3-6-0 tall by	n/a al wood she ind verticals iling directly ection. Ply ave=4ft; Ca Lumber ints specific 2-0-0 wide	n/a eathing directly s, y applied or 10 t.	e applied or 5-10-	S LEE NSE
CLL 0.0 * CDL 10.0 CDL 10.0 CDL 10.0 CDL 10.0 CDL 10.0 CDL 2x4 SP CDC CHORD 2	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 356170, 3-4=-3682/34, 4-5= :0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 ed as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at ofollows: 2x4 - 1 row at 0-9- follows: 2x4 - 1 row at 0-9- follows: 2x4 - 1 row at 0-9- follows: 2x4 - 1 row at 0-9- fored equally applied to all pl a been provided to distribute loads have been considere ult=130mph (3-second gust i=0.18; MWFRS (directiona DOL=1.60 Project engineer responsible ss component. ainage to prevent water por designed for a 10.0 ps fott to designed for a live load of ottom chord and any other is connection (by others) of tru -3-10d (0.148"x3") or 3-12d connection device(s) shall b	NO 2014 3) (LC 1) s 250 (lb) or less -3682/34, 6-7=-3 0/2585, 7-8=0/25 8-29/187, 5-9=-37 0.131"x3") nails at t 0-9-0 oc. w at 0-9-0 oc. 0 oc. 10 c. 10 c	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, a follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ; lied roof live load s d nonconcurrent w ottom chord in all a the capable of withs pe-nails per NDS gui ent to support com-	Ho BR TOI BO NO S-7=-2979/0 ack (B) face ess otherwis DL=6.0psf; H end vertica shown cover ith any other reas where standing 11 I uidlines. centrated lo	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD T CHORD 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1	0.06 7 Structura except e Rigid cei D CASE(S) so 5ft; L=24ft; ea ht exposed; i ng requireme 3-6-0 tall by int 7 and 3 lt o down and 1	n/a al wood she ind verticals iling directly ection. Ply ave=4ft; Ca Lumber ints specific 2-0-0 wide o uplift at 36 lb up at	n/a eathing directly s, y applied or 10 t.	applied or 5-10- -0-0 oc bracing.	10 oc purlins, S LEE N S C 4869
CLL 0.0 * CDL 10.0 IMBER- DP CHORD 2x4 SP DT CHORD 2x4 SP EBS 2x4 SP EACTIONS. (size Max Hu Max U Max G DRCES. (lb) - Max. DRCES. (lb)	Rep Stress Incr Code FBC2020/TPI: No.2 No.2 No.2 P) 7=0-3-8, 2=0-3-8 orz 2=117(LC 7) plift 7=-11(LC 5), 2=-3(LC 8 rav 7=2132(LC 1), 2=1919) Comp./Max. Ten All force 3561/0, 3-4=-3682/34, 4-5= 0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- 0/730, 3-9=-112/734, 4-9=- nected together with 10d (0 de as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at ected as follows: 2x4 - 1 row at of as follows: 2x4 - 1 row at 0-9-0 red equally applied to all pi e been provided to distribute loads have been considere ult=130mph (3-second gus) i=0.18; MWFRS (directiona DOL=1.60 Project engineer responsible is component. ainage to prevent water por designed for a 10.0 psf bott n designed for a live load of ottom chord and any other 1 connection (by others) of tru -3-10d (0.148*x3") or 3-12d connection device(s) shall t and 358 lb down at 7-0-0	NO 2014 3) (LC 1) s 250 (lb) or less -3682/34, 6-7=-3 0/2585, 7-8=0/25 8-29/187, 5-9=-37 0.131"x3") nails at t 0-9-0 oc. w at 0-9-0 oc. 0 oc. 10 c. 10 c	WB 0.49 Matrix-MS except when show 94/92 85 7/1301, 5-8=0/544, a follows: ed as front (F) or ba d as (F) or (B), unle TCDL=6.0psf; BCI and right exposed ; lied roof live load s d nonconcurrent w ottom chord in all a the capable of withs pe-nails per NDS gui ent to support com-	Ho BR TOI BO NO S-7=-2979/0 ack (B) face ess otherwis DL=6.0psf; H end vertica shown cover ith any other reas where standing 11 I uidlines. centrated lo	orz(CŤ) ( CACING- P CHORD T CHORD T CHORD T CHORD 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1	0.06 7 Structura except e Rigid cei D CASE(S) so 5ft; L=24ft; ea ht exposed; i ng requireme 3-6-0 tall by int 7 and 3 lt o down and 1	n/a al wood she ind verticals iling directly ection. Ply ave=4ft; Ca Lumber ints specific 2-0-0 wide o uplift at 36 lb up at	n/a eathing directly s, y applied or 10 t.	applied or 5-10- -0-0 oc bracing.	10 oc purlins, S LEE N S C 4869

### Continued on page 2

+

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 5/19/2020 BEFORE USE, Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Zieger	T27168016
ZIEGER	B12GIR	Half Hip Girder	1	2	Job Reference (optional)	121100010
Mayo Truss Compa	ny, Inc., Mayo, FL - 320	66,		.430 s Aug	16 2021 MiTek Industries, Inc. Thu Mar 17 1 op2zce56-l8bJdigOBvKNCVZJaOOJdFw_VZ	

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-6=-60, 7-11=-20

Concentrated Loads (lb)

Vert: 3=-184(F) 10=-358(F) 14=-125(F) 15=-125(F) 16=-125(F) 18=-125(F) 20=-125(F) 21=-125(F) 22=-125(F) 23=-132(F) 24=-62(F) 25=-62(F) 26=-62(F) 27=-62(F) 28=-62(F) 29=-62(F) 30=-62(F) 31=-64(F)

WARNING - Verify deegn parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss work and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ouclasse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses safe truss systems, see **ANXTPT duality criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=27ft; eave=4ft II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 13-6-0, Exterior(2R) 13-6-0 to 16-6-0, Interior(1) 16-6-0 to 28-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 1.5x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13, 10, 9, 8, 6, 6.

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors, This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, rection and bracing of trusses safet muss systems, see <u>ANX/TPH Quality Criteria</u>, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.
 Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not
 a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
 building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
 is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
 fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see
 ANSUTPTI Quality Criteria, DSB-89 and BCSI Building Component
 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component

6904 Parke East Blvd. Tampa, FL 36610



ters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTeK® connectors. This design is based only upon parameters and with PERCECE PAGE MILLARY RW. STREADED BEFORE USE. Design valid for use only with MiTeK® connectors. This design is based only upon parameters and properly incorporate this design into the overall a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSI/TPH1 Quality Criteria, DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Zieger	2000000000
ZIEGER	D1GIR	Half Hip Girder	1		1 Color	T27168020
				2	Job Reference (optional)	
Mayo Truss Company	, Inc., Mayo, FL - 320	66.	8	.430 s Aug	16 2021 MiTek Industries, Inc. Thu Mar 17	11:10:48 2022 Page 2

ID:jEMSs5LWG\_wtIMKZIObop2zce56-6eS2p2Is0uFqOV2pN4bTjT1e2HroOv2UTvmiX2za2Rr

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-6=-60, 7-10=-20 Concentrated Loads (lb)

Vert: 4=-184(F) 9=-358(F) 13=-125(F) 14=-125(F) 16=-125(F) 17=-125(F) 19=-127(F) 20=-62(F) 21=-62(F) 22=-62(F) 23=-62(F) 24=-62(F) 25=-63(F)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, defivery, erection and bracing of trusse systems, see **ANSUTPI Quality Criterie, DSB-89 and BCSI Building Comport Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code FBC2020/TPI2014	WB 0.39 Matrix-AS	Horz(CT) 0.	03 8 n/a	n/a	Weight: 128 lb	FT = 20%
BOT CHORD 2x4	SP No.2 SP No.2 SP No.2		BRACING- TOP CHORD BOT CHORD	Structural wood s Rigid ceiling direct		applied, except end	d verticals.
REACTIONS. (s Max Max	ize) 2=0-3-8, 8=Mechanical Horz 2=171(LC 11) Uplift 2=-36(LC 12), 8=-1(LC 12) Gray 2=894(LC 1), 8=797(LC 1)						
FORCES. (Ib) - Ma TOP CHORD 2-3 BOT CHORD 2-1	<ul> <li>x. Comp./Max. Ten All forces 250 (lb) of 3=-1385/55, 3-4=-887/100, 4-5=-546/101, 11=-195/1185, 10-11=-195/1185, 9-10=-1 11=0/254, 3-10=-529/63, 4-10=0/346, 4-9</li> </ul>	5-6=-546/101 39/723	63/736.				

WEBS 529/63, 4-10=0/346, 4-9=-294/66, 5-9=-275/74, 6-9=-63/736, 6-8=-745/171

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 11-0-0, Exterior(2R) 11-0-0 to 15-2-12, Interior(1) 15-2-12 to 19-2-0, Exterior(2E) 19-2-0 to 20-0-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev, 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system, Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, defivery, erection and bracing of trusses systems, see **ANSE/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

MiTek'

6904 Parke East Blvd. Tampa, FL 36610

Date:



WEBS 2-7=0/367, 2-6=-673/89, 4-6=-106/585

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 15-1-0, Exterior(2R) 15-1-0 to 18-1-0, Interior(1) 18-1-0 to 20-0-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek@ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblese with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BRACING-

WEBS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

2-10

Rigid ceiling directly applied.

1 Row at midpt

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.2 WEBS

#### REACTIONS. (size) 1=Mechanical, 9=0-3-8, 6=0-3-8 Max Horz 1=-140(LC 10) Max Uplift 9=-82(LC 12), 6=-129(LC 12) Max Grav 1=721(LC 1), 9=1439(LC 1), 6=388(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten, - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-1055/16, 2-3=-359/78, 3-4=-329/90, 4-5=-27/366

BOT CHORD 1-11=0/875, 10-11=0/871, 9-10=-290/147

2-11=0/366, 2-10=-672/41, 4-10=-34/804, 4-9=-1047/95, 5-9=-487/340 WEBS

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=30ft; eave=4ft; Cat. II: Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 3-0-3, Interior(1) 3-0-3 to 15-1-0, Exterior(2R) 15-1-0 to 18-1-3, Interior(1) 18-1-3 to 31-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 6=129.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21.2022



MARNING - Verify design param eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valor for use only with MTeR® connectors. This design is based only upon parameters abover, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing fabrication, storage, delivery, erection and bracing of truss systems, see **ANSUTH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	Zieger	T27168026
ZIEGER	E1GIR	Hip Girder	1	2	Job Reference (optional)	12/100020
Mayo Truss Company	y, Inc., Mayo, FL - 32	066,	8	.430 s Aug	16 2021 MiTek Industries, Inc. Thu Mar 17 11:	11:15 2022 Page 2

ID:jEMSs5LWG\_wtIMKZIObop2zce56-pqbGTHdofBf?7ec?SFaoJAfNNA2ECBWSgcpe\_za2RQ

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 4=-184(F) 5=-184(F) 9=-62(F) 17=-125(F) 19=-358(F) 20=-358(F)

WARNING - Verify design parameters and REAO NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 rov. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oclapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see <u>ANXTPTI Quality Criteria, DSB-89 and BCSI Building Component</u> **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	H	4-6-4 4-6-4		1	8-6-0 3-11-12			12-5 3-11		1	17-0-0 4-6-4	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.17	Vert(LL)	-0.03	8	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.32	Vert(CT)	-0.07	8-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(CT)	0.03	6	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS	1 A A A A A A A A A A A A A A A A A A A					Weight: 81 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2

REACTIONS. (size) 6=0-3-8, 2=0-3-8 Max Horz 2=80(LC 11) Max Uplift 2=-39(LC 12) Max Grav 6=676(LC 1), 2=774(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1174/160, 3-4=-816/160, 4-5=-816/167, 5-6=-1182/172

### BOT CHORD 2-9=-101/1007, 8-9=-101/1007, 7-8=-102/1028, 6-7=-102/1028

WEBS 4-8=-47/454, 5-8=-414/94, 3-8=-389/85

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 8-6-0, Exterior(2R) 8-6-0 to 11-6-0, Interior(1) 11-6-0 to 8-6-0, Exterior(2R) 8-6-0 to 11-6-0, Interior(1) 11-6-0 to 17-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see <u>ANXTPTI Quality Criteria</u>, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		4-6-4	1	8	-6-0	10-10-0				18	3-3-10	18-4-0
	1	4-6-4		3-1	1-12	2-4-0	1			7	-5-10	0-0-6
Plate Offse	ets (X,Y)- [	[6:Edge,0-1-8]				1	_					
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.52	Vert(LL)	-0.08	7-8	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.50	Vert(CT)	-0.16	7-8	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.03	7	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS	0.028503505.0305					Weight: 99 lb	FT = 20%

TOP CHORD

BOT CHORD

WEBS

LL		

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.2 REACTIONS. (size) 7=0-4-3, 2=0-3-8 Max Horz 2=123(LC 11)

Max Uplift 2=-38(LC 12)

Max Grav 7=724(LC 1), 2=821(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1278/125, 3-4=-917/136, 4-5=-908/149

2-10=-223/1100, 9-10=-223/1100, 8-9=-175/1069, 7-8=-170/1075 BOT CHORD

3-9=-389/81, 4-9=-72/636, 5-9=-510/69, 5-8=0/258, 5-7=-1083/148 WEBS

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II: Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 8-6-0, Exterior(2E) 8-6-0 to 10-10-0, Interior(1) 10-10-0 to 18-2-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

# \* No 34' 4 GIT

Structural wood sheathing directly applied, except end verticals.

5-7

Rigid ceiling directly applied.

1 Row at midpt

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing tabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSI/TPI Quality Criteria</u>, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

MiTek 6904 Parke East Blvd. Tampa, FL 36610



March 21,2022

WARNING - Varify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verity the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANS/TPI7 duality criteria</u>, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

MITEK 6904 Parke East Blvd. Tampa EL 36610
Job	Truss	Truss Type	Qty	Ply	Zieger	T27168030
ZIEGER	E5GIR	Half Hip Girder	1	2	Job Reference (optional)	
Mayo Truss Comp	bany, Inc., Mayo, FL - 320	966,			ug 16 2021 MiTek Industries, Inc. Thu Mar 17 11: IObop2zce56-TQrDZBq5lu66Kaerbb_HLHCsvj	

LOAD CASE(S) Standard 1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-5=-60, 5-8=-60, 2-9=-20

Concentrated Loads (lb)

Vert: 18=-1760(B) 19=-777(B) 20=-777(B) 21=-781(B) 22=-781(B) 23=-781(B) 24=-781(B) 25=-781(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek@ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





						14-0-0	_					
		1				14-0-0						1
Plate Offse	ets (X,Y)	[2:0-4-0.0-2-1], [3:0-4-10,	0-2-0], [7:0-4-1	0,0-2-0], [8:	0-4-0,0-2-1]							
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.14	Vert(LL)	-0.01	9	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.06	Vert(CT)	-0.01	9	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	8	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-S						Weight: 70 lb	FT = 20%

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.2

REACTIONS. All bearings 14-0-0.

(lb) - Max Horz 2=65(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8, 13, 11

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3E) -1-6-0 to 1-6-0, Exterior(2N) 1-6-0 to 7-0-0, Corner(3R) 7-0-0 to 10-0-0, Exterior(2N) 10-0-0 to 15-6-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8, 13, 11.
 Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 8.

No 34869

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see <u>ANSVTPH Quality Criterie</u>, DSB-89 and BCSI Building Compone Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.60	Vert(LL)	-0.09	4-7	>969	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.50	Vert(CT)	-0.21	4-7	>398	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS	1.					Weight: 25 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied.

#### TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=111(LC 12) Max Uplift 3=-44(LC 12), 2=-21(LC 12)

Max Grav 3=185(LC 1), 2=377(LC 1), 4=124(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 6-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 rev, 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





Dista Off						5-0-0	R.				1	
Plate Off	sets (X,Y)- [	2:0-4-4,0-0-4]		1								and a final set
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.28	Vert(LL)	0.03	4-7	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.24	Vert(CT)	-0.05	4-7	>999	180	1000000000	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS	100000000000					Weight: 18 lb	FT = 20%

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-TOP CHORD BOT CHORD

5-0-0

Structural wood sheathing directly applied. Rigid ceiling directly applied.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=87(LC 12)

Max Uplift 3=-29(LC 12), 2=-29(LC 12) Max Grav 3=126(LC 1), 2=301(LC 1), 4=88(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 4-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see <u>ANSUTPH Quality Criteria</u>, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





						5-0-0	_					
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.31	Vert(LL)	-0.03	3-6	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.25	Vert(CT)	-0.06	3-6	>968	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	1	n/a	n/a	AT ANY THE VALUE AND ANY ANY	
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS						Weight: 16 lb	FT = 20%

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=0-3-8, 2=Mechanical, 3=Mechanical

Max Horz 1=59(LC 12) Max Uplift 2=-33(LC 12)

Max Grav 1=198(LC 1), 2=133(LC 1), 3=90(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 4-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Bivd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, eraction and bracing of trusses and truss systems, see <u>ANXTPTI Quality Criteria. DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





					1		3-0-0	_				
OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.14	Vert(LL)	-0.00	4-7	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.07	Vert(CT)	-0.01	4-7	>999	180	1002/000-0004	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MP						Weight: 12 lb	FT = 20%

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=63(LC 12) Max Uplift 3=-12(LC 12), 2=-40(LC 12)

Max Grav 3=65(LC 1), 2=230(LC 1), 4=50(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 2-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 rev, \$/19/2020 BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, defivery, erection and bracing of trusses systems, see **ANSUTPIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 Waren 21,2022





								1-0-0				
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.14	Vert(LL)	0.00	7	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.02	Vert(CT)	0.00	7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	4	n/a	n/a	Story Devenue	
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-MP	No. 60 Million 1997					Weight: 6 lb	FT = 20%

TOP CHORD

BOT CHORD

1-0-0

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical

Max Horz 2=39(LC 12)

Max Uplift 3=-7(LC 1), 2=-71(LC 12), 4=-22(LC 1) Max Grav 3=12(LC 12), 2=198(LC 1), 4=22(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and

right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek@ connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oblapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





#### NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 13-2-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

ters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. A WARNING - Verify design pa Design valid for use only with MTEKE connectors. This statu indicates the state of the state state of the state state of the state of t fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Qu. Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





6904 Parke East Blvd. Tampa, FL 36610



				111	4-5-0			_	4-0-10		0.0.0	
OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.20	Vert(LL)	0.03	6-9	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.22	Vert(CT)	-0.03	6-9	>999	180	1.	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.19	Horz(CT)	-0.01	5	n/a	n/a		
BCDL	10.0	Code FBC2020/TI	PI2014	Matri	x-AS						Weight: 48 lb	FT = 20%

TOP CHORD

BOT CHORD

#### LUMBER-

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

REACTIONS. (size) 2=0-3-8, 5=0-4-3 Max Horz 2=148(LC 11) Max Uplift 2=-114(LC 12), 5=-79(LC 12) Max Grav 2=461(LC 1), 5=357(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-488/365

BOT CHORD 2-3-460/305 BOT CHORD 2-6=-453/395, 5-6=-453/395 WEBS 3-5=-439/453

NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 9-1-3 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=114.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

# No 34869 \* No 34869 \* ORIDACINA

Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITeR® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusse systems, see <u>ANSUFTI Quality Criteria, DSB-89 and BCSI Building Component</u> Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	<b>l/defl</b>	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.60	Vert(LL)	0.25	6-9	>339	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.50	Vert(CT)	-0.21	6-9	>393	180	100-15-25000	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS						Weight: 30 lb	FT = 20%

BOT CHORD

TOP CHORD 2x4 SP No.2 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS

REACTIONS. (size) 6=0-4-3, 2=0-3-8 Max Horz 2=118(LC 11)

Max Uplift 6=-59(LC 12), 2=-97(LC 12)

Max Grav 6=280(LC 1), 2=378(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II: Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 7-2-15 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.

6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.

Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTFeK® connectors. This AND INCLUDED MTFER REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTFeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criterie, DSB-89 and BCSI Building Compor Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





OADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.28	Vert(LL)	-0.02	6-9	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.24	Vert(CT)	-0.05	6-9	>999	180	0.023097777	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-AS						Weight: 23 lb	FT = 20%

BOT CHORD

Rigid ceiling directly applied.

 TOP CHORD
 2x4 SP No.2

 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.2

REACTIONS. (size) 6=0-4-3, 2=0-3-8 Max Horz 2=87(LC 11) Max Uplift 2=-40(LC 12) Max Grav 6=197(LC 1), 2=302(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 5-3-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.
- 6) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Julius Lee PE No.34869 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

March 21,2022

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







