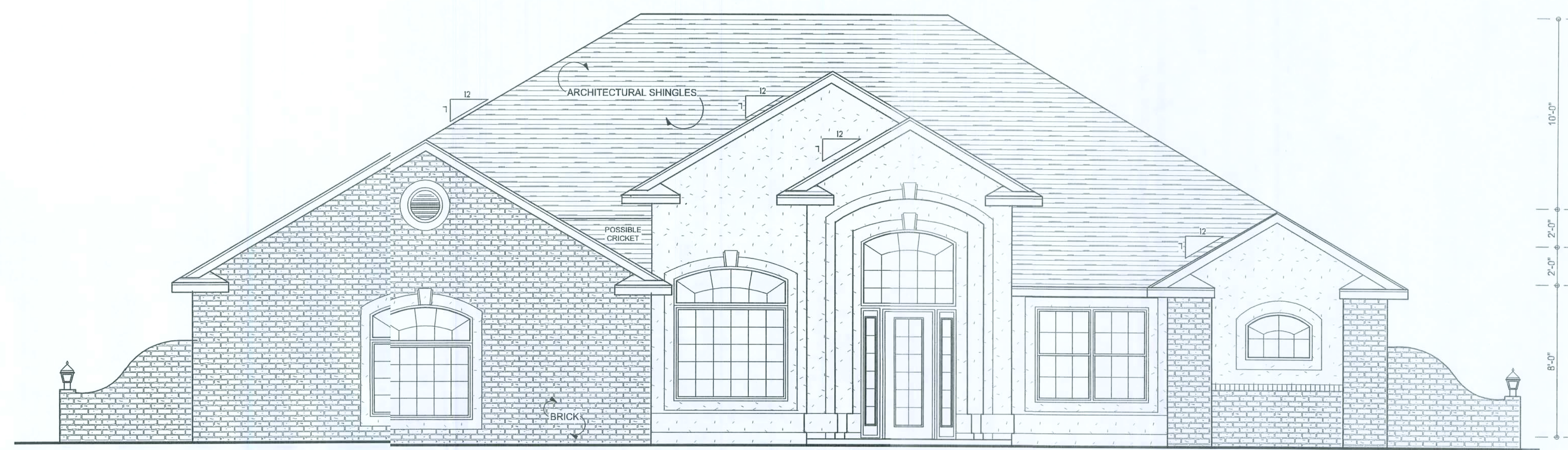


REVISIONS	

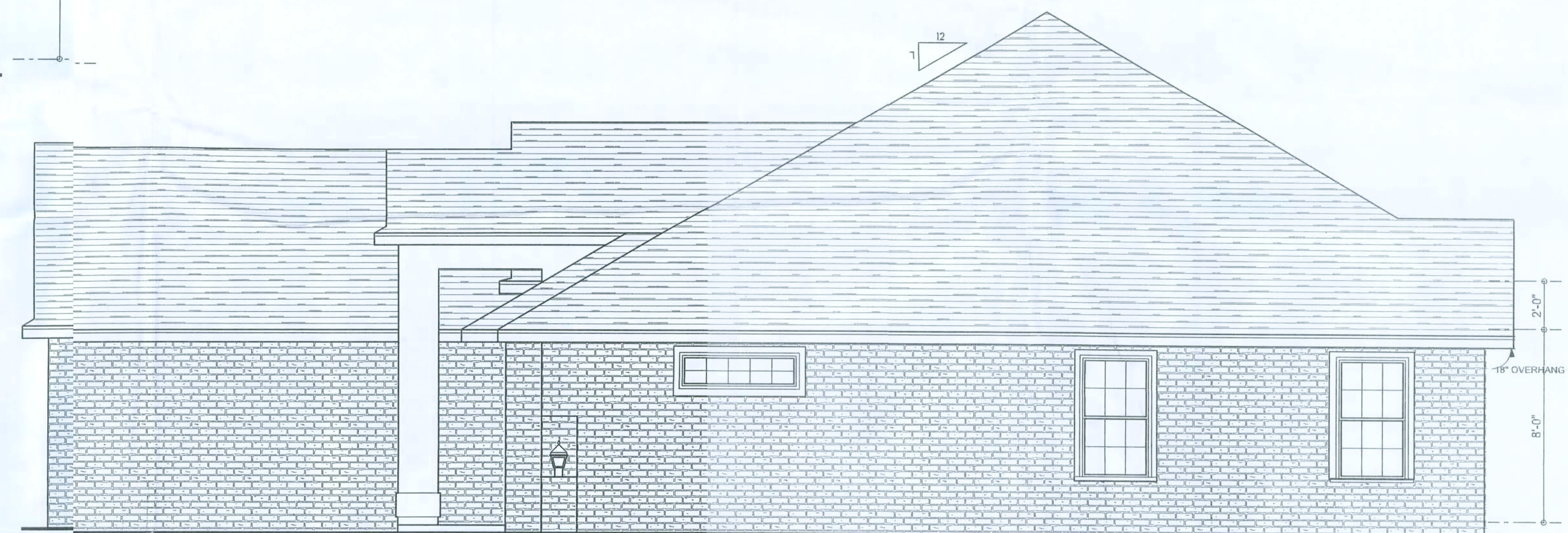


NOTE: BUILDER TO VERIFY WINDOW
HIGHT BEFORE BEGINING CONSTRUCTION

FRONT ELEVATION
SCALE: 1/4" = 1'-0"



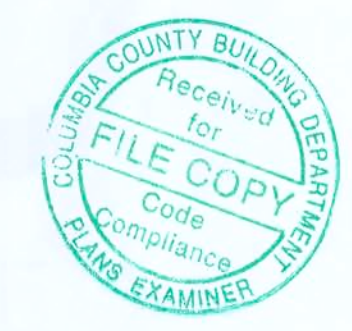
LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



REQUIRED ROOF VENTILATION:
AS PER FLORIDA BUILDING CODE 2309.7

RIDGE VENT
MIN. 50% TOTAL VENT AREA
LOCATED IN THE UPPER PORTION OF ATTIC (MIN. 3' ABOVE EAVE)
3247 S.F. / 300 x 50% = 5.5 S.F. RIDGE VENT AREA REQUIRED
50 FEET OF RIDGE VENT REQUIRED

SOFFIT VENT
3247 S.F. / 300 x 50% = 5.5 S.F. SOFFIT VENT AREA REQUIRED
184 FEET OF SOFFIT VENT REQUIRED

BUILDER MUST VERIFY THE FOLLOWING MINIMUM NET FREE VENT AREAS:

1. RIDGE VENTS = 16 IN2/FT (.11 FT2/FT)
2. OFF-RIDGE VENTS = .70 FT2 PER 4' UNIT
3. SOFFIT VENTS = 4.3 IN2/FT (.03 FT2/FT)

ERKINGER HOME BUILDERS, INC.

RANCALL & MAKSHA BURNHAM

ADDRESS:
Lot #7, Cubbestone S/D
Columbia County, Florida

PRINTED DATE:
July 20, 2009

DRAWN BY: Matthew A. Erkinger CHECKED BY: Evan Beamsley

DESIGNED BY:
Matthew A. Erkinger

FINAL DATE:

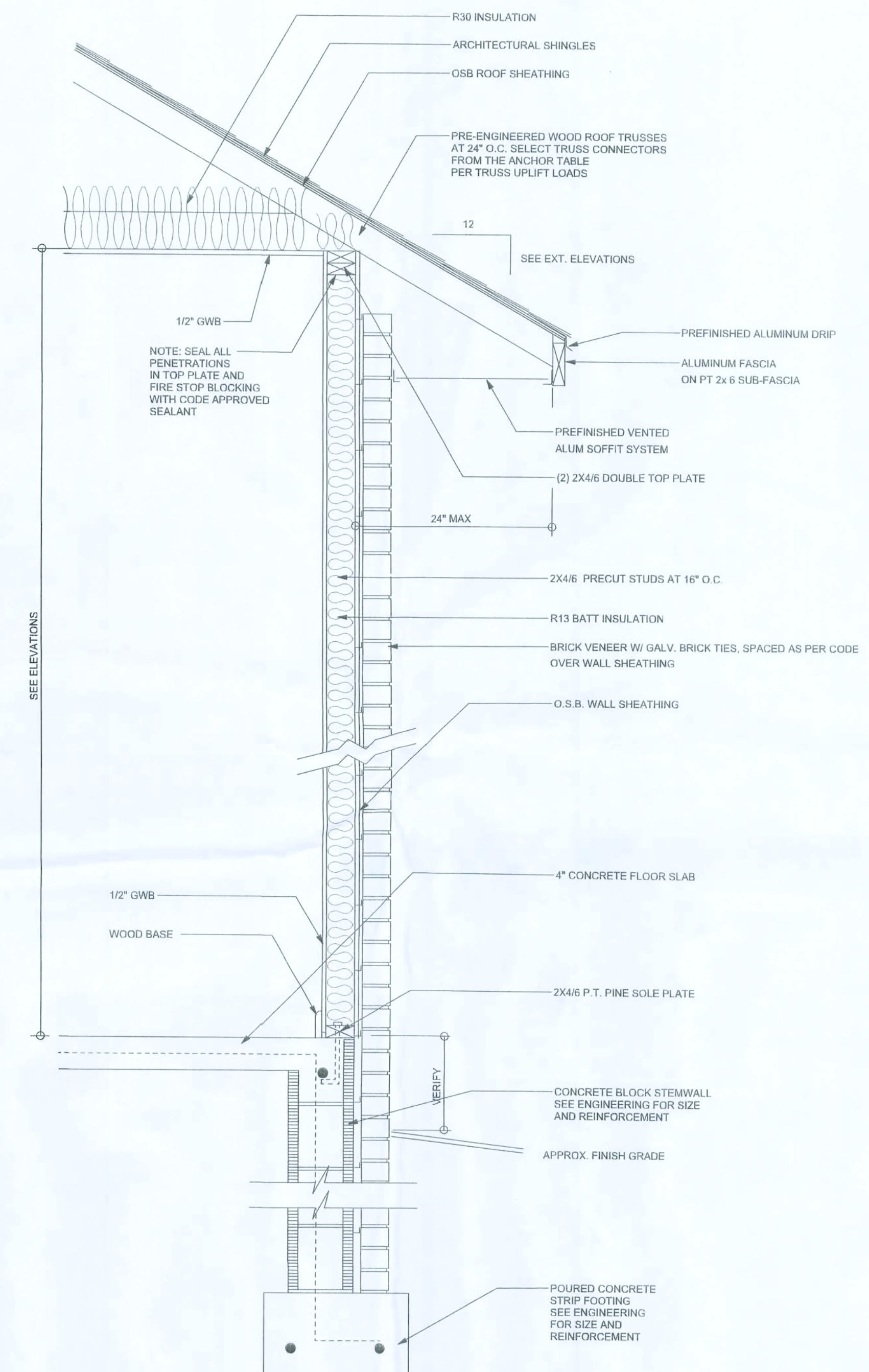
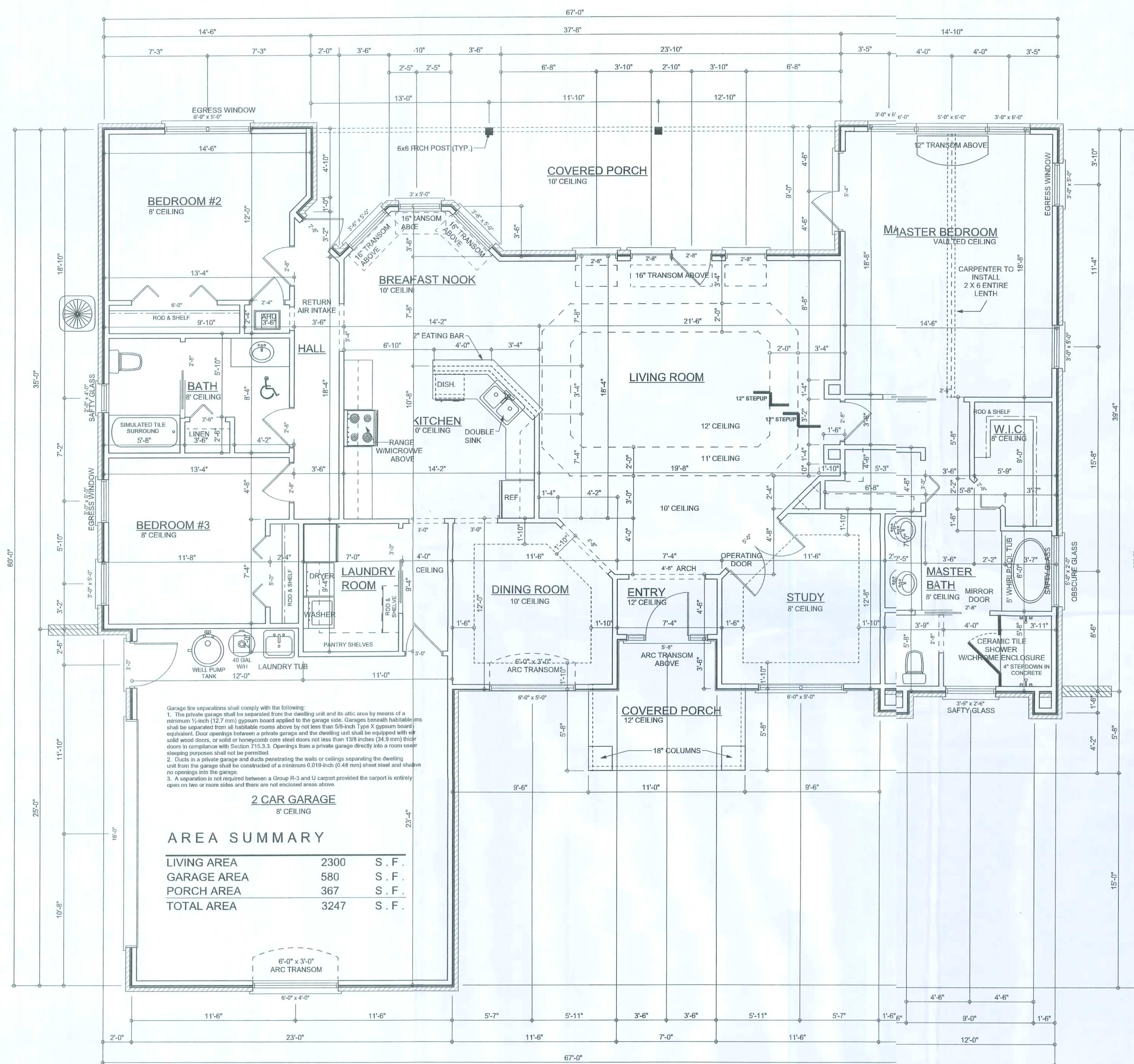
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901101

DRAWINGNUMBER
A1

OF 3 SHEETS

REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



TYPICAL DESIGN WALL SECTION NON - STRUCTURAL DATA

SCALE: 1" = 1'-0"

SCALE: 1" = 1'-0"

SCALE: 1" = 1'-0"

**BERKINGEE HOME
BUILDERS, INC.**

RANDALL &
MARSHA
BURNHAM

ADDRESS:
#7, Cobblestone S/D
Columbia County, Florida

PRINTED DATE:
July 20, 2019

DRAWN BY: Drew A. Erking	CHECKED BY: Evan Beamsley
-----------------------------	------------------------------

DESIGNED BY:

Matthew A. Erking

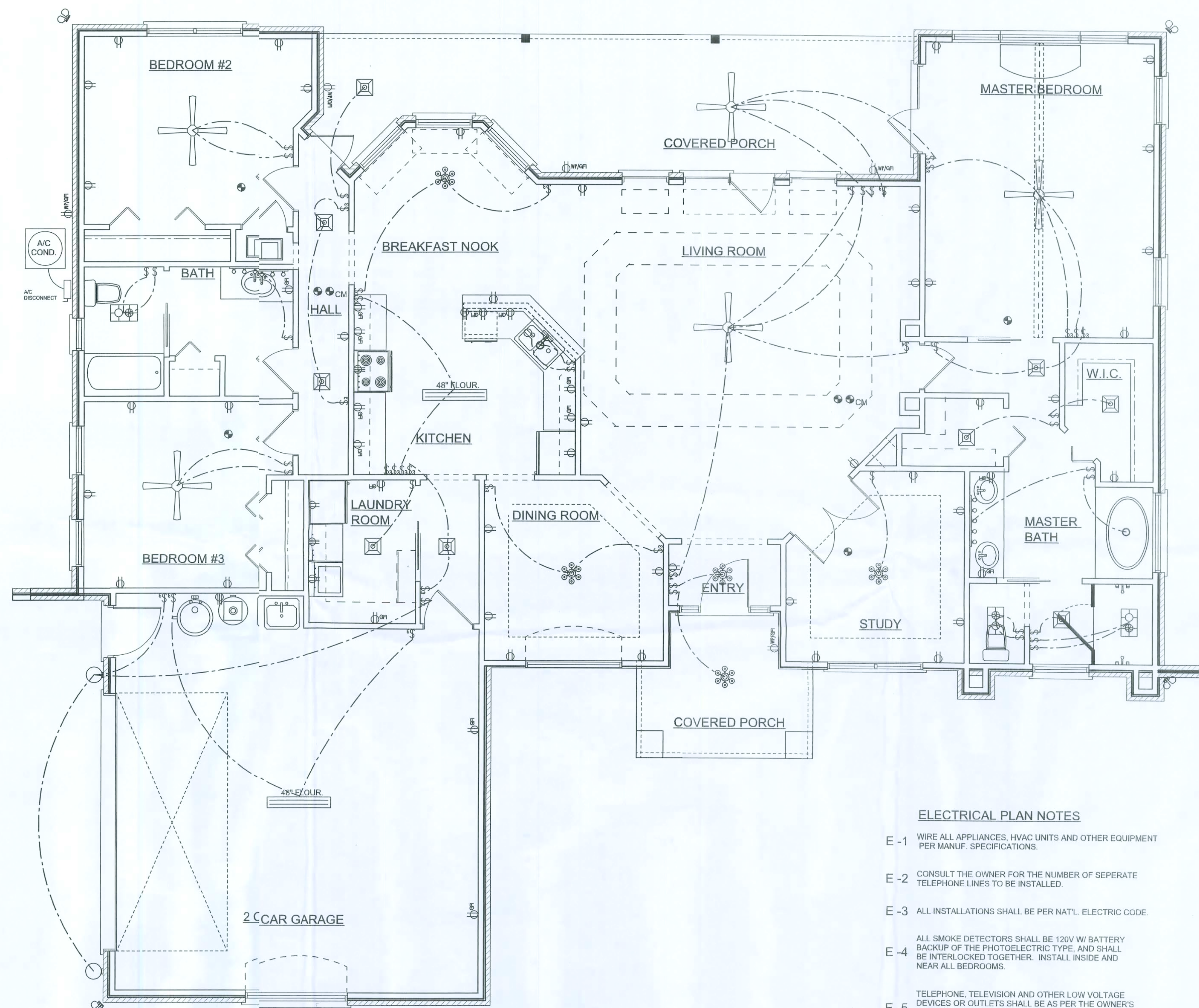
EXPIRATION DATE:	
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JOB NUMBER:
907101

DRAFTING NUMBER

A2

3 SHEETS



ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"

ELECTRICAL PLAN NOTES

- E-1 WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- E-2 CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- E-3 ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- E-4 ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- E-5 TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- E-6 ELECTRICAL CONTR. SHALL BE RESPONSIBLE FOR THE DESIGN & SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- E-7 ENTRY OF SERVICE (UNDERGROUND OR OVERHEAD) TO BE DETERMINED BY POWER COMPANY.
- E-8 ALL BEDROOM RECEPTACLES SHALL BE AFCI (ARC FAULT CIRCUIT INTERRUPT)
- E-9 ALL OUTLETS TO BE LOCATED ABOVE BASE FLOOD ELEVATION

ERKINGER HOME BUILDERS, INC.

RANDALL & MARSHA BURNHAM

ADDRESS:
Lot #7, Cobblestone S/D
Columbia County, Florida

PRINTED DATE:
July 20, 1998

DRAWN BY: Matthew A. ErKinger
CHECKED BY: Evan Beamsley

DESIGNED BY:
Matthew A. ErKinger

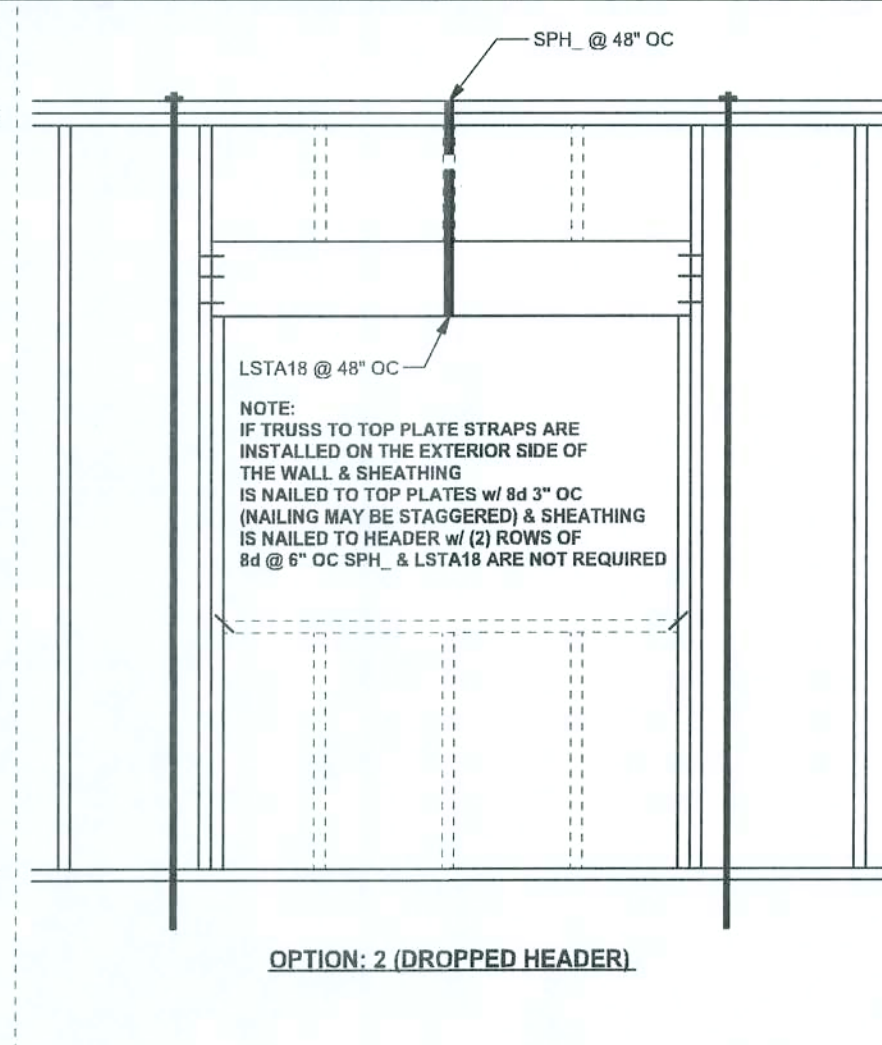
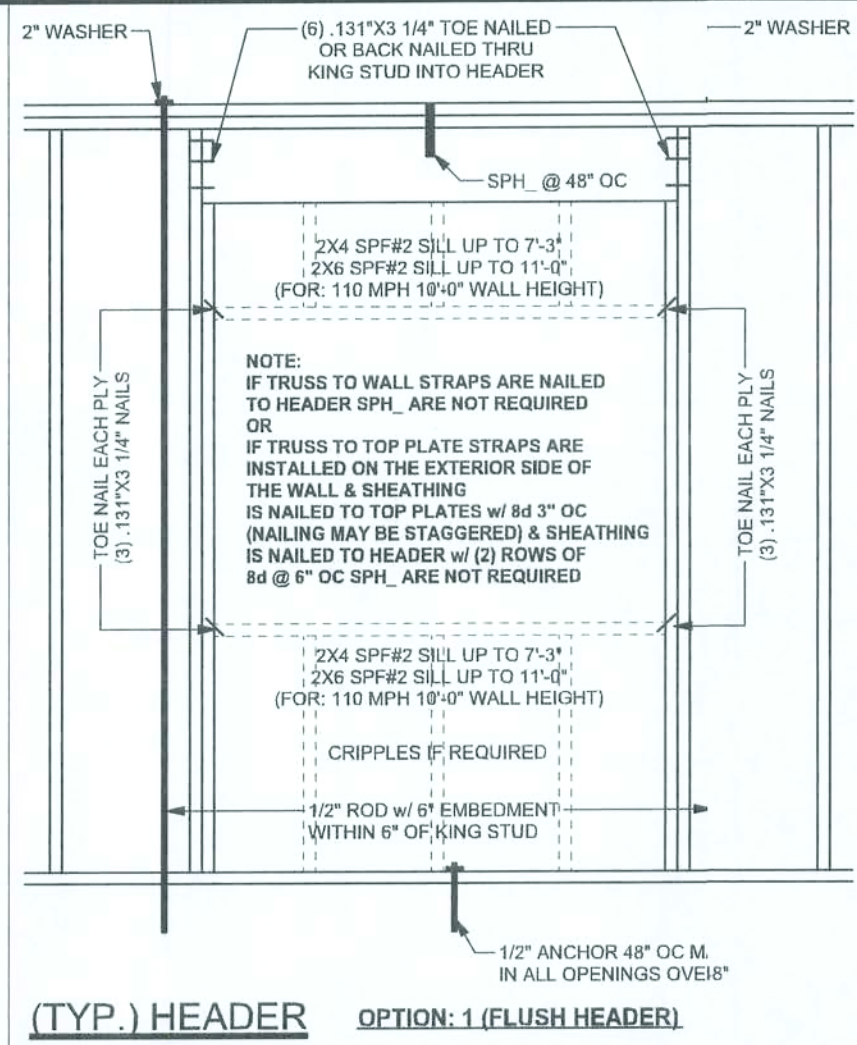
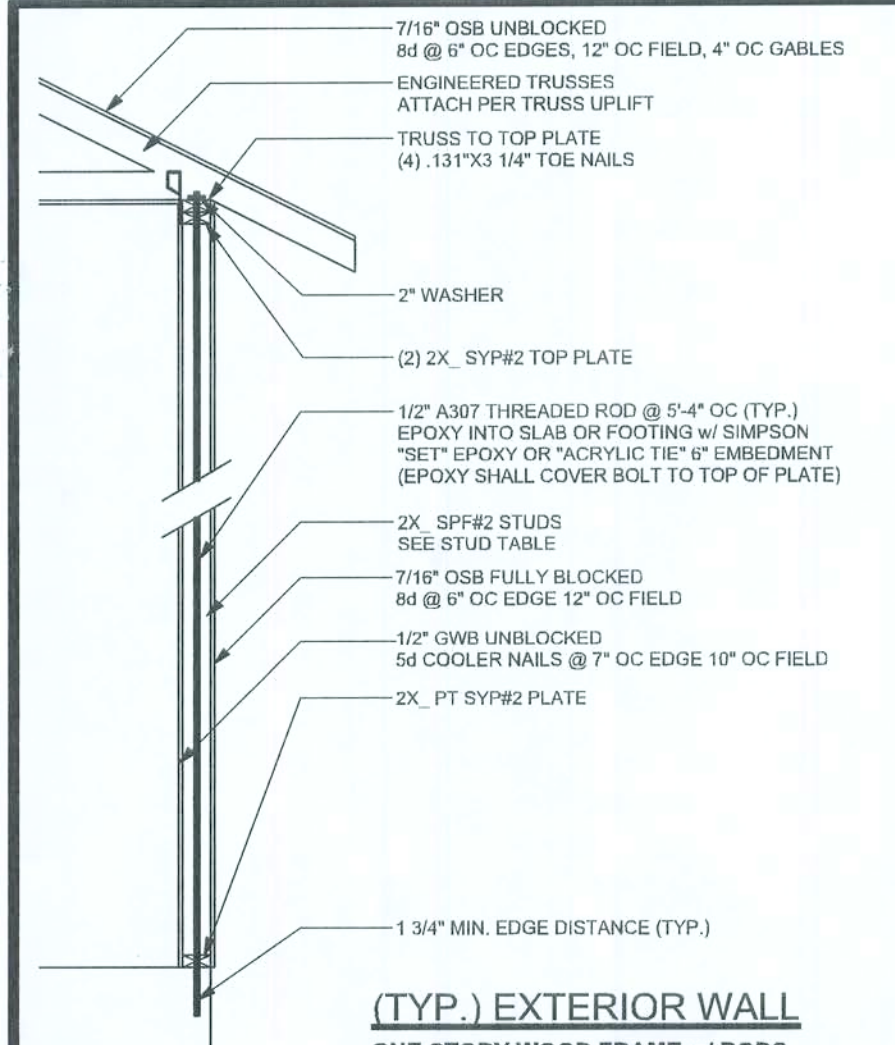
FINALS DATE:

JOB NUMBER:
90*101

DRAWING NUMBER

A3

OF 3 SHEETS



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SPF	F1 SYP	F2 SYP	F1 SPF	F2 SPF	TO RAFTER/TRUSS	TO PLATES
H5	455	265	11115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	12125	160	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	15150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	11110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H4-1	1465	1050	51515	265	480	245	12-8d x 1 1/2"	13-8d
H4-2	1465	1050	51515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	58585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	45555	365	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12-LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12-MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16-HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEDOWNS								
LG12	2050	1785	70700	170	700	170	14-16d	14-16d
LG13-SDS2.5	3685	2655	79795	410	795	410	12-SDS 1/4" x 12"	26-16d5
LG14-SDS3	4060	3600	200000	675	2000	675	12-SDS 1/4" x 3"	36-16d5
MG1	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								
SSP DOUBLE TOP PLATE	435	435					3-10d	4-19d
SSP SINGLE SILL PLATE	455	420					1-10d	4-19d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-19d
DSP SINGLE SILL PLATE	825	600					2-10d	8-19d
SP1	585	535					4-10d	6-10d
SP2	1065	605					6-10d	6-10d
SP4	885	760					6-10d	6-10d
SPH4	1240	1065					6-10d x 1 1/2"	6-10d x 1 1/2"
SPH1	885	760					6-10d x 1 1/2"	6-10d x 1 1/2"
SPH8	1240	1065					10-10d x 1 1/2"	10-10d x 1 1/2"
LSTA18	1235	1110					14-10d	
LSTA21	1235	1235					16-10d	
CS28	1030	1030					14-10d	
CS16	1705	1705					22-10d	
STUD ANCHORS								
LTT119	1350	1305					8-16d	1/2" ANCHOR
LTT131	2310	2310					18-18d x 1 1/2"	5/8" ANCHOR
H2A	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					18-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR

- (1) W/ INSTALLATION OF 4-16d5 OPTIONAL NAIL HOLES
(2) FOR SYP GIRDER & SPF STUDS

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-0" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 16" O.C. X 18.5" = 13'-0" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCE 2007. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, LOCATION PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSSES TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING'S DESIGN AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X8 RAFTERS WITH MN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 760 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN. FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, F_y = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/ WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/WW OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE).

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F_y = 60 KSI. ALL LAP SPLICES 40" DB (25" FOR 85 BARS). UNCL. REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-96, UNCL.

GLULAM BEAMS: GLB, 24F-V3SP, F_b = 2.4kSI, E = 1800kSI. UNCL. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (13d), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNCL.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 5/64", WITH 5/8" BOLTS TO BE 3" x 3" x 5/64", WITH 3/4" BOLTS TO BE 3" x 3" x 5/64", WITH 1/2" BOLTS TO BE 3" x 3" x 5/64", UNCL.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBCE TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S DESIGN WORK:
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCE 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, LOCATION PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS TO TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCE 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCE 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE COMPLETE ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD CHECK CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 100 FT IN EXPOSED, 50 FT IN EXPOSED, C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE.

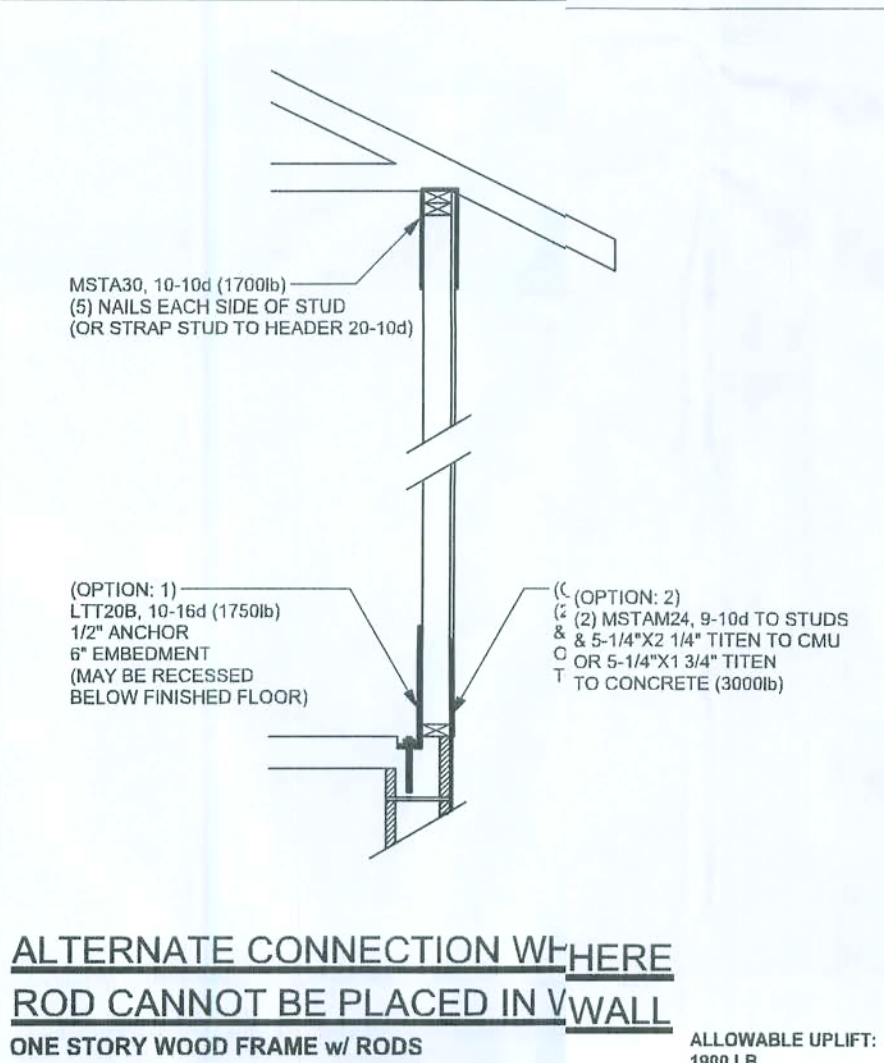
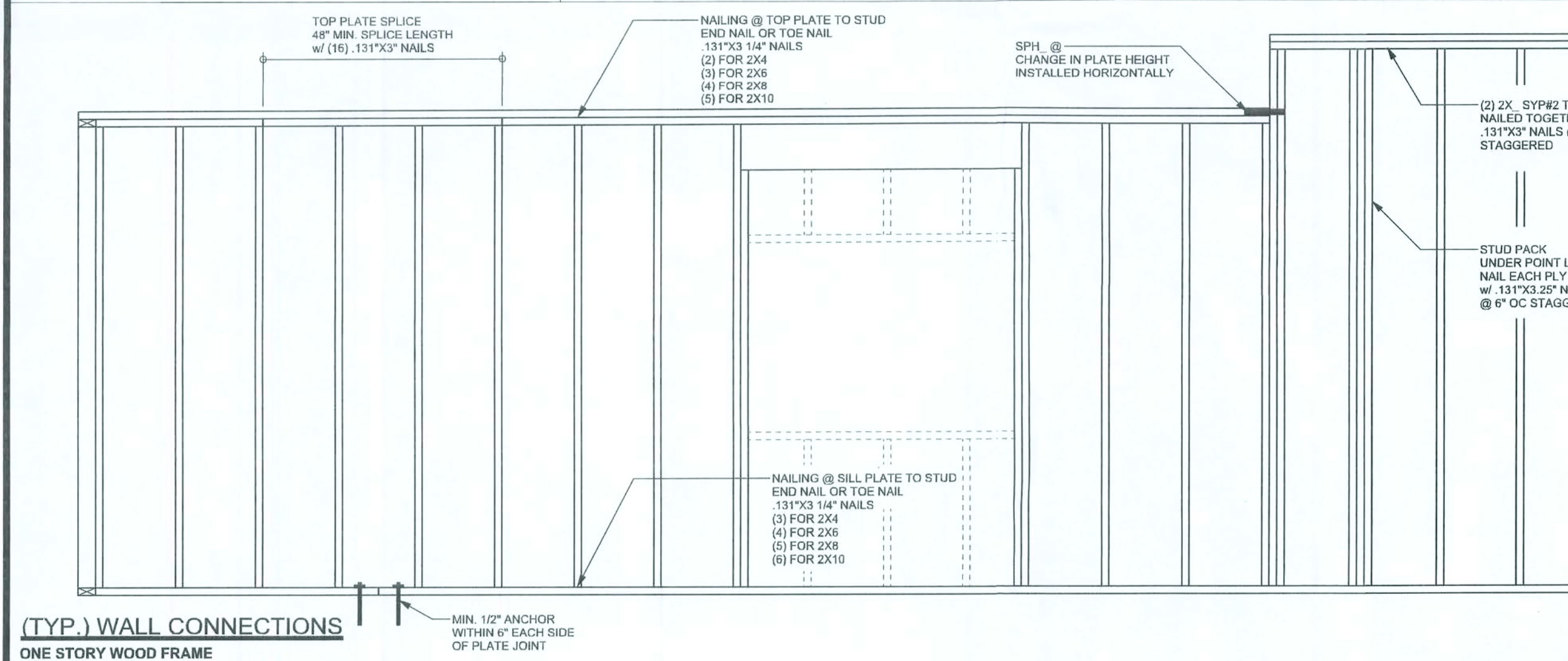
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION.

- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

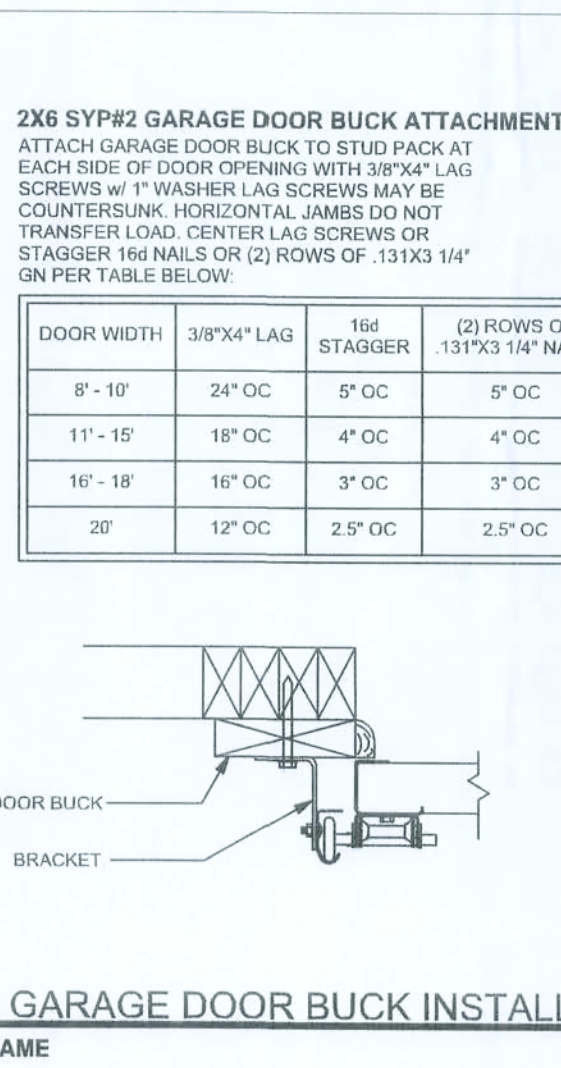
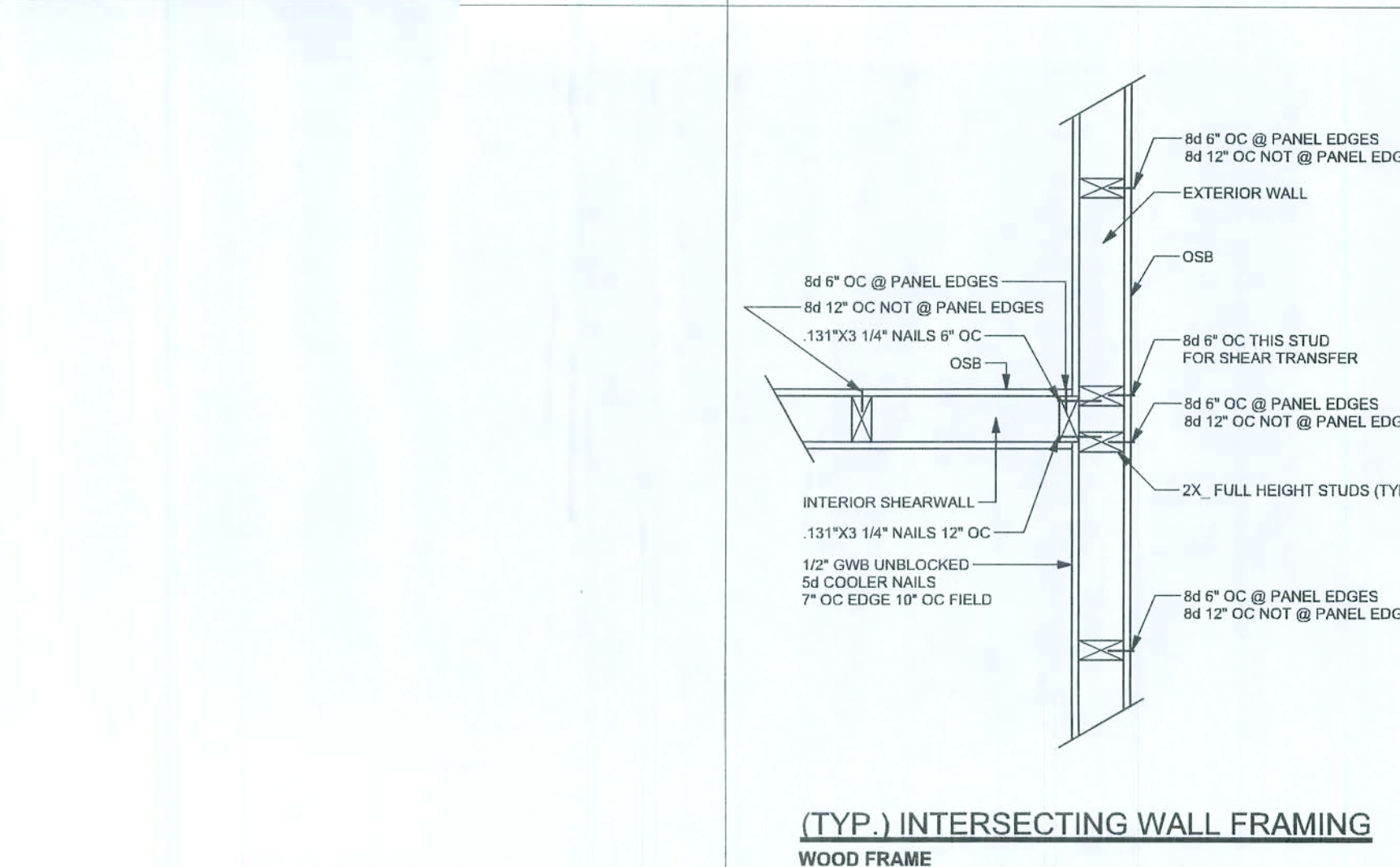
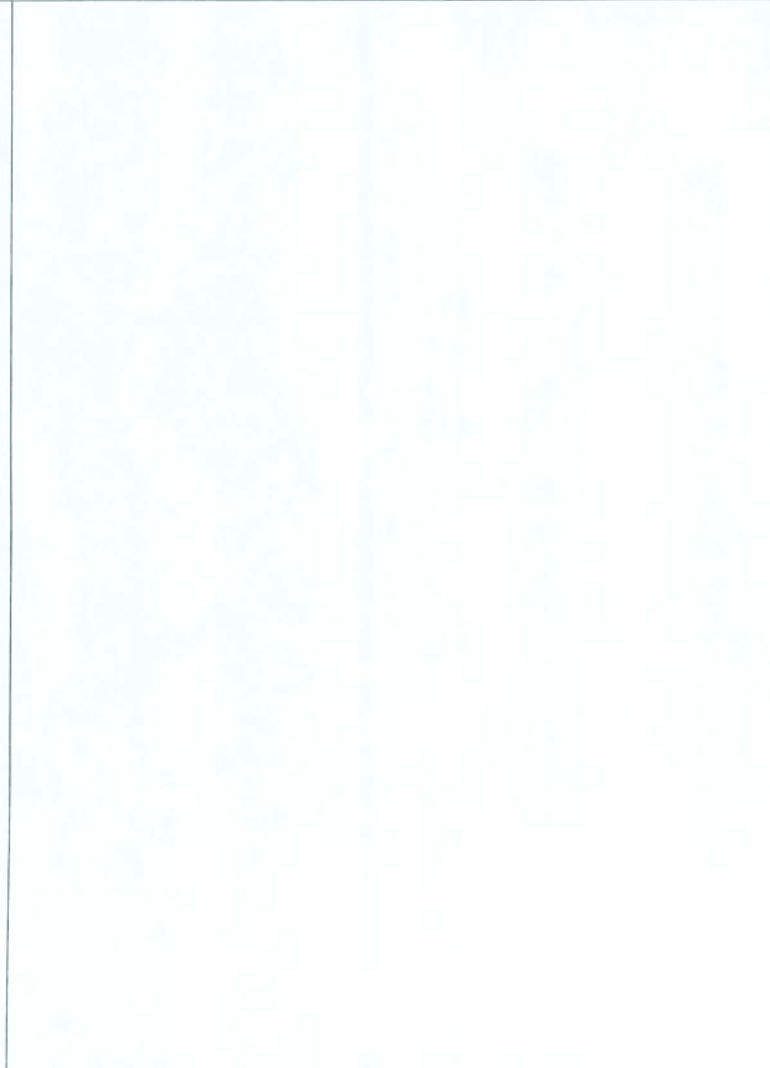
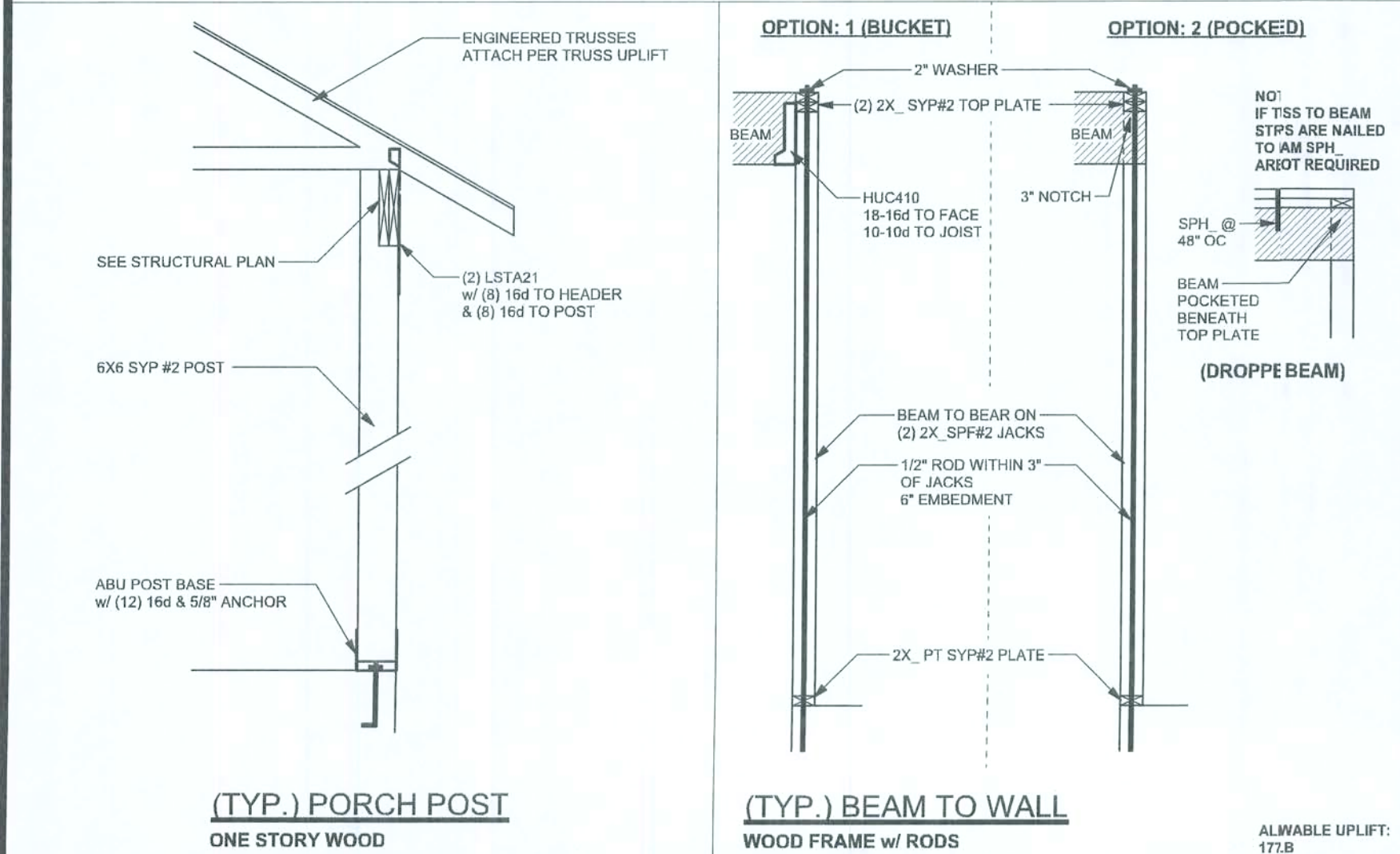
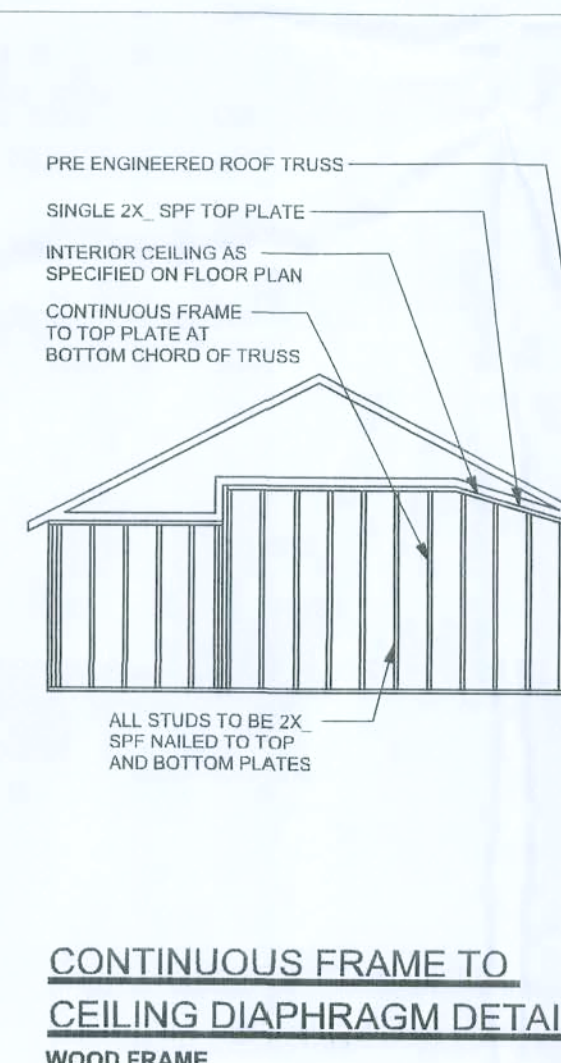
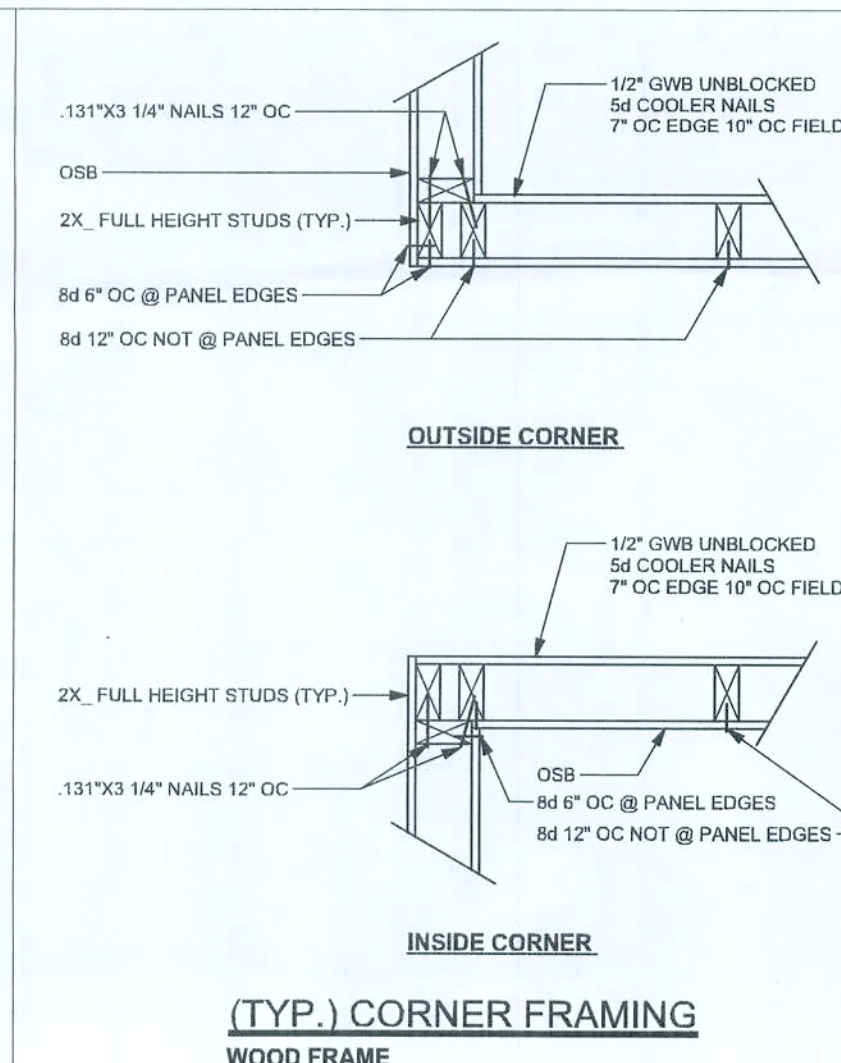
Zone	Effective Wind Area (A _E)	10	15	20
1	19.9	21.8	19.1	19.1
2	19.9	25.5	19.1	21.8
2 Onq		-40.5		-40.5
3	19.9	25.5	19.1	-21.8
3 Onq		-48.3		-42.4
4	21.8	-23.6	19.5	-20.4
5	21.8	29.1	19.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft)		21.8	-29.1	
8x7 Garage Door		19.5	-22.9	
16x7 Garage Door		19.5	-22.6	
20x7 Garage Door		19.5	-22.6	

DESIGN LOADS	
FLOOR: 40 PSF (ALL OTHER DWELLING ROOMS)	
30 PSF (SLEEPING ROOMS)	
30 PSF (ATTICS WITH STORAGE)	
10 PSF (ATTICS WITHOUT STORAGE, <3:12)	
ROOF: 20 PSF (FLAT OR <4:12)	
16 PSF (4:12 TO <12:12)	
12 PSF (12:12 AND GREATER)	
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

NOT IN FLOOD ZONE (BUILDER TO VERIFY)



ALLOWABLE UPLIFT: 1800 LB



REVISIONS

SOFTPLAN

ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:

Mark Disosway, P.E.

No.33915, PCB 865, Lak City, FL 32056, 386-754-5419

DIMENSIONS:

Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION:

I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION:

This design is valid for one building, at specified location.

MARK DISOSWAY

P.E. 056

210009

SEAL

Erkinger Builders

Randall & Marsha Burnham Residence

ADDRESS:

Lot #7, Cobblestone S/D

Columbia County, Florida

Mark Disosway P.E.

P.O. Bo: 868

Lake City, Florida 32056

Phone: (386) 754 - 5419

Fax: (386) 219 - 4871

PRINTED DATE:

July 20, 2009

DRAWN BY:

David Disosway

STRUCTURAL BY:

David Disosway

FINALS DATE:

17 Jul 09

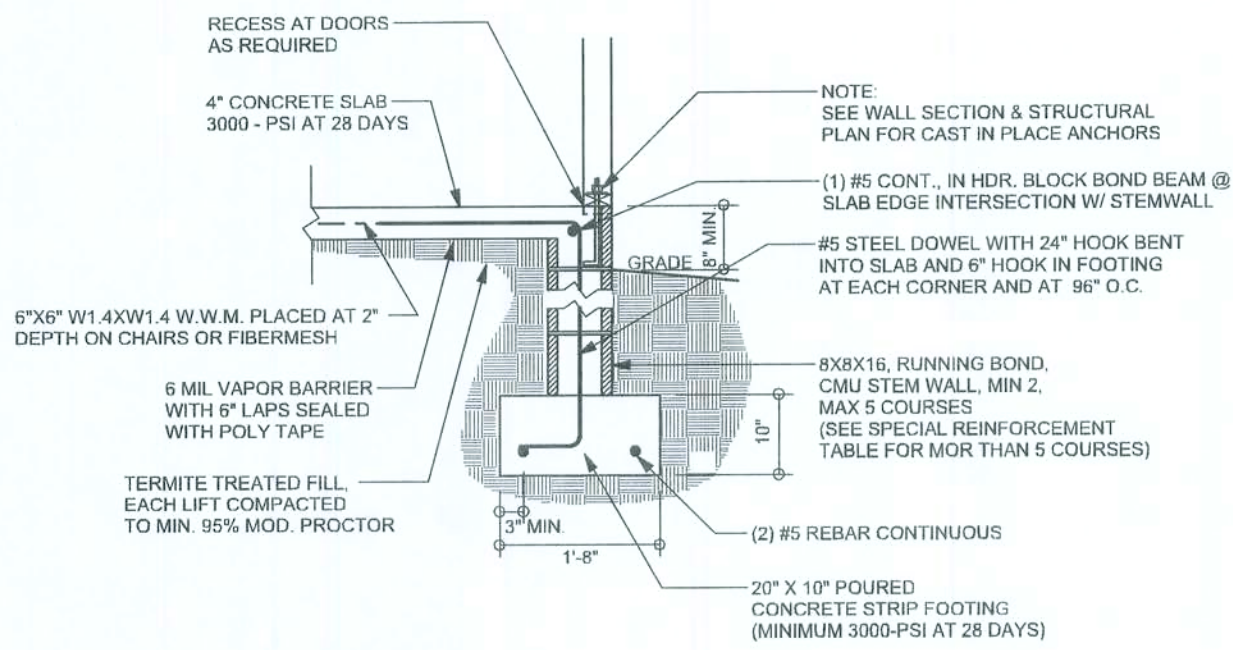
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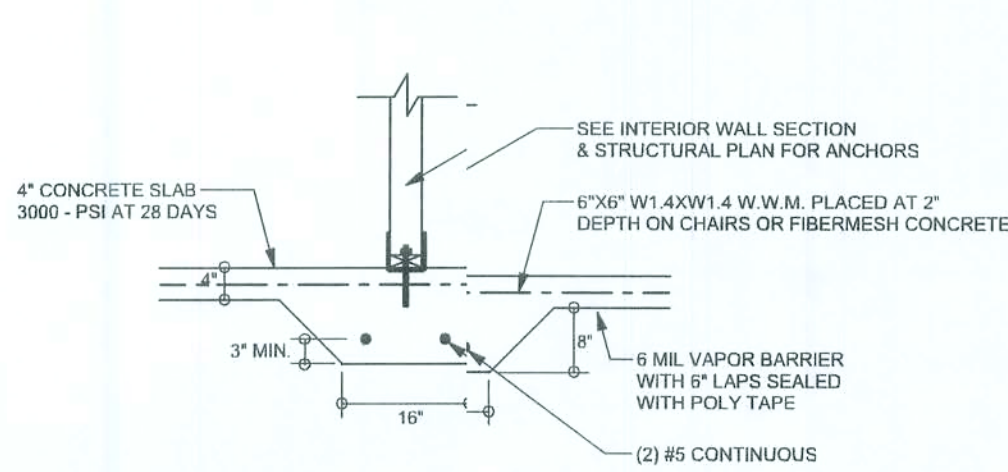
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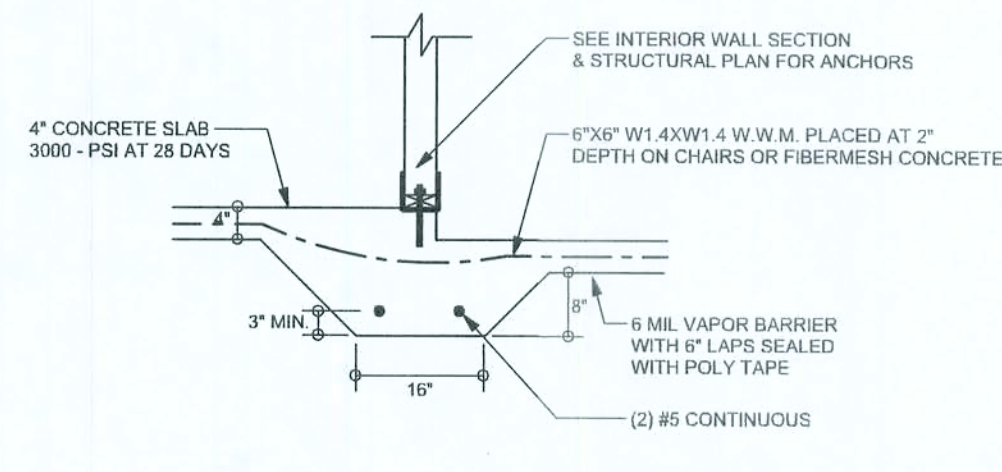
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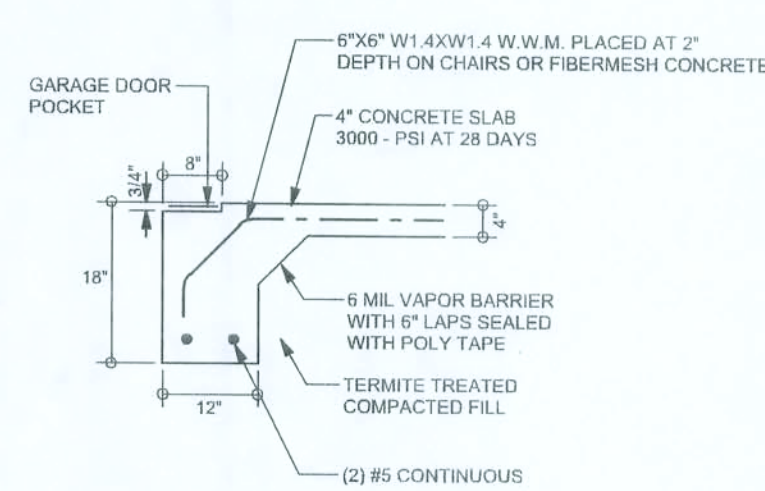
6
S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



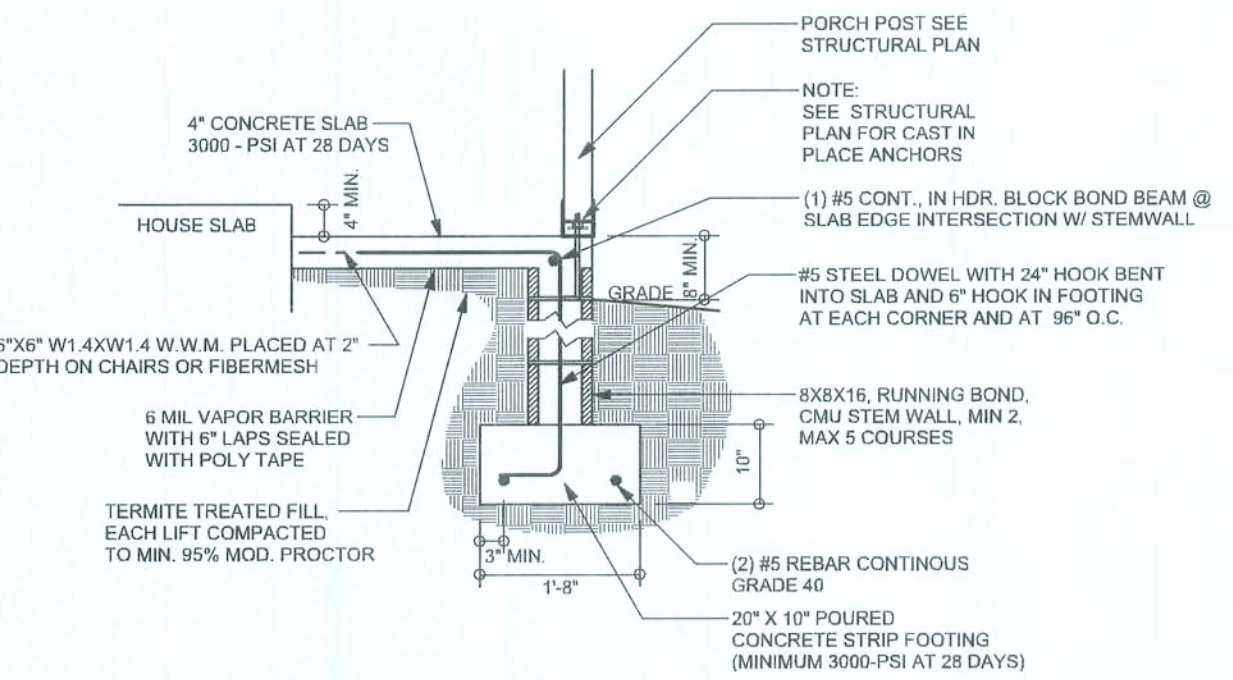
F2
S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F3
S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F4
S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"

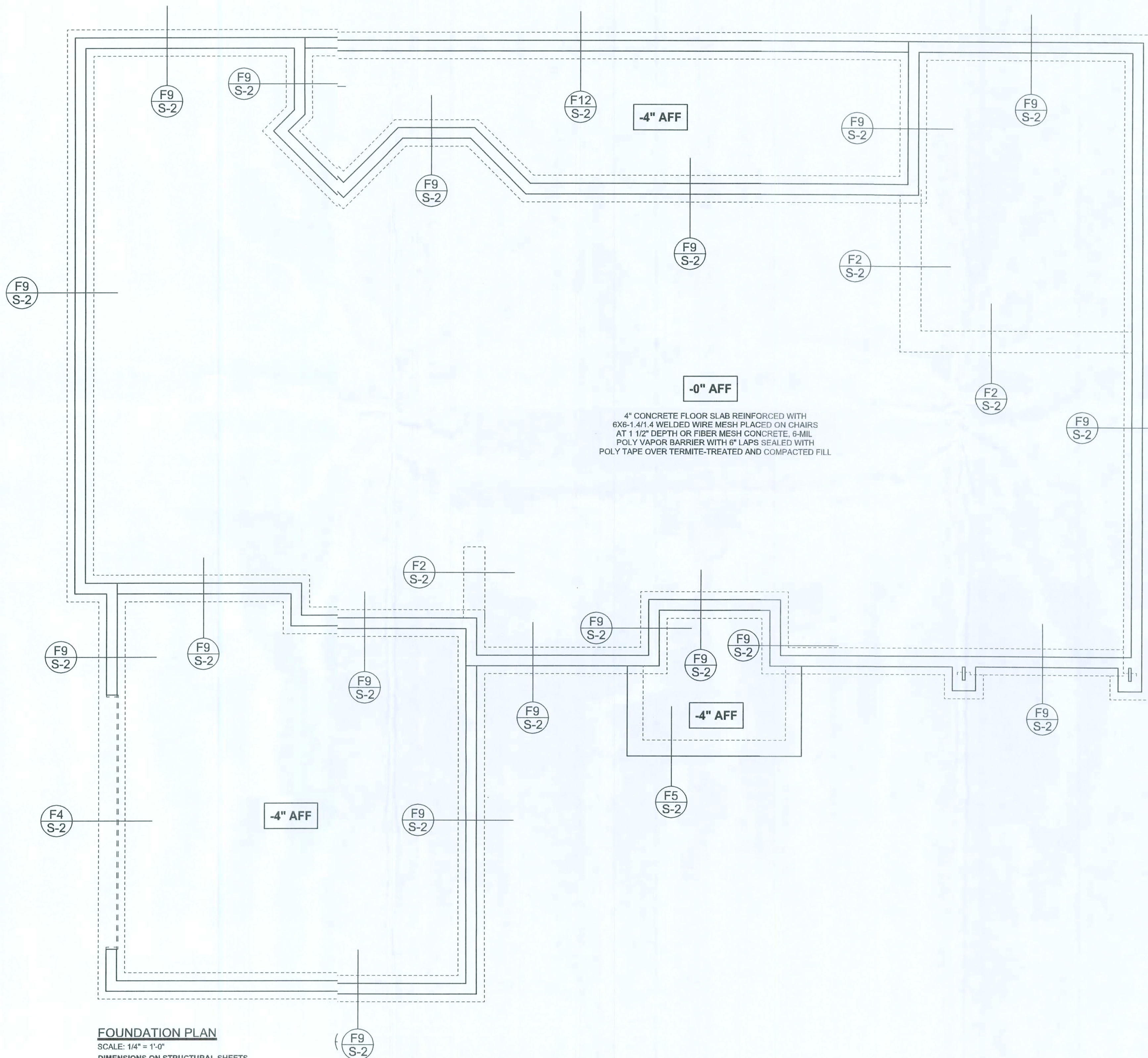


F12
S-2 STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 10' high, add Durowall ladder reinforcement at 10"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS

SOFTWARE
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER
Mark Discoway, P.E.
No. 53515, P.O. Box 868, Lake City, FL 32056
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Discoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2007, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISCOWAY
P.E. 3915

21 Jul 09
DEM

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Randall & Marsha
Burnham Residence

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Columbia County, Florida

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PRINTED DATE:

July 20, 2009

DRAWN BY:
David Discoway

STRUCTURAL BY:
David Discoway

FINALS DATE:
17 Jul 09

JOB NUMBER:
907101

DRAWING NUMBER
S-2

OF 3 SHEETS

REVISIONS

SOFTWARE
ARCHITECTURAL SOFTWARE

WINDLOAD ENGINEER:
Mark Discoway, P.E.
No. 53815, PCB 868, Lake City, FL 32056,
386-754-5419

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S3

OF 3 SHEETS

STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS
SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS
SHALL HAVE (1) JACK STUD & (1) KING STUD
EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS
ARE NOT EXACT. REFER TO ARCHITECTURAL
FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT
LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.
LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03.
BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3
ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED
TRUSS PACKAGE

WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

THREADED ROD LEGEND

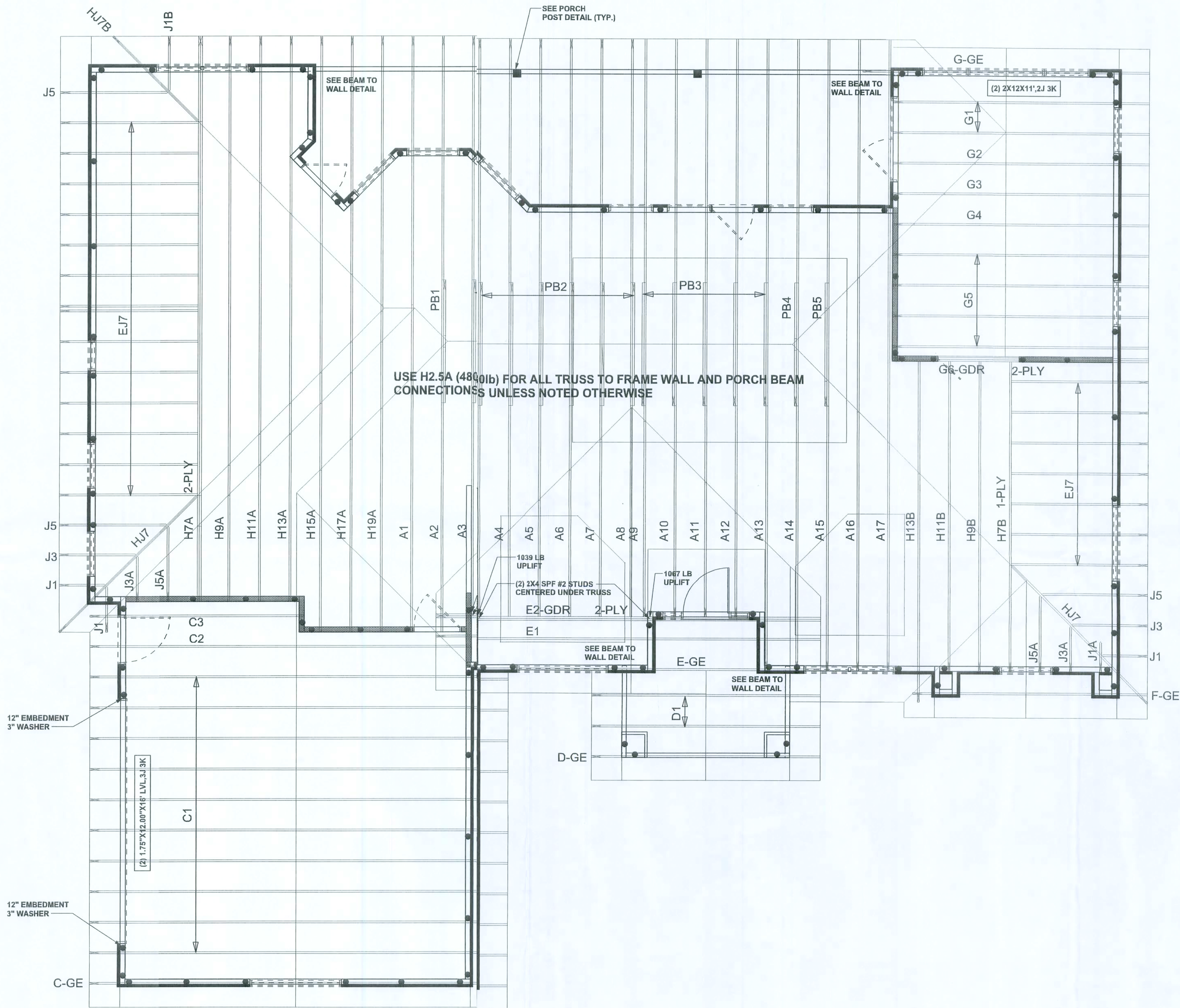
- INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD
- INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER LEGEND

- (2) 2X12X0'-1J 1K
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLYS IN HEADER

TOTAL SHEAR WALL SEGMENTS

	INDICATES SHEAR WALL SEGMENTS	REQUIRED	ACTUAL
TRANSVERSE		44.5'	84.7'
LONGITUDINAL		38.2'	53.0'



STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

CONNECTIONS, WALL & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER, ANDERSON TRUSS CO.
JOB #8-152