

**EXTERIOR WALL A-A** 

ONE STORY CMU

SCALE: 1/2" = 1'-0"

FOOTING DOWEL-

w/ STD HOOK

# DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT 3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 2' O.C. 1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 16" O.C. 1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W

3/16" TAPCONS @ 12" O.C.

1/4" TAPCONS @ 18" O.C. GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS & 16" O.C. (2) 1/4" TAPCONS & 24" O.C.

GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C. (2) 1/4" TAPCONS & 12" O.C.

- ADDITIONAL TOP REBAR IF RQD FOR UPLIFT

STD. 90° HOOK -

MUST EXTEND 24" PAST OPENING

\_OPENING

1. FILL LINTEL AND ALL CELLS ABOVE LINTEL.

2. VERIFY THAT ALL REINFORCEMENT HAS

3. SEE LINTEL TYPE DESIGNATION TABLE

-FILLED CELL WITH #5 VERTICAL -

SEE STRUCTRUAL PLANS FOR LOCATIONS

-CLEAN OUT RQD FOR GROUT LIFT > 5'-0" -

TYPICAL FILLED LINTEL ASSEMBLY

OR ADDITIONAL INFORMATION.

- KNOCK - OUT BLOCK

WITH (1) #5

- PRECAST LINTEL

NOTES:

**CMU WALL** 

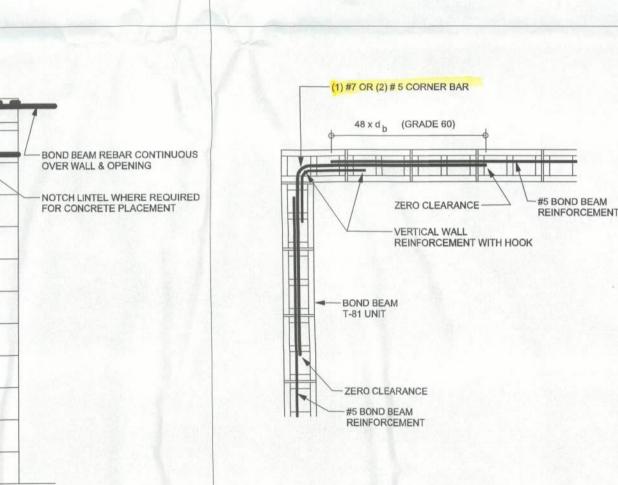
SCALE: 1/2" = 1'-0"

### EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' WALL HEIGHT

#### **GRADE & SPECIES TABLE**

		Fb (psi)	E (10 <sup>6</sup> psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0



# TYPICAL BOND BEAM CORNER

# SCALE: 1/2" = 1'-0"

#### SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

<	CASTI	्रियाच		SAFE	LOAD -	- POUND	S PER I	INEAR	FOOT	
		TYPE	01.10	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0E
LENGTI	Н		8U8	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1E
DI 4011	(0.48)	DDESAGE		3069	4605	6113	7547	8974	10394	11809
2'-10"	(34")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809
01.05	( 4 mm)			3069	3719	5163	6607	8054	9502	10951
3'-6"	(42")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809
4'-0"	/AP"\	BBECAST	4000	2561	2751	3820	4890	5961	7034	8107
4-0	(48")	PRECAST	1966	2693	4605	6113	7547	8974	10394	11809
4'-6"	(54")	PRECAST	4500	1969	2110	2931	3753	4576	5400	6224
4-0	(34)	FRECASI	1599	2189	4375	6113	7547 (7)	8672	10294	11809
E1 48	(C (III)	DDFOAGT		1349	1438	1999	2560	3123	3686	4249
5'-4"	(64")	PRECAST	1217	1663	3090	5365	7547(36)	7342 (19)	8733 (19)	10127(1
EL 401	(7011)	DDECAST		1105	1173	1631	2090	2549	3009	3470
5'-10"	(70°)	PRECAST	1062	1451	2622	4360	7168 (45)	6036 (19)	7181 (19)	8328 (2
6'-6"	(78")	PRECAST		1238	2177	3480	3031	3707	4383	5061
0-0	(10)	PRECASI	908	1238	2177	3480	5381	8360	10394(37)	8825 (1
71.011	(0011)	DDCCAGT	7.0	1011	1729	2632	2205	2698	3191	3685
7'-6"	(90")	PRECAST	743	1011	1729	2661	3898	5681	8467(44)	6472 (1
9'-4"	(112")	PRECAST	554	699	1160	1625	2564	3486	2818	3302
3-4	(112)			752	1245	1843	2564	3486	4705(37)	6390(4
10'-6"	(426")	PPECAST	10-10-1	535	890	1247	2093	2777	2163	2536
10-0	(126")	PRECAST	475	643	1052	1533	2093	2781	3643 (38)	4754 (4
4.41.411	(42611)	DDECAST	000	582	945	1366	1846	2423	3127	4006
11'-4"	(136")	PRECAST	362	582	945	1366	1846	2423	3127	4006
401.01	(4.4.48)	PPECACT	007	540	873	1254	1684	2193	2805	3552
12'-0"	(144")	PRECAST	337	540	873	1254	1684	2193	2805	3552
401.48	(40011)	DDEGAGE		471	755	1075	1428	1838	2316	2883
13'-4"	(160")	PRECAST	296	471	755	1075	1428	1838	2316	2883
4.41.011	(40011)	DDECAGE		424	706	1002	1326	1697	2127	2630
14'-0"	(168")	PRECAST	279	442	706	1002	1326	1697	2127	2630
4.41.00	(47CII)	DRESTRESSED		NR	NR	NR	NR	NR	NR	NR
14'-8"	(176")	PRESTRESSED	N.R.	458	783	1370	1902	2245	2517	2712
451.49	(40.411)	PRESTRESSED		NR	NR	NR	NR	NR	NR	NR
15'-4"	(184")	FRESTRESSED	N.R.	412	710	1250	1733	2058	2320	2513
17'-4"	(200")	DDESTRESSED		NR	NR	NR	NR	NR	NR	NR
17-4	(208")	PRESTRESSED	N.R.	300	536	950	1326	1609	1849	2047
19'-4"	(232")	PRESTRESSED		NR	NR	NR	NR	NR	NR	NR
10-4	(202)	I NESTRESSED	N.R.	235	418	750	1037	1282	1515	1716
21'-4"	(256")	PRESTRESSED		NR	NR	NR	NR	NR	NR	NR
217	(200)	TRESTRESSED	N.R.	180	340	598	845	1114	1359	1468
22'-0"	(264")	PRESTRESSED		NR	NR	NR	NR	NR	NR	NR
22-0	(204)	T NEG TREGGED	N.R.	165	315	550	784	1047	1285	1399

## SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

129 250 450 654 884 1092 1222

24'-0" (288") PRESTRESSED N.R.

<	CAST	- Carre	SAFI	LOAD	- POUIN	IDS PE	R LINE	AR FOO	Т
_		TYPE	8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-
LENG	TH		8F8-2T	8F12-2T	8F16-2T	8F20-2T	8F24-2T	8F28-2T	8F32-
			1972	3173	4460	5747	7034	8321	9608
2'-10"	(34")	PRECAST	1972	3173	4460	5747	7034	8321	960
			1569	2524	3547	4569	5591	6613	763
3'-6"	(42")	PRECAST	1569	2524	3547	4569	5591	6613	763
41.00	/40II\	DDECAST	1363	2192	3079	3966	4853	5740	662
4'-0"	(48")	PRECAST	1363	2192	3079	3966	4853	5740	662
4'-6"	(E4")	DDECAST	1207	1940	2724	3508	4292	5077	586
4-0	(54")	PRECAST	1207	1940	2724	3508	4292	5077	586
F1 48	40 AIII)	DDEGAGE	1016	1632	2290	2949	3607	4265	492
5'-4"	(64")	PRECAST	1016	1632	2290	2949	3607	4265	492
E1 40F	/70m	DDECAST	909	1492	2093	2694	3295	3897	449
5'-10"	(70")	PRECAST	929	1492	2093	2694	3295	3897	449
OL OIL	/70m	PPEGAGE	835 (12)	1340	1880	2419	2959	3498	403
6'-6"	(78")	PRECAST	835	1340	1880	2419	2959	3498	403
			727 (23)	1021	1634 (122)	2102 (11)	2571(10)		-
7'-6"	(90")	PRECAST	727	1166	1634	2102	2571	3039	350
01.48	(4.40II)	DDEGAGE	591	680	1133 (155)	1471 (15)	1811 (15)	2152 (16)	249
9'-4"	(112")	PRECAST	591	851	1326	1705	2084	2463	284
401.01	// CON	DDECLOT	530	552	914 (15)	1185 (15)	1458 (15)	1732 (15)	_
10'-6"	(126")	PRECAST	530	686	1183	1526	1865	2204	254
			474	485	798 (15)	1034 (15)	1272 (15)	100000	_
11'-4"	(136")	PRECAST	494	599	1028	1422	1738	2053	236
	pro-Contract		470 (9)	441	723 (141)	936 (14)	1151(15)	1366 (15)	
12'-0"	(144")	PRECAST	470	543	928	1349	1649	1948	224
			418 (15)	373	606 (141)	783 (14)	962 (14)	1141 (14)	
13'-4"	(160")	PRECAST	428	455	770	1145	1444	1718	1993
	***		384 (15)	346	559 (141)	723 (14)	887 (14)		
14'-0"	(168")	PRECAST	410	420	709	1050	1434 (8)		
			230	323	519 (133)	671 (13)	823 (13)	976 (14)	
14'-8"	(176")	PRESTRESSED	246	390	655	968	1324 (8)		
	C1V 7	DDESTREASE	224	302	485 (133)	626 (13)	767 (13)		-
15'-4"	(184")	PRESTRESSED	230	364	609	897	1224 (8)		
	/000W		187	255	404 (122)	520 (12)	637 (12)		
17'-4"	(208")	PRESTRESSED	192	303	500	732	993 (8)	1	
401.41	(000II)	DDECTDECOE	400	222	347 (111)	446 (11)	-		-
19'-4"	(232")	PRESTRESSED	166	261	424	616	831 (8)	COCCODING.	10000
041.48	(OF 011)	DDECTDECOE	142	198	306 (111)	393 (11)	480 (11)	567 (11)	
21'-4"	(256.)	PRESTRESSED	142	230	369	531	713 (7)	903 (13)	10000
201.0"	(0C 411)	DDECTDECOE	137	192	295 (100)	378 (11)	461 (10)	545 (11)	629
22'-0"	(264")	PRESTRESSED	137	221	354	508	681 (7)	861 (13)	997
0.41.05	(DE 011)	DDEGTOO	124	175	267 (103)	341 (10)	416 (10)	491 (10)	566
24'-0"	(288")	PRESTRESSE	124	200	316	450	600 (7)	756 (12)	875

# SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

			SAFE LOAD - POUNDS PER LINEAR FOOT							
		TYPE		8RF6-0B	8RF10-0B3	8RF140B	8RF18-0B	8RF22-0B	8RF26-0B	8RF30-0B
LENG	TH		8RU6	8RF6-1B	8RF10-1B3	8RF141B	8RF18-1B	8RF22-1B	8RF26-1B	8RF30-1B
41. 411	/FO!!\	PRECACE	4005	1749	3355	3280	4349	5421	6493	7567
4'-4"	1'-4" (52") PRECAST	1635	1891	3699	5206	6639	8060	9479	10893	
41 00	4'-6" (54") PRECAST	PPEGAGE	4404	1596	3063	2992	3968	4946	5924	6904
4-6		1494	1756	3699	5208	6639	8060	9479	10893	
5' 9"	5'-8" (68") PRECAST	866	920	1770	1716	2277	2839	3402	3966	
0-0			1167	2481	4567	6389	8060 (34)	7917 (19)	9311 (19	
5'-10"	(70")	PRECAST	940	859	1653	1600	2124	2649	3174	3700
0 10	(,0)	TILLOAGT	810	1113	2342	4242	6639 (10)	8060 (39)	7402 (19)	8706 (19)
6'-8"	(80")	PRECAST	707	901	1825	3120	5048	7747	9448	7360
0-0	(00)	PRECASI	797	901	1825	3120	5048	7915	9479	10893 (32)
7'-6"	7'-6" (90") PRE	PRECAST	000	755	1490	2459	3776	5743	7239	5623
7 -0		FILLONOT	669	755	1490	2459	3776	5743	8998 (19)	10893 (48)
9'-8"	(116")	PRECAST	411	466	999	1568	2253	3129	4091	3146
	(III) I INLUMOI	70 1	1 22-55		The second second	The second second	11 - 3 - 10 1 1 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	THE PERSON NAMED IN COLUMN TWO		

## 9-8 (110 ) PRECAST 411 526 999 158 2253 3129 4150 5891 (47) SAFE UPLIFT LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

			SAFE LOAD - POUNDS PER LINEAR FOOT						
		TYPE	8RF6-1T	8RF10-1T	8RF14-1T	8RF18-1T	8RF22-1T	8RF26-1T	8RF30-1T
LENG	ENGTH		8RF6-2T	8RF10-2T	8RF14-2T	8RF18-2T	8RF22-2T	8RF26-2T	8RF30-2T
41 48	41 (FOIL) PDECAGE	PRECACE	905	1748	2635	3522	4409	5296	6183
4'-4" (52")	PRECAST	905	1748	2635	3522	4409	5296	6183	
AL CII	-6" (54") PRECAST	867	1675	2525	3374	4224	5074	5924	
4'-6"		PRECASI	867	1675	2525	3374	4224	5074	5924
5'-8"	8" (68") PR	PRECAST	675	1301	1960	2618	3277	3935	4594
J -O			675	1301	1960	2618	3277	3935	4594
5'-10"	(70")	PRECAST	655	1262	1900	2538	3176	3815	4453
0-10	(10)	FRECASI	655	1262	1900	2538	3176	3815	4453
21 011	(0.011)	PRECACT	570	1012	1651	2204	2758	3312	3865
6'-8"	(80")	PRECAST	570	1097	1651	2204	2758	3312	3865
71 611	(00")	PRECAST	506	797	1462 (8))	1952 (7)	2442 (6)	2931 (6)	3257
7-0	"-6" (90")		506	967	1462	1952	2442	2931	3421
9'-8"	0" /446"\	DDECAST	395	491	931 (12))	1301 (15)	1640 (15)	1980 (15)	2322 (16
9-0	(110)	PRECAST	395	589	1135	1514	1893	2272	2652

## **WOOD ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS, SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24	77979	went to be the	
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROI 12" EMBEDMENT
< 10980	< 6485	HGT-2	=	16 -10d	2-5/8" THREADED RO
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED RO 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROI 12" EMBEDMENT
I I I I I I I I I I I I I I I I I I I		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d	Mark Service	
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d	Test survey of	1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

### **GENERAL NOTES**

1. Provide full mortar bed and head joints. 2. Shore filled lintels as required. 3. Installation of lintel must comply with the architectural and/or structural documents. 4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection 6. Bottom field added rebar to be located at the bottom

of the lintel cavity. 7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.

8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

#### 9. Safe load rating based on rational design analysis per ACI 318 and ACI 530 10. Product Approvals: Miami-Dade County, Florida No.

03-0606.05 11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating. 12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

Applied vertical load
Safe vertical load
Safe horizontal load

13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

# MATERIALS

1. fc 8" precast lintel = 3500 psi 2. fc prestressed lintel = 6000 psi 3. Grout per ASTM C476 fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump 4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi 5. Rebar per ASTM A615 grade 60 6. Prestressing strand per ASTM A416 grade 270 low relaxation 7. Mortar per ASTM C270 type M or S

TYPE DESIGNATION

F = FILLED WITH GROUT / U = UNFILLED / S = SOLID **OUANTITY OF #5 FIELD ADDED REBAR AT** BOTTOM OF LINTEL CAVITY

SAFE LOAD TABLE NOTES

Exception: Safe loads for unfilled lintels must

3. Safe loads are superimposed allowable loads.

4. Safe loads based on grade 40 or grade 60

5. One #7 rebar may be substituted for two

The designer may evaluate concentrated

calculating the maximum resisting moment

loads from the safe load tables by

1. All values based on minimum 4 inch

be reduced by 20% if bearing length is

nominal bearing.

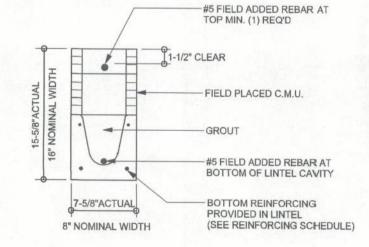
less than 6 1/2 inches

#5 rebars in 8" lintels only

2. N.R. = Not Rated

field rebar.

8F16-1B/1T-4'-0" ---LENGTH NOMINAL WIDTH -NOMINAL HEIGHT-QUANTITY OF #5 FIELD ADDED REBAR AT TOP



#### **GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS, TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOAD

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

#### BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS.

#### TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

### **ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING, THE BUILDER SHOULD USE CARE CHECKING TH DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

#### MASONRY TRUSS ANCHOR TABLE OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

	UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
or composite lintel heights not shown,	< 1205	TA22	10-10d x 1 1/2"
use safe load from next lower height shown.	< 1605	TA22	11-10d
B. For lintels lengths not shown, use safe load from next longest length shown  D. All safe loads in units of pounds per linear foot  O. All safe loads based on simply supported	< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
	< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
span.	< 1040	META20	7-10d, 1 1/2"
11. The number in the the parenthesis	< 1490	META20	10-10d, 1 1/2"
indicates the percent reduction for grade 40 field added rebar.	< 1780	HETA20	7-16d
Example 7'-6" lintel type 8F32-1B safe gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15%	< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
reduction 6472 ⇒ (.85) = 5501 plf	< 2130	HHETA20	17-10d, 1 1/2"
	< 2310	HHETA24	21-10d, 1 1/2"
	< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
	< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
	< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

# **MASONRY NOTES:**

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements			
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi			
2.1	Mortar	ASTM C 270, Type N, UNO			
2.2	Grout	ASTM C 476, admixtures require approval			
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block			
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"			
2.4 Reinforcing bars, #3 - #11		ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)			
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS			
2.4F Coating for corrosion protection		Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS			
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.			
3.3.E.7	Movement joints	Contractor assumes responsibility for ty and location of movement joints if not detailed on project drawings.			

**REVISIONS** 

IGINEER OF RECORD: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

Stated dimensions supercede scaled imensions. Refer all questions to ark Disosway, P.E. for resolution Do not proceed without clarification.

DIMENSIONS:

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Sheet A-0, attached, comply with applicable portions of the Florida Building Code 2007 & 009 supplements, to the best of my knowled IMITATION: This design is valid for one

CERTIFICATION: These plans and Cover

building at specified location. In case of conflict tructural requirements, scope of work, and builder responsibilities control.



**Blake Construction** 

New Dentist Office for: **Family Health Center** of Columbia County

ADDRESS: Lake City, Florida Columbia County

Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32025 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871 windloadengineer@bellsouth.net PRINTED DATE:

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FINALES DATE: 2011-06-01

> JOB NUMBER: 1104014 DRAWING NUMBER

> > **S-1** OF 10 SHEETS