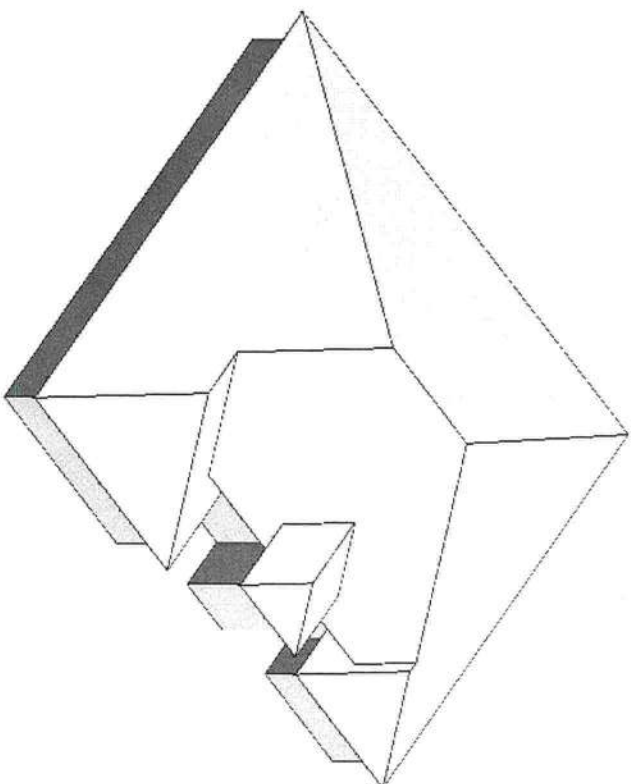
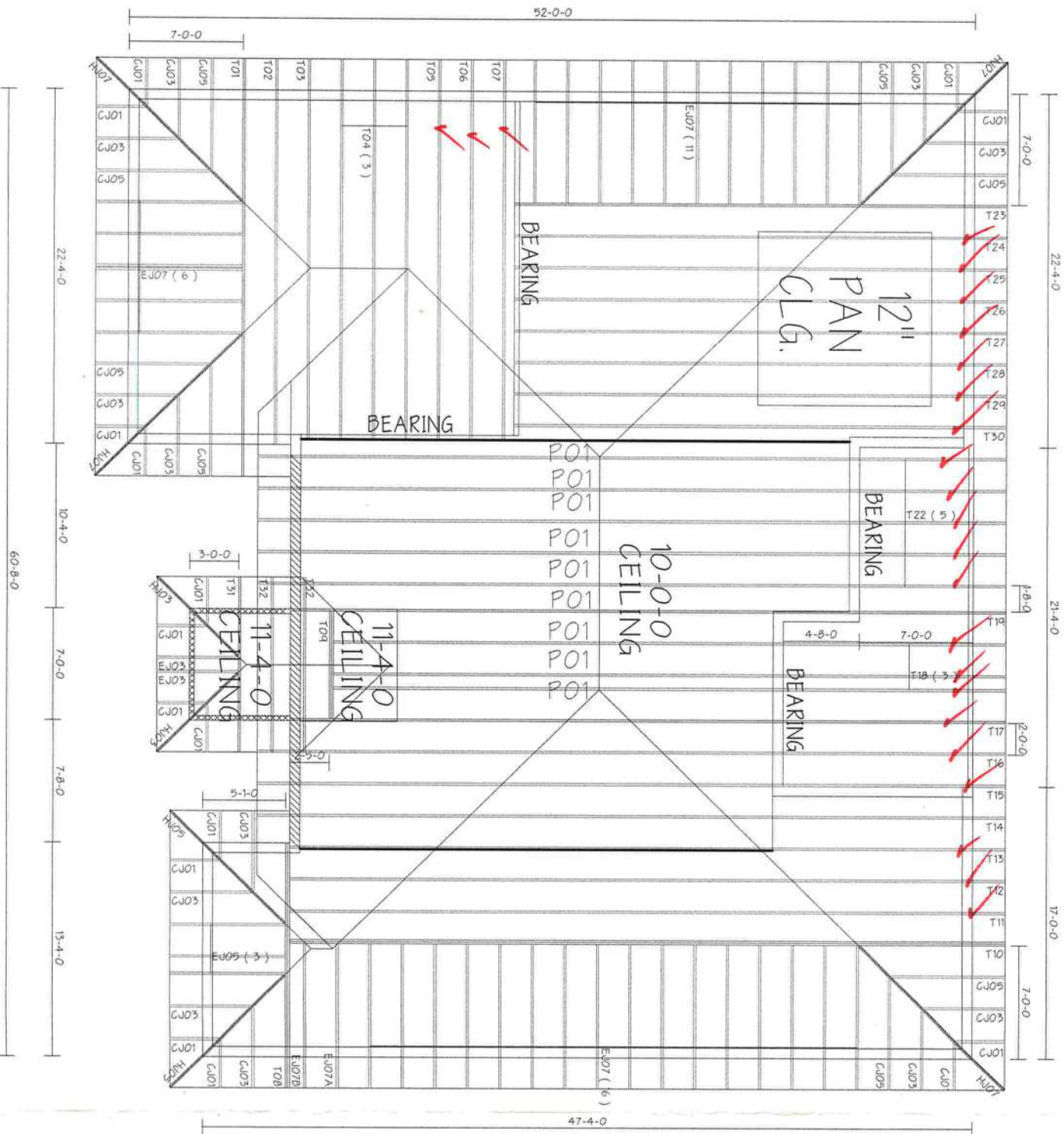


6/12 PITCH
2'-0" OVERHANGS



✓ BRACED TRUSSES (WEBBS)

BEARING HEIGHT SCHEDULE

	8' - 0"
	10' - 0"
	11' - 4"

NOTES:

- 1) REFER TO HB 91 (RECOMMENDATIONS FOR BRACKING INSTALLATION AND TEMPORARY BRACKING) AND HB 92 (ENGINEERED DRAWINGS FOR PERMANENT BRACKING REQUIRED).
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V05 FOR ALTERNATE BRACKING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2% MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) BEAM/HEADER/INTEL (HOK) TO BE FURNISHED BY BUILDER.



PHONE: 904-772-6000 FAX: 904-772-1973
Jacksonville
Bunnell
PHONE: 386-437-3349 FAX: 386-437-3994
Lake City
PHONE: 386-755-6894 FAX: 386-755-7973
Sanford
PHONE: 407-322-0059 FAX: 407-322-5553
Freeport
PHONE: 850-835-4541 FAX: 850-835-6635
Treasure Coast
PHONE: 772-468-8160 FAX: 772-468-8181
Tampa
PHONE: 813-621-9831 FAX: 813-628-9956

BUILDER:

SPARKS CONSTR.

IDEAL OFFICE:

TBD

NOTE:

BARTH RES.

DATE: 1/25/10 EMD 323087

REVISION: 1/25/10 EMD 323087

REVISION: 1/25/10 EMD 323087

REVISION: 1/25/10 EMD 323087

Julius Lee Engineering

RE: 323087 - SPARKS - BARTH RES.

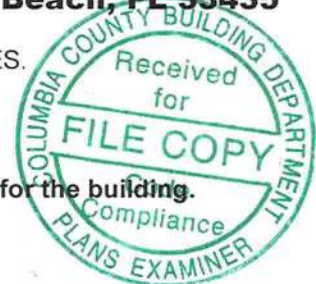
**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: SPARKS CONST. Project Name: 323087 Model: BARTH RES.
Lot/Block: Subdivision:
Address: 100 SW GRASSY LANE
City: COLUMBIA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: JOSHUA D. SPARKS License #: CBC1252260
Address: 130 SW COUNTRY CT
City: LAKE CITY, State: FL



General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 42 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4204310	CJ01	1/25/010	18	I4204327	T06	1/25/010
2	I4204311	CJ03	1/25/010	19	I4204328	T07	1/25/010
3	I4204312	CJ05	1/25/010	20	I4204329	T08	1/25/010
4	I4204313	EJ03	1/25/010	21	I4204330	T09	1/25/010
5	I4204314	EJ05	1/25/010	22	I4204331	T10	1/25/010
6	I4204315	EJ07	1/25/010	23	I4204332	T11	1/25/010
7	I4204316	EJ07A	1/25/010	24	I4204333	T12	1/25/010
8	I4204317	EJ07B	1/25/010	25	I4204334	T13	1/25/010
9	I4204318	HJ03	1/25/010	26	I4204335	T14	1/25/010
10	I4204319	HJ05	1/25/010	27	I4204336	T15	1/25/010
11	I4204320	HJ07	1/25/010	28	I4204337	T16	1/25/010
12	I4204321	P01	1/25/010	29	I4204338	T17	1/25/010
13	I4204322	T01	1/25/010	30	I4204339	T18	1/25/010
14	I4204323	T02	1/25/010	31	I4204340	T19	1/25/010
15	I4204324	T03	1/25/010	32	I4204341	T22	1/25/010
16	I4204325	T04	1/25/010	33	I4204342	T23	1/25/010
17	I4204326	T05	1/25/010	34	I4204343	T24	1/25/010

*SEE REVERSE
THIS SHEET*

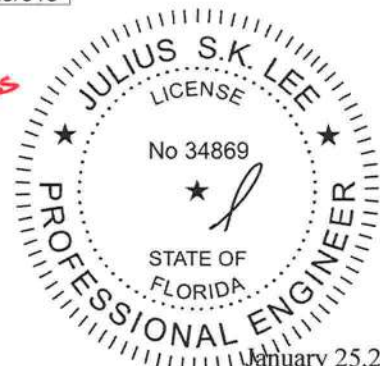
The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

WEB BRACED TRUSS

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2011.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



Julius Lee

Job 323087	Truss CJ01	Truss Type JACK	Qty 16	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204310
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:03 2010 Page 1			

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.00	2	>999	240		
									Weight: 7 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD
BOT CHORD 2 X 4 SYP No.2	BOT CHORD

Structural wood sheathing directly applied or 1-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

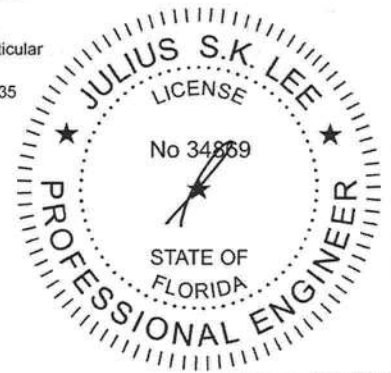
REACTIONS (lb/size) 2=256/0-3-8, 4=5/Mechanical, 3=-90/Mechanical
Max Horz 2=109(LC 6)
Max Uplift 2=-371(LC 6), 4=-12(LC 4), 3=-90(LC 1)
Max Grav 2=256(LC 1), 4=14(LC 2), 3=163(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 371 lb uplift at joint 2, 12 lb uplift at joint 4 and 90 lb uplift at joint 3.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 25, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.

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1109 Coastal Bay Blvd.
Boynton, FL 33435

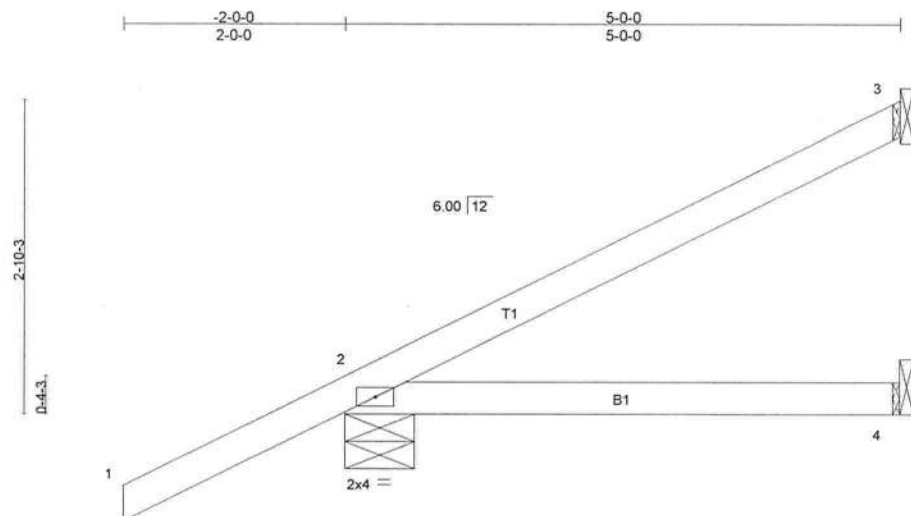
Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.
323087	CJ05	JACK	8	1	

I4204312

Job Reference (optional)

Builders FrstSource, Lake City, FL 32055

7,140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:04 2010 Page 1



Scale = 1:19.7

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.42	Vert(LL) -0.02	2-4	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.15	Vert(TL) -0.04	2-4	>999	240		
BCLL 0.0	Lumber Increase 1.25	WB 0.00	Horz(TL) -0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Wind(LL) 0.00	2	****	240		
	Code FBC2007/TPI2002						Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

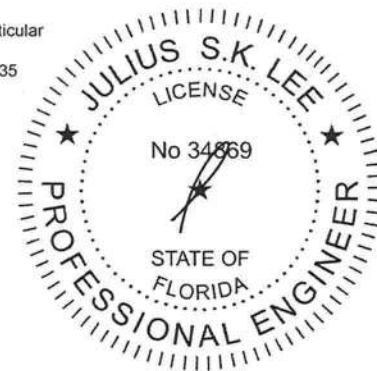
REACTIONS (lb/size) 3=94/Mechanical, 2=304/0-7-8, 4=23/Mechanical
Max Horz 2=224(LC 6)
Max Uplift 3=-104(LC 6), 2=-280(LC 6)
Max Grav 3=94(LC 1), 2=304(LC 1), 4=69(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 3 and 280 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 25, 2010

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MH-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

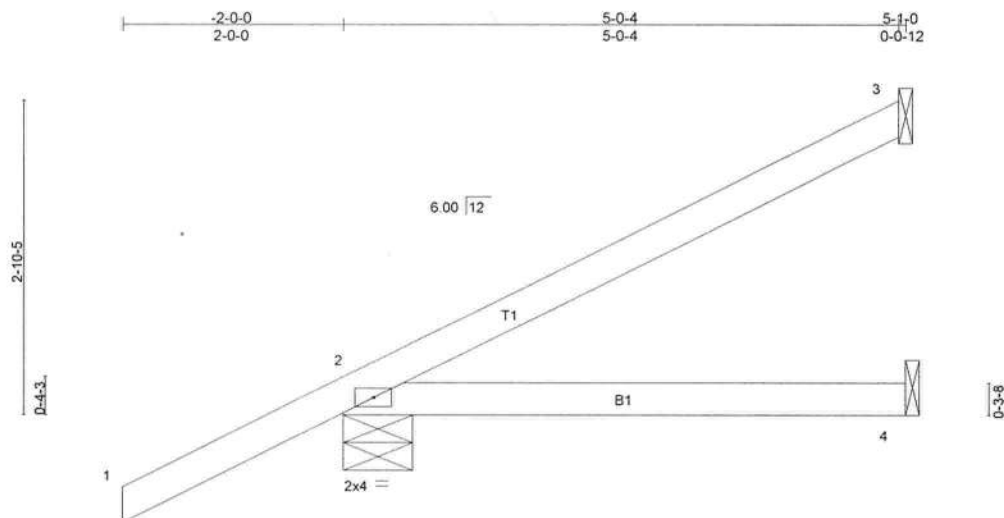
Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.
323087	EJ05	JACK	3	1	

I4204314

Job Reference (optional)

Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:04 2010 Page 1



Scale = 1:19.7

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.42	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.16	Vert(LL) -0.03 2-4 >999 360		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.05 2-4 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.00 2 **** 240		
				Weight: 19 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 5-1-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

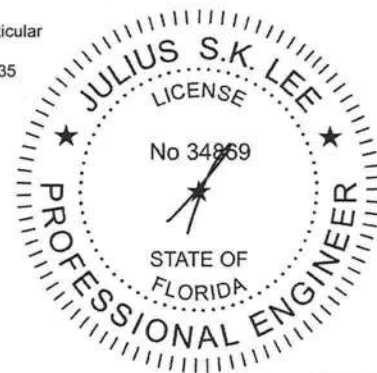
REACTIONS (lb/size) 3=94/Mechanical, 2=305/0-7-8, 4=24/Mechanical
Max Horz 2=224(LC 6)
Max Uplift 3=-105(LC 6), 2=-280(LC 6)
Max Grav 3=94(LC 1), 2=305(LC 1), 4=71(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No.2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 3 and 280 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 25, 2010

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 323087	Truss EJ07A	Truss Type MONO TRUSS	Qty 1	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204316
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:05 2010 Page 1			

Scale = 1:24.4

Plate Offsets (X,Y): [2-0-2-10-0-1-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	-0.08	2-4	>993
TCDL 7.0	Lumber Increase	1.25	BC 0.27	Vert(TL)	-0.15	2-4	>522
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.06	2-4	>999
							Weight: 25 lb
LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2				BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.			
REACTIONS (lb/size) 3=149/Mechanical, 2=357/0-7-8, 4=39/Mechanical Max Horz 2=202(LC 6) Max Uplift 3=-110(LC 6), 2=-203(LC 6) Max Grav 3=149(LC 1), 2=357(LC 1), 4=93(LC 2)				MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.			
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.							
NOTES (8-9) 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SYP No.2. 5) Refer to girder(s) for truss to truss connections. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 3 and 203 lb uplift at joint 2. 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss. 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code. 9) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435							
LOAD CASE(S) Standard							

January 25, 2010



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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 323087	Truss HJ03	Truss Type JACK	Qty 2	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204318
Builders FirstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:06 2010 Page 1			

LOADING (psf) TCCL 20.0 TCCL 7.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2007/TPI2002	CSI TC 0.62 BC 0.09 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.01 2-4 >999 360 Vert(TL) -0.01 2-4 >999 240 Horz(TL) -0.00 3 n/a n/a Wind(LL) 0.02 2-4 >999 240	PLATES MT20 GRIP 244/190 Weight: 18 lb
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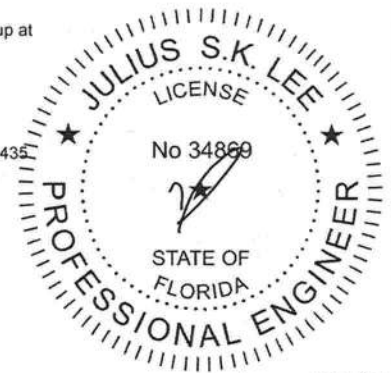
LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 4-2-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
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REACTIONS (lb/size) 3=18/Mechanical, 2=297/0-5-11, 4=16/Mechanical
 Max Horz 2=167(LC 3)
 Max Uplift 3=-57(LC 6), 2=-487(LC 3), 4=-52(LC 3)
 Max Grav 3=18(LC 1), 2=297(LC 1), 4=49(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 3, 487 lb uplift at joint 2 and 52 lb uplift at joint 4.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 38 lb up at 1-5-12, and 38 lb up at 1-5-12 on top chord, and 16 lb up at 1-5-12, and 16 lb up at 1-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 2-4=-10
 Concentrated Loads (lb)
 Vert: 5=76(F=38, B=38) 6=10(F=5, B=5)



January 25, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oro Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 323087	Truss HJ07	Truss Type MONO TRUSS	Qty 4	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	14204320
Builders FirstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:07 2010 Page 1			

Scale: 1/2"=1'

Plate Offsets (X,Y): [2-0-2-4,0-1-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.57	Vert(LL)	-0.04	6-7	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.07	6-7	>999
BCLL 0.0	Rep Stress Incr	NO	WB 0.16	Horz(TL)	0.01	5	n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	-0.02	2-7	>999
							Weight: 44 lb

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
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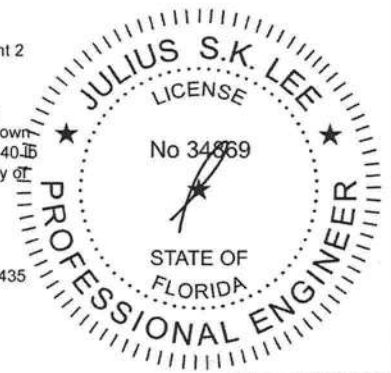
REACTIONS (lb/size) 4=139/Mechanical, 2=411/0-11-6, 5=166/Mechanical
 Max Horz 2=204(LC 3)
 Max Uplift 4=94(LC 3), 2=-348(LC 3), 5=-65(LC 4)
 Max Grav 4=139(LC 1), 2=411(LC 1), 5=219(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-8=-385/138, 8-9=-390/131, 3-9=-374/131
 BOT CHORD 2-11=-199/343, 11-12=-199/343, 7-12=-199/343, 7-13=-199/343, 6-13=-199/343
 WEBS 3-6=-372/216

NOTES (10-11)
 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2.
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 4, 348 lb uplift at joint 2 and 65 lb uplift at joint 5.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 38 lb up at 1-5-12, 38 lb up at 1-5-12, 10 lb down and 42 lb up at 4-3-11, 10 lb down and 42 lb up at 4-3-11, and 37 lb down and 44 lb up at 7-1-10, and 37 lb down and 44 lb up at 7-1-10 on top chord, and 16 lb up at 1-5-12, 16 lb up at 1-5-12, 10 lb down at 4-3-11, 10 lb down at 4-3-11, and 40 lb down at 7-1-10, and 40 lb down at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2



January 25, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 323087	Truss P01	Truss Type PIGGYBACK	Qty 9	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204321
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:07 2010 Page 1			

Scale = 1:7.8

0'-1'-10"

Plate Offsets (X,Y): [3'-0"-2'-0",Edge]							
LOADING (psf)	SPACING	2'-0'-0"	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.12	Vert(LL)	-0.01	2-4	>999
TCDL 7.0	Lumber Increase	1.25	BC 0.06	Vert(TL)	-0.01	2-4	>999
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.01	5	n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.01	2-4	>999
							L/d
							240
							n/a
							240
							Weight: 10 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=114/0-5-8, 5=114/0-5-8

Max Horz 1=-14(LC 4)

Max Uplift 1=-37(LC 6), 5=-37(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 1 and 37 lb uplift at joint 5.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 4'-0"-0" oc purlins.

BOT CHORD Rigid ceiling directly applied or 10'-0"-0" oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



January 25, 2010



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.	I4204322
323087	T01	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:08 2010 Page 2

LOAD CASE(S) Standard

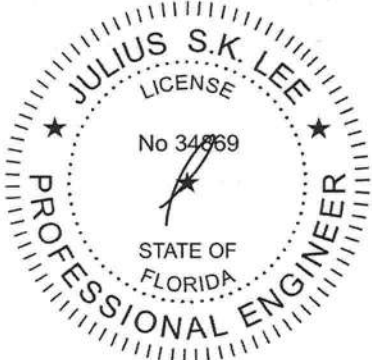
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-54, 5-7=-54, 2-6=-10

Concentrated Loads (lb)

Vert: 3=-95(F) 5=-95(F) 10=-321(F) 9=-57(F) 4=-190(F) 8=-321(F) 11=-95(F) 12=-95(F) 13=-29(F) 14=-29(F)



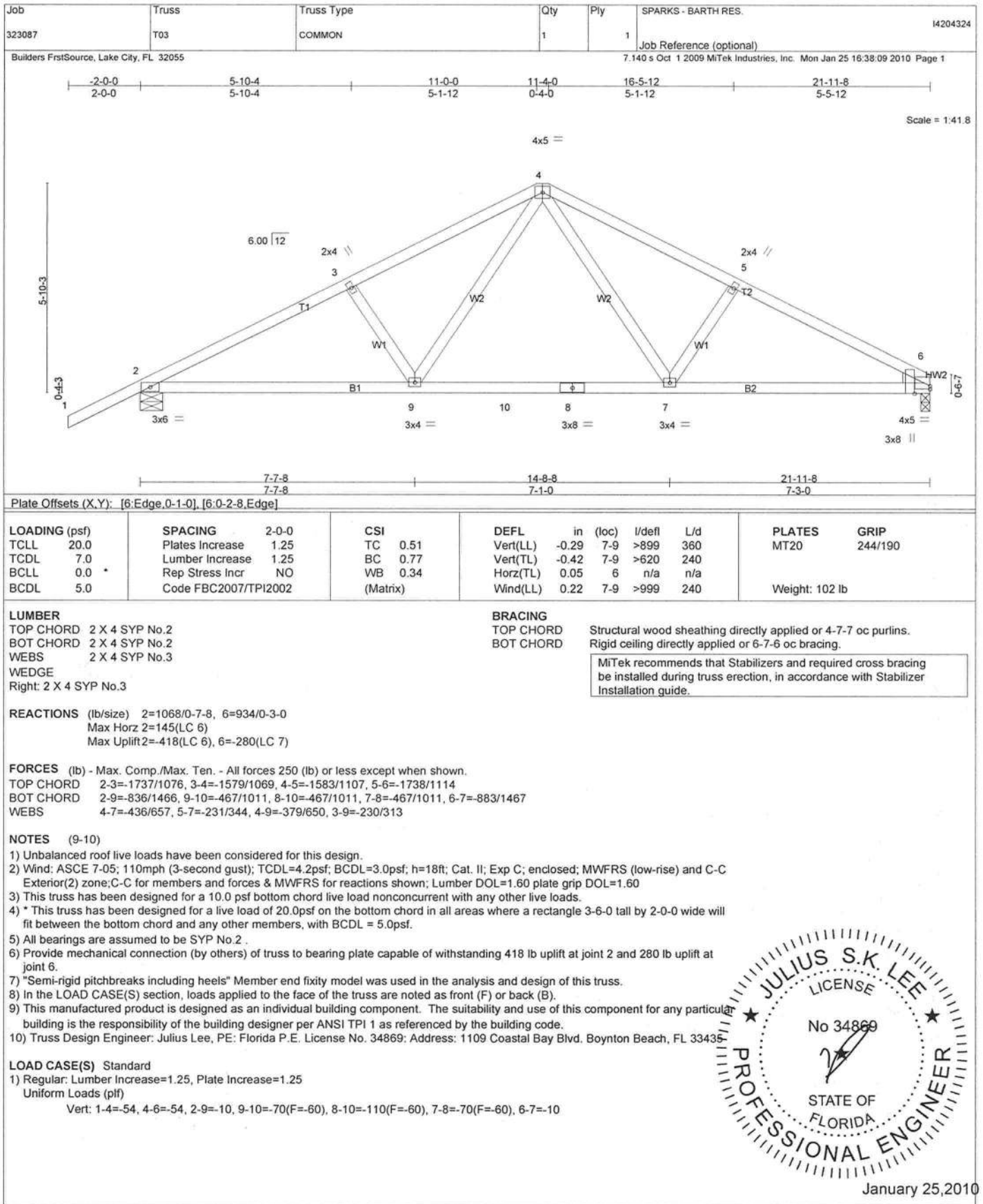
January 25, 2010

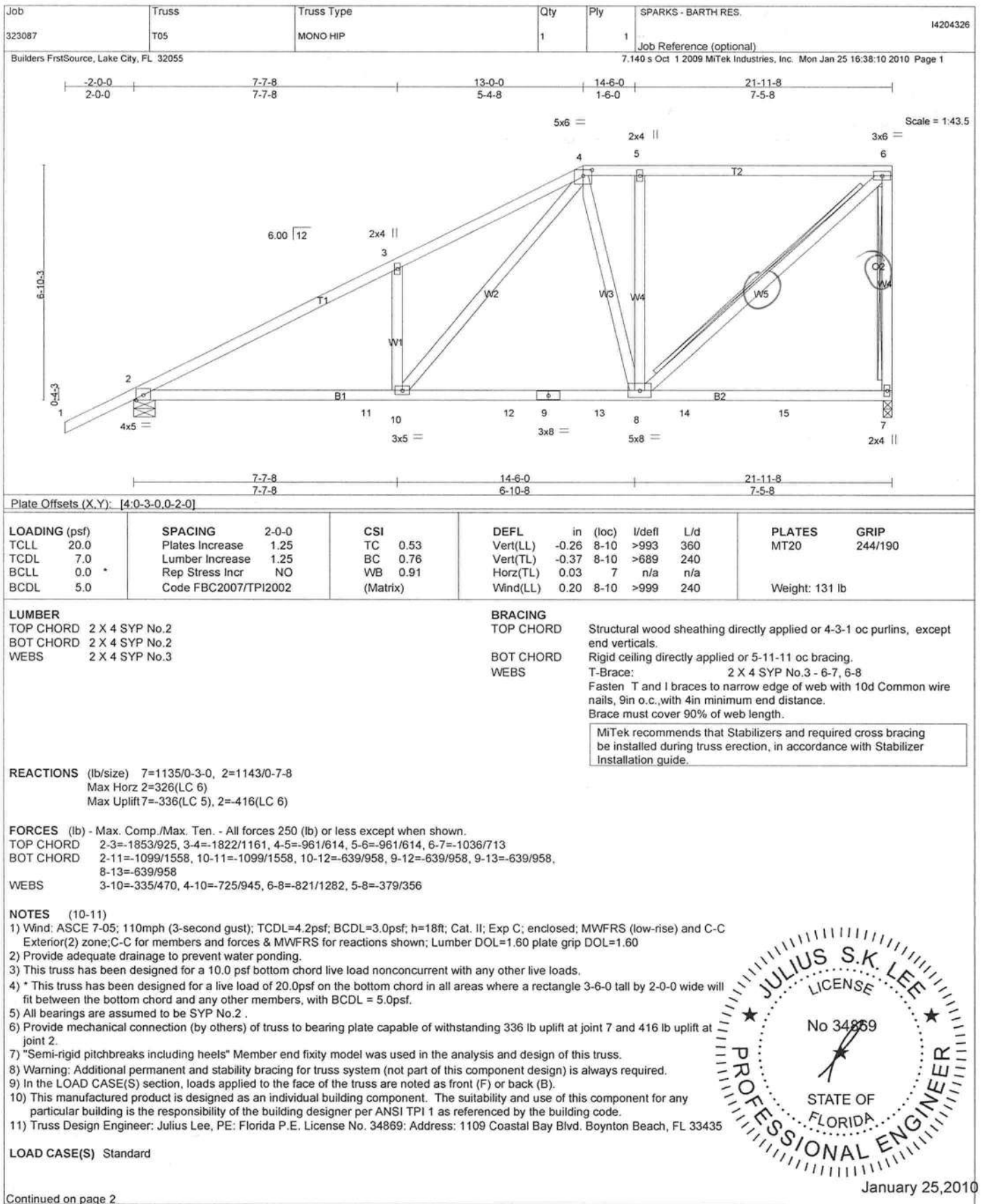


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII 7473 BEFORE USE.

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Boynton, FL 33435





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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 323087	Truss T06	Truss Type MONO HIP	Qty 1	Ply 1	SPARKS - BARTH RES. Job Reference (optional) 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:11 2010 Page 1
Builders FirstSource, Lake City, FL 32055					

Scale: 1/4"=1'

Plate Offsets (X,Y): [2-0-2-10,0-1-8], [4-0-6-0,0-2-8]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.45	Vert(LL)	-0.08 2-9	>999	360
TCDL 7.0	Lumber Increase	1.25	BC 0.35	Vert(TL)	-0.16 2-9	>999	240
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.43	Horz(TL)	0.03 6	n/a	n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.06 9	>999	240
				PLATES		GRIP	
				MT20		244/190	
				Weight: 125 lb			

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 6=781/0-3-0, 2=850/0-7-8

Max Horz 2=366(LC 6)

Max Uplift 6=-260(LC 5), 2=-346(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1181/605, 3-4=-659/339

BOT CHORD 2-9=-884/969, 8-9=-884/969, 7-8=-884/969, 7-10=-400/511, 6-10=-400/511

WEBS 3-7=-528/552, 4-7=-212/433, 4-6=-742/585

NOTES (9-10)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SYP No.2.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 6 and 346 lb uplift at joint 2.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-12 oc purlins, except end verticals.

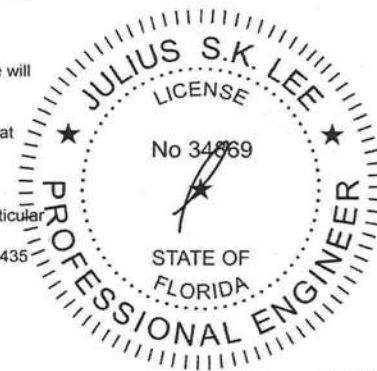
BOT CHORD Rigid ceiling directly applied or 6-8-11 oc bracing.

WEBS T-Brace: 2 X 4 SYP No.3 - 5-6, 3-7, 4-6

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.



January 25, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 323087	Truss T08	Truss Type HIP	Qty 1	Ply 2	SPARKS - BARTH RES. Job Reference (optional)	I4204329
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Builders FirstSource, Lake City, FL 32055

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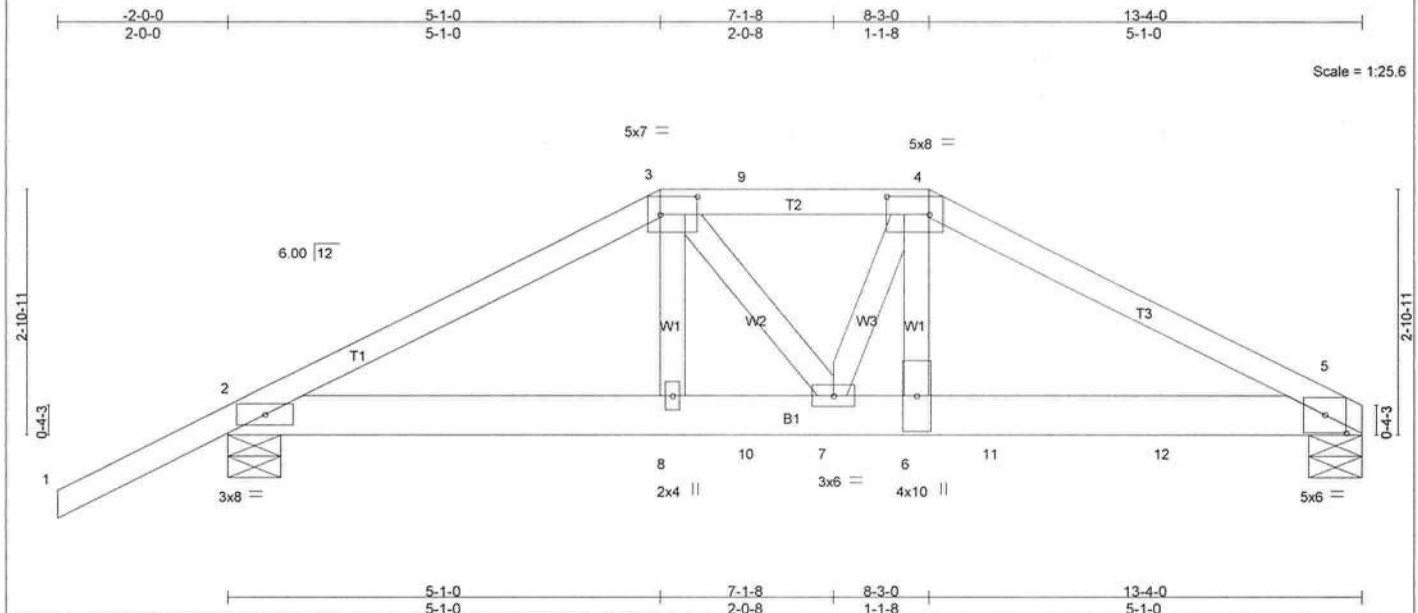


Plate Offsets (X,Y): [3:0-5-4,0-2-8], [4:0-6-0,0-2-8], [5:0-3-0,0-2-9]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25		TC 0.39	Vert(LL) -0.08	5-6	>999	360		MT20	244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.75	Vert(TL) -0.16	5-6	>978	240			
BCLL 0.0 *	Rep Stress Incr NO		WB 0.55	Horz(TL) 0.03	5	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL) 0.08	5-6	>999	240			
									Weight: 141 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 5=4163/0-7-8, 2=2608/0-7-8
Max Horz 2=104(LC 5)
Max Uplift 5=1590(LC 6), 2=1175(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

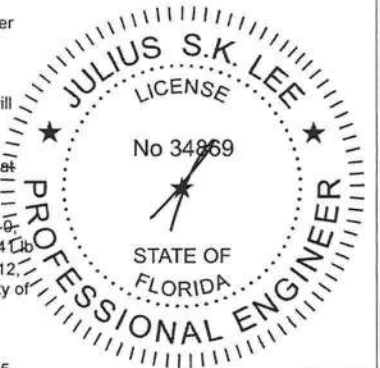
TOP CHORD 2-3=-4969/2066, 3-9=-5875/2487, 4-9=-5875/2487, 4-5=-6740/2707
BOT CHORD 2-8=-1771/4337, 8-10=-1774/4346, 7-10=-1774/4346, 6-7=-2405/6133, 6-11=-2340/5935,
11-12=-2340/5935, 5-12=-2340/5935
WEBS 3-8=-49/280, 4-6=-1125/3402, 3-7=-1035/2461, 4-7=-669/47

NOTES (12-13)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1590 lb uplift at joint 5 and 1175 lb uplift at joint 2.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 37 lb down and 44 lb up at 5-1-0, and 37 lb down and 44 lb up at 6-2-3, and 37 lb down and 44 lb up at 8-3-0 on top chord, and 128 lb down and 61 lb up at 5-1-0, 4 lb down at 6-2-3, 2688 lb down and 1234 lb up at 7-1-8, 128 lb down and 61 lb up at 8-2-3, and 1316 lb down and 465 lb up at 9-0-12, and 1469 lb down and 431 lb up at 11-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Continued on page 2

LOAD CASE(S) Standard



January 25, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MTL-7473 BEFORE USE.

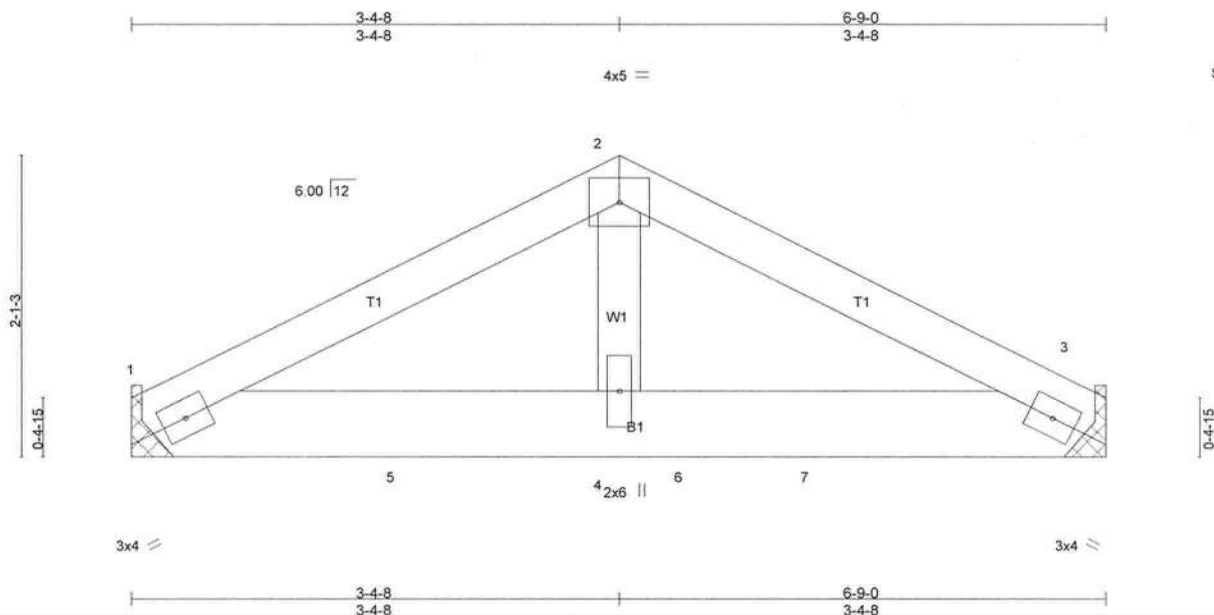
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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 323087	Truss T09	Truss Type COMMON	Qty 1	Ply 2	SPARKS - BARTH RES.	i4204330
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Builders FrstSource, Lake City, FL 32055

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.14	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.37	Vert(LL) -0.02 3-4 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.32	Vert(TL) -0.03 3-4 >999 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 3 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.02 3-4 >999 240		
				Weight: 59 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=1534/Mechanical, 3=1684/Mechanical
Max Horz 1=-28(LC 3)
Max Uplift 1=-552(LC 5), 3=-606(LC 6)

FORCES

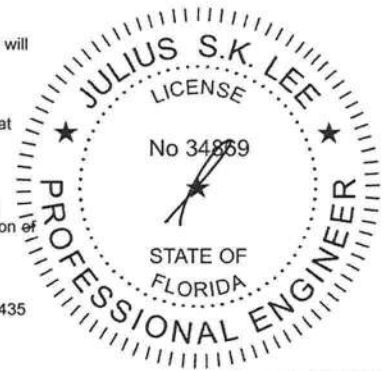
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-2480/875, 2-3=-2480/875
BOT CHORD 1-5=-724/2145, 4-5=-724/2145, 4-6=-724/2145, 6-7=-724/2145, 3-7=-724/2145
WEBS 2-4=-678/1983

NOTES (13-15)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SYP No.2.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 552 lb uplift at joint 1 and 606 lb uplift at joint 3.
- Girder carries tie-in span(s): 4-6-0 from 0-0-0 to 6-9-0
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 859 lb down and 309 lb up at 1-11-4, and 859 lb down and 309 lb up at 3-11-4, and 859 lb down and 309 lb up at 4-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HGUS26-2 to attach Truss to Carrying member

LOAD CASE(S) Standard

Continued on page 2



January 25, 2010



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 323087	Truss T10	Truss Type SPECIAL	Qty 1	Ply 2	SPARKS - BARTH RES. Job Reference (optional)	I4204331
Builders FirstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:15 2010 Page 1			

Scale = 1:76.9

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.51	Vert(LL)	-0.41 17-18	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.75	Vert(TL)	-0.79 17-18	>627	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.57	Horz(TL)	0.19 12	n/a	n/a		
BCDL 5.0	Code FBC2007/TP12002		(Matrix)	Wind(LL)	0.46 17-18	>999	240		Weight: 468 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2 *Except* T4: 2 X 6 SYP No.1D	TOP CHORD Structural wood sheathing directly applied or 4-7-15 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 7-4-4 oc bracing.
WEBS 2 X 4 SYP No.3 *Except* W5: 2 X 6 SYP No.1D	

REACTIONS (lb/size) 2=2651/0-7-8, 12=2698/Mechanical
Max Horz 2=191(LC 5)
Max Uplift 2=-1181(LC 5), 12=-1245(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

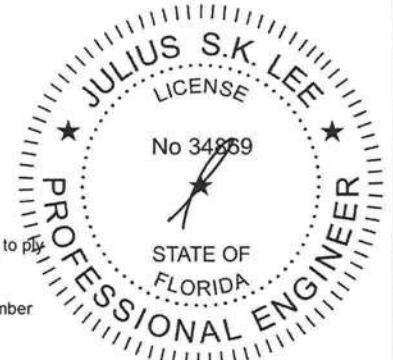
TOP CHORD 2-3=-5019/2224, 3-22=-6532/3020, 22-23=-6532/3020, 4-23=-6531/3020, 4-24=-6532/3020, 24-25=-6532/3020, 25-26=-6532/3020, 5-26=-6532/3020, 5-27=-7709/3565, 27-28=-7709/3565, 6-28=-7709/3565, 6-29=-7709/3565, 29-30=-7709/3565, 7-30=-7709/3565, 7-31=-4958/2300, 31-32=-4958/2300, 32-33=-4958/2300, 8-33=-4958/2300, 8-34=-4958/2299, 34-35=-4958/2299, 9-35=-4959/2300, 9-10=-2275/1042

BOT CHORD 2-21=-1996/4355, 21-36=-2000/4373, 36-37=-2000/4373, 20-37=-2000/4373, 20-38=-3508/7608, 38-39=-3508/7608, 19-39=-3508/7608, 19-40=-3508/7608, 18-40=-3508/7608, 18-41=-3508/7608, 41-42=-3508/7608, 17-42=-3508/7608, 17-43=-3148/6820, 16-43=-3148/6820, 16-44=-3148/6820, 15-44=-3148/6820, 15-45=-3148/6820, 45-46=-3148/6820, 46-47=-3148/6820, 14-47=-3148/6820, 14-48=-914/1986, 48-49=-914/1986, 13-49=-914/1986, 13-50=-706/1537, 12-50=-706/1537

WEBS 3-21=-89/543, 3-20=-1255/2640, 4-20=-575/408, 5-20=-1324/614, 5-18=0/324, 6-17=-540/378, 7-17=-493/1074, 7-15=0/322, 7-14=-2246/1033, 8-14=-569/400, 9-14=-1646/3553, 9-13=-1320/716, 10-13=-721/1554, 10-12=-2862/1311

NOTES (13-15)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.
323087	T11	HIP	1	1	

I4204332

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

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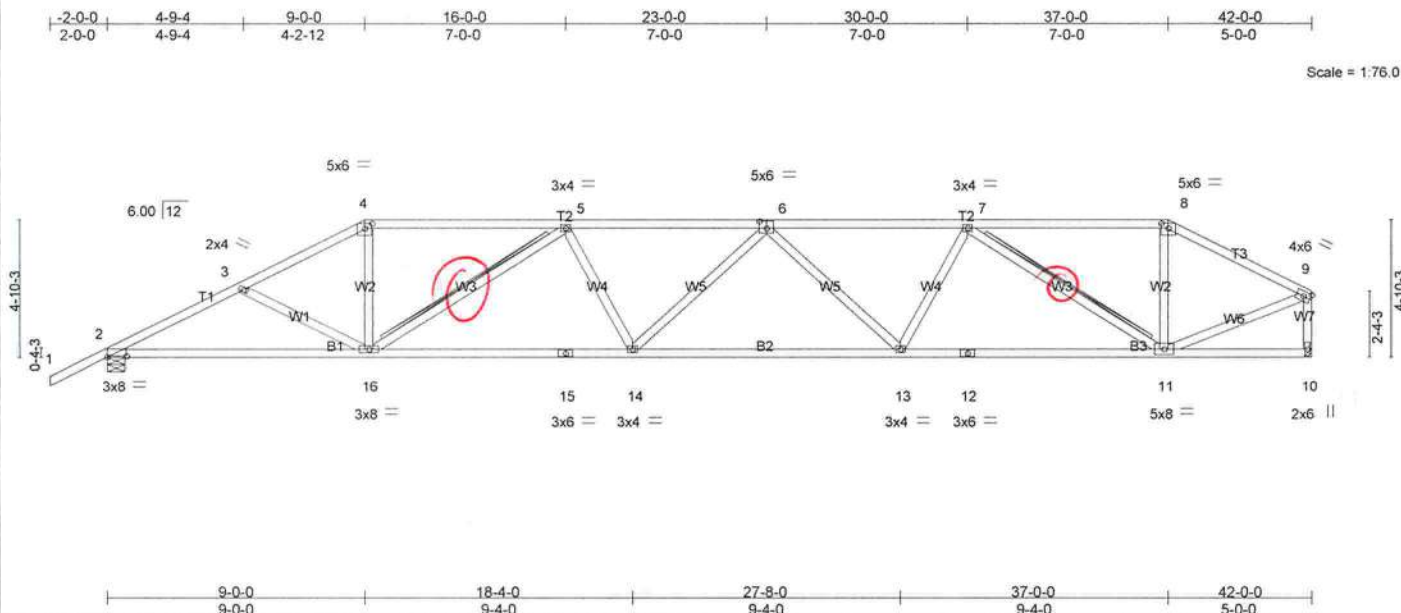


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [4:0-3-0,0-2-0], [6:0-3-0,0-3-0], [8:0-3-0,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.53	Vert(LL)	-0.27 13-14	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.64	Vert(TL)	-0.58 13-14	>865	240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.60	Horz(TL)	0.16 10	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.41 13-14	>999	240		Weight: 218 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3 *Except*
 W7: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-5-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 4-4-0 oc bracing.
 WEBS T-Brace: 2 X 4 SYP No.3 - 5-16, 7-11
 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
 Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1458/0-7-8, 10=1326/Mechanical
 Max Horz 2=183(LC 6)
 Max Uplift 2=-474(LC 6), 10=-459(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2487/1751, 3-4=-2303/1648, 4-5=-2038/1555, 5-6=-2957/2195, 6-7=-2715/2017,
 7-8=-1309/1041, 8-9=-1507/1077, 9-10=-1307/963

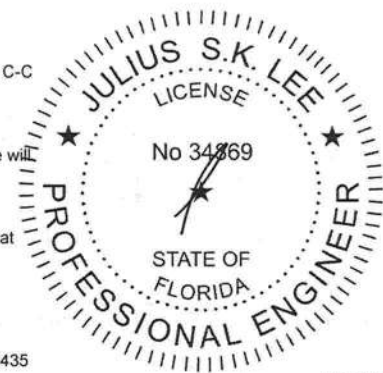
BOT CHORD 2-16=-1569/2128, 15-16=-2021/2878, 14-15=-2021/2878, 13-14=-2126/3023,
 12-13=-1758/2512, 11-12=-1758/2512

WEBS 4-16=-443/711, 5-16=-1074/747, 5-14=-38/269, 6-13=-445/371, 7-13=-226/467,
 7-11=-1472/1048, 8-11=-175/389, 9-11=-913/1375

NOTES (11-13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) All bearings are assumed to be SYP No.2.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 474 lb uplift at joint 2 and 459 lb uplift at joint 10.
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- 13) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



January 25, 2010

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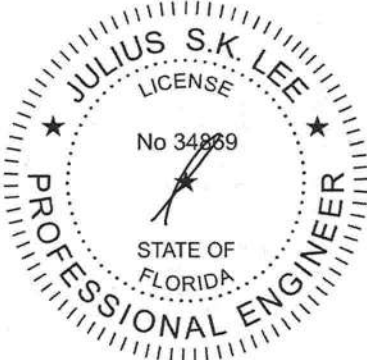
Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.	I4204333
323087	T12	HIP	1	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

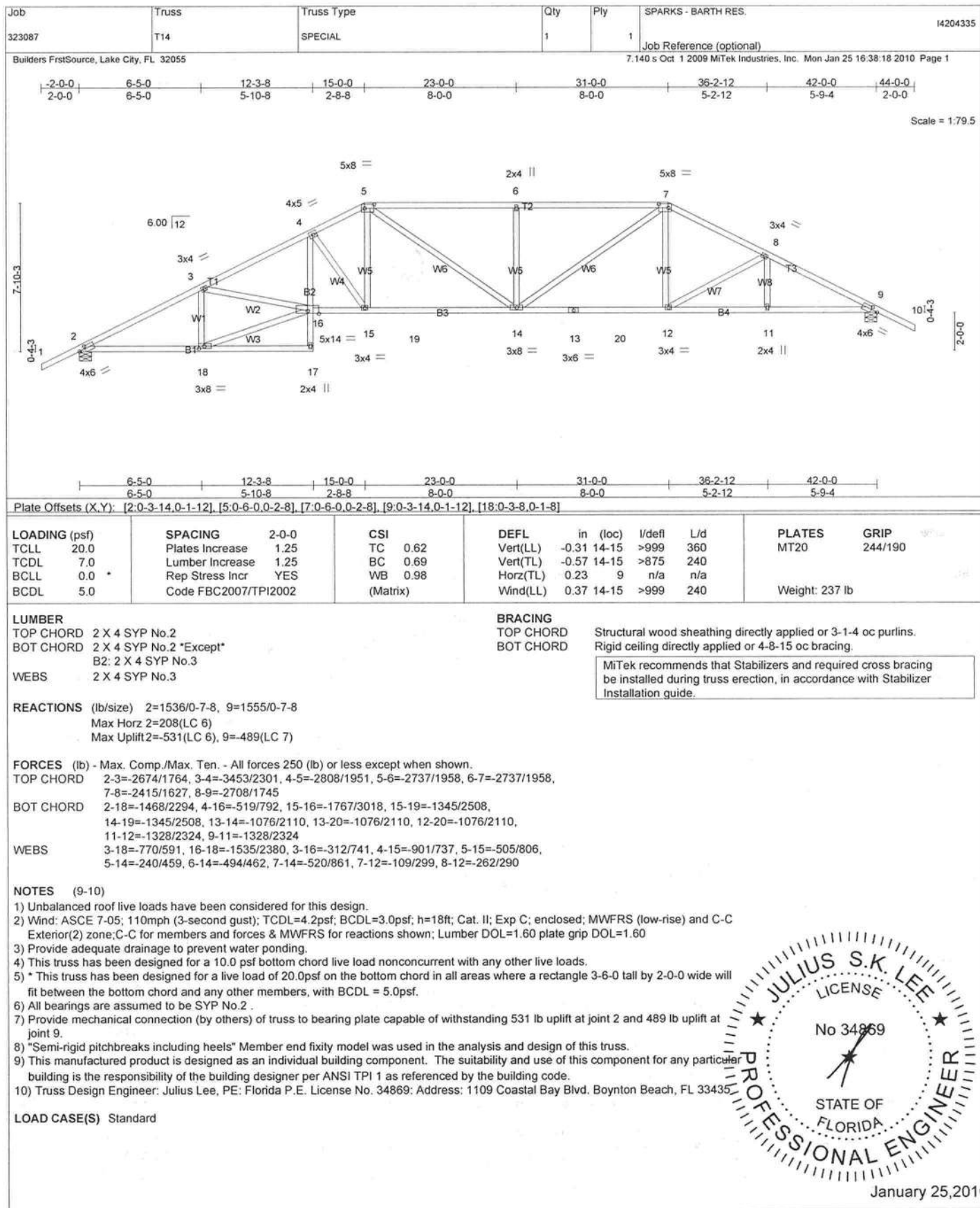


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 is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and IBCS11 Building Component**
Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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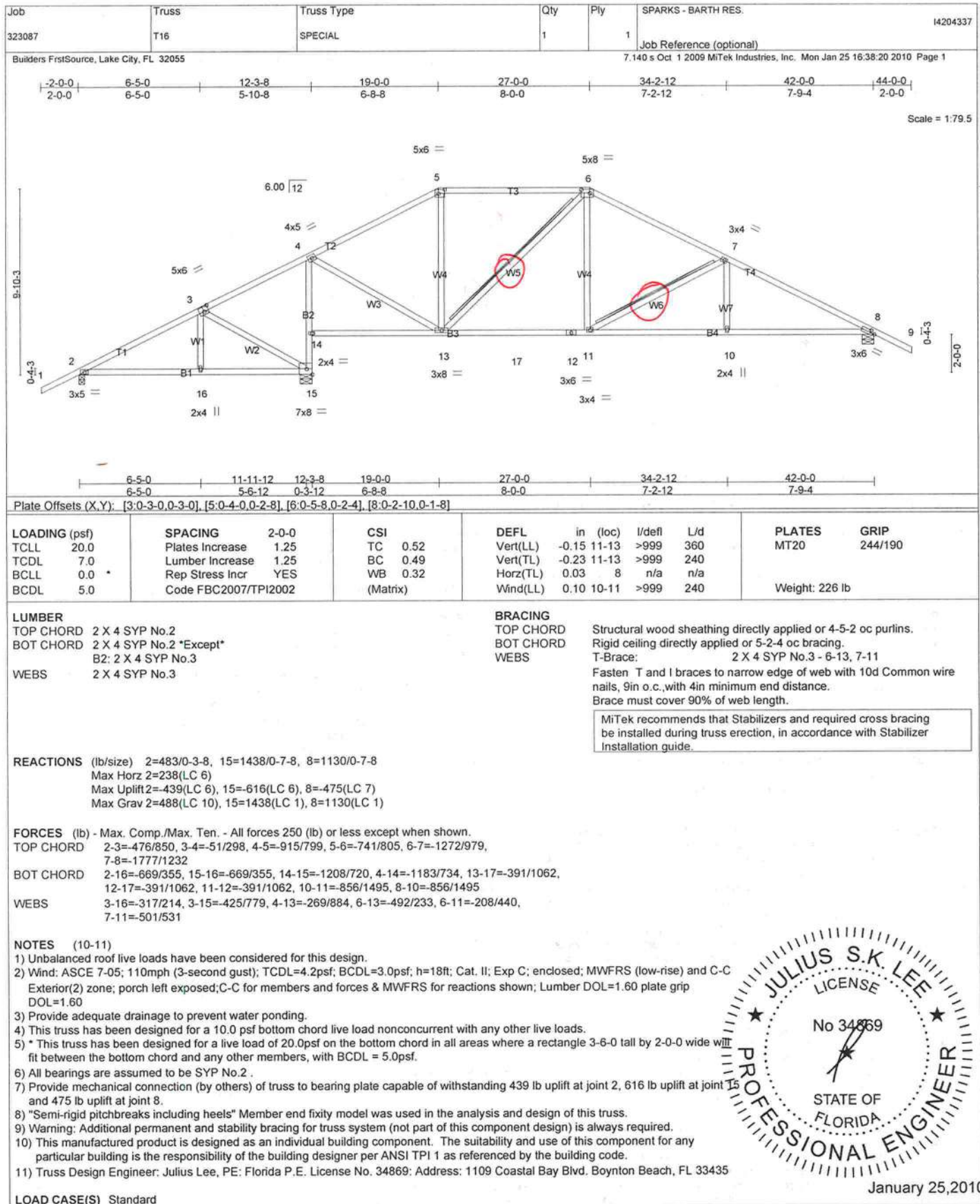


January 25, 2010



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 Boynton, FL 33435



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Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.
323087	T17	SPECIAL	1	1	Job Reference (optional)

I4204338

Builders FirstSource, Lake City, FL 32055

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NOTES (12-13)

- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-7=-54, 7-8=-54, 8-13=-54, 2-23=-10, 13-22=-10
 Concentrated Loads (lb)
 Vert: 14=-1570(F)



January 25, 2010

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 Boynton, FL 33435

Job 323087	Truss T18	Truss Type SPECIAL	Qty 3	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204339
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Builders FrstSource, Lake City, FL 32055 7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:22 2010 Page 2

12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



January 25, 2010



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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job 323087	Truss T19	Truss Type SPECIAL	Qty 1	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204340
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Builders FrstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:23 2010 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-7=-54, 7-8=-54, 8-13=-54, 2-25=-10, 13-24=-10

Concentrated Loads (lb)

Vert: 14=-1570(F)



January 25, 2010



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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.	I4204341
323087	T22	SPECIAL	5	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 25, 2010



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Boynton, FL 33435

Job 323087	Truss T23	Truss Type MONO HIP	Qty 1	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204342
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Builders FrstSource, Lake City, FL 32055 7,140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:25 2010 Page 2

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

11) Truss Design Engineer; Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-7=-54, 2-8=-10

Concentrated Loads (lb)

Vert: 3=-95(F) 13=-321(F) 6=-95(F) 9=-29(F) 14=-95(F) 15=-95(F) 16=-95(F) 17=-95(F) 18=-95(F) 19=-95(F) 20=-95(F) 21=-95(F) 22=-95(F) 23=-29(F) 24=-29(F) 25=-29(F) 26=-29(F) 27=-29(F) 28=-29(F) 29=-29(F) 30=-29(F) 31=-29(F)



January 25, 2010

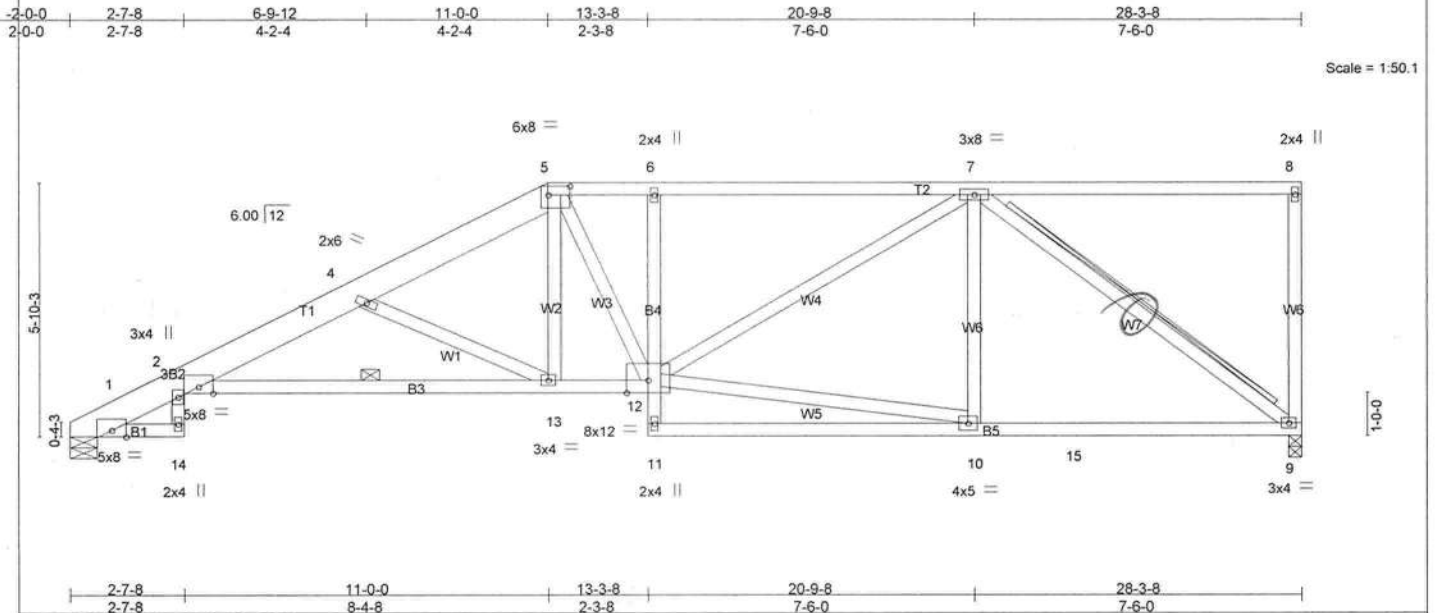


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.

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Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job 323087	Truss T25	Truss Type SPECIAL	Qty 1	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204344
Builders FirstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:26 2010 Page 1			



LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	Plates Increase	2-0-0	TC	0.57	Vert(LL)	-0.16 3-13 >999 360	MT20		244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.34 3-13 >955 240				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.16 9 n/a n/a				
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.21 3-13 >999 240				Weight: 186 lb

LUMBER		BRACING	
TOP CHORD	2 X 8 SYP No.1D *Except* T2: 2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-0-6 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2 *Except* B2,B4: 2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 6-2-9 oc bracing. Except: 4-9-0 oc bracing: 3-13
WEBS	2 X 4 SYP No.3	WEBS	T-Brace: 2 X 4 SYP No.3 - 7-9 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

REACTIONS (lb/size) 1=889/0-7-8, 9=939/0-3-8
Max Horz 1=223(LC 6)
Max Uplift 1=251(LC 6), 9=349(LC 5)

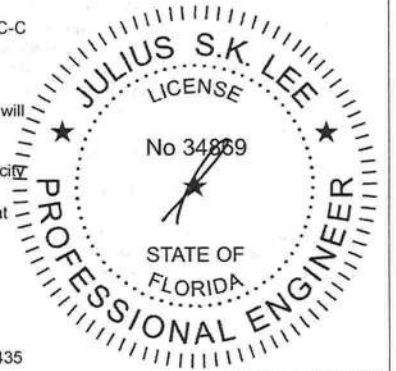
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-621/193, 2-3=-517/396, 3-4=-2039/1538, 4-5=-1523/1094, 5-6=-1365/1071, 6-7=-1366/1078
BOT CHORD 3-13=-1720/1914, 12-13=-1036/1298, 6-12=-254/246, 10-15=-694/982, 9-15=-694/982
WEBS 4-13=-692/763, 5-13=-292/437, 10-12=-658/893, 7-12=-452/451, 7-9=-1206/854

- NOTES** (11-12)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
 - 5) All bearings are assumed to be SYP No.2.
 - 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 251 lb uplift at joint 1 and 349 lb uplift at joint 9.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
 - 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435



January 25, 2010

Job 323087	Truss T27	Truss Type SPECIAL	Qty 1	Ply 1	SPARKS - BARTH RES. Job Reference (optional)	I4204346
Builders FrstSource, Lake City, FL 32055			7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:27 2010 Page 1			

2-0-0 | 2-7-8 | 7-11-8 | 13-3-8 | 15-0-0 | 21-7-12 | 28-3-8

2-0-0 | 2-7-8 | 5-4-0 | 5-4-0 | 1-8-8 | 6-7-12 | 6-7-12

4x6 = 3x4 = 3x5 = Scale = 1:51.9

6.00 | 12

7-10-3

0-4-3

3x4 ||

2x4 ||

5x8 = 15

2x4 ||

6x8 =

4

14

2x4 ||

13

12

2x4 ||

6x10 =

11

10

2x4 ||

2x4 ||

5x8 =

9

16

8

2x4 ||

1-0-0

2-7-8 | 7-11-8 | 13-3-8 | 15-0-0 | 21-7-12 | 28-3-8

2-7-8 | 5-4-0 | 5-4-0 | 1-8-8 | 6-7-12 | 6-7-12

Plate Offsets (X,Y): [4:0-3-4-0-3-0], [12:0-4-4-0-2-8]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.56	Vert(LL) -0.13 13-14	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.43	Vert(TL) -0.25 13-14	>999	240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.83	Horz(TL) -0.16 8	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.24 3-14	>999	240		
						Weight: 189 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 *Except* T1: 2 X 8 SYP No.1D BOT CHORD 2 X 4 SYP No.2 *Except* B2,B4: 2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 5-4-14 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 4-10-2 oc bracing. WEBS T-Brace: 2 X 4 SYP No.3 - 7-8, 7-9 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.
--	--

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

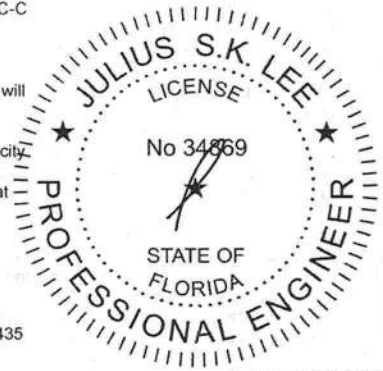
REACTIONS (lb/size) 8=989/0-3-8, 1=875/0-7-8
 Max Horz 1=305(LC 6)
 Max Uplift 8=-343(LC 5), 1=-265(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-612/88, 2-3=-618/389, 3-4=-1833/1330, 4-5=-1148/812, 5-6=-931/808, 6-7=-632/499, 7-8=-865/710
 BOT CHORD 3-14=-1615/1671, 13-14=-1615/1676, 12-13=-1546/1653
 WEBS 4-12=-804/871, 6-12=-434/419, 6-9=-700/685, 7-9=-756/950, 9-12=-427/591

NOTES (11-12)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Provide adequate drainage to prevent water ponding.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 5) All bearings are assumed to be SYP No.2
- 6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 343 lb uplift at joint 8 and 265 lb uplift at joint 1.
- 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 25, 2010

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Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.
323087	T29	SPECIAL	1	1	

I4204348

Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

7.140 s Oct 1 2009 MiTek Industries, Inc. Mon Jan 25 16:38:29 2010 Page 1

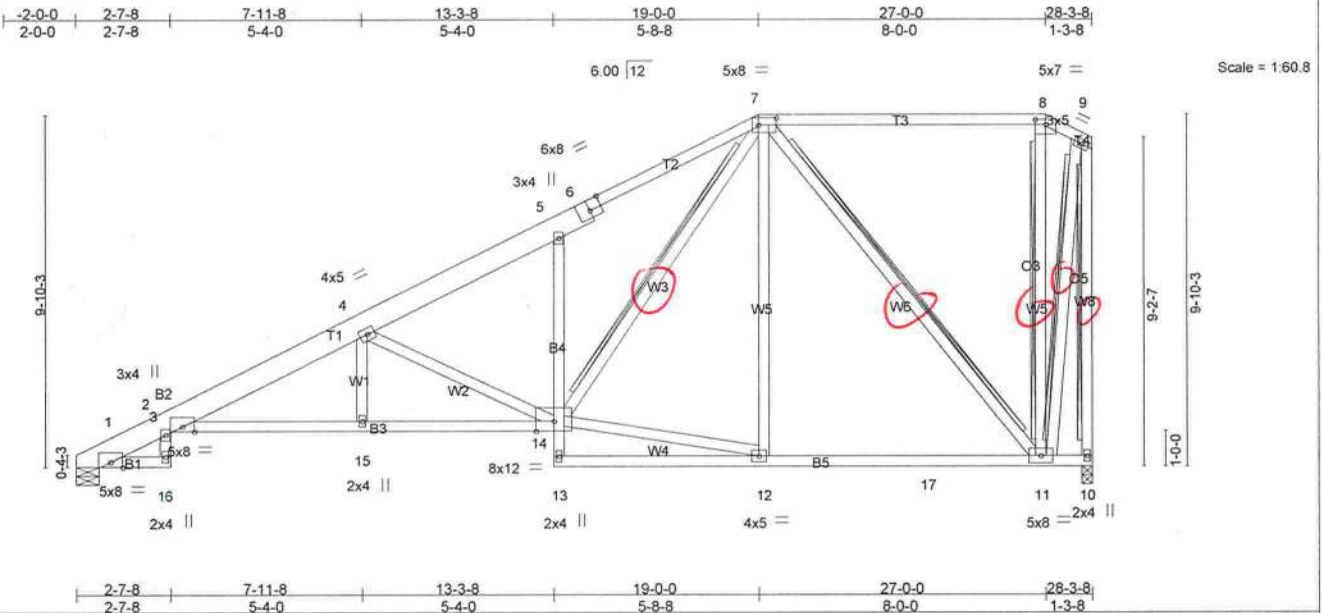


Plate Offsets (X,Y): [6:0-4-0,Edge], [7:0-6-0,0-2-8], [8:0-3-8,0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.59	Vert(LL)	-0.17 11-12	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.46	Vert(TL)	-0.26 11-12	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.69	Horz(TL)	-0.17 10	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.25 3-15	>999	240		Weight: 229 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 *Except*
T1: 2 X 8 SYP No.1D
BOT CHORD 2 X 4 SYP No.2 *Except*
B2,B4: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W8: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-1-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-8-1 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-14, 7-11, 8-11, 9-10, 9-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=913/0-7-8, 10=1007/0-3-8
Max Horz 1=371(LC 6)
Max Uplift 1=-269(LC 6), 10=-303(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-639/11, 2-3=-686/408, 3-4=-1996/1344, 4-5=-1380/920, 5-6=-1351/1101,
6-7=-1225/1110, 9-10=-1076/691
BOT CHORD 3-15=-1732/1835, 14-15=-1732/1835, 5-14=-233/364, 12-17=-519/671, 11-17=-519/671
WEBS 4-14=-774/788, 12-14=-484/654, 7-14=-930/865, 7-11=-810/655, 8-11=-324/300,
9-11=-729/1104

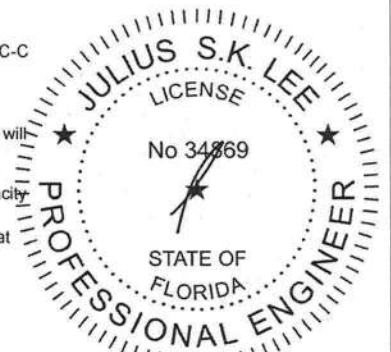
NOTES (12-13)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) All bearings are assumed to be SYP No.2.
- 7) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 1 and 303 lb uplift at joint 10.
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

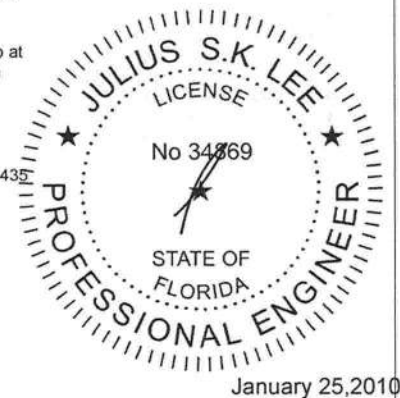
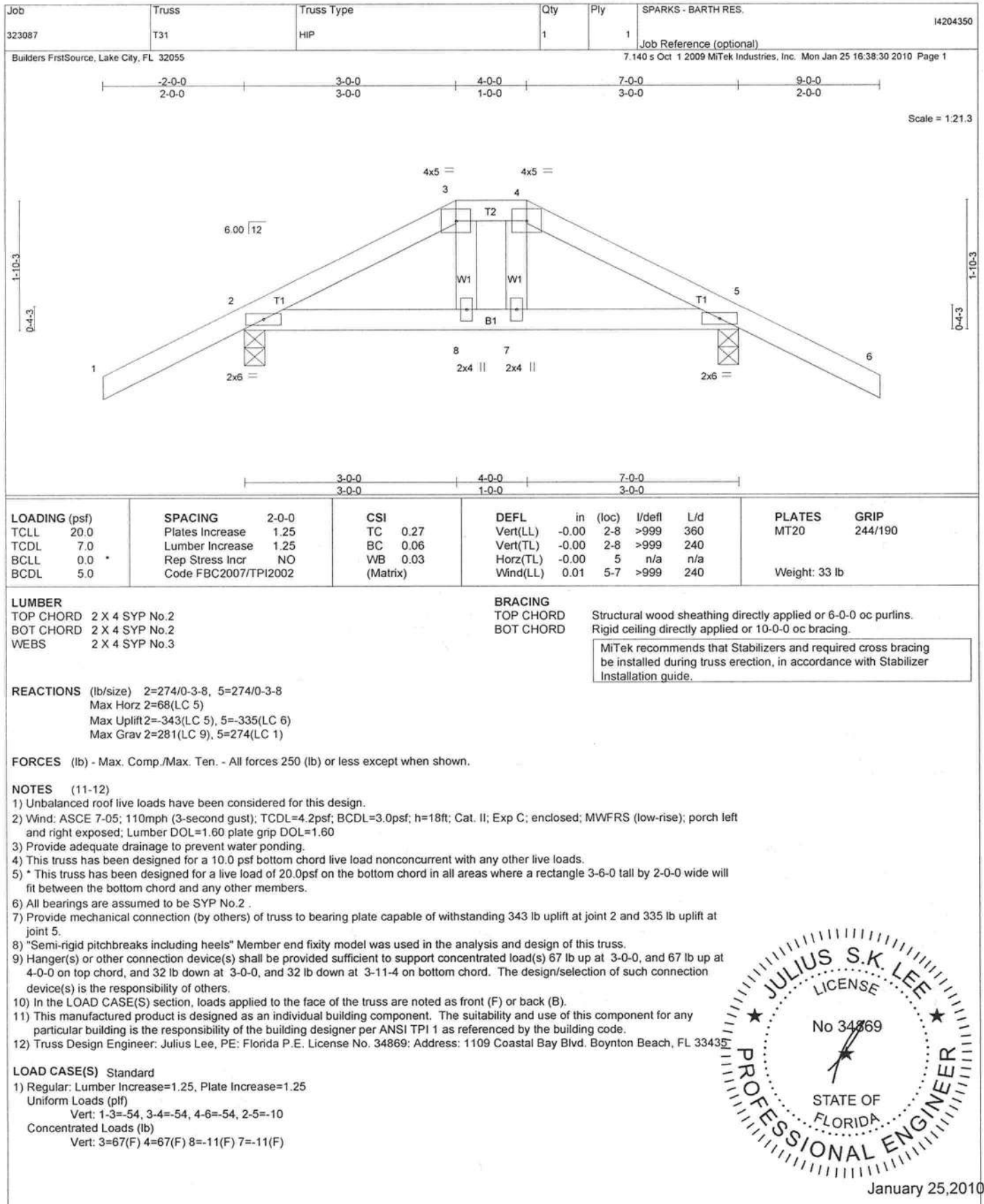
LOAD CASE(S) Standard

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January 25, 2010



January 25, 2010



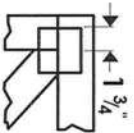
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.

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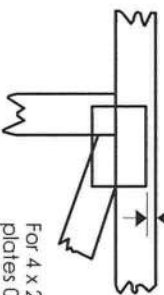
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Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless X, Y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{1}{16}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in **MITek 20/20** software or upon request.

PLATE SIZE

4 X 4

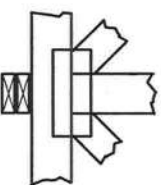
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

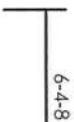


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

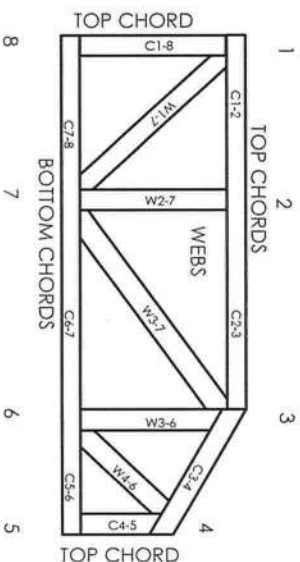
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A, NER-487, NER-561, 95110, 84-32, 96-67, ER-3907, 9432A

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/TPI 1.
7. The environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

STEPDOWN CORNER SET

	TOP	CHORD	2X4	SO.	PINE	#2	or Better
	BOT	CHORD	2X4	SO.	PINE	#2	or Better
		WEBS	2X4	SO.	PINE	#3	or Better

120 MPH MAX

Setback

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

400# or Less

LOC:

UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND
DIR. LOC.

OF THE DRAUGHTS ON THE MEAN HGT=28 FT. ENCLOSED. (ASCE 7-02)
SPEED=120 "C" MPH.

PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED. TILE

400# or Less

1.051

WIND TOTAL DEAD LOAD. WIND UPLIFT BASED ON 15.0 PSF

UPLIFT BASED ON 13.0 KIP TOTAL DEAD LOAD. MEAN HGT (of jacks)=28 FT. ENCLOSED. (ASCE 7-02)
SPEED=120 "C" MPH. WIND

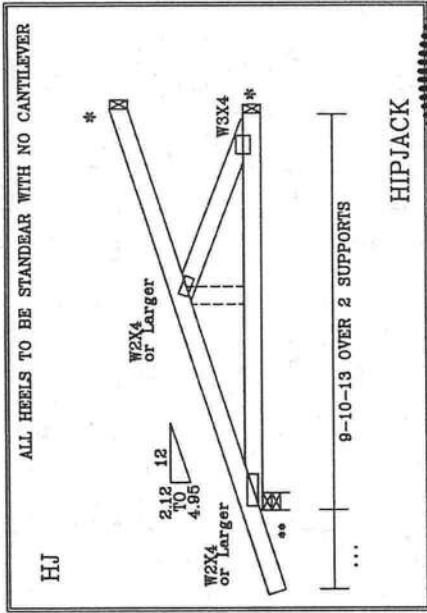
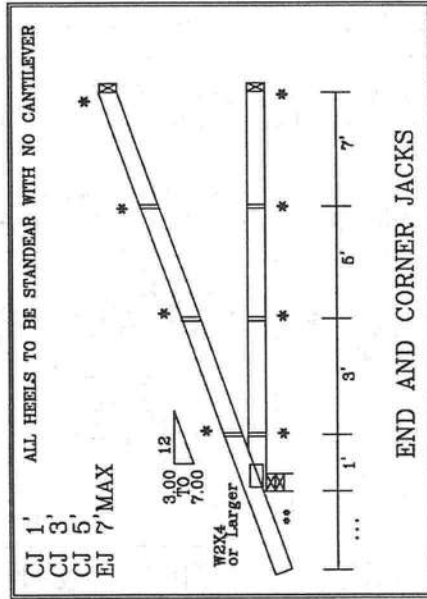
PROVIDE UPLIFT CONNECTIONS AT BEARINGS AS INDICATED.

400# or Less

LOC:

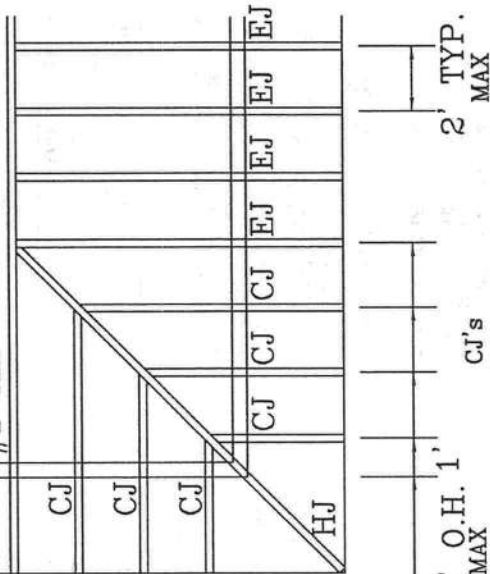
UPLIFT BASED ON 7.2 PSF TOTAL DEAD LOAD. WIND

UP LIFT BASED ON 7.5 TON TOTAL
SPEED=120 "B" MPH. MEAN HGT
(of jacks)=28 FT. ENCLOSED. (ASCE 7-02)



#2 HIP OR COMMON TRUSS

#1 HIP TRUSS



* (3) 16d TOENAILS

SEE EOR FOR TIE DOWN

END AND CORNER JACKS

HIP-JACK

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

REF	7'MAX STBK CS
DATE	Jun./27/2008
DRWG	
-ENG	
REVIEWED By Julius Ito at 10:52 am, Jun 27, 2008	

CORNER SET
SETBACK

7'0" MAX

TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. REFER TO 1-83 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE CROSS PLATE INSTITUTE, 8631 DODD RD., SUITE 200, MADISON, WISCONSIN 53717. TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WISCONSIN 53717, HAS PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE NOTICED, A CARRIER SHALL HAVE PROPERLY ATTACHED AND PROPERLY ATTACHED, BUILT & CURED.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR ALPINE ENGINEERED BUILDINGS TRUSSES FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BRACING OF TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION AND THE FOLLOWING COMMENTS BY ACPA AND ALPINE CONNECTOR PLATES ARE MADE OF 20'x18GA UH/S430 ASTM A693 GRADE 40 GALV STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE SHALL BE PER ANNEX A3 OF TPI-2008 SEC 3.1 A SEAL FOR ANY CRACKS OR DISINTEGRATED DESIGN SHOWS PROFESSIONAL ENGINEERING IS REQUIRED FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANTI/TPI 1 SEC 2.

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		(1) 1X4 "L" BRACE •		(2) 2X4 "L" BRACE •		(1) 2X6 "L" BRACE •		(2) 2X8 "L" BRACE •	
GABLE VERTICAL SPACING	SPECIES	GRADE	#1 / #2	GROUP A		GROUP B		GROUP A		GROUP B		GROUP A	
				GROUP A		GROUP B		GROUP A		GROUP B		GROUP A	
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 6"	5' 6"	6' 6"	7' 10"	8' 0"	7' 10"	8' 0"	10' 3"	12' 3"
	HF	#3	3' 1"	4' 5"	5' 10"	4' 5"	5' 10"	7' 10"	7' 10"	7' 10"	7' 10"	9' 1"	12' 3"
	STUD	STANDARD	3' 1"	4' 6"	5' 10"	4' 6"	5' 10"	7' 10"	7' 10"	7' 10"	7' 10"	9' 1"	12' 3"
	SP	#1	2' 11"	3' 9"	3' 9"	3' 9"	3' 9"	6' 9"	6' 9"	6' 9"	6' 9"	7' 10"	10' 7"
24" O.C.	SP	#2	3' 8"	5' 6"	5' 11"	5' 11"	5' 11"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	DFL	#3	3' 3"	4' 6"	4' 6"	4' 6"	4' 6"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	STUD	STANDARD	3' 3"	4' 6"	4' 6"	4' 6"	4' 6"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	SP	#1 / #2	3' 3"	4' 6"	4' 6"	4' 6"	4' 6"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
16" O.C.	SPF	#1 / #2	3' 8"	5' 6"	5' 11"	5' 11"	5' 11"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	HF	#3	3' 7"	5' 5"	5' 5"	5' 5"	5' 5"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	STUD	STANDARD	3' 7"	5' 5"	5' 5"	5' 5"	5' 5"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	SP	#1	3' 7"	5' 5"	5' 5"	5' 5"	5' 5"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
12" O.C.	SPF	#1 / #2	3' 8"	5' 6"	5' 11"	5' 11"	5' 11"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	HF	#3	3' 11"	5' 3"	5' 3"	5' 3"	5' 3"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	STUD	STANDARD	3' 11"	5' 3"	5' 3"	5' 3"	5' 3"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"
	SP	#1	3' 11"	5' 3"	5' 3"	5' 3"	5' 3"	7' 10"	7' 10"	7' 10"	7' 10"	10' 3"	12' 3"

BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPRUC-PINE-FIR	MEM-FIR
#1 / #2 STANDARD	#2 STUD
#3 STUD	STANDARD
DOUGLAS FIR-LARCH	
#1	#2
STUD	STANDARD
SOUTHERN PINE	
#1	#2
STUD	STANDARD
GROUP B:	
MEM-FIR	#1
#1 & 2	#1
DOUGLAS FIR-LARCH	
#1	#2
STUD	STANDARD

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 180 PLF OVER CONTINUOUS BEARING (6 PSF VC DEAD LOAD).

CABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

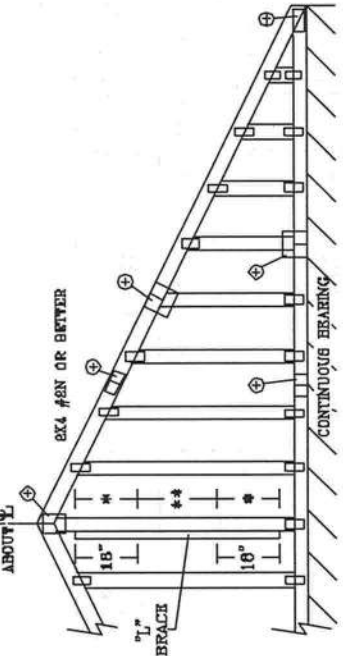
ATTACH EACH "L" BRACE WITH 104 NAILS.

* FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.

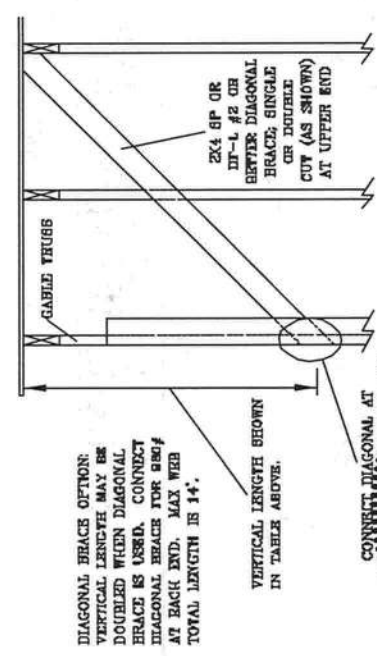
** FOR (2) "L" BRACE: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X4
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2.5X4
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	



REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.



DIAGONAL BRACE OPTION:
VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 80% AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT 18" FROM GABLE END.

ASCE 7-02-GAB10030

DATE 11/26/03

DWG MYKE STD GABLE 90' E HT

-ENG

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 SW 4th AVENUE
DELBAY BEACH, FL 33444-0161

No. 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

PROFESSIONAL ENGINEER
JULIUS LEE
NO. 34869
STATE OF FLORIDA

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPICES MUST BE STAGGERED SO THAT ONE SPICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT., ASCE 7-02, CLOSED BLDG., LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, EBC

ENCLOSED BLDG., LOCATED ANYWHERE IN ROOF

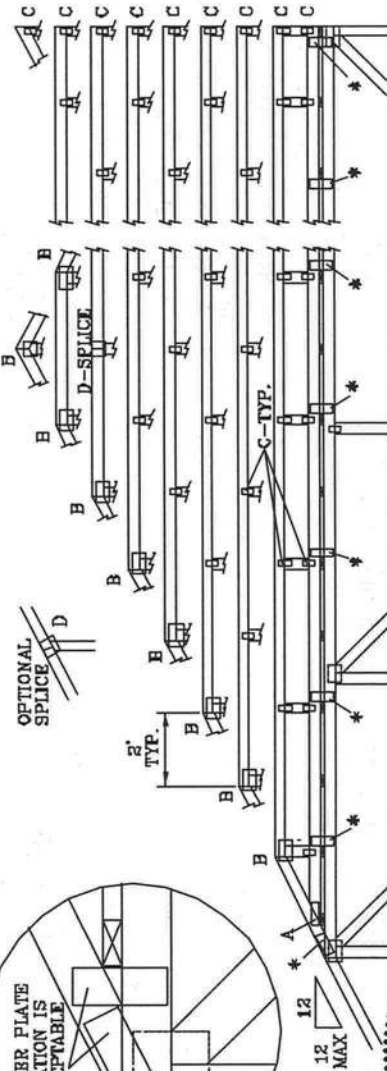
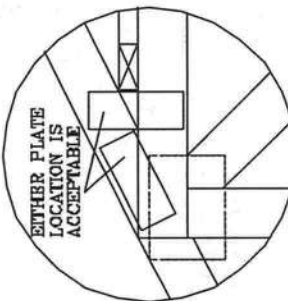
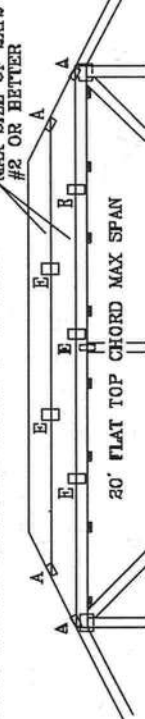
WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E.*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG., LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

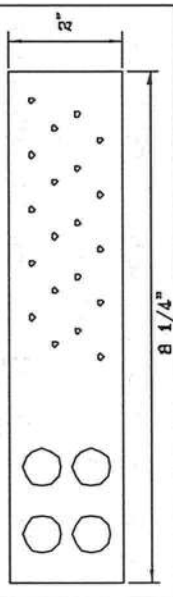
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X6
B	4X8	6X8	6X8	5X8
C	1.5X8	1.5X4	1.5X4	1.5X4
D	5X4	5X6	5X6	5X6
E	4X8 OR 3X8 TRULOX AT 4' OC, ROTATED VERTICALLY			



WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4' OC.
10' TO 14'	2X4 T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.

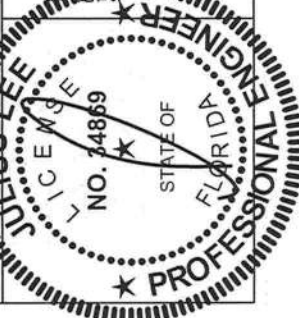
* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634.018 634.017 & 847.045

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 4th AVENUE
OCEAN BEACH, FL 33444-2181

MAX LOADING
55 PSF AT
1.33 DUR. FAC.
50 PSF AT
1.25 DUR. FAC.
47 PSF AT
1.15 DUR. FAC.

REF PIGGYBACK
DATE 09/12/07
DRWGMITEK STD PIGGY
-ENG JL

SPACING 24.0"

No: 34869
STATE OF FLORIDA

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD."

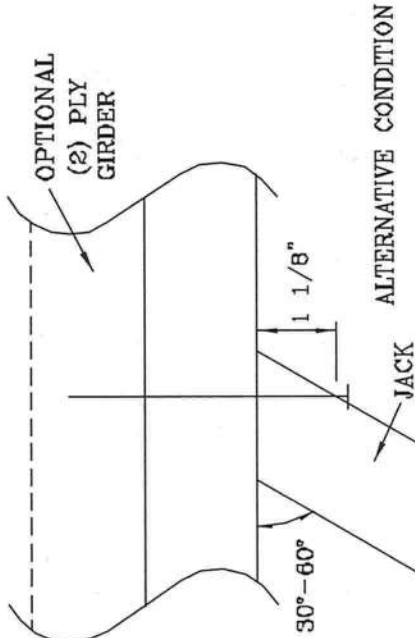
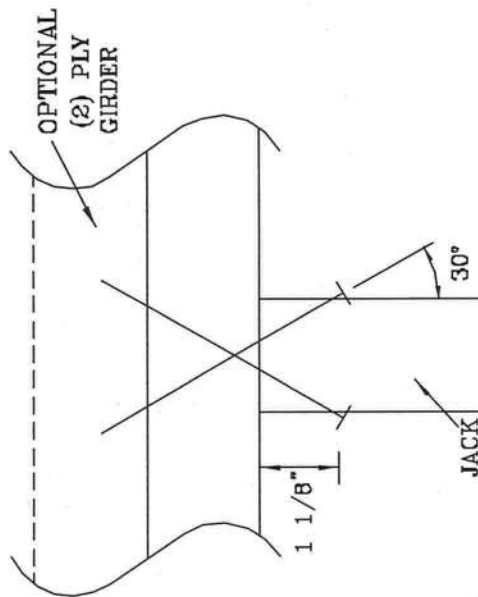
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040

JULIUS LEE'S CONS. ENGINEERS P.A. 1400 ST. 4th AVENUE DEERAY BEACH, FL 33444-2161 No: 34869 STATE OF FLORIDA		TC LL	PSF	REF	TOE-NAIL
		TC DL	PSF	DATE	09/12/07
		BC DL	PSF	DRWG	CNTONAIL103
		BC LL	PSF	-ENG	JL
		TOT. LD.	PSF		
		DUR. FAC.	1.00		
		SPACING			

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

NO. 24869
STATE OF FLORIDA
PROFESSIONAL ENGINEER

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR TRUSS CONSTRUCTION, PUBLISHED BY THE TRUSS ASSOCIATION OF AMERICA, ENTERPRISE, L.A., MOBILE, AL 36619 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

TRULOX CONNECTION DETAIL

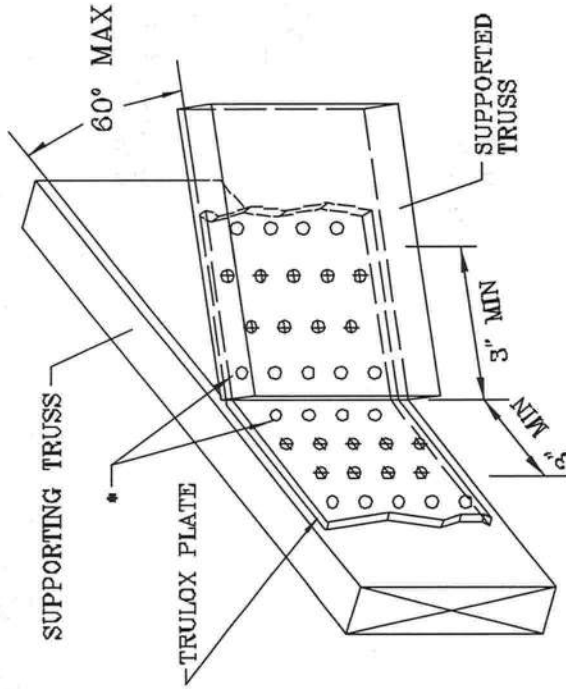
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

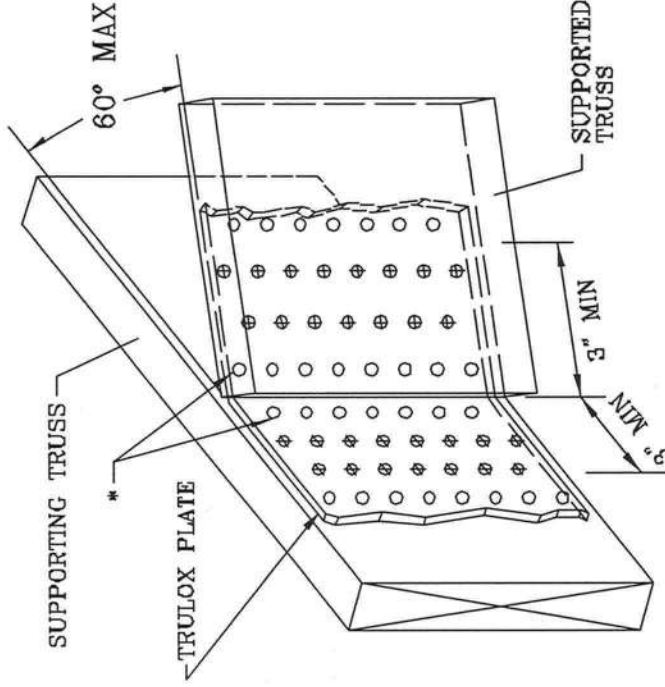


MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	15	990#

REVIEWED

By Julius Lee at 11:58 am, Jun 11, 2008



MINIMUM 5X6 TRULOX PLATE

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2881

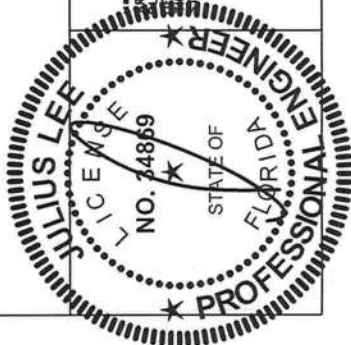
REF TRULOX

DATE 11/26/03

DRWG CNTRULOX1103

-ENG JL

No: 34869
STATE OF FLORIDA



WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1-03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 565 DOWNTOWN DR., SUITE 300, NORTON, VA 25719) AND THE TRUSS COUNCIL FOR AMERICAN BUILDING (TRUSSCOUNCIL.ORG) FOR MORE INFORMATION. TRUSSES ARE DESIGNED FOR THE FOLLOWING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

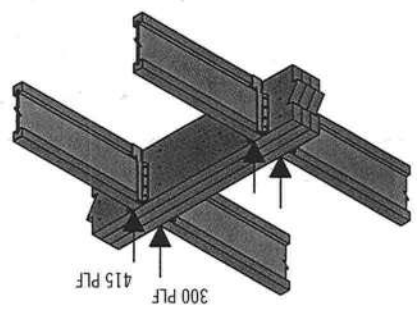
Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
1/2" A307 Through Bolts ⁽²⁾⁽³⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
SDS 1/4" x 5" (3)(4)	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
USP WS35 ⁽⁴⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
USP WS6 ⁽³⁾⁽⁴⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
3 3/4" TrussLok ⁽⁴⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
USP WS6 (3)(4)	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
6 3/4" TrussLok ⁽⁴⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
6 3/4" TrussLok ⁽⁴⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
6 3/4" TrussLok ⁽⁴⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		
6 3/4" TrussLok ⁽⁴⁾	2	12"	370	280	245	370	860	340
		12"	555	415	415	270		
		24"	505	380	520	465		
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
		24"	680	510	640	565		
	2	19.2"	850	640	765			
		16"	1,020					
		24"				455	465	455
		19.2"				565	580	565
		24"				680		
		16"				565		

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 3/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bolt Heliac** cells indicate **Connector Pattern** must be installed on both sides.
- Stagger fasteners on opposite side of beam by 1/4 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.



Uniform Load Design Example

First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:
Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6(1)	4	1,915	1,435(4)	1,435	1,275	1,860(2)	1,405(2)
	6	2,870	2,150(4)	2,150	1,915	2,785(2)	2,110(2)
	8	3,825	2,870(4)	2,870	2,550	3,715(2)	2,810(2)
3 1/2" or 5" TrussLok™	4	2,545	1,910(4)	1,910	1,695	1,925(2)	1,775(3)
	6	3,815	2,860(4)	2,860	2,545	2,890(2)	2,665(3)
	8	5,090	3,815(4)	3,815	3,390	3,855(2)	3,550(2)

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(2) 6" long screws required.

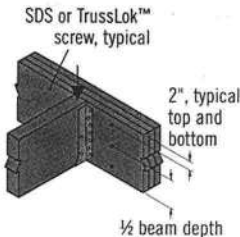
(3) 5" long screws required.

(4) 3 1/2" and 3 3/4" long screws must be installed on both sides.

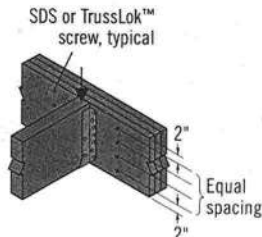
See General Notes on page 38

Connections

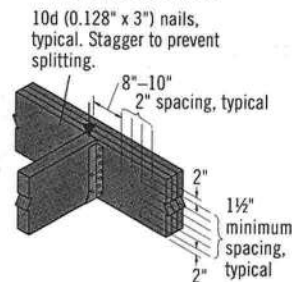
4 or 6 or Screw Connection



8 Screw Connection

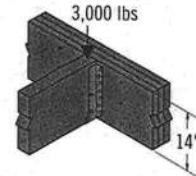


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 1/2" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 1/2" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

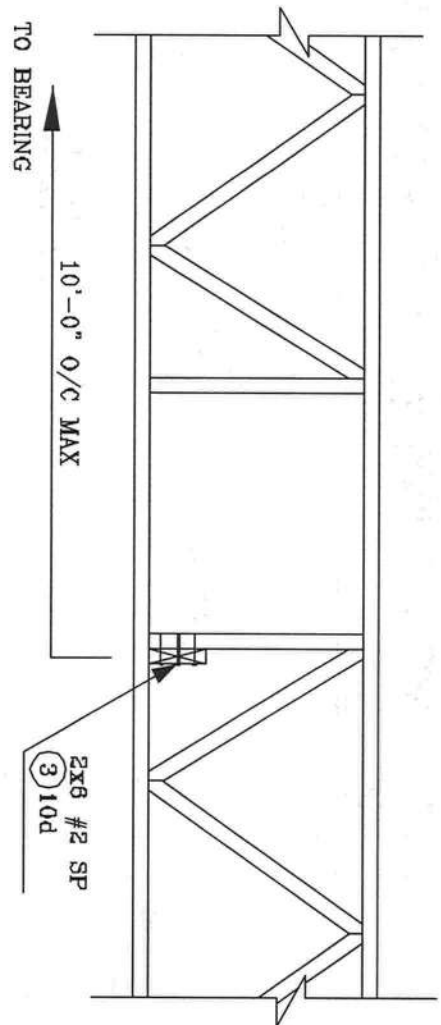
- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



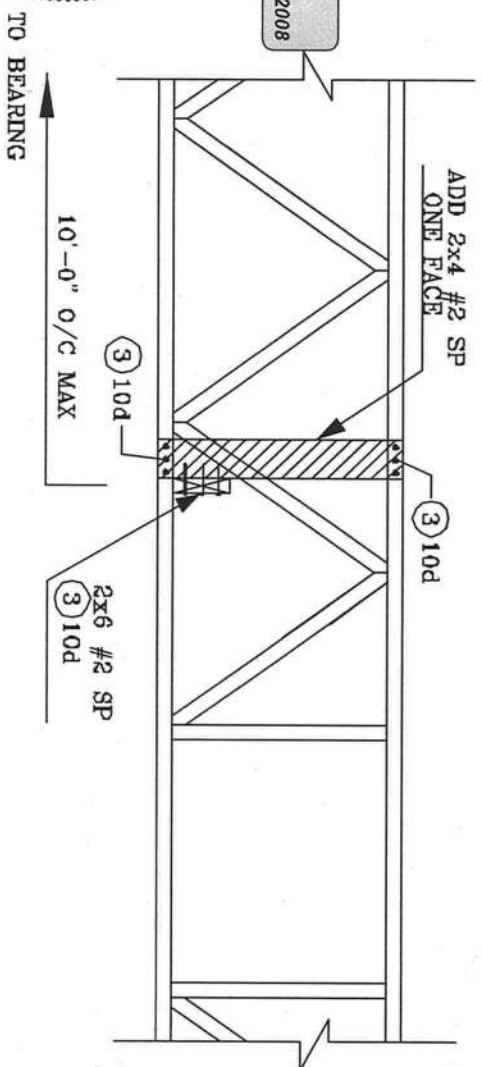
L6

Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"

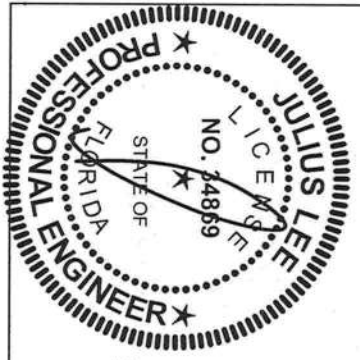
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius lee at 11:58 am, Jun 11, 2008



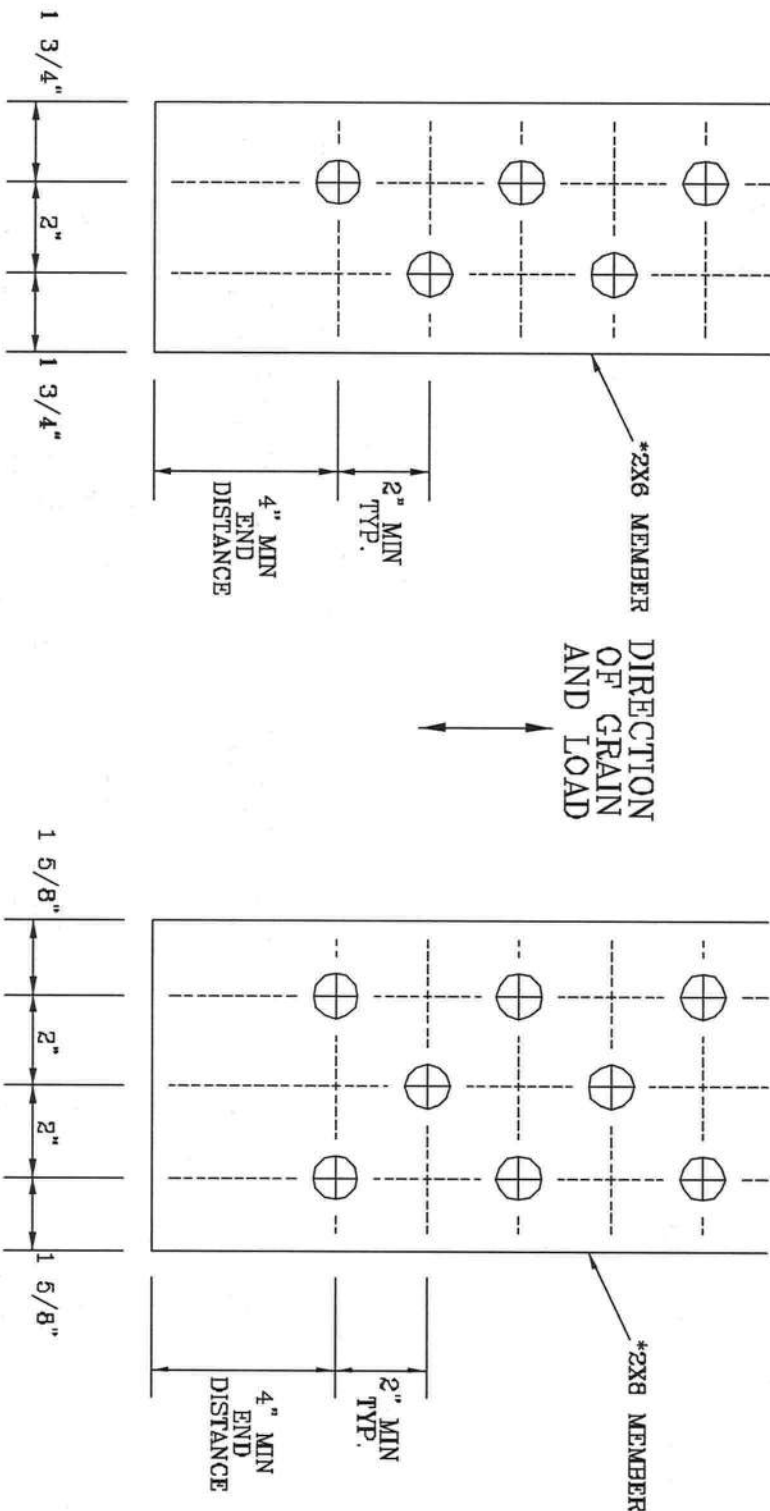
JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DIKRAY BRIDGE, FL 33444-2161

No. 34869
STATE OF FLORIDA

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

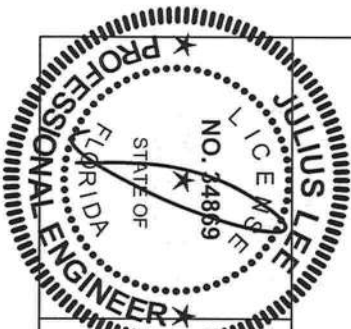
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, UNLOADING AND ERECTION. REFER TO POST-1-00 BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 200 E. MAIN ST., SUITE 200, HANOVER, VA 22979 AND APCA CURED TRUSS COUNCIL, 1400 37th AVENUE, DELRAY BEACH, FL 33444-2161. THESE FUNCTIONS OVERLAP AND SHALL BE PERFORMED BY THE SAME PERSONNEL. STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED GRID CELING.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 37th AVENUE
DELAIR BEACH, FL 33444-2161

No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

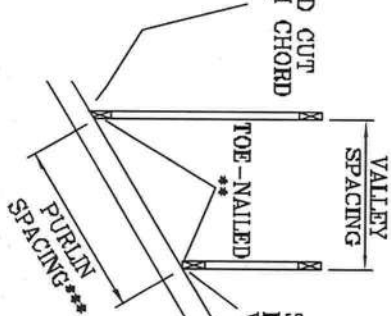
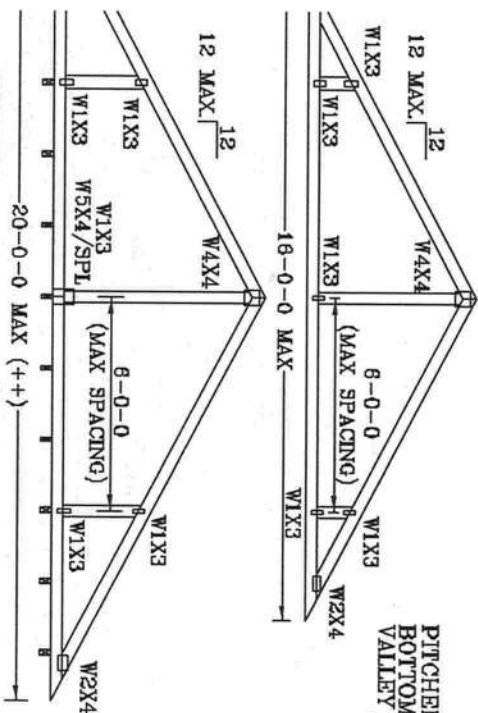
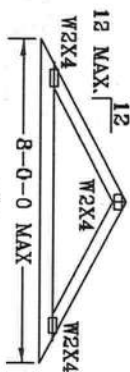
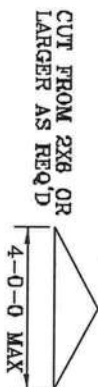
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

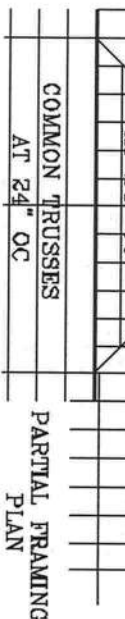
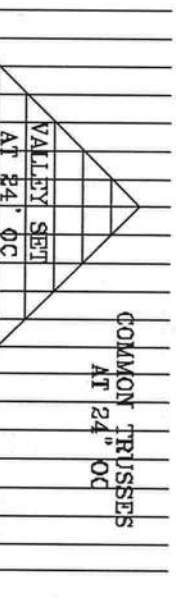
* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF.



SQUARE CUT
BOTTOM CHORD
VALLEY



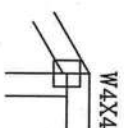
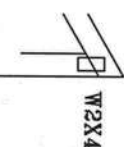
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS
BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES
NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80%
LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED
WITH 8d BOX (0.135" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING,
EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:

PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON
ENGINEERS' SEALED DESIGN.

OPTIONAL STUB
END DETAIL



THIS DRAWING REPLACES DRAWING A105

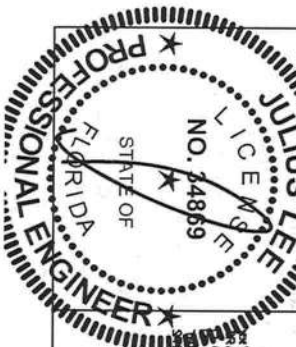
JULIUS LEE'S
CONS. ENGINEERS P.A.

1655 SW 44th AVENUE
DELRAY BEACH, FL 33444-2401

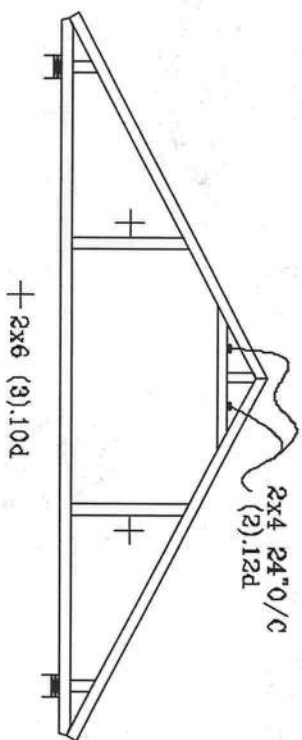
TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		

No. 34869
STATE OF FLORIDA

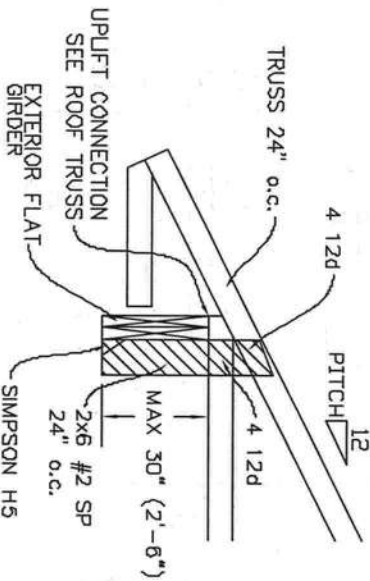
REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



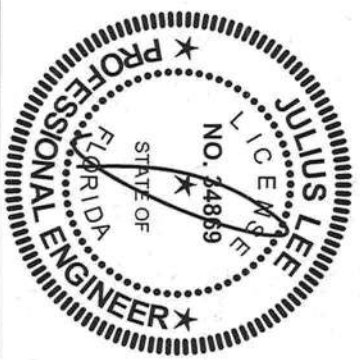
TYPICAL ATTIC TRUSS BRACING



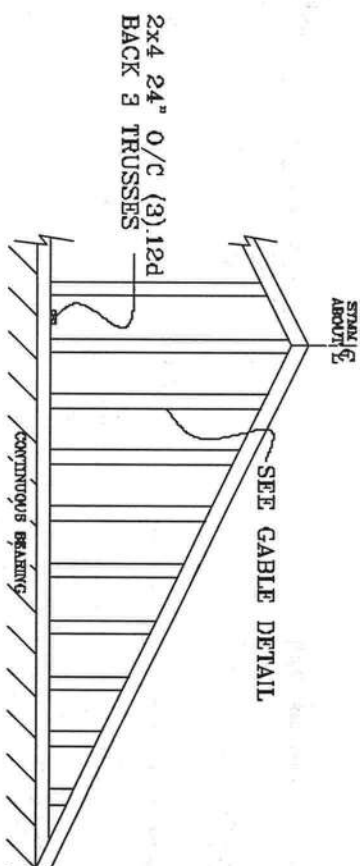
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

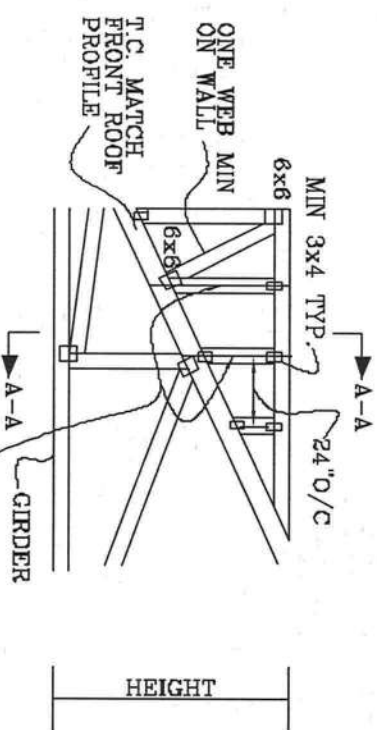


GABLE END TRUSS DETAIL



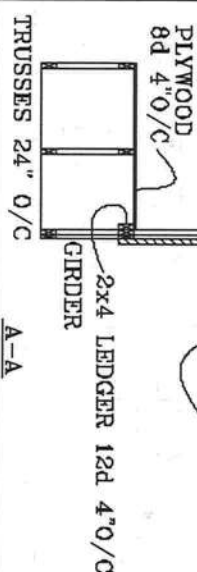
MINIMUM BC BRACING ON GABLE ENDS. OTHER PERMANENT BRACING DESIGNED BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT
ROOF 24" O/C

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL

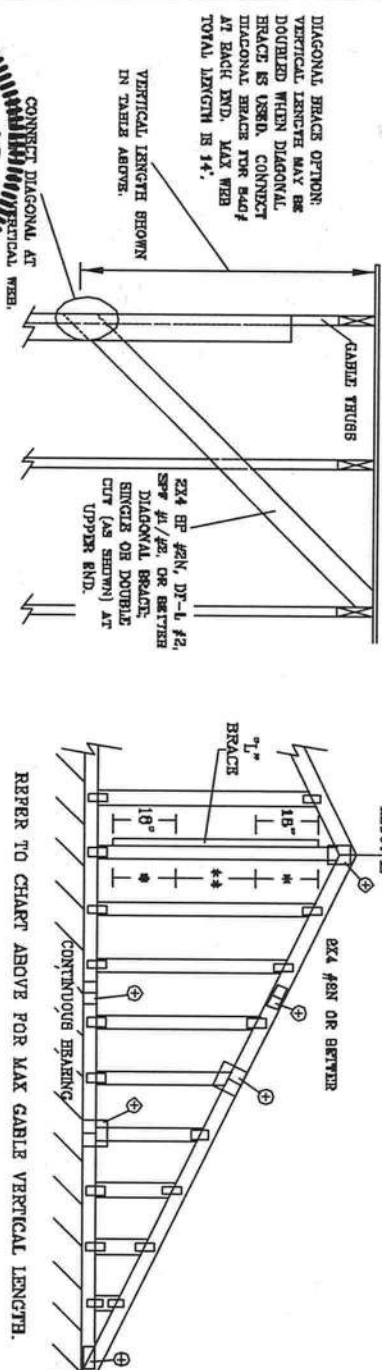


JULIUS LEE'S
CONS. ENGINEERS P.A.
1426 SW 4th AVENUE
ORLANDO, FL 32804-2101

No: 34869
STATE OF FLORIDA

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X8 "L" BRACE *	
GABLE VERTICAL SPACING	2X4 BRACE SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"		
		#3	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"		
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"		
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	9' 6"	9' 6"	11' 8"	11' 8"		
16" O.C.	SPF	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 11"		
		#2	3' 7"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 11"		
		#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"		
		STUD	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 3"	10' 3"	12' 11"	13' 7"		
24" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 6"	12' 9"	14' 0"	14' 0"		
		#3	3' 9"	6' 0"	6' 0"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"		
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"		
		STANDARD	3' 9"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"		
12" O.C.	SPF	#1	4' 3"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		#3	4' 0"	8' 1"	8' 1"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		STUD	4' 0"	8' 1"	8' 1"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
16" O.C.	SPF	#1	4' 3"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		#2	4' 2"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		#3	4' 0"	8' 1"	8' 1"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		STUD	4' 0"	8' 1"	8' 1"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
24" O.C.	SPF	#1 / #2	4' 3"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		#3	4' 2"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		STUD	4' 2"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		
		STANDARD	4' 2"	8' 8"	8' 8"	9' 11"	10' 2"	10' 2"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"		



BRACING GROUP SPECIES AND GRADES:		GROUP A:		GROUP B:	
SPRICE-PINE-LARCH	#1 / #2	STUD	#3	STUD	#3
	STUD	STANDARD	STANDARD	STANDARD	STANDARD
DOUGLAS FIR-LARCH	#1	STUD	#2	STUD	#2
	STUD	STANDARD	STANDARD	STANDARD	STANDARD
GROUP B:		DOUGLAS FIR-LARCH		DOUGLAS FIR-LARCH	
#1 & #2		#1		#2	
#3		#3		#3	

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
 PROVIDE UP-LIFT CONNECTIONS FOR 136 PSF OVER CONTINUOUS BEARING (6 PSF VC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4' 0" OUTLINE WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.
 ATTACH EACH "L" BRACE WITH 10K NAILS.
 * FOR (1) "L" BRACE, BRACE NAILS AT 8" O.C. IN 16' END ZONES AND 4" O.C. BETWEEN ZONES.
 ** FOR (2) "L" BRACES, BRACE NAILS AT 3" O.C. IN 16' END ZONES AND 4" O.C. BETWEEN ZONES.
 "L" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT	2X4
LESS THAN 11' 0"	2X4
GREATER THAN 11' 0"	2X6

+ REFER TO COMMON TRUSS DESIGN FOR PEAK SPLICE AND BEEL PLATES.

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BR3-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TP3 (TRUSS PANELS), 300 DUNSTON DR., SUITE 200, MOBILE, AL 36688 AND VITA (VOID TRUSS COLLAPSE) FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSS PANELS SHALL BE ATTACHED TO THE STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

JULIUS LEE'S
 CONS. ENGINEERS P.A.
 1455 BT 4th AVENUE
 DEERAN BEACH, FL 33441-8161

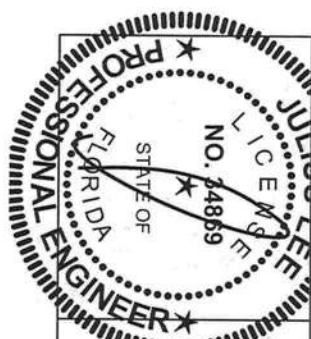
REVIEWED

By Julius Lee at 12:00 pm, Jun 11, 2008

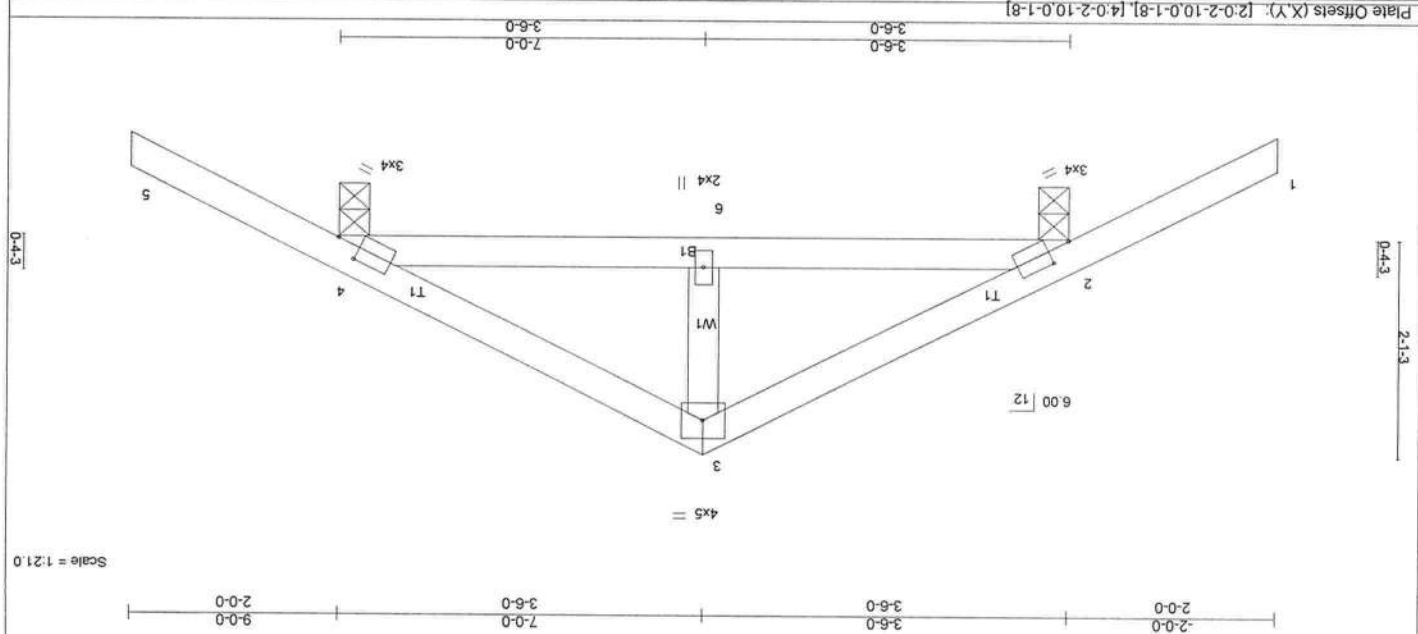
No. 34869
 STATE OF FLORIDA

MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCE7-02-CAB13015
 DATE 11/26/03
 DRWG NITEL STD CABLE 16 B ET
 -ENG



Job	Truss	132	COMMON	1	Ply	SPARKS - BARTH RES.	14204351
323087				2	Qty		
						Job Reference (optional)	
						7:140 S Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:31 2010 Page 1	



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d	PLATES	Weight: 31 lb
TCLL 20.0	Plates Increase 1.25	TC 0.37	Vert(TL) -0.00	2-6	>999	MT20	
TCCL 7.0	Lumber Increase 1.25	BC 0.07	Vert(TL) -0.01	4-6	>999	GRIP	244/190
BCLL 0.0	Rep Stress Incr YES	WB 0.03	Horz(TL) -0.00	4	n/a		
BCCL 5.0	Code FBC2007/TP12002	(Matrix)	Wind(TL) 0.01	4-6	>999		

LUMBER	TOP CHORD	2 X 4 SYP No.2	BOT CHORD	2 X 4 SYP No.2	WEBS	2 X 4 SYP No.3
REACTIONS	(lb/size)	2=331/0-3-8, 4=331/0-3-8	Max Horiz 2=71(LC 6)	Max Uplift 2=315(LC 6), 4=315(LC 7)	FORCES	(lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.
NOTES	(8-9)	1) Unbalanced roof live loads have been considered for this design.	2) Wind: ASCE 7-05; 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C	DOL=1.60	3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.	5) All bearings are assumed to be SYP No.2.

7) "Semi-rigid pitchbreaks including heels" Member and fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

9) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

Professional Engineer
STATE OF FLORIDA
No 34869
JULIUS S.K. LEE
LICENSE

January 25, 2010

Design void for use only with Mitek connectors. This design is based upon parameters shown, and is for an individual building component. Application of design parameters and proper incorporation of building design is responsibility of building designer, not Mitek designer. Strong shown for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/SPRI Quality Criteria, DSS-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

LOAD CASE(S) Standard

- (1) Unbalanced roof live loads have been considered for this design.
(2) Wind: ASCE 7-05; 10mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=18ft; Cal. Int. Exp. C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnp DOL=1.60
(3) Provide adequate drainage to prevent water ponding.
(4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
(5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
(6) All bearings are assumed to be SYP No.2.
(7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 416 lb uplift at joint 2 and 330 lb uplift at joint 8.
(8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
(9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
(10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
(11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

WEBB

- FORCES (nb) - Max. Comp/Max. Ten. - 1256/17, 3-4=-1256/17, 5-6=-695/45, 5-6=-291/307, 6-7=-362/290, TOP CHORD - 2-13=-212/764, BOT CHORD - 2-13=-1250/1438, 12-13=-1251/1436, 11-12=-854/1052, 11-14=-854/1052, WEBS - 3-12=-441/455, 10-15=-398/549, 9-15=-398/549, 7-9=-590/839, 5-9=-761/561,

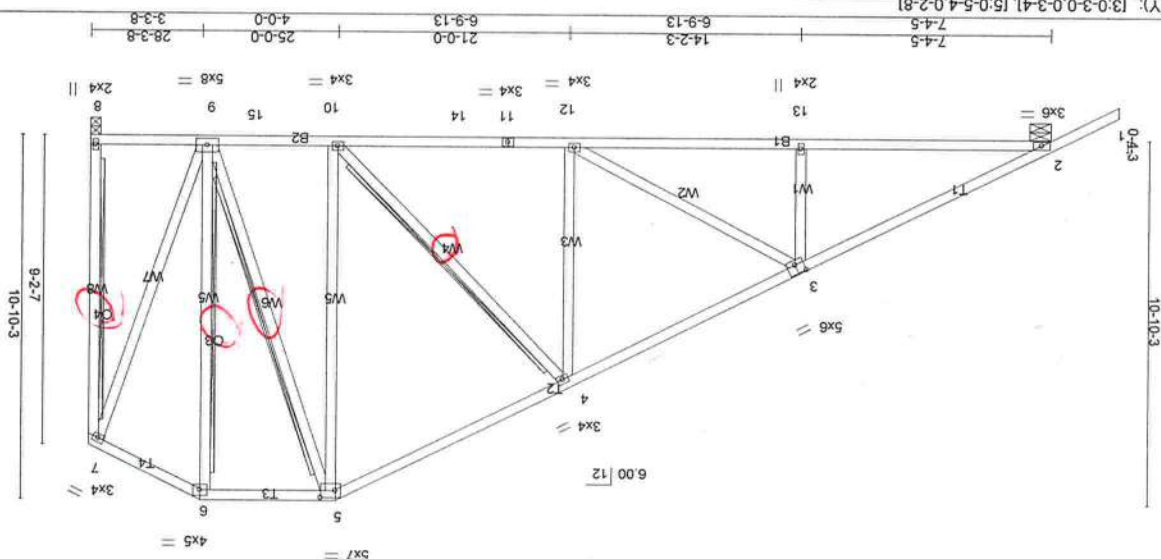
REACTIONS (lb/size) 2=1097/0-7-8, 8=1028/0-3-8
Max Horiz 2=447(LC 6)
Max Uplift 2=-416(LC 6), 8=-330(LC 6)

Miltek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

Structural wood sheathing applied or 4-5-9 oc purlins, except
and vertical.
-T:Rigid ceiling directly applied or 5-7-15 oc bracing.
-T:Rigid ceiling directly applied or 5-7-15 oc bracing.
2 X 4 SVP No. 3, 4-10, 5-9, 6-9, 7-8
-F:Fasten T and I braces to narrow edge of web with 10d Common wire
nails, 9in o.c. with 4in minimum end distance.
Brace must cover 90% of web length.

BRACING
TOP CHORD[illegible]

Plate Offsets (X,Y): [3:0-3:0-0-3-4],[5:0-5:4-0-2-8]



Scale: 3/16"=1'

323087	Job	Truss	T30	HIP	Qty	Ply	1	Job Reference (optional)	14204349
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A circular professional engineer seal for Julius S.K. Lee. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "JULIUS S.K. LEE" at the bottom, separated by two stars. The inner circle contains the text "STATE OF FLORIDA" at the top and "LICENSE" at the bottom, also separated by two stars. In the center, the license number "No 34269" is displayed. The seal features a clock-like scale with tick marks around the perimeter and two hands pointing to approximately 10:10.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-2=64/133, 2-3=666/40, 3-4=1995/134, 4-5=1374/4894, 5-6=1261/915.
BOT CHORD	6-7=1295/1030, 7-8=543/400, 8-9=543/400, 9-10=955/731
WEBS	3-15=1708/1832, 14-15=1708/1832, 12-17=618/775, 11-17=618/775
	4-14=734/822, 12-14=598/773, 7-11=422/396, 8-11=348/330,
	9-11=734/491

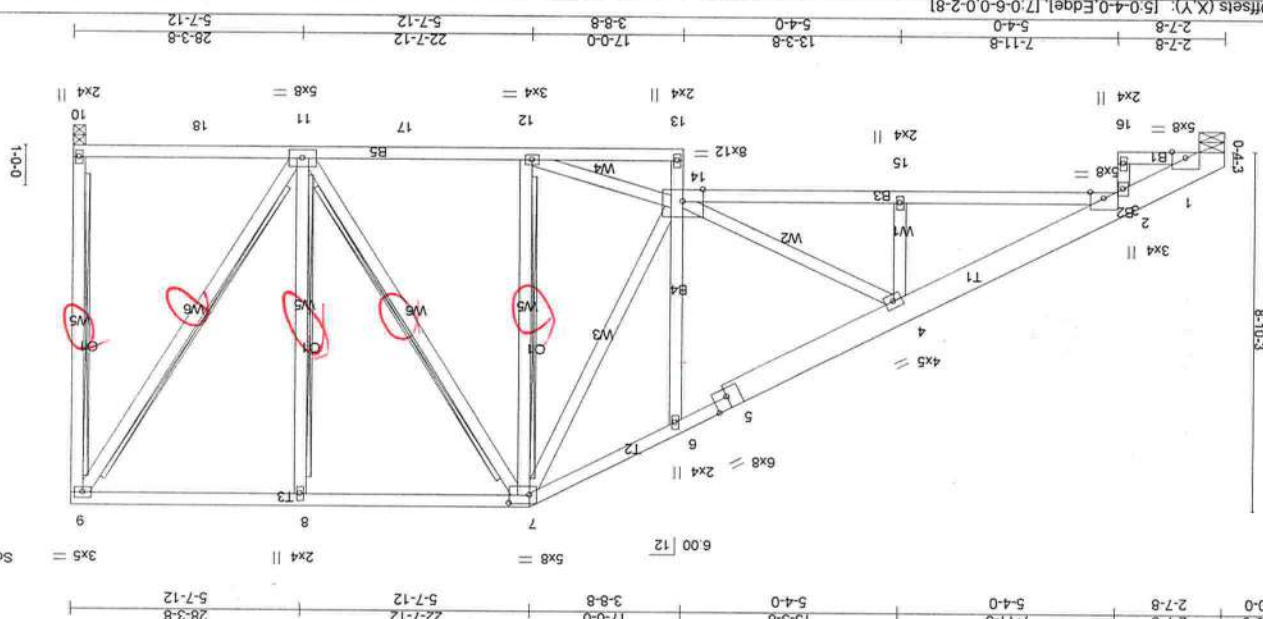
Structural wood sheathing directly applied or 5-1-8 oc purfins, except
and verticals.
T-Ridge: Rigid ceiling directly applied or 4-8-7 oc bracing.
T-Grace: 2 X 4 SYP No.3 - 9-10, 7-12, 7-11, 8-11, 9-11
F-fasten T and I branes to narrow edge of web with 10d Common wire
Brails must cover 90% of web length.
Brails, 9in o.c. with 4in minimum end distance.

BRACING
TOP CHORD
BOT CHORD
WEBS

LUMBER
 TOP CHORD 2 X 4 SYP No.2 *Except*
 T1: 2 X 8 SYP No.1D
 BOT CHORD 2 X 4 SYP No.2 *Except*
 B2,B4: 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

[illegible]

Plate Offsets (X,Y): [5:0-4:0,Edge], [7:0-6:0,0-2-8]



NOTES

(1) Wind: ASCE 7-05; 10mph (3-second gust); TCDF=4.2psf; BCDF=3.0psf; n=18ft; Cal. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone:C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gap DOL=1.60

(3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

(4) * This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDF = 5.0psf.

(5) All bearings are assumed to be SYP No.2.

(6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

(7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 262 lb uplift at joint 1 and 345 lb uplift at joint 8.

(8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

(9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

(10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

(11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

(12) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

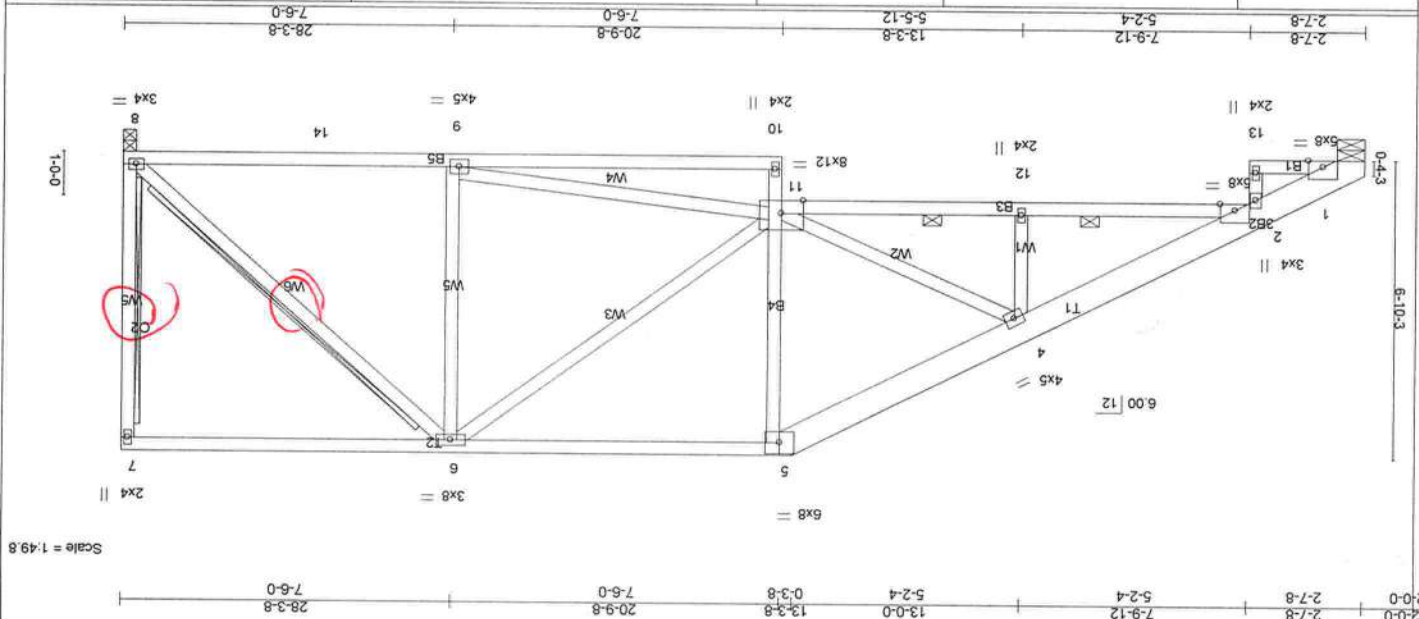
REACTIONS
Max Hozr t=265(L 6)
(b)ize=1-894/0-7-8
=965/0-3-8
FORCES (lb) - Max. Comp/Max. Ten. - All forces 250
TOP CHORD
1-2=825/L 2, 2-3=659/399, 3-4=19
1-2=1609/1788, 11-12=1609/1788
WEBS
4-11=736/763, 9-11=582/815, 6-11=

TOP CHORD 2 X 8 SYP No.1D *Except*
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACI
TOP CH
BOT CH
WEBS

Structural steel wood sheathing directly applied or 5-2-13 oc purlins, except
and verticals.
Rigid ceiling directly applied or 10-0-0 oc bracing. Except
8-0-9 oc bracing: 8-9
4-10-0 oc bracing: 3-11
T-Brace:
2 X 4 SYP No.3 - 7-8, 6-8
Fasten T and I braces to narrow edge of web with 10d Common wire
nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.
MiTek recommends that Stabilizers and required cross bracing
be installed during truss erection, in accordance with Stabilizer
Installation guide.

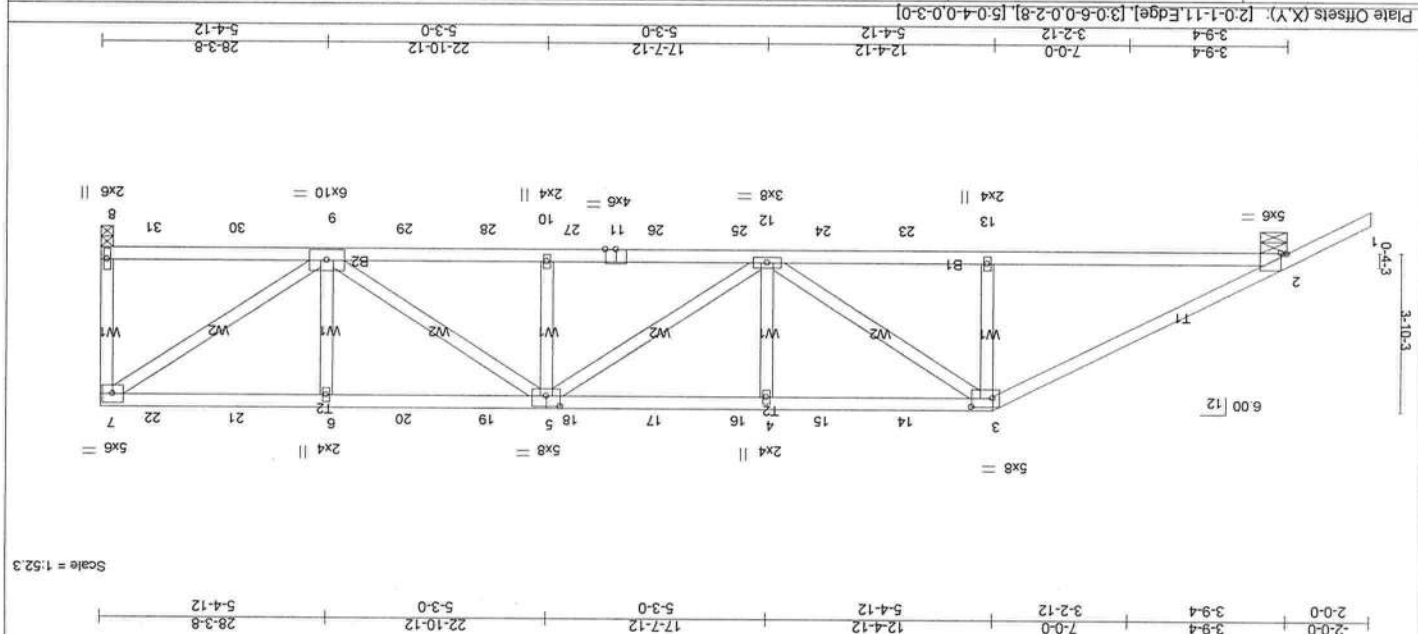
LOADING (psf)	TCLL	20.0	
	TCDL	7.0	
	BCLL	0.0	*
	BCDL	5.0	
LUMBERS			
SPACING	2-0-0		
	Plates Increase	1.25	
	Lumber Increase	1.25	
	Rep Stress Incr	YES	
	Code FBC2007/TF12002		
(Matrix)			
CSI	TC	0.58	
	BC	0.50	
	VWB	0.62	
DEFLECT			
	Wind(LL)	0.23	
	Horz(TL)	0.17	
	Vert(TL)	-0.24	
	Ver(LL)	-0.13	
	m (loc)	3-12	
	I/dell	>999	
	n/a	n/a	
	n/a	n/a	
	Weight: 188 lb		
	MT20	240	
	GRIP	244/190	
	PLATES		



A circular professional engineer seal for Julius S.K. Lem. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "JULIUS S.K. LEM" at the bottom, separated by two stars. The inner circle contains the text "STATE OF FLORIDA" at the top and "LICENSE No 34269" at the bottom. The center of the seal features a stylized graphic of a compass and a pencil.

Job	Truss	T24	SPECIAL	Qty	1	Ply	SPARKS - BARTH RES.	1A204343	Job Reference (optional)	7,140 S Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:25 2010 Page 1
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Job	Truss	123	MONO HIP	Qty	1	Ply	SPARKS - BARTH RES.	14204342
323087							Job Reference (optional)	
							7.140 S Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:25 2010 Page 1	



LOADING (psf)	SPACING	DEFL	BRACING	TOP CHORD	BOT CHORD
TCLL 20.0	2-0-0	in (loc)	Wind (LL) 0.23 10-12 >999	Structural wood sheathing directly applied or 2-7-13 oc purlins, except end verticals.	2 X 4 SYP No. 2
TCDL 7.0	Plates Increase 1.25	Vert (LL) -0.21 10-12 >999	Horz (TL) 0.12 8 n/a	Rigid ceiling directly applied or 5-0-10 oc bracing.	2 X 4 SYP No. 3
BCLL 0.0	Rep Stress Incr NO	Vert (TL) -0.42 10-12 >805	Wind (TL) 0.23 10-12 >999	Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.	
BCDL 5.0	Code FBC2007/TP12002	L/d	Weight: 148 lb		

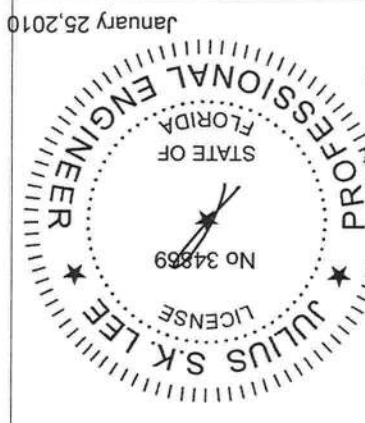
LUMBER	TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No. 2	2 X 4 SYP No. 2	2 X 4 SYP No. 2	2 X 4 SYP No. 3

REACTIONS	(lb/size)	8=1775/0-3-8, 2=1783/0-7-8	Max Horiz 2=205(C 5)	Max Uplift 8=819(C 4), 2=826(C 5)
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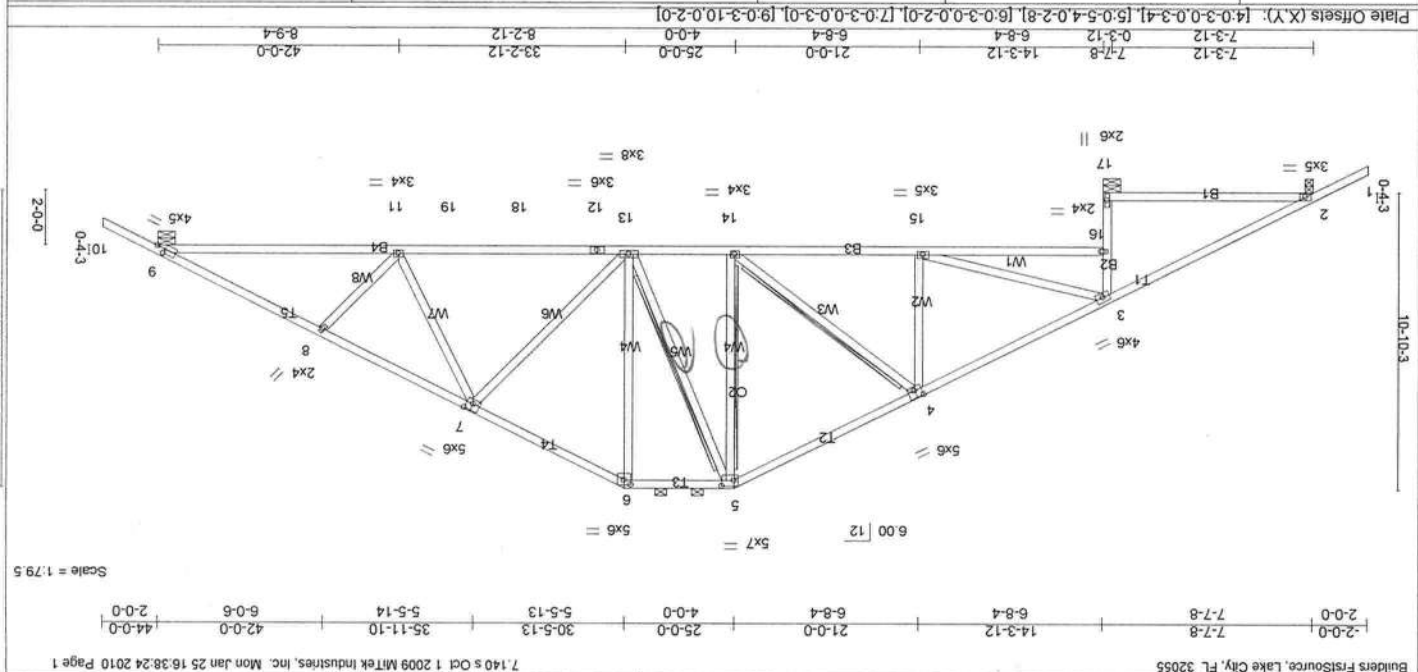
FORCES (lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.	TOP CHORD	BOT CHORD	WEBS
2-3=3173/1369, 3-14=3584/1655, 4-15=3583/1655, 4-16=3583/1655, 16-17=3583/1655, 17-18=3583/1655, 5-18=3583/1655, 5-19=2231/1030, 19-20=2231/1030, 6-20=2231/1030, 6-21=2231/1030, 21-22=2231/1030, 7-22=2231/1030, 7-8=-1713/841, 12-25=-1250/2725, 13-23=-1253/2742, 23-24=-1253/2742, 12-24=-1253/2742, 12-25=-1250/2725, 25-26=-1560/3381, 11-26=-1560/3381, 11-27=-1560/3381, 10-27=-1560/3381, 28-29=-1560/3381, 9-29=-1560/3381, 3-12=-538/1006, 4-12=-564/410, 5-10=0/317, 5-9=-1390/641, 6-9=-578/407, 7-9=-1224/2651			

NOTES	(10-11)
1) Wind: ASCE 7-05, 10 mph (3-second gust); TCDDL=4.2psf; BCDDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate gyp DOL=1.60	
2) Provide adequate drainage to prevent water ponding.	
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.	
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.	
5) All beams are assumed to be SYP No. 2.	
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 819 lb uplift at joint 8 and 826 lb uplift at joint 2.	
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.	
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 95 lb down and 93 lb up at 7-0-0, 95 lb down and 93 lb up at 9-0-12, 95 lb down and 93 lb up at 11-0-12, 95 lb down and 93 lb up at 13-0-12, 95 lb down and 93 lb up at 15-0-12, 95 lb down and 93 lb up at 17-0-12, 95 lb down and 93 lb up at 19-0-12, 95 lb down and 93 lb up at 21-0-12, 95 lb down and 93 lb up at 23-0-12, 95 lb down and 93 lb up at 25-0-12, and 93 lb down at 27-0-12 on bottom chord. The design/section of such connection device(s) is the responsibility of others.	
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).	

Continued on page 2



Job	323087	Truss	T22	SPECIAL	5	Ply	SPARKS - BARTH RES.	1	Job Reference (optional)	7/14/05 Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:24 2010 Page 1
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LOADING (psf)	SPACING	DEFL	in (loc)	l/def	L/d	PLATES	GRIP	MT20	Weight: 238 lb
TCLL 20.0	Plates Increase 1.25	TC 0.48	-0.21	11-13	>999	360			
TCDL 7.0	Lumber Increase 1.25	BC 0.54	-0.34	11-13	>999	240			
BCDL 5.0	Rep Stress Incr YES	WB 0.62	0.06	9	n/a	240			

LUMBER	TOP CHORD	BOT CHORD	WEBS
2 X 4 SYP No. 2	2 X 4 SYP No. 2 "Except"	2 X 4 SYP No. 3	2 X 4 SYP No. 3

Structural wood sheathing directly applied or 4'-6" oc purlins, except 2'-0" oc purlins (5'-9"-14" max.): 5-6.
Rigid ceiling directly applied or 4'-10-9" oc bracing.
T-Brace: 2 X 4 SYP No. 3 - 4'-14", 5'-14", 5'-13".
Fasten T and I braces to narrow edge of web with 10d Common wire nails, sin o.c. with 4in minimum end distance.
Brace must cover 90% of web length.
Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
(lb/size) 2-322/0-3-8, 17=1399/0-7-8, 9=1283/0-7-8	Max Horiz 2=253(LC 6) Max Uplift 2=273(LC 6), 17=611(LC 6), 9=503(LC 7) Max Gray 2=326(LC 10), 17=1399(LC 1), 9=1283(LC 1)	3-4=-1415/1100, 4-5=-1300/1096, 5-6=-1159/1093, 6-7=-1365/1140, 7-8=-1937/1412, 8-9=-2139/1475 16-17=-1355/989, 3-16=-1322/1005, 14-15=-651/1193, 13-14=-420/1084, 12-13=-809/1529, 12-18=-809/1529, 18-19=-809/1529, 11-19=-809/1529, 9-11=-1086/1821, 3-15=-666/1260, 4-14=-197/297, 5-13=-88/307, 6-13=-279/343, 7-13=-537/522, 7-11=-167/355, 8-11=-219/299		

- NOTES (11-12)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCFL=4.2psf; BCFL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members, with BCFL = 5.0psf.
 - All bearings are assumed to be SYP No. 2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 273 lb uplift at joint 2, 611 lb uplift at joint 17 and 503 lb uplift at joint 9.
 - *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

Continued on page 2

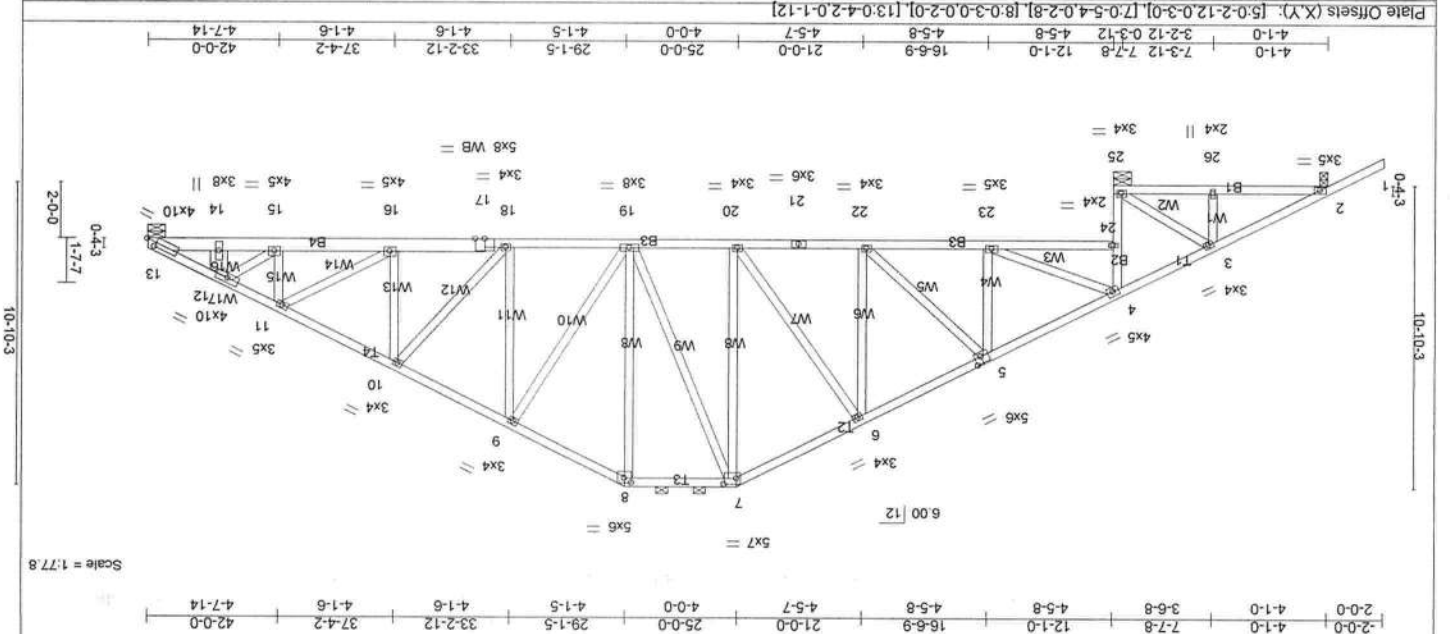


A circular clock face with a black border. The text "PROFESSIONAL ENGINEER" is written in a semi-circle at the top, and "JULIUS S.K. LEF" is written in a semi-circle at the bottom. The text "FLORIDA" is at the top, "STATE OF" is below it, and "LICENSE" is at the bottom. The text "No 34899" is in the center. The clock has two hands: a shorter hour hand pointing to 10 and a longer minute hand pointing to 2. There are two stars on the left and right sides of the clock face.

REACTIONS	(lb/size)	13=2554/0-7-8, 2=3310/3-8, 25=1459/0-7-8
Max Horiz	2=271(LC 5)	
Max Uplift	13=963(LC 6), 2=152(LC 5)	
Max Grav	13=2554(LC 1), 2=336(LC 9), 25=1459(LC 1)	
FORCES (lb) -	Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD	4-5=1341/433, 5-6=1493/506, 6-7=1395/526, 7-8=1294/550, 8-9=1492/578,	
	9-10=1298/712, 10-11=22512/900, 11-12=35214/1296, 12-13=4686/1759	
BOT CHORD	2-4-25=1339/504, 4-24=1308/513, 22-23=360/1147, 21-22=354/1289, 20-21=354/1289,	
	19-20=258/1197, 18-19=413/1680, 17-18=666/2204, 16-17=666/2205,	
	15-16=1097/1150, 14-15=1508/4072, 13-14=1508/4072	
WEBS	4-23=4011/301, 5-23=434/178, 7-19=147/353, 8-19=164/434, 9-19=77/1379,	
	9-18=235/568, 10-18=769/371, 10-16=223/559, 11-16=1088/493, 11-15=296/782,	
	12-15=1096/479, 12-14=498/1226	

TOP CHORD	Structural wood sheathing directly applied or 2-8 oc purlins, except 2-0-0 oc purlins (5-5-10 max.); 7-8.
BOT CHORD	Rigid ceiling directly applied or 4-8-10 oc bracing.

LOADING (psf)	TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 5.0	SPACING 2-0-0	Code FBC2007/TP12002	CSi (Matrix)	Wind(LL) 0.11 Horz(TL) 0.06 Vent(TL) -0.23 Vent(LL) -0.12 DEFL in (loc) 16 L/d/ell 240	Weight: 288 lb
			Rep Stress Incr NO Lumber Increase 1.25 Plates Increase 1.25	WB 0.76 BC 0.76 TC 0.36	16 13 16-18 18 16 n/a	MT20 GRIP 244/190
						PLATES



Job	T19	SPECIAL	Qty	Ply	SPARKS - BARTH RES.	14204340
323087	T19		1			
Builders FirstSource, Lake City, FL 32055	7/14/05 Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:23 2010 Page 1					

NOTES (12-14)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05, 110mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) Extended(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gnl DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) All bearings are assumed to be SYP No.2.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 303 lb uplift at joint 10, 410 lb uplift and 655 lb uplift at joint 18.
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

NOTES (12-14)

TOP CHORD
FORCES (lb) - Max. Comp./Max. Ten. - all forces ≥ 250 (lb) or less except when shown.
9-10=-1805/1368
20-21=-284/499, 16-21=-284/499, 15-16=-261/639, 14-15=-261/639, 13-14=-582/911,
9-11=-177/273, 10-11=-1147/1570
WEBS
3-19=-321/216, 3-18=-439/799, 5-17=-970/548, 5-16=-344/385,
11-13=-546/868, 8-11=-702/811

REACTIONS (lb/size) 10=869/Mechanical, 2=469/0-3-8, 18=1409/0-7-8
Max Horiz 2=303(LC 6)
Max Uplift 10=303(LC 7), 2=410(LC 6), 18=655(LC 6)
Max Gray 10=869(LC 1), 2=473(LC 10), 18=1409(LC 1)

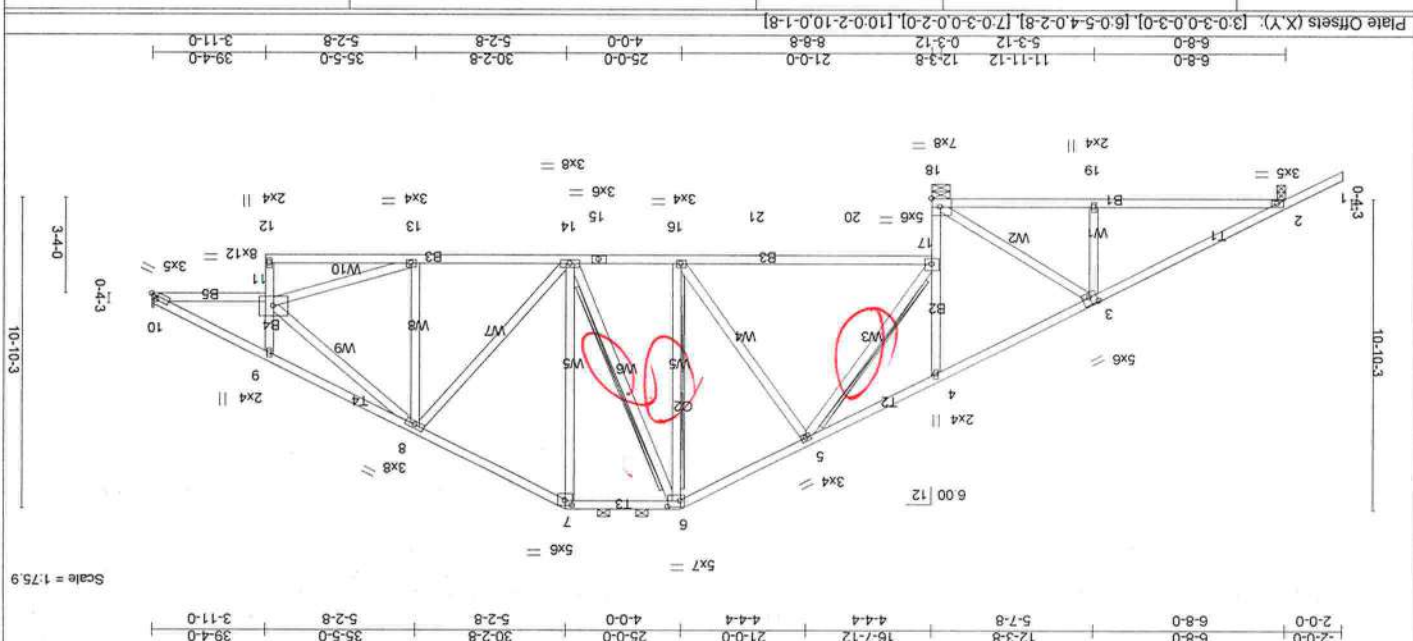
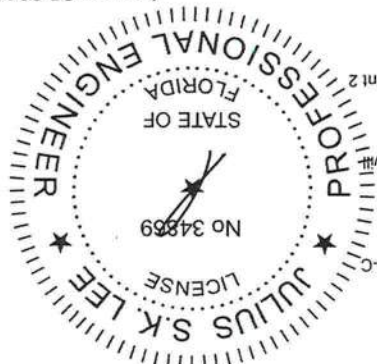
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except
B2,B4: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 4-7-T oc purlins, except
2-20 oc purlins (6-0-0 max.); 6-7;
T-Trace: Rigid ceiling directly applied or 4-10-1 oc bracing
2 X 4 SYP No.3 - 1-17, 6-16, 6-14
Falls: T and I Traces to narrow edge of web with 10d Common wire
nails, 9in o.c. with 4in minimum end distance.
Brace must cover 90% of web length.

Miltek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

January 25, 2010



A circular professional engineer seal for Julius S.K. Lee. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "JULIUS S.K. LEE" at the bottom, separated by two stars. The inner ring contains "STATE OF FLORIDA" at the top and "LICENSE" at the bottom. In the center, the text "No 34869" is displayed above a stylized signature of "Julius S.K. Lee". The seal is surrounded by a series of short radial lines.

NOTES (12-13)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 10mph (3-second gust); TCFL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); porch left exposed; Lumber DCL=1.60 plate gip DCL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SYP No.2.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1594 lb uplift at joint 13, 321 lb uplift at joint 2 and 905 lb uplift at joint 23.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated loads (s) 1570 lb down and 1413 lb up at 39'-5-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) Member on additional permanent and stability bracing for truss system (not part of this component design) is always required.

NOTES (12-13)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	TOP CHORD	BOT CHORD	WEBS
2-30=300/231, 4-5=377/187, 5-6=505/177, 6-7=795/368, 7-8=850/468.	8-9=300/1488, 9-10=1442/698, 10-11=2025/1031, 11-12=3048/1831,	12-13=4255/2914	12-15=1141/1081
			10-18=768/561, 10-16=427/599, 11-16=1098/912, 11-15=670/792, 12-14=1137/1249,
			6-20=2034/466, 7-20=338/188, 7-19=303/528, 8-19=728/504, 9-18=372/575,
			3-24=2662/271, 4-24=277/226, 4-22=461/1420, 5-21=475/1197, 6-21=741/1337,
			15-16=1581/2277, 14-15=2516/3695, 13-14=2636/3695
			20-21=324/403, 19-20=1105/653, 18-19=401/1246, 17-18=783/1769, 16-17=783/1769,
			2-25=277/214, 24-25=277/214, 22-23=1653/863, 5-22=1478/670, 21-22=482/453,

REACTIONS (lb/size) 13=2330/0-7-8, 2=345/0-3-8, 23=1670/0-7-8
Max Horz 2=271(LC 5)
Max Uplift 13=-1594(LC 6), 2=-321(LC 5), 23=-905(LC 5)
Max Gray 13=2330(LC 1), 2=378(LC 9), 23=1670(LC 1)

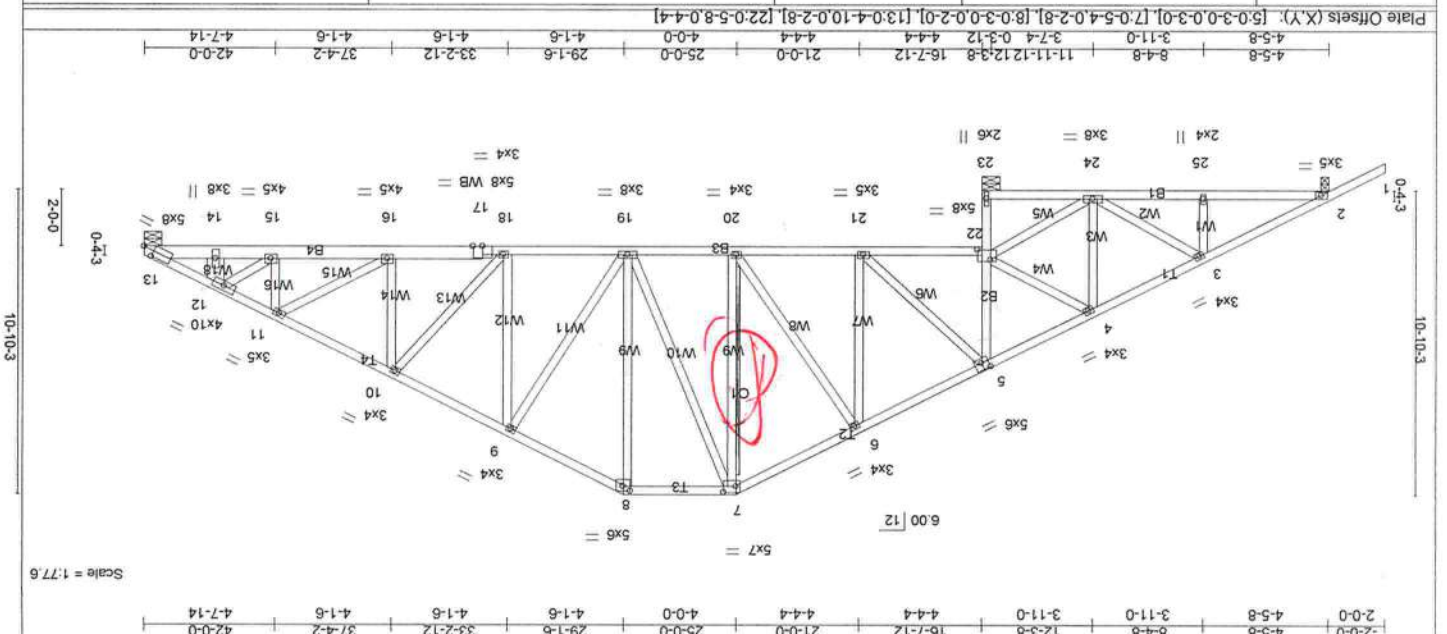
REACTIONS

TOP CHORD 2 X 4 SYP No.2
LUMBER
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3, B4; 2 X 6 SYP No.1D
W18: 2 X 8 SYP No.1D

BRACING
TOP CHORD
BOT CHORD
WEBS

MilTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

Trace must cover 90% of web length.
T-trace: T and I braces to narrow edge of web with 10d Common wire
2 X 4 SYP No.3, 7-20
-Brace:
rigid ceiling directly applied or 4-6d oc bracing.
Structural wood sheathing directly applied or 2-10-9 oc purlins.

[illegible]

A circular professional engineer seal for Julius S.K. Lem. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "JULIUS S.K. LEM" at the bottom, separated by two stars. The inner circle contains the text "STATE OF FLORIDA" at the top and "LICENSE" at the bottom. In the center, the text "No 34869" is displayed above a star. Two hands are shown: a longer hand pointing towards the bottom-left and a shorter hand pointing towards the top-right.

NOTES (10-11)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 10 mph (3-second gust); TCDD=4.2psf; BCD=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.66 plate gfp

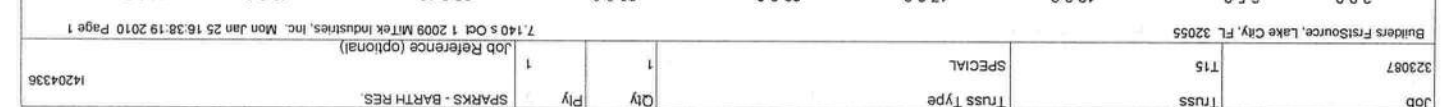
BOT CHORD 2-18=-669/352, 17-18=-661/350, 16-17=-1238/696, 4-16=-1218/703, 15-19=-198/650, 14-19=-198/650, 13-14=-491/1172, 12-20=-491/1172, 11-12=-868/1535, 9-11=-868/1535

Max Grav 2=486(LC 10), 17=1476(LC 1), 9=1134(LC 1)

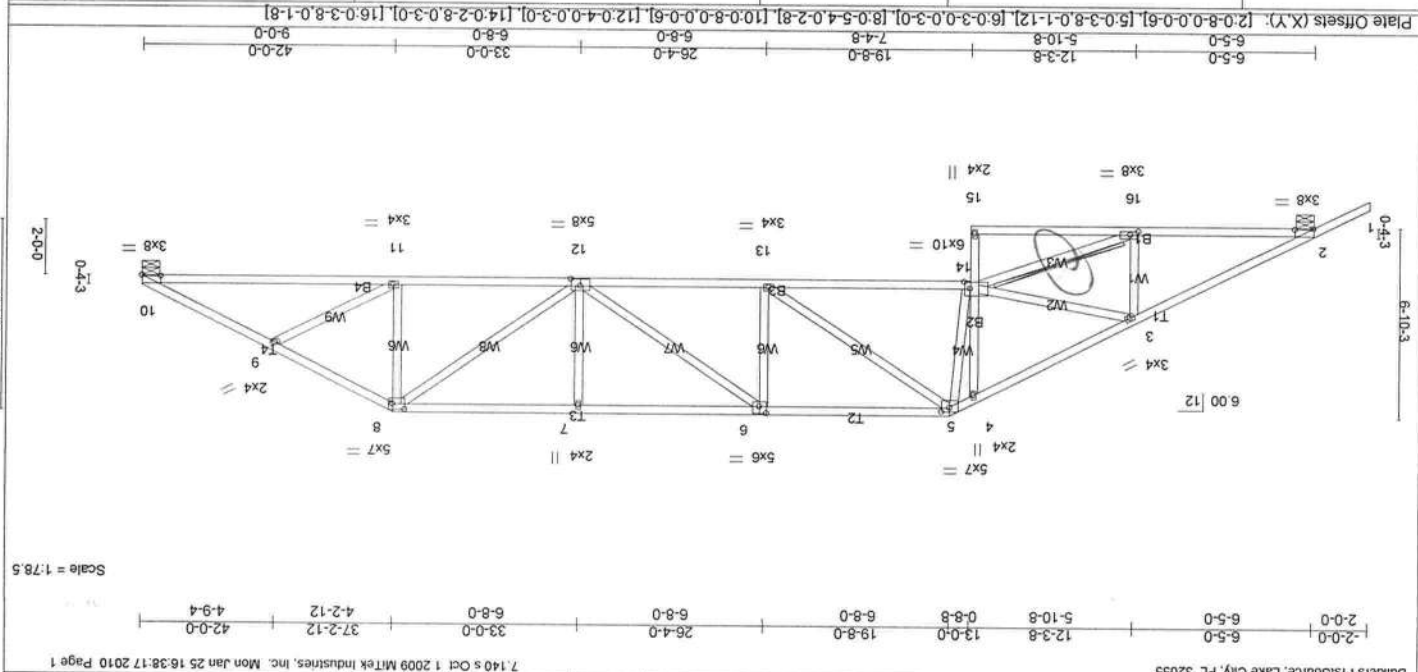
Structural wood sheathing directly applied or 4-6-0 oc purlins.
-T-Brace:
2 X 4 SYP No. 3 - 7-14
-F-fasten T and I braces to narrow edge of web with 10d Common wire
Brace must cover 90% of web length.

BRACING
TOP CHORD
BOT CHORD
WEBS

6-5-0	5-8-12	11-1-12	12-8-8	17-0-0	23-0-0	29-0-0	6-0-0	6-2-12	6-9-4	42-0-0
Plate Offsets (X,Y): [3-0-3-0-0-3-0], [5-0-5-4-0-2-8], [7-0-5-4-0-2-8]										



Job	323087	Truss	T13	SPECIAL	Qty	1	Ply	SPARKS - BARTH RES.	Job Reference (optional)	7.140 5 Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:17 2010 Page 1
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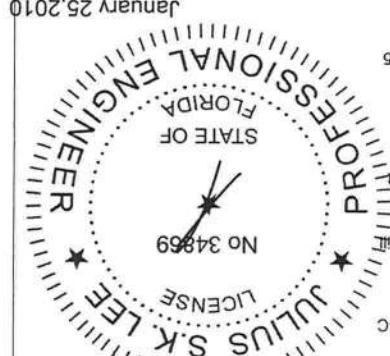


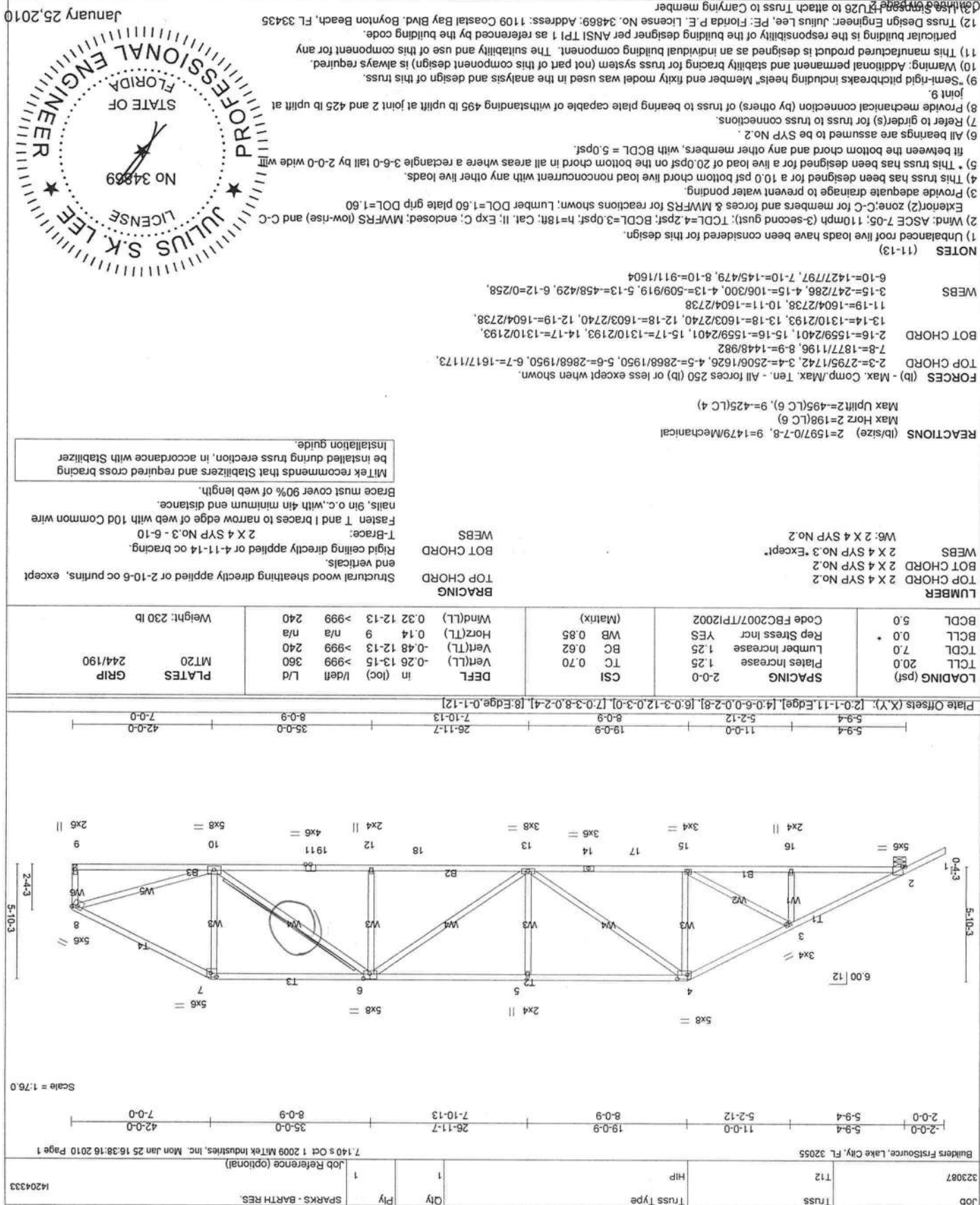
LOADING (psf)	SPACING	CS	DEFL	BRACING	LUMBER	REACTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
20.0	2.0-0	TC 0.51	in (loc)	Wind(LL) 0.46	BCDL 5.0	10=1321/0-7.8, 2=1452/0-7.8	(b) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.	2 X 4 SYP No. 2	2 X 4 SYP No. 2	2 X 4 SYP No. 3
7.0	1.25	BC 0.61	Horz(TL) 0.23	Horz(TL) 0.23	BCL 0.0	Max Horiz 2=210(LC 6)		B2. 2 X 4 SYP No. 3	2 X 4 SYP No. 3	2 X 4 SYP No. 3
0.0	YES	WB 0.79	Vert(TL) -0.60	Vert(TL) -0.60	Rep Stress Incr	Max Uplift 10=-388(LC 4), 2=-513(LC 6)		2 X 4 SYP No. 2	2 X 4 SYP No. 2	2 X 4 SYP No. 3
5.0	2.0-0		in (loc)	Wind(LL) 0.46	Code FBC 2007/TP12002			2 X 4 SYP No. 3	2 X 4 SYP No. 3	2 X 4 SYP No. 3

Structural wood sheathing directly applied or 3'-2'-12 oc purlins.
T-Brace: 2 X 4 SYP No. 3 - 14-16
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance.
Brace must cover 90% of web length.
Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

NOTES (10-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TC DL=4.2psf; BC DL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate gfp DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6'-0 tall by 2'-0'-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SYP No. 2.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 388 lb uplift at joint 10 and 513 lb uplift at joint 2.
8) "Semi-rigid pitchbreaks including heels" Member and fixity model was used in the analysis and design of this truss.
9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer during construction. The responsibility of the building designer for general guidance regarding fabricator, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, B58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.

LOAD CASE(S) Standard
(11) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435
particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
(10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer during construction. The responsibility of the building designer for general guidance regarding fabricator, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, B58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719.





Job	Truss	T10	SPECIAL	1	Qty	Ply	SPARKS - BARTH RES.	14204331
2 Job Reference (optional)								

Builders FirstSource, Lake City, FL 32055

NOTES (13-15)

7) * This truss has been designed for a live load of 20 kpsft on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

other members.

9) Refer to girder(s) for truss connections.

10) Provide mechanical connection (by others)

11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

(10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1181 lb uplift at joint 2 and 1245 lb uplift at joint 12.

[illegible]

1.3) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

[illegible]

15) Use Simpson HGU26-2 to attach Truss to Carrying member

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: $1-3=-54, 3-9=-54, 9-10=-54, 10-11=-54, 11-12=-10$

Concentrated Loads (lb)

Ver: 3-95(B) 9-95(B) 21-32(B) 6-95(B) 17-29(B) 13-29(B) 22-95(B) 23-95(B) 24-95(B) 25-95(B) 26-95(B) 27-95(B) 28-95(B) 29-95(B) 30-95(B) 31-95(B) 32-95(B) 33-95(B) 35-95(B) 36-29(B) 37-29(B) 38-29(B) 39-29(B) 40-29(B) 41-29(B) 42-29(B) 43-29(B) 44-29(B) 45-29(B) 46-29(B) 47-29(B) 48-29(B) 49-29(B) 50-177(B)



January 25, 2010

WARNING - Verify design parameters and read notes on this and included nitter reference page MII 7473 before use. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not, in any designer, brocking shown for listed support of individual web members only. Additional temporary brocking to insure stability during construction is the responsibility of the erector. Additional permanent brocking of the overall structure is the responsibility of the building designer for general guidance regarding erection, quality control, storage, delivery, erection and brocking, consult www.mitek.com or Mitek Customer Service at 1-800-368-5883. **Safe Information** available from Truss Plate Institute, Inc., 583 O'Donnell Drive, Madison, WI 53719. **ANSI/P101 Quality Criteria, D58-89 and BC511 Building Component**

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	COMMON	Qty	1	Ply	SPARKS - BARTH RES.	14204330
323087	T09					2	Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-2=-89(F=-35), 2-3=-89(F=-35), 1-3=-10
 Concentrated Loads (lb)
 Vert: 5=-859(F) 6=-859(F) 7=-859(F)



January 25, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M11-7473 BEFORE USE.
 Design void for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of the designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding erection, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-87 and ECSI Building Component** available from Truss Plate Institute, 583 D'Oncio Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	T08	HIP	Qty	Ply	SPARKS - BARTH RES.	14204329
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Builders FirstSource, Lake City, FL 32055

Job Reference (optional)
7.140 s Oct 1 2009 Mittek Industries, Inc. Mon Jan 25 16:38:12 2010 Page 2

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=-54, 3-4=-54, 4-5=-54, 2-5=-10
 Concentrated Loads (lb)
 Vert: 3=-37(B) 4=-37(B) 8=-117(B) 6=-117(B) 7=-2688(F) 9=-37(B) 10=-14(B) 11=-1316(F) 12=-1469(F)

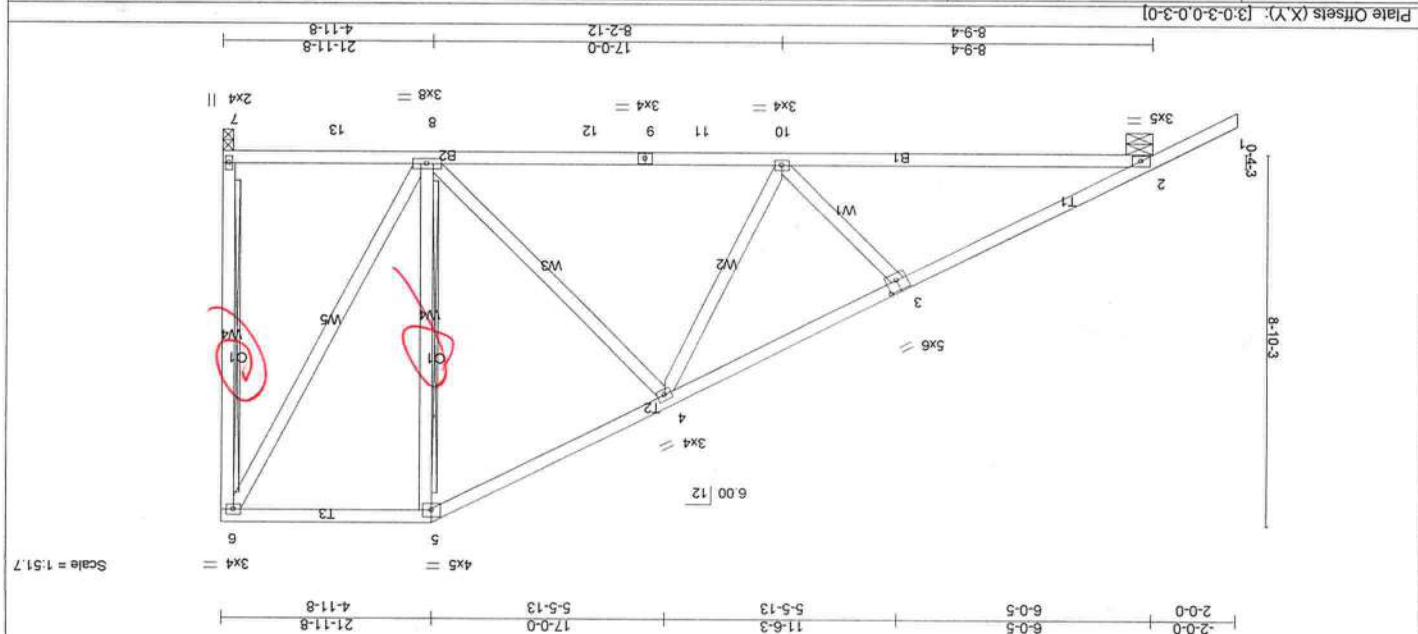


January 25, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer, brocking shown is for lateral support of individual webs and members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-87 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Oroville Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	107	MONO HIP	Qty	1	Ply	SPARKS - BARTH RES.	1	Job Reference (optional)	7.140 s Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:12 2010 Page 1	14204328
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LOADING (psf)	SPACING	CSI	DEFL	BRACING	PLATES	GRIP
TCLL 20.0	2.0-0	TC 0.42	in (loc)	Wind(LL) 0.07	MT20	Weight: 137 lb
TCDL 7.0	Lumber Increase 1.25	BC 0.42	in (loc)	Horz(TL) 0.03		
BCLL 0.0	Rep Stress Incr YES	WB 0.95	in (loc)	Vert(TL) -0.21		
BCDL 5.0	Code FBC2007/TP12002		in (loc)	Wind(LL) 0.07		

LUMBER	TOP CHORD	2 X 4 SYP No. 2	TOP CHORD	2 X 4 SYP No. 2	WEBS	2 X 4 SYP No. 3
REACTIONS	(lb/size)	7=832/0-3-0, 2=873/0-7-8	Max Horiz 2=407(LC 6)	Max Uplift 7=277(LC 6), 2=339(LC 6)		
FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.					
TOP CHORD	2-3=-1272/600, 3-4=-1064/535, 4-5=-471/238, 5-6=-354/280, 6-7=-760/616					
BOT CHORD	2-10=-961/1058, 10-11=-653/732, 9-12=-653/732, 8-12=-653/732					
WEBS	3-10=-242/329, 4-10=-199/377, 4-8=-541/531, 6-8=-578/729					

Mitek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

Structural wood sheathing directly applied or 5-5-5 oc purlins, except end verticals.
Rigid ceiling directly applied or 6-4-12 oc bracing.
T-Brace:
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c. with 4in minimum end distance.
Brace must cover 90% of web length.

- NOTES** (9-10)
- 1) Wind: ASCE 7-05: 110mph (3-second gust); TC DL=4.2psf; BC DL=3.0psf; h=18ft; Cal. II; Exp C; enclosed; MWFRS (low-rise) and C-C
 - 2) Exterior(2) zone-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BC DL = 5.0psf.
 - 5) All bearings are assumed to be SYP No. 2.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 7 and 339 lb uplift at joint 2.
 - 7) *Semi-rigid pitchbreaks including heels' Member end kxly model was used in the analysis and design of this truss.
 - 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

PROFESSIONAL ENGINEER
STATE OF FLORIDA
JULIUS S.K. LEE
LICENSE No. 34869

January 25, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erection, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 Quality Criteria, D58-87 and BCS1 Building Component

available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.	14204326
323087	T05	MONO HIP	1		Job Reference (optional)	

Builders FirstSource, Lake City, FL 32055

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LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (p/l)

Vert: 1-4=-54, 4-6=-54, 2-11=-10, 11-12=-70(F=-60), 12-13=-110(F=-60), 13-14=-70(F=-60), 14-15=-10, 7-15=-50

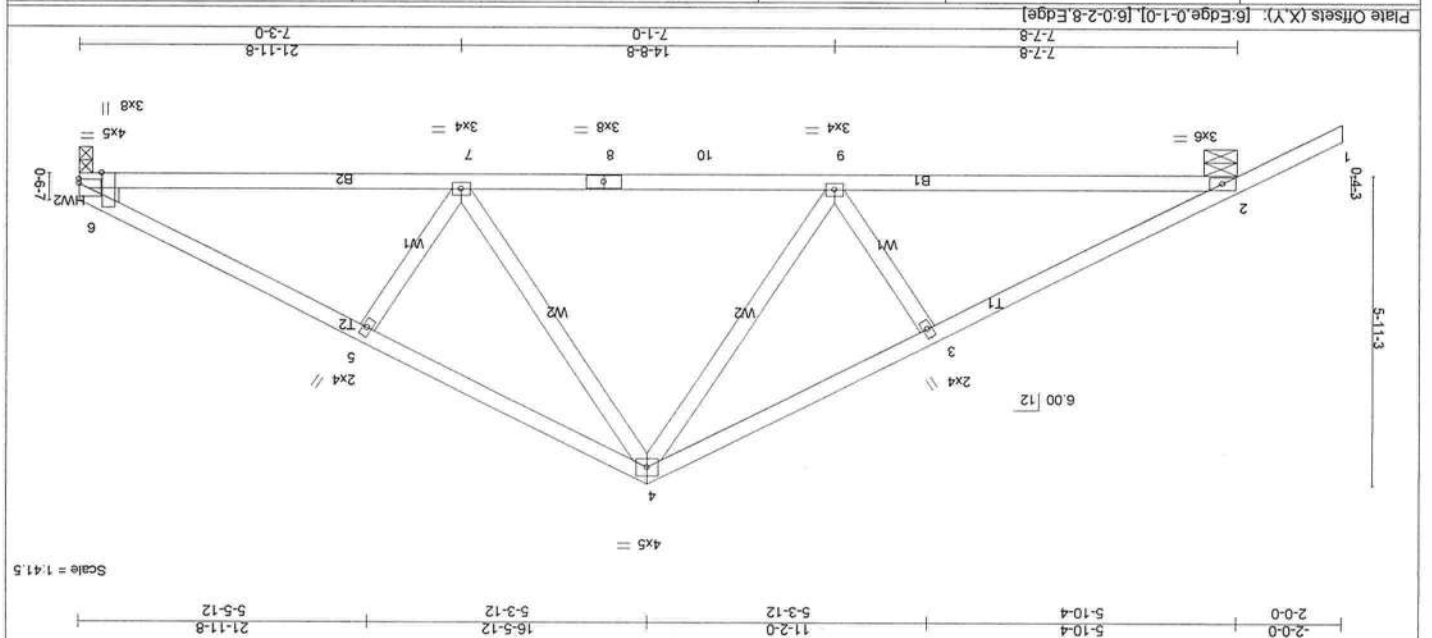


January 25, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component**
 available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	Truss Type	Qty	Ply	SPARKS - BARTH RES.	Job Reference (optional)	Builders FirstSource, Lake City, FL 32055	7.140 s Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:10 2010 Page 1
323087	T04	COMMON	3		1			



LOADING (psf)	SPACING	DEFL	BRACING	LOADING (psf)
TCLL 20.0	Plates Increase 1.25	Vert(TL) -0.29	in (loc)	TCLL 20.0
TCCL 7.0	Lumber Increase 1.25	Horz(TL) 0.05	n/a	TCCL 7.0
BCCL 0.0	Rep Stress Incr NO	Vert(TL) -0.42	7-9 >620	BCCL 0.0
BCDL 5.0	Code FBC2007/TFP12002	Wind(TL) 0.22	7-9 >999	BCDL 5.0

LUMBER	TOP CHORD 2 X 4 SYP No.2	BOT CHORD 2 X 4 SYP No.2	WEBS	WEDGE	Right: 2 X 4 SYP No.3
REACTIONS (lb/size)	2=1068/0-7.8, 6=934/0-3.0	Max Horiz 2=145(LC 6)	Max Uplift 2=418(LC 6), 6=280(LC 7)	FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	TOP CHORD 2-3=-1737/1076, 3-4=-1579/1069, 4-5=-1583/1107, 5-6=-1738/1114

NOTES	(9-10)
1) Unbalanced roof live loads have been considered for this design.	2) Wind: ASCE 7-05: 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C

LOAD CASE(S) Standard	Uniform Loads (psf)	1) Regular: Lumber Increase=1.25, Plate Increase=1.25
10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435		

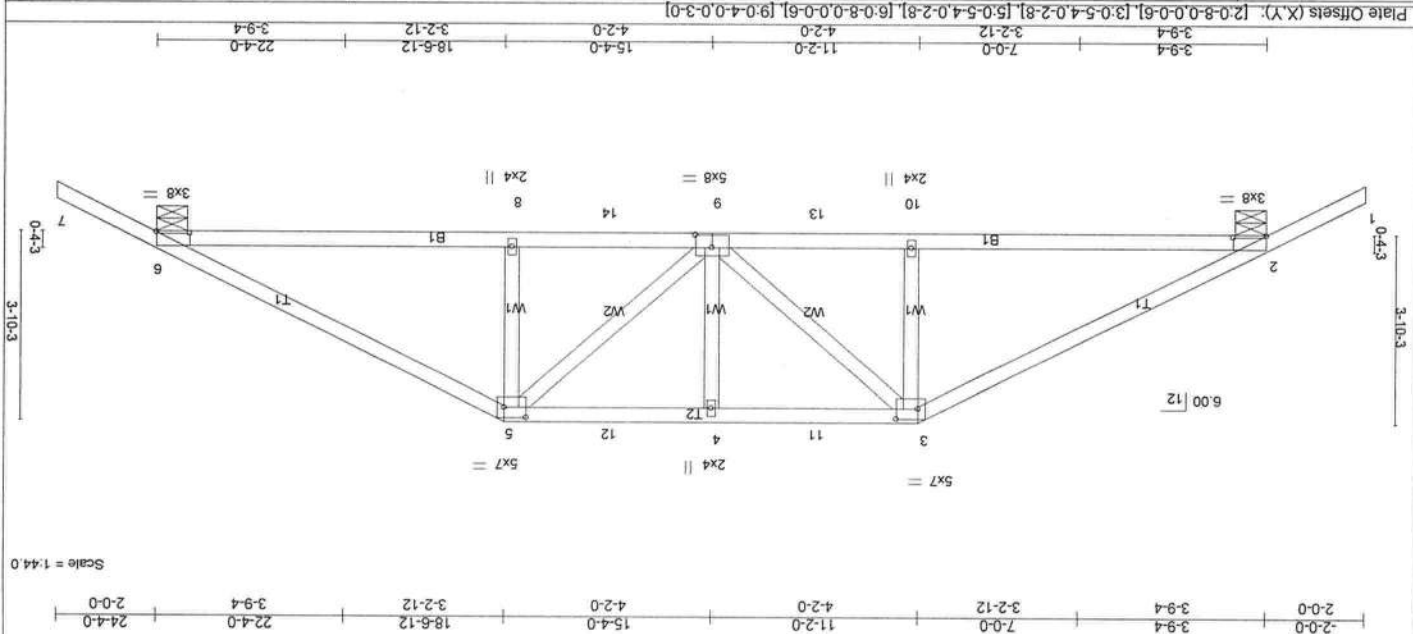
LOAD CASE(S) Standard	Uniform Loads (psf)	1) Regular: Lumber Increase=1.25, Plate Increase=1.25
10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435		

LOAD CASE(S) Standard	Uniform Loads (psf)	1) Regular: Lumber Increase=1.25, Plate Increase=1.25
10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435		

LOAD CASE(S) Standard	Uniform Loads (psf)	1) Regular: Lumber Increase=1.25, Plate Increase=1.25
10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435		

Job	1	Truss Type	1	Qty	1	Ply	SPARKS - BARTH RES.	14204323
	T02		HIP					
Builders FirstSource, Lake City, FL 32055 7,140 S Oct 1 2009 MITek Industries, Inc. Mon Jan 25 16:38:09 2010 Page 1								

Job	Truss	T01	323087
Truss type	HIP	1	
Qty	1		
Ply			
SPARKS - BARTH RES.			
Job Reference (optional)			7,140 S Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:08 2010 Page 1
14204322			



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.48	Ver(LL) -0.11	9	>999	360	
TCDL 7.0	Lumber Increase 1.25	BC 0.54	Ver(TL) -0.21	9	>999	240	
BCLL 0.0	Rep Stress Incr NO	WB 0.18	Horz(TL) 0.08	6	n/a		
BCDL 5.0	Code FBC2007/TP12002	(Matrx)	Wind(LL) 0.11	9	>999	240	Weight: 105 lb

LUMBER	BRACING	RECTIONS	FORCES	TOP CHORD	BOT CHORD	WEBS
TOP CHORD 2 X 4 SYP No.2	TOP CHORD	Max Horz 2=97(LC 5)	(lb) - Max. Comp/Max. Ten. - All forces 250 (lb) or less except when shown.	2-10=-879/2151, 10-13=-882/2168, 9-14=-849/2168, 8-14=-849/2168, 5-6=-2525/1057	2-10=-879/2151, 10-13=-882/2168, 9-14=-849/2168, 8-14=-849/2168, 5-6=-2525/1057	3-10=-96/518, 3-9=-306/564, 4-9=-531/392, 5-9=-306/564, 5-8=-96/518

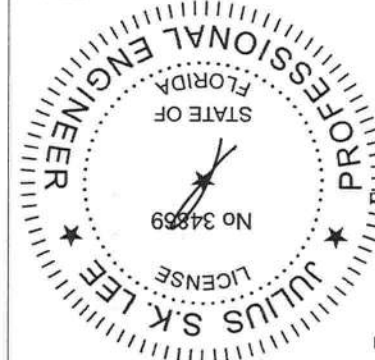
Max Uplift 2=720(LC 5), 6=720(LC 6)
Max Horz 2=97(LC 5)
2=1482/0-7-8, 6=1482/0-7-8

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05: 110mph (3-second gust). TCCL=4.2psf; BCCL=3.0psf; h=18ft; Cal. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) All bearings are assumed to be SYP No.2.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 720 lb uplift at joint 2 and 720 lb uplift at joint 6.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 95 lb down and 93 lb up at 7-0-0, 95 lb down and 93 lb up at 11-0-12, and 95 lb down and 134 lb up at 7-0-0, 63 lb down at 9-0-12, 126 lb down at 11-0-12, and 63 lb down at 13-3-4, and 321 lb down and 134 lb up at 15-3-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer. For general guidance regarding application of design parameters and proper incorporation of component responsibility of building designer, not truss designer, bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erection, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Oroffo Drive, Madison, WI 53719.

LOAD CASE(S) Standard
12) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Continued on page 2

January 25, 2010



Job	Truss	HJ07	MONO TRUSS	Qty	4	Ply	SPARKS - BARTH RES.	14204320
Job Reference (optional)								

Builders FirstSource, Lake City, FL 32055

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LOAD CASE(S) Standard

Uniform Loads (plf)

Concentrated Loads (lb)

Vert: 8=76(F=38, B=38) 9=84(F=42, B=42) 10=-74(F=-37, B=-37) 11=10(F=5, B=5) 12=-7(F=-3, B=-3) 13=-27(F=-13, B=-13)

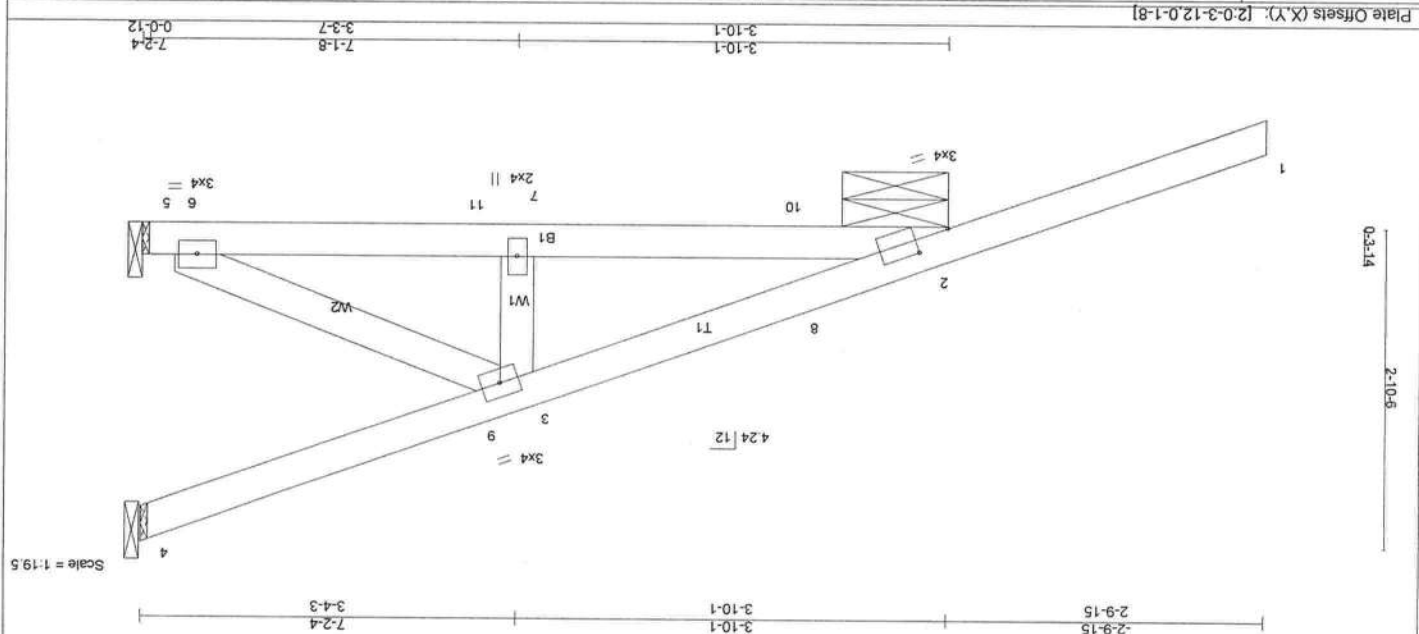


January 25, 2010

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE M11-7473 BEFORE USE.
 Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the fabricator. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Ondell Drive, Madison, WI 53719.

Julius Lee Engineering
 1109 Coastal Bay Blvd.
 Boynton, FL 33435

Job	Truss	HJ05	323087
Truss Type	MONO TRUSS		
Qty	2		
Ply	1		
SPARKS - BARTH RES.			
Job Reference (optional)			7.1405 Oct 1 2009 MITTEK Industries, Inc. Mon Jan 25 16:38:06 2010 Page 1
14204319			



LOADING (psf)	SPACING	DEFL	BRACING	WEBS	REACTIONS	FORCES	NOTES
BCDL 5.0	Code FBC2007/TP12002	Wind(LL) -0.02	TOP CHORD	2 X 4 SYP No.2	Max Horz 2=227(LC 3)	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	(10-11)
BCLL 0.0	Rep Stress Incr NO	Horz(TL) -0.00	BOT CHORD	2 X 4 SYP No.2	Max Grav 4=78(LC 1), 2=324(LC 1), 5=95(LC 2)		
TCDL 7.0	Lumber Increase 1.25	Vert(TL) -0.01					
TCLL 20.0	Plates Increase 1.25	Vert(LL) -0.01					
	2.0-0	in (loc) -0.01					

MITTEK recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=78/Mechanical, 2=324/0-11-6, 5=39/Mechanical

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)

1) Wind: ASCE 7-05: 110mph (3-second gust); TCCL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No. 2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 4, 443 lb uplift at joint 2 and 49 lb uplift at joint 5.

7) Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 38 lb up at 1-5-12, 38 lb up at 1-5-12, and 10 lb down and 42 lb up at 4-3-11, and 10 lb down and 42 lb up at 4-3-11 on top chord, and 16 lb up at 1-5-12, 16 lb up at 1-5-12, and 10 lb down at 4-3-11, and 10 lb down at 4-3-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

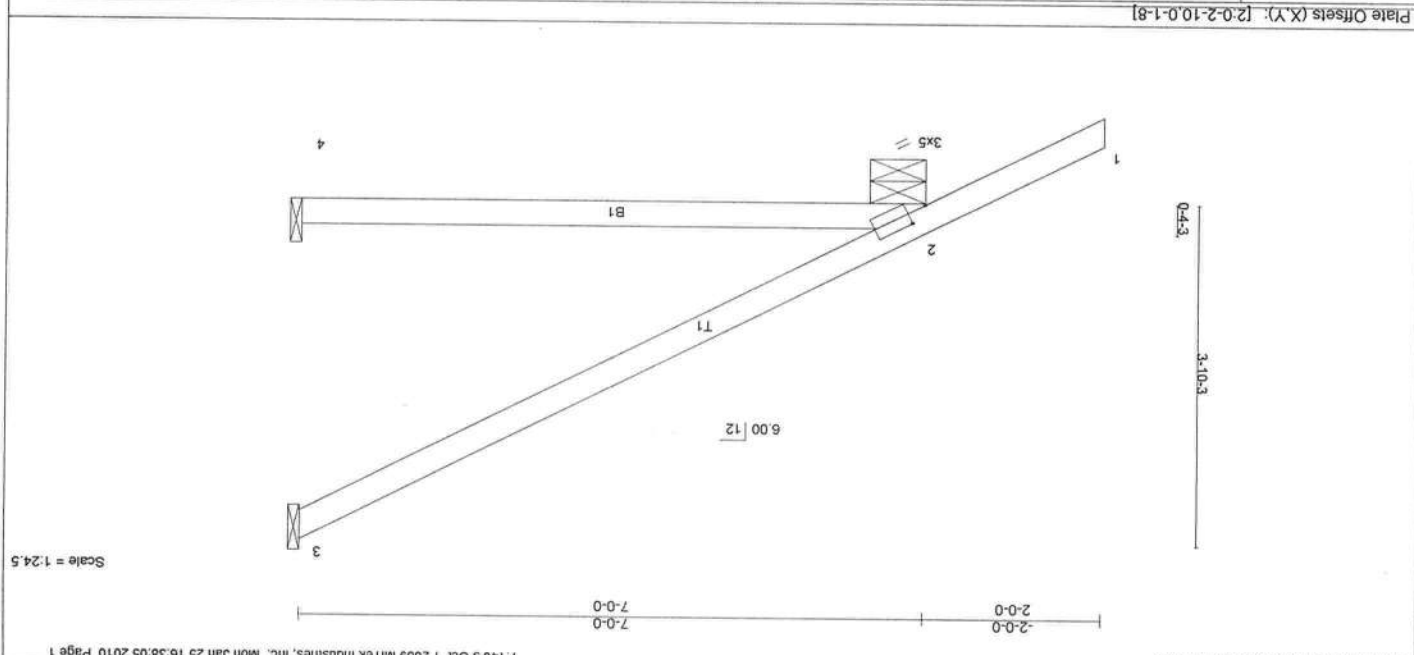
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

January 25, 2010

A circular professional engineer seal for Julius S.K. Lem. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "JULIUS S.K. LEM" at the bottom, separated by two stars. The inner circle contains the text "STATE OF FLORIDA" at the top and "LICENSE" at the bottom. In the center, the license number "No 34989" is displayed. Two hands are shown: a shorter hour hand pointing between 10 and 11, and a longer minute hand pointing at 2.

323087	RUSS	RUSS TYPE	MONO HIP	1	Qty	1	SPARKS - BARTH RES.	1	Job Reference (optional)	1A204317
323088	RUSS	RUSS TYPE	MONO HIP	1	Qty	1	SPARKS - BARTH RES.	1	Job Reference (optional)	1A204317

Job	Truss	Truss Type	Qty	33	1	Job Reference (optional)	7.140 s Oct 1 2009 Mitek Industries, Inc. Mon Jan 25 16:38:05 2010 Page 1
323087	EJ07	MONO TRUSS	Ply			SPARKS - BARTH RES.	
						14204315	



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	IDEAL	L/D
BCDL 5.0	Code FBC2007/TP12002	(Matrx)	Wind(LL)	0.06	2-4	>999
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.00	3	n/a
TCDL 7.0	Lumber Increase 1.25	BC 0.27	Vert(TL)	-0.15	2-4	>520
TCLL 20.0	Plates Increase 1.25	TC 0.57	Vert(LL)	-0.08	2-4	>992
					360	

LUMBER	TOP CHORD 2 X 4 SYP No.2	BOT CHORD 2 X 4 SYP No.2
REACTIONS (lb/size)	3=151/Mechanical, 2=359/0-7-8, 4=39/Mechanical	
Max Horz 2=203(LC 6)		
Max Uplift 3=111(LC 6), 2=203(LC 6)		
Max Grav 3=151(LC 1), 2=359(LC 1), 4=93(LC 2)		

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

1) Wind: ASCE 7-05: 110mph (3-second gust), TCDL=4.2psf, BCDL=3.0psf, h=18ft, Cat. II, Exp C, enclosed, MWFRS (low-rise) and C-C Extent(2) zone-C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a live load of 20 psf on the bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20 psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) All bearings are assumed to be SYP No. 2.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 111 lb uplift at joint 3 and 203 lb uplift at joint 2.

7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

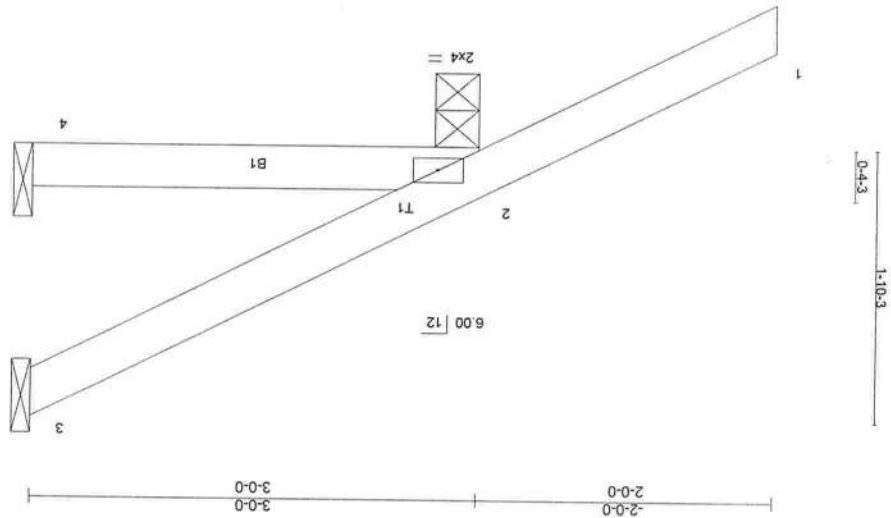
9) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Julius S.K. Lee, Professional Engineer, License No. 34869, State of Florida

January 25, 2010

Job	323087	Truss	EJ03	JACK	1	2	1	SPARKS - BARTH RES.	Job Reference (optional)	7,140 s Oct 1 2009 MitTek Industries, Inc. Mon Jan 25 16:38:04 2010 Page 1
										14204313



LOADING (psf)	20.0	SPACING	2.0-0	Code FBC2007/TP12002	YES	Rep Stress Incr	1.25	Lumber Increase	1.25	TC	0.37	CSI	0.00	WB	0.00	(Matrix)	0.01	DEFL	in (loc)	2.4	>999	360	L/d	240	n/a	240	n/a	240	Weight: 13 lb
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LUMBER	TOP CHORD	2 X 4 SYP No.2	TOP CHORD	2 X 4 SYP No.2	BRACING	TOP CHORD	2 X 4 SYP No.2	Bot CHORD	2 X 4 SYP No.2	Structural wood sheathing directly applied or 3-0-0 oc purlins.	Rigid ceiling directly applied or 10-0-0 oc bracing.	MitTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer installation guide.
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REACTIONS (lb/size) 3=31/Mechanical, 2=250/0-3-8, 4=14/Mechanical
 Max Horiz 2=166(LC 6)
 Max Uplift 3=-36(LC 7), 2=-31(LC 6), 4=-36(LC 4)
 Max Grav 3=31(LC 1), 2=250(LC 1), 4=42(LC 2)

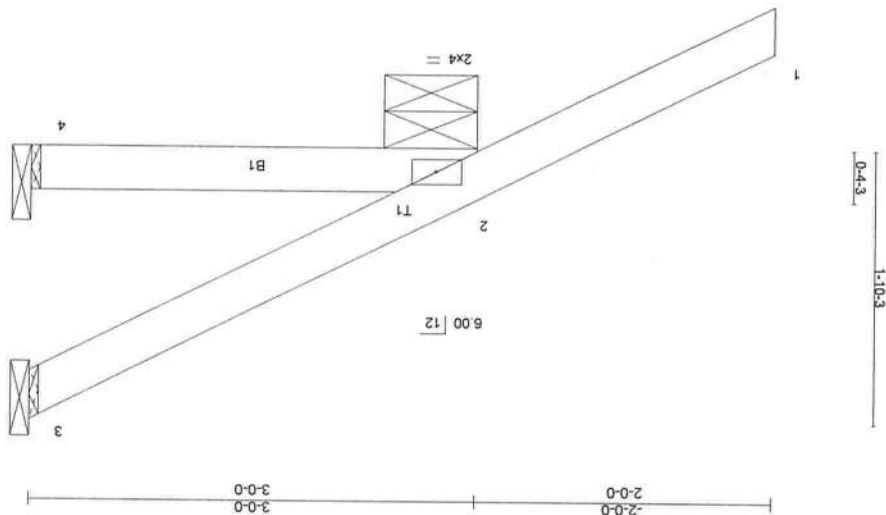
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 (8-9)
 1) Wind: ASCE 7-05: 110mph (3-second gust): TC DL=4.2psf; BC DL=3.0psf; h=18ft; Cat. II; Exp C: enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 4) All bearings are assumed to be SYP No.2.
 5) Refer to girder(s) for truss to truss connections.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 3, 31 lb uplift at joint 2 and 36 lb uplift at joint 4.
 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 9) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
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PROFESSIONAL ENGINEER
STATE OF FLORIDA
LICENSE
No. 34869
JULIUS S.K. LEE

January 25, 2010

Job	323087	C03	JACK	Qty	12	Ply	SPARKS - BARTH RES.	1	Job Reference (optional)	7.140 s Oct 1 2009 Mittek Industries, Inc. Mon Jan 25 16:38:03 2010 Page 1
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Scale = 1:14.7

LOADING (psf)	20.0	TCDL	7.0	BCDL	5.0
SPACING	2-0-0	Plates Increase	1.25	TC	0.42
		Lumber Increase	1.25	BC	0.05
		Rep Stress Incr	YES	WB	0.00
		Code FBC2007/TP12002			
LUMBER					
TOP CHORD	2 X 4 SYP No. 2				
BOT CHORD	2 X 4 SYP No. 2				

REACTIONS (lb/size) 3=16/Mechanical, 2=264/0-7-8, 4=13/Mechanical

Max Horiz 2=166(LC 6)
Max Uplift 3=30(LC 7), 2=289(LC 6)
Max Grav 3=21(LC 4), 2=264(LC 1), 4=39(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-9)

- 1) Wind: ASCE 7-05: 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grp DOL=1.60
- 2) This truss has been designed for a live load of 20.0psf on the bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) All bearings are assumed to be SYP No. 2.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 3 and 289 lb uplift at joint 2
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, P.E.; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



January 25, 2010

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ANSI/TP1 Quality Criteria, D58-87 and BCS11 Building Component available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee Engineering
1109 Coastal Bay Blvd.
Boynton, FL 33435

RE: 323087 - SPARKS - BARTH RES.

Site Information:

Project Customer: SPARKS CONST. Project Name: 323087 Model: BARTH RES.
Lot/Block:
Address: 100 SW GRASSY LANE
City: COLUMBIA CTY
State: FL

No.	Seal#	Truss Name	Date
35	I4204344	T25	1/25/010
36	I4204345	T26	1/25/010
37	I4204346	T27	1/25/010
38	I4204347	T28	1/25/010
39	I4204348	T29	1/25/010
40	I4204349	T30	1/25/010
41	I4204350	T31	1/25/010
42	I4204351	T32	1/25/010