

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844

Florida Engineering Certificate of Authorization Number: 0 278

Florida Certificate of Product Approval # FL1999

Page 1 of 1 Document ID: 1TEL8228Z0331072803

Truss Fabricator: Anderson Truss Company

Job Identification: dear--OWNER BUILDER deardorf -- , **

Truss Count: 77

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Versions 7.24, 7.37, 7.36.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: A11030EE-GBLLETIN-BRCLBSUB-A11015EE-CNBRGBLK-

Seal Date: 01/31/2008

-Truss Design Engineer-

James F. Collins Jr.

Florida License Number: 52212

1950 Marley Drive

Haines City, FL 33844

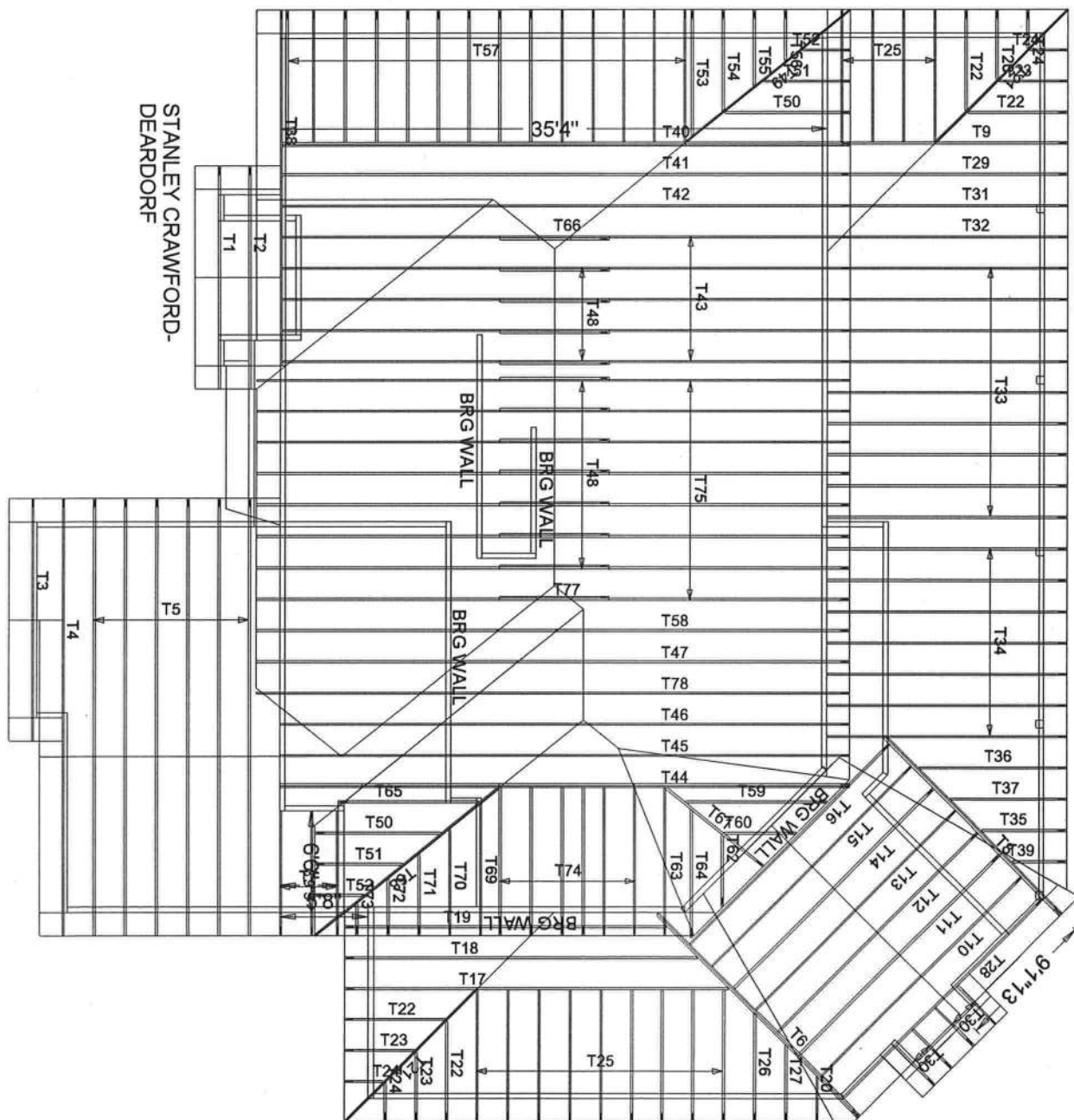
#	Ref	Description	Drawing#	Date
1	76956--T1		08031070	01/31/08
2	76957--T10		08031071	01/31/08
3	76958--T11		08031072	01/31/08
4	76959--T12		08031073	01/31/08
5	76960--T13		08031074	01/31/08
6	76961--T14		08031075	01/31/08
7	76962--T15		08031076	01/31/08
8	76963--T16		08031077	01/31/08
9	76964--T17		08031078	01/31/08
10	76965--T18		08031079	01/31/08
11	76966--T19		08031080	01/31/08
12	76967--T2		08031081	01/31/08
13	76968--T20		08031082	01/31/08
14	76969--T21		08031083	01/31/08
15	76970--T22		08031084	01/31/08
16	76971--T23		08031085	01/31/08
17	76972--T24		08031086	01/31/08
18	76973--T25		08031087	01/31/08
19	76974--T26		08031088	01/31/08
20	76975--T27		08031089	01/31/08
21	76976--T28		08031090	01/31/08
22	76977--T29		08031091	01/31/08
23	76978--T3		08031092	01/31/08
24	76979--T30		08031093	01/31/08
25	76980--T31		08031094	01/31/08
26	76981--T32		08031095	01/31/08
27	76982--T33		08031096	01/31/08
28	76983--T34		08031097	01/31/08
29	76984--T35		08031098	01/31/08
30	76985--T36		08031099	01/31/08
31	76986--T37		08031100	01/31/08
32	76987--T38		08031101	01/31/08
33	76988--T39		08031102	01/31/08
34	76989--T4		08031077	01/31/08
35	76990--T40		08031103	01/31/08
36	76991--T41		08031104	01/31/08
37	76992--T42		08031105	01/31/08
38	76993--T44		08031106	01/31/08

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40	76995--T46		08031108	01/31/08
41	76996--T47		08031109	01/31/08
42	76997--T49		08031110	01/31/08
43	76998--T5		08031078	01/31/08
44	76999--T50		08031111	01/31/08
45	77000--T51		08031112	01/31/08
46	77001--T52		08031113	01/31/08
47	77002--T53		08031114	01/31/08
48	77003--T54		08031115	01/31/08
49	77004--T55		08031116	01/31/08
50	77005--T56		08031117	01/31/08
51	77006--T57		08031118	01/31/08
52	77007--T58		08031119	01/31/08
53	77008--T59		08031120	01/31/08
54	77009--T6		08031121	01/31/08
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59	77014--T64		08031126	01/31/08
60	77015--T65		08031127	01/31/08
61	77016--T68		08031128	01/31/08
62	77017--T69		08031129	01/31/08
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69	77024--T78		08031136	01/31/08
70	77025--T8		08031137	01/31/08
71	77026--T9		08031138	01/31/08
72	77027--T43		08031075	01/31/08
73	77028--T66		08031079	01/31/08
74	77029--T48		08031080	01/31/08
75	77030--T75		08031076	01/31/08
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#	Ref	Description	Drawing#	Date
77	77032--T99		08031082	01/31/08



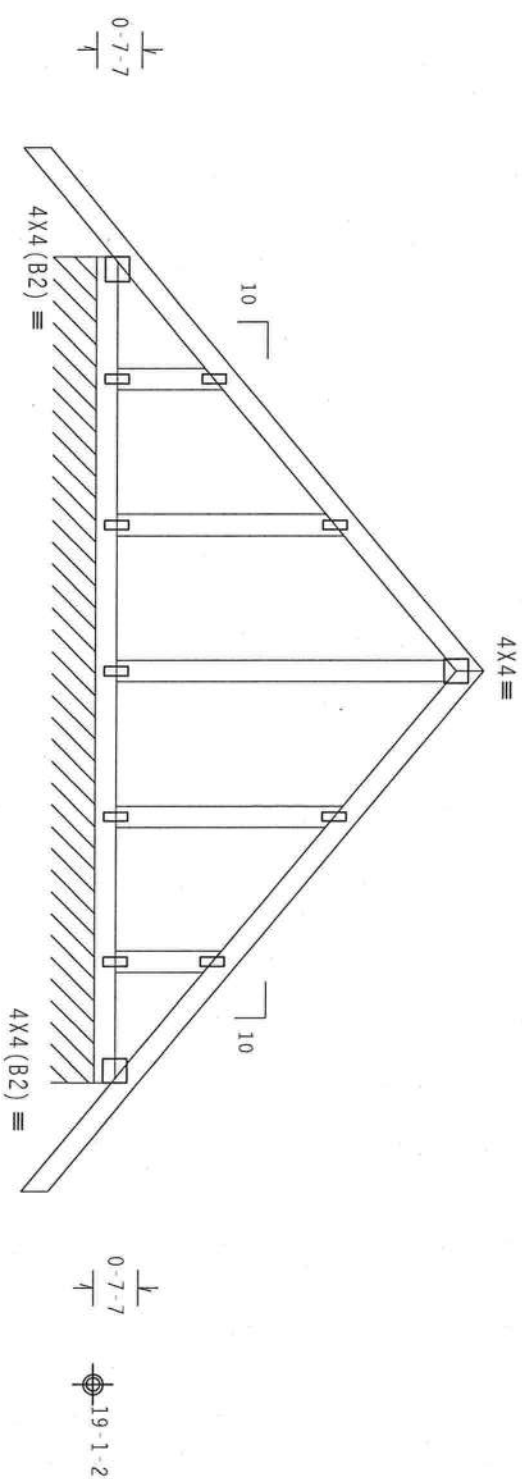
STANLEY CRAWFORD-
DEARDORF



(dear-OWNER BUILDER deardorf - - , ** - T1)

110 mph wind, 21.45 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcp1(+/-)=0.18

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.



1-6-0

5-8-0

5-8-0

1-6-0

11-4-0 Over Continuous Support

R=207 PLF U=76 PLF W=11-4-0

PLT TYP. Wave	Design cric.	IFI-2002 (SID)/FBC Cq/RT=1.00(1.25)/
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7.24.1230.16

QTY:1 FL/-/4/-/E/-/-

Scale = .375"/Ft.



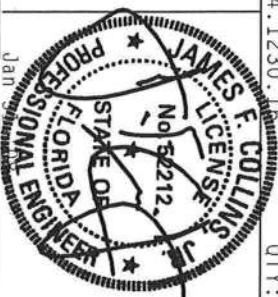
Building Components Group Inc.

Haines City, FL 33844

FL Certificate of Authorization # 0

****WARNING**** *Holes require extreme care in fabrication, handling, shipping, installing and bracing. Refer to 2021 (BUILDING COMPONENT CARE IN FABRICATION), published by THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314 and WEA 4000 TRUSS COMPANY OF AMERICA, 6500 ENTERPRISE LANE, MIDDLETON, WI. 53717 for safety practices pertain to performing these functions. *OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** *TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ASH/TP-1 SEC. 2.



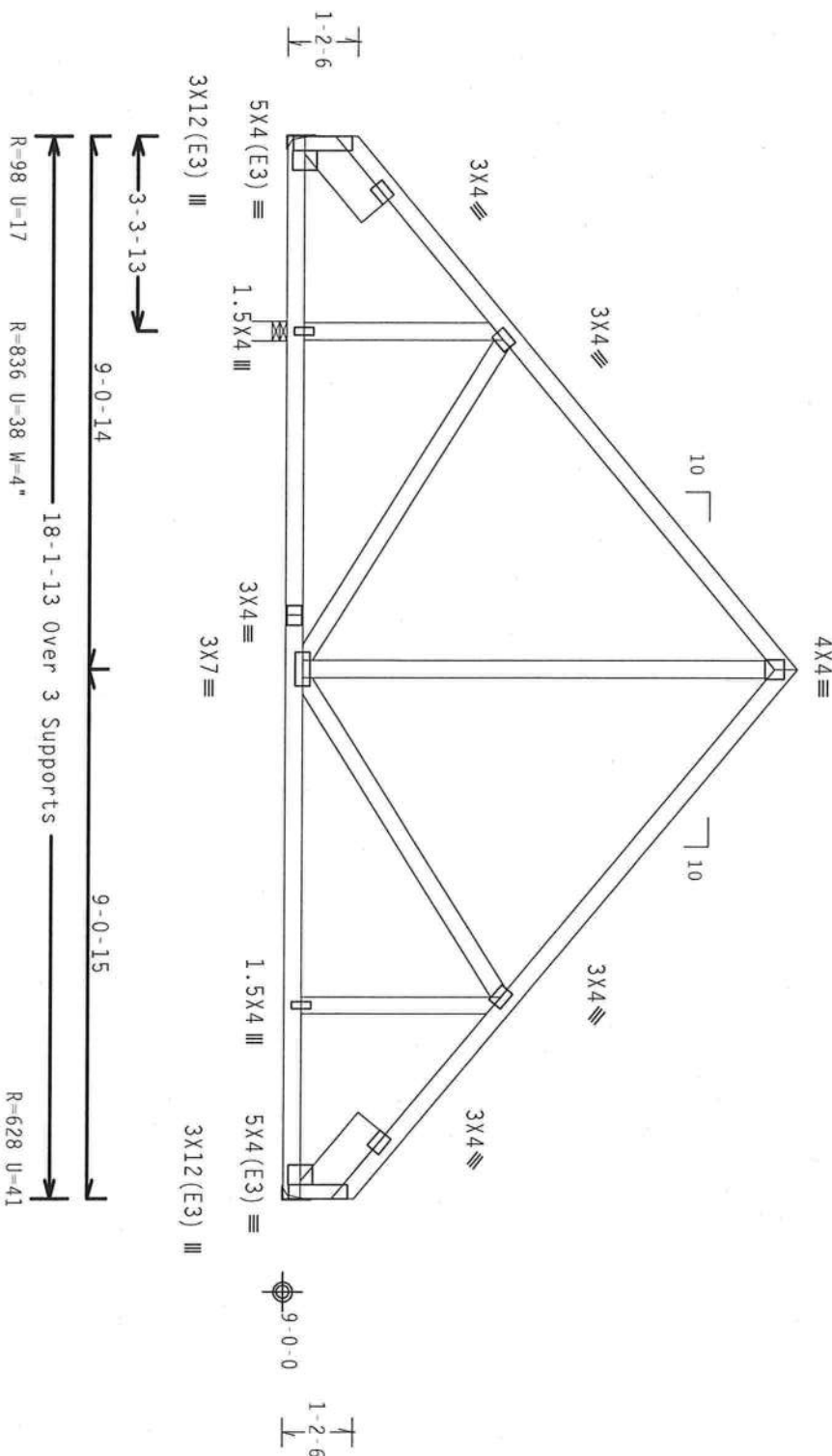
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BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43773
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	ITELR8228203

המחברת מודה לפרופ' ד"ר יעקב גורן, מנהל המכון למחקר ביטחוני, על סיועו במחקר.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MAFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-14/-1E/-1-

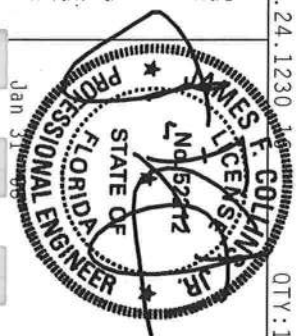
Scale = .3125"/Ft.

WARNING—PANELS (BOTH END EXISTING CASE IN FABRICATION), HANDLING, SHIPPING, INSTALLING AND BROCKING REFER TO RC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 FOR SA. PRELIMINARY TRUSS COUNCIL OF AMERICA, 65000 ENTERPRISE LANE, MIDDLETON, WI 53519 FOR SA. PRACTICES AND PRELIMINARY TRUSS COUNCIL OF AMERICA, INTERSESS OTHERWISE INDICATED THAT CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED GRID CEILING.



Building Components Group Inc.

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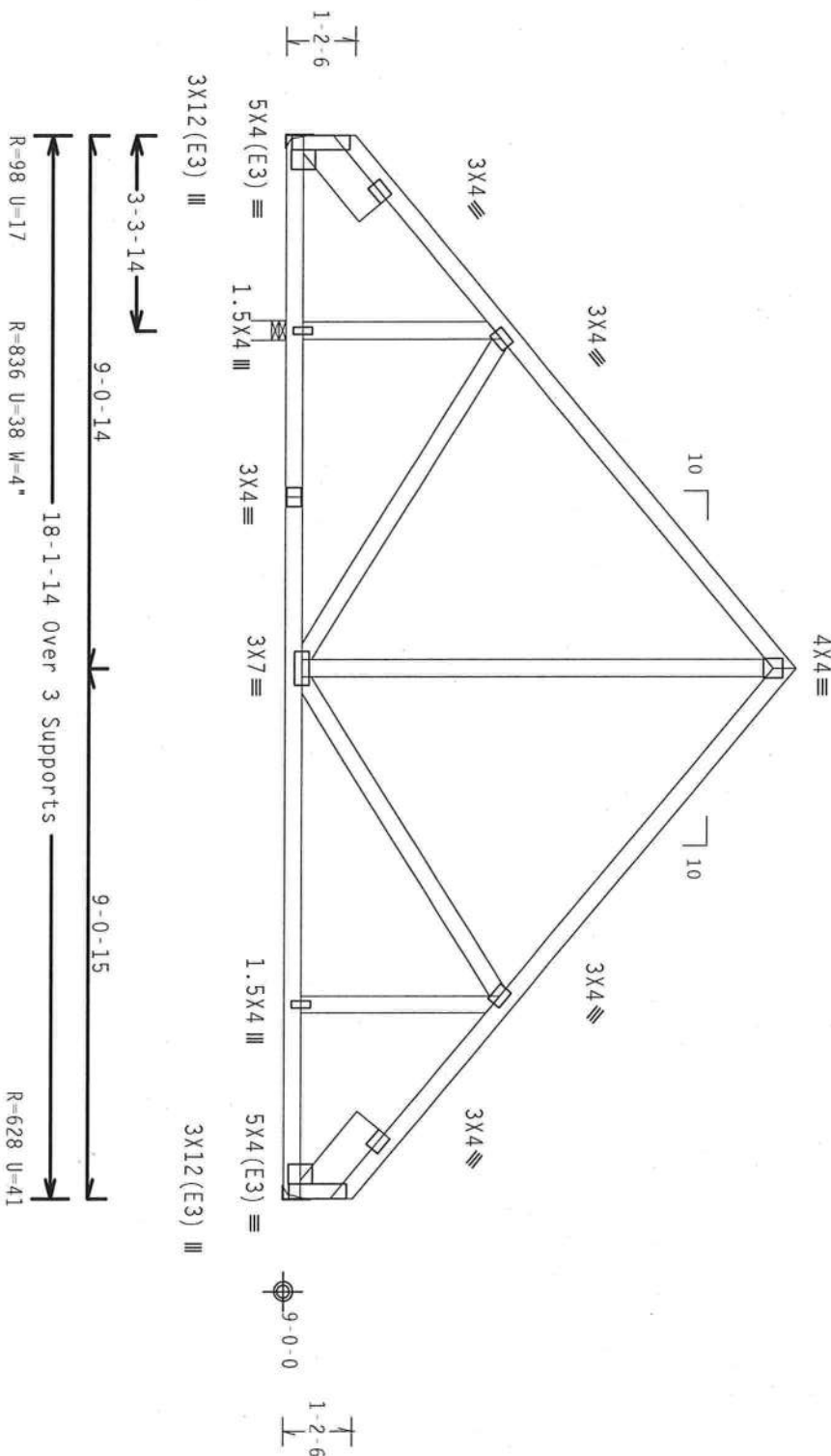
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BC DL	10.0 PSF	DRW	HCURS8228 08031071
BC LL	0.0 PSF	HC-ENG	EC/MHK
TOT.LD.	40.0 PSF	SEQN-	43977
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	Webb	2x4	SP	#3		

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, Cat II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ gcpi (+/-)=0.18

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .3125" / Ft.



Building Components Group Inc

Haines City, FL 33844
 FI Certificate of Authorization # 0779

****WARNING**** PRIORS TO THE EXISTING CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BROCKING REFERENCE TO 6051 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TROSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND UFGA (GOOD TRUSS COUNCIL OF AMERICA, 65000 ENTERPRISE LANE, MOUNTAIN VIEW, TX 75119) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE TPO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** THROUGH A COPY OF THIS DECISION TO THE INSTALLATION CONTRACTOR, THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONTRACTS WITH APPLICABLE PROVISIONS OF 605 (ADDITIONAL DESIGN INFO) BY AIA/AI AND TPI. CONNECTION PLATES ARE MADE OF 2010/91564 (40/55/5) ASTM A563 GRADE 40/60 (4" X 4/5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEX AS OF TPI-2102-SEC.3. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLICIT FOR THE TRUSS COMPONENTS OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 76958
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	H0586228 08031072
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43983
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

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( (dear--OWNER BUILDER deardorf --, ** - T12 )
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110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf lw=1.00 GCPI(+/-)=0.18

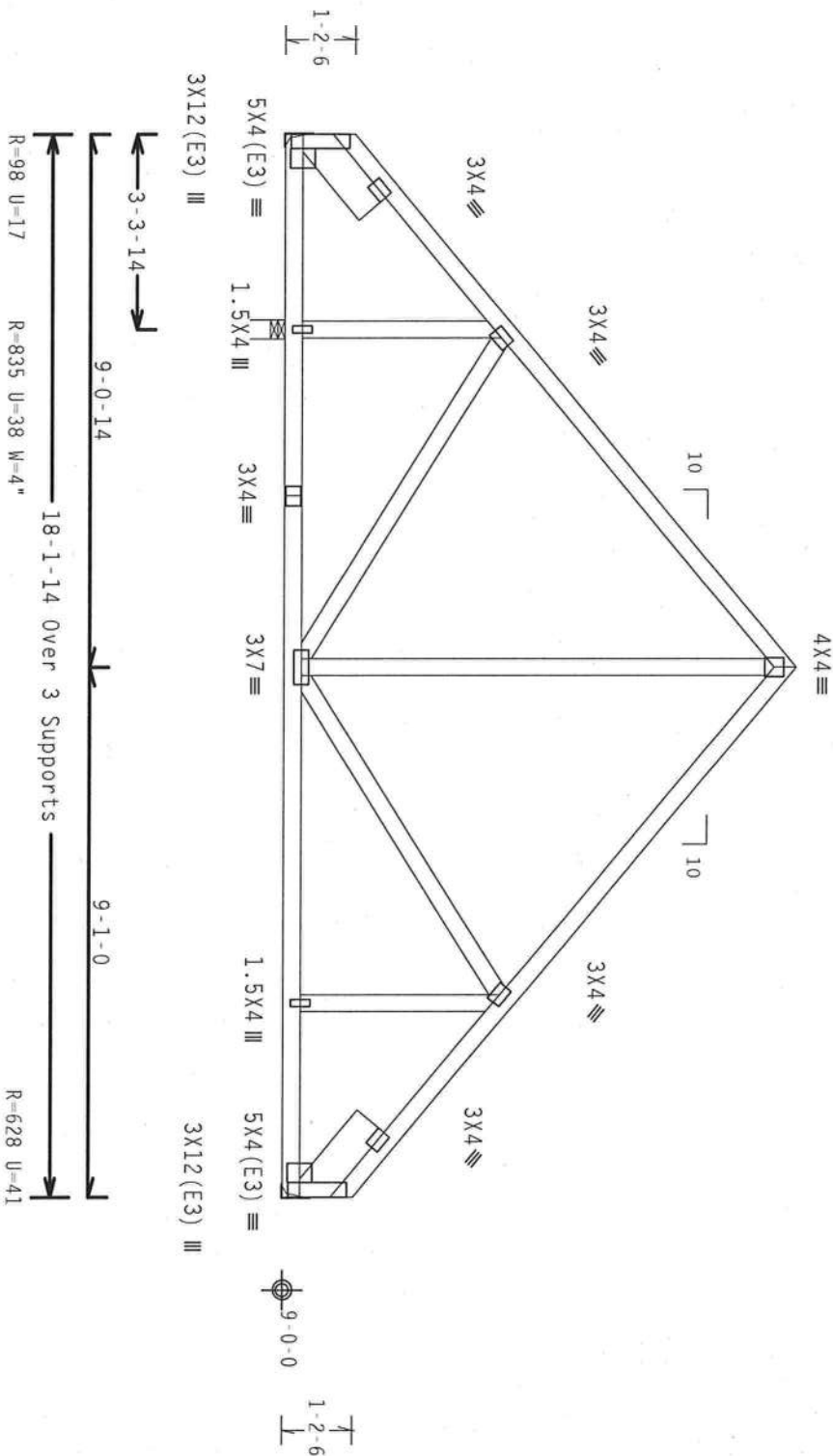
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: Lt Slider 2x8 SP SS: BLOCK LENGTH = 1.750'
: Rt Slider 2x8 SP SS: BLOCK LENGTH = 1.778'

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Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Wind reactions based on MWFRS pressures.



Design Cr1t: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .3125"/Ft.



Building Components Group Inc.

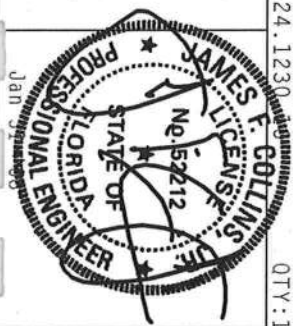
Haines City, FL 33844
FL Certificate of Authorization #

****WARNING**** WHILES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO DCST (BUILDING COMPONENT SPECIFICATION), PUBLISHED BY IPT (STEEL PLATE INSTITUTE, 214 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MICHIGAN AVE., CHICAGO, IL 60610) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITR BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE REQUIREMENTS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MICHIGAN AVE., CHICAGO, IL 60610) AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 500 N. MICHIGAN AVE., CHICAGO, IL 60610) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

CONNECTION PLATES ARE MADE OF 2018/1656A (W/H/SS/TS) ASTM A575 GRADE 40/50 (W/2X55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND (1) SHALL BE PER AISC A3 OR T1-1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SEAL FOR THE TRUSS COMPONENT OF THE DESIGN. SEAL FOR THE BUILDING DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AISC/T1-1 SEC. 2.

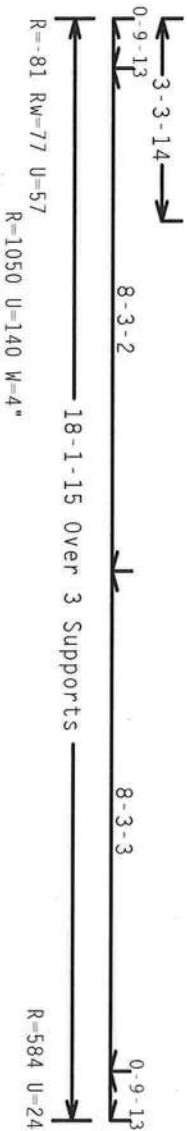


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TOT.LD.	40.0 PSF	SEQN-	43989
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

THIS WORK PREPARED FROM COMPUTER INPUT (LVALUES & DIMENSIONS) SUBMITTED BY IKUZO MATS.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 gcpi (+/-) -0.18

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

QTY:1

FL/14/1E/1

Scale = .3125"/Ft.

James Collins License

TC LL	20.0 PSF	REF	R8228 - 76960
TC DL	10.0 PSF	DATE	01/31/08

Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization #

[illegible]

James Collins

TC LL	20.0 PSF	REF	R8228- 76960
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031074
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT. LD.	40.0 PSF	SEQN -	43792
DUR. FAC.	1.25	FROM	JP
SPACING	24.0"	JREF -	1TEL8228203

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, closed bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 Gcpl(+/-)=0.18

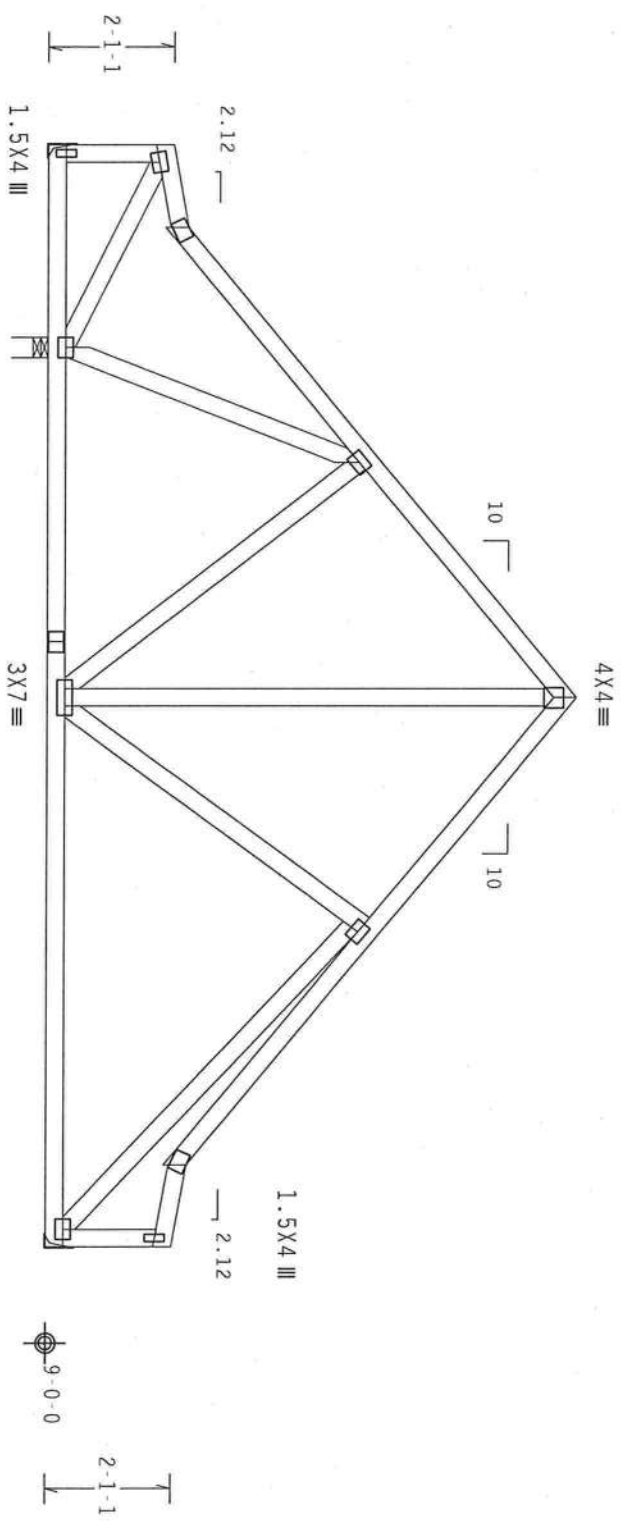


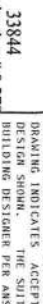
Diagram of a continuous beam with three supports. The beam is divided into four spans. The first span is 3'-3" (1-4" overhang), the second is 7'-8", the third is 7'-8", and the fourth is 1'-4" (1-4" overhang). The total length is 18'-1". The beam is labeled "18-1-15 Over 3 Supports". The beam is supported by three supports, indicated by upward arrows. The beam is labeled "R=99 RW=78 U=54" and "R=578 U=25".

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave	Design Crit:	IP1-2002(SID)/FBC
		Cq/RT=1.00(1.25)

7.24.1230 QTY:1 FL/-/4/-/E/-/-

Scale = .3125"/Ft.



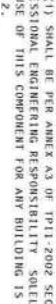
Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0778

****WARNING**** BRISSES, RIGID, EXTERIOR, CASE, IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO RC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WEA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED BRID CELLING.

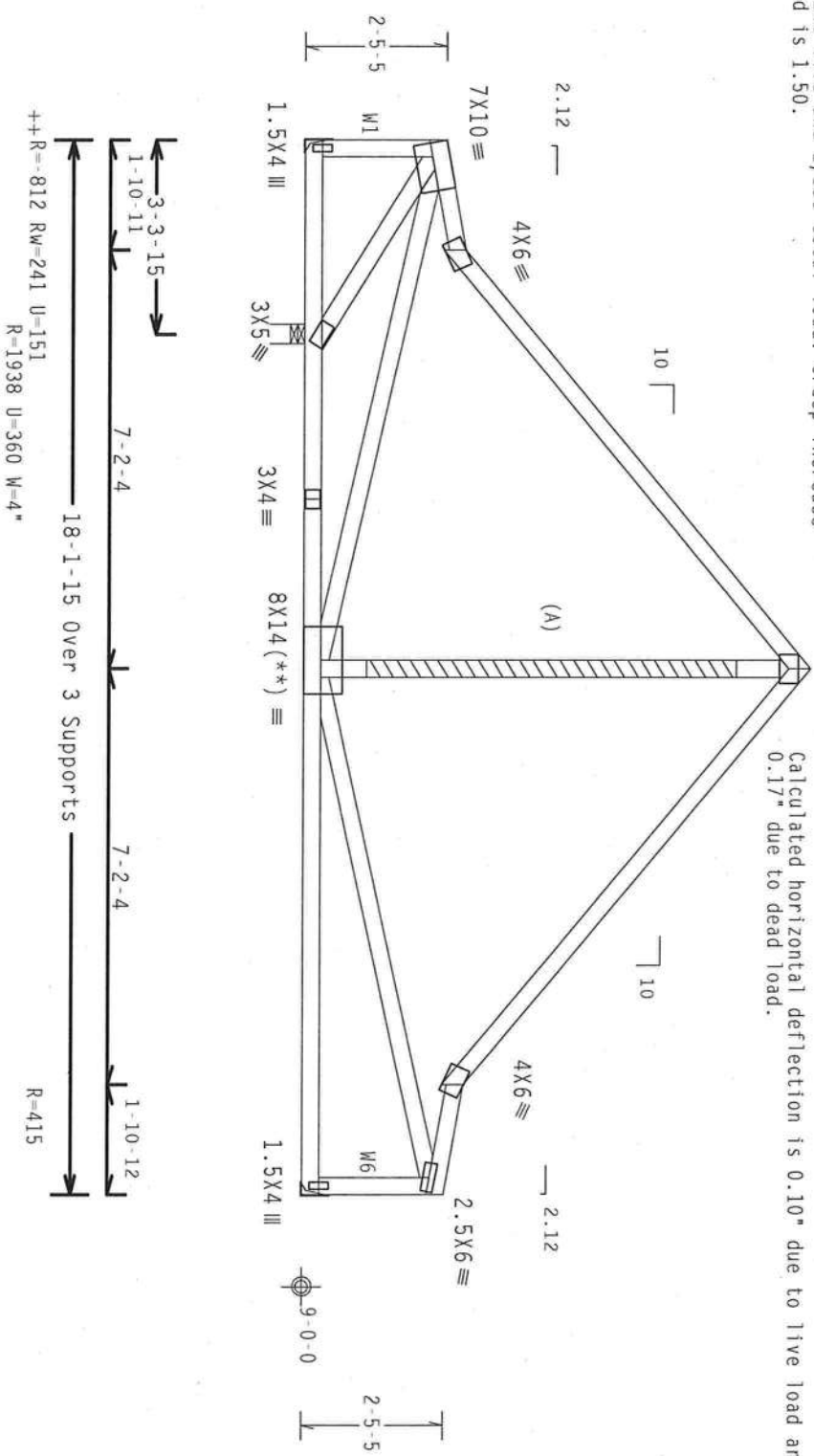
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING, OF BRISSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. TPI REG. CONNECTION PLATES ARE MADE OF 20/19/16GA (W/15/5/5) ASTM A653 GRADE 40/50 (W, W/H, 551 GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITIONING PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIA 63.03 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SILENT FOR THE TRUSS COMPONENT DESIGNER. SIGNATURE AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/DP1 1 SEC. 2.



TC LL	20.0 PSF	REF R8228 - 76961
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031075
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT. LD.	40.0 PSF	SEQN- 43794
DUR. FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W1, W6 2x4 SP #2 Dense:
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.
Wind reactions based on MMFRS pressures.
End verticals not exposed to wind pressure.
(A) #3 or better scab brace. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.37.0521 F. COLLINS
OTV:1 FL/-/4/-/E/-/-

Scale = .3125"/ft.

Building Components Group Inc.

Haines City, FL 33844
Ft. Certificate of Authorization #0778

PLT TYP. Wave

OTV:1 FL/-/4/-/E/-/-

Scale = .3125"/ft.

REF R8228- 76962
DATE 01/31/08
DRW HCUSR8228 08031076
HC-ENG EC/WHK
SEON- 1918 REV
FROM JP
JREF- ITEL8228203

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf 1w=1.00 gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.



Scale = .3125"/Ft.

SEALS OF THE
JAMES F. GOLLIGER
ENGINEERING
FIRM

STATE OF

[illegible][illegible]

(dear--OWNER BUILDER deardorf -- , ** - T17)

Top chord 2x6 SP #2 :T1 2x4 SP #2 Dense:
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3 :W6 2x4 SP #2 Dense:

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

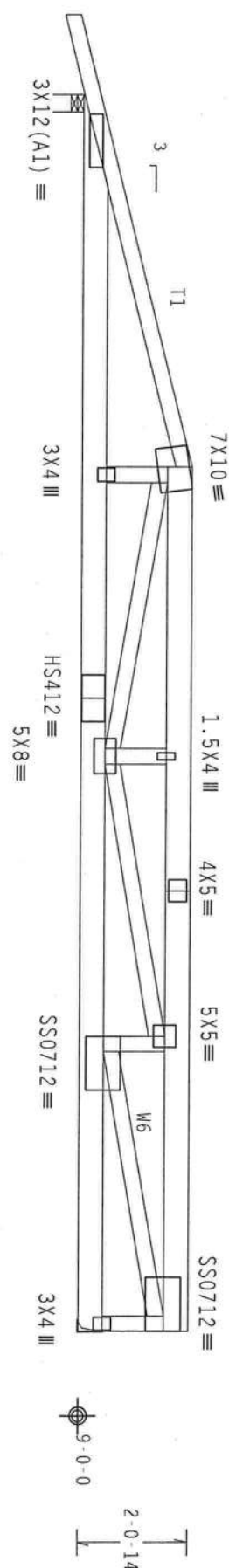
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.



1-6-0

7-0-0
23-4-4 Over 2 Supports
16-4-4
R=1907 U=206 W=4"
R=2026 U=195

PLT TYP. 20 Gauge HS, 18 Gauge HS, Design Crit: TPI-2002(STD)/FBC
Wave Cq/RT=1.00(1.25)/0(0)

7.24.1230 1. COLLINS QTY:1 FL/-/4/-/E/-/ Scale = .3125"/ft.

WARNINGBUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC&I (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

Haines City, FL 33844
TPI Certificate of Authorization #0-778



FC LL	20.0 PSF	REF R8228- 76964
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031078
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEON- 43960
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

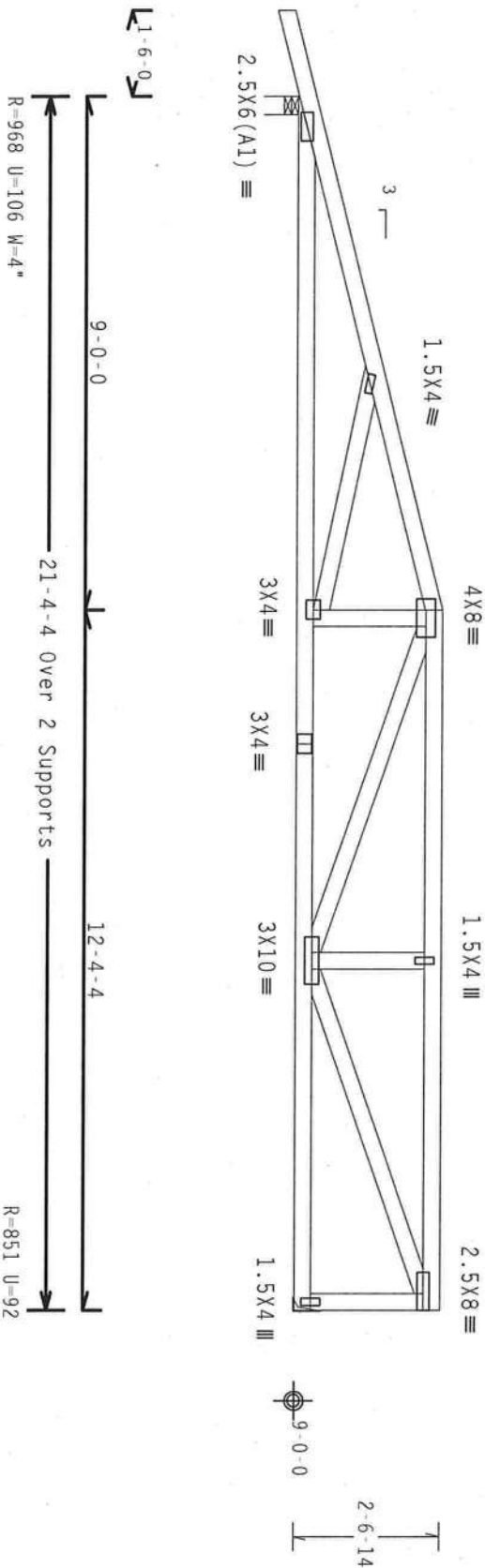
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCP(+/)=0.18$

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 1

FL/-/4/-/E/-/-

Scale = .3125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6100 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



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Haines City, FL 33844

PT Certificate of Authorization #00798



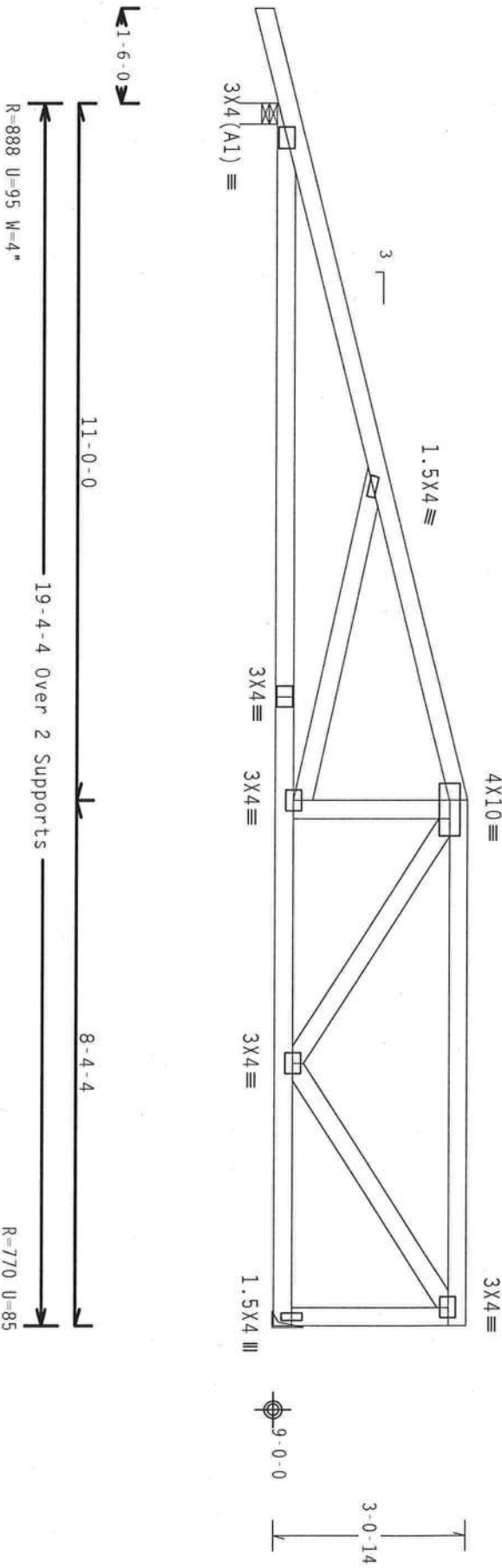
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCSR8228 08031079
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SECON-	43801
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF	1TEL8228203

(dear--OWNER BUILDER deardorf -- , ** - T19)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 GCPI (+/-)=0.18
Wind reactions based on MWFRS pressures.
Right end vertical not exposed to wind pressure.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 7.24.1230 QTY:1 FL/-/4/-/E/-/- Scale =.375"/Ft.

WARNINGMUSHS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

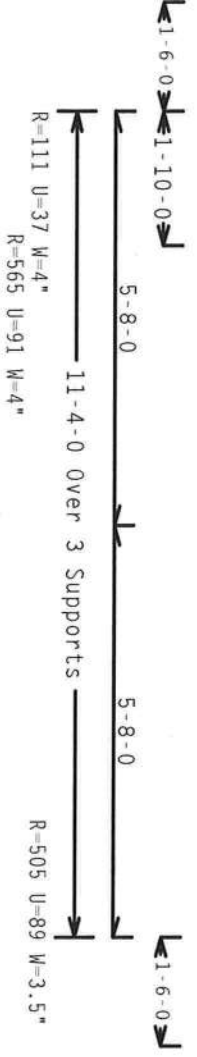
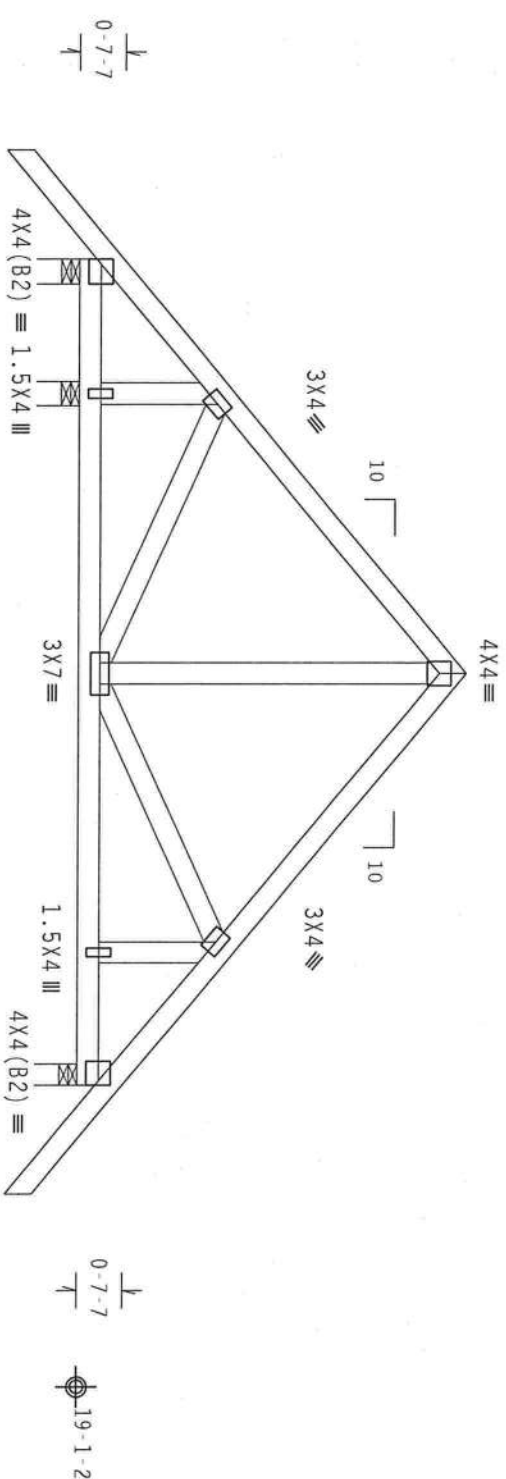
Haines City, FL 33844
FL Certificate of Authorization #0778



TC LL	20.0 PSF	REF	R8228- 76966
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCU8R8228 08031080
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43803
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

(dear--OWNER BUILDER deardorf -- . ** - T2)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
110 mph wind, 21.45 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI (+/-)=0.18

Roof overhang supports 2.00 psf soffit load.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.
Wind reactions based on MMFRS pressures.



PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)
7.24.1230
QTY: 1
FL/-/4/-/E/-/-
Scale = .375"/Ft.

WARNING MUSTS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Haines City, FL 33844
Plt Certificate of Authorization #0-078
JAIN SINGH
PROFESSIONAL ENGINEER
FLORIDA
No. 62243
JREF- 1TEL8228203

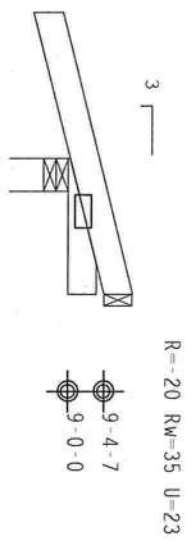
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.



1-6-0-0

1-4-11 over 2 supports
R=230 U=77 W=4"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 1 FL/-/4/-/E/-/-

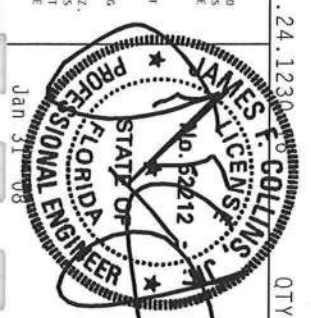
Scale = .5"/ft.

WARNINGBUILDES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AITCA (GOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

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FL Certificate of Authorization # 0778



TC LL	20.0 PSF	REF R8228- 76968
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031082
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT. LD.	40.0 PSF	SECON- 43819
DUR. FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

(dear--OWNER BUILDER deardorf -- , ** - T21)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not
located within 4.50 ft from roof edge, CAT II, EXP B, Wind TC
DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

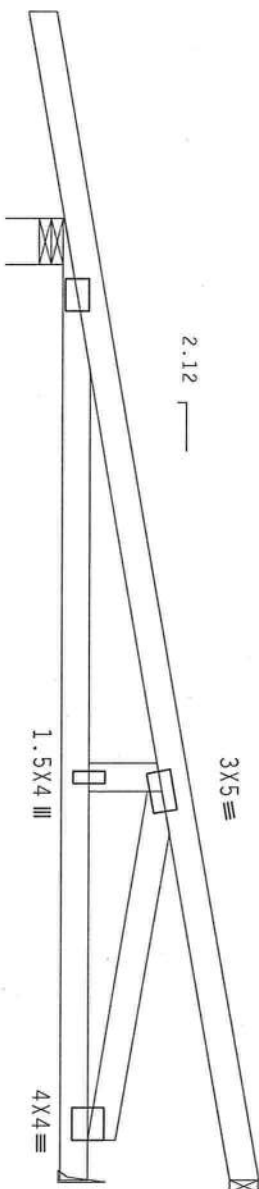
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

LOADING HAS BEEN CALCULATED BY THE TRUSS MANUFACTURER.
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO
VERIFY AND APPROVE THE LOADING.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 60 PLF at -2.12 to 60 PLF at 9.90
BC - From 4 PLF at -2.12 to 4 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 9.90
TC - 43 LB Conc. Load at 1.48
TC - 117 LB Conc. Load at 4.31
TC - 242 LB Conc. Load at 7.13
BC - 12 LB Conc. Load at 1.48
BC - 51 LB Conc. Load at 4.31
BC - 113 LB Conc. Load at 7.13



2.12

3X5

1.5X4

3X4 (A1)

R=153 U=40

2'-0"

10'-9-3

R=555 U=39

9'-0-0



R=802 U=130 W=5.656"

PLT TYP. Wave

Design Crit: TPI-2002 (STD) / FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 1

FL/-/4/-/E/-/-

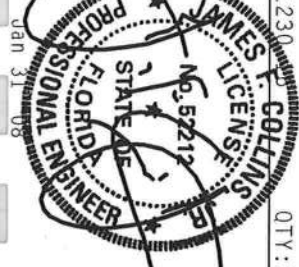
Scale = .5"/Ft.

WARNING MUSTS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2100
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6200
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.



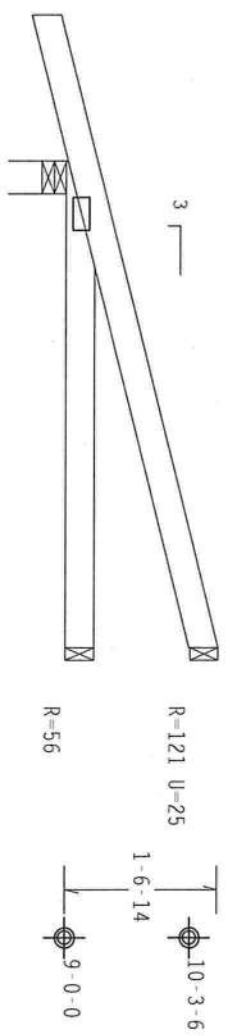
Building Components Group Inc.

Haines City, FL 33844
TPI Certificate of Authorization #0-778



TC LL	20.0 PSF	REF R8228- 76969
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031083
BC LL	0.0 PSF	HC-ENG EC/MHK
TOT. LD.	40.0 PSF	SEON- 43931
DUR. FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18
Wind reactions based on MMFRS pressures.

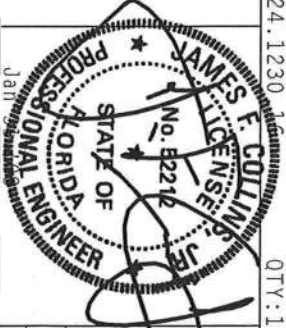


1-6-0-0
5-0-0 Over 3 Supports
R=323 U=58 W=4"

PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)
7.24.1230
QTY:1
FL/-/4/-/E/-/-
Scale =.5"/Ft.

WARNINGBUILERS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22319 AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AIA/AMA AND TPI). DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AIA/AMA AND TPI). ANY DEVIATION FROM THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE USER. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Building Components Group Inc.
Haines City, FL 33844
PL Certificate of Authorization # 0 778

TC LL	20.0 PSF	REF R8228- 76970
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031084
BC LL	0.0 PSF	HC-ENG EC/WHK *
TOT. LD.	40.0 PSF	SEQN- 43806
DUR. FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

(dear--OWNER BUILDER dear-- , ** - T23)

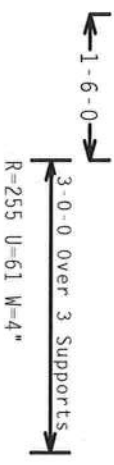
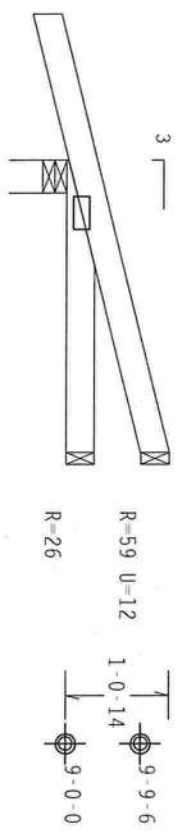
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 1 FL/-/4/-/E/-/-

Scale = .5"/ft.

****WARNING**** MUSTERS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

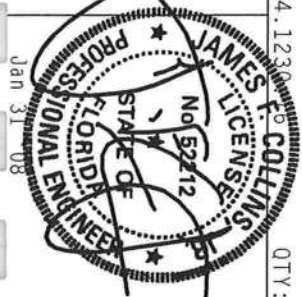
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONTRACTOR WITH APPLICABLE PROVISIONS OF BOB (NATIONAL DESIGN SPEC. BY AREA) AND TPI. DESIGN CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL TRUSS PLATES, STEEL, WELDS, AND ALL OTHER MATERIALS AND METHODS OF CONSTRUCTION. ANY INSPECTION OF PLATES FOLLOWED BY (C) SHALL BE PER AMEX AS OF TPI-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Building Components Group Inc.

Haines City, FL 33844

FL Certificate of Authorization #00728



TC LL	20.0 PSF	REF	R8228- 76971
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031085
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT. LD.	40.0 PSF	SEON-	43808
DUR. FAC.	1.25	FROM	JP
SPACING	24.0"	UREF-	1TEL8228203

(dear--OWNER BUILDER deardorf -- , ** - T24)

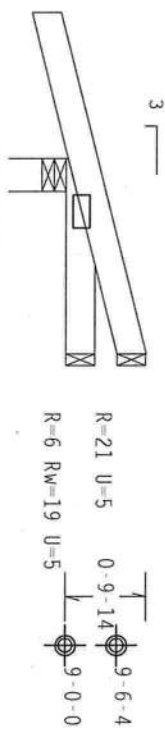
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.



1'-6-0-0
2'-0-0 Over 3 Supports
R=231 U=67 W=4"

PLT TYP. Wave

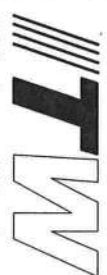
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 1 FL/-/4/-/E/-/-

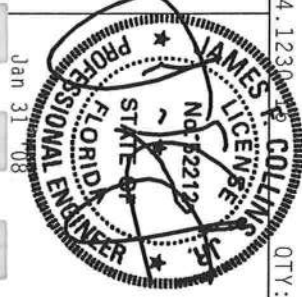
Scale =.5"/Ft.

WARNINGBUILDS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NCSE (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

Haines City, FL 33844
TPI Certificate of Authorization #0-778



TC LL	20.0 PSF	REF R8228- 76972
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031086
BC LL	0.0 PSF	HC-ENG EC/WHK *
TOT.LD.	40.0 PSF	SEON- 43925
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

(dear--OWNER BUILDER deardorf -- , ** - T25)

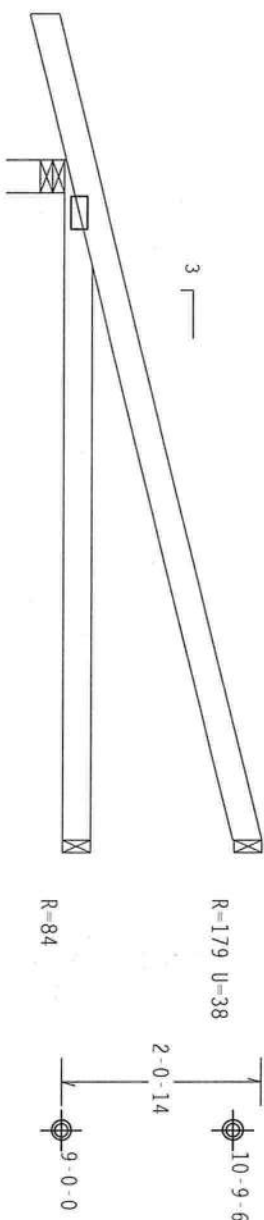
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 gcpl (+/-)=-0.18

Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC

Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale =.5"/Ft.

WARNINGTRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

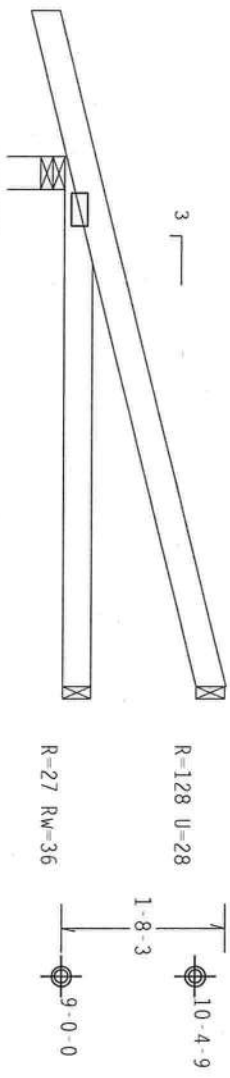
Haines City, FL 33844
Ft Certificate of Authorization #0-078



TC LL	20.0 PSF	REF R8228- 76973
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031087
BC LL	0.0 PSF	HC-ENG EC/WHK *
TOT.LD.	40.0 PSF	SEQN- 43814
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$
Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD) /FBC

Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 1

FL/-/4/-/E/-/-

Scale = .5" / Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WPCA (WOOD TRUSS COUNCIL OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN COMPLIANCE WITH APPLICABLE PROVISIONS OF ROS (NATIONAL DESIGN SPEC. BY AIA/AIA AND TPI. THE BCG CONTRACTS ARE MADE ON 20/10/10/10/10 (4/11/55/55) ASTM A505 GRADE 40/50 (4. R/H/55) GALV. STEEL. APPLY TO ALL PARTS OF THIS DESIGN. SECTION PER DRAWINGS 100-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED BY A LICENSED PROFESSIONAL ENGINEER. THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Building Components Group Inc.

Haines City, FL 33844

FL Certificate of Authorization #A-0778

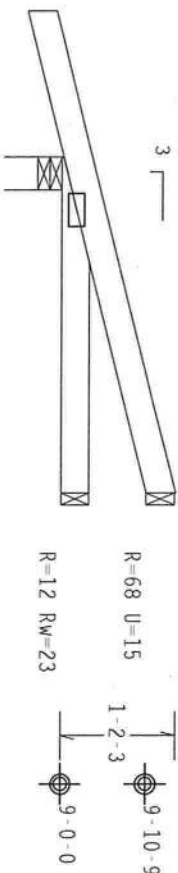
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TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031088
BC LL	0.0 PSF	HC-ENG EC/WHK *
TOT.LD.	40.0 PSF	SEON- 43942
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

```
( (dear--OWNER BUILDER deardorf --, ** - T27)
```

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.


$$2 \times 4(A1) =$$

1-6-0

3-5-5 Over 3 Supports

R=245 U=60 W=4"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/Ft.

WARNING—90105 (BUILDING EXTERIOR, CEILING, FLOORING, SHIPING, INSTALLING, AND REPAIRING REFER TO 90251 (BUILDING COMPONENT SHEET INFORMATION))—PUBLISHED BY THE TRUSS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK AND WICK COMPANY, TRUSS CONSULTING, 65000 ENTERPRISE LANE, MONTICELLO, VA 54319) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIVITIES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

Haines City, FL 33844

FI Certificate of Authorization # 00278



Jan 31 '08

TC LL	20.0 PSF	REF	R8228- 76975
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCSR8228 08031089
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43949
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

... ..

SPECIAL LOADS

	(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From	66 PLF at 0.00 to 66 PLF at 9.07
TC - From	66 PLF at 9.07 to 66 PLF at 18.15
BC - From	20 PLF at 0.00 to 20 PLF at 18.15
PLB -	42 LB Conc. Load at (7.88,9.04), (11.88,9.04), (13.42,9.04)

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

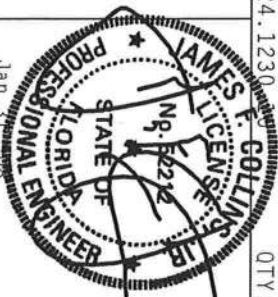
LOADING HAS BEEN CALCULATED BY THE TRUSS MANUFACTURER. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO VERIFY AND APPROVE THE LOADING.



Scale = .3125" / Ft.



Haines City, FL 33844
FL Certificate of Authorization # 00779



TC LL	20.0 PSF	REF	R8228- 76976
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031090
BC LL	0.0 PSF	HC-ENG	EC/MHK
TOT.LD.	40.0 PSF	SEQN-	43971
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

(dear--OWNER BUILDER deardorf - , ** - T29)

Roof overhang supports 2.00 psf soffit load.

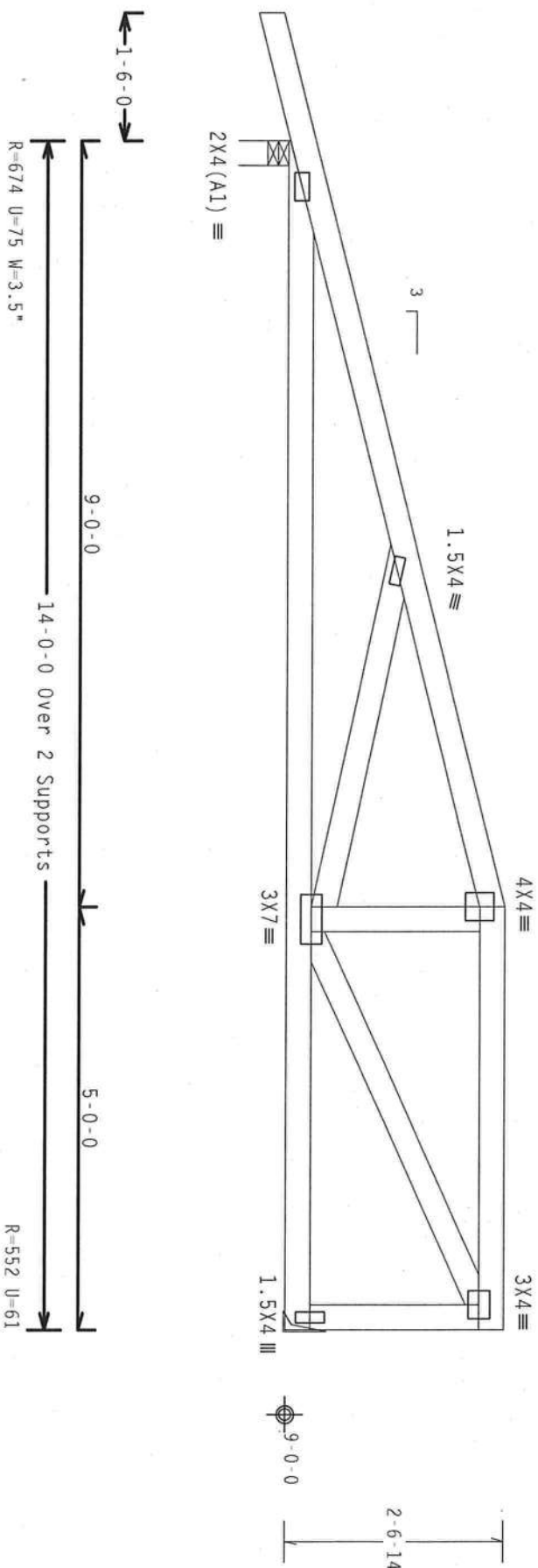
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 Gcpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

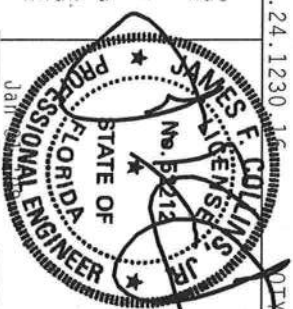
7.24.1230 16mm
VIX:1

~~NOTY: 1~~

FL/-/4/-/E/-/-

Scale = .5" / Ft.

WARNING: PRIORS (REUSE EXISTING CASE IN FABRICATION), WELDING, SHIPPING, INSTALLING AND PROTECTING REINFORCING BARS (REUSE EXISTING CASE IN FABRICATION), PUBLISHED BY THE TRUSS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, PHONE (703) 690-0100 TRUSS CONSULTING OF AMERICA, 65000 INTERSTATE LANE, MANDALAY, HI 96719 FOR SAFETY PRACTICES AND PRIOR TO REMOVAL OF THE STRUCTURES, INTERSECTIONS INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED CHORD CEILING.

[illegible]

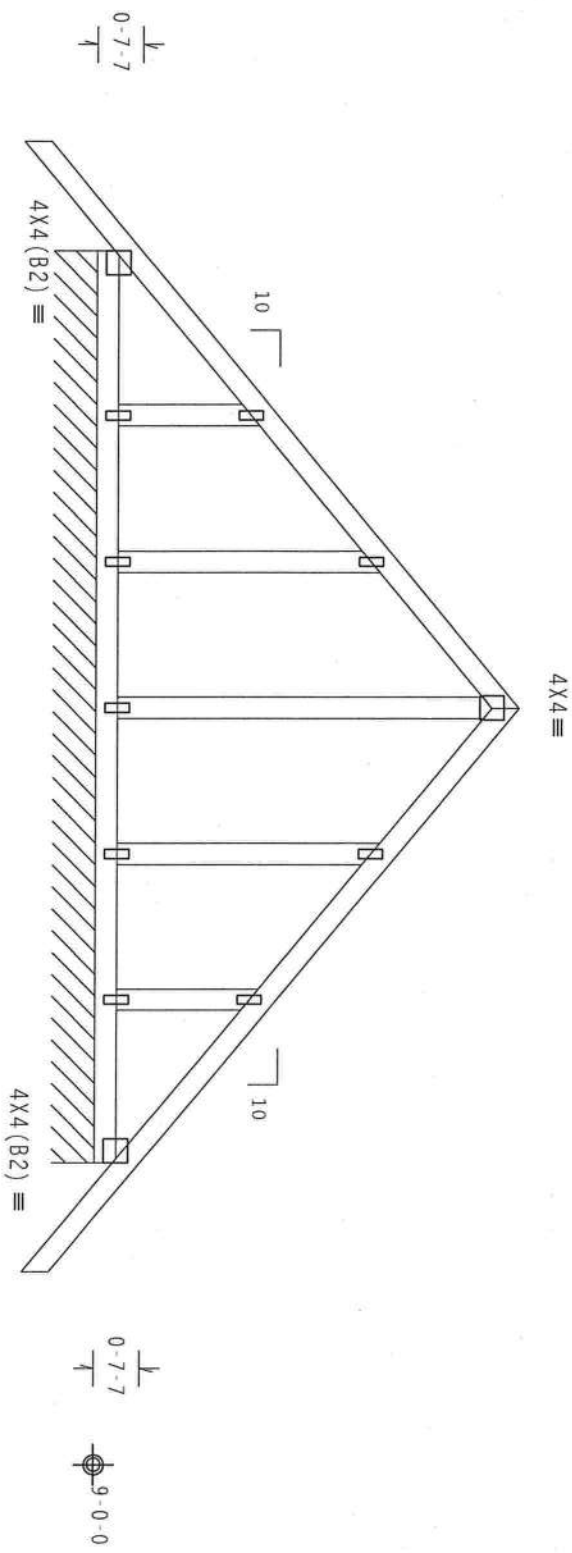
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031091
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43821
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

(dear--OWNER BUILDER deardorf -- , ** - T3)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 GCPI(+/-)=0.18
Wind reactions based on MWFRS pressures.
See DWGS A11015FE0207 & GBLETTIN0207 for more requirements.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



1-6-0
6-3-0
12-6-0 Over Continuous Support
R-206 PLF U=53 PLF W=12-6-0

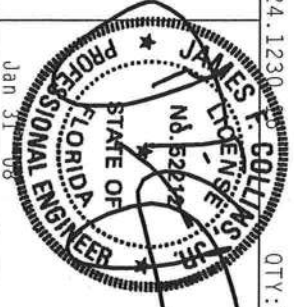
Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

WARNING MUSTERS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO MCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO MCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERX A3 OF TPI-2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMSI/TPI 1 SEC. 2.



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Haines City, FL 33844
TPI Certificate of Authorization #0-778

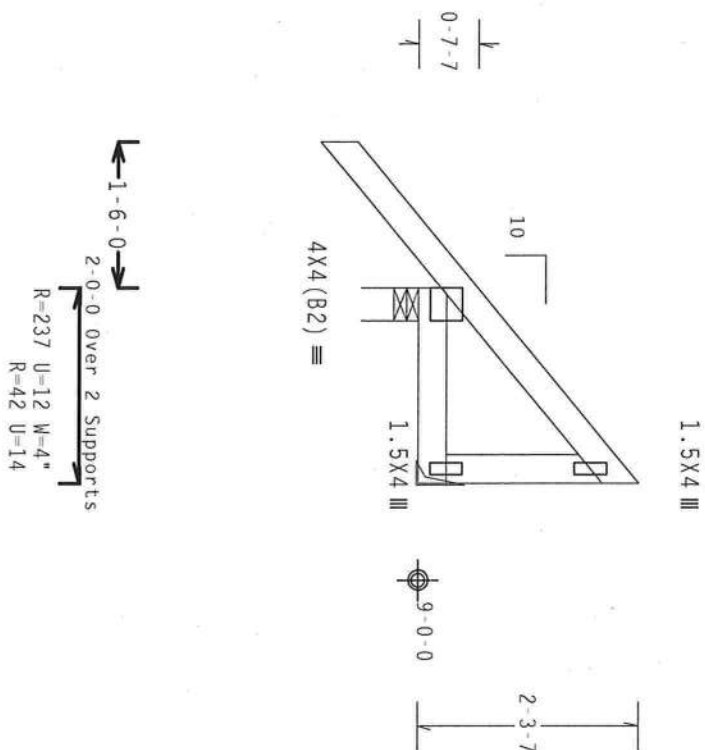
OTV:1	FL/-/4/-/E/-/-	Scale = .375"/ft.
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TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031092
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEON- 43777
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- ITEL8228203

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
Webs	2x4	SP	#3	

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, Cat II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

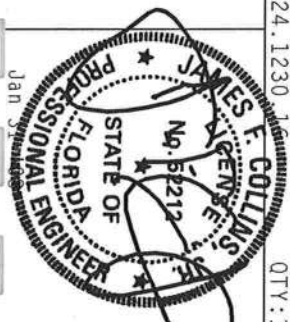
Scale = .5" / Ft.

WARNING: *MOLDS (BULIDING EXPERTISE CASE IN THE INFORMATION), MANDATORY, CHOPPING, DISTASTING, AND PROTECTING REFER TO M231 (BULIDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRESS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 65000 ENTERPRISE LANE, SUITE 501, 51739) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. UNDESIRABLE CONDITIONS INDICATED FOR GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.



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Haines City, FL 33844
FL Certificate of Authorization # 00778

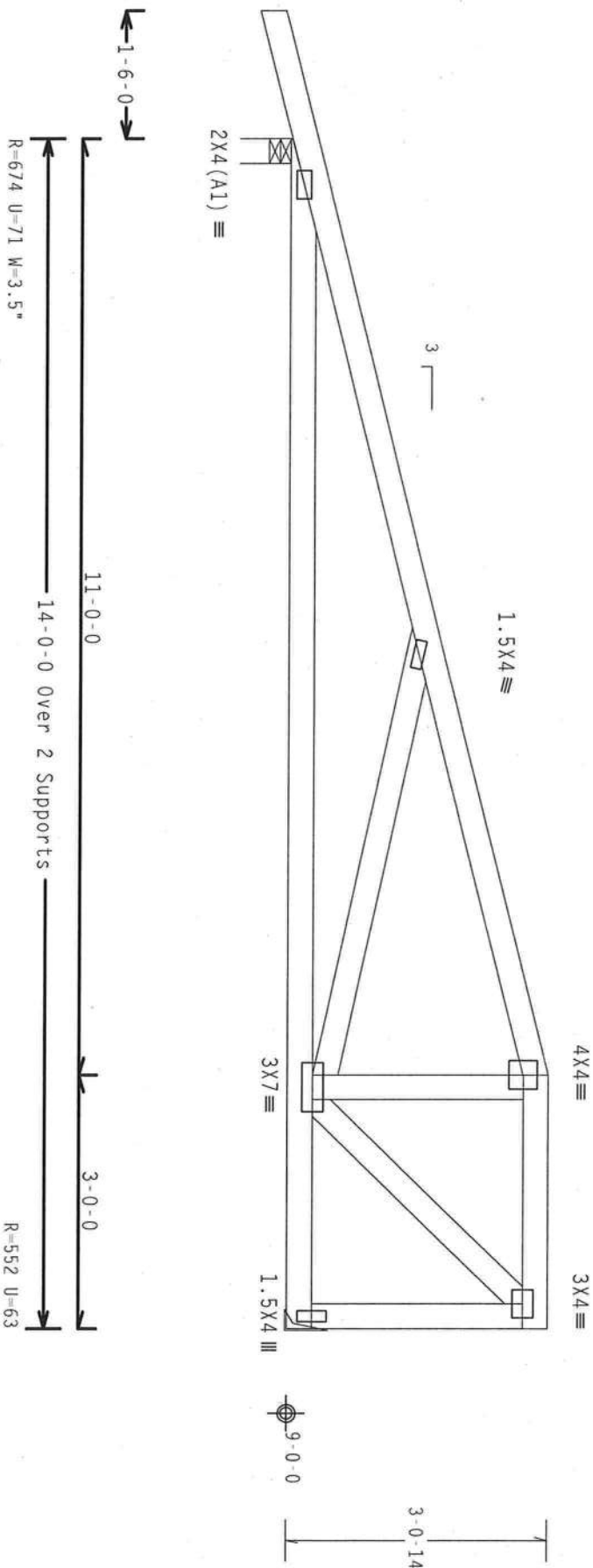


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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCU8R8228 08031093
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEON-	43811
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	Wabs	2x4	SP	#3	

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ Gcpl (+/-)=0.18



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230.16

QTY:1

FL/-/4/-/E/-/-

Scale = .5" / Ft.



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0778

****WARNING**** BUILDERS REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO EC31 (BUILDING COMPONENTS EXISTENCE INFORMATION), PUBLISHED BY THE (STEEL PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (4000 TRUSS COMPANY, OF AMERICA, 6300 FARMERS ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TYPE OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE REQUIREMENTS OF NDS (NATIONAL DESIGN SPEC. BY AISC), AND TPI.

CONNECTION PLATES ARE MADE OF 2010/16GA (H/155/25) ASTM A563 GRADE 50/60 (W K/L/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMX AS OF TPI-2002 SEC.3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENTS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 76980
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031094
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43823
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

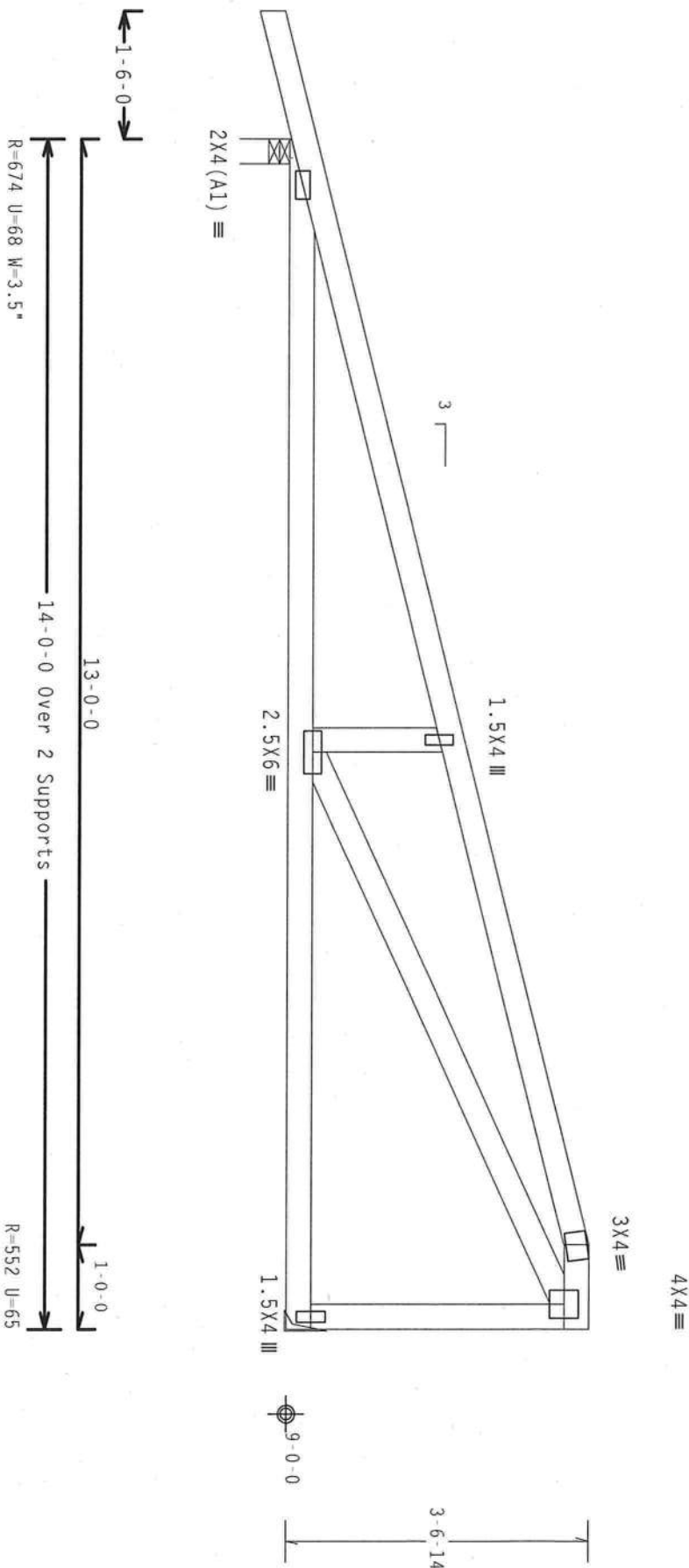
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, IW=1.00 Gcpi(+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

QTY: 1

FL/-/4/-/E/-/-

Scale = .5"/ft.

****WARNING**** MUSTERS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. REFER TO DCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CITRUS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22319 AND WICA (WOOD TRUSS CONCRETE OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CORRELATION WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AISC) AND TPI. CONNECTIONS ARE MADE OF 20/10/160A (24/10/160A) ASH 6053 GRADE 40/60 (4. 6/11.55) GALV. STEEL. THE BCG DESIGNER SHALL BE RESPONSIBLE FOR THE DESIGN, CONSTRUCTION PER DRAWINGS, 160A/2. ANY INSPECTION OF PLATES FOLLOWED BY ALL SHALL BE THE RESPONSIBILITY OF THE TRUSS COMPONENT DESIGNER. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Haines City, FL 33844
FL Certificate of Authorization #0378

TC LL	20.0 PSF	REF	R8228 - 76981
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCU8228 08031095
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT. LD.	40.0 PSF	SEQN-	43825
DUR. FAC.	1.25	FROM	JP
SPACING	24.0"	UREF-	1TEL8228203

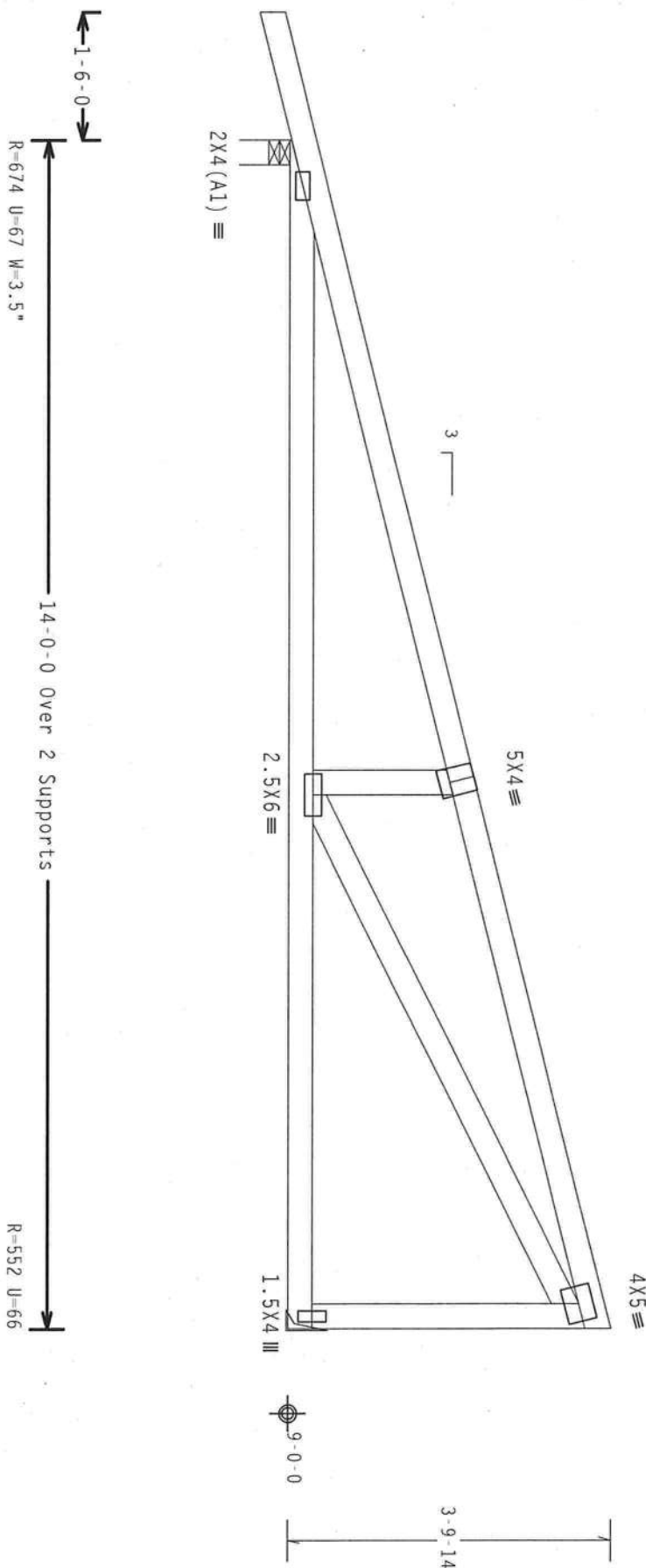
	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	webs	2x4	SP	#3		

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind; 15.00 ft mean hgt, ASCE 7-02, closed bldg, not located within 4.50 ft from roof edge, Cat II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ $G_{cpi} (+/-) = 0.18$

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/Ft.

WARNING WHILES BUILDING COMPONENTS EXISTENCE CASE IN INFORMATION, HANDLING, SHIPPING, INSTALLING AND BROCKING REFER TO BECI (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND (800) TRUSS CENTER OF AMERICA, 6500 MIDWAY DRIVE, ENTERPRISE LAKE, MISSISSAUGA, ONTARIO, CANADA L4X 1L7 FOR SAFETY PRACTICES WITH REGARD TO PERFORMING THESE FUNCTIONS. UNDESIRABLE EFFECTS INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT**

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 160A-2 CONNECTION PLATES ARE MADE OF 20/10/1600A (M, H/35/K) AS1610053 GRADE 40/50 (M, K/H/35) GALV. STEEL. APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

330
QTY: 1

James F. Collins
Professional Engineer
No. 62212
State of Florida
Jan 31 08

CO TC IPO

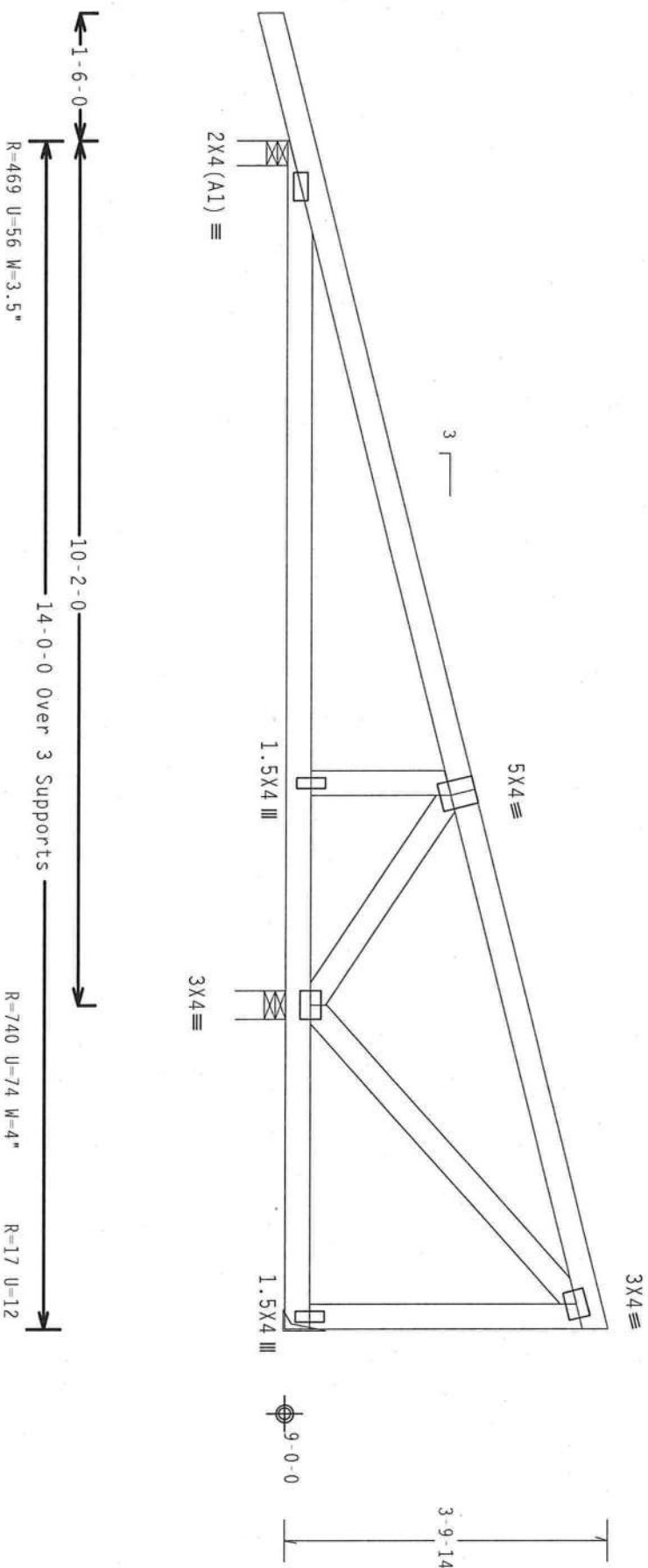
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCU8R8228 08031096
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43827
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

(dear--OWNER BUILDER deardorf -- , ** - T34)

Roof overhang supports 2.00 psf soffit load.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

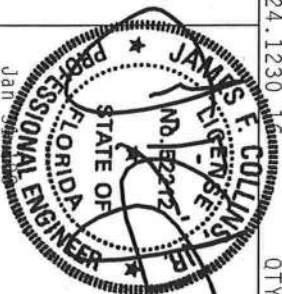
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Building Components Group Inc.

Haines City, FL 33844

FI Certificate of Authorization # 0 278

[illegible]

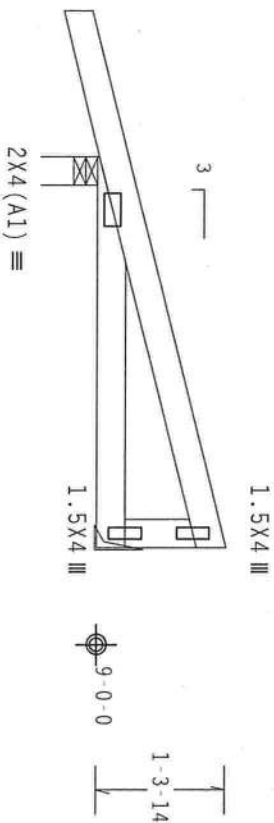
FL/-/4/-/E/-/-		Scale = .5"/Ft.	
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031097
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43829
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

(dear--OWNER BUILDER deardorf -- , ** - T35)

Roof overhang supports 2.00 psf soffit load.

Wind reactions based on MMFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



1-6-0

← 4-0-2 Over 2 Supports →
R=287 U=59 W=3.5" R=133 U=14

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

 $C_q/RT=1.00(1.25)/0(0)$

7.24.1230.16

OTY:1

FL/14/1E/1-

Scale = .5" / Ft.

WARNING *MOLDS* *GROWING* *ON* *THE* *EXTERIOR* *FACE* *IN* *FABRICATION*, *HANDLING*, *SHIPPING*, *INSTALLING* *AND* *BRACING* *REFER* *TO* *BCS1* *(BUILDING* *ROUTING* *COMPONENT* *SAFETY* *INFORMATION)*. *PUBLISHED* *BY* *TPI* *(TRUSS* *PLATE* *INSTITUTE)*, *218* *NORTH* *LEE* *STREET*, *SUITE* *312*, *ALEXANDRIA*, *VA.,* *22314* *OR* *CALL* *(800) *TRUSS* *COUNCIL* *OF* *AMERICA*, *63500* *ENTERPRISE* *LANE*, *MIDWEST*, *AL* *53191* *FOR* *SAFETY* *PRACTICES* *PREFACE* *TO* *PERFORMING* *THESE* *FUNCTIONS*. *UNDESIGNED* *MEMBERS* *INDICATED* *FOR* *CHORD* *SHALL* *HAVE* *PROPERLY* *ATTACHED* *STRUCTURAL* *PANELS* *AND* *BOTTOM* *CHORD* *SHALL* *HAVE* *PROPERLY* *ATTACHED* *RIGID* *CEILING*.

[illegible]

Building Components Group Inc

Haines City, FL 33844

FI Certificate of Authorization # 0778



TC LL	20.0 PSF	REF	R8228- 76984
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031098
BC LL	0.0 PSF	HC-ENG	EC/WHK *
TOT.LD.	40.0 PSF	SEQN-	43835
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	ITEL8228Z03

M.M. KAMINSKY (KAMENSKAYA) SUBSTITUCED DIIMIDES

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf 1W=1.00 GCPI(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

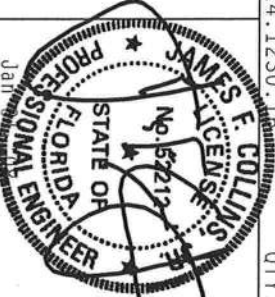
 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230.16

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/Ft



TC LL	20.0 PSF	REF	R8228 - 76985
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031099
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43831
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREFE -	ITEI R2282703

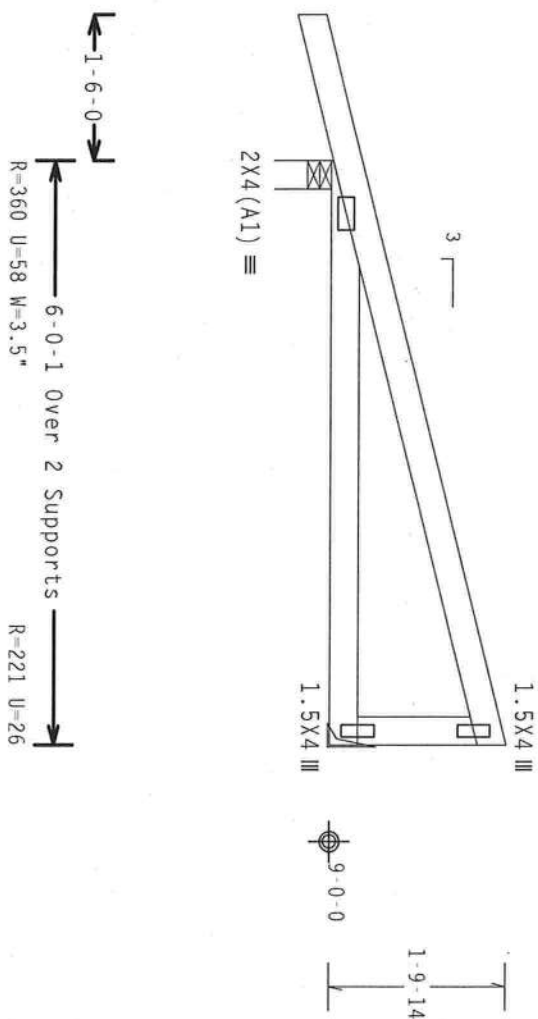
JREF - 1TEL8228Z03

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind; 15.00 ft mean hgt, ASCE 7-02, closed bldg, not located within 4.50 ft from roof edge, Cat II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCPI (+/-) 0.18

Wind reactions based on MWFRS pressures.



Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230 16mm
QTY:1

QTY:1 FL/-/4/-/E/-/-

Scale = .5" / Ft.

WARNING—PROCESSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BROCKING REFER TO RC21 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY FBI STRESS PAPER INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND BRICA (IRON TRUSS COUNCIL OF AMERICA, 65000 ROCKY HILL ENTERPRISE LAKE, MADISON, WI, 53719) FOR SAFETY PRECAUTIONS TO GUARANTEE THESE PRODUCTS. UNDESIRABLE OVERSTRESS INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTION PLATES ARE MADE OF 20/18/16GA (W, H/SS/R) ASTM A653 GRADE 40/60 (W, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 1604-1

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT AND INSPECTION OF DETAILS FOLLOWED BY (1) SHALL BE PER ANNEA AS OF 1P13-2002 SEC.3. A SEAL ON THIS

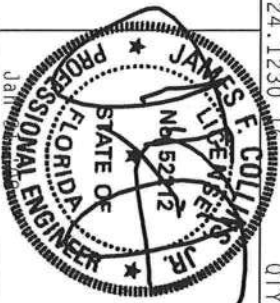
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

100

Haines City, FL 33844
FL Certificate of Authorization # 0 278



Building Components Group Inc.



TC LL	20.0 PSF	REF	R8228- 76986
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031100
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43833
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

(dear--OWNER BUILDER deardorf -- , ** - T38)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

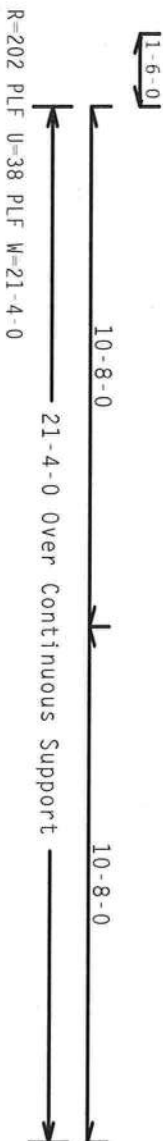
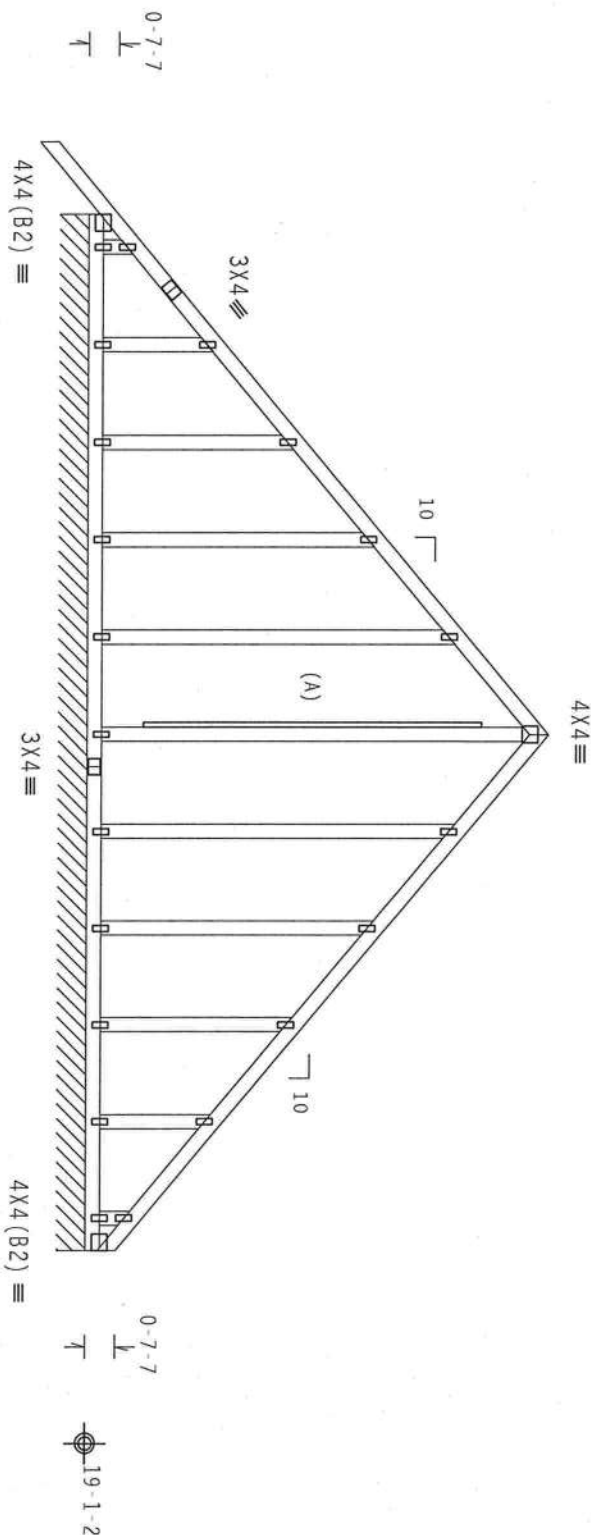
THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF THE ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, AND SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO THE GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

110 mph wind, 23.53 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MMFRS pressures.

See DWGS A11030FE0207 & GBLETTIN0207 for more requirements.

(A) 1x4 #3 or better "L" brace, 80% length of web member. Attach with 8d box or gun (0.113"x2.5", min.) nails @ 6" OC.



Note: All Plates Are 1.5X4 Except As Shown.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY:1

FL/-/4/-/E/-/-

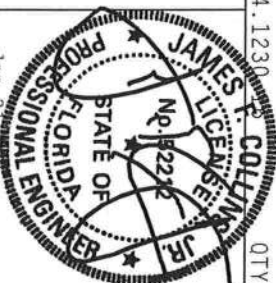
Scale = .25"/ft.

WARNINGBUILDES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

Haines City, FL 33844
TPI Certificate of Authorization #0778



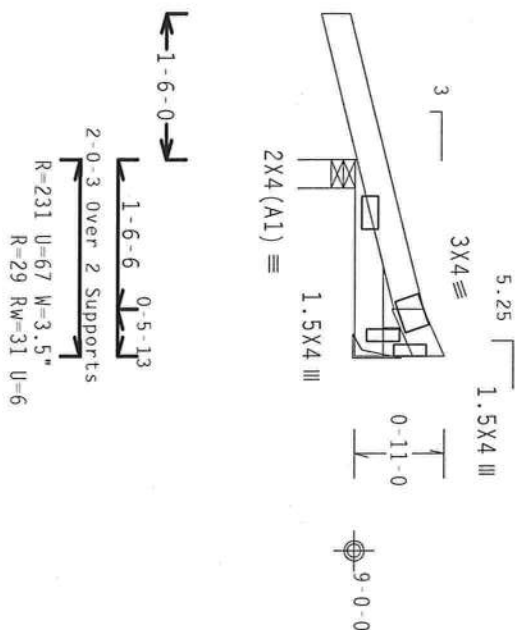
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031101
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEON-	43786
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP 8, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCFI (+/-)=0.18

Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

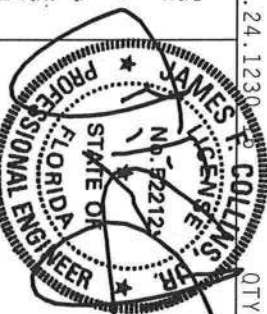
Scale = .5" / Ft.

WARNING—WHILES (BULIDING EXTERING CASE IN FABRICATION), SHIPING, SHIPING, INSTALLING, AND BRACKING REFER TO BC31 (BULIDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRESS PAIL INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS CENTER OF AMERICA, 65000 INDUSTRIAL LANE, MOUNTAIN, MI 48159) FOR SAFETY PRACTICES AND FOR DETERMINING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH

Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0078



Jan 31 08

TC LL	20.0 PSF	REF	R8228- 76988
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031102
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43837
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

LEARNING AND DIFFERENTIATION) SUBSTITUTED BY INCREASED

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCDF(+)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical exposed to wind pressure. Deflection meets $L/240$ criteria for brittle and flexible wall coverings.

In lieu of structural panels use purlins to brace all flat TC @ 24"

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from



R=2202 U=92 W=4"

Shown.

 $\text{Ca}/\text{RT}=1.00(1.25)/0(0)$

OTY:1

Scale = 1875"/Ft

317C



INTERNATIONAL ENVIRONMENTAL

Jan 31 2025

TC LL	20.0 PSF	REF	R8228 - 76989
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031077
BC LL	0.0 PSF	HC-ENG EC/WHK	
TOT.LD.	40.0 PSF	SEQN-	73650
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

(dear--OWNER BUILDER deardorf -- , ** : 140)
Top chord 2x4 SP #2 Dense : T1 2x8 SP SS:
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at -1.50 to 64 PLF at 9.13
TC - From 64 PLF at 9.13 to 64 PLF at 35.21
BC - From 5 PLF at -1.50 to 5 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 35.21
TC - 207 LB Conc. Load at 9.19, 11.19, 13.19, 15.19, 17.19
19.19, 21.19, 23.19, 25.19, 27.19, 29.19, 31.19, 33.19, 34.90
BC - 1234 LB Conc. Load at 9.12
BC - 81 LB Conc. Load at 11.19, 13.19, 15.19, 17.19, 19.19
21.19, 23.19, 25.19, 27.19, 29.19, 31.19, 33.19, 34.90

Wind reactions based on MMFRS pressures.

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

LOADING HAS BEEN CALCULATED BY THE TRUSS MANUFACTURER.
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO
VERIFY AND APPROVE THE LOADING.

2 COMPLETE TRUSSES REQUIRED

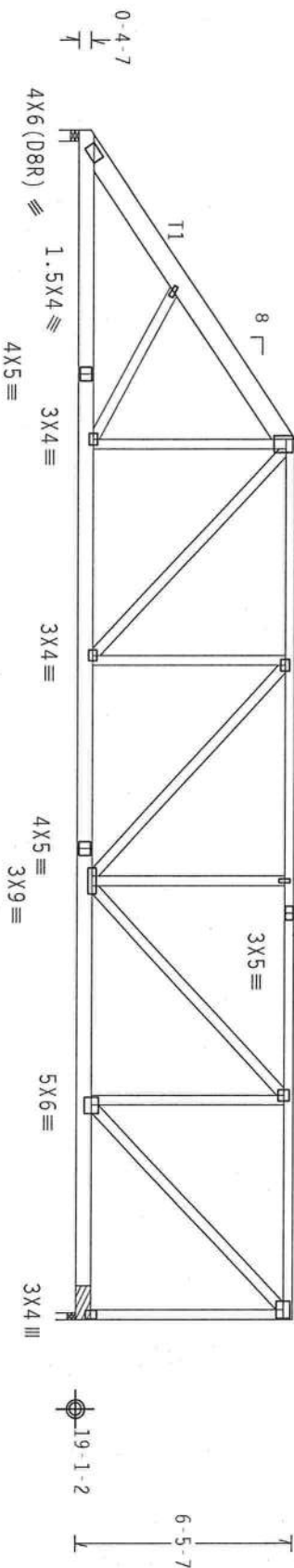
Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)
Top Chord: 1 Row @ 4.00" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Bearing blocks: Nail type: 12d Common (0.148"x3.25", min.)_nails
BRG X-LOC #BLOCKS LENGTH/BLK #NAILS/BLK WALL PLATE
2 35.000' 1 12" 4 Rigid Surface
Bearing block to be same size and species as bottom chord.
Refer to drawing CNBRGK0207 for additional information.

110 mph wind, 22.01 ft mean hgt, ASCE 7-02, CLOSED bldg, not
located within 4.50 ft from roof edge, CAT II, Exp B, wind TC
DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCpi(+/-)=0.18

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.



9-1-8 26-1-0
35-2-8 Over 2 Supports
R=3859 U=1197 W=4"
R=4283 U=1452 W=2.5"

PLT TYP. WAVE

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

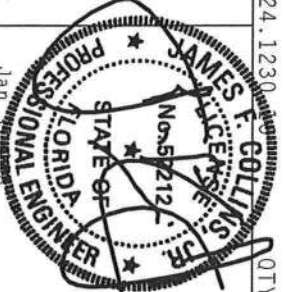
QTY:1

FL/-/4/-/E/-/-

Scale = .1875"/ft.

WARNINGMUSSES REQUIRE EXTREME CARE IN FABRICATING, WARNING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
THIS: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AIA/NSI) AND TPI.
PRODUCTION PLATES ARE MADE ON 20/10/1604 (24/15/25/25) ASH/ALSA GRADE 40/60 (4, 6/10, 25) GALT, STEEL. APPLY
PROTECTIVE COATING TO ALL EXPOSED SURFACES. THE TRUSS SHALL BE PERMANENTLY IDENTIFIED BY THE BCG, INC.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE THE OWNER AS OF THE 2002 SPECIFICATION PER DRAWINGS OF THE
DRAWING. INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPANIES
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Haines City, FL 33844
FL Certificate of Authorization #0378

TC LL	20.0 PSF	REF R8228-76990
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031103
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT. LD.	40.0 PSF	SEQN-43908
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF-1TEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5".min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

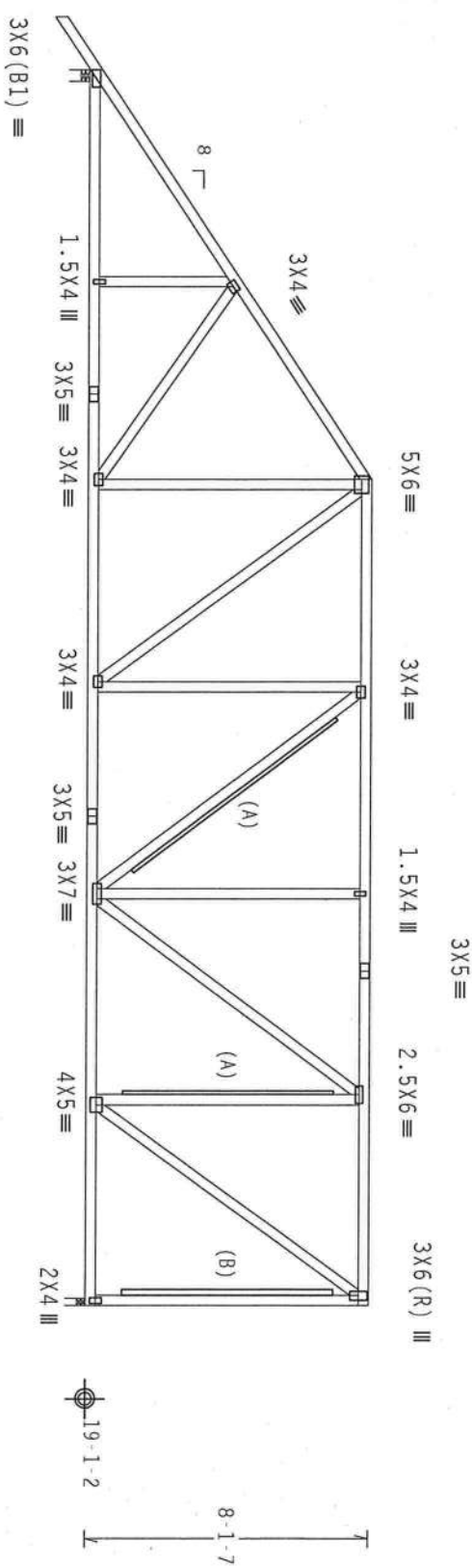
110 mph wind. 22.84 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 gcpl(+/-)-0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5".min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



1-6-0
11-7-8
23-7-0
35-2-8 Over 2 Supports
R=1591 U=256 W=4"
R=1471 U=295 W=2.5"

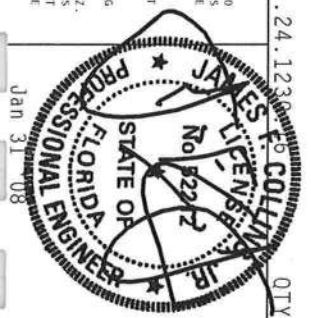
PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

WARNING MUSTS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22319 AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

Haines City, FL 33844
TPI Certificate of Authorization #0778



QTY:1 FL/-/4/-/E/-/- Scale = .1875"/Ft.

TC LL	20.0 PSF	REF R8228- 76991
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031104
BC LL	0.0 PSF	HC-ENG EC/WHK *
TOT.LD.	40.0 PSF	SEON- 43840
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

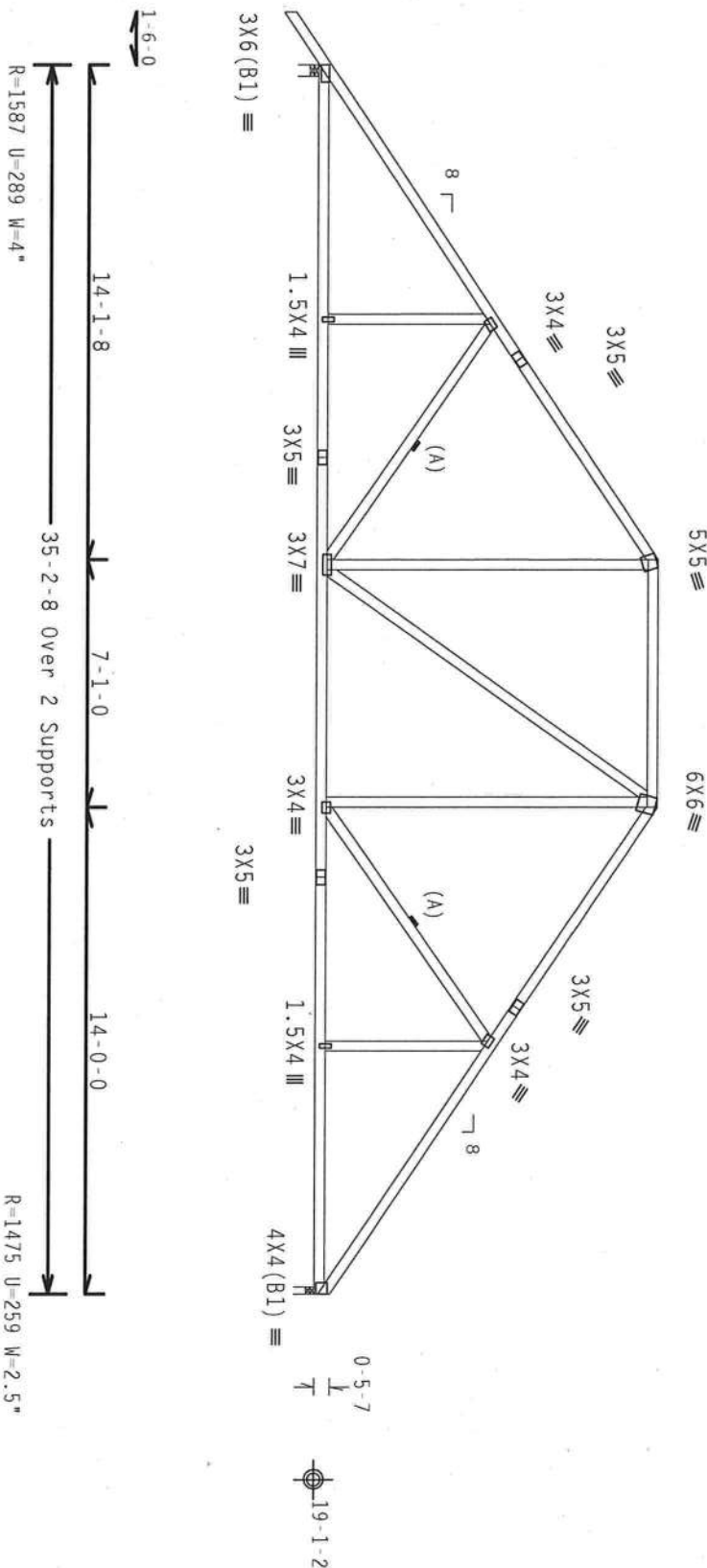
Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

(A) Continuous lateral bracing equally spaced on member.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind; 23.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ Gcpi (+/-) 0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230.56 COTY:1

QTY:1 FL/-/4/-/E/-/-

Scale = .1875"/Ft.

WARNING—WHILES (BUILDING EXTREME CASE IN FABRICATION), HANDLING, SHIPPING, INSTALLING, AND PROTECTING REFER TO DESI (ROUTING COMPONENT SET INFORMATION). PUBLISHED BY THE STRESS PRACTICE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND TRUSS COUNCIL OF AMERICA, 65000 INDUSTRIAL ENTERPRISE LANE, SUITE 410, W-31319 FOR SAFETY PRACTICES, PRIOR TO PERFORMING THESE OPERATIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****-FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

1P1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH ADDITIONAL PROVISIONS OF AISC (SECTION 10.10.2) AND THE

CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/SS/K) ASTM A653 GRADE 40/60 (W, K/H, SS) GALV. STEEL, APPLY

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING, IS THE RESPONSIBILITY OF THE

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	52
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Haines City, FL 33844
FL Certificate of Authorization # 00779

QTY: 30
JAMES F. COLLINS, JR.
ENGINEER
No. 15224
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Jan 31 08

TC LL	20.0 PSF	REF	R8228 - 76992
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCSR8228 08031105
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEON -	43842
DUR.FAC.	1.25	FROM JP	
SPACING	24.0"	JREF -	1TE18228203

```
( ( dear--OWNER BUILDER deardorf -- , ** - T44 )
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3 COMPLETE TRUSSES REQUIRED

17s)

Top chord:	1 Row	@ 4.00" O.C.
Bot Chord:	1 Row	@ 12.00" O.C.
Walls:	1 Row	@ 4" O.C.

-----	(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 63 PLF at -1.37 to 63 PLF at 11.76	
TC - From 63 PLF at 11.76 to 63 PLF at 20.48	

TC - From	64 PLF at 20.48 to
TC - From	63 PLF at 29.36 to
BC - From	20 PLF at -0.04 to

PLT- 287 LB Conc. Load at (11.74, 26.32), (13.74, 26.34), (15.08, 26.34)
PLT- 287 LB Conc. Load at (11.74, 26.32), (13.74, 26.34), (15.08, 26.34)

PLB-	191 LB Conc.	Load at (9.67,19.13)	(13.74 19.13)	(15.08 19.13)
P1B-	46 LB Conc	load at (11.74 19.13)		

PLB-1569 LB Conc. Load at (20.48, 19.13)

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=3456 U=1408 W=4*

Diagram of a continuous beam with three spans. The spans are labeled 11-9-1, 8-8-12, and 8-10-8. The supports are labeled 5-3-4. The total length is 34-7-9 over 2 supports.

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .1875"/Ft.

****IMPORTANT** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT**

Building Components Group Inc.

Haines City, FL 33844

FL Certificate of Authorization # 00778

00 TC 1180

TC LL	20.0 PSF	REF R8228- 76993
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031106
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEQN- 44006
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

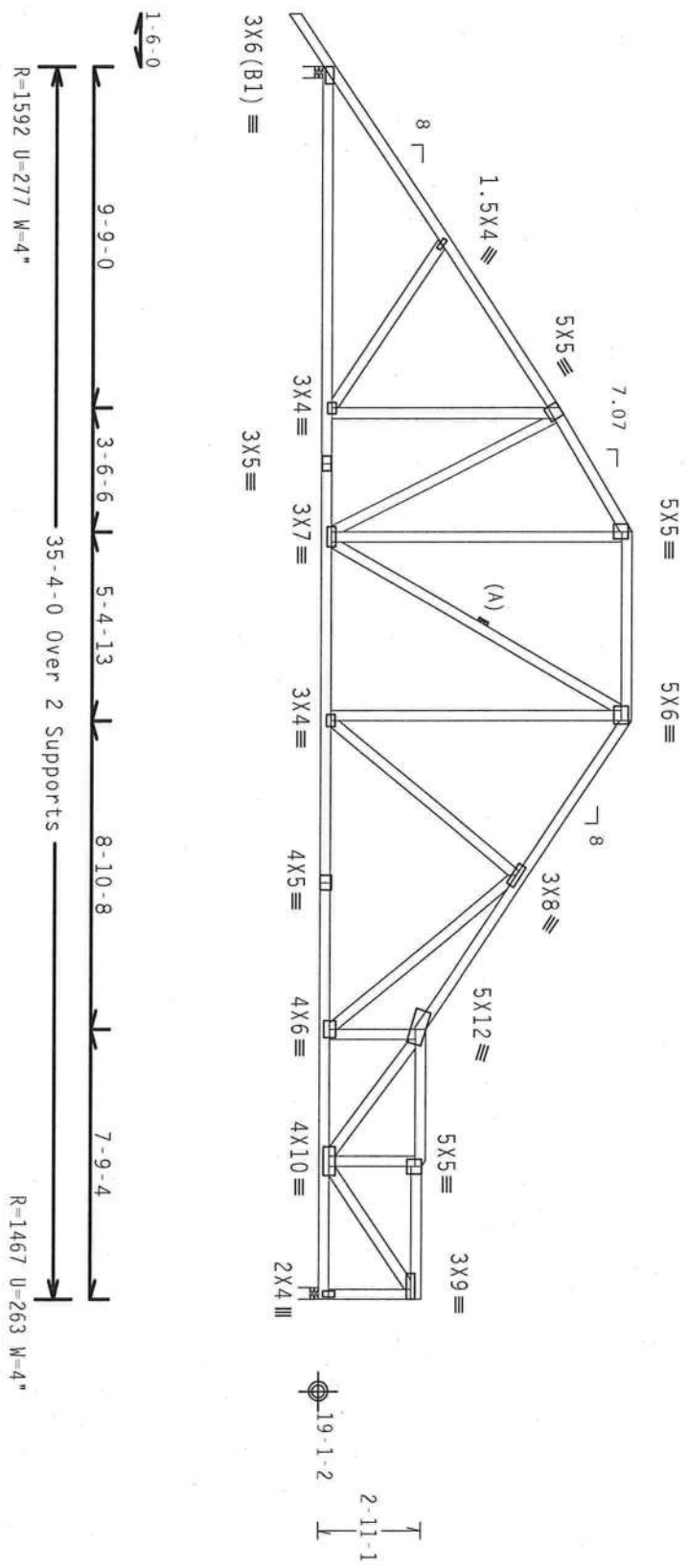
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 23.26 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 1 FL/-/4/-/E/-/-

Scale = .1875"/ft.



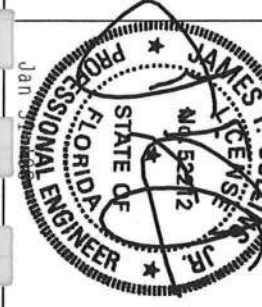
Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization #0-278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA, ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

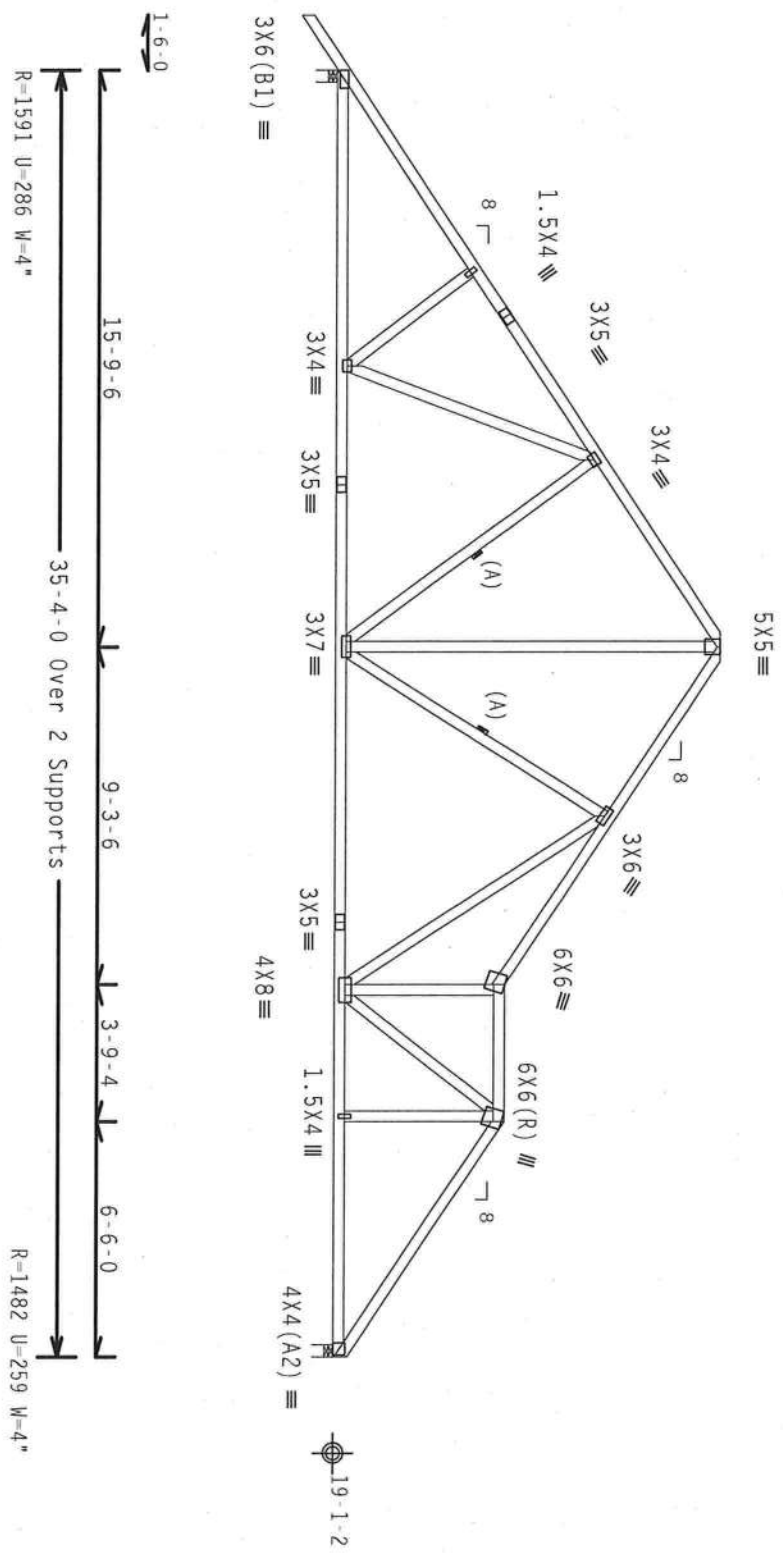
THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.



TC LL	20.0 PSF	REF	R8228- 76994
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUS8228 08031107
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEON-	43994
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	UREF-	ITEL8228203

(A) Continuous lateral bracing equally spaced on member. Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 24.09 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ Gcpi (+/-)=0.18



PLT TYP. Wave	Design Crit:	TPI-2002 (STD) /FBC
	Cq/RT=1.00	(1.25) /

7.24.1230.16 QTY:1 FL/-/4-/E/-/- Scale =.1875"/Ft.

Building Components Group Inc.

Haines City, FL 33844

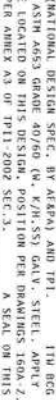
FL Certificate of Authorization # 0 278



WARNING *HOUSE* REQUIRE EXTREME CARE IN INFORMATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** *FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TPI DCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

CONNECTION PLATES ARE MADE OF 20/18/16GA (U./I./SS/Y) ASTM A653 GRADE 40/60 (K./H./SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMES AS OF THIS 2002 SEC. 2. A SEAL ON THIS DESIGN INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SILENT FOR THE TRUSS COMPONENT DESIGNER. THE USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AMSE/TP1 1 SEC. 2.



Jan

TC LL	20.0 PSF	REF R8228- 76995
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 0803108
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT. LD.	40.0 PSF	SEON- 43868
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228Z03

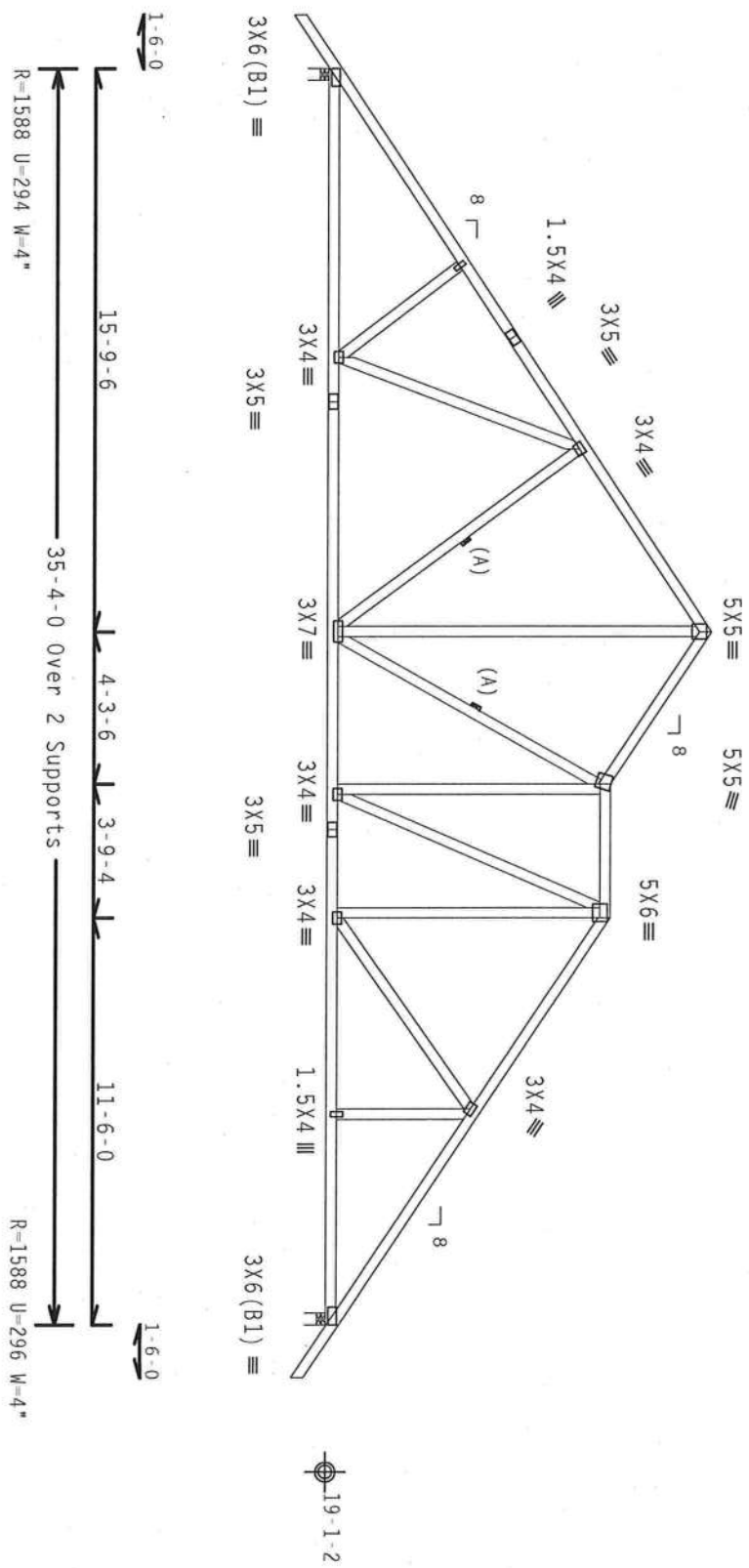
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 24.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. IW=1.00 Gcpl(+/-)-0.18
Wind reactions based on MWFRS pressures.
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230 TC

QTY: 1

FL/-/4/-/E/-/-

Scale = .1875"/ft.

Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization #0 778

WARNING* ROOFERS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 ENTERPRISE LANE, MANTOLONG, NJ 08719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR THE PROPER FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

TC LL	20.0 PSF	REF	R8228- 76996
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031109
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEON-	43874
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	ITEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.96 ft mean hgt, ASCE 7-02, CLOSED bldg, not
located within 4.50 ft from roof edge, CAT II, EXP B, wind TC
DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Roof overhang supports 2.00 psf soffit load.

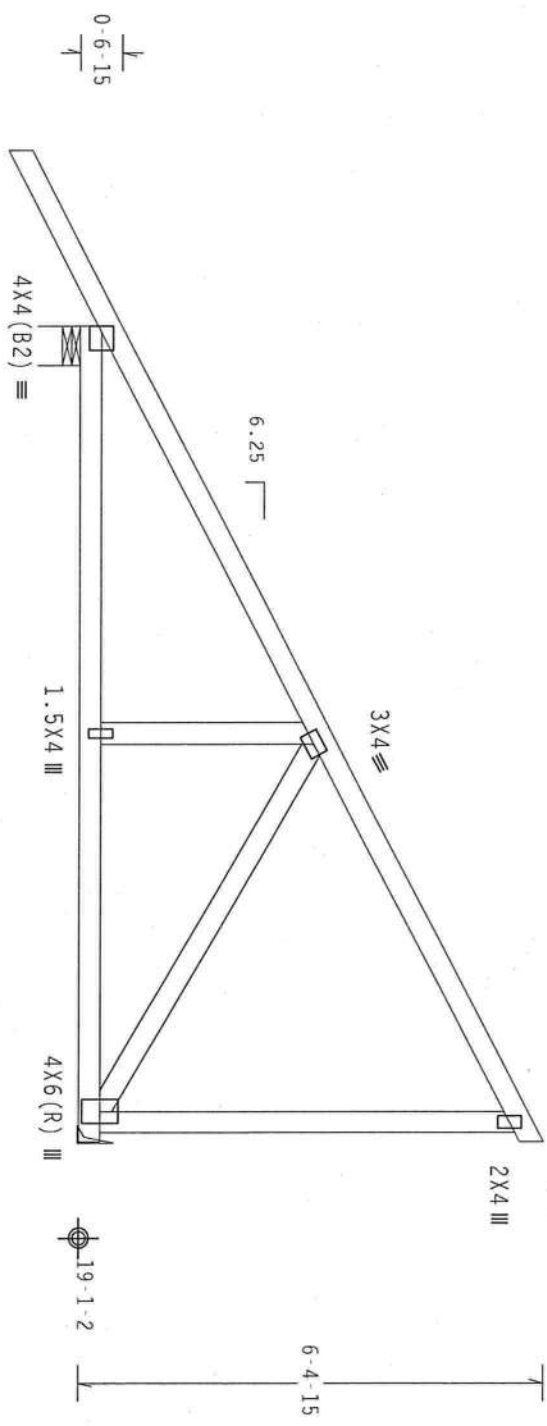
Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

LOADING HAS BEEN CALCULATED BY THE TRUSS MANUFACTURER.
IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO
VERIFY AND APPROVE THE LOADING.

SPECIAL LOADS

TC - From	DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25
BC - From	63 PLF at -2.40 to 63 PLF at 11.21
BC - From	5 PLF at -2.40 to 5 PLF at 0.00
BC - From	20 PLF at 0.00 to 20 PLF at 11.21
TC -	18 LB Conc. Load at 2.94
TC -	103 LB Conc. Load at 4.87
TC -	88 LB Conc. Load at 5.51
TC -	325 LB Conc. Load at 8.07
TC -	194 LB Conc. Load at 10.63
BC -	10 LB Conc. Load at 2.94
BC -	41 LB Conc. Load at 4.87
BC -	32 LB Conc. Load at 5.51
BC -	130 LB Conc. Load at 8.07
BC -	75 LB Conc. Load at 10.63

Right end vertical not exposed to wind pressure.



R=949 U=311 W=6.403"

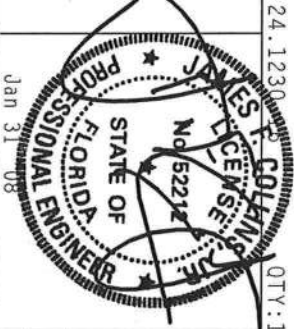
2-4-13

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

WARNINGBIDDERS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCS, INC. SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. THE TRUSS IN COMPLIANCE WITH
TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING, BY ACPA AND TPI. THE BCS
CONTRACTOR SHALL BE RESPONSIBLE FOR THE QUALITY OF THE TRUSS. THE BCS SHALL NOT BE
RESPONSIBLE FOR THE QUALITY OF THE TRUSS. THE BCS SHALL NOT BE RESPONSIBLE FOR THE QUALITY OF THE TRUSS.
PLATES TO EACH FACE OF TRUSS AND BY (1) SHALL BE PER AMERICAN OF TPI-2002 SEC.3. A SEAL ON THIS
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT
DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF R8228- 76997
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031110
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEON- 43906
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #2:
Bot chord 2x10 SP SS :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 9-0-1 to 21-0-1.

Right end vertical exposed to wind pressure. Deflection meets L/240 criteria for brittle and flexible wall coverings.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cg/RT=1.00(1.25)

OTY:1

F11-141-1F1-1-

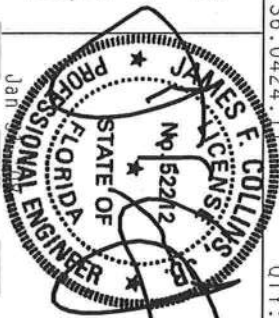
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WARNING—HAZARDOUS FIBROUS EXTREMELY TOXIC IN FABRICATION. HARMFUL, IRRITATING AND BRACING. REPAIRS TO BE COMPLETED BY THE INSTALLATION. COLLAPSE OF THE TRUSS PLATE 18x11x10, 210 NORTH LA STREET, MOBILE, AL 36680. PREPARED BY: ALABAMA DEPARTMENT OF CONSTRUCTION, 2500 UNIVERSITY BLVD., MOBILE, AL 36688. FOR SAFETY PURPOSES, THE FOLLOWING INFORMATION IS BEING FURNISHED. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PURLINS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHANGING.



Building Components Group Inc

Haines City, FL 33844
FL Certificate of Authorization # 0778



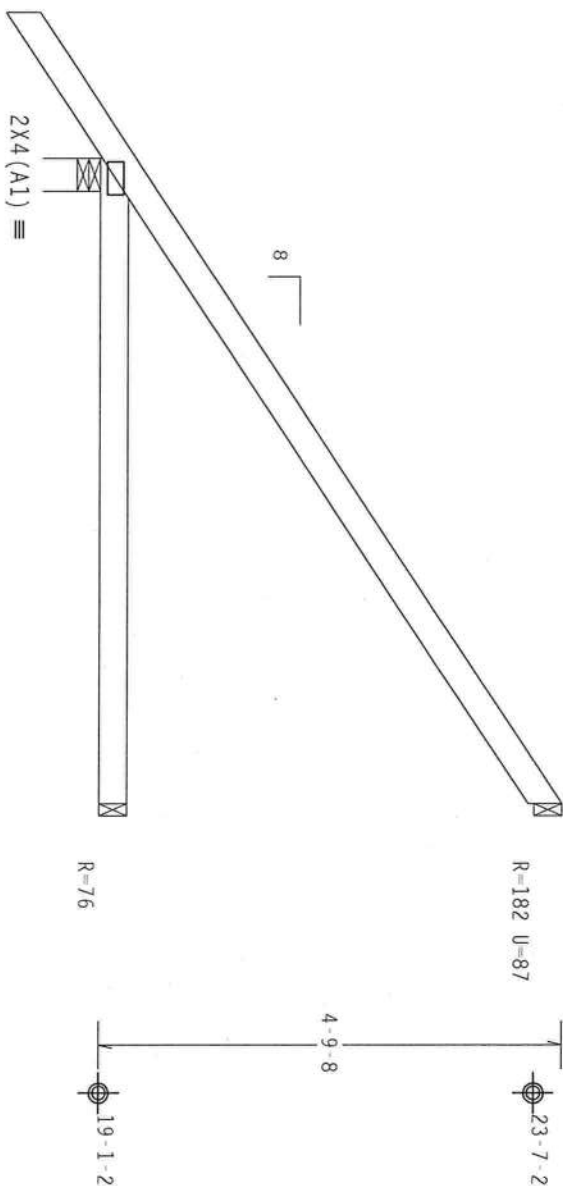
Scale = 1.0/5 / ft.	
IC LL	20.0 PSF
TC DL	10.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT. LD.	40.0 PSF
DUR. FAC.	1.25
SPACING	24.0"
JREF- 1TEL8228Z03	REF R8228- 76998
	DATE 01/31/08
	DRW HCSUR8228 08031078
	HC-ENG EC/WHK
	SEON- 73641
	FROM JP

INTRA-UTERINE FETAL SURVIVAL (LUNGS & BILIARY TRACT) SUBSTITUTION OF INTRACRANIAL

110 mph wind, 21.18 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT I, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpi (+/-)0.18

Wind reactions based on MWFRS pressures.

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/Ft.



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0079

WARNING—BOLTS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRIVING REFER TO EC31 (BUILDING CONSTRUCTION SAFETY INFORMATION), PUBLISHED BY THE STEEL PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND AISC (6000 TRUSS COMPANY OF AMERICA, 63000 ENTERPRISE LANE, MOBILE, AL 36619) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THIS DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TYPE ON FABRICATING, HANDLING, SHIPPING, INSTALLING AND DRIVING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TP1.

CONNECTION PLATES ARE MADE OF 2010/1664 (H.A./S.S.) ASTM A551 GRADE 40/60 (4' K/L/S5) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN AS OF TP1-1-2002 SEC.3.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSSES COMPONENTS OF THIS DESIGN SHOWS THE SUITABILITY AND USE OF THIS CONSTRUCTION FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AM/TP1 1 SEC. 2.



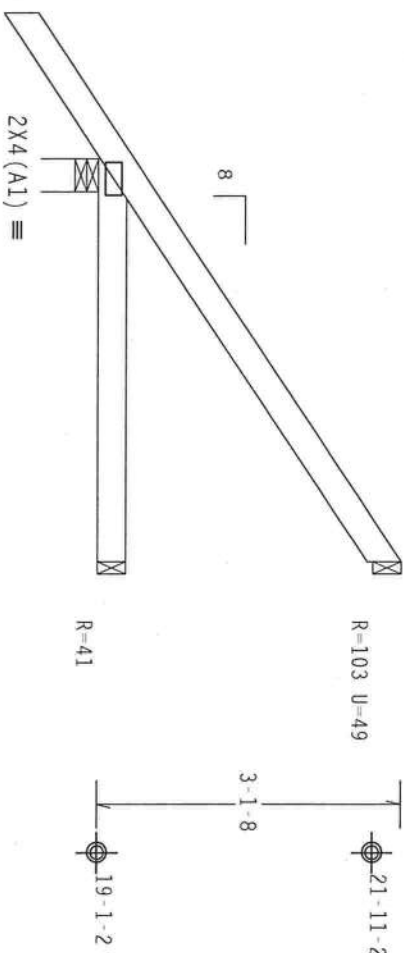
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031111
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN -	43850
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF -	1TEL8228203

```
( (dear--OWNER BUILDER deardorf --, ** - T51)
```

110 mph wind, 20.34 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ GCp1(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



A horizontal beam of length 10 feet is shown. A point load of 10 kips is applied at the left end. A uniformly distributed load of 3 k/ft is applied over the entire length of the beam. The beam is supported by three supports: a pin support at the left end, a roller support at 4 feet from the left end, and a roller support at the right end.

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5" / Ft.

FARRING MUSTS REQUIRE EXISTING CASE IN INFORMATION, HANDLING, SHIPMENT, INSTALLING AND BRACKETING TO BE DONE (BUILDING COMPONENT SAFETY INCORPORATION). PUBLISHED BY THE FARRING CASE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND ALSO GOOD TRUSS CONSULTING OF AMERICA, 6500 ENTERPRISE LANE, MONTICELLO, VT 55119 FOR SAFETY PRACTICES AND PRELIMINARY TRUSS DESIGN. UNDESIGNED OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC., SHALL NOT

TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. COULD FOR THE TRUCK COMPONENTS

DESIGNER OF THE BUILDING AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

[illegible]

Haines City, FL 33844
FL Certificate of Authorization # 0079

Building Components Group Inc.



2330 QTY

JAMES F. COLLINGS
No. 52212
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Jan 31, 2008

TC LL	20.0 PSF	REF	R8228- 77000
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 0803112
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43853
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

(dear--OWNER BUILDER dear-- ** - T52)

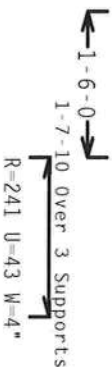
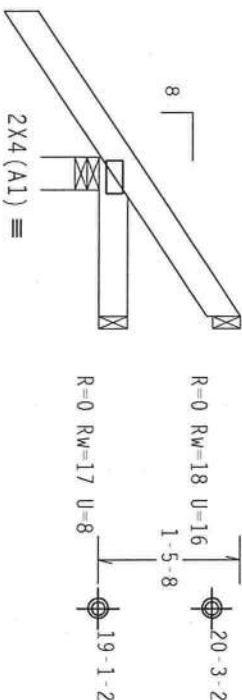
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.51 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

NOTY:1

FL/-/4/-/E/-/-

Scale =.5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION. PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (4000 TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

THIS TRUSS DESIGN IS THE PROPERTY OF ITW BCG, INC. AND IS NOT TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM. WITHOUT THE WRITTEN PERMISSION OF ITW BCG, INC. ANY REPRODUCTION OR TRANSMISSION OF THIS DESIGN IS PROHIBITED. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Building Components Group Inc.

Haines City, FL 33844

FL Certificate of Authorization #0 978



TC LL	20.0 PSF	REF	R8228- 77001
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031113
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43855
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	UREF-	1TEL8228203

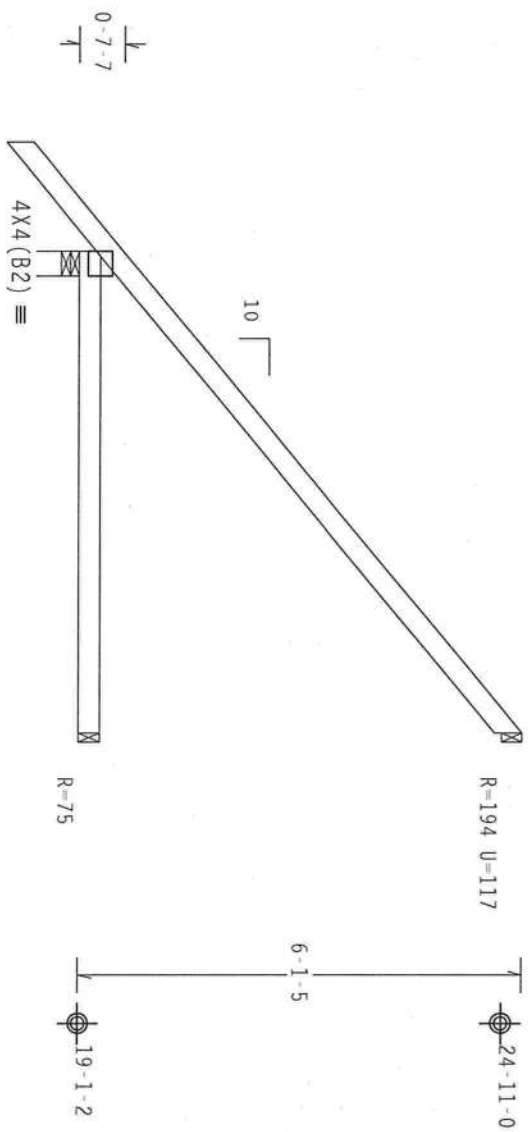
(dear--OWNER BUILDER dear-- . ** - T53)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 21.83 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, lw=1.00 GCPI(+/-)=0.18
Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY:1

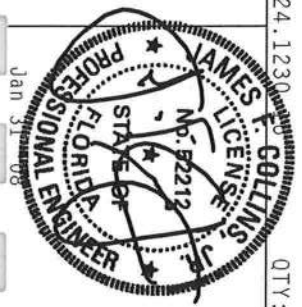
FL/-/4/-/E/-/-

Scale = .375"/ft.

****WARNING**** HUSSEY'S REQUIRE EXTERIOR CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 10155 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Haines City, FL 33844
FL Certificate of Authorization #0378



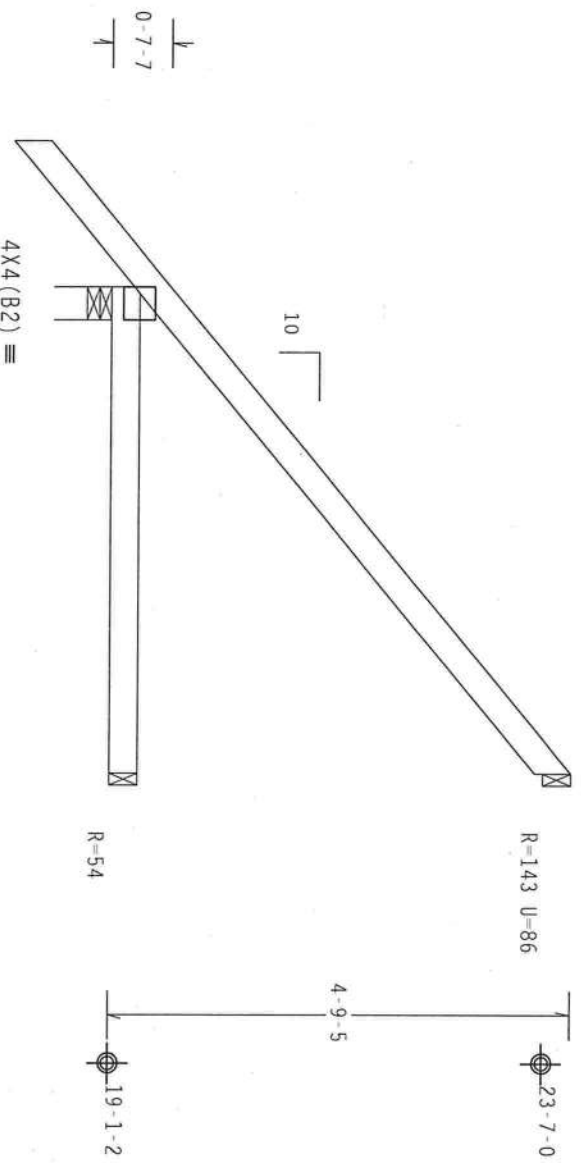
TC LL	20.0 PSF	REF	R8228- 77002
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031114
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT. LD.	40.0 PSF	SEQN-	43857
DUR. FAC.	1.25	FROM	JP
SPACING	24.0"	UREF-	ITEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 21.17 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{cpl}(+/-)=-0.18$
Wind reactions based on MFRS pressures.



1-6-0
4-11-14 Over 3 Supports
R=340 U=9 W=4"

Design Crit: TPI-2002(STD)/FBC
 $C_q/R_T=1.00(1.25)/0(0)$

PLT TYP. Wave

7.24, 1230 1

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/ft.

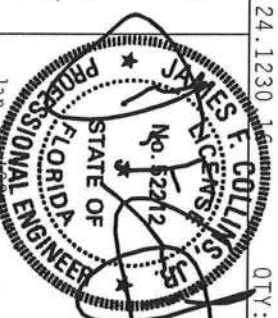


Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0278

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DECS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6500 KENTWOOD LANE, HADSDON, MI 48431) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED BY DECS OR WICA, THE TRUSS SHALL BE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DAMAGE TO THE TRUSS OR TO THE BUILDING. THE TRUSS IS TO BE INSTALLED IN CONFORMANCE WITH THE TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING, BRACING, AND BRACING. BY AREA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/H/55K) ASTM A653 GRADE 40/60 (W, K/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. A SEAL ON THIS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



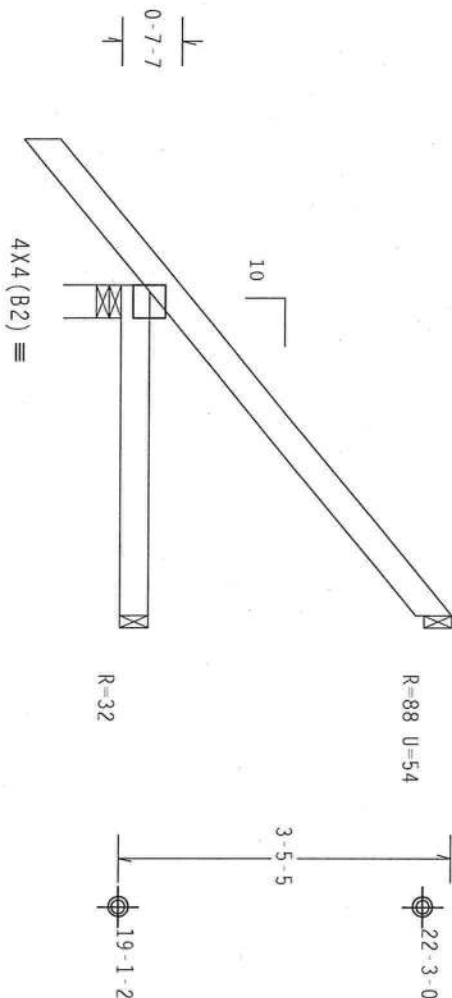
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TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031115
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEON- 43859
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

THE UNIVERSITY OF CHICAGO PRESS

110 mph wind, 20.50 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MWFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



$\overrightarrow{1-6-0}$
 $\overrightarrow{3-4-11}$ Over 3 Supports
 $R=279 \quad U=16 \quad W=4''$

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

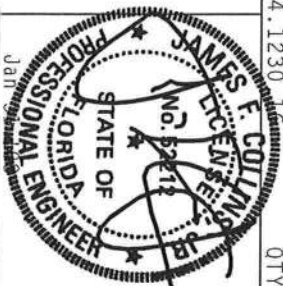
Scale = .5" / Ft.

***WARNING:** *RIGS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND BROCKING. REFER TO DC21 (CONSIDER COMPONENTS TO BE REMOVED). PUBLISHED BY THE STRESS PAPER INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA, 6000 TRUSS COMPANY, OF AMERICA, 6500 ENTERPRISE LANE, MIDDLETOWN, MI, 48157 FOR SAFETY PRACTICES. PRIOR TO PERFORMING THESE OPERATIONS, DIMENSIONS INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

[illegible]

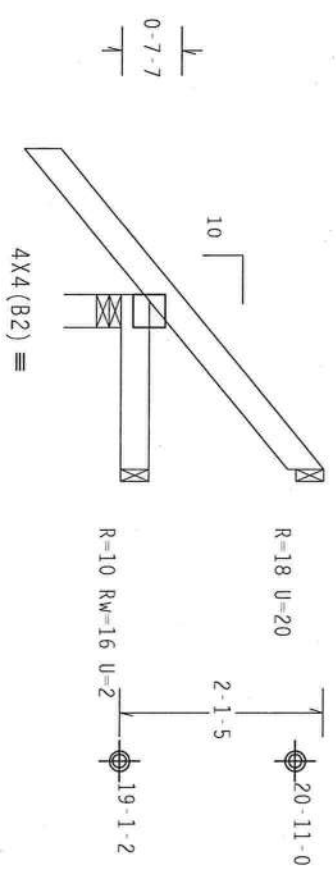
Building Components Group Inc.

Haines City, FL 33844
 FL Certificate of Authorization # 0778



TC LL	20.0 PSF	REF	R8228- 77004
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031116
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43861
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Roof overhang supports 2.00 psf soffit load.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



1-6-0
1-9-8 Over 3 Supports
R=233 U=29 W=4"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY:1 FL/-/4/-/E/-/-

Scale =.5"/ft.

WARNINGBUILDES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND UTCA (GOOD TRUSS CONCEPT OF AMERICA, 6200 ENTERPRISE LANE, MANASSAS, VA 20108) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

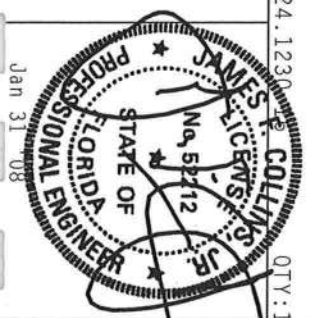
DESIGN THE TRUSS WITH APPLICABLE PROVISIONS OF BCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND UTCA (GOOD TRUSS CONCEPT OF AMERICA, 6200 ENTERPRISE LANE, MANASSAS, VA 20108) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PERFORMED AS OF TPI-2002 SEC. 1.1. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization #0-0778



TC LL	20.0 PSF	REF R8228- 77005
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031117
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEON- 43863
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

ИИИЗ УНИ ИНТЕРНАЦИОНАЛ ИНСТИТУТ (ЛУНУС & ДИПЛОМАТИЈА) СУБМИТЕД ДИ ИНУС ПРК.

110 mph wind, 22.01 ft mean hgt., ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

 $Cq/RT=1.00(1.25)/0(0)$

QTY:1 FL/-/4/-/E/-/-

Scale = .375"/Ft.

NO. 52212



The seal of the State of Florida Professional Engineer is visible in the bottom right corner. It is a circular emblem with the text "STATE OF FLORIDA" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by stars. The center of the seal contains the word "ENGINEER" and the number "10000".

Jan 31 08

TC LL	20.0 PSF	REF	R8228- 77006
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031118
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43865
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	ITEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

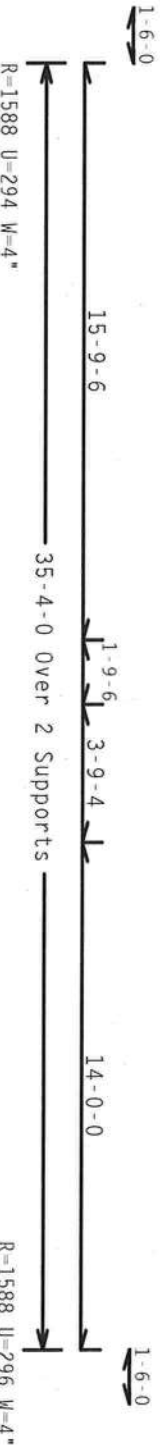
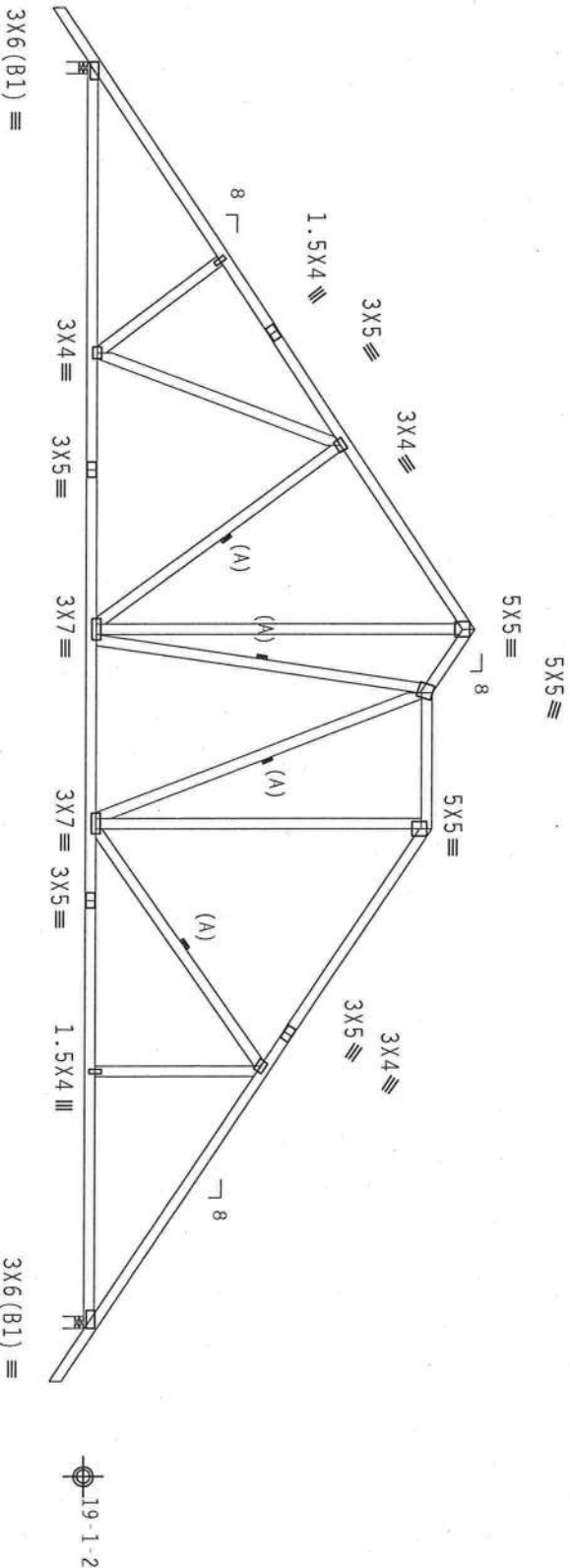
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 24.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, Wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MMFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/0(0)

7.24.1230.16

QTY:1

FL/-/4/-/E/-/-

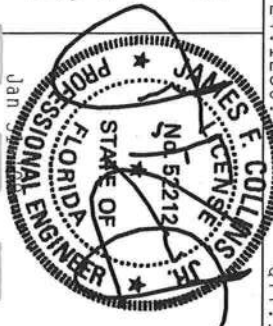
Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

Haines City, FL 33844
Fl Certificate of Authorization #0-778



TC LL	20.0 PSF	REF R8228- 77007
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031119
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEQN- 43876
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

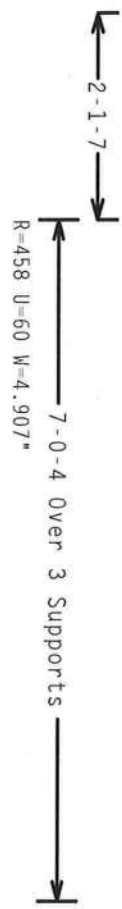
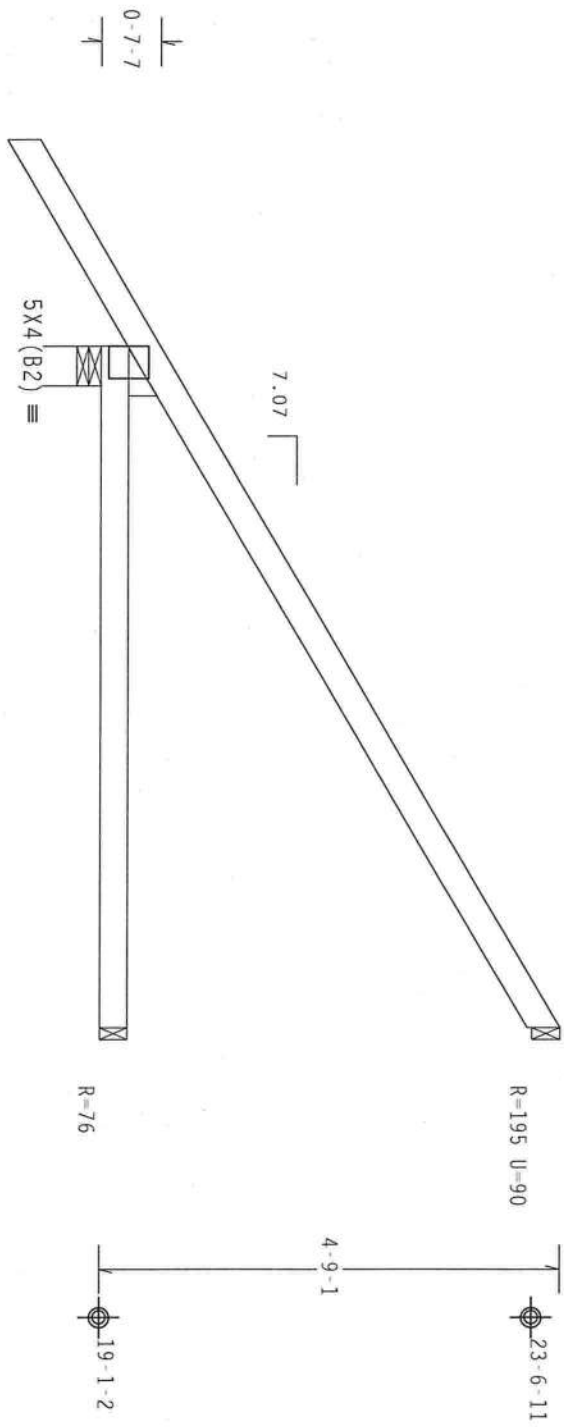
(dear--OWNER BUILDER deardorf -- , ** - T59)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
: Lt Wedge 2x4 SP #3:

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 21.16 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $GCP(+/)=0.18$
Wind reactions based on MMFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY: 1

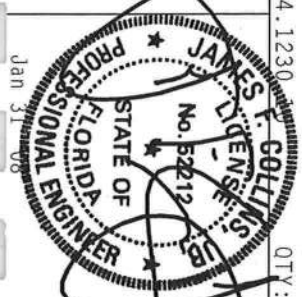
FL/-/4/-/E/-/-

Scale = .5"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6200 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Haines City, FL 33844
TPI Certificate of Authorization #0778



FL	-	4	-	E	-	-	REF	R8228-77008
TC	LL						DATE	01/31/08
TC	DL						DRW	HCUSR8228 08031120
BC	DL						HC-ENG	EC/WHK
BC	LL						SEON-	43893
TOT.	LD.						FROM	JP
DUR.	FAC.						JREF-	1TEL8228203
SPACING								

115)

Top Chord: 1 Row @ 3.00" o.c.
Bot Chord: 1 Row @ 4.00" o.c.

: 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf 1w=1.00 GCPI(+/-)=0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

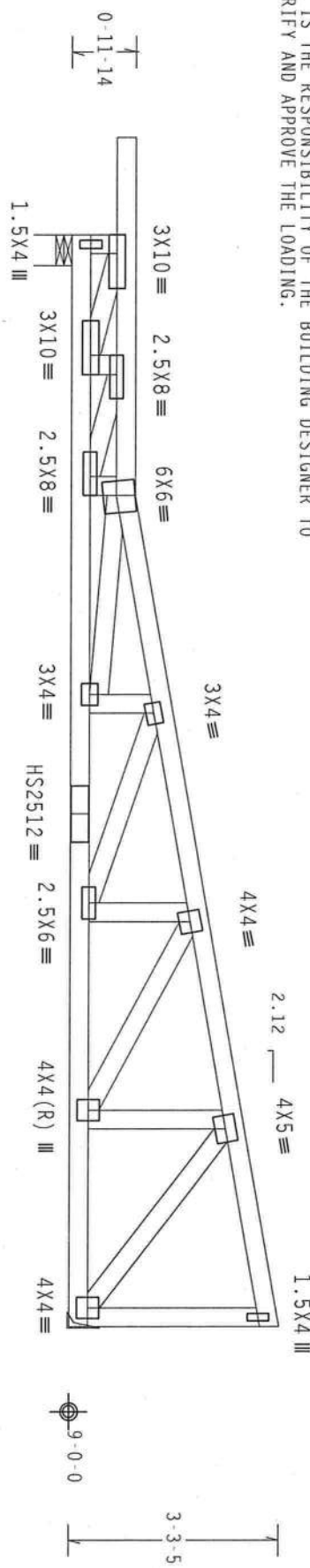
Root overhang supports 2.00 pst soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
 TC - From 60 PLE at -1.51 to 60 PLE at 4.03

TC	From	60 PLF	at 4.03	to	60 PLF	at 16.97
BC	From	4 PLF	at -1.51	to	4 PLF	at 0.00
BC	From	20 PLF	at 0.00	to	20 PLF	at 16.97
TC	843 LB	Conc.	load at	0.06		
TC	68 LB	Conc.	load at	2.06		
BC	628 LB	Conc.	load at	2.06	4.06	6.06
BC	12 LB	Conc.	load at	5.08		
BC	584 LB	Conc.	load at	8.06		
BC	578 LB	Conc.	load at	10.06		
BC	2026 LB	Conc.	load at	10.24		
BC	415 LB	Conc.	load at	12.06		
BC	851 LB	Conc.	load at	13.07		
BC	768 LB	Conc.	load at	14.06		
BC	770 LB	Conc.	load at	15.89		

LOADING HAS BEEN CALCULATED BY THE KROSS MANUFACTURER. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO VERIFY AND APPROVE THE LOADING.



4-0-6 12-11-5
16-11-10 Over 2 Supports
R=4954 U=444 W=5.657"
R-5305 U=435

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

QTY:1

QTY:1 FL/-/4/-/E/-/-

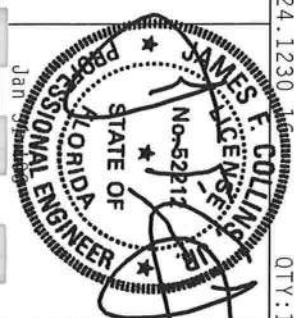
Scale = .375" / Ft.

WARNING: THIS IS A REQUIRE EIGHT CASE IN INFORMATION, HANDLING, SHIPPING, INSTALLING, AND BROCKING. REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY THE (FIRESS PAST INSTITUTE, 218 HOH LE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICK (GOOD THERMS CONTROL OF AMERICA, 6300 ENTERPRISE LANE, WATSONVILLE, CA 95071) FOR SAFETY PRACTICES. PERFORM TO PREVENT THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CELLS.



Building Components Group Inc.

Haines City, FL 33844
 FI Certificate of Authorization # 00778



TC LL	20.0 PSF	REF- R8228- 77009
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCU8R8228 08031121
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEQN- 44038
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
1lt Wedge	2x4	SP	#3	

Roof overhang supports 2.00 psf soffit load. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.83 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCpf (+/-)=0.18

Wind reactions based on MWFRS pressures.

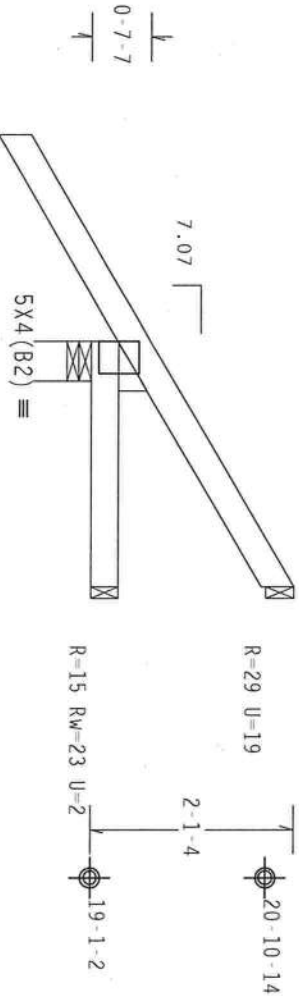


Diagram of a beam with a uniformly distributed load. The total length is 21'-7". The load is 2.6 k/ft over the 3 supports. The reaction at the left support is R=309 lb, U=58 lb/ft, and W=4.907 ft.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/Ft.



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 FI Certificate of Authorization # 0778

****WARNING*****BUILDING EXTREME CORE IN FABRICATION. CONSULT BY TPI THIRDS PARTY INSTITUTE, 216
REFER TO DCS1 (ELECTRIC COMPONENT SAFETY INFORMATION) CONSULTED BY TPI THIRDS PARTY INSTITUTE, 216
NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND UICA (GOOD TRUSS COUNCIL OF AMERICA), 6500
ENTERPRISE LANE, MIDDLETON, WI 53189 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT*****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITG BCG, INC., SHALL NOT
BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AND FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH
TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIRD) AND TPI.
TRUSS TYPE: COMBINATION OF TOP/TOE PLATE AND BOTTOM/TOE PLATE (AS SHOWN).
PLAYERS TO EACH FACE OF TRUSS AND JOINTS: MINIMUM OF TWO (2) PLAYERS PER JOINT.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMERICAN S&P OF 10-1-2002 SEC.3.
LOADING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SEE THE SEAL ON THIS
DESIGN SHEET. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE
BUILDING DESIGNER PER ANSI/TPI 1. SEC. 2.



TC LL	20.0 PSF	REF	R8228- 77010
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031122
BC LL	0.0 PSF	HC-ENG	EC/MHK
TOT.LD.	40.0 PSF	SEQN-	43891
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

RECEIVED BY THE DIRECTOR, FBI, 11/11/68

Scale = .375 /Ft.	
FL/-/4/-/E/-/-	REF R8228- 77011
TC LL 20.0 PSF	DATE 01/31/08
TC DL 10.0 PSF	DRW HCU8R8228 08031123
BC DL 10.0 PSF	HC-ENG EC/MHK
BC LL 0.0 PSF	SEON- 43912
TOT.LD. 40.0 PSF	FROM JP
DUR.FAC. 1.25	JREF- 1TEL8228Z03
SPACING 24.0"	

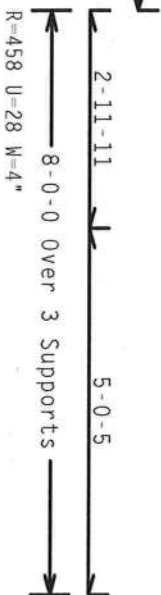
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCSUR8228 08031124
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43880
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	ITEL8228203

```
( (dear--OWNER BUILDER deardorf -- , ** - T63 )
```

110 mph wind, 21.81 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf lw=1.00 gcpi (+/-)-0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230.16

QTY:1

FL^{-/-}/4⁻/E⁻/-

Scale = .375"/Ft.

WARNING - **SHIES** (BIDDING) REQUIRE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING, AND BRACING REFER TO NC51 (THOUGHTFUL COMPONENT SPECIFICATION) - PUBLISHED BY TPI (TENSION PLATE INSTITUTE), 210 BOHLE LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND WICA (WOOD JOINTS CONFERENCE OF AMERICA), 65000 ENTERPRISE LANE, MIDLOTHIAN, MI (53179) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED GRID CELLING.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BULD THE TROSS IN CONFORMANCE WITH T01: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSSES.

EXISTING CONDITIONS, THE APPLICABLE PROVISIONS OF MOST BUILDING CODES, THE APPLICABLE PROVISIONS OF THE CONDUCTOR PLATES ARE MADE OF 20/24/1664 (H/JL/SS) ASTM A563 DESIGN SPEC. (BY AREA) AND TPL. THE CONDUCTOR PLATES TO EACH FACE OF TROSS BAR, DIRECT OR INDIRECTLY LOCATED ON TPL-2002 SEC. 3. A SEAL ON THIS AS AN INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF TPL-2002 SEC. 3. ORANGE, INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TROSS COMPONENT TROSS SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANNEX 1 SEC. 2.

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R8228- 77013
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCSR8228 0803125
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43901
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Lt Wedge 2x4 SP #3:

Roof overhang supports 2.00 psf soffit load.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 20.89 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCp1(+/-)=0.18

Wind reactions based on MMFRS pressures.

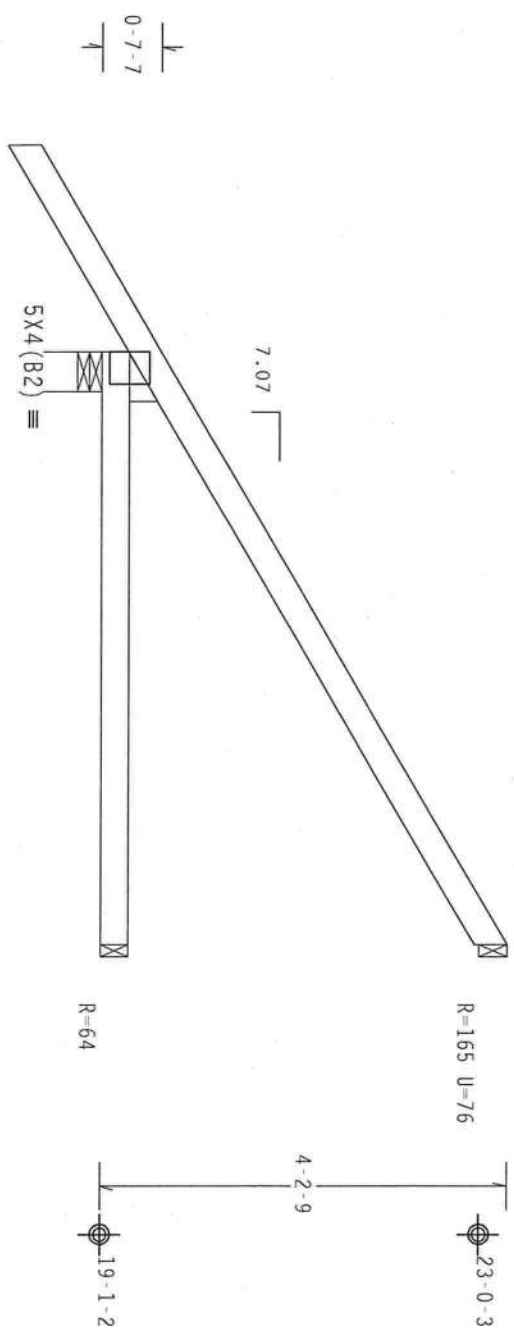


Diagram of a beam with dimensions and material properties:

- Span: 6-1-3 Over 3 Supports
- Height: 2-1-7
- Material Properties: R=423 U=58 W=4.907"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/Ft.



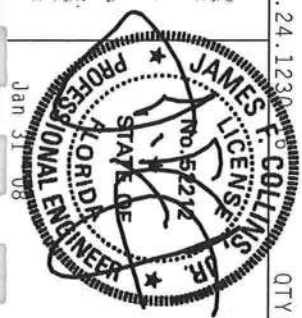
Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0778

****WARNING**** *HOLDS REQUIRE EXPLICIT CARE IN FABRICATION, HANDLING, SHIPMENT, INSTALLING AND BRACING. REFER TO NC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLAN INSTITUTE, 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** *TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, AND FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING, A BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE REQUIREMENTS OF NDS (NATIONAL DESIGN SPEC., BY AIA/P) AND TPI. THE REG. CONNECTION PLATES ARE MADE OF 20/20/16GA (H, H/SS/SS) ASTM A653 GRADE 40/60 (H, K/H/55) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIA/KS OR TPI-2002 SEC.2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

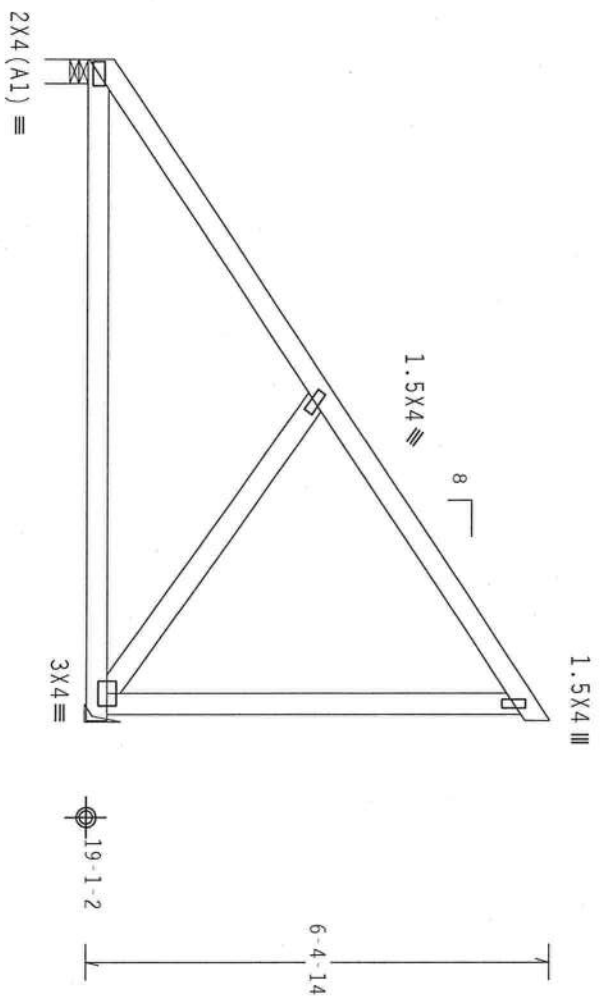


TC LL	20.0 PSF	REF	R8228 - 77014
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031126
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN -	43895
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF -	1TEL8228203

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 22.48 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. lw=1.00 Gcpl(+/-)=0.18
Wind reactions based on MMFRS pressures.
Right end vertical not exposed to wind pressure.



9'-0-11 Over 2 Supports
R-383 U=3 W=4"
R-378 U=136

PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)
7.24.1230

OTY:1 FL/-/4/-/E/-/-
Scale = .375"/Ft.

****WARNING**** MUSSERS REQUIRE EXTERIOR CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF BCS (NATIONAL DESIGN SPEC. BY AIA/PA) AND TPI. DESIGNER'S RESPONSIBILITY FOR THE TRUSS SHALL BE LIMITED TO THE DESIGN OF THE TRUSS AND THE TRUSS PLATES TO EACH CHORD. THE TRUSS SHALL BE DESIGNED TO RESIST ALL LOADS AND STRESSORS AS SHOWN ON THE DRAWING. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEK AS OF TPI-2002, SEC.2.1.5.1. ANY INSPECTION OF THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOCIETY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



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TC LL	20.0 PSF	REF R8228- 77015
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031127
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEON- 43897
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

SPECIAL LOADS

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

(B)(1) 2X4X7-11-2 SP #2 Dense scab at right end. Attach scab to face of chord with: 12d Common (0.148"x.25" min.) nails @ 8" OC. Plus additional nail clusters at: BRG.: (5), heel: (5), 1st panel point: (0).

[illegible]

R=1026 U=484 W=6.403°

R=1524 U=514

PLT TYP. Wave

3 → R=1524 U

← 12-9-11 Over 2 Supports →

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.24.1230.16mm

QTY:1 FL/-/4/-/E/-/-

Scale = .3125" / Ft.



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[illegible]

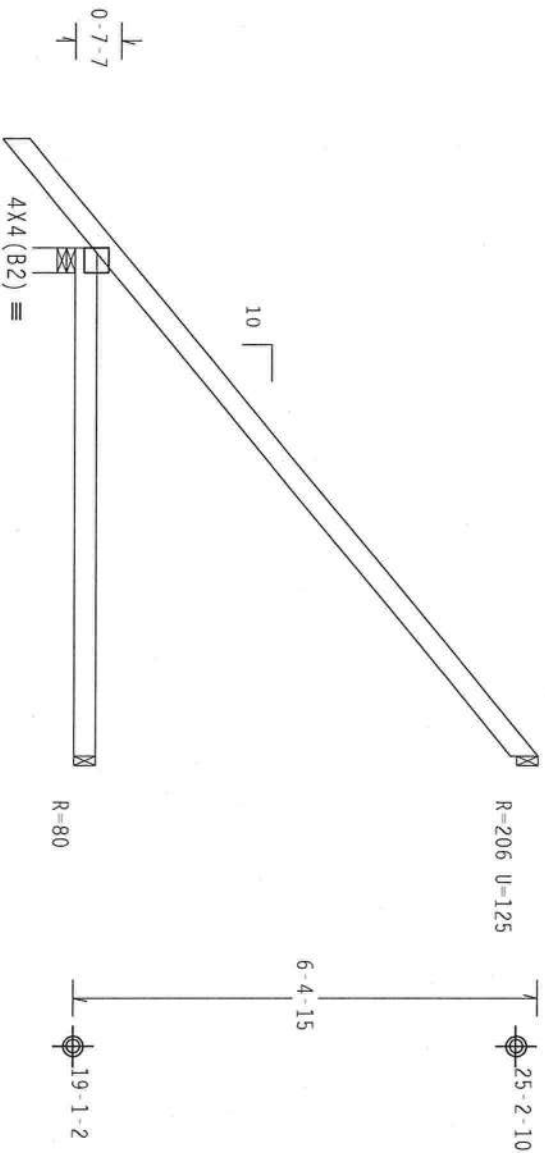
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BC DL	10.0 PSF	DRW	HCUSR8228 08031128
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43910
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 21.99 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $G_{CPI}(+/-)=-0.18$

Wind reactions based on MWFRS pressures.



WFO-6-01

6-11-7 Over 3 Supports $R=419$ $W=4''$

Design Crit: TPI-2002(STD)/FBC

$$\text{Cd}/\text{RT}=1.00(1.25)/0(0)$$

7.24.1230

QTY: 1

FL1 - /41 - /F1 - / -

Scale = 375"/Ft



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0778

[illegible]

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. ITM BCG

CONNECTOR PLATES ARE MADE OF 20/10/16GA (W, W/SS/K) ASTM A653 GRADE 40/60 (W, W/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND BRACES OTHERWISE LOCATED ON THIS SECTION. POSITION PER DRAWING 1004-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER (SEE ANSICD 1.000.2).

OVERVIEW OF SIGNIFICANT ACHIEVEMENTS



TC LL	20.0 PSF	REF	R8228- 77017
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031129
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43863
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

(dear-- OWNER BUILDER deardorf -- , ** - T7)

Left end vertical exposed to wind pressure. Deflection meets $L/240$ criteria for brittle and flexible wall coverings.

Roof overhang supports 2.00 psf soffit load.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

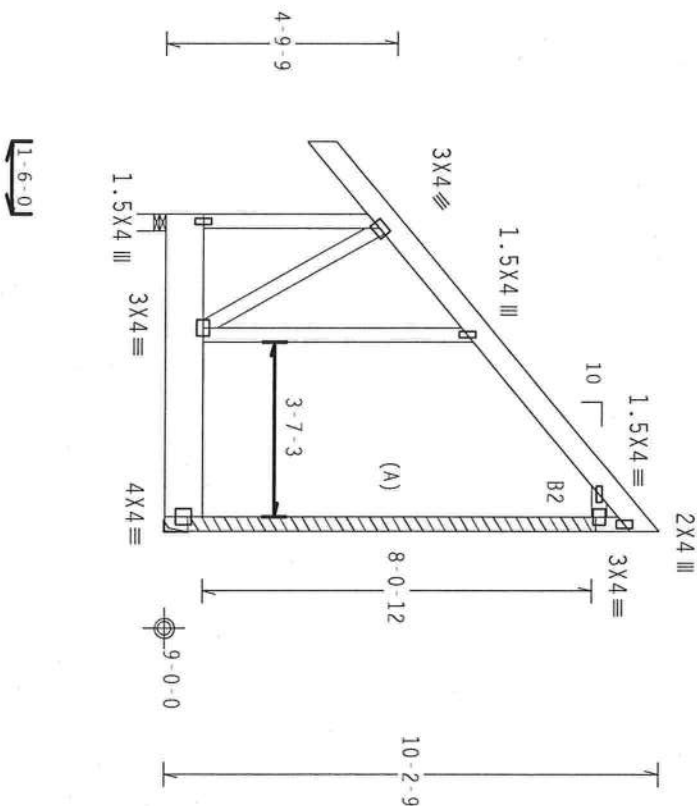
110 mph wind, 15.88 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf 1w=1.00 Gcpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 2-7-5 to 6-2-8.

(A) SCAB BRACE. 80% LENGTH OF WEB MEMBER. SAME SIZE SPECIES & GRADE OR BETTER. ATTACH WITH 10d NAILS (0.128"x3.0",min.) @ 6" OC.



6-2-8 0.318
6-6-0 Over 2 Supports
R=618 W=4" R=759 U=191

PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)$

 $Cq/RT=1.00(1.25)/0(0)$

7.36.0424

OTY:1

FL/141-1E1-1-

Scale = .25"/Ft.

* **WARNING**—PROCESSES OF REMEDIATION CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DEMOLITION (BUILDING COMPONENTS OF SAFETY INFORMATION).
 REFER TO GC51 (BUILDING COMPONENTS OF SAFETY INFORMATION).
 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WORLD TRUSS COUNCIL OF AMERICA, 65000
 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
 OTHERWISE INDICATED, FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
 PROPERLY ATTACHED RIGID CELLING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. TIV B&G, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TP1, OR FABRICATING, HANDLING, SHIPPING, INSTALLING, OR BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AISC) AND TP1.

Building Components Group Inc.



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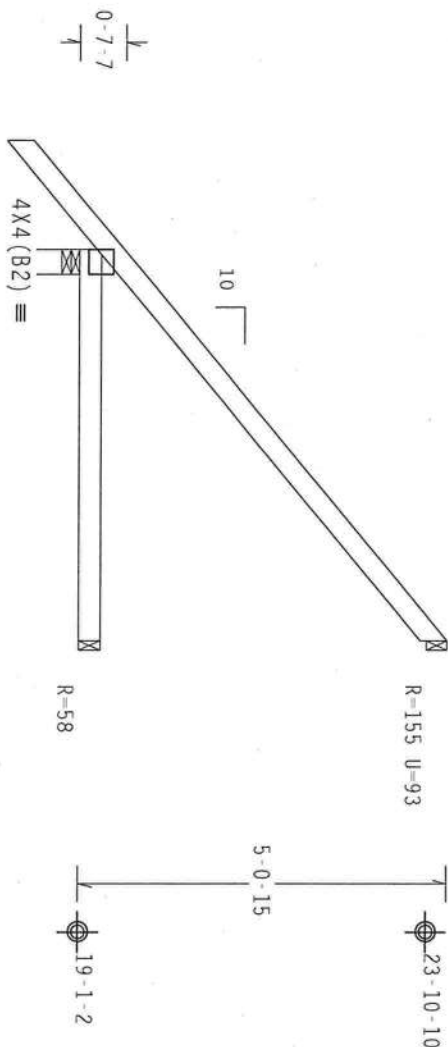
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031130
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	73548
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

THE UNIVERSITY OF CHICAGO PRESS

110 mph wind, 21.32 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ $G_{CPI} (+/-) = 0.18$

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



1-6-0

← 5-4-4 Over 3 Supports →
R=354 U=7 W=4"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230.16

QTY:1

FL/-/4/-/E/-/-

Scale = .375"/Ft.



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Haines City, FL 33844

FI Certificate of Authorization # 00778

[illegible]

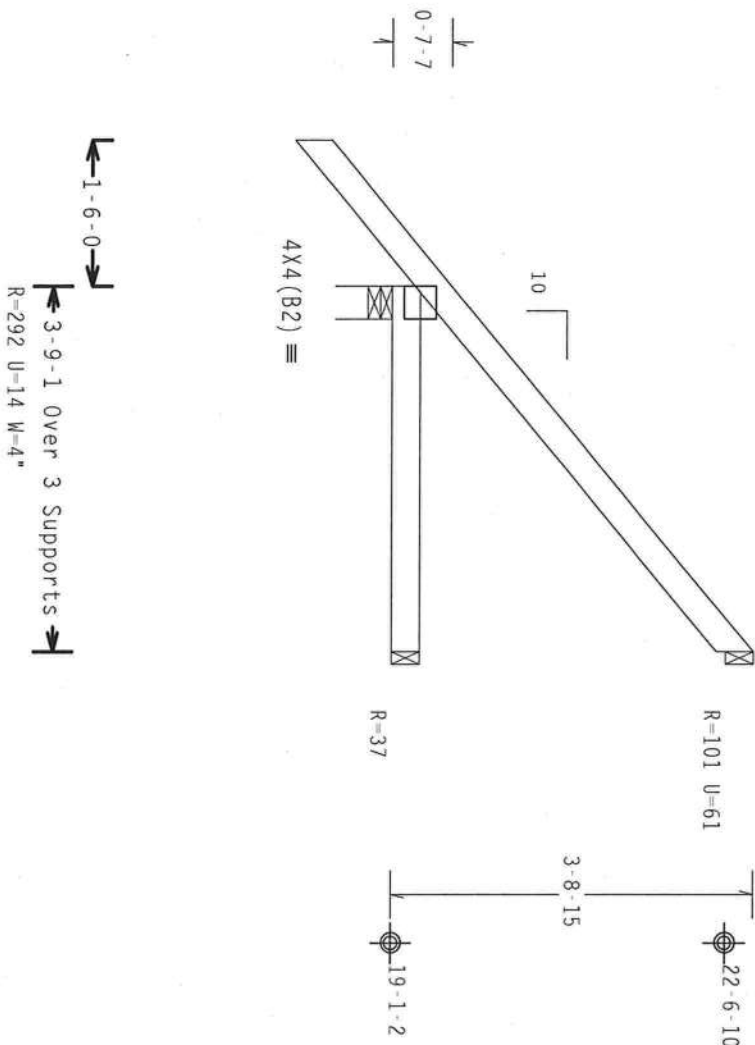
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031131
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT. LD.	40.0 PSF	SEQN -	43885
DUR. FAC.	1.25	FROM JP	
SPACING	24.0"	JREF -	1TEL8228203

ИЗДАНИЕ НА ПЕРВОМ ПЕЧАТѢ (1924) ВЪЗМЪНОВАННОЕ

110 mph wind, 20.65 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ Gcpl(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

~~QTY: 1~~

FL/14/1E/1-1-

Scale = .5" / Ft.

WARNING: *WORKS BEHIND EXISTING CEILING CAUSE INJUNCTION, HANDLING, SHIPPING, INSTALLING AND PROJECTING TO RECIPIENT (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY THE CRUSS PAPER INSTITUTION, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314, AND CRUSS TRUSS COMPANY, OF AMERICA, 61000 ENTERPRISE LANE, SUITE 1500, W. 53719 FOR SAFETY PRACTICES AND PRICE TO PERFORMING THESE FUNCTIONS. ADDRESSES OTHERWISE INDICATED FOR THIS CORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-1 OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONDITIONS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AISC) AND IP. CONNECTOR PLATES ARE MADE OF 20/10/16GA (W/1/55(K) ASTM A653 GRADE 50/60 (14, K/11/55) GALV. STEEL. PLATES TO EACH END OF BEAMS AND JOISTS ATTACHED TO END OF BEAM OR JOIST ON THE BEAM POSITION. SEE DRAWING 3004-2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEAS OF TPII-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS TOOK

DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

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FL Certificate of Authorization # 00778

TC LL	20.0 PSF	REF	R8228- 77020
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031132
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEON-	43887
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	ITEL8228203

(dear--OWNER BUILDER de			
Top chord	2x4	SP #2	Dense
Bot chord	2x4	SP #2	Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.99 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, Cat II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-)=0.18

Wind reactions based on MWFRS pressures.

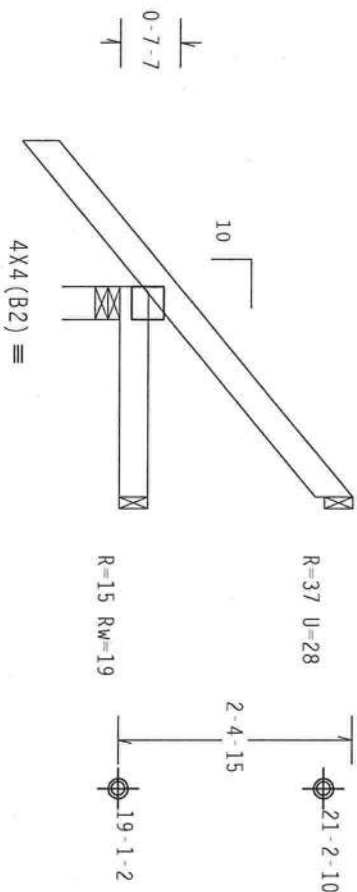


Diagram of a continuous beam with three supports. The beam is divided into two equal spans of 21'-13" each. The total length is 42'-6". The beam is labeled "2-1-13 Over 3 Supports". The reaction at the left support is R=240 and the reaction at the right support is U=25 W=4".

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

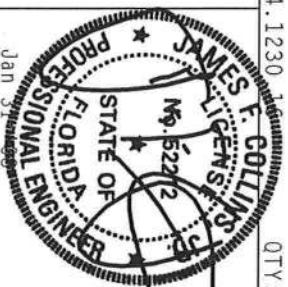
Scale = .5" / Ft.

WARNING: *HOLE'S RIGID FIBRE CEMENT BOARD, MANUFACTURED BY THE TRUSS PAVING INSTITUTE, 210
REFLECTED TO RESIST (BULIDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PAVING INSTITUTE, 210
MORRIS LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICK, OHIO TRUSS COUNCIL OF AMERICA, 6700
ENTERPRISE LANE, MANSION, OH 44130 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, INTERSECT
OTHERWISE INDICATED TOP GOOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM GOOD SHALL HAVE
PROPERLY ATTACHED FIELD CEILING.



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FL Certificate of Authorization # 0278



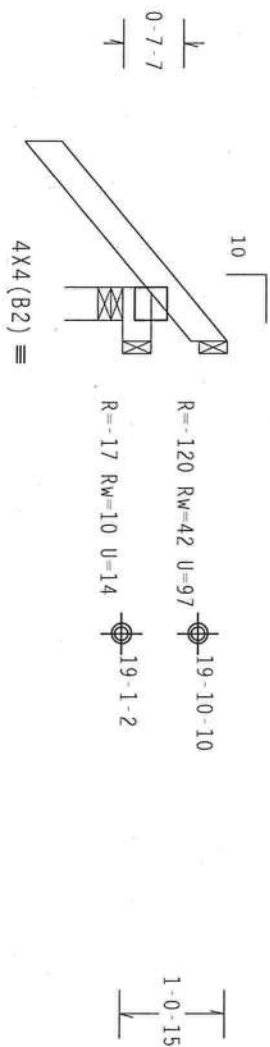
TC LL	20.0 PSF	REF	R8228 - 77021
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031133
BC LL	0.0 PSF	HC-ENG	EC/WHK *
TOT.LD.	40.0 PSF	SEQN -	43889
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF -	ITEL8228203

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.32 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCp1(+/-)=0.18

Wind reactions based on MWFRS pressures.



0-6-0

0-6-10 066-30 Supports

R=292 U=71 W=4

PLT TYP. Wave

Design Crft: TPI-2002(STD) / FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5" / Ft.



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 FL Certificate of Authorization # 0778

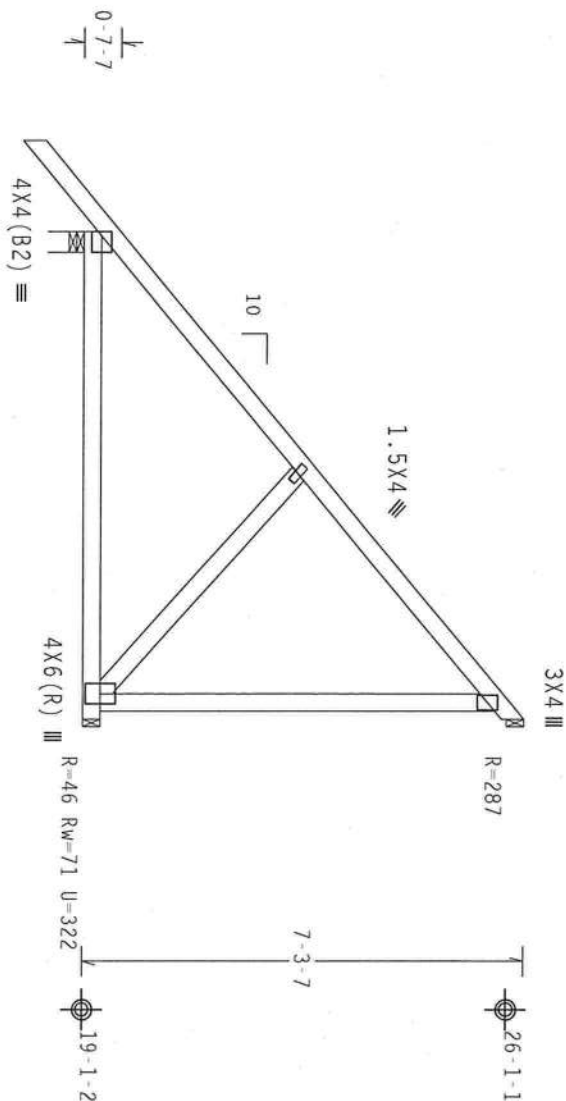
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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031134
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	43899
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	Webbs	2x4	SP	#3		

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind; 22.42 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, 1w=1.00 gcpi(+/-)-0.18

Wind reactions based on MWFRS pressures.



✓0-6-1✓

8-0-0 Over 3 Supports $R=463$ $W=4"$

PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .3125"/Ft.

WARNING: *HOLDS (BULIDING COMPONENTS EXCEPT CASE IN INFORMATION), HANDLING, SHIPPING, INSTALLING AND PROTECTING REFER TO 6251 (BULIDING COMPONENTS EXCEPT CASE IN INFORMATION). PUBLISHED BY THE TRUSS PLATING INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND 6000 TRUSS COMPANY OF AMERICA, 6500000, INTERSTATE LANE, MOBILE, AL 36689 FOR SAFETY PRACTICES AND PRICES TO PERFORM THESE OPERATIONS. INTERESTED INDIVIDUALS FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED CHORD CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TROSS IN COMPLIANCE WITH THE; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS TO APPLICABLE PROVISIONS OF MD5 (NATIONAL DESIGN SPEC., BY AIAA) AND TPI.
CONNECTOR PLATES ARE MADE OF 20/18/1/66A (H, M/SS/K) ASTM A653 GRADE 40/60 (H, K/H, SS) GALV. STEEL.
PLATES TO EACH FACE OF BIRDS AND THREE OUTRIGGER LOCATED ON THIS DESIGN POSITION ARE NOT REQUIRED. APPLY

PLATES TO EXCEED 10003 AND, THREE SURVEILLANCE LOCATIONS ON THIS DESIGN, POSITION PER DRAWINGS 10003 AND 10004. INSPECTION OF PLATES FOLLOWS BY (1) SHALL BE PER AMERICAN SOCIETY OF MECHANICAL ENGINEERS, SECTION 5, ARTICLE 3.3, A SEAL ON THIS DRAWING, INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE THROAT CONDUIT.

DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/FPI 1 SEC. 2.

TC LL	20.0 PSF	REF	R8228- 77023
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031135
BC LL	0.0 PSF	HC-ENG EC/MHK	*
TOT.LD.	40.0 PSF	SEON-	43903
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

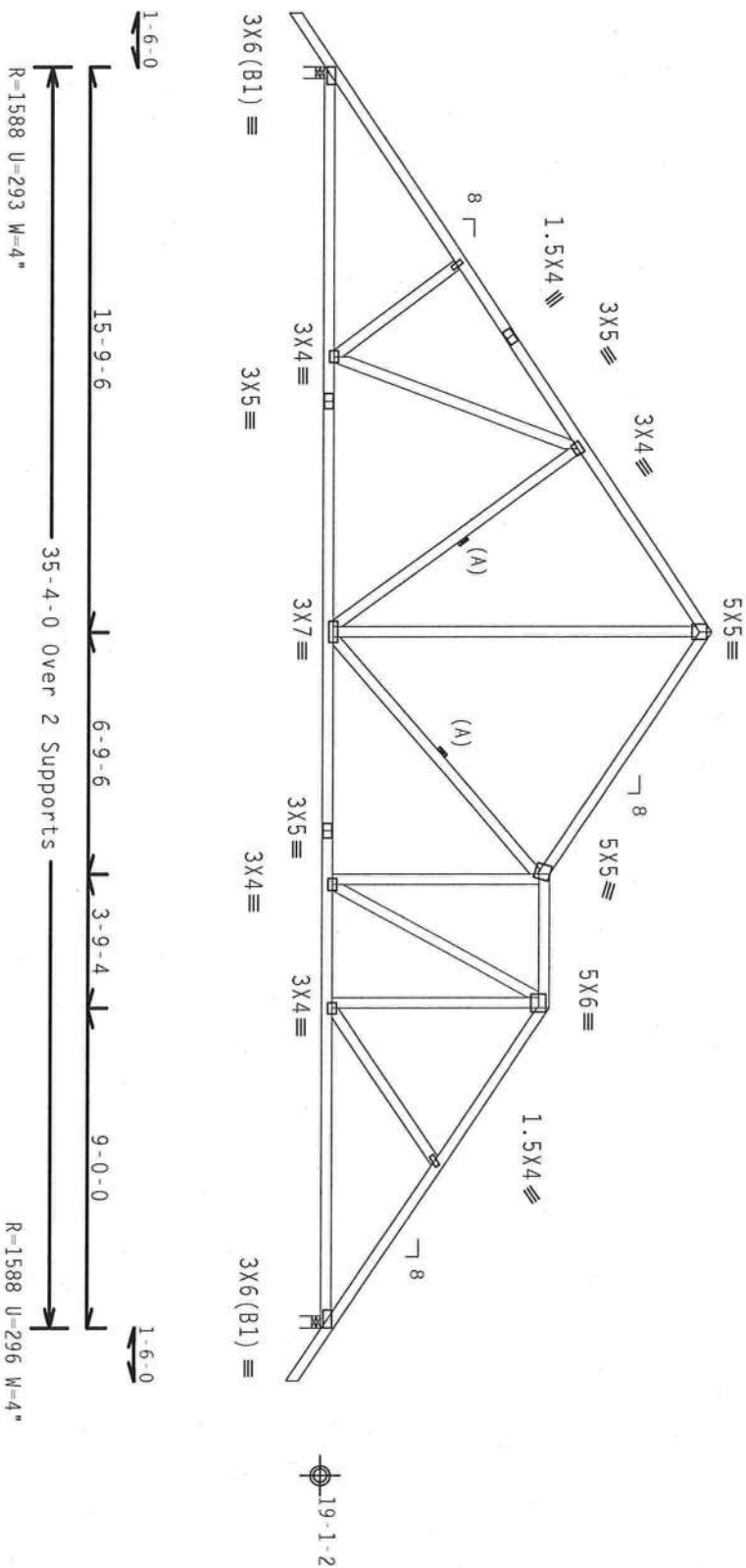
	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
Wbs	2x4	SP	#3			

(A) Continuous lateral bracing equally spaced on member.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 24.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ gcpi (+/-) -0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .1875"/Ft.



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0 278

*****WARNING***** MUSTS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND DRACING REFER TO RCSI FABRICATING COMPONENTS SPECIFICATION, CONSULTED BY TPI THRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND RCSI 6000 THRUSS COMPONENTS OF AMERICA, 6300 ENTERPRISE LANE, MOBILE, AL 36619 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE RCSI, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, OR FAILURE TO BUILD THE THRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING A DRACING OF THRUSSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/A) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/1664 G4-H/SS/35 ASHT A653 GRADE 40/60 (4, K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF THRUSS AND (1) SHALL BE PER AIBX 43 OF TPI-1-2002 SEC.3. A SEAL ON THIS IS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE THRUSS COMPONENTRY OF THE BUILDING SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIBX/TPI 1 SEC. 2.



TC LL	20.0 PSF	REF	R8228- 77024
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 0803136
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	43872
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

4 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25",_min.)_nails)

Top Chord: 1 Row @ 4.00" o.c.
Bot Chord: 1 Row @ 3.75" o.c.
Webs : 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 Gcpi(+/)-0.18

Wind reactions based on MMFRS pressures.

Right end vertical not exposed to wind pressure.

Roof overhang supports 2.00 psf soffit load.

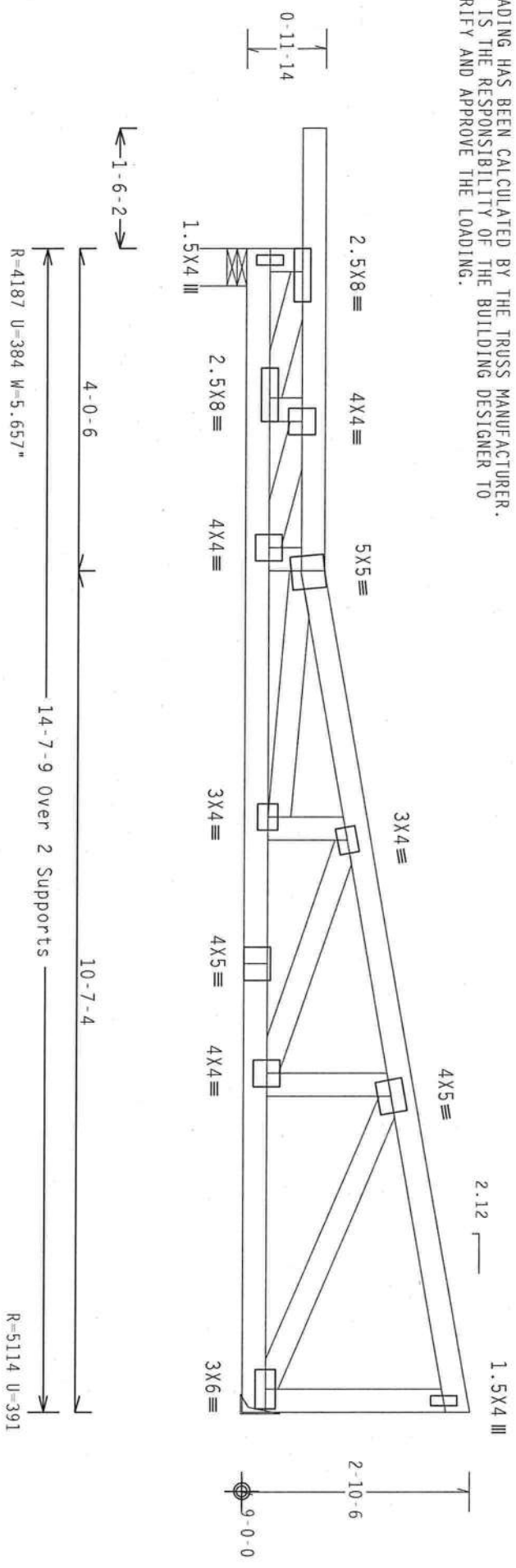
In lieu of structural panels use purlins to brace all flat TC @ 24" o.c.

SPECIAL LOADS

TC - From	DUR.FAC=-1.25 / PLATE DUR.FAC=-1.25)	60 PLF at -1.51 to 60 PLF at 4.03
TC - From	60 PLF at -1.51 to 60 PLF at 4.03	60 PLF at 4.03 to 60 PLF at 14.63
BC - From	4 PLF at -1.51 to 4 PLF at 0.00	4 PLF at 0.00 to 20 PLF at 14.63
BC - From	20 PLF at 0.00 to 20 PLF at 14.63	
TC - 843 LB Conc.	Load at 0.06	
TC - 68 LB Conc.	Load at 5.08	
BC - 628 LB Conc.	Load at 2.06, 4.06, 6.06	
BC - 12 LB Conc.	Load at 5.08	
BC - 584 LB Conc.	Load at 8.06	
BC - 578 LB Conc.	Load at 10.06	
BC - 2026 LB Conc.	Load at 10.24	
BC - 415 LB Conc.	Load at 12.06	
BC - 851 LB Conc.	Load at 13.07	
BC - 768 LB Conc.	Load at 14.06	

Deflection meets L/240 live and L/180 total load. Creep increase Factor for dead load is 1.50.

LOADING HAS BEEN CALCULATED BY THE TRUSS MANUFACTURER. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO VERIFY AND APPROVE THE LOADING.



PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

7.24.1230 QTY:1 FL/-/4/-/E/-/ Scale =.5"/Ft.

WARNINGBUILDS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NC31 (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Haines City, FL 33844
TPI Certificate of Authorization #0-0778



TC LL	20.0 PSF	REF R8228- 77025
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCURS8228 08031137
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEON- 44044
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

THIS WORK PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY KUSS M.K.

Roof overhang supports 2.00 psf soffit load.

110 mph wind, 23.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ $Gc p_i(+/-)=0.18$

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.36.0424

QTY:1

FL1-141-1E1-1-

Scale = .1875"/Ft.



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0778

****WARNING*****BULDOZING EXISTING CORE IN FABRICATION. HANDING, SHIPPING, INSTALLING AND DRACING. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES.



Jan 31 08

TC LL	20.0 PSF	REF	R8228 - 7/027
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031075
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN -	73629
DUR.FAC.	1.25	FROM JP	
SPACING	24.0"	JREF -	ITEL8228Z03

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 29.71 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, Wind TC DL=5.0 psf, wind BC DL=2.0 psf, $I_w=1.00$ $G_{CPI}(+/-)=0.18$

Wind reactions based on MFRS pressures.

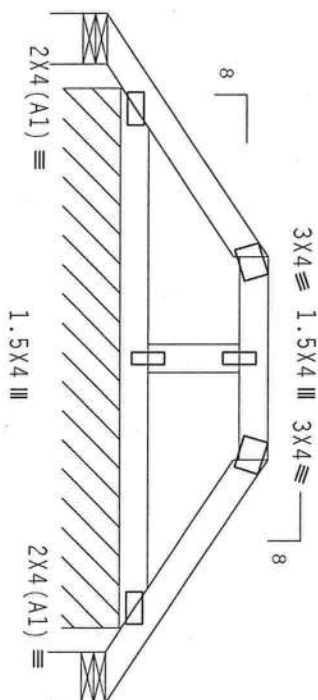
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

REFER TO DRAWING PIGBACKB0204 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C.

SPECIAL LOADS

----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at 0.00 to 64 PLF at 2.50
TC - From 64 PLF at 2.50 to 64 PLF at 4.58
TC - From 64 PLF at 4.58 to 64 PLF at 7.08
BC - From 4 PLF at 0.00 to 4 PLF at 7.08

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.



Ø-28-10-9

Ø-29-0-5

2-9-3
1-8-11 2-1-0 1-8-11
7-1-0 Over 3 Supports
R=12 Rw=28 U=28 W=6.31"
R=76 PLF U=36 PLF W=5-6-6
R=12 U=7 W=6.31"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.36.0424

QTY:1

FL/-/4/-/E/-/-

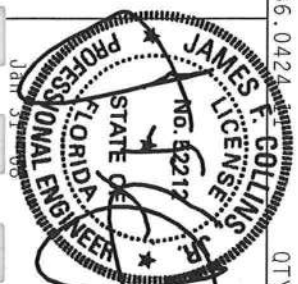
Scale =.5"/Ft.

WARNINGOWNERS REQUIRE EXTERIOR GABLE TRUSS FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WICK GOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization #0 278



TC LL	20.0 PSF	REF R8228- 77028
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUR8228 08031079
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEQN- 73631
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
Wets		2x4	SP	#3		

110 mph wind, 30.06 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. $I_w=1.00$ GCD1(+/-)=0.18

Wind reactions based on MMFRS pressures.

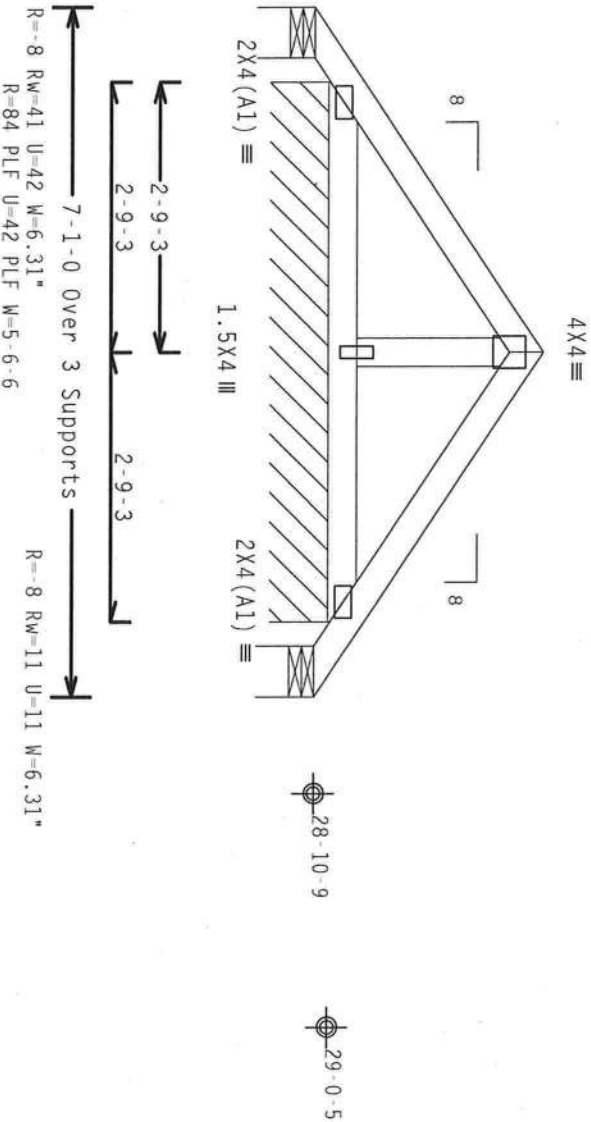
REFER TO DRAWING PIGBACK0204 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE
BRACED @ 24" O.C.

SPECIAL LOADS

----- (LUMBER	DUR.FAC.=1.25	/	PLATE	DUR.FAC.=1.25)
IC - From	64 PLF at	0.00	to	64 PLF at 3.54
TC - From	64 PLF at	3.54	to	64 PLF at 7.08
BC - From	4 PLF at	0.00	to	4 PLF at 7.08

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
$$Cq/RT=1.00(1.25)/0(0)$$

7.36.0424.11

QTY:1

FL1-141-1E1-1-

Scale = .5" / Ft.



Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0278

[illegible]

FL/-/4/-/E/-/-		Scale=.5"/Ft.
TC LL	20.0 PSF	REF R8228- 77029
TC DL	10.0 PSF	DATE 01/31/08
BC DL	10.0 PSF	DRW HCUSR8228 08031080
BC LL	0.0 PSF	HC-ENG EC/WHK
TOT.LD.	40.0 PSF	SEQN- 73633
DUR.FAC.	1.25	FROM JP
SPACING	24.0"	JREF- 1TEL8228203

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	webs	2x4	SP	#3		

(A) Continuous lateral bracing equally spaced on member.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

110 mph wind, 23.67 ft mean hgt, ASCE 7-02, closed bldg, not located within 4.50 ft from roof edge, CAT 1, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_w=1.00$ Gcpi (+/-)-0.18

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

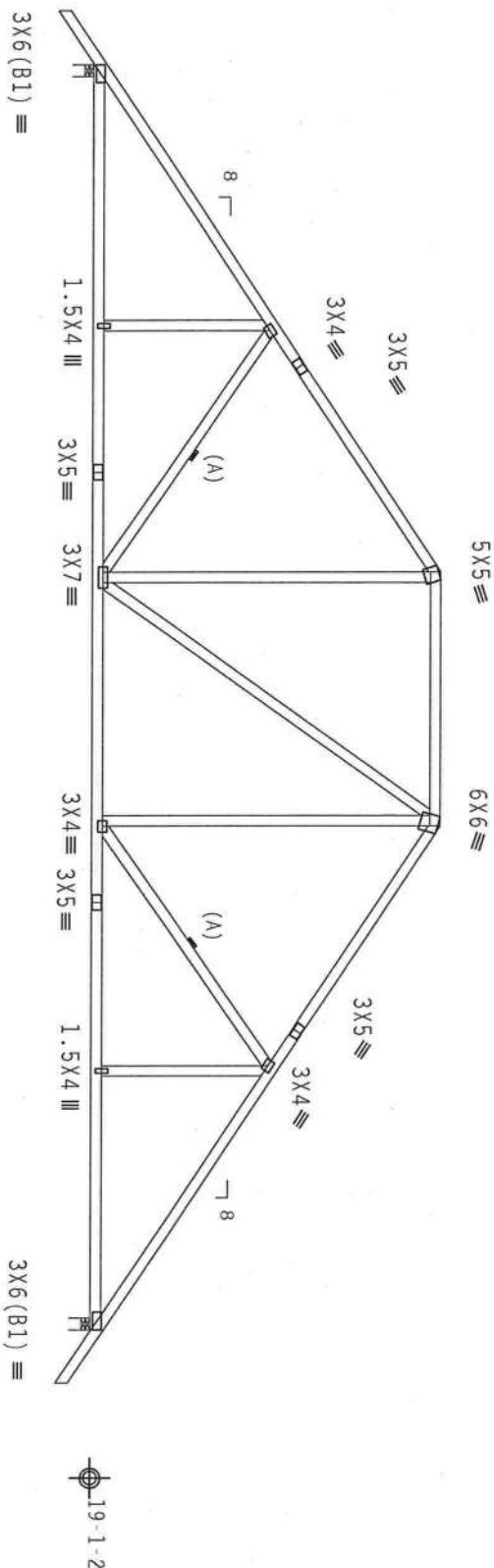


Diagram of a continuous beam with three supports. The beam is divided into four segments with lengths 14-1-8, 7-1-0, and 14-1-8. The total length is 35-4-0. The beam is labeled "Over 2 Supports". The beam is supported at three points, with the middle support being a roller support and the end supports being pin supports. The beam is labeled "R=1588 U=289 W=4"

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.36.0424

QTY:1

FL/-/4/-/E/-/-

Scale = .1875"/Ft.

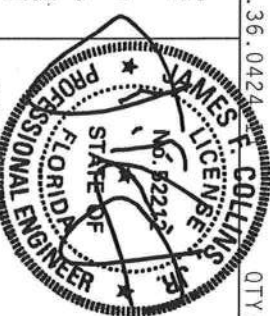


Building Components Group Inc.

Haines City, FL 33844
FL Certificate of Authorization # 0778

WARNING *HOLDS REQUIRE EXISTING GATE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BROCKING NEED TO BESEI (BUILDING CONTRACTOR SEE INFORMATION). PUBLISHED BY THE TROSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TROSS COUNCIL OF AMERICA, 65000 ENTERPRISE LANE, NORTON, MA 01919) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE ACTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT*** *TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TROSS ON FABRICATING, HANDLING, SHIPPING, INSTALLING A BUCKING OF TROSSSES. DESIGN CONDITIONS WITH APPLICABLE PROPERTIES OF WOOD (NATIONAL DESIGN SPEC. BY AIA/AA AND TP1. THE REG. INC. SHALL BE RESPONSIBLE FOR THE TROSS ON FABRICATING, HANDLING, SHIPPING, INSTALLING A BUCKING OF TROSSSES. CONNECTION PLATES ARE MADE OF 2010/1664 (W/H/55/2) ASTM A563 GRADE 40/60 (W/H/55) GALV. STEEL. APPLY PLATES TO EACH FACE OF TROSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AIA/AA OR TP1-2002 SECTION. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SUELY FOR THE TROSS COMPONENTS OF THE DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER AIA/TP1 SEC. 2.



Jan 31 08

TC LL	20.0 PSF	REF	R8228- 77030
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031076
BC LL	0.0 PSF	HC-ENG EC/WHK	*
TOT.LD.	40.0 PSF	SEQN-	73635
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228Z03

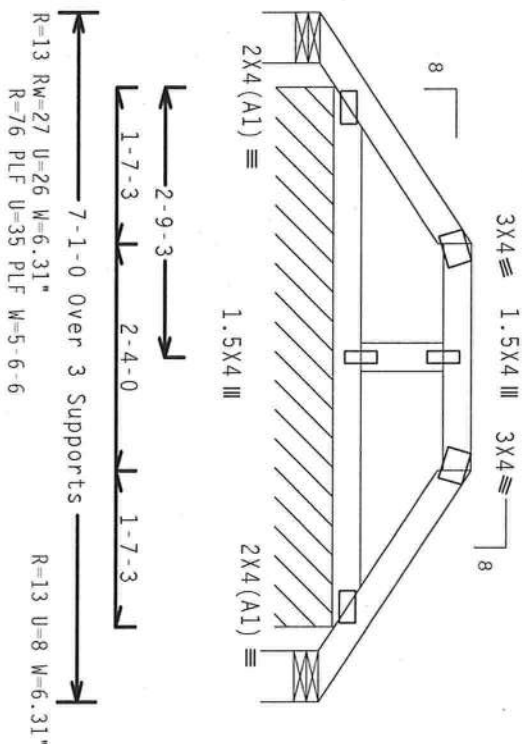
	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	webs	2x4	SP	#3		

110 mph wind, 29.67 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf, 1w=1.00 gcpi(+/-)-0.18

Wind reactions based on MMFRS pressures.

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50.

REFER TO DRAWING PIGBACK80204 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE
BRACED @ 24" O.C.



	(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From	64 PLF at 2.38 to 64 PLF at 4.71
TC - From	64 PLF at 2.38 to 64 PLF at 4.71
TC - From	64 PLF at 4.71 to 64 PLF at 7.08
BC - From	4 PLF at 0.00 to 4 PLF at 7.08

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" OC, all BC @ 24" OC.



PLT TYP. Wave

Design Crit: $TPI-2002(STD)/FBC$
 $Cq/RT=1.00(1.25)/0(0)$

7.36.0424

QTY:1

FL/-/4/-/E/-/-

Scale = .5" / Ft.



Building Components Group Inc.

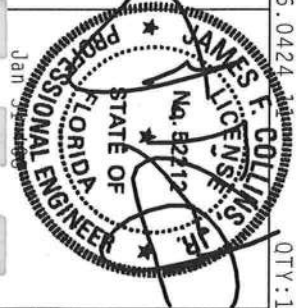
Haines City, FL 33844
FL Certificate of Authorization # 0778

WARNING - BEFORE REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING CODES FOR EXTREME CARE IN FABRICATION. MANUFACTURED BY TPI (TROSS PULTRUSTRATE INSTITUTE), 2100 HUNTERS LANE, SUITE 312, ALEXANDRIA, VA, 22304 AND UFG (UNITED TRUSS COMPANY OF AMERICA, 65000 WILSON ROAD, SUITE 100, WILSON, NC 27157). THE FOLLOWING DIRECTIONS, UNLESS OTHERWISE INDICATED, FOR GROUND SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM GROUND SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT - TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTORS. THE REG., INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TYPE OR FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AOS (NATIONAL DESIGN SPEC. BY AIAA) AND TPI. THE REG. SHALL CONDUCT PLATES ARE MADE OF 2018/1606 (ALUMINUM) ASH OR A6063 GRADE 40/60 (OR 47H/55) GALV. STEEL. APPLY AN INSULATION OF 1/2" POLYSTYRENE FOAM OR 1" POLYSTYRENE FOAM TO THE INSIDE OF THE TRUSS. POSITION PER DRAWINGS. T606-2, AN INSULATION OF PLATES FORMED BY THE INSIDE OF THE TRUSS. POSITION PER DRAWINGS. T606-2.

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TRUSS COMPONENTS OF THE DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

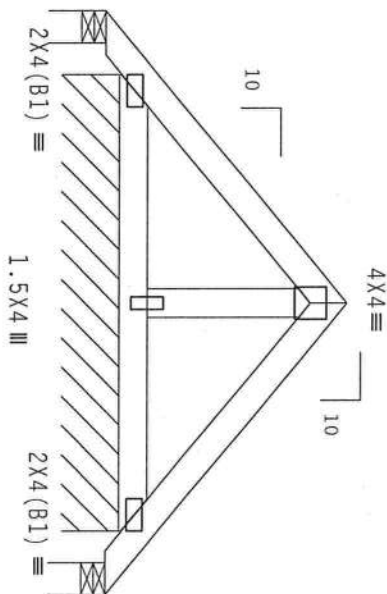


TC LL	20.0 PSF	REF	R8228- 77031
TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031081
BC LL	0.0 PSF	HC-ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN-	73637
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL8228203

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

REFER TO DRAWING PIGBACKB0204 FOR PIGGYBACK DETAILS
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE
BRACED @ 24" O.C.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. 1w=1.00 GCPI (+/-)=0.18



0-0-6

9-1-12

Diagram of a three-span continuous beam with dimensions and support conditions:

- Span 1 (Left): 2-4-2
- Span 2 (Middle): 2-4-2
- Span 3 (Right): 2-4-2
- Supports: 6-0-0 Over 3 Supports
- Boundary Conditions:
 - Left end: R=-1, R_w=10, U=46, W=4"
 - Right end: R=-1, R_w=13, W=4"
- Internal Load: R=102 PLF, U=9 PLF, W=4-8-4

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.36.0424

QTY:1

FL/-/4/-/E/-/-

Scale = .5" / Ft.



Building Components Group Inc.

Haines City, FL 33844

FI Certificate of Authorization # 00778

WARNING WHILE REMOVING EXISTING CASE IN INFORMATION, HANDLING, UNLOADING, INSTALLING AND BRACING REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PAPER 18511111), 218 NORTH LEE STREET, SUITE 312, WEST VIRGINIA, VA, 22314 AND GOOD TRUSS COMPANY OF AMERICA, 65000 ENTERPRISE LANE, SUITE 117, 53719 FOR SAFETY PRACTICES AND PITCH TO PREVENT THESE STRUCTURES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

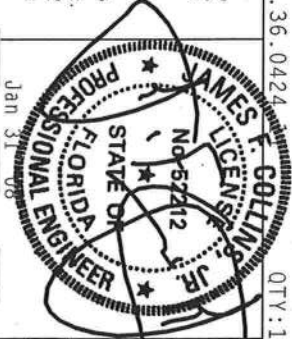
****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

TP1; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

CONNECTOR PLATES ARE MADE OF 20/18/16GA (U.S./SS/K) ASTM A653 GRADE 40/60 (U.S./K/U.S.S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 16GA-2

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT AND INSPECTION FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IP11 2002 SEC.3, A SEAL ON THIS

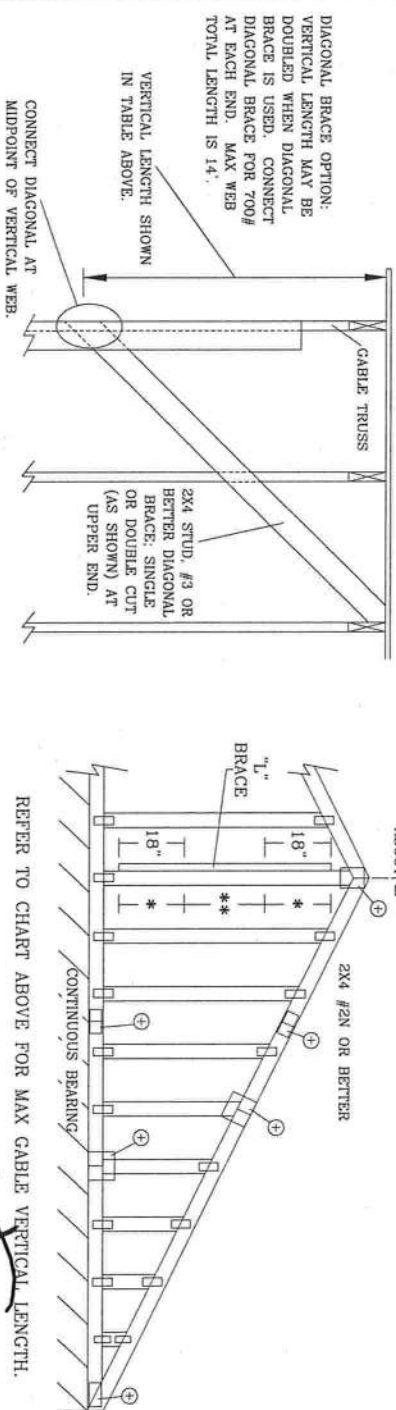
BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



Jan 31 08

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TC DL	10.0 PSF	DATE	01/31/08
BC DL	10.0 PSF	DRW	HCUSR8228 08031082
BC LL	0.0 PSF	HC - ENG	EC/WHK
TOT.LD.	40.0 PSF	SEQN -	73654
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF -	1TEL8228203

MAX GABLE VERTICAL LENGTH		2x4		BRACE		NO		(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE **	
GABLE VERTICAL SPACING	SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STUD	3' 9"	5' 7"	5' 7"	7' 3"	7' 3"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	STANDARD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
24" O.C.	SPF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 7"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SPF	#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	STUD	4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"



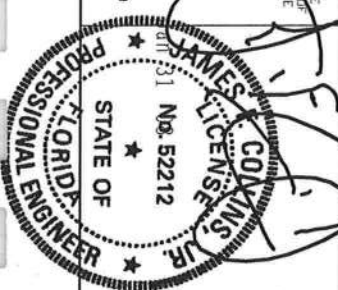
CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).	
GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 10d NAILS.	
* FOR (1) "L" BRACE: SPACE NAILS AT 2' O.C. IN 18" END ZONES AND 4' O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES: SPACE NAILS AT 3' O.C. IN 18" END ZONES AND 6' O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	
CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND HEEL PLATES.	

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

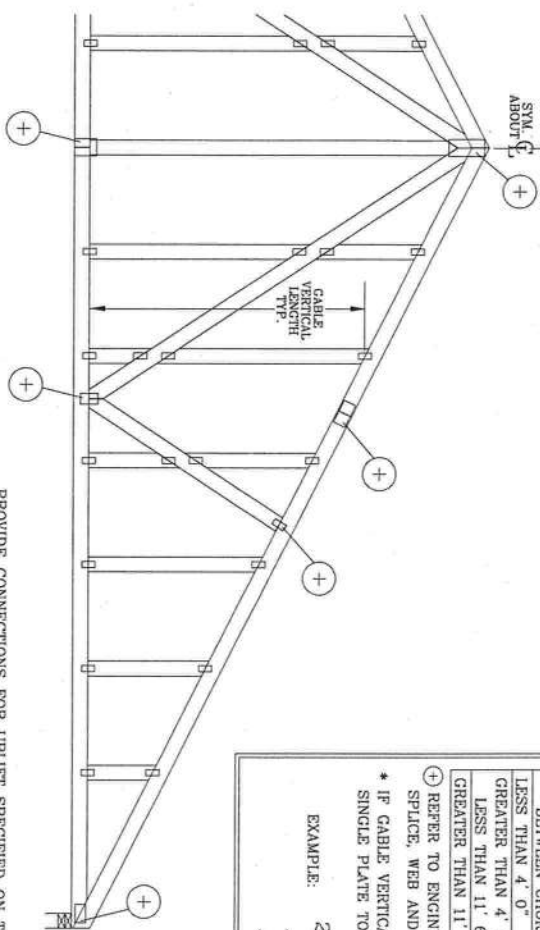
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY ITW TRUSSES PLATE DESIGN, FOR PROPER TRUSS HANDLING, STORAGE, AND ERECTION. SEE DESIGNER'S SPECIFICATIONS FOR ALL TRUSS FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSSES ACCORDING TO THE DESIGN, OR ANY DAMAGE TO THE TRUSSES DURING FABRICATING, HANDLING, SHIPPING, OR INSTALLING. ITW BCG CONNECTOR PLATES ARE MADE OF 20/8/16GA GALV/SS/20 ASTM A653 GRADE 40/60 GALV/SS GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED IN THIS DESIGN, POSITION PER DRAWINGS 16A-2. AN INSPECTION OF PLATES FOLLOWED BY GD SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER ANSI/AP1 1 SEC. 6.



REF	ASCE7-02-CAB1030
DATE	2/23/07
DRWG	A11030E0207
-ENG	
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

GABLE DETAIL FOR LET-IN VERTICALS



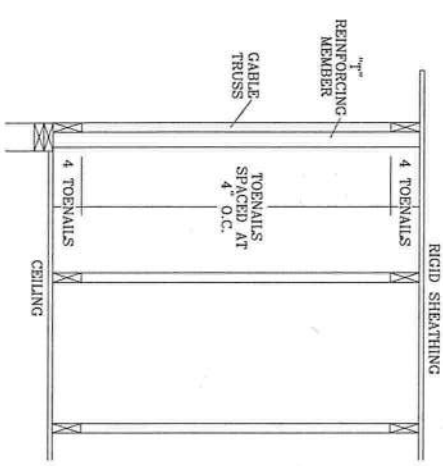
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

* REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

IF CABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

HAND DRIVEN NAILS:

10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS

(4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:

8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS

(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS

A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

ASCE 7-98 GABLE DETAIL DRAWINGS

A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A09015EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A09030EC0207

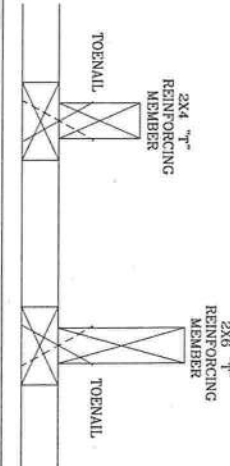
ASCE 7-02 GABLE DETAIL DRAWINGS

A13015EB0207, A12015EB0207, A11015EB0207, A10015EB0207, A09015EB0207, A13030EB0207, A12030EB0207, A11030EB0207, A10030EB0207, A09030EB0207

ASCE 7-05 GABLE DETAIL DRAWINGS

A13015ES0207, A12015ES0207, A11015ES0207, A10015ES0207, A09015ES0207, A13030ES0207, A12030ES0207, A11030ES0207, A10030ES0207, A09030ES0207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.



TO CONVERT FROM "T" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "T" BRACE GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED "T" REINFORCING MEMBER SIZE	SBCCI	ASCE
110 MPH 2x4	10 %	10 %
15 FT 2x6	40 %	50 %
110 MPH 2x4	10 %	10 %
30 FT 2x6	50 %	50 %
100 MPH 2x4	10 %	10 %
15 FT 2x6	30 %	50 %
100 MPH 2x4	10 %	10 %
30 FT 2x6	40 %	40 %
90 MPH 2x4	20 %	10 %
15 FT 2x6	20 %	40 %
90 MPH 2x4	10 %	10 %
30 FT 2x6	30 %	50 %
80 MPH 2x4	10 %	20 %
15 FT 2x6	20 %	30 %
80 MPH 2x4	10 %	10 %
30 FT 2x6	20 %	40 %
70 MPH 2x4	0 %	20 %
15 FT 2x6	0 %	20 %
70 MPH 2x4	10 %	20 %
30 FT 2x6	10 %	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH

MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

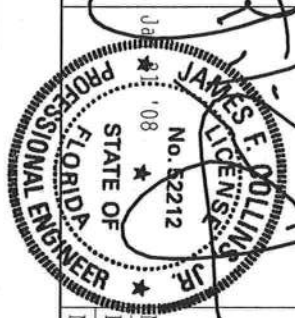
(1) 2X4 "T" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"

THIS DRAWING REPLACES DRAWINGS GAB98117 876.719 & HC26294035



ITV BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA



REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLETTIN0207
ENG	DJ/KAR
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE, FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



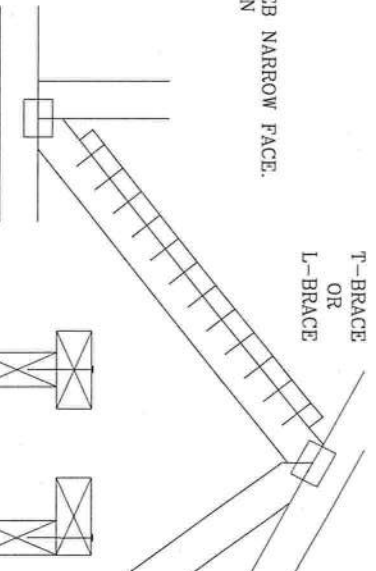
INTELLIGENT COMPONENTS GROUP, INC.
POMERANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 218 NORTH LEE ST., SUITE 312, ALEXANDRIA, VA 22314 AND VITA CADD TRUSS COUNCIL OF AMERICA, 6360 ENTERPRISE LN., HANSON, MI 48030 FOR SAFETY PRACTICES PRIOR TO PERFORMING THE TRUSS. TRUSS CHORDS SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE TRUSS PLATE INSTITUTE, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONTRACTOR WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AIRWAY AND TRUSS PLATE INSTITUTE. TRUSS CHORDS SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND SKESS OTHERS. GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND SKESS OTHERS. DESIGN, POSITION PER DRAWINGS 1604-Z. ANY INSPECTION OF PLATES FOLLOWED BY CD SHALL BE PER ANNEK A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

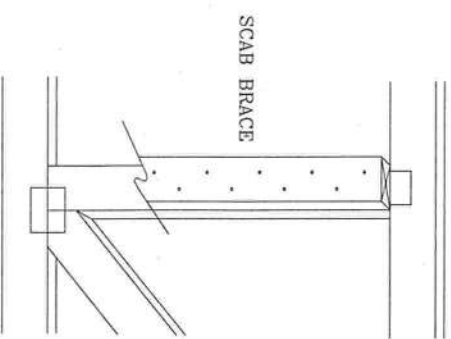
T-BRACING
OR
L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C. BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



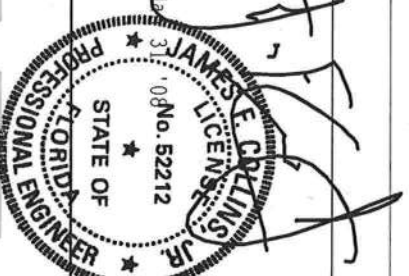
SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN
(0.128" x 3" MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH

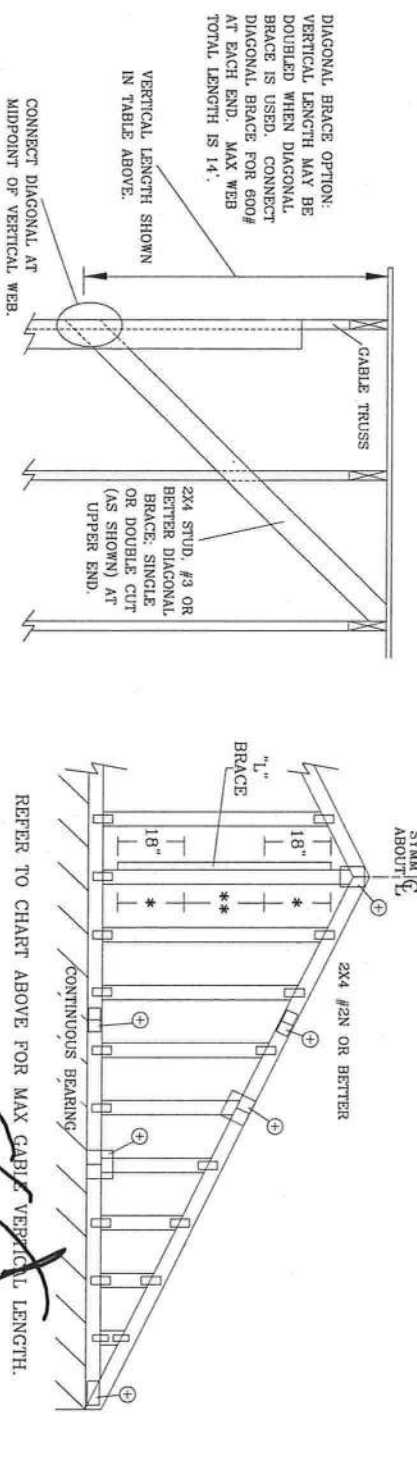


THIS DRAWING REPLACES DRAWING 579.640

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCB SUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			



MAX GABLE VERTICAL LENGTH																			
2x4 GABLE VERTICAL				BRACE		NO		(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE **	
SPACING		SPECIES		GRADE		BRACES		GROUP A		GROUP B		GROUP A		GROUP B		GROUP A		GROUP B	
12" O.C.	SPF	#1 / #2	STUD	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"			
		HF	STANDARD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"			
		SP	#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				#3	4' 0"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
			#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		SPF	#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				HF	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		16" O.C.	SPF	STANDARD	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
					#1	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SP	#2			4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				#3	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
DFL	STANDARD			4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				#1 / #2	4' 11"	6' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
24" O.C.	SPF	STANDARD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			#1	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		HF	STANDARD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		SP	#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
	DFL	STANDARD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			#1	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		



GABLE VERTICAL PLATE SIZES		NO SPLICE	
VERTICAL LENGTH	LESS THAN 4' 0"	1x4 OR 2x3	2x4
VERTICAL LENGTH	GREATER THAN 4' 0" BUT LESS THAN 11' 6"	2x4	2.5x4
VERTICAL LENGTH	GREATER THAN 11' 6"	2.5x4	2.5x4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

TRUSS BUILDING COMPONENTS GROUP, INC.
POMPAHO BEACH, FLORIDA

STATE OF FLORIDA
JAMES E. COLLINS, JR.
PROFESSIONAL ENGINEER
No. 52212
08/31/08

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF ASCE 7-02-CAB11015
DATE 2/23/07
DRWG A11015E0207
-ENG

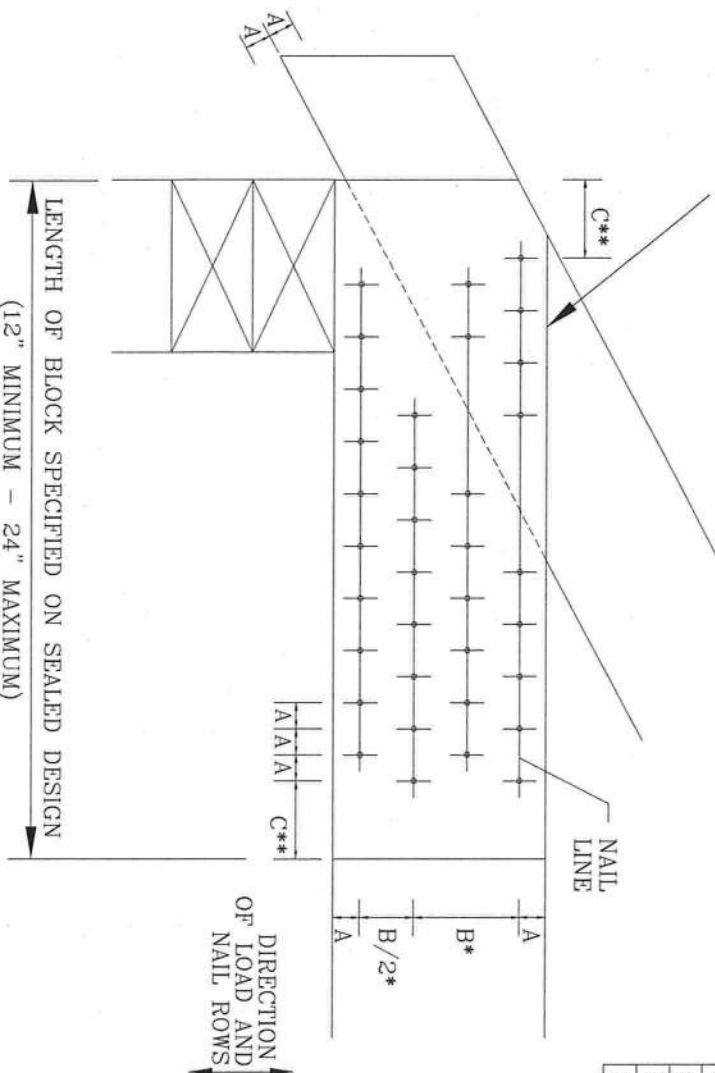
MAXIMUM NUMBER OF LINES PARALLEL TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C - END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

- * SPACING MAY BE REDUCED BY 50%
- ** SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (Fc-perp) IS AT LEAST THAT OF THE CHORD.



NAIL TYPE	CHORD SIZE					
	2X4	2X6	2X8	2X10	2X12	
8d BOX (0.113"X 2.5",MIN)	3	6	9	12	15	
10d BOX (0.128"X 3.",MIN)	3	5	7	10	12	
12d BOX (0.128"X 3.25",MIN)	3	5	7	10	12	
16d BOX (0.135"X 3.5",MIN)	3	5	7	10	12	
20d BOX (0.148"X 4.",MIN)	2	4	5	6	8	
8d COMMON (0.131"X 2.5",MIN)	3	5	7	10	12	
10d COMMON (0.148"X 3.",MIN)	2	4	6	8	10	
12d COMMON (0.148"X 3.25",MIN)	2	4	6	8	10	
16d COMMON (0.162"X 3.5",MIN)	2	4	6	8	10	
GUN (0.120"X 2.5",MIN)	3	6	8	11	14	
GUN (0.131"X 2.5",MIN)	3	5	7	10	12	
GUN (0.120"X 3.",MIN)	3	6	8	11	14	
GUN (0.131"X 3.",MIN)	3	5	7	10	12	

MINIMUM NAIL SPACING DISTANCES

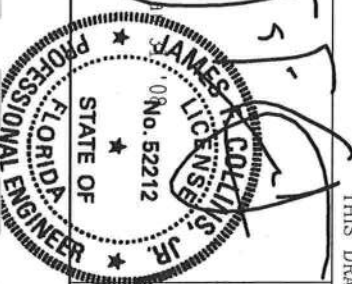
NAIL TYPE	DISTANCES		
	A	B*	C**
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"

THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK06999

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

IMPORTANT
 TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC&I BOLDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSSING INSTITUTE, 218 NORTH LEA STR., SUITE 312, ALABAMA, VA 22314, AND VITA C/O DODD TRUSS COMPANY, 6300 ENTERPRISE LN, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STROBE/ROD PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.
 IMPORTANT
 CLARIFY COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. THE BCG INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS CONFORMANCE WITH THE FABRICATING, HANDLING, SHIPPING, INSTALLING, BRACING, OR TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE NATIONAL DESIGN SPEC. BY AREA AND THE U.S. BCG CONNECTOR PLATES ARE MADE OF 20/18/616GA (C/A/S/S/S) ASTM A653 GRADE 40/60 (C/A/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TOP AND UNLESS FOLDED-IN LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1604-Z. ANY INSPECTION OF PLATES FOLDED-IN BY GD SHALL BE PERFORMED AT THE RESPONSIBILITY OF THE USER. THE BOLDING INDICATES ACCEPTANCE OF THE DESIGN OF THIS COMPONENT. THE BOLDING INDICATES ACCEPTANCE OF THE DESIGN OF THE TRUSS AND THE USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER U.S. BCG TRP1, SEC. 2.



REF	BEARING BLOCK
DATE	2/23/07
DRWG	CNBRGBLK0207
-ENG	SJP/KAR

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: ITEL487-Z0130162358

Truss Fabricator: Anderson Truss Company
Job Identification: deardorf-
Truss Count: 22
Model Code: Florida Building Code 2004 and 2006 Supplement
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.24.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration
Floor - 55.0 PSF @ 1.00 Duration
Wind - 110 MPH ASCE 7-02 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: -



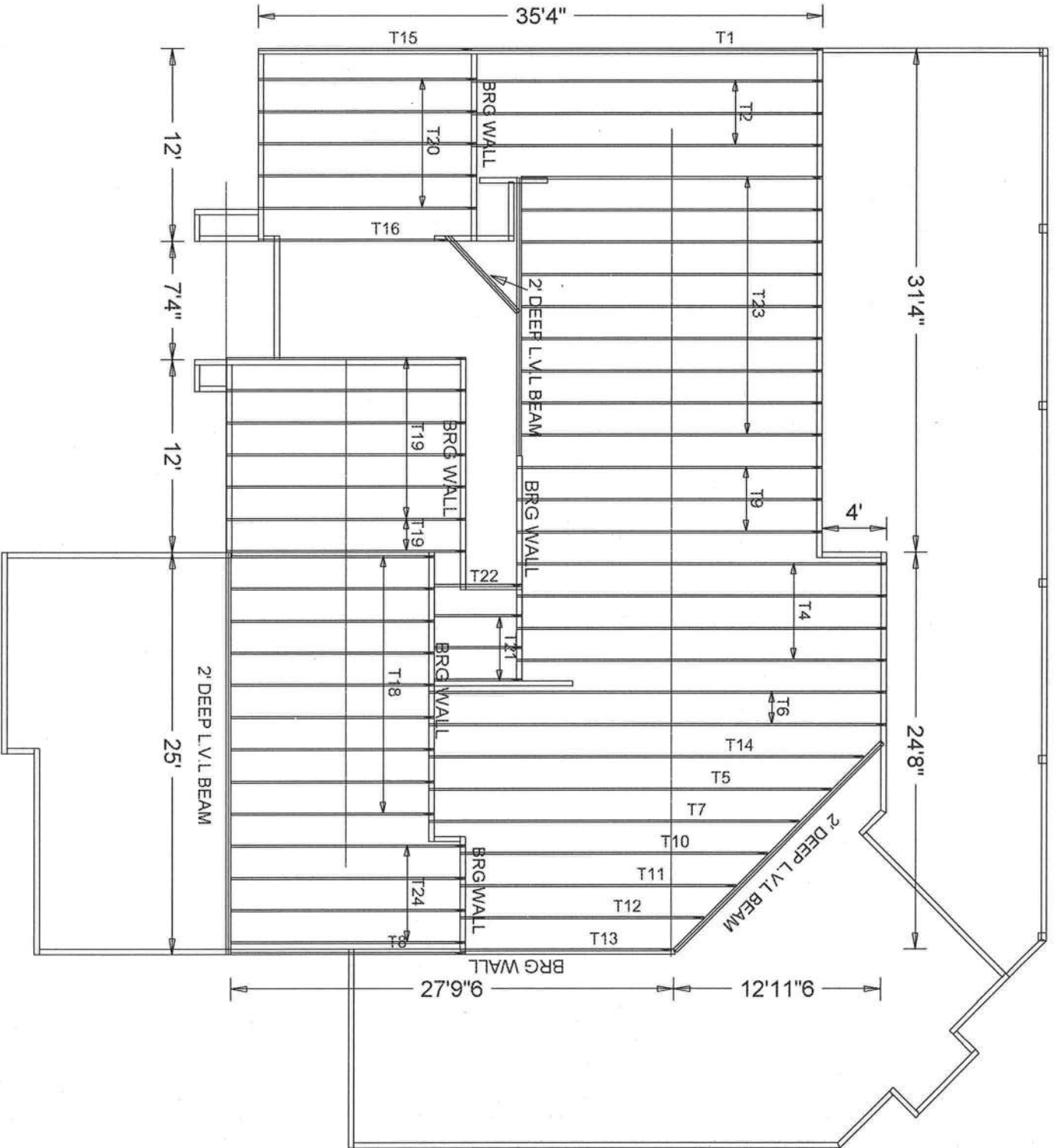
Seal Date: 01/30/2008

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	75075--T2		08030115	01/30/08
2	75076--T23		08030116	01/30/08
3	75077--T9		08030117	01/30/08
4	75078--T4		08030118	01/30/08
5	75079--T1		08030119	01/30/08
6	75080--T6		08030120	01/30/08
7	75081--T14		08030121	01/30/08
8	75082--T5		08030122	01/30/08
9	75083--T7		08030123	01/30/08
10	75084--T15		08030124	01/30/08
11	75085--T19		08030125	01/30/08
12	75086--T18		08030126	01/30/08
13	75087--T24		08030127	01/30/08
14	75088--T8		08030128	01/30/08
15	75089--T10		08030129	01/30/08
16	75090--T11		08030130	01/30/08
17	75091--T12		08030131	01/30/08
18	75092--T13		08030132	01/30/08
19	75093--T20		08030133	01/30/08
20	75094--T21		08030134	01/30/08
21	75095--T22		08030135	01/30/08
22	75096--T16		08030136	01/30/08



STANLEY CRAWFORD / DEARDORF FLOOR



JOB LOCATION:

JOB DESCRIPTION:

DESIGNED BY:
Jenny Patterson

JOB NO:
deardorf

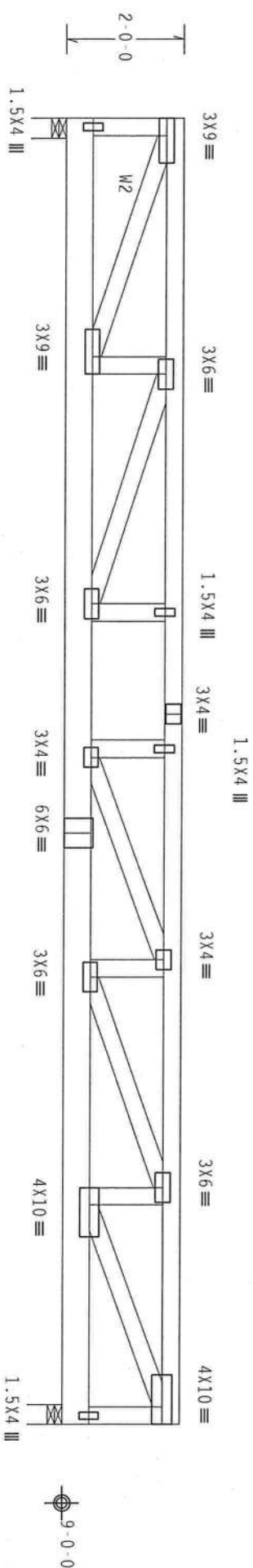
PAGE NO:
1 OF 1

(deardorf - T2)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3 :W2 2x4 SP #2 Dense:

Truss must be installed as shown with top chord up.

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1210 W=4" 22-0-0 Over 2 Supports R=1210 W=4"

PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 7.24.1230 J.E. COLLINS QTY:3 FL/-/4/-/E/-/- Scale = .375"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6200 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



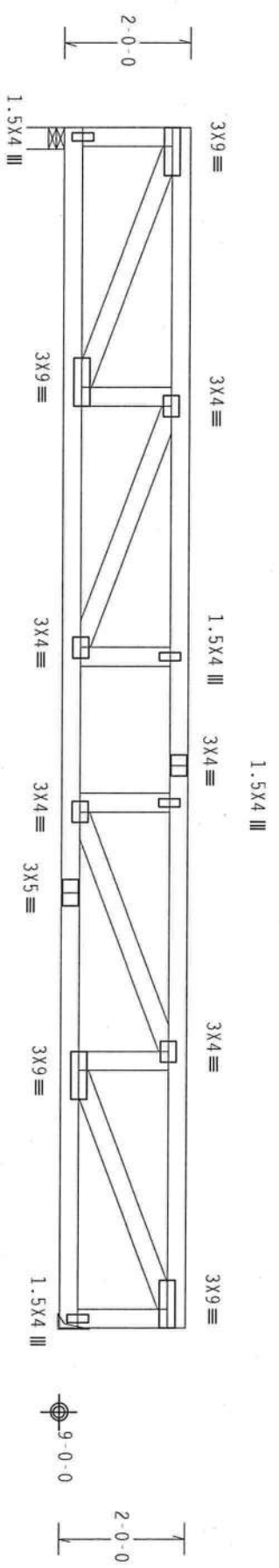
Building Components Group, Inc.
TW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #00378



TC LL	40.0 PSF	REF	R487--	75075
TC DL	10.0 PSF	DATE	01/30/08	
BC DL	5.0 PSF	DRW	HCUSR487	08030115
BC LL	0.0 PSF	HC-ENG	JB/AP	
TOT. LD.	55.0 PSF	SEON-	43627	
DUR. FAC.	1.00	FROM	JP	
SPACING	24.0"	UREF-	ITEL487	201

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	Wbs	2x4	SP	#3	

Truss must be installed as shown with top chord up.



$R=1038 \text{ W-4"} \quad 18-10-8 \text{ Over 2 Supports} \quad R=1038$

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY: 9

FL/-/4/-/E/-/-

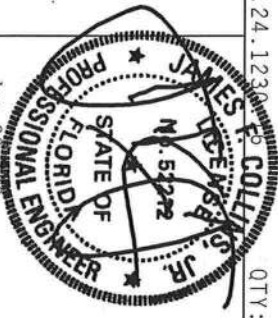
Scale = .375" / Ft.

***WARNING:** *MUST RECOGNIZE EXTREME CARE IN FABRICATION, HANDLING, SHIPMENT, INSTALLING AND BRACING. REFER TO BEST AVAILABLE CODES FOR DESIGN REQUIREMENTS. THIS PRODUCT IS A TRADE SECRET OF THE TROSS HALL INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA 22314 AND WICA GROUP TRUSS COMPANY OF AMERICA, 6700 ENTERPRISE LAKE, HOUSTON, TX 55719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.



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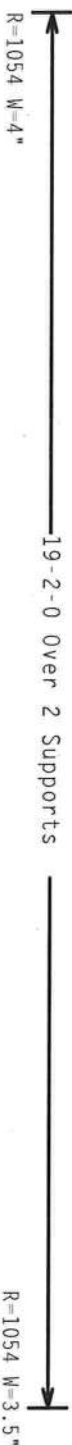
ITW Building Components Group, Inc.
Haines City, FL 33844
FI Certificate of Authorization # 0 078



FL/-/4/-/E/-/-		Scale = .375"/Ft.
TC LL	40.0 PSF	REF R487 - - 75076
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUR487 08030116
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SECN - 43636
DUR.FAC.	1.00	FROM JP
SPACING	24.0"	JREF - 1TEL487 201

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

See ANSII/TPI 1-02 Sect 7.5 for additional bracing requirements.



Scale = .375"/Ft.



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ITW Building Components Group, Inc.

F-1 Certificate of Authorization # 0779

FARING—MRS. RICHIE EPPERT CASE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BEARING. REFER TO SC-1 (DOWDING COMPONENT OF SAFETY INFORMATION). PUBLISHED BY FBI (THROSS PLANT INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WPCA (WOOD ROTH ROSS COUNCIL OF AMERICA, 6200 GARDEN ENTERPRISE LANE, MADISON, WI, 53719) FOR SAFETY PRACTICES PRIOR TO REFORMING THESE COMPONENTS. UNLESS OTHERWISE INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CHALLING.

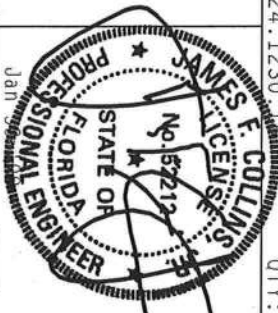
****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT

TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS 160A-3 CONNECTION PLATE WITH THICKNESS OF 20/10/10mm (W, H, 32/8) MAIN ROSS WROTE 90/50 (W, H, 32/8) GALV. STEEL, APPLY

DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

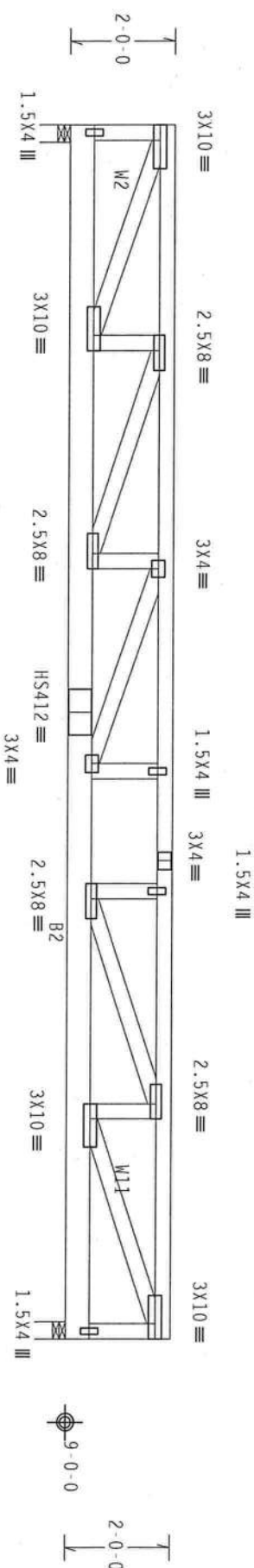


TC LL	40.0 PSF	REF	R487-- 75077
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCUSR487 08030117
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43643
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	ITEL487 Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2 : B2 2x6 SP #1 Dense:
Webs 2x4 SP #3 : W2, W11 2x4 SP #2 Dense:

Truss must be installed as shown with top chord up.

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



23-2-0 Over 2 Supports
R=1274 W=4"

PLT TYP. 20 Gauge HS.Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY:4 FL/-/4/-/E/-/-

Scale = .3125"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE CONCRETE OF AMERICA, 6300 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WPCA (WOOD TRUSS CONCRETE OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



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Haines City, FL 33844
FL Certificate of Authorization #0 778

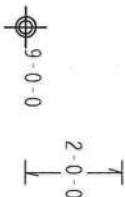


TC LL	40.0 PSF	REF R487-- 75078
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUR487 08030118
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SEON- 43652
DUR.FAC.	1.00	FROM JP
SPACING	24.0"	JREF- 1TEL487 201

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements.

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.



R=1577 W=4

Scale = .25"/Ft.

NO. 5212

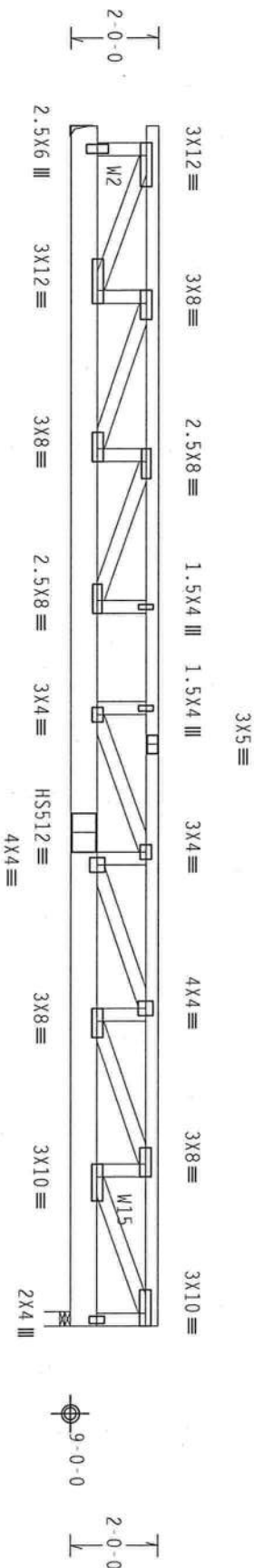
Building Components Group Inc

Jan 30 08

TC LL	40.0 PSF	REF	R487 - - 75080
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCSR487 08030120
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43675
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF -	1TEL487 201

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x8	SP	SS	
	Webs	2x4	SP	#3	:W2, W15 2x4 SP #2 Dense:

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



27-3-2 Over 2 Supports $R=1499$

R=1499 W=4"

PLT TYP. 20 Gauge HS, Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .25" / Ft.

WARNING SERIES REQUIRE EVIDENCE CAME IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING REFER TO GC51 (BUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY FBI (FBI/ISSP PUBLICATION), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND MICA (MILITARY INTELLIGENCE CENTER OF AMERICA), 6500 ROCKFORD ENTERPRISE LANE, MIDDLETON, WI, 53521 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE OPERATIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED FIELD CILLING.

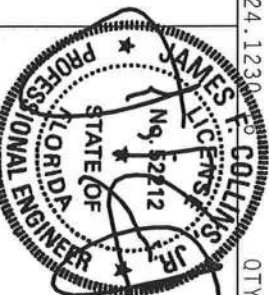


Building Components Group Inc

ITW Building Components Group, Inc.

Haines City, FL 33844

F1 Certificate of Authorization # 0 779



Jan 30 08

TC LL	40.0 PSF	REF	R487-- 75081
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HGUSR487 08030121
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43683
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	1TEL487 Z01

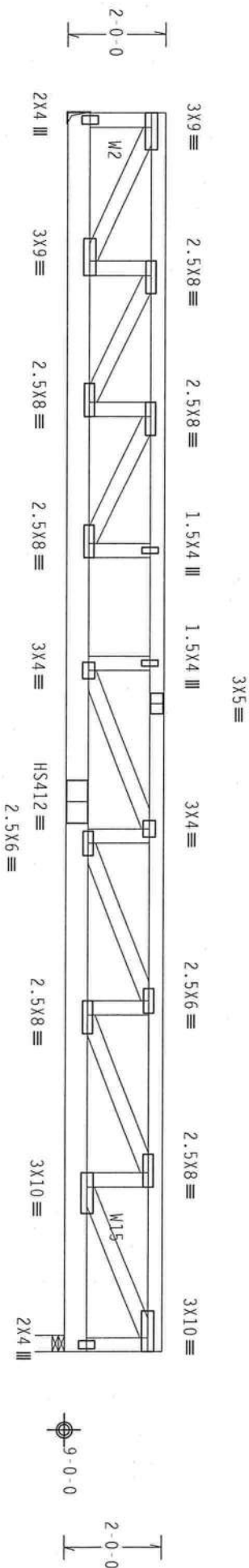
(deardorf - T5)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3 :W2, W15 2x4 SP #2 Dense:

Truss must be installed as shown with top chord up.

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=1389
25'-3"-2 Over 2 Supports
R=1389 W=4"

PLT TYP. 20 Gauge HS.Wave

Design Crit: TPI-2002(STD) /FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .3125"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 2800 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6000 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



Building Components Group Inc.

ITW Building Components Group, Inc.

Haines City, FL 33844
FPI Certificate of Authorization #A-0778



TC LL	40.0 PSF	REF R487-- 75082
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUSR487 08030122
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SEON- 43694
DUR.FAC.	1.00	FROM JP
SPACING	24.0"	JREF- 1TEL487 201

THE UNIVERSITY OF CHICAGO PRESS

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements. Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

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R=1279 W=4"

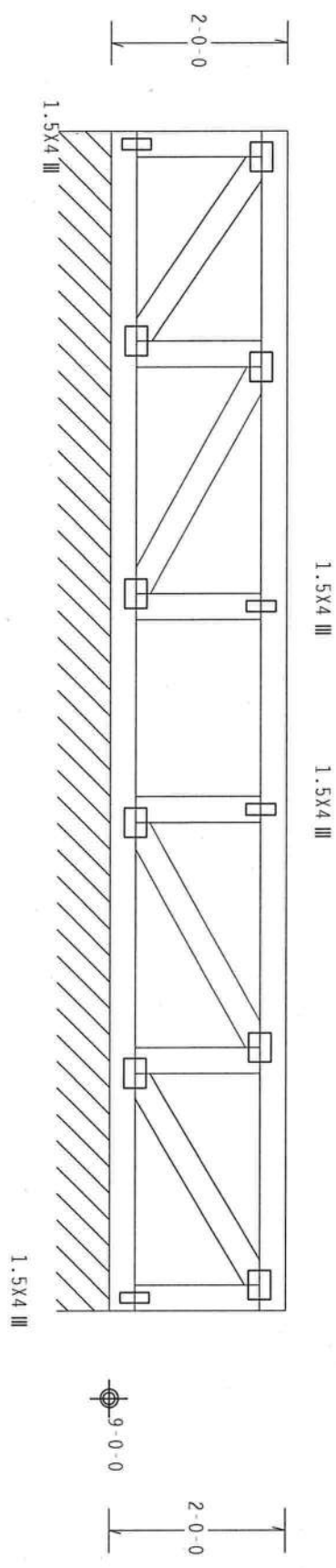
Scale = .3125"/Ft.



TC LL	40.0 PSF	REF	R487 - 75083
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCUSR487 08030122
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43704
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	ITEL487 201

(deardorf - T15)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.
Truss must be installed as shown with top chord up.



R=110 PLF W=13-4-0
13-4-0 Over Continuous Support

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave Design Crtt: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230 16 QTY:1 FL/-/4/-/E/-/- Scale = .5"/ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



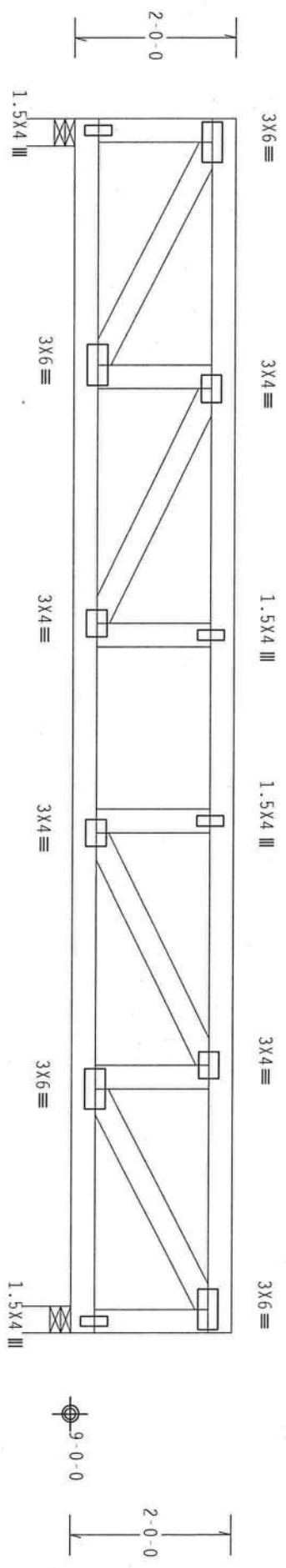
Building Components Group Inc.
TW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0278



TC LL	40.0 PSF	REF R487 - 75084
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUSR487 08030124
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SEQN- 43746
DUR.FAC.	1.00	FROM JP
SPACING	24.0"	JREF- ITEL487_201

(deardorf - T19)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.
Truss must be installed as shown with top chord up.



R-825 W=4" 15'-0" Over 2 Supports R-825 W=4"

PLT TYP. Wave Design Cr1t: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0.00 7.24.1230 QTY: 8 FL/-/4/-/E/-/- Scale = .5"/ft.

WARNING BRUSSES PROHIBIT EXTERIOR CODE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO NCST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND WCA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

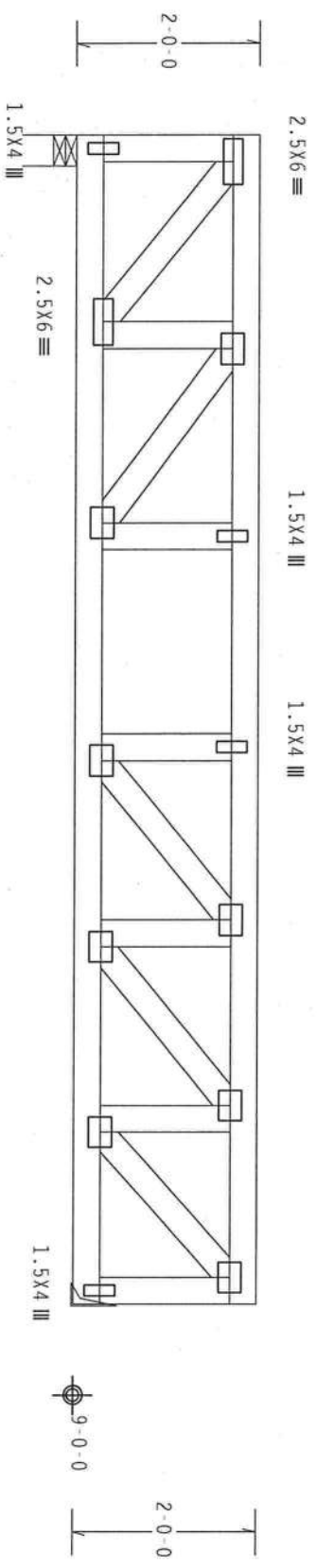
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0778

Professional Engineer Seal: JAMES E. COLLINS, No. 62212, State of Florida, Jan 30 08

TC LL	40.0 PSF	REF	R487-- 75085
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCUSR487 08030125
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43752
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	ITEL487 Z01

(deardorf - T18)
 Top chord 2x4 SP #2 Dense
 Bot chord 2x4 SP #2 Dense
 Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.
 Truss must be installed as shown with top chord up.



R=700 W=4"
 12-8-12 Over 2 Supports
 R=700

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave
 Design Crit: TPI-2002(STD)/FBC
 Cq/RT=1.00(1.25)/0.00

7.24.1230
 QTY: 9 FL/-/4/-/E/-/-

Scale = .5"/ft.

****WARNING**** BRUSSES REQUIRE EXTENSIVE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO DCSTI (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA/PA) AND TPI. ITW BCG PLATES TO EXIST FACE OF TRUSSES 20/16/10/4 (4/1/3/5/1) ASH K665 (GRK 40/60 (R, R/2/5/5) GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE THE OWNER AS OF THIS DATE. SECTION PER DRAWINGS 1000-2, DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUSTAINABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

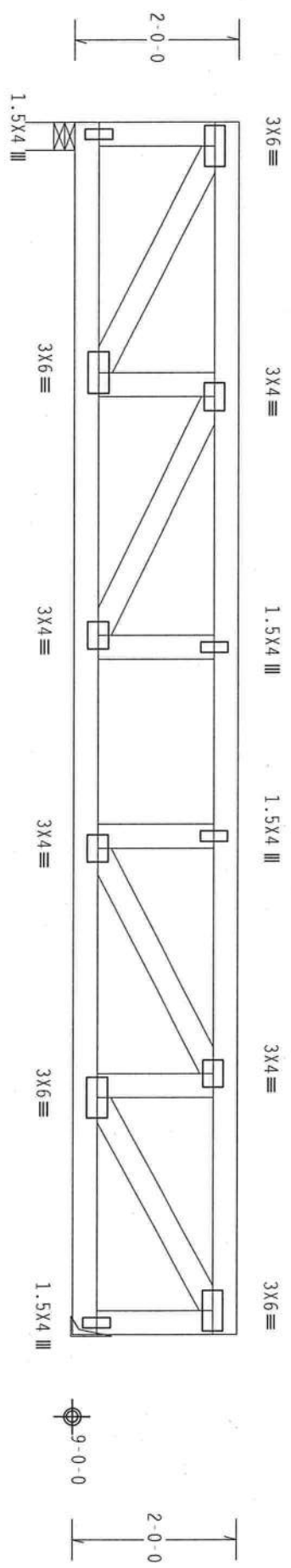
ITW Building Components Group Inc.
 Haines City, FL 33844
 PL Certificate of Authorization #A-0378



TC LL	40.0 PSF	REF	R487--	75086
TC DL	10.0 PSF	DATE	01/30/08	
BC DL	5.0 PSF	DRW	HCUSR487	08030126
BC LL	0.0 PSF	HC-ENG	JB/AP	
TOT.LD.	55.0 PSF	SEQN-	43758	
DUR.FAC.	1.00	FROM	JP	
SPACING	24.0"	JREF-	ITEL487	201

(deardorf - T24)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Truss must be installed as shown with top chord up.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



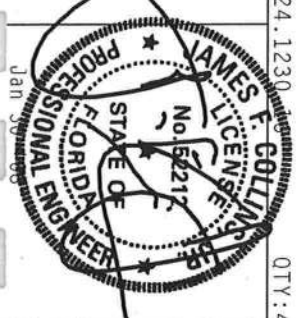
14'-8 1/2" over 2 supports
R-810 W-4"

PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)
7.24.1230
QTY: 4
FL/-/4/-/E/-/-
Scale = .5"/ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSS (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 210 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



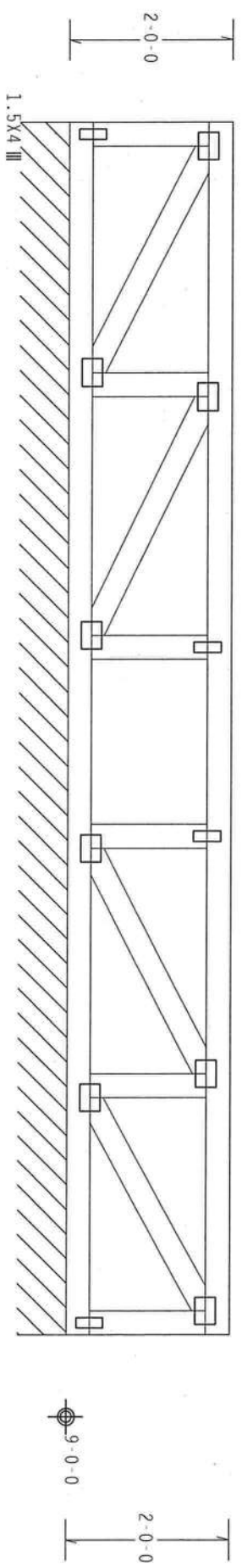
Building Components Group Inc.
TW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0778



TC LL	40.0 PSF	REF R487 - 75087
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUSR487 08030127
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SEQN- 43763
DUR.FAC.	1.00	FROM JP
SPACING	24.0"	JREF- ITEL487_201

(deardorf - T8)
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.
Truss must be installed as shown with top chord up.



R=110 PLF W=14-8-12
14-8-12 Over Continuous Support

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)

7.24.1230
QTY: 1
FL/-/4/-/E/-/-
Scale = .5"/ft.

WARNING RUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND AISC (STEEL CONSTRUCTION INSTITUTE, 500 N. MICHIGAN, SUITE 1700, CHICAGO, IL 60610) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



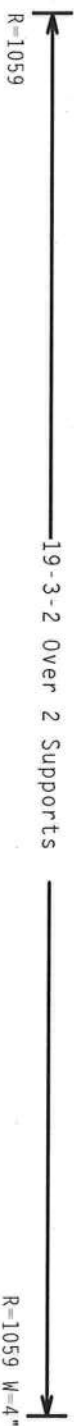
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization #0778



TC LL	40.0 PSF	REF R487 - 75088
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUSR487 08030128
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SEON - 43769
DUR.FAC.	1.00	FROM JP
SPACING	24.0"	JREF - ITEL487 Z01

Top chord	2x4	SP	#2	Dense
Bot chord	2x6	SP	#2	
Webbs	2x4	SP	#3	

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230.1 QTY:1

QTY:1

FL/-/4/-/E/-/-

Scale = .375" / Ft.



Building Components Group Inc.

ITW Building Components Group, Inc.

Haines City, FL 33844

FI Certificate of Authorization # 00779

FARRING WHO'S WHOSE ROUTINE EXHIBIT CASE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BROCKING REFER TO BC51 (0010106 COMPONENT SAFETY INFORMATION). PUBLISHED BY THE FIRE ENGINEERING INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314 AND ALSO (6000) TRUSS COMPANY, INC., 65000 AMERICA ENTERPRISE LANE, MADISON, WI, 53719 FOR SAFETY PRACTICES AND PICTURES TO PREVENTING FIRE OR OTHER UNDESIRABLE INCIDENTS INDICATED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT*** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TROSSSES.

CONTRACTORS ARE THE APPLICABLE PROVIDERS OF NON-STRUCTURAL PLASTER AND FINISHES. THE APPLICABLE CONCRETE PLASTER AREA OF 20,181/1604 (M²/HVS/FS) ASIN A655/4620 (40/60 (4/31/55) GALV. STEEL, APPLICABLE TO EACH FACE OF THISS AM, UNLESS OTHERWISE LOCATED ON THIS DECISION, POSITION PER DRAWINGS 1604-2, 1604-3, 1604-4, 1604-5, 1604-6, 1604-7, 1604-8, 1604-9, 1604-10, 1604-11, 1604-12, 1604-13, 1604-14, 1604-15, 1604-16, 1604-17, 1604-18, 1604-19, 1604-20, 1604-21, 1604-22, 1604-23, 1604-24, 1604-25, 1604-26, 1604-27, 1604-28, 1604-29, 1604-30, 1604-31, 1604-32, 1604-33, 1604-34, 1604-35, 1604-36, 1604-37, 1604-38, 1604-39, 1604-40, 1604-41, 1604-42, 1604-43, 1604-44, 1604-45, 1604-46, 1604-47, 1604-48, 1604-49, 1604-50, 1604-51, 1604-52, 1604-53, 1604-54, 1604-55, 1604-56, 1604-57, 1604-58, 1604-59, 1604-60, 1604-61, 1604-62, 1604-63, 1604-64, 1604-65, 1604-66, 1604-67, 1604-68, 1604-69, 1604-70, 1604-71, 1604-72, 1604-73, 1604-74, 1604-75, 1604-76, 1604-77, 1604-78, 1604-79, 1604-80, 1604-81, 1604-82, 1604-83, 1604-84, 1604-85, 1604-86, 1604-87, 1604-88, 1604-89, 1604-90, 1604-91, 1604-92, 1604-93, 1604-94, 1604-95, 1604-96, 1604-97, 1604-98, 1604-99, 1604-100, 1604-101, 1604-102, 1604-103, 1604-104, 1604-105, 1604-106, 1604-107, 1604-108, 1604-109, 1604-110, 1604-111, 1604-112, 1604-113, 1604-114, 1604-115, 1604-116, 1604-117, 1604-118, 1604-119, 1604-120, 1604-121, 1604-122, 1604-123, 1604-124, 1604-125, 1604-126, 1604-127, 1604-128, 1604-129, 1604-130, 1604-131, 1604-132, 1604-133, 1604-134, 1604-135, 1604-136, 1604-137, 1604-138, 1604-139, 1604-140, 1604-141, 1604-142, 1604-143, 1604-144, 1604-145, 1604-146, 1604-147, 1604-148, 1604-149, 1604-150, 1604-151, 1604-152, 1604-153, 1604-154, 1604-155, 1604-156, 1604-157, 1604-158, 1604-159, 1604-160, 1604-161, 1604-162, 1604-163, 1604-164, 1604-165, 1604-166, 1604-167, 1604-168, 1604-169, 1604-170, 1604-171, 1604-172, 1604-173, 1604-174, 1604-175, 1604-176, 1604-177, 1604-178, 1604-179, 1604-180, 1604-181, 1604-182, 1604-183, 1604-184, 1604-185, 1604-186, 1604-187, 1604-188, 1604-189, 1604-190, 1604-191, 1604-192, 1604-193, 1604-194, 1604-195, 1604-196, 1604-197, 1604-198, 1604-199, 1604-200, 1604-201, 1604-202, 1604-203, 1604-204, 1604-205, 1604-206, 1604-207, 1604-208, 1604-209, 1604-210, 1604-211, 1604-212, 1604-213, 1604-214, 1604-215, 1604-216, 1604-217, 1604-218, 1604-219, 1604-220, 1604-221, 1604-222, 1604-223, 1604-224, 1604-225, 1604-226, 1604-227, 1604-228, 1604-229, 1604-230, 1604-231, 1604-232, 1604-233, 1604-234, 1604-235, 1604-236, 1604-237, 1604-238, 1604-239, 1604-240, 1604-241, 1604-242, 1604-243, 1604-244, 1604-245, 1604-246, 1604-247, 1604-248, 1604-249, 1604-250, 1604-251, 1604-252, 1604-253, 1604-254, 1604-255, 1604-256, 1604-257, 1604-258, 1604-259, 1604-260, 1604-261, 1604-262, 1604-263, 1604-264, 1604-265, 1604-266, 1604-267, 1604-268, 1604-269, 1604-270, 1604-271, 1604-272, 1604-273, 1604-274, 1604-275, 1604-276, 1604-277, 1604-278, 1604-279, 1604-280, 1604-281, 1604-282, 1604-283, 1604-284, 1604-285, 1604-286, 1604-287, 1604-288, 1604-289, 1604-290, 1604-291, 1604-292, 1604-293, 1604-294, 1604-295, 1604-296, 1604-297, 1604-298, 1604-299, 1604-300, 1604-301, 1604-302, 1604-303, 1604-304, 1604-305, 1604-306, 1604-307, 1604-308, 1604-309, 1604-310, 1604-311, 1604-312, 1604-313, 1604-314, 1604-315, 1604-316, 1604-317, 1604-318, 1604-319, 1604-320, 1604-321, 1604-322, 1604-323, 1604-324, 1604-325, 1604-326, 1604-327, 1604-328, 1604-329, 1604-330, 1604-331, 1604-332, 1604-333, 1604-334, 1604-335, 1604-336, 1604-337, 1604-338, 1604-339, 1604-340, 1604-341, 1604-342, 1604-343, 1604-344, 1604-345, 1604-346, 1604-347, 1604-348, 1604-349, 1604-350, 1604-351, 1604-352, 1604-353, 1604-354, 1604-355, 1604-356, 1604-357, 1604-358, 1604-359, 1604-360, 1604-361, 1604-362, 1604-363, 1604-364, 1604-365, 1604-366, 1604-367, 1604-368, 1604-369, 1604-370, 1604-371, 1604-372, 1604-373, 1604-374, 1604-375, 1604-376, 1604-377, 1604-378, 1604-379, 1604-380, 1604-381, 1604-382, 1604-383, 1604-384, 1604-385, 1604-386, 1604-387, 1604-388, 1604-389, 1604-390, 1604-391, 1604-392, 1604-393, 1604-394, 1604-395, 1604-396, 1604-397, 1604-398, 1604-399, 1604-400, 1604-401, 1604-402, 1604-403, 1604-404, 1604-405, 1604-406, 1604-407, 1604-408, 1604-409, 160

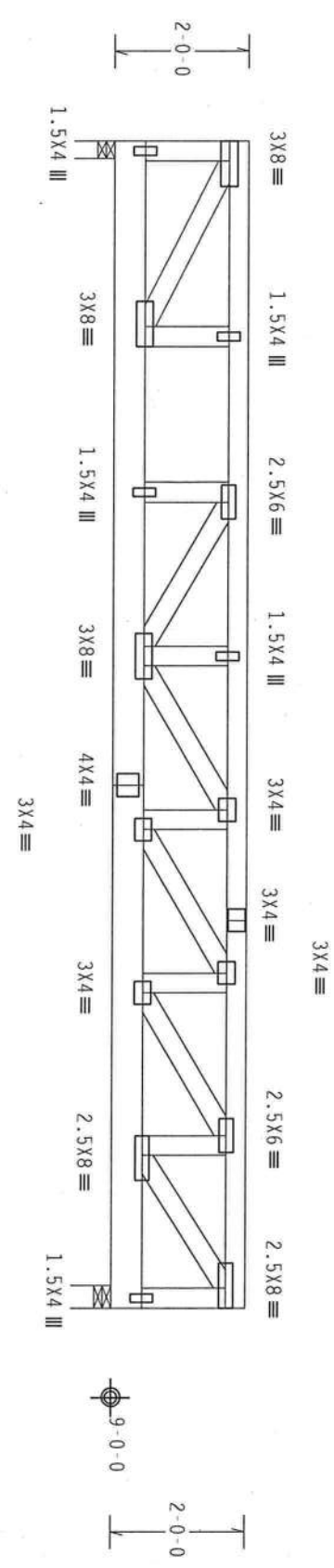


TC LL	40.0 PSF	REF	R487 - 75089
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCUSR487 08030129
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43715
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	ITEL487 201

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

Truss must be installed as shown with top chord up.

See ANSI/TPI 1-02 Sect 7.5 for additional bracing requirements.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R-949 W=3" 17-3-2 Over 2 Supports R-949 W=4"

PLT TYP. Wave Design Cr't: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0.00 7.24.1230 01/30/08 OTY:1 FL/-/4/-/E/-/ Scale =.375"/Ft.

ITW Building Components Group Inc.
Haines City, FL 33844
F1 Certificate of Authorization #0778

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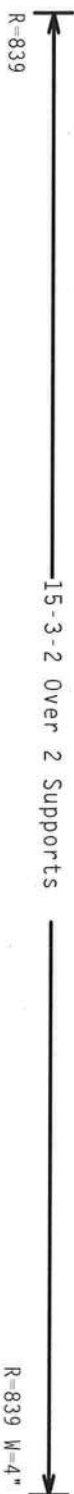
IMPORTANT - FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND UTEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

James I. Colman
Professional Engineer
State of Florida
No. 56272

TC LL	40.0 PSF	REF	R487 - 75090
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCUSR487 08030130
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN	43722
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF	1TEL487 201

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	webs	2x4	SP	#3		

Truss must be installed as shown with top chord up.



Scale = .5" / Ft.



Building Components Group Inc.

ITW Building Components Group, Inc.

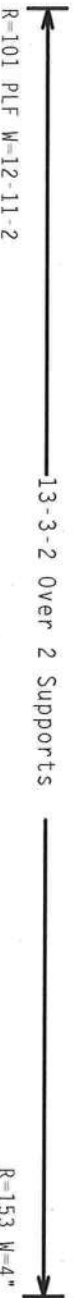
Haines City, FL 33844

FI Certificate of Authorization # 00778

TC LL	40.0 PSF	REF	R487 - 75091
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCUSR487 08030131
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43727
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	1TEL487 201

	Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense	
Web	2x4	SP	#3		

Truss must be installed as shown with top chord up.



Scale = 5" / Ft

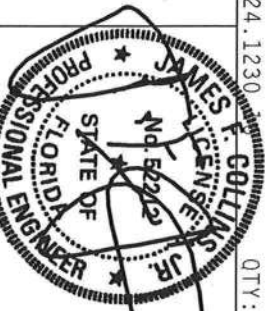


Building Components Group Inc.

ITW Building Components Group, Inc.

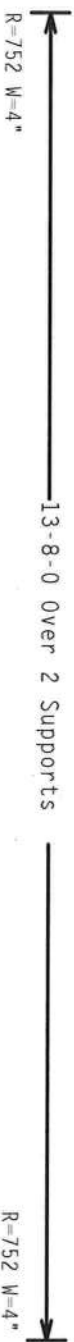
Building Components Group
Haines City, FL 33844

FL Certificate of Authorization # 0078



TC LL	40.0 PSF	REF	R487-- 75092
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCSR487 08030132
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEON-	43733
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	ITEL487 201

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

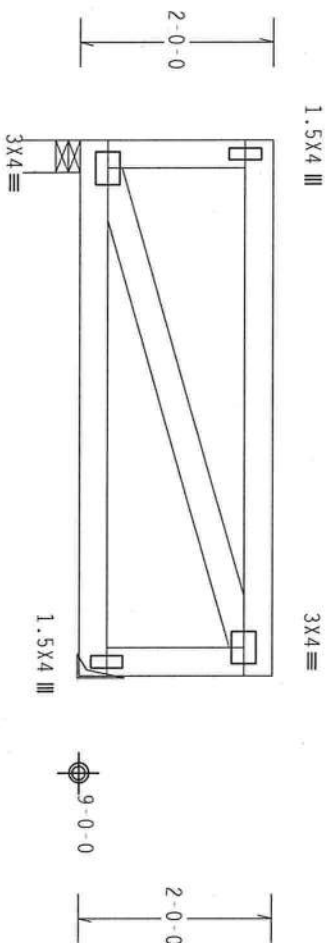


7.24.1230.16 QTY:5 FL/-/4/-/E/-/- Scale =.5"/Ft.

TC LL	40.0 PSF	REF R487 - 75093
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUR487 08030133
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	55.0 PSF	SEQN- 43737
DUR.FAC.	1.00	FROM JP
SPACING	24.0"	JREF- ITEL487 Z01

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

Truss must be installed as shown with top chord up.



5-6-0 Over 2 Supports
R=302 W=4" R=302

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

$$Cq/RT=1.00(1.25)/0(0)$$

7.24.1230.16

QTY:3

FL/-/4/-/E/-/-

Scale = .5" / Ft.



Building Components Group Inc.

ITW Building Components Group, Inc.

Haines City, FL 33844

FI Certificate of Authorization #00770

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TROUS IN COMPLIANCE WITH THE TYPE OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TROUSERS.

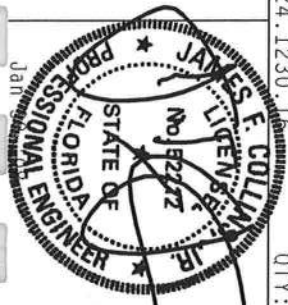
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC. BY AISC/A AND TIT. THE BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE PLATES AND THE CONNECTIONS.

CONNECTION PLATES ARE MADE OF 2010/40/600A (A572/50) ASH 6055 GRADE 40/60 (4.74/55) GALV. STEEL. APPLY THE FOLLOWING CONNECTIONS TO THE PLATES. THE CONNECTIONS SHALL BE MADE IN ACCORDANCE WITH THE AISC CONNECTIONS MANUAL, 13TH EDITION, 1989.

ANY INSPECTION OF PLATES FOLLOWED BY A BCG SHALL BE THE RESPONSIBILITY OF THE BCG. THE BCG SHALL BE RESPONSIBLE FOR THE DESIGN OF THE PLATES AND THE CONNECTIONS.

DESIGNING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TROUS COMPLEMENTATION OF THE DESIGN. THE DESIGNING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOCIETY FOR THE TROUS COMPLEMENTATION OF THE DESIGN.

THE SUFFICIENCY AND USE OF THIS COMPLIMENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

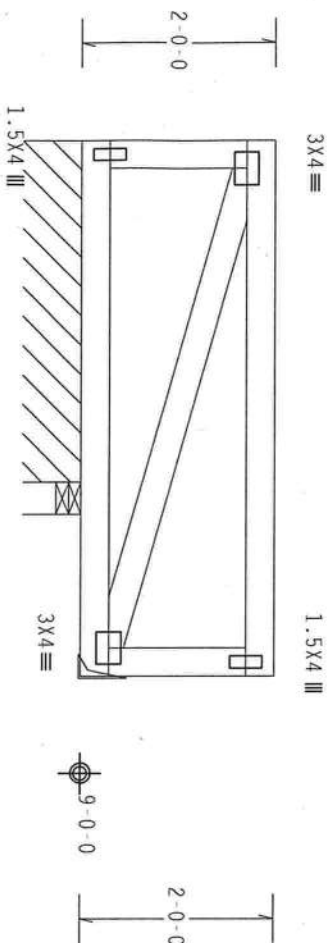


TC LL	40.0 PSF	REF	R487 - - 75094
TC DL	10.0 PSF	DATE	01/30/08
BC DL	5.0 PSF	DRW	HCUSR487 08030134
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	55.0 PSF	SEQN-	43776
DUR.FAC.	1.00	FROM	JP
SPACING	24.0"	JREF-	ITEL487_201

	Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense	
	webs	2x4	SP	#3		

Deflection meets $L/360$ live and $L/240$ total load. Creep increase factor for dead load is 1.50.

110 mph wind; 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 11, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. $I_w=1.00$ GCPI (+/-) -0.18



$\overrightarrow{\hspace{1.5cm}}$ 3-8-0 $\overleftarrow{\hspace{1.5cm}}$
 $\overrightarrow{\hspace{1.5cm}}$ 5-6-0 Over 3 Supports $\overleftarrow{\hspace{1.5cm}}$
 R=81 PLF U=3 PLF W=3-6-0 R=18 U=14 W=4"
 R=176 U=34

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)

 $Cq/RT=1.00(1.25)/0(0)$

7.24.1230

QTY:1

FL/-/4/-/E/-/-

Scale = .5"/Ft.



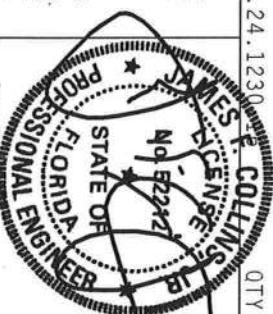
Building Components Group Inc.

ITW Building Components Group, Inc.
Haines City, FL 33844
EI Certificate of Authorization # 0 079

***WARNING** - BRICKS REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND DRAGGING REFER TO RC-31 (BUILDING COMPONENT SAFETY INFORMATION) . PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 218 BOWTIE LANE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WFLA GOOD TRUSS COUNCIL OF AMERICA, 6300 WESTLAKE AVENUE, GAITHERSBURG, MD 20878. THESE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT** * THROUGH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR, THE REG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

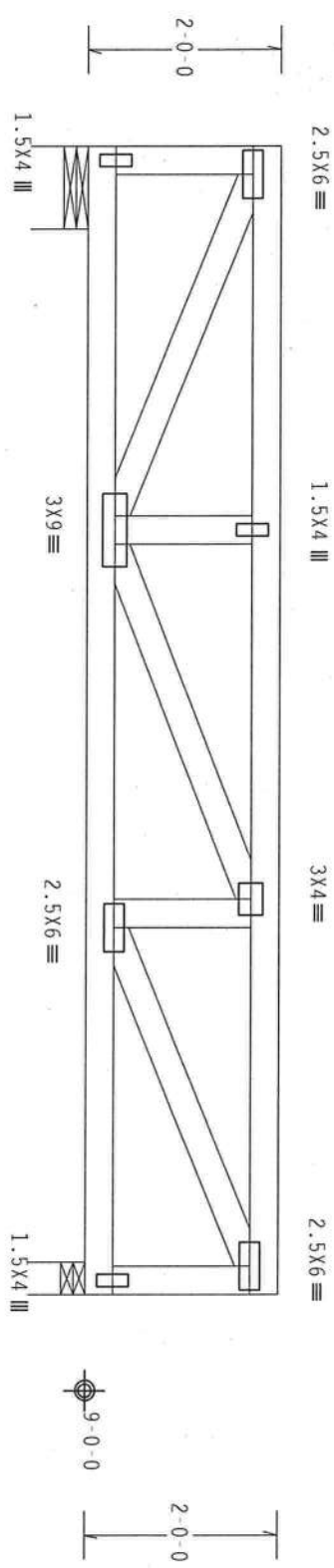
DESIGN COMPLIES WITH APPLICABLE PROVISIONS OF 905 NATIONAL DESIGN SPEC., BY AREA) AND TPI - THE REG. INC. CONNECTION PLATES ARE MADE OF 2010/16GA (K/H/S/VS) STEEL A655 GRADE 40/50 (K/H/S/VS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 1606-2. DETAILING OF PLATES FOLLOWED BY (1) SHALL BE PER AREA AS OF TPI-1-2012 SEC.3.3. A SEAL ON THIS DETAILING WILL BE REQUIRED TO VERIFY THAT THE CORRECT TYPE OF MATERIAL IS USED FOR THE TRUSS COMPONENTS. DESIGN SHOWS THE SUFFICIENCY AND USE OF ANCHORS, BOLTING, ETC. FOR THE TRUSS COMPONENTS. THE BUILDING DESIGNER PER ANSI/TPI SEC. 2,



TC LL	20.0 PSF	REF	R487 - 75095
TC DL	10.0 PSF	DATE	01/30/08
BC DL	10.0 PSF	DRW	HCUSR487 08030135
BC LL	0.0 PSF	HC-ENG	JB/AP
TOT.LD.	40.0 PSF	SEQN-	43781
DUR.FAC.	1.25	FROM	JP
SPACING	24.0"	JREF-	1TEL487_Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

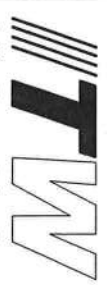
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.
Truss must be installed as shown with top chord up.



11'-10-4 Over 2 Supports
R=652 W=10.226"
R=652 W=4"

PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0)
7.24.1230
OTY:1 FL/-/4/-/E/-/ Scale =.5"/ft.

****WARNING**** BRUSSES REQUIRE EXTERIOR GALE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION) PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 216 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22304 AND WICA (WOOD TRUSS COUNCIL OF AMERICA), 6300 ENTERPRISE LANE, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

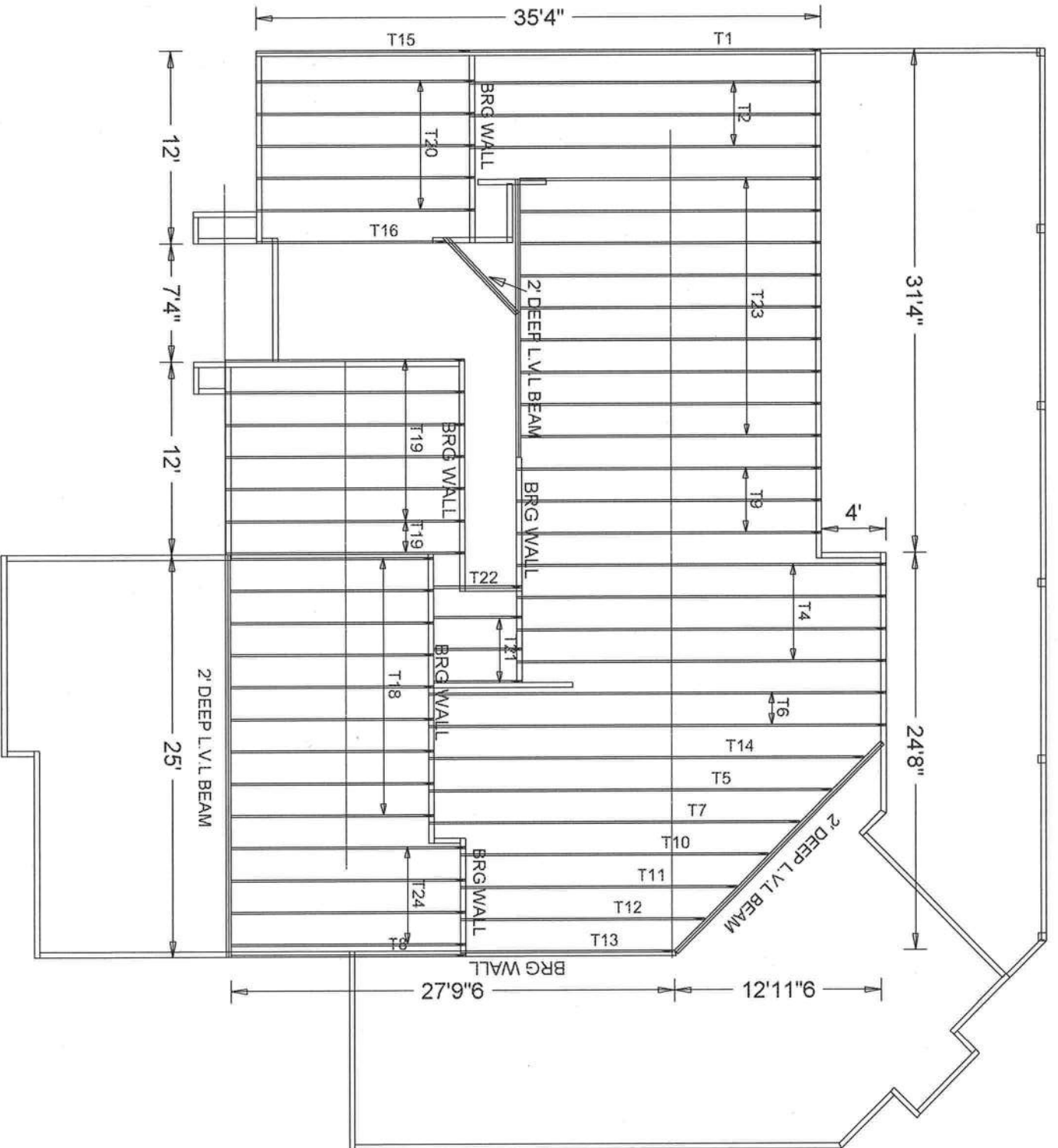


Building Components Group, Inc.
TW Building Components Group, Inc.
Haines City, FL 33844
PT Certificate of Authorization #0-378



TC LL	40.0 PSF	REF R487-- 75096
TC DL	10.0 PSF	DATE 01/30/08
BC DL	5.0 PSF	DRW HCUR487 08030136
BC LL	0.0 PSF	HC-ENG JB/AP
TOT. LD.	55.0 PSF	SEQN- 43791
DUR. FAC.	1.00	FROM JP
SPACING	24.0"	JREF- 1TEL487 201

STANLEY CRAWFORD / DEARDORF FLOOR



JOB LOCATION:

JOB DESCRIPTION:

DESIGNED BY:
Jenny Patterson

JOB NO.:
deardorf

PAGE NO.:
1 OF 1



PRODUCTION TESTING. In addition to the rigorous testing performed on prototype units during the development phase of the TrimJoist®. All production runs are subject to periodic, random tests. These tests are conducted to the standards of the Truss Plate Institute (TPI) Test Truss Specification. No other production floor truss is subjected to such ongoing scrutiny and improvement.

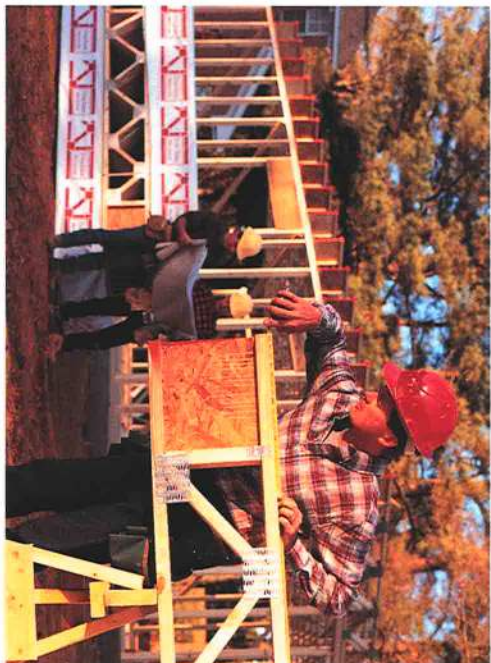
ENVIRONMENT. The TrimJoist® design allows for the utilization of lumber from plantation-grown trees, thus sparing the "old-growth" forests that are necessary to produce all traditional wide-board joists. In addition, our engineered strength allows for longer spans and greater on-center spacing. This not only reduces construction costs, but can reduce the board footage of wood fiber required by up to 12% over traditional joists.

By design, approximately 40% of the TrimJoist® material content is from recycled sources. In addition, our manufacturing process recovers over 97% of its generated waste.

INNOVATION. The TrimJoist® is produced in stock lengths that range from 4' to 30', in standard 2-foot increments. Because each joist has 12-inches of trim at each end, a continuous range of spans is possible. Complex angled walls? No problem for the TrimJoist®.

The open web design of the TrimJoist® allows for the utilization of that valuable floor cavity space. It's now practical for duct work, wiring, and plumbing to be contained within this cavity. This eliminates unsightly and expensive furring. Free access to the floor cavity requires no cutting or notching that can severely impair the structural integrity of other joists products of inferior design.

SERVICE. Put simply, the TrimJoist® is the superior product for floor joist applications. To aid architects, building designers, and contractors we offer toll-free access to a TrimJoist® representative. By simply calling (800) 844-8281 you can gain immediate assistance with your question or framing issue.



SILENCE. The TrimJoist® provides a full 3-1/2" wide surface for the proper nailing and gluing of subfloor materials. By providing such a wide surface, the TrimJoist® allows the builder to use subfloor framing tolerances that will prevent squeaks. The wide surface also allows for the application of sufficient glue to make the TrimJoist® and the subfloor truly act as a unified system.

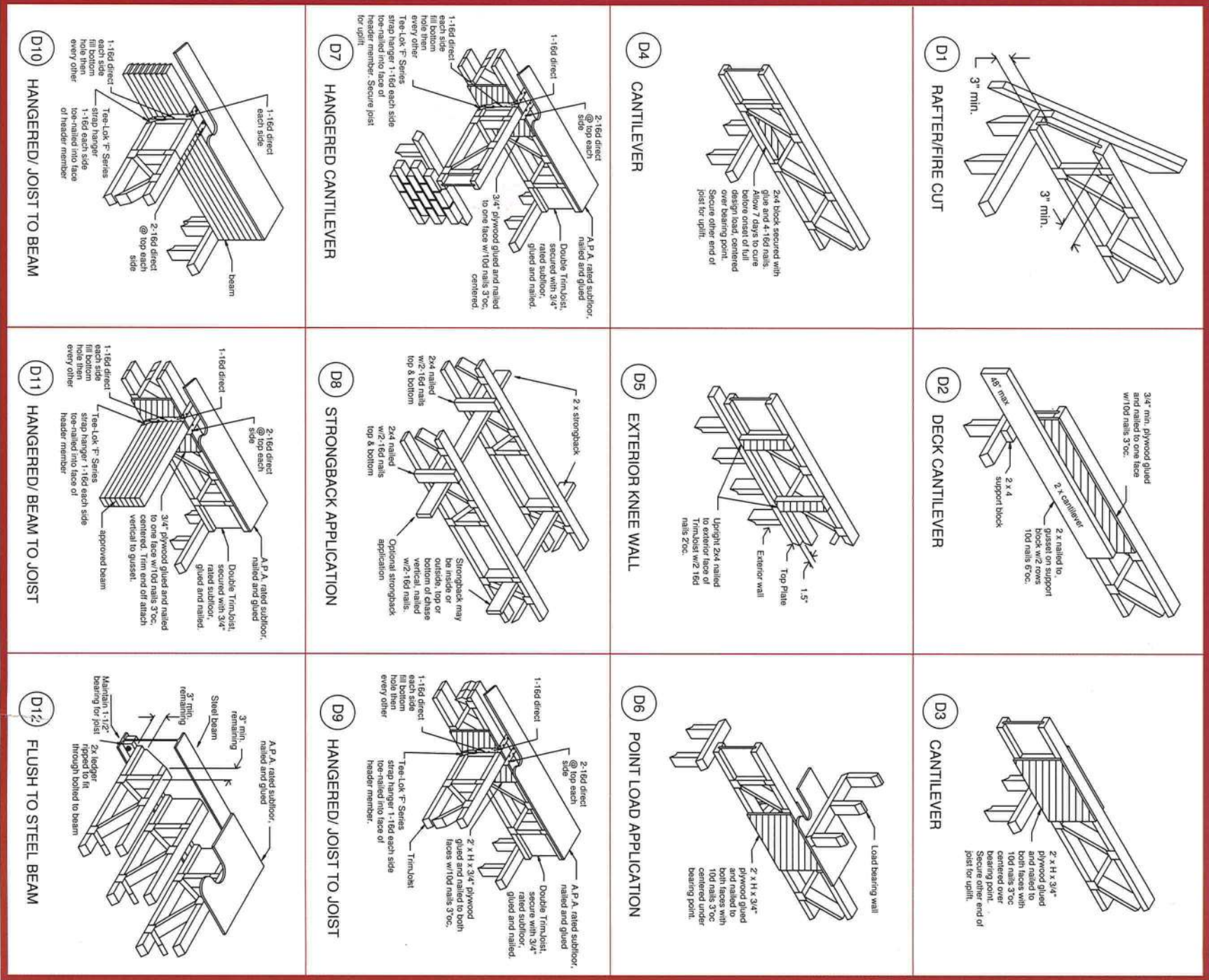
QUALITY. The TrimJoist® addresses product consistency and product quality from three key perspectives:

DESIGN. The TrimJoist® was initially designed using the most capable structural engineers and computer software systems available from both inside and outside the forest products industry. This includes the PPSA system from Purdue University as well as Rasna finite element software from Parametric Technology.

MATERIAL CONTROL. All raw material components are structurally rated by various independent agencies such as the Truss Plate Institute (TPI), Southern Pine Inspection Bureau (SPIB), American Plywood Association (APA), American Society for Testing and Materials (ASTM), and of course ANSI. In fact, all TrimJoist® floor trusses now conform to the new ANSI/TP1-1995 standard. In addition, all lumber components of the TrimJoist® are subject to a strict "cull" process where questionable pieces are discarded before entering into the production process.

WARRANTY TrimJoist® Corporation warrants its products to be free from defects in material and workmanship as generally accepted in the industry. Said products are further warranted as to adequacy of design, provided products are used in strict accordance with TrimJoist® Corporation's then currently published design limits and are installed in a workmanlike manner. Said warranties do not apply in the event products are altered in any way other than intended by design. TrimJoist® Corporation's obligations under this warranty shall be limited to the replacement or repair of those products demonstrated to be defective. Such remedy shall constitute Customers sole and exclusive remedy and customer, through the use of this product, agrees that no other remedy (including, but not limited to claims for INCIDENTAL, CONSEQUENTIAL or SPECIAL DAMAGES, OR ANY CAUSE, LOSS, ACTION, CLAIM OR DAMAGE, INCLUDING LOSS OF TIME WHATSOEVER, OR INJURY TO PERSONS OR PROPERTY OR ANY OTHER CONSEQUENTIAL DAMAGE OR INCIDENTAL OR ECONOMIC LOSS) shall be available to customer whether said claims be asserted on the basis of warranty, negligence, strict liability, or otherwise. THIS WARRANTY IS EXPRESSLY IN LIEU OF ALL OTHER WARRANTIES, EXPRESSED OR IMPLIED, INCLUDING ANY WARRANTIES OF MERCHANTABILITY AND FITNESS FOR ANY PARTICULAR PURPOSE. ALL OF SUCH OTHER WARRANTIES BEING HEREBY EXPRESSLY EXCLUDED.

Installation Details Available in CAD format at trimjoist.com and sweets.com



LOAD TABLES AND DESIGN NOTES

The uniform load span charts below indicate the maximum design spans (including a 1-1/2" minimum bearing at each end) for each family of TrimJoist® floor joists. Each chart is divided into columns which represent common design loadings and rows which show typical spacings. Most residential designs require a minimum of 55 psf loading. Floors used for heavy traffic and/or heavy floor coverings (e.g. Tile) should be designed at 60 psf minimum. All loads are broken down into Live, Top-dead and Bottom-dead components. For example, the 55 psf column is really 40 psf live plus 10 psf top-dead plus 5 psf bottom-dead for a total of 55 psf. Dead loads are the weight of construction materials and are always present for the whole life of the structure. Live loads, on the other hand, are transient and are never constant over the life of the structure. Select the appropriate column based on the dead loads of your construction materials. These charts are for uniformly loaded, clear span, simply supported joists. For special applications requiring concentrated loads, asymmetrical continuous loads, cantilevers, or special bearing conditions please consult a TrimJoist® representative or authorized dealer. The TPDS computer program can be used to analyze almost any loading and/or bearing condition.

Loading	55 PSF (40/10/5)	60 PSF (40/10/10)
12" Spacing	24'-0" L/497	24'-0" L/497
16" Spacing	22'-0" L/485	22'-0" L/485
19.2" Spacing	21'-2" L/453	21'-2" L/453
24" Spacing	19'-7" L/455	19'-7" L/455

Loading	55 PSF (40/10/5)	60 PSF (40/10/10)
12" Spacing	28'-0" L/676	28'-0" L/676
16" Spacing	28'-0" L/507	28'-0" L/507
19.2" Spacing	27'-4" L/453	27'-4" L/453
24" Spacing	25'-5" L/450	25'-5" L/450

12" Spacing	26'-0" L/633	26'-0" L/633
16" Spacing	26'-0" L/475	26'-0" L/475
19.2" Spacing	24'-10" L/453	24'-10" L/453
24" Spacing	23'-0" L/452	22'-0" L/517

12" Spacing	30'-0" L/710	30'-0" L/710
16" Spacing	30'-0" L/532	30'-0" L/532
19.2" Spacing	29'-10" L/451	29'-10" L/451
24" Spacing	27'-7" L/468	27'-3" L/473

Maximum Reaction Table

- Notes on Span Charts
- Spans are based on uniformly loaded joists and include allowances for repetitive use members.
 - Live loads of 40 psf are assumed. Additional dead loads should be chosen based on construction materials.
 - All TrimJoist® floor joists have a TOP orientation and should not be installed upside-down.
 - Stiffness factors (L/xxx) assume a minimum 3/4-inch span-rated subfloor that has been both glued and nailed.
 - Limit total reaction (per end) to that indicated in the Maximum Reaction Table at the right.
 - Do not apply center supports, cantilevers, concentrated, or asymmetrical continuous loads without first consulting a TrimJoist® representative.

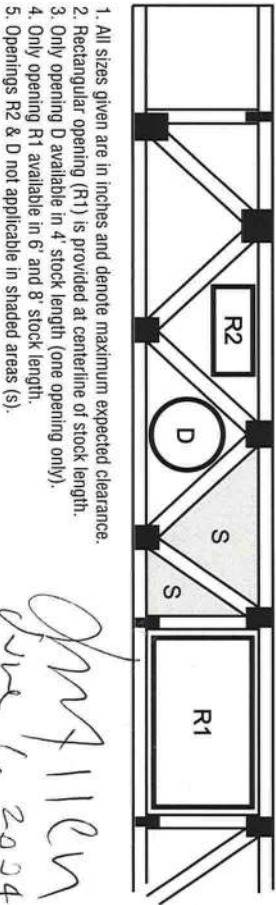
Width	1 1/2	3 1/2	5 1/2
Max	3000	3500	4000

Width is the width of the loaded wall above, or the bearing wall width whichever is less.

Note About Floor Stiffness: Floor performance is greatly influenced by joist stiffness. Experience has shown that a floor system designed to minimum code acceptance may not meet the expectations of discerning owners. TrimJoist® Corporation strongly recommends that floor spans be limited to those indicated in the charts above. The numbers in these charts far exceed minimum code requirements and are based on both gluing and nailing the subfloor. In cases where the subfloor is nailed only, spans remain the same, but the stiffness must be reduced by 20%. For optimal performance use screws in lieu of nails.

Opening Sizes

	J12	J14	J16	J18
H	11 1/4"	14"	16"	18"
D	5"	8"	9"	10"
R1	8x16	10x24	12x24	14x24
R2	4x9	4x10	4x12	4x14
		6x6	6x8	6x10
				8x8



GOOD FRAMING PRACTICE

- DO** Install TrimJoist® right side up. TOP is stamped on the top of each joist.
- DO** Make sure that each TrimJoist® bears on the bottom flange beneath the TrimEnd section or beneath the first metal plate if the TrimEnd section has been removed.
- DO** Use strongback stiffeners. Although not required for structural performance, strongback adds additional resistance to impact loadings.
- DO** Provide appropriate bearing width at each end of the TrimJoist®. The required width can be found in the Maximum Reaction Table above. Use vertical web stiffeners where reactions exceed these values.
- DO** Use TrimJoist® approved hangers for flush-mounted bearing conditions. These may be purchased from your local TrimJoist® dealer.
- DO** Use an appropriately rated sub-floor that has been both glued and nailed/screwed to the top flange of the TrimJoist®.
- DO** Consult your TrimJoist® dealer or representative about special loading or bearing conditions not addressed in this Application Guide.

DO NOT cut any part of the TrimJoist® except for the TrimEnd® sections which are specifically designed to be field cut.

DO NOT remove, cut or alter any metal plate connector on the TrimJoist® without first consulting a factory engineer.

DO NOT install the TrimJoist® upside down without first consulting a TrimJoist® factory engineer.

DO NOT use a TrimJoist® as a header or beam except as may be instructed by a TrimJoist® engineer.

DO NOT allow the TrimJoist® to be supported by the top flange. All support must be from under the bottom flange.

DO NOT depend on "ice nailing" to provide adequate support capacity for flush-mounted framing. Consult your local TrimJoist® dealer or a TrimJoist® factory engineer for proper hanger selection.

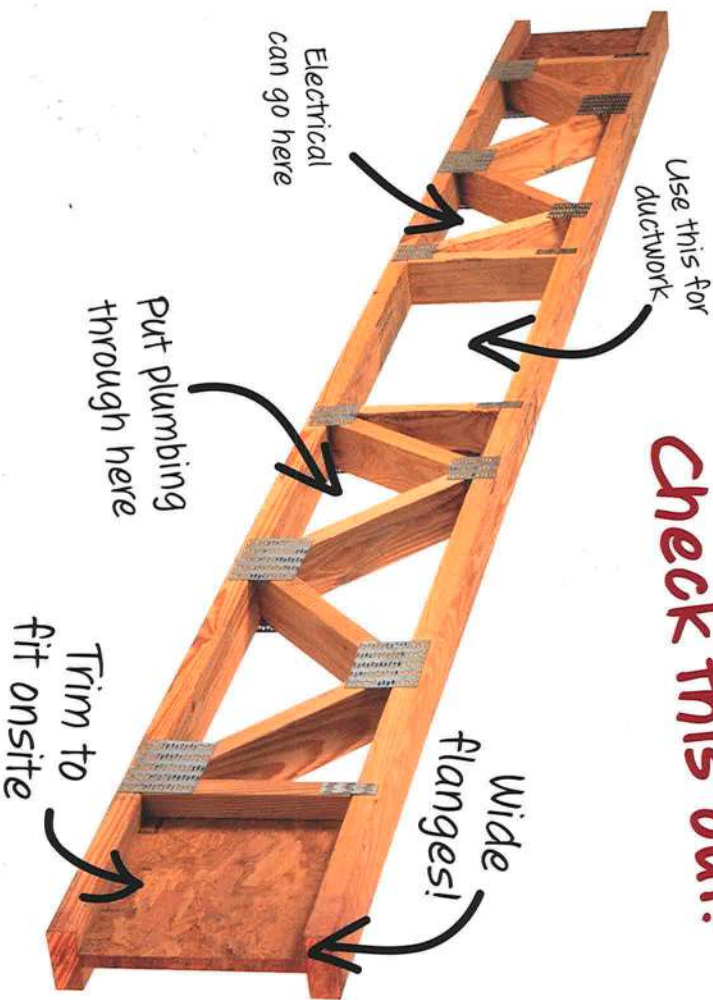
DO NOT apply special support or load conditions without first consulting a TrimJoist® representative.

TrimJoist®

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BuyLine 4916

Hey Bob!
Check this out!



Wood truss joists that can be trimmed at the jobsite

IT'S CONTRACTOR FRIENDLY.

The end sections can be trimmed onsite.

IT SAVES MONEY AND TIME.

With strut-webbing, there's no need for subcontractors to cut holes.

IT'S STRONGER.

You don't weaken the joist with holes.

IT HAS WIDE FLANGES.

With 3.5-inch flanges on the top and bottom, subfloor application is simple. Nailing and gluing are easier.

IT COMES WITH A GREAT TECHNICAL SUPPORT TEAM.

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Load Short Form Entire House

Touchstone Heating and Air, Inc.

Job: Deardorf Residence

Date: Jan 29, 2008

By: ell

P.O. Box 327, Lake Butler, FL 32054 Phone: 386-496-3467 Fax: 386-496-3147

Project Information

For: Stanley Crawford Construction
Lake City, FL 32025

Design Information

	Htg	Clg	Infiltration	
Outside db (°F)	33	92	Method	Simplified
Inside db (°F)	68	75	Construction quality	Average
Design TD (°F)	35	17	Fireplaces	0
Daily range	-	M		
Inside humidity (%)	-	50		
Moisture difference (gr/lb)	-	52		

HEATING EQUIPMENT

Make
Trade
Model

Efficiency

0 HSPF

Heating input

0 Btuh @ 47°F

Heating output

0 °F

Temperature rise

Actual air flow

2184 cfm

Air flow factor

0.048 cfm/Btuh

Static pressure

0.00 in H2O

Space thermostat

COOLING EQUIPMENT

Make
Trade

Cond
Coil

Efficiency

0 EER

Sensible cooling

0 Btuh

Latent cooling

0 Btuh

Total cooling

0 Btuh

Actual air flow

2184 cfm

Air flow factor

0.044 cfm/Btuh

Static pressure

0.00 in H2O

Load sensible heat ratio

0.69

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
BR 5	166	3960	2726	191	119
Bath	54	789	796	38	35
Study	180	4186	3047	202	133
Great Room	492	3983	2875	192	126
Foyer	108	1940	858	94	38
Dining	143	2716	2405	131	105
Pantry	54	0	0	0	0
Laundry	90	0	2842	0	124
Morn/Kitch/Keep	698	7355	11480	355	503
Office	61	1506	1422	73	62
BR 2	154	2450	2942	118	129
J & J Bath	108	669	826	32	36
BR 3	157	2140	2810	103	123
Media Rm	206	2171	2309	105	101
Gallery	190	264	521	13	23
Master BR	273	3244	3734	157	163
BR 4	152	1198	1296	58	57

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wrightsoft

Right-Sube Residential 6.0.90 RSR25972

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Page 1

Bath 2	90	437	674	21	29
Closet	39	54	106	3	5
Master Bath	160	1990	3161	96	138
WIC	210	2679	1837	129	80
Foyer 2	203	1453	1216	70	53
Entire House	3985	45183	49882	2184	2184
Other equip loads		17179	8344		
Equip. @ 0.97 RSM			56480		
Latent cooling			26708		
TOTALS	3985	62362	83187	2184	2184

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



Duct System Summary

Entire House

Touchstone Heating and Air, Inc.

Job: Deardorf Residence
Date: Jan 29, 2008
By: ell

P.O. Box 327, Lake Butler, FL 32054 Phone: 386-496-3487 Fax: 386-496-3147

Project Information

For: Stanley Crawford Construction
Lake City, FL 32025

	Heating	Cooling
External static pressure	0.00 in H2O	0.00 in H2O
Pressure losses	0.15 in H2O	0.15 in H2O
Available static pressure	-0.1 in H2O	-0.1 in H2O
Supply / return available pressure	-0.07 / -0.07 in H2O	-0.07 / -0.07 in H2O
Lowest friction rate	0.880 in/100ft	0.880 in/100ft
Actual air flow	2184 cfm	2184 cfm
Total effective length (TEL)	0 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg. Eqv Ln (ft)	Trunk
BR 5	h 3960	191	119	0.880	8	0x0	VIFx	0.0	0.0	
Bath	h 789	38	35	0.880	4	0x0	VIFx	0.0	0.0	
Study	h 4186	202	133	0.880	8	0x0	VIFx	0.0	0.0	
Great Room	h 3983	192	126	0.880	8	0x0	VIFx	0.0	0.0	
Foyer	h 1940	94	38	0.880	6	0x0	VIFx	0.0	0.0	
Dining	h 2718	131	105	0.880	7	0x0	VIFx	0.0	0.0	
Pantry	c 0	0	0	0.880	0	0x0	VIFx	0.0	0.0	
Laundry	c 2842	0	124	0.880	7	0x0	VIFx	0.0	0.0	
Mom/Kitch/Keep-A	c 3826	118	167	0.880	8	0x0	VIFx	0.0	0.0	
Mom/Kitch/Keep-B	c 3826	118	167	0.880	8	0x0	VIFx	0.0	0.0	
Mom/Kitch/Keep	c 3827	119	168	0.880	8	0x0	VIFx	0.0	0.0	
Office	h 1506	73	62	0.880	5	0x0	VIFx	0.0	0.0	
BR 2	c 2942	118	129	0.880	7	0x0	VIFx	0.0	0.0	
J & J Bath	c 826	32	36	0.880	4	0x0	VIFx	0.0	0.0	
BR 3	c 2810	103	123	0.880	7	0x0	VIFx	0.0	0.0	
Media Rm	h 2171	105	101	0.880	6	0x0	VIFx	0.0	0.0	
Gallery	c 521	13	23	0.880	4	0x0	VIFx	0.0	0.0	
Master BR	c 3734	157	163	0.880	8	0x0	VIFx	0.0	0.0	
BR 4	h 1198	58	57	0.880	5	0x0	VIFx	0.0	0.0	
Bath 2	c 674	21	29	0.880	4	0x0	VIFx	0.0	0.0	
Closest	c 106	3	5	0.880	4	0x0	VIFx	0.0	0.0	
Master Bath	c 3161	96	138	0.880	7	0x0	VIFx	0.0	0.0	
WIC	h 2679	129	80	0.880	7	0x0	VIFx	0.0	0.0	
Foyer 2	h 1453	70	53	0.880	5	0x0	VIFx	0.0	0.0	

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	2184	2184	0.0	0.880	592	26	0x 0		VIFx	



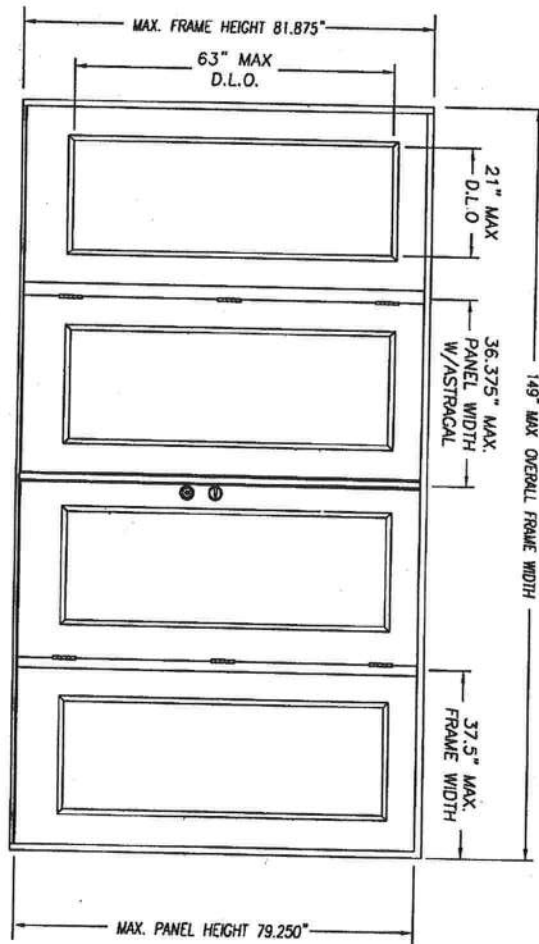
SIDE-HINGED WOOD-EDGE STEEL DOOR UNIT
6" 8" GLAZED DOUBLE DOOR WITH / WITHOUT SIDELITES

GENERAL NOTES

1. EVALUATED FOR USE IN LOCATIONS ADHERING TO THE FLORIDA BUILDING CODE AND WHERE PRESSURE REQUIREMENTS AS DETERMINED BY ASCE 7, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, DOES NOT EXCEED THE DESIGN PRESSURES LISTED
2. HURRICANE PROTECTIVE SYSTEM (SHUTTERS) IS REQUIRED
3. POLYURETHANE CORE FLAME SPREAD INDEX OF 50 AND SMOKE DEVELOPED INDEX OF 60 PER ASTM E84
4. PLASTICS TESTING OF LITE FRAME MATERIAL:

TEST DESCRIPTION	DESIGNATION	RESULT
SELF IGNITION TEMP	ASTM D1929	680 °F > 650 °F
RATE OF BURNING	ASTM D635	1.10 IN./MIN
SMOKE DENSITY	ASTM D2843	69.62
TENSILE STRENGTH*	ASTM D638	-7.48% DIFF

* COMPARATIVE TENSILE STRENGTH AFTER WEATHERING 4500 HOURS XENON ARC METHOD 1



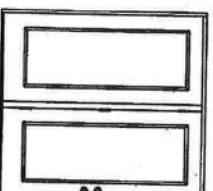
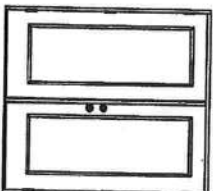
DOUBLE INSWING UNIT W/SIDELITES

As shown to NIM

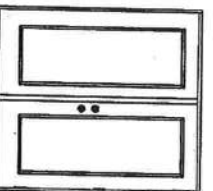
Confession by: NICOLLO
Reviewed by: 8/10/05
Date Reviewed: 8/10/05



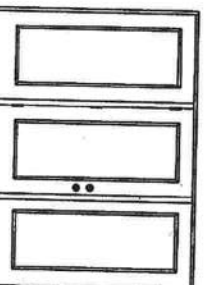
SINGLE DOOR UNIT DOUBLE DOOR UNIT



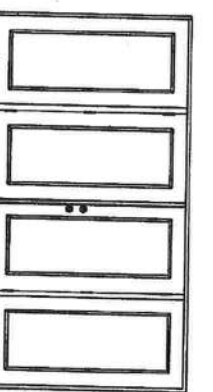
SINGLE DOOR UNIT WITH SIDELITE



SINGLE DOOR UNIT WITH SIDELITE



SINGLE DOOR UNIT W/SIDELITES



DOUBLE DOOR UNIT W/SIDELITES

TABLE OF CONTENTS

SHEET #	DESCRIPTION
1	TYPICAL ELEVATIONS & GENERAL NOTES
2	ANCHORING LOCATIONS & DETAILS
3	ANCHORING LOCATIONS & DETAILS

CONFIG	MAX WIDTH	DESIGN PRESSURE RATING	WHERE WATER INFILTRATION PERFORMANCE IS REQUIRED TO BE 15% OF DESIGN PRESSURE
X	37.5"	INSWING: +50.5 / -50.5	OUTSWING: +50.5 / -50.5
XX	74"	INSWING: +50.5 / -50.5	OUTSWING: +50.5 / -50.5
OK or XO	75"	INSWING: +50.5 / -50.5	OUTSWING: +50.5 / -50.5
OXO	112.5"	INSWING: +50.5 / -50.5	OUTSWING: +50.5 / -50.5
OXOX	149"	INSWING: +50.5 / -50.5	OUTSWING: +50.5 / -50.5

NO.	DATE	REVISIONS	BY

DATE: 7/11/05
SCALE: N.T.S.
DWG. BY: SWS
CHK. BY:
DRAWING NO.:
DWG-MA-F10130-05
SHEET 1 of 3

PRODUCT:	"EXTERIOR DOOR PRODUCT" DOUBLE 6" 8" GLAZED WOOD-EDGE STEEL DOOR
PART OR ASSEMBLY:	TYPICAL ELEVATIONS & GENERAL NOTES

MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216

MASONITE INTERNATIONAL CORP.
7300 REAMES RD.
CHARLOTTE, NC 28216



- ### TYPICAL ANCHOR INSTALLATION



- Certification No: NI0006110
 Reviewed By: [Signature]
 Date Reviewed: 8/10/05

DATE: 7/14/05 SCALE: N.T.S. DWG. BY: SWS CHK. BY: DRAWING NO.: DWG.-M4-F-01130-05 SHEET 3 OF 3				PRODUCT: "EXTERIOR DOOR PRODUCT" 6'-8" WOOD-EDGE STEEL GLAZED DOUBLE DOOR UNIT PART OR ASSEMBLY: ANCHORING LOCATIONS & DETAILS		MASONITE INTERNATIONAL CORP. 7300 REAMES RD. CHARLOTTE, NC 28216	
NO. DATE BY				REVISIONS			



SITE NAVIGATION



Home

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tionFlorida
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BuildingsPrototype
Building

Surcharges



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Commission

PRODUCT APPROVAL

Product Type Detail

Overview

Product Search

Organization
SearchProduct
Application

User: Public User - Not Associated with Organization -

[Need Help ?](#)

Application #:

FL4904

Date Submitted:

07/25/2005

Code Version:

2004

Product Manufacturer:

Masonite International

Address/Phone/email:

One North Dale Mabry
Suite 950
Tampa, FL 33609
(615) 441-4258

Category:

Exterior Doors

Subcategory:

Swinging

Evaluation Method:

Certification Mark or Listing

Referenced Standards from the Florida Building Code:

Section	Standard	Year
	TAS 201	1994
	TAS 202	1994
	TAS 203	1994
	ASTM	1998
	E1300	
	ASTM	2002
	E1300	

Section
2612 HVHZ
PI

Certification Agency:

National Accreditation & Management Institute,

Quality Assurance Entity:

Validation Entity:

Authorized Signature:

Steve Schreiber
sschreiber@masonite.com

Evaluation/Test Reports Uploaded:
 Installation Documents Uploaded:

[PTID_4904_I_Install 68 WE
Glazed.pdf](#)
[PTID_4904_I_Install 68 WE
Opaque.pdf](#)
[PTID_4904_I_Install 80 WE
Glazed.pdf](#)
[PTID_4904_I_Install 80 WE
Opaque.pdf](#)

Product Approval Method:

Method 1 Option A

Application Status:

Approved

Date Validated:

09/27/2005

Date Approved:

10/06/2005

Date Certified to the 2004 Code:

Page:

Page 1 / 1

App/Seq #	Product Model # or Name	Model Description	Limits of Use
4904.1	Wood-edge Steel Side-Hinged Door Units	6'-8" Opaque I/S and O/S Single Door	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 3'-0" x 6'-8" max nominal size Max DP = +/- 76.0. When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0128-05 for additional information.
4904.2	Wood-edge Steel Side-Hinged Door Units	8'-0" Opaque I/S and O/S Single Door	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 3'-0" x 8'-

			0" max nominal size Max DP = +/- 70.0. When large missile impact resistance is required, hurricane protective system is NOT required. See installation drawing DWG-MA-FL0129-05 for additional information.
4904.3	Wood-edge Steel Side-Hinged Door Units	6'-8" Opaque I/S and O/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 6'-8" max nominal size. Max DP = +/- 55.0. When large missile impact resistance is required, hurricane protective system is NOT required on opaque panels, but is required on glazed panels. See installation drawing DWG-MA-FL0128-05 for additional information.
4904.4	Wood-edge Steel Side-Hinged Door Units	8'-0" Opaque I/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size. Max DP = + 45.0 / -50.0. When large missile impact resistance is required, hurricane protective system is NOT required on opaque panels, but is required on glazed panels. See installation drawing DWG-MA-FL0129-05 for additional information.
			Evaluated for use in

4904.5	Wood-edge Steel Side-Hinged Door Units	8'-0" Opaque O/S w/ or w/o Sidelites	locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size. Max DP = + 50.0 / -45.0. When large missile impact resistance is required, hurricane protective system is NOT required on opaque panels, but is required on glazed panels. See installation drawing DWG-MA-FL0129-05 for additional information.
4904.6	Wood-edge Steel Side-Hinged Door Units	6'-8" Glazed I/S and O/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 6'-8" max nominal size. Max DP = +/- 50.5. When large missile impact resistance is required, hurricane protective system is required. See installation drawing DWG-MA-FL0130-05 for additional information.
4904.7	Wood-edge Steel Side-Hinged Door Units	8'-0" Glazed I/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size.

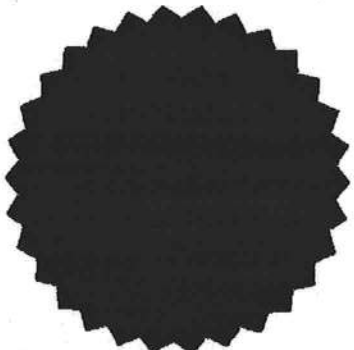
			Max DP = +40.0 / -45.0. When large missile impact resistance is required, hurricane protective system is required. See installation drawing DWG-MA-FL0131-05 for additional information.
4904.8	Wood-edge Steel Side-Hinged Door Units	8'-0" Glazed O/S Door w/ or w/o Sidelites	Evaluated for use in locations adhering to the Florida Building Code including the High Velocity Hurricane Zone, and where pressure requirements as determined by ASCE 7, Minimum Design Loads for Buildings and Other Structures, does not exceed the design pressures listed. 12'-0" x 8'-0" max nominal size. Max DP = +45.0 / -40.0. When large missile impact resistance is required, hurricane protective system is required. See installation drawing DWG-MA-FL0131-05 for additional information.

[Next](#)

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A

NAMI NOTICE OF PRODUCT LINE CERTIFICATION



Certification No.: NI006110-Page 1

Date: 07/23/05

Revision Date: _____

Certification Program: Structural

Company: Masonite International

Code: M-703-1

The "Notice of Product Line Certification" is valid only when Administrator's Seal is applied to the upper left hand portion of this form and a certification label is applied to the product. This certification seal represents product conformity to the applicable specification and that all certification criteria has been satisfied.

The products and systems listed below are approved for listing in the Directory of Certified Products at www.NAMICertification.com. Please review, and advise NAMI immediately if data, as shown requires corrections.

Company: **Masonite International Corporation**
1955 Powis Road
West Chicago, IL 60185

Product Line: **Masonite Wood-Edge Steel Side-Hinged Door Units**

Test Report: **NCTL-210-2929-1/210-2930-1/210-2930-7/210-2930-7/210-3121-1/
210-3123-1/210-3125-1/CTLA-919W**

Section 1: General Description of the Products and Systems under this Certification

- 1.1 **Frame:** The frame jambs consist of finger jointed pine with all corners coped, butted, and sealed using three 2" long wire staples (.04375").
- 1.2 **Mullion Construction:** Where used, each mullion constructed of laminated lumber with a pine cap and attached to the header and threshold with three #10 x 3" Philips Flat Head Wood Screws.
- 1.3 **Glazing:** Where used, the overall insulated glass was glazed into a rigid plastic lip-lite frame. Consisted of symmetric monolithic insulated glass with 3mm (0.118) tempered glass.
- 1.4 **Door Leaf Construction:** Each door leaf was constructed from 0.017"(6'8" height) or 0.020"(8'0" height) thick galvanized steel facings.

National Accreditation & Management Institute, Inc.
11870 Merchants Walk Suite 202-Newport News, VA 23606
TEL(757) 594.8658 FAX(757)594-8659

Section 2: Registered Suppliers

- | | | |
|------------|--------------------|----------------------------------|
| 2.1 | Door Lites: | ODL, Specialty or Trinity |
| 2.2 | Astragal: | Endura Ultimate |

Section 3: Additional Supportive Test or Acceptance Data Provided with Certification Documentation included:

- 3.1** Miami-Dade Building Code Compliance Notice of Acceptance for Lite Frame Material, NOA#02-0429.11; #02-1216.06 and #03-0303.07.
- 3.2** Surface Burning Characteristics for Foam Filled Door performed by Omega Point Laboratories to ASTM E84-98, "Standard Test Method for Surface Burning Characteristics of Building Materials-Report No. 15977-104313.
- 3.3** ASTM E1300 Glass Load Resistance Report provided by National Certified Testing Laboratories NCTL-110-9735-1.
- 3.4** Anchor Calculations for:
Anchor Performance Calculation Report-Performed by Harold E. Rupp, P.E. (Florida No. 15935.)

See additional Pages of Certification for Certified Product Line Matrix(s) and Installation Details. If you have any questions regarding this certification, please contact NAMI at (757)594-8658.

National Accreditation & Management Institute, Inc.
11870 Merchants Walk Suite 202-Newport News, VA 23606
TEL(757) 594.8658 FAX(757)594-8659

NOTICE OF PRODUCT CERTIFICATION

Company:

Masonite International Corporation
1955 Powis Road
West Chicago, IL 60185

Certification No.: NI006110-Page 3
Certification Date: 07/23/2005
Expiration Date: 12/31/2008

Product:

Wood-Edge Opaque Inswing or Outswing Door w/ and w/o Non-Impact Rated Sidelites (w/Wood Frame unless noted)
Specifications Tested To: PA 201-94/202-94/203-94

The "Notice of Product Certification" is only valid if the NAMI Certification Label has been applied to the product as described within this document. The certification label represents product conformity to the applicable specification and that all certification criteria has been satisfied. This product has been approved for listing within NAMI's Certified Product Listing at www.Namincertification.com. NAMI's Certification Program is accredited by The American National Standards Institute (ANSI).

Configuration	Inswing or Outswing	Glazed or Opaque	Maximum Size	Design Pressure Pos/Neg	Missile Impact Rated	Test Report Number Drawing Number & Comments
X Single	I/S	Opaque	3'0" x 6'8"	+76/-76	Yes	NCTL-210-2929-1 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
X Single	O/S	Opaque	3'0" x 6'8"	+76/-76	Yes	NCTL-210-2929-1 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
XX Double	I/S	Opaque	6'0" x 6'8"	+55/-55	Yes	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
XX Double	O/S	Opaque	6'0" x 6'8"	+55/-55	Yes	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
XO/OX Single w/Sidelite	I/S	Opaque Door Glazed Sidelite	6'0" x 6'8"	+55/-55	Door-Yes Sidelite-No	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
XO/OX Single w/Sidelites	O/S	Opaque Door Glazed Sidelite	6'0" x 6'8"	+55/-55	Door-Yes Sidelite-No	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
OXO Single w/Sidelites	I/S	Opaque Door Glazed Sidelites	9'0" x 6'8"	+55/-55	Door-Yes Sidelites-No	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
OXO Single w/Sidelites	O/S	Opaque Door Glazed Sidelites	9'0" x 6'8"	+55/-55	Door-Yes Sidelites-No	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
OXOX Double w/Sidelites	I/S	Opaque Doors Glazed Sidelites	12'4" x 6'8"	+55/-55	Doors-Yes Sidelites-No	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05
OXOX Double w/Sidelites	O/S	Opaque Doors Glazed Sidelites	12'4" x 6'8"	+55/-55	Doors-Yes Sidelites-No	NCTL-210-2930-1 Maximum Panel Size: 3'0" x 6'8" Sidelite: 3'0" x 6'8" Installation Drawings-MA-FL0128-05

National Accreditation & Management Institute, Inc./11870 Merchants Walk Suite 202/Newport News, VA 23606
Tel-757.594.8658/Fax-757.594.8659

NAMI AUTHORIZED SIGNATURE: 

NOTICE OF PRODUCT CERTIFICATION

Company: Masonite International Corporation
1955 Powis Road
West Chicago, IL 60185

Certification No.: NI006110-Page 4
Certification Date: 07/23/2005
Expiration Date: 12/31/2008

Product: Wood-Edge Steel Opaque Inswing or Outswing Door w/ and w/o Non-Impact Rated Sidelites (w/Wood Frame unless noted)
Specifications Tested To: PA201-94/202-94/203-94

The "Notice of Product Certification" is only valid if the NAMI Certification Label has been applied to the product as described within this document. The certification label represents product conformity to the applicable specification and that all certification criteria has been satisfied. This product has been approved for listing within NAMI's Certified Product Listing at www.Namincertification.com. NAMI's Certification Program is accredited by The American National Standards Institute (ANSI).

Configuration	Inswing or Outswing	Glazed or Opaque	Maximum Size	Design Pressure Pos/Neg	Missile Impact Rated	Test Report Number Drawing Number & Comments
X Single	I/S	Opaque	3'0" x 8'0"	+70/-70	Yes	NCTL-210-3121-1/CTLA919W Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
X Single	O/S	Opaque	3'0" x 8'0"	+70/-70	Yes	NCTL-210-3121-1/CTLA919W Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
XX Double	I/S	Opaque	6'0" x 8'0"	+45/-50	Yes	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
XX Double	O/S	Opaque	6'0" x 8'0"	+50/-45	Yes	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
XO/OX Single w/Sidelite	I/S	Opaque Door Glazed Sidelite	6'0" x 8'0"	+45/-50	Door-Yes Sidelite-No	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
XO/OX Single w/Sidelites	O/S	Opaque Door Glazed Sidelite	6'0" x 8'0"	+50/-45	Door-Yes Sidelite-No	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
OXO Single w/Sidelites	I/S	Opaque Door Glazed Sidelites	9'0" x 8'0"	+45/-50	Door-Yes Sidelites-No	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
OXO Single w/Sidelites	O/S	Opaque Door Glazed Sidelites	9'0" x 8'0"	+50/-45	Door-Yes Sidelites-No	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
OXOX Double w/Sidelites	I/S	Opaque Doors Glazed Sidelites	12'4" x 8'0"	+45/-50	Doors-Yes Sidelites-No	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05
OXOX Double w/Sidelites	O/S	Opaque Doors Glazed Sidelites	12'4" x 8'0"	+50/-45	Doors-Yes Sidelites-No	NCTL-210-3123-1 Maximum Panel Size: 3'0" x 8'0"/Sidelite: 3'0" x 8'0" Installation Drawings-MA-FL0129-05

National Accreditation & Management Institute, Inc./11870 Merchants Walk Suite 202/Newport News, VA 23606
Tel-757.594.8658/Fax-757.594.8659

NAMI AUTHORIZED SIGNATURE: 

NOTICE OF PRODUCT CERTIFICATION

Company: Masonite International Corporation
1955 Powis Road
West Chicago, IL 60185

Certification No.: NI006110-Page 5
Certification Date: 07/23/2005
Expiration Date: 12/31/2008

Product: Wood-Edge Steel Glazed Inswing or Outswing Door w/ and w/o Non-Impact Rated Sidelites (w/Wood Frame unless noted)
Specifications Tested To: PA 202-94

The "Notice of Product Certification" is only valid if the NAMI Certification Label has been applied to the product as described within this document. The certification label represents product conformity to the applicable specification and that all certification criteria has been satisfied. This product has been approved for listing within NAMI's Certified Product Listing at www.Namincertification.com. NAMI's Certification Program is accredited by The American National Standards Institute (ANSI).

Configuration	Inswing or Outswing	Glazed or Opaque	Maximum Size	Design Pressure Pos/Neg	Missile Impact Rated	Test Report Number Drawing Number & Comments
X Single	I/S	Glazed	3'0" x 6'8"	+50.5/-50.5	No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
X Single	O/S	Glazed	3'0" x 6'8"	+50.5/-50.5	No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
XX Double	I/S	Glazed	6'0" x 6'8"	+50.5/-50.5	No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
XX Double	O/S	Glazed	6'0" x 6'8"	+50.5/-50.5	No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
XO/OX Single w/Sidelite	I/S	Glazed Door Glazed Sidelite	6'0" x 6'8"	+50.5/-50.5	Door-No Sidelite-No	NCTL-210-2930-7 MA-WL0115/16/17/18/19/20/21-02 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
XO/OX Single w/Sidelites	O/S	Glazed Door Glazed Sidelite	6'0" x 6'8"	+50.5/-50.5	Door-No Sidelite-No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
OXO Single w/Sidelites	I/S	Glazed Door Glazed Sidelites	9'0" x 6'8"	+50.5/-50.5	Door-No Sidelites-No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
OXO Single w/Sidelites	O/S	Glazed Door Glazed Sidelites	9'0" x 6'8"	+50.5/-50.5	Door-No Sidelites-No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
OXOX Double w/Sidelites	I/S	Glazed Doors Glazed Sidelites	12'6" x 6'8"	+50.5/-50.5	Doors-No Sidelites-No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05
OXOX Double w/Sidelites	O/S	Glazed Doors Glazed Sidelites	12'6" x 6'8"	+50.5/-50.5	Doors-No Sidelites-No	NCTL-210-2930-7 Maximum Panel Size: 3'0" x 6'8" Installation Drawings-MA-FL0130-05

National Accreditation & Management Institute, Inc./11870 Merchants Walk Suite 202/Newport News, VA 23606

Tel-757.594.8658/Fax-757.594.8659

NAMI AUTHORIZED SIGNATURE:

NOTICE OF PRODUCT CERTIFICATION

Company: Masonite International Corporation
1955 Powis Road
West Chicago, IL 60185

Certification No.: NI006110-Page 6
Certification Date: 07/23/2005
Expiration Date: 12/31/2008

Product: Wood-Edge Steel Glazed Inswing or Outswing Door w/ and w/o Non-Impact Rated Sidelites (w/Wood Frame unless noted)
Specifications Tested To: PA 202-94

The "Notice of Product Certification" is only valid if the NAMI Certification Label has been applied to the product as described within this document. The certification label represents product conformity to the applicable specification and that all certification criteria has been satisfied. This product has been approved for listing within NAMI's Certified Product Listing at www.Namincertification.com. NAMI's Certification Program is accredited by The American National Standards Institute (ANSI).

Configuration	Inswing or Outswing	Glazed or Opaque	Maximum Size	Design Pressure Pos/Neg	Missile Impact Rated	Test Report Number Drawing Number & Comments
X Single	I/S	Glazed	3'0" x 8'0"	+40/-45	No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
X Single	O/S	Glazed	3'0" x 8'0"	+45/-40	No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
XX Double	I/S	Glazed	6'0" x 8'0"	+40/-45	No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
XX Double	O/S	Glazed	6'0" x 8'0"	+45/-40	No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
XO/OX Single w/Sidelite	I/S	Glazed Door Glazed Sidelite	6'0" x 8'0"	+40/-45	Door-No Sidelite-No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
XO/OX Single w/Sidelites	O/S	Glazed Door Glazed Sidelite	6'0" x 8'0"	+45/-40	Door-No Sidelite-No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
OXO Single w/Sidelites	I/S	Glazed Door Glazed Sidelites	9'0" x 8'0"	+40/-45	Door-No Sidelites-No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
OXO Single w/Sidelites	O/S	Glazed Door Glazed Sidelites	9'0" x 8'0"	+45/-40	Door-No Sidelites-No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
OXOX Double w/Sidelites	I/S	Glazed Doors Glazed Sidelites	12'6" x 8'0"	+40/-45	Doors-No Sidelites-No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05
OXOX Double w/Sidelites	O/S	Glazed Doors Glazed Sidelites	12'6" x 8'0"	+45/-40	Doors-No Sidelites-No	NCTL-210-3125-1 Maximum Panel Size: 3'0" x 8'0" Installation Drawings-MA-FL0131-05

National Accreditation & Management Institute, Inc./11870 Merchants Walk Suite 202/Newport News, VA 23606
Tel-757.594.8658/Fax-757.594.8659

NAMI AUTHORIZED SIGNATURE:



**AAMA/WDMA 101/LS. 2-97
TEST REPORT**

Rendered to:

JORDAN COMPANIES

**SERIES/MODEL: 8500
TYPE: PVC Single Hung Window**

Title of Test	Results
AAMA/WDMA Rating	H-R40 (44 x 84)
Uniform Load Deflection Test Pressure	± 40.0 psf
Operating Force	10 lbs max.
Air Infiltration	0.21 cfm/ft ²
Water Resistance Test Pressure	6.00 psf
Uniform Load Structural Test Pressure	± 60.0 psf
Deglazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to full report for test specimen description and data.

Report No: 02-48976.02
Report Date: 02-26-04
Expiration Date: 02-25-08



Architectural Testing

AAMA/WDMA 101/LS-2-97 TEST REPORT

Rendered to:

JORDAN COMPANIES
P.O. Box 18377
Memphis, Tennessee 38118

Report No: 02-48976.02
Test Date: 02/25/04
Report Date: 02/26/04
Expiration Date: 02/25/08

Project Summary: Architectural Testing, Inc. (ATI) was contracted by Jordan Companies to perform tests on a Jordan Companies Series 8500 Single Hung Window. The sample tested successfully met the performance requirements for a H-R40 44 x 84 rating. Test specimen description and results are reported herein.

Test Procedure: The test specimen was evaluated in accordance with AAMA/NWDMA 101/LS-2-97, "*Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.*"

Test Specimen Description:

Series/Model: 8500

Type: PVC Single Hung Window

Overall Size: 3' 8" wide by 7' 0" high

Sash Size: 3' 4-3/8" wide by 2' 5" high

Fixed D.L.O. Size: 3' 4-3/4" wide by 4' 5" high

Screen Size: 3' 4-3/4" wide by 2' 4-1/4" high

Finish: All PVC was white

Test Specimen Description: (Continued)

Glazing Type: The window utilized nominal 3/4" insulating glass comprised of two single-strength annealed sheets in the operating sash and two double-strength sheets in the fixed lite and a desiccant-filled metal spacer system. The glass for the fixed area was set from the interior into a bed of silicone sealant with PVC stops used on the interior. The sash was glazed from the exterior into a bed of silicone sealant with PVC stops used on the exterior.

Weatherstripping:

<u>Description</u>	<u>Quantity</u>	<u>Location</u>
0.260" high by 0.187" backed pile with center fin	1 Row	Sash top and bottom rails
0.260" high by 0.187" backed pile with center fin	2 Rows	Sash stiles

Frame Construction: Frame corners were miter-cut and welded. Aluminum reinforcement was utilized in the fixed meeting rail (Jordan part number H-2447).

Sash Construction: Sash corners were miter-cut and welded. Aluminum reinforcement was utilized in the top rail (Jordan part number H-2448).

Hardware:

Metal cam locks with keepers	2	6" from ends and meeting rail
Plastic tilt latches	2	Sash top rail corners
Metal tilt pins	2	Sash bottom rail corners
Block-and-tackle balances	2	One per jamb

Drainage:

3/16" by 5/8" slots	2	1-3/4" from ends in sill pocket to hollow below
1/8" by 1/2" slots	4	1-3/4" and 2" from each end through sill exterior face

Installation: The unit was installed into a Grade 2 SPF 2" by 8" wood test buck secured through the flange with 1-5/8" screws spaced 4" from corners and 8" on center. The nail fin was sealed to the buck with silicone.

Test Results: The results are tabulated as follows.

<u>Paragraph</u>	<u>Title of Test</u>	<u>Results</u>	<u>Allowed</u>
2.2.1.6.1	Operating Force		
	Force to initiate motion	10 lbs	30 lbs max.
	Force to keep in motion	8 lbs	30 lbs max.
2.1.2	Air Infiltration per ASTM E 283-97 (See Note #1) @ 1.57 psf (25 mph)	0.21 cfm/ft ²	0.30 cfm/ft ²
<i>Note #1: The tested specimen meets the performance levels specified in AAMA/WDMA 101/1.S.2-97 for air infiltration.</i>			
2.1.3	Water Resistance per ASTM 547-97 (See Note #2)		
2.1.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #2)		
2.1.4.2	Uniform Load Structural per ASTM E 330-97 (See Note #2)		
<i>Note #2: The client opted to start at a pressure higher than the minimum required. Those results are listed under "Optional Performance."</i>			
2.2.1.6.2	Deglazing Test per ASTM E 987		
	In operating direction @ 70 lbs		
	Top rail	0.04"/8%	0.500"/100%
	Bottom rail	0.06"/12%	0.500"/100%
	In remaining direction @ 50 lbs		
	Left stile	0.04"/8%	0.500"/100%
	Right stile	0.03"/6%	0.500"/100%
2.1.7	Corner Weld Test	Meets as stated	Meets as stated
2.1.8	Forced Entry Resistance per ASTM F 588-97		
	Type A		
	Grade 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A7	No entry	No entry
	Lock Manipulation Test	No entry	No entry

Test Results: (Continued)

<u>Paragraph</u>	<u>Title of Test</u>	<u>Results</u>	<u>Allowed</u>
<u>Optional Performance:</u>			
4.3	Water Resistance per ASTM E 547-97 WTP = 6.00 psf	No leakage	No leakage
4.4.1	Uniform Load Deflection per ASTM E 330-97 (See Note #3) (Measurements reported were taken on the meeting rail) (Loads were held for 60 seconds)		
	@ 40.0 psf (positive)	0.45"	(See Note #3)
	@ 40.0 psf (negative)	0.52"	(See Note #3)
4.4.2	Uniform Load Structural per ASTM E 330-97 (Measurements reported were taken on the meeting rail) (Loads were held for 10 seconds)		
	@ 60.0 psf (positive)	0.03"	0.16" max.
	@ 60.0 psf (negative)	0.03"	0.16" max.

Note #3: The Uniform Load Deflection test is not a AAMA/NWWDA 101/I.S. 2-97 requirement for this product designation. The data is recorded in this report for information only.

Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the designated test methods and they indicate compliance with the performance requirements of the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator. This report may not be reproduced except in full without the approval of Architectural Testing, Inc.

For ARCHITECTURAL TESTING, INC.

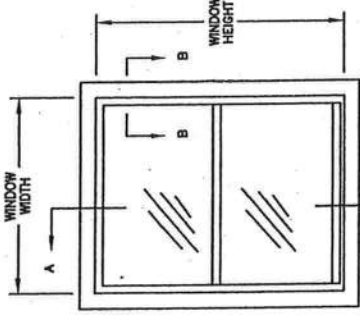
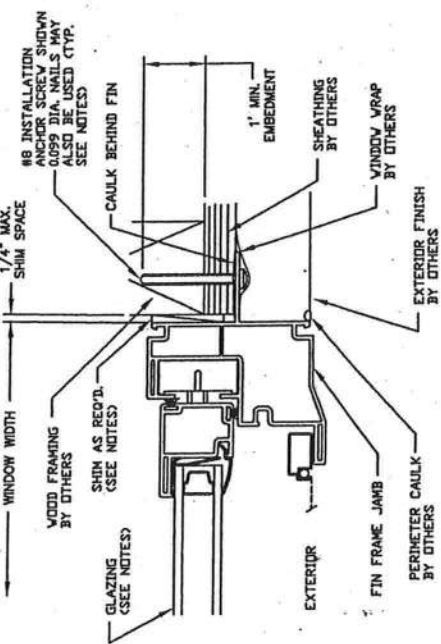

Digitally Signed by: Paul L. Spiess

Paul L. Spiess
Project Manager

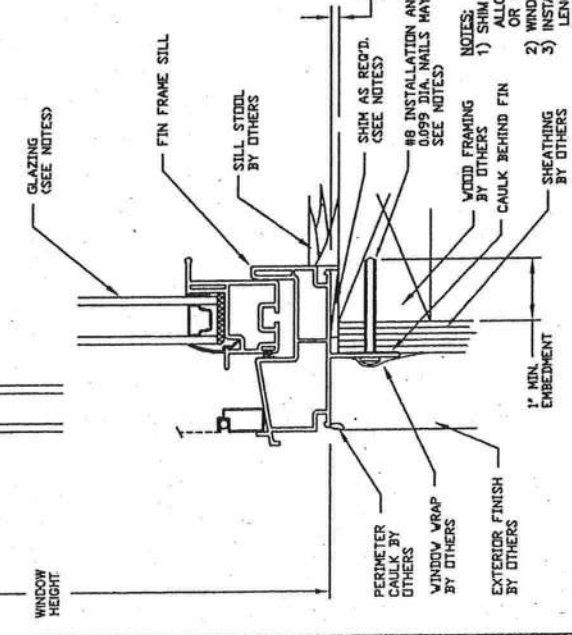

Digitally Signed by: Daniel A. Johnson

Daniel A. Johnson
Regional Manager

#185054 13783 DO 12853 SLDR 13843



ELEVATION
VIEWED FROM EXTERIOR



SECTION A-A

- NOTES:
- 1) SHIM AS REQ'D. AT EACH INSTALLATION ANCHOR WITH LOAD BEARING SHIM. MAX. ALLOWABLE SHIM STACK TO BE 1/4". APPLY SHIM WHERE SPACE OF 1/16" OR GREATER IS PRESENT.
 - 2) WINDOW FRAME MATERIAL: POLYVINYL CHLORIDE
 - 3) INSTALLATION ANCHORS, #8 SCREW & .089 DIA. NAILS MUST BE OF SUFFICIENT LENGTH TO ACHIEVE MIN. EMBEDMENT OF 1" INTO WOOD FRAME. USE INSTALLATION ANCHOR CHART FOR NUMBER OF ANCHORS REQ'D.
 - 4) IF EXACT WINDOW SIZE IS NOT LISTED USE NEXT LARGER SIZE IN THE INSTALLATION CHART FOR NUMBER OF ANCHORS REQ'D.
 - 5) USE CAULK FOR PERIMETER SEAL AROUND EXTERIOR OF WINDOW FRAME.
 - 6) GLASS THICKNESS MAY VARY PER REQUIREMENTS OF ASTM E1300.
 - 7) GLASS THICKNESS MAY VARY PER REQUIREMENTS OF ASTM E1300.
 - 8) INSTALLATION AS STATED HEREIN COMPLIES WITH THE FLORIDA BUILDING CODE AT THE DESIGN PRESSURES INDICATED ON THE COMPARATIVE ANALYSIS & ANCHOR CHART.

COMPARATIVE ANALYSIS & ANCHOR CHART									
WINDOW SIZE (INCHES)	WIDTH/HEIGHT	DESIGN PRESSURE CAPACITY IN PSF		NO. OF #8 SCREWS REQ'D. IN FRAME		NO. OF .089 DIA. NAILS REQ'D. IN FRAME		NO. OF .089 DIA. NAILS REQ'D. IN FRAME	
		EXTERIOR	INTERIOR	HEAD & SILL	8 EA JAMB	HEAD & SILL	8 EA JAMB	HEAD & SILL	8 EA JAMB
23-5/8 X 35-5/8	23-5/8 X 35-5/8	40	120	2	3	3	3	3	5
23-5/8 X 43-5/8	23-5/8 X 43-5/8	40	108	2	3	3	3	3	6
23-5/8 X 47-5/8	23-5/8 X 47-5/8	40	96	2	3	3	3	3	6
23-5/8 X 51-5/8	23-5/8 X 51-5/8	40	91	2	3	3	3	3	7
23-5/8 X 55-5/8	23-5/8 X 55-5/8	40	80	2	3	3	3	3	7
23-5/8 X 59-5/8	23-5/8 X 59-5/8	40	66	2	3	3	3	3	7
23-5/8 X 63-5/8	23-5/8 X 63-5/8	40	56	2	3	3	3	3	7
27-5/8 X 35-5/8	27-5/8 X 35-5/8	40	99	2	3	3	3	3	5
27-5/8 X 43-5/8	27-5/8 X 43-5/8	40	91	2	3	3	3	3	6
27-5/8 X 47-5/8	27-5/8 X 47-5/8	40	86	2	3	3	3	3	6
27-5/8 X 51-5/8	27-5/8 X 51-5/8	40	81	2	3	3	3	3	6
27-5/8 X 55-5/8	27-5/8 X 55-5/8	40	71	2	3	3	3	3	7
27-5/8 X 59-5/8	27-5/8 X 59-5/8	40	63	2	3	3	3	3	7
27-5/8 X 63-5/8	27-5/8 X 63-5/8	40	53	2	3	3	3	3	7
29-5/8 X 35-5/8	29-5/8 X 35-5/8	40	90	2	3	3	3	3	4
29-5/8 X 43-5/8	29-5/8 X 43-5/8	40	85	2	3	3	3	3	4
29-5/8 X 47-5/8	29-5/8 X 47-5/8	40	81	2	3	3	3	3	6
29-5/8 X 51-5/8	29-5/8 X 51-5/8	40	75	2	3	3	3	3	6
29-5/8 X 55-5/8	29-5/8 X 55-5/8	40	67	2	3	3	3	3	7
29-5/8 X 59-5/8	29-5/8 X 59-5/8	40	58	2	3	3	3	3	7
29-5/8 X 63-5/8	29-5/8 X 63-5/8	40	50	2	3	3	3	3	7
31-5/8 X 35-5/8	31-5/8 X 35-5/8	40	83	2	3	3	3	3	4
31-5/8 X 43-5/8	31-5/8 X 43-5/8	40	78	2	3	3	3	3	6
31-5/8 X 47-5/8	31-5/8 X 47-5/8	40	73	2	3	3	3	3	6
31-5/8 X 51-5/8	31-5/8 X 51-5/8	40	71	2	3	3	3	3	7
31-5/8 X 55-5/8	31-5/8 X 55-5/8	40	63	2	3	3	3	3	7
31-5/8 X 59-5/8	31-5/8 X 59-5/8	40	53	2	3	3	3	3	7
31-5/8 X 63-5/8	31-5/8 X 63-5/8	40	47	2	3	3	3	3	7
35-5/8 X 35-5/8	35-5/8 X 35-5/8	40	70	3	3	3	3	4	4
35-5/8 X 43-5/8	35-5/8 X 43-5/8	40	69	3	3	3	3	4	5
35-5/8 X 47-5/8	35-5/8 X 47-5/8	40	67	3	3	3	3	4	6
35-5/8 X 51-5/8	35-5/8 X 51-5/8	40	61	3	3	3	3	4	6
35-5/8 X 55-5/8	35-5/8 X 55-5/8	40	48	3	3	3	3	4	6
35-5/8 X 59-5/8	35-5/8 X 59-5/8	40	46	3	3	3	3	4	7
35-5/8 X 63-5/8	35-5/8 X 63-5/8	40	42	3	3	3	3	4	7
39-5/8 X 35-5/8	39-5/8 X 35-5/8	40	61	3	3	3	3	4	3
39-5/8 X 43-5/8	39-5/8 X 43-5/8	40	60	3	3	3	3	4	4
39-5/8 X 47-5/8	39-5/8 X 47-5/8	40	58	3	3	3	3	4	5
39-5/8 X 51-5/8	39-5/8 X 51-5/8	40	56	3	3	3	3	4	5
39-5/8 X 55-5/8	39-5/8 X 55-5/8	40	51	3	3	3	3	4	6
39-5/8 X 59-5/8	39-5/8 X 59-5/8	40	43	3	3	3	3	4	7
39-5/8 X 63-5/8	39-5/8 X 63-5/8	40	40	3	3	3	3	4	7
43-5/8 X 35-5/8	43-5/8 X 35-5/8	40	53	3	3	3	3	4	3
43-5/8 X 43-5/8	43-5/8 X 43-5/8	40	52	3	3	3	3	4	4
43-5/8 X 47-5/8	43-5/8 X 47-5/8	40	49	3	3	3	3	4	5
43-5/8 X 51-5/8	43-5/8 X 51-5/8	40	48	3	3	3	3	4	5
43-5/8 X 55-5/8	43-5/8 X 55-5/8	40	45	3	3	3	3	4	6
43-5/8 X 59-5/8	43-5/8 X 59-5/8	40	40	3	3	3	3	4	7
43-5/8 X 63-5/8	43-5/8 X 63-5/8	40	40	3	3	3	3	4	7
44 X 35-5/8	44 X 35-5/8	48	52	3	3	3	3	4	3
44 X 43-5/8	44 X 43-5/8	40	51	3	3	3	3	4	4
44 X 47-5/8	44 X 47-5/8	40	50	3	3	3	3	4	5
44 X 51-5/8	44 X 51-5/8	40	46	3	3	3	3	4	5
44 X 55-5/8	44 X 55-5/8	40	44	3	3	3	3	4	6
44 X 59-5/8	44 X 59-5/8	40	40	3	3	3	3	4	7
44 X 63-5/8	44 X 63-5/8	40	40	3	3	3	3	4	7

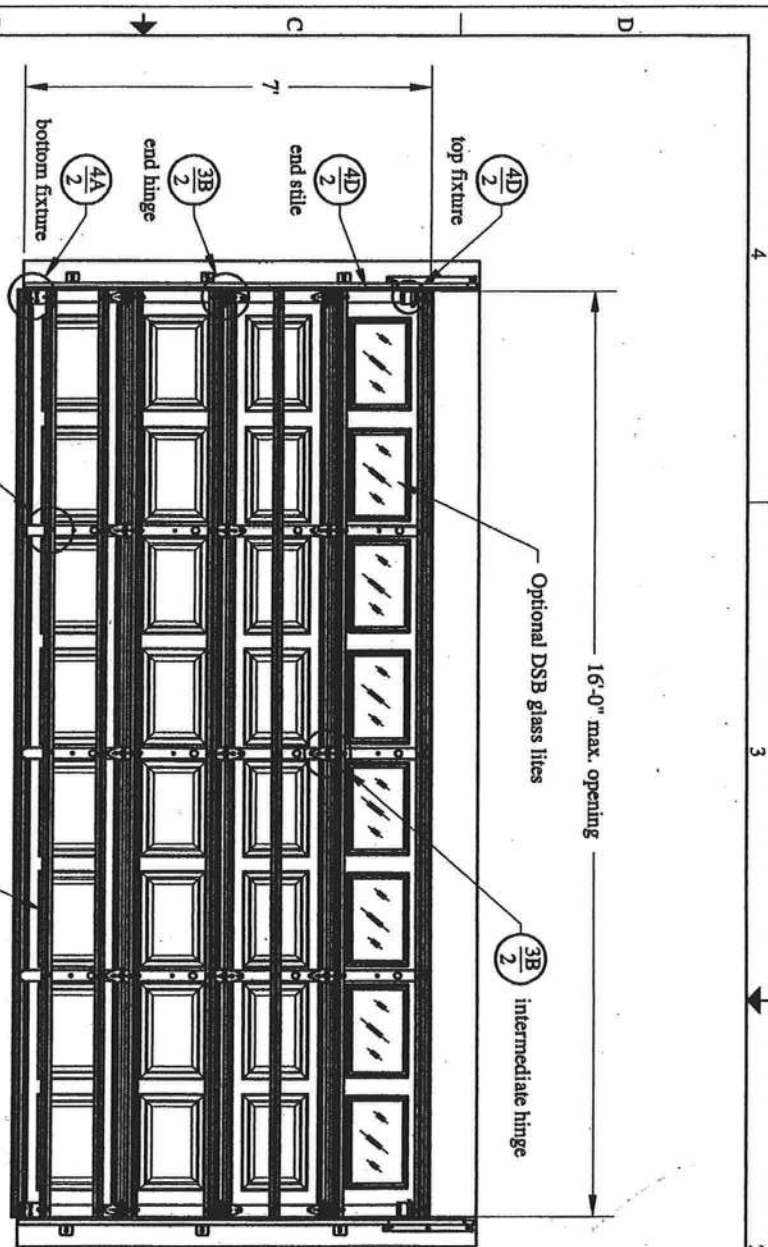
JORDAN

P.O. BOX 18377
MIAMI, FLORIDA 33188
PHONE (305) 253-2521

DESCRIPTION: SERIES 8500 SINGLE HUNG
INSTALLATION DETAIL

DATE: 6/21/02 FILE NAME: JRDN0025 DRAWN BY: BB

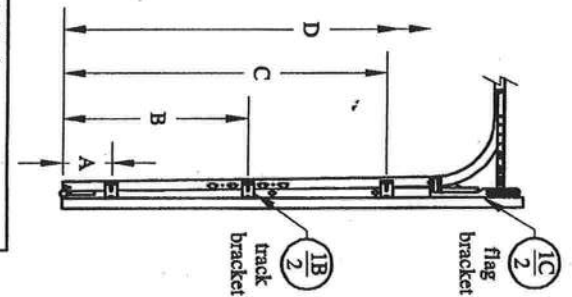
REV	DESCRIPTION	DATE
A	REVISED DP CAPACITIES & ANCHOR QNT'YS.	08-03-02



Door Model	Gauge	Decimal
2250/2251	25	.0185
4250/4251	25	.0185
2240/2241	24	.0225
4240/4241	24	.0225
5240/5241	24	.0225

door height	section quantity	strut quantity	trk bkt per side
6'-6" to 7'-0"	4	7	3
7'-6" to 8'-0"	5	8	4
8'-3" to 8'-9"	5	9	4
9'-0" to 10'-6"	6	11	5
10'-9" to 12'-3"	7	13	6
12'-6" to 14'-0"	8	15	7

Refer to Supplemental Instructions for strut placement on doors over 7'-0" high



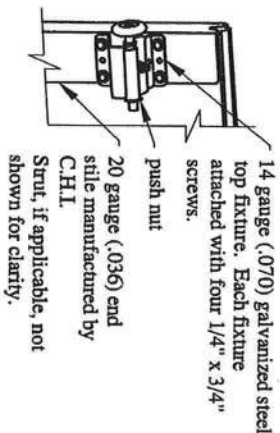
Track Bracket Chart	door height									
	6'-6"	6'-9"	7'-0"	7'-6"	7'-9"	8'-0"	8'-3"	8'-6"	8'-9"	
track brackets	D	n/a	n/a	n/a	72"	69"	72"	81"	84"	87"
	C	60"	63"	66"	58"	55"	58"	60"	63"	66"
	B	35"	35"	38"	34"	31"	34"	32"	35"	38"
	A	10"	7"	10"	10"	7"	10"	4"	7"	10"

Track bracket locations shown above are for doors up to five sections high. Additional door sections may be added for a maximum door height of 14'-0". One track bracket (per track) must be added for each section and spaced at a distance not greater than the corresponding section height.

This door has been tested in accordance with ANSI/DASMA 108-2002
 Design Pressure (DP): 18.5 pos / 20.7 neg
 Test Pressure (TP): 27.8 pos / 31.1 neg
 Per 2004 FBC Table 1609.6E, DP meets or exceeds basic wind speed of:
 V = 110 MPH for Exposure B and mean roof height of 30' or less
 V = 93 MPH for Exposure C and mean roof height of 30' or less
 Maximum door size: 16'-0" wide by 14'-0" tall
 Glazing and door have not been tested for windborne debris.
 Wood buck and supporting structural elements shall be designed by a registered professional engineer for wind loads shown on this drawing.
 If door is not electrically operated, a lock must be installed.

Professional Engineer's seal provided only for verification of windload construction details

John E. Scates, P.E.
 1411 LeMay Street #205
 Carrollton, Texas 75007
 Florida P.E. # 51737



The 2x6 vertical wood jambs are to be grade 2 or better southern pine. Fasteners may be countersunk to provide a flush mounting surface.

2" x 7/16" (nominal) Stop molding required (not supplied by C.H.I.)

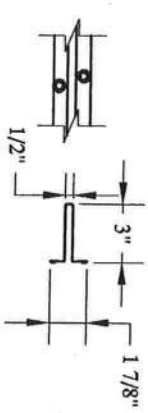
20 gauge (.036) center stile manufactured by C.H.I.
2" steel roller

2" x .051 min. galvanized steel track fastened to track brackets. Each track bracket attached with one 1/4" x 5/8" track bolt and nut.

12 gauge (.095) galvanized steel track bracket fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.

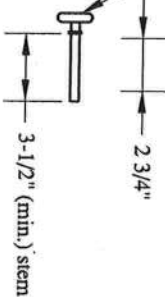
End Hinge
16 gauge (.058) galvanized steel end hinge fastened to section with four 1/4" x 3/4" screws.
push nut
2 3/4"

Intermediate Hinge
18 gauge (.047) galvanized steel intermediate hinge fastened to section with four 1/4" x 3/4" screws.
2 3/4"



12 gauge (.102) galvanized steel bottom bracket manufactured by C.H.I. Each bracket attached with four red 1/4" x 3/4" screws.
push nut
Aluminum extrusion
Vinyl weatherstrip

2" steel track roller.



12 gauge (.086) galvanized steel flag bracket fastened to wood jamb with three 5/16" x 1-5/8" wood lag screws.

Flag bracket attached to horizontal track with two 1/4" x 5/8" track bolts and nuts.

Flag bracket attached to vertical track with two 1/4" x 5/8" track bolts and nuts.

12 gauge (.095) galvanized steel track bracket fastened to wood jamb with one 5/16" x 1-5/8" wood lag screw per bracket.

Each track bracket attached with one 1/4" x 5/8" track bolt and nut. Or two 1/4" x 11/32" rivets.

Details on some views may have been omitted for clarity.

Design Load: 18.5 pos / 20.7 neg
Test Load: 27.8 pos / 31.1 neg
page 2 of 2

Professional Engineer's seal provided only for verification of windload construction details

John E. Scates, P.E.
1411 LeMay Street #205
Carrollton, Texas 75007
Florida P.E. # 51737

Model 2250/51 (16-0" wide)
C.H.I. Drawing: Z3-1607-01100

FLORIDA DEPARTMENT OF Community Affairs



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Product Approval

USER: Public User

[Product Approval Menu](#) > [Product or Application Search](#) > [Application List](#)

Search Criteria

Code Version	2004	FL#	ALL
Application Type	ALL	Product Manufacturer	Elk Corpor.
Category	Roofing	Subcategory	ALL
Application Status	ALL	Compliance Method	ALL

Search Results - Applications

FL#	Type	Manufacturer	Validated By
FL586-R2 History	Revision	Elk Corporation Category: Roofing Subcategory: Asphalt Shingles	
FL728-R1 History	Revision	Elk Corporation Category: Roofing Subcategory: Asphalt Shingles	
FL1476-R2 History	Revision	Elk Corporation Category: Roofing Subcategory: Asphalt Shingles	
FL2143-R2 History	Revision	Elk Corporation Category: Roofing Subcategory: Asphalt Shingles	
FL3453-R1 History	Revision	Elk Corporation Category: Roofing Subcategory: Underlayments	
FL3461-R2 History	Revision	Elk Corporation Category: Roofing Subcategory: Underlayments	PRI Asphalt Technologies, Inc (813) 621-5777
FL5178	New	Elk Corporation Category: Roofing Subcategory: Other	
FL5511-R1 History	Revision	Elk Corporation Category: Roofing Subcategory: Underlayments	
FL5524	New	Elk Corporation Category: Roofing Subcategory: Asphalt Shingles	
FL5683	New	Elk Corporation Category: Roofing Subcategory: Asphalt Shingles	
FL5783	New	Elk Corporation Category: Roofing	PRI Asphalt Technologies, Inc (813) 621-5777



ELK

A Division of Lake City Industries


**PRESTIQUE®
HIGH DEFINITION®**

RAISED PROFILE®
**Prestique Plus High Definition
and Prestique Gallery Collection™**

Product size 13K"x 39K"
Exposure 5K"
Pieces/Bundle 18
Bundles/Square 4/98.5 sq.ft.
Squares/Pallet 11

50-year limited warranty period:
5-7**years non-prorated coverage for
shingles and application labor with
prorated coverage for remainder of
limited warranty period, plus an
option for transferability*. 5-year
limited wind warranty*. Wind
Coverage: standard 80 mph, extended
110 mph***

Raised Profile

Product size 13K"x 39K"
Exposure 5K"
Pieces/Bundle 22
Bundles/Square 2/100 sq.ft.
Squares/Pallet 18

20-year limited warranty period:
5-7**years non-prorated coverage for
shingles and application labor with
prorated coverage for remainder of
limited warranty period, plus an
option for transferability*. 5-year
limited wind warranty*. Wind
Coverage: standard 70 mph.

Prestique I High Definition

Product size 13K"x 39K"
Exposure 5K"
Pieces/Bundle 18
Bundles/Square 4/98.5 sq.ft.
Squares/Pallet 14

40-year limited warranty period:
5-7**years non-prorated coverage for
shingles and application labor with
prorated coverage for remainder of
limited warranty period, plus an
option for transferability*. 5-year
limited wind warranty*. Wind
Coverage: standard 80 mph, extended
90 mph***

HIP AND RIDGE SHINGLES
Seal-A-Ridge™ w/FLX™

Size: 12"x 12"
Exposure: 6K"
Pieces/Bundle: 45
Coverage: 4 Bundles =
100 linear feet

Vented RidgeCrest™ w/FLX™

Size: 12"x 13K"
Exposure: 9K"
Pieces/Box: 28
Coverage: 5 boxes =
100 linear feet

Prestique High Definition

Product size 13K"x 39K"
Exposure 5K"
Pieces/Bundle 22
Bundles/Square 3/100 sq.ft.
Squares/Pallet 16

30-year limited warranty period:
5-7**years non-prorated coverage for
shingles and application labor with
prorated coverage for remainder of
limited warranty period, plus an
option for transferability*. 5-year
limited wind warranty*. Wind
Coverage: standard 80 mph.

Elk Starter Strip

52 Bundles/Pallet
18 Pallets/Truck
936 Bundles/Truck
18 Pieces/Bundle
1 Bundle = 120.33 linear feet

Available Colors (Check Availability): Antique Slate, Weatheredwood, Shakeswood, Sablewood, Hickory, Barlowood, Forest Green, Wedgewood, Birchwood, Sandalwood.
Gallery Collection: Balsam Forest™, Weathered Sage™, Sierra Sunset™.

All Prestique, Raised Profile and Seal-A-Ridge, and Prestique Starter Strip roofing products contain sealant which activates with the sun's heat, bonding shingles into a wind and weather resistant cover that resists blow-offs and leaks.

Check for availability with built-in StainGuard™ treatment to inhibit the discoloration of roofing granules caused by the growth of certain types of algae.

All Prestique and Raised Profile shingles meet UL: Wind Resistant (UL 997) and Class "A" Fire Ratings (UL 780);
and ASTM Specifications D 3010, Type-I; D 3161, Type-I; E 108 and the requirements of ASTM D 3462.

All Prestique and Raised Profile shingles have approval from the Florida Building Code Commission, Metro-Dade County, ICBO, and Texas Department of Insurance.

*See actual limited warranty for conditions and limitations.

**Effective January 1, 2004, the seven year non-prorated Underlayment Coverage Period applies only when a full Elk Roof System is installed with the original installation of the Elk shingles, all in accordance with Elk's application instructions for each product. A full Elk roof system includes Elk Hip and Ridge shingles on all hips and ridges, Elk Starter Strip along all eaves and gable eaves, an Elk ventilation system, and Elk All-Climate Self-Adhering Underlayment in all valleys. Additional Elk All-Climate Self-Adhering Underlayment is required along the rakes and lower edges of the roof in and out of the states of VA, KY, MD, KS, CO, UT, NV, & OR.

***For a Limited Wind Warranty up to 110 mph for Prestique Gallery Collection, Prestique Plus, or 90 mph for Prestique I or Grand, at least six (6) properly placed nails and Elk Starter Strip shingles are required. See application instructions printed on the shingle wrapper for additional requirements.

SPECIFICATIONS

Scope: Work includes furnishing all labor, materials and equipment necessary to complete installation of (name) shingles specified herein. Color shall be (name of color). Hip and ridge type to be Elk Seal-A-Ridge with formula FLX.

All exposed metal surfaces (flashing, vents, etc.) to be painted with matching Elk roof accessory paint.

PREPARATION OF ROOF DECK: Roof deck to be dry, well-seasoned 1" x 6" (25.4mm x 152.4mm) boards; exterior-grade plywood (exposure 1 rated sheathing) at least 3/8" (9.525mm) thick conforming to the specifications of the American Plywood Association; 7/16" (11.074mm) oriented strandboard; or chipboard. Most fire retardant plywood decks are NOT approved substrates for Elk shingles. Consult Elk Field Service for application specifications over other decks and other slopes.

Materials: Underlayment for standard roof slopes, 4" per foot (101.6/304.8mm) or greater; apply non-perforated No. 15 or 30 asphalt-saturated felt underlayment. For Low slopes (4" per foot (101.6/304.8mm) to a minimum of 2" per foot (50.8/304.8mm)), use two pieces of underlayment overlapped a minimum of 18". Fasteners shall be of sufficient length and holding power for securing material as required by the application instructions printed on shingle wrapper.

For areas where algae is a problem, shingles shall be (name) with StainGuard treatment, as manufactured by the Elk Tuscaloosa plant. Hip and ridge type to be Seal-A-Ridge with formula FLX with StainGuard treatment.

Complete application instructions are published by Elk and printed on the back of every shingle bundle. All warranties are contingent upon the correct installation as shown on the instructions. These instructions are the minimum required to meet Elk application requirements. In some areas, building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements less than those contained in its application instructions.

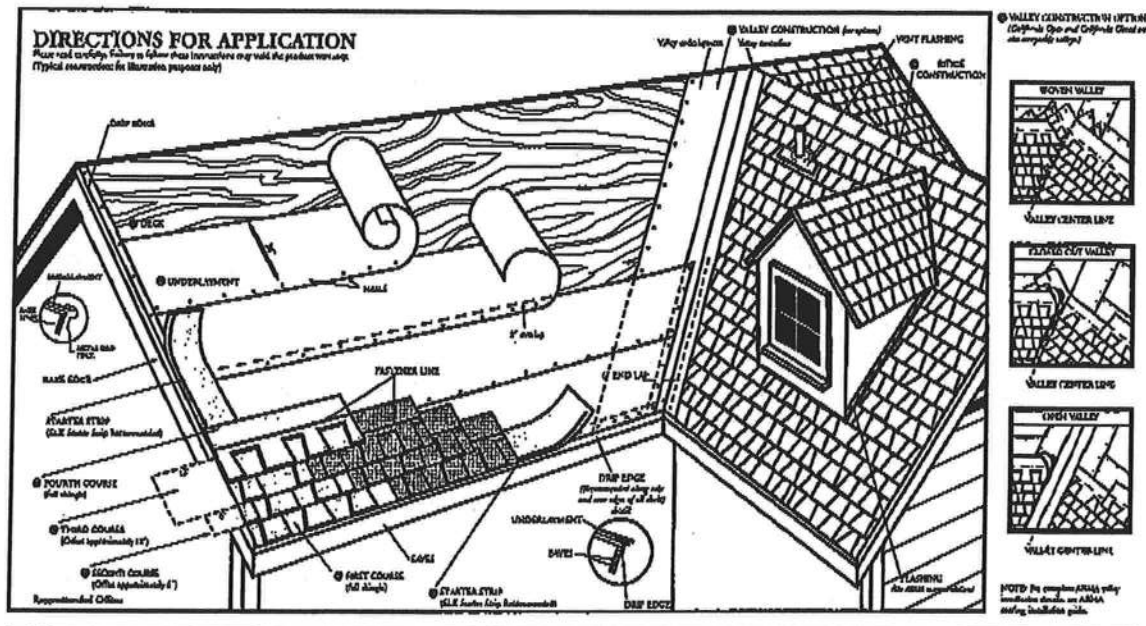
For specifications in CSI format, call 800.354.SPEC (7732) or e-mail specinfo@elkcorp.com.

**SOUTHEAST &
ATLANTIC OFFICE:**
800.945.5551

CORPORATE HEADQUARTERS:
800.354.7732

PLANT LOCATION:
800.945.5545

ELK
The Premium Choice®
www.elkcorp.com
SS00T 06/04



DIRECTIONS FOR APPLICATION

These application instructions are the minimum required to meet Elk's application requirements. Your failure to follow these instructions may void the product warranty. In some areas, the building codes may require additional application techniques or methods beyond our instructions. In these cases, the local code must be followed. Under no circumstances will Elk accept application requirements that are less than those printed here. Shingles should not be jammed tightly together. All edges should be properly ventilated. Note: It is not necessary to remove tape on back of shingle.

DECK PREPARATION

Roof decks should be dry, well-seasoned 1" x 6" boards or exterior grade plywood minimum 3/8" thick and conform to the specifications of the American Plywood Association or 7/16" oriented strandboard, or 7/16" chipboard.

UNDERLAYMENT

Apply underlayment (Non-Perforated No. 15 or 30 asphalt saturated felt, Elk Vortexshield® or self-adhering underlayment is also acceptable. Cover drip edge at eaves only.

For low slope (2/12 up to 4/12), completely cover the deck with one piece of underlayment overlapping a minimum of 18". Begin by fastening a 18" wide strip of underlayment placed along the eaves. Place a full 36" wide sheet over the eaves, horizontally placed along the eaves and completely overlapping the starter strip.

EAVE FLASHING FOR ICE DAMS (ASK A ROOFING CONTRACTOR, REFER TO ARMA MANUAL OR CHECK LOCAL CODES)

For standard slope (4/12 to less than 21/12), use coated roll roofing of no less than 50 pounds over the full underlayment extending from the eave edge to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

For low slope (2/12 up to 4/12), use a continuous layer of asphalt plastic cement between the two piles of underlayment from the eave edge up roof to a point at least 24" beyond the inside wall of the living space below or one layer of a self-adhered eave and flashing membrane.

Consult the Elk Technical Services Department for application specifications over other decks and other slopes.

STARTER SHINGLE COURSE

USE AN ELK STARTER STRIP OR THE HEADLAP OF A STRIP SHINGLE WITH THE ADHESIVE STRIP POSITIONED AT THE EAVE EDGE. With at least 3" overlap from the end of the first shingle, start at the rake edge overlapping the eave and rake edges 1/2" to 3/4". Fasten 2" from the lower edge and 1" from each side.

FIRST COURSE

Start at rake and continue course with full shingles laid flush with the starter course. Shingles may be applied with a course alignment of 45° on the roof.

SECOND COURSE

Offset the second course of shingles with respect to the first by approximately 6". Other offsets are approved if greater than 4".

THIRD COURSE

Offset the next course by 6" with respect to the second course, or consistent with the original offset.

FOURTH COURSE

Start at the rake and continue with full shingles across roof.

FIFTH AND SUCCEEDING COURSES.

Repeat application as shown for second, third, and fourth courses. Do not rock shingles straight up the roof. Offsets may be adjusted around valleys and penetrations.

VALLEY CONSTRUCTION

Open, woven and closed cut valleys are acceptable when applied by Asphalt Roofing Manufacturing Association (ARMA) recommended procedures. For metal valleys, use 36" wide vertical underlayment prior to applying metal flashing (secure edge with nails). No nails are to be within 6" of valley center.

RIDGE CONSTRUCTION

For ridge construction Elk recommends Class "A" Z-Ridge or Seal-A-Ridge® with formula FLX® or RidgeCrest® with FLX (See ridge package for installation instructions). Vented RidgeCrest or 3-tab shingles are also approved.

FASTENERS

While nailing is the preferred method for Elk shingles, Elk will accept fastening methods according to the following instructions.

Using the fastener line as a reference, nail or staple the shingle in the double thickness corner head area. For shingles without a fastener line, nails or staples must be placed between and/or in the sealant dots.

NAILS: Corrosive resistant, 3/8" head, minimum 12-gauge roofing nails. Elk recommends 1-1/4" for new roofs and 1-1/2" for re-roofs. In cases where you are applying shingles to a roof that has an exposed overhang, for new roofs only, 3/4" ring shank nails are allowed to be used from the eave's edge to a point up the roof that is past the outside wall line. 1" ring shank nails allowed for re-roof.

STAPLES: Corrosive resistant, 16-gauge minimum, crown width minimum of 15/16". Note: An improperly adjusted staple gun can result in raised staples that can cause a fish-mouthed appearance and can prevent sealing.

Fasteners should be long enough to obtain 3/4" deck penetration or penetration through deck, whichever is less. This product meets the requirements of the IRC 2003 code when fastened with 4 nails.

MANSARD APPLICATIONS

Correct fastening is critical to the performance of the roof. For slopes exceeding 60° for 21/12 use six fasteners per shingle. Locate fasteners in the fastener area 1" from each side edge with the remaining four fasteners equally spaced along the length of the double thickness (laminated) area. Only fastening methods according to the above instructions are acceptable.

LIMITED WIND WARRANTY

For a Limited Wind Warranty, all Prestique and Raised Profile® shingles must be applied with 4 properly placed fasteners, or in the case of mansard applications, 6 properly placed fasteners per shingle.

For a Limited Wind Warranty up to 110 MPH for Prestique Gallery Collection or Prestique Plus or 90 MPH for Prestique I, shingles must be applied with 6 properly placed NAILS per shingle. SHINGLES APPLIED WITH STAPLES WILL NOT QUALIFY FOR THIS ENHANCED LIMITED WIND WARRANTY. Also, Elk Starter Strip shingles must be applied at the eaves and rake edges to qualify Prestique Plus, Prestique Gallery Collection and Prestique I shingles for this enhanced Limited Wind Warranty. Under no circumstances should the Elk Shingles or the Elk Starter Strip overhang the eaves or rake edge more than 3/4" of an inch.

HELP STOP BLOW-OFFS AND CALL-BACKS

A minimum of four fasteners must be driven into the DOUBLE THICKNESS (laminated) area of the shingle. Nails or staples must be placed along - and through - the "fastener line" or on products without fastener lines, nail or staple between and in line with sealant dots. CAUTION: Do not use fastener line for shingle alignment.



Refer to local codes which in some areas may require specific application techniques beyond those Elk has specified.

All Prestique and Raised Profile shingles have a UL® Wind Resistance Rating when applied in accordance with these instructions using nails or staples on re-roofs as well as new construction.

CAUTION TO WHOLESALE: Careless and improper storage or handling can harm fiberglass shingles. Keep these shingles completely covered, dry, reasonably cool, and protected from the weather. Do not store near various sources of heat. Do not store in direct sunlight until applied. DO NOT DOUBLE STACK. Systematically rotate all stock so that the material that has been stored the longest will be the first to be moved out.

ELK
The Premium Choice.
www.elkcorp.com

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Water Wells
Pumps & Service

Phone: (386) 752-6677
Fax: (386) 752-1477

Lynch Well Drilling, Inc.

173 SW Young Place
Lake City, FL 32025
www.lynchwelldrilling.com

April 12, 2007

Columbia County Building Department
P. O. Box 1529
Lake City, FL 32056

To Whom It May Concern:

As required by building code regulations for Columbia County in order that a building permit can be issued, the following well information is provided with regard to the above-referenced well:

Size of Pump Motor:	1 Horse Power
Size of Pressure Tank:	81-Gallon Bladder Tank
Cycle Stop Valve Used:	No

Should you require any additional information, please contact us.

Sincerely,



Linda Newcomb
Lynch Well Drilling, Inc.

Water Wells
Pumps & Service

Phone: (386) 752-6677
Fax: (386) 752-1477

Lynch Well Drilling, Inc.

173 SW Young Place
Lake City, FL 32025
www.lynchwelldrilling.com

Casing Size 4 inch Steel Pump Installation: Deep Well Submersible

Pump Make Aermotor Pump Model S20-100 HP 1

System Pressure (PSI) On 30 Off 50 Average Pressure 40

Pumping System GPM at average pressure and pumping level 20(GPM)

Tank Installation: Bladder /Galvanized Make Challenger

Model PC 244 Size 81 gallon

Tank Drawdown per cycle at system pressure 25.1 gallons

Linda Newcomb
Signature

2609
License Number

Linda Newcomb
Print Name

4/12/07
Date

CERTIFICATE OF OCCUPANCY

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 17-3S-16-02168-110

Building permit No. 000026746

Use Classification SFD, UTILITY

Fire: 70.62

Permit Holder STANLEY CRAWFORD

Waste: 184.25

Owner of Building JOHN & KAREN DEARDORFF

Total: 254.87

Location: 861 NW BLACKBERRY CIRCLE, LAKE CITY, FL

Date: 11/03/2008

Harry Dick

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: 536 SE Baya DR

City Lake City

Phone 752-1703

Site Location: Subdivision _____

Lot # _____ **Block#** _____ **Permit #** 26746

Address 861 NW Blackberry Cir.

Product used

Active Ingredient

% Concentration

☒ Premise Imidacloprid 0.1%

☐ Termidor Fipronil 0.12%

☐ Bora-Care Disodium Octaborate Tetrahydrate 23.0%

Type treatment:

☒ Soil

☐ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

MB, DP, FP, & Grg's

3696

330

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

5/29/08
Date

7:57
Time

Guy
Print Technician's Name

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink