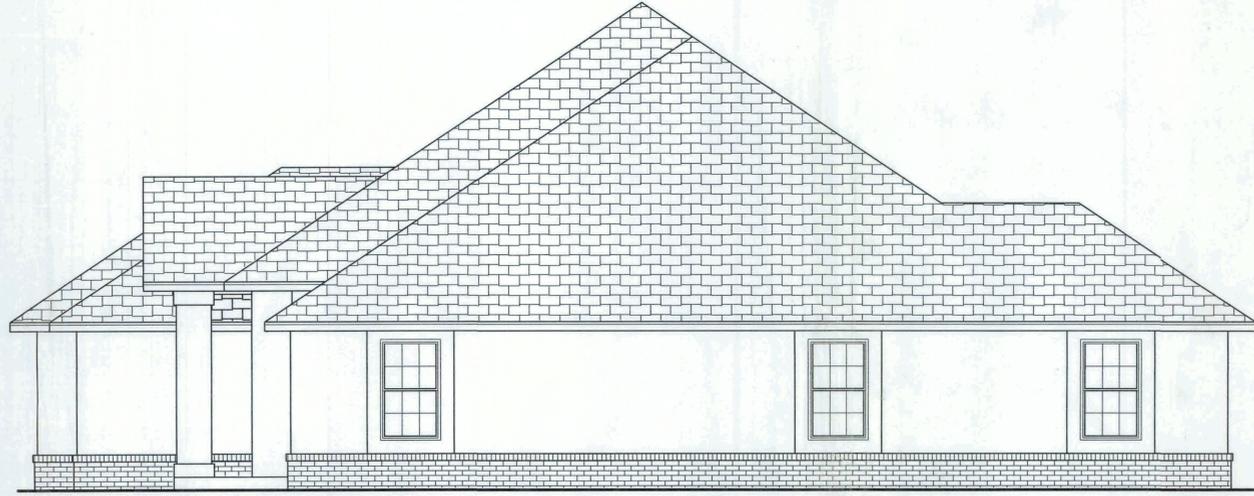




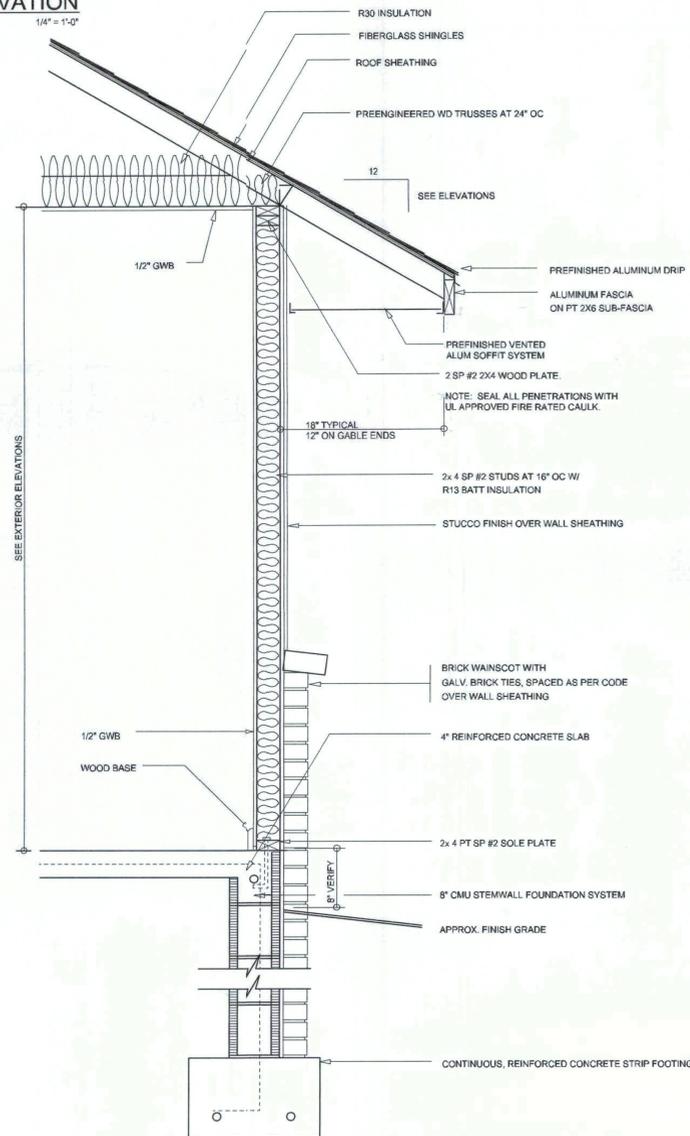
**LEFT ELEVATION**

SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**

SCALE: 1/4" = 1'-0"



**TYP. WAINSCOT WALL SECTION**

SCALE: 1" = 1'-0"



**REAR ELEVATION**

SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**

SCALE: 1/4" = 1'-0"

REVISIONS  
April 10, 2006



EXTERIOR ELEVATIONS  
SCALE: 1/4" = 1'-0"

TYPICAL WALL SECTION  
SCALE: 1" = 1'-0"

A SPEC. HOUSE FOR:

**SHIRLEY HITSON**

PROJECT ADDRESS: LOT 51 EMERALD COVE, LAKE CITY, FLORIDA 32065

© WILLIAM MYERS DESIGN  
P.O. BOX 1513  
LAKE CITY, FL 32065  
(386) 758-8406  
itsours@bellsouth.net



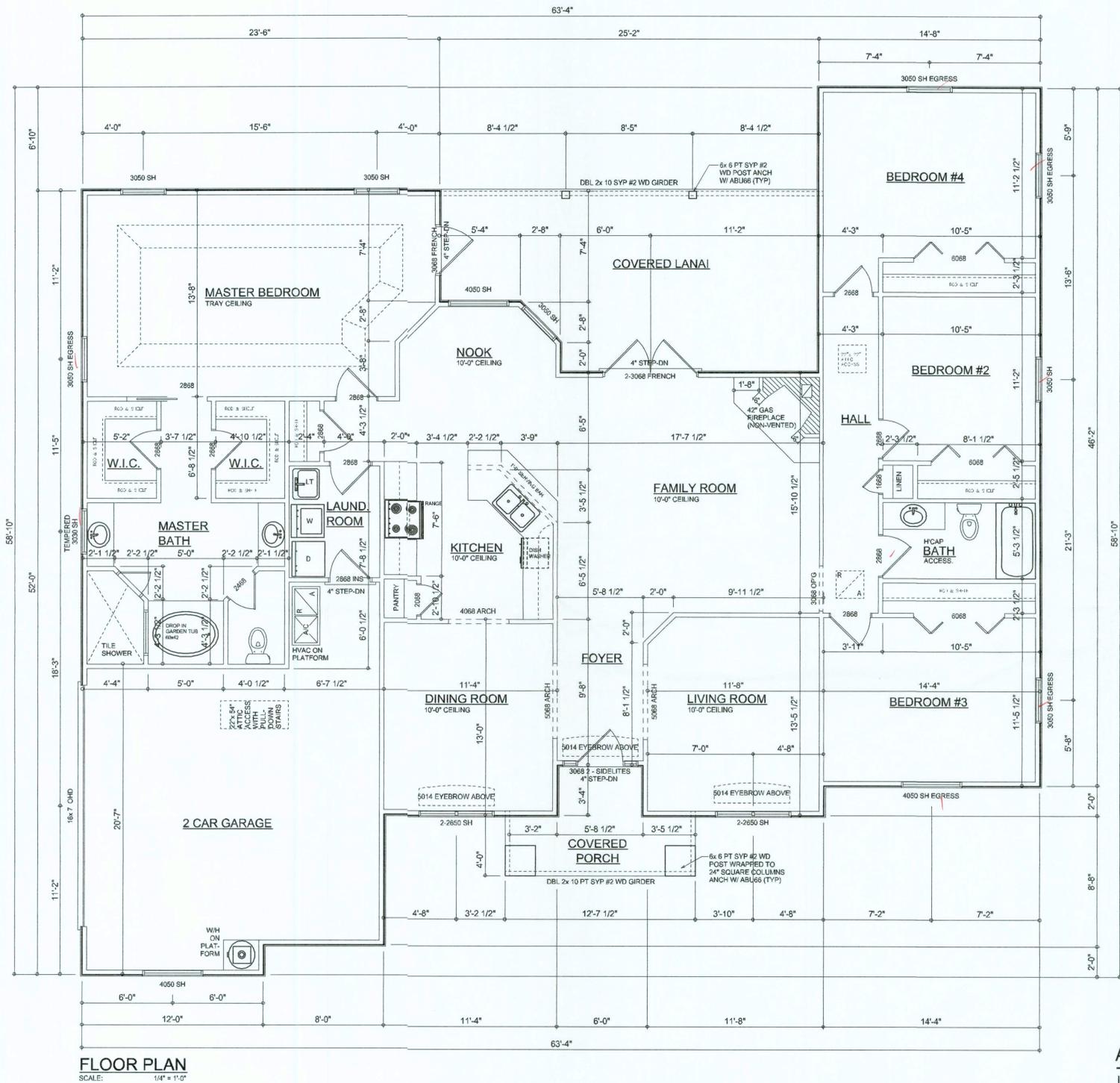
JOB NUMBER  
060318

SHEET NUMBER

**A.1**  
OF 3 SHEETS

*William Myers*

OFFICE COPIE



**FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

**Garage fire separations shall comply with the following:**

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 1 3/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.

**AREA SUMMARY**

LIVING AREA	2188	S.F.
GARAGE AREA	420	S.F.
COVERED PORCH AREA	262	S.F.
ENTRY PORCH AREA	86	S.F.
<b>TOTAL AREA</b>	<b>2956</b>	<b>S.F.</b>

REVISIONS

1	APRIL 10, 2006
---	----------------



**FLOOR PLAN**  
SCALE: 1/4" = 1'-0"

A SPEC. HOUSE FOR:  
**SHIRLEY HITSON**  
PROJECT ADDRESS: LOT 5, EMERALD COVE, LAKE CITY, FLORIDA 32055

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P.O. BOX 1513  
LAKE CITY, FL 32056  
(386) 758-8406  
myours@bellsouth.net



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**060318**

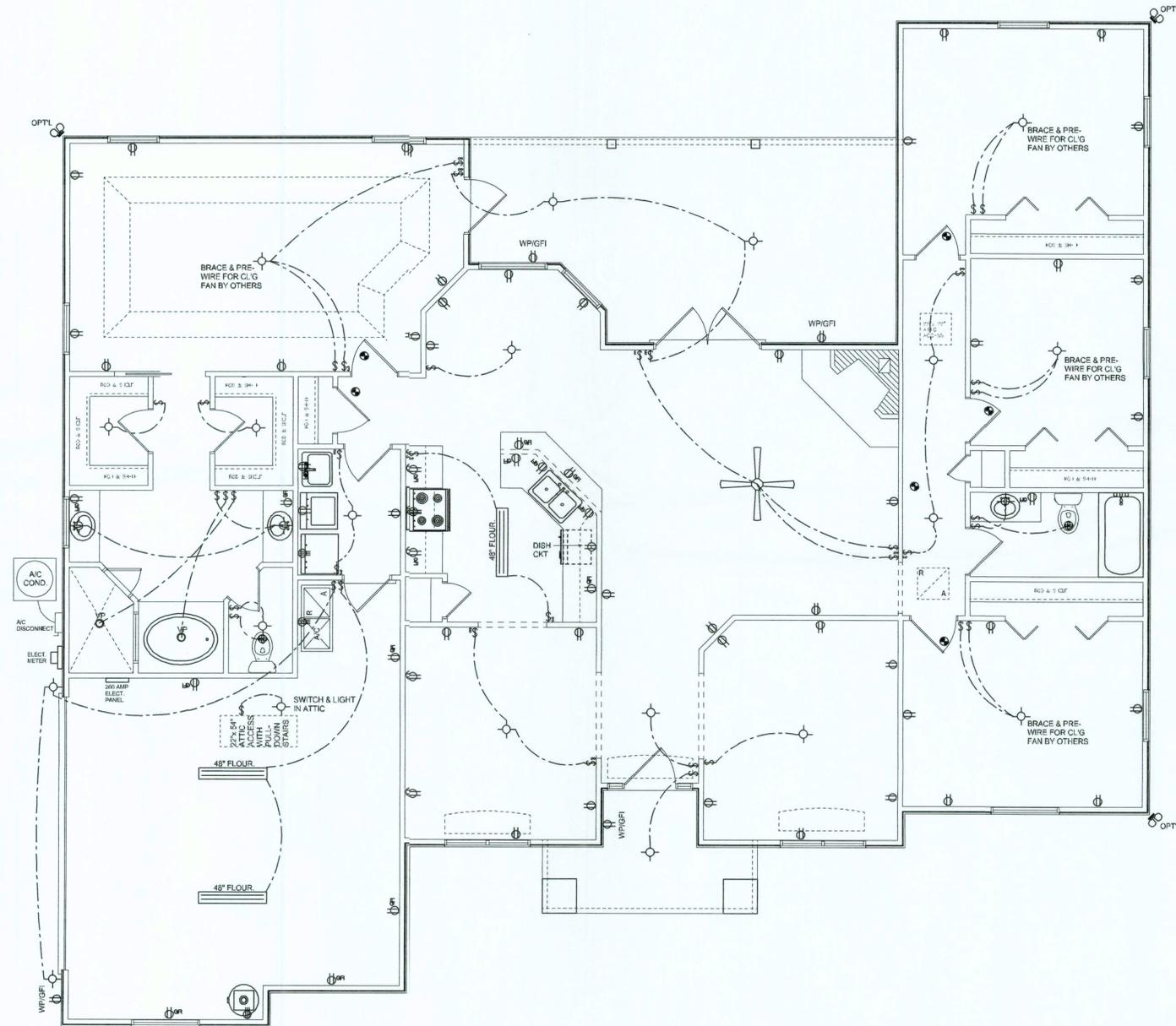
SHEET NUMBER  
**A.3**  
OF 3 SHEETS

*William Myers*

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	48' FLOOR, 2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:  
 ALL BEDROOM RECEPTACLES SHALL BE AFCI  
 (ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTORS SHALL HAVE BATTERY BACKUP POWER  
 AND ALL WIRED TOGETHER SO IF ANY ONE UNIT'S ACTUATED THEY  
 ALL ACTIVATE.



**ELECTRICAL PLAN**  
 SCALE: 1/4" = 1'-0"

REVISIONS
April 10, 2006

**SOFTPLAN**  
 ARCHITECTURAL DESIGN SOFTWARE

**ELECTRICAL PLAN**  
 SCALE: 1/4" = 1'-0"

A SPEC. HOUSE FOR:  
**SHIRLEY HITSON**  
 PROJECT ADDRESS: LOT 5, EMERALD COVE, LAKE CITY, FLORIDA, 32055

© WILLIAM MYERS  
 DESIGN  
 P.O. BOX 1513  
 LAKE CITY, FL 32055  
 (386) 758-8406  
 wmyers@bellsouth.net



JOB NUMBER  
 060318

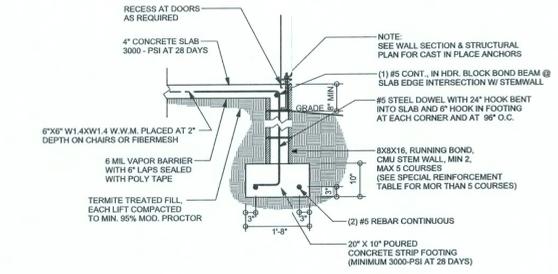
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**A.4**  
 OF 3 SHEETS

*William Myers*

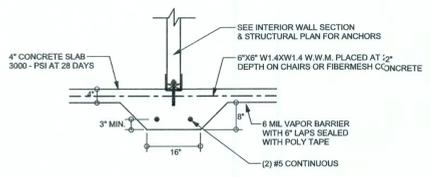


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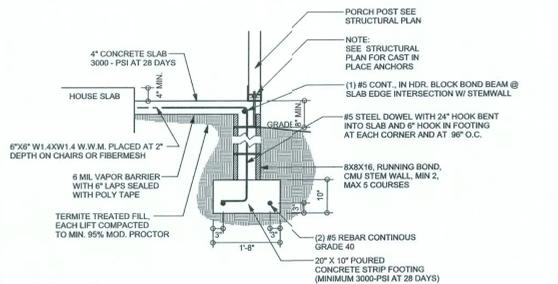
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ARCHITECTURAL DESIGN SOFTWARE



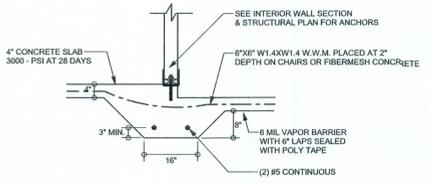
**F9 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



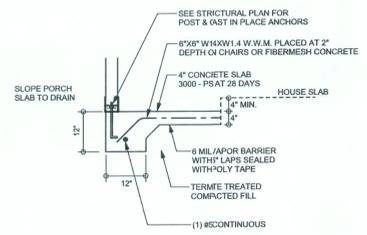
**F2 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



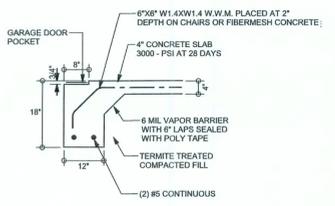
**F12 ALT. STEM WAL PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



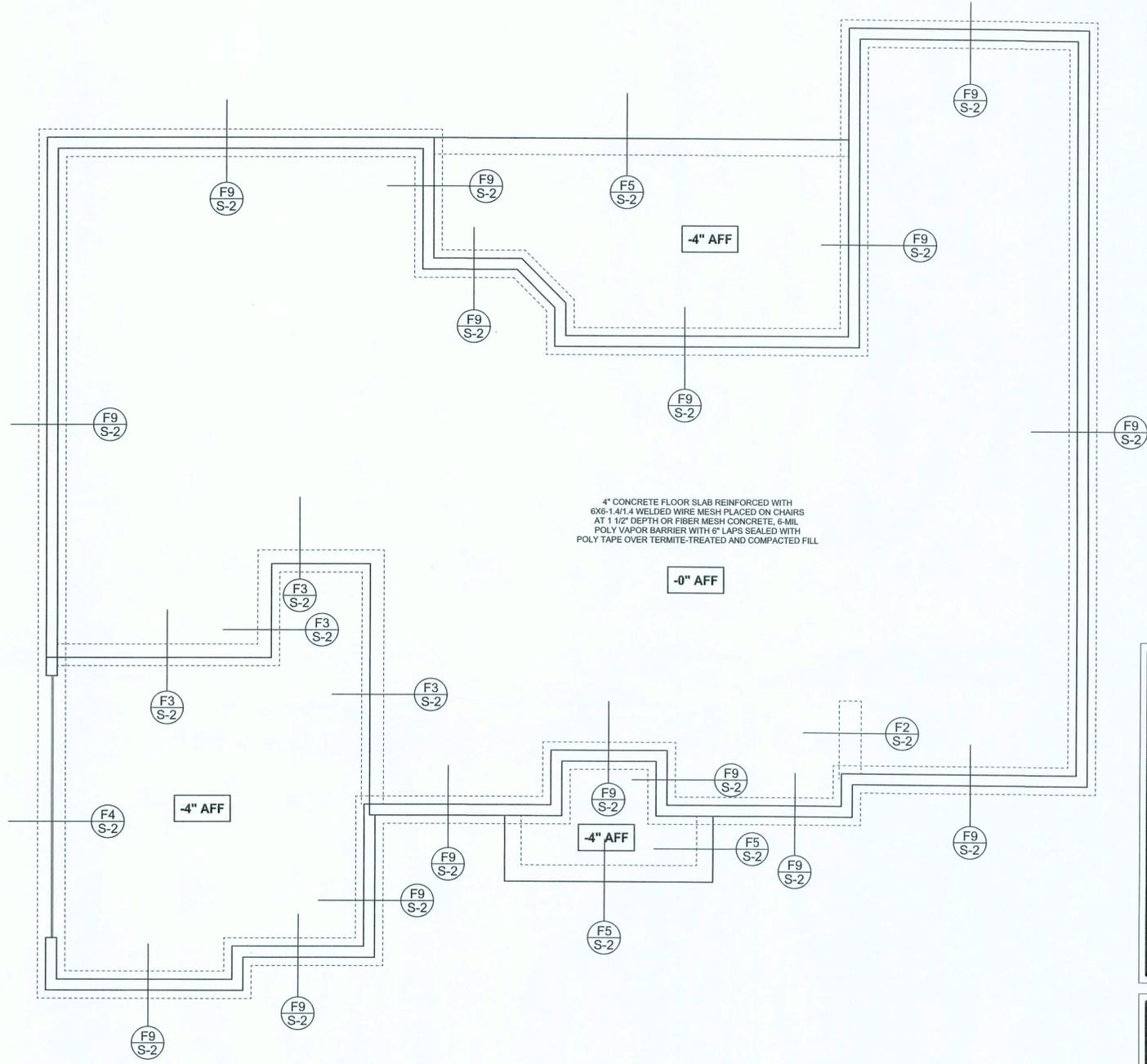
**F3 INTERIOR BEARING STEP FOOTING**  
SCALE: 1/2" = 1'-0"



**F5 PORCH FOOTING**  
SCALE: 1/2" = 1'-0"



**F4 GARAGE DOOR FOOTING**  
SCALE: 1/2" = 1'-0"



**FOUNDATION PLAN**  
SCALE: 1/4" = 1'-0"  
DIMENSIONS ON STRUCTURAL SHEETS  
ARE NOT EXACT. REFER TO ARCHITECTURAL  
FLOOR PLAN FOR ACTUAL DIMENSIONS

**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in footing and bent 34" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Diaphragm ladder reinforcement at 16" vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher part of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 4" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

WINDLOAD ENGINEER: Mark Disoway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOWAY  
P.E. 53915

*Mark Disoway*  
13 APR 06  
SEAL

Shirley Hitson

Spec House  
Lot 5  
Emerald Cove S/D

ADDRESS:  
Lot 5 Emerald Cove S/D  
Columbia County, Florida

Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
April 13, 2006

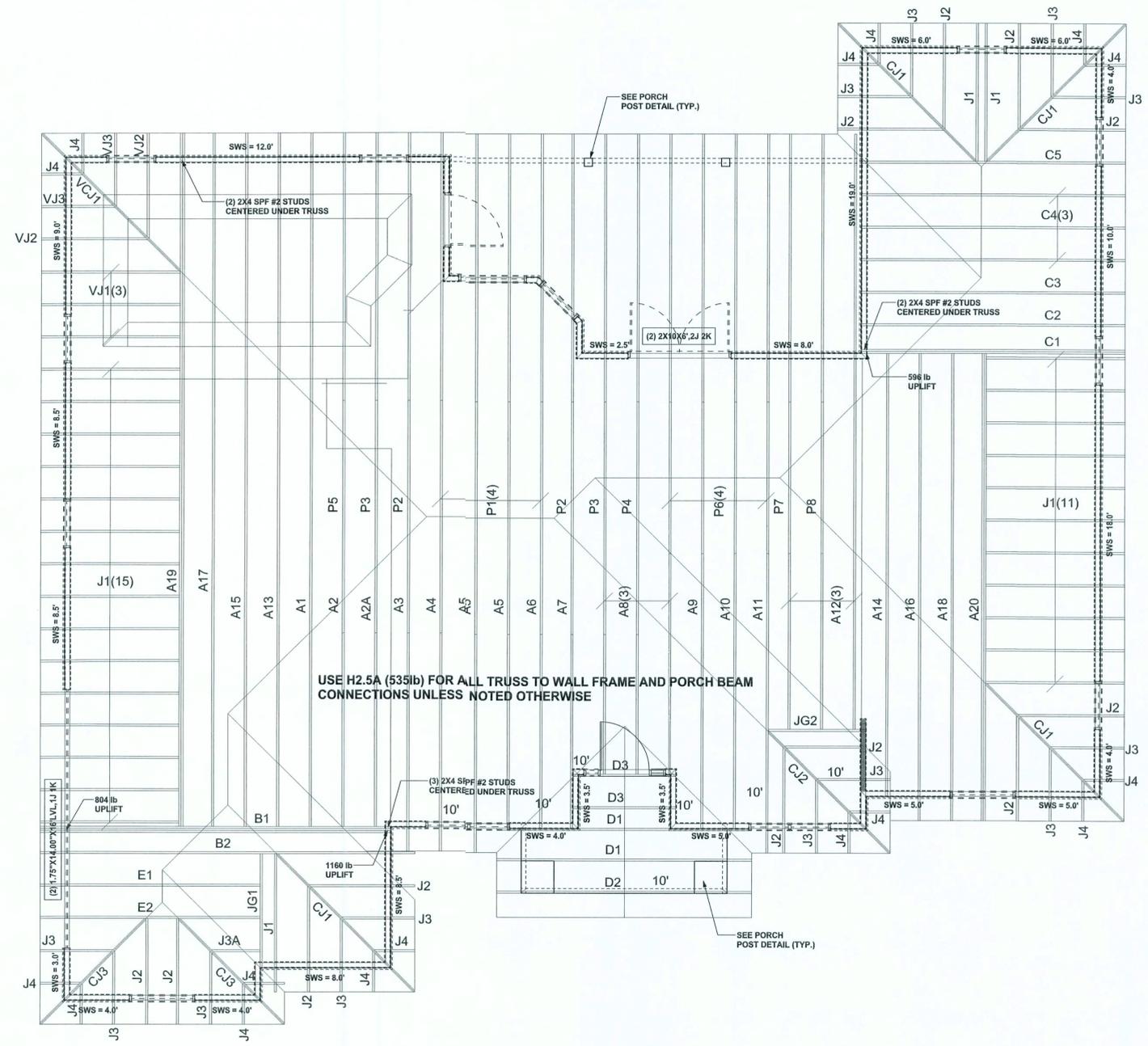
DRAWN BY: David Disoway      CHECKED BY:

FINAL DATE:  
13 / Apr / 06

JOB NUMBER:  
603203

DRAWING NUMBER  
**S-2**  
OF 3 SHEETS

REVISIONS	



**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

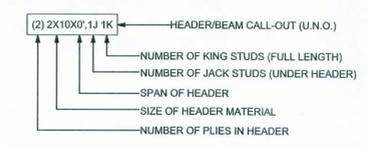
**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**WALL LEGEND**

	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

**HEADER LEGEND**



**TOTAL SHEAR WALL SEGMENTS**  
SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	32.5'	99.5'
LONGITUDINAL	28.4'	69.5'

CONNECTIONS, WALL & HEADER DESIGNIS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. MAYO TRUSS CO. JOB #HITSONLOTS

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419  
DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.  
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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.  
LIMITATION: This design is valid for one building, at specified location.

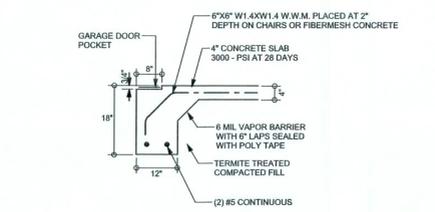
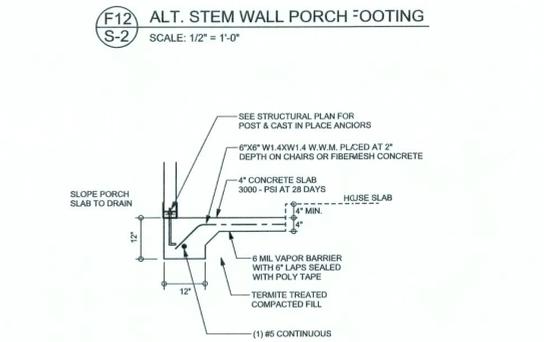
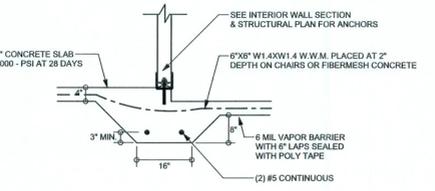
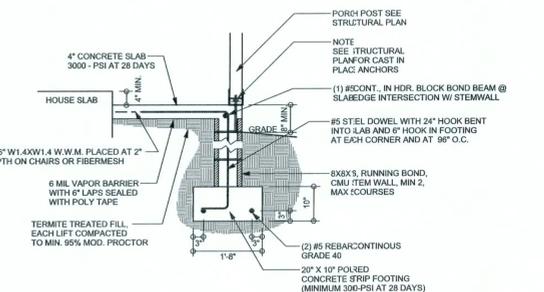
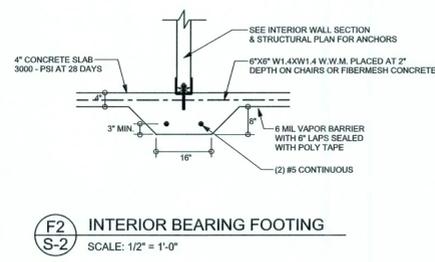
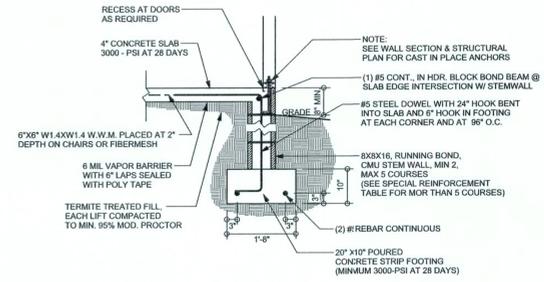
MARK DISOSWAY  
P.E. 53915  
*Mark Disosway*  
3/24/2006  
SEAL

**Shirley Hitson**  
Spec House  
Lot 5  
Emerald Cove S/D  
ADDRESS:  
Lot 5 Emerald Cove S/D  
Columbia County, Florida  
Mark Disosway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871  
PRINTED DATE:  
April 13, 2006  
DRAWN BY: David Disosway  
CHECKED BY:  
FINALS DATE:  
13 / Apr / 06  
JOB NUMBER:  
603203  
DRAWING NUMBER  
**S-3**  
OF 3 SHEETS



REVISIONS	

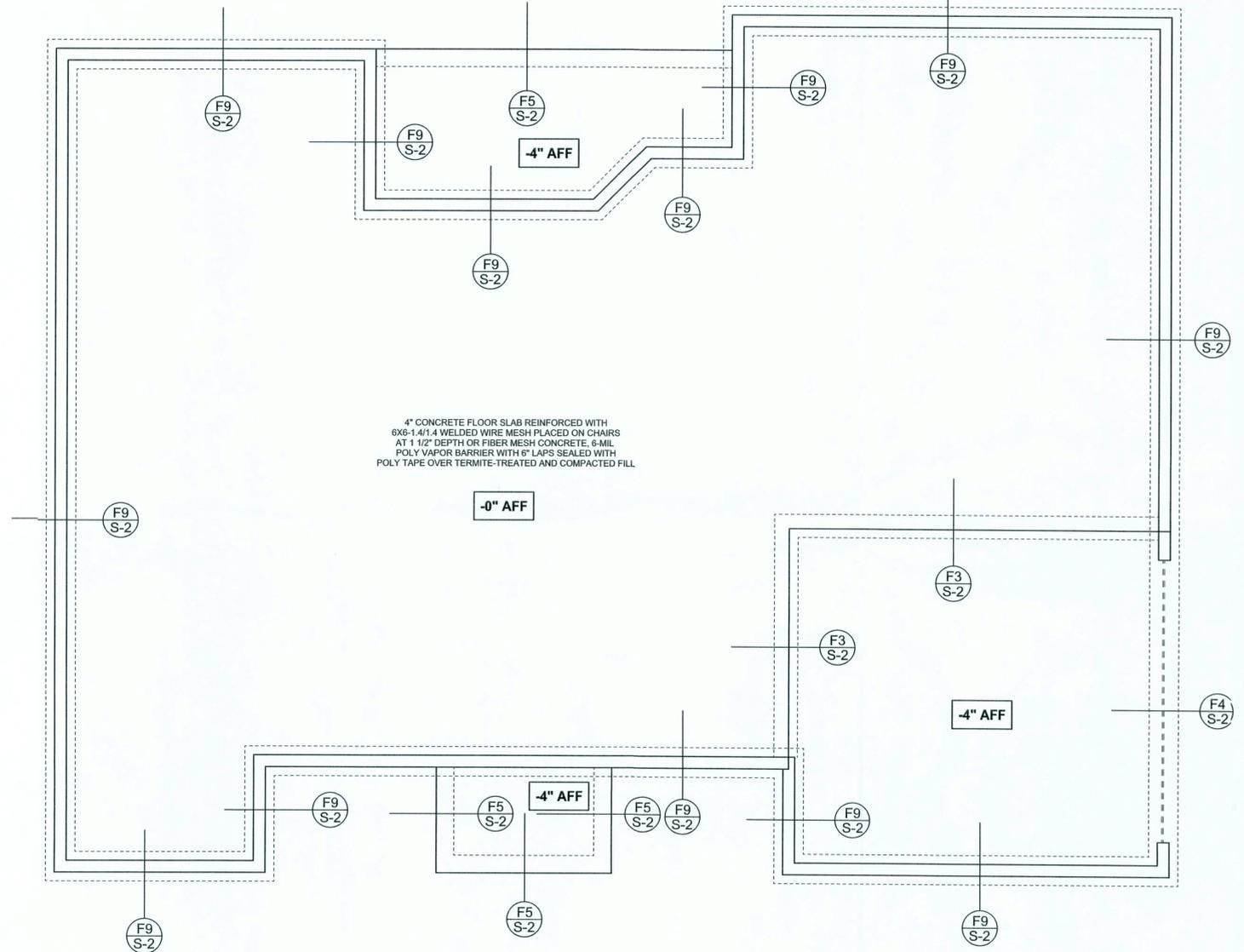
**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE



**TALL STEM WALL TABLE**

The table assumes 80 ksi reinforcing bars with 6" hook in the footing and bent 4" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall. If the wall is over 8' high, add Durowall backer reinforcement at 16" O.C. vertically or a horizontal bond beam with #5 continuous at mid height. For higher parts of the wall 12" CMU say be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



WINDLOAD ENGINEER: Mark Disosway  
P.E. No. 53915, PGB 868, Lake City, FL  
32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 53915

*Mark Disosway*  
160606  
SEAL

**Shirley Hitson**

Spec House  
Lot #5  
Emerald Cove S/D

ADDRESS:  
Lot #5 Emerald Cove S/D  
Columbia County, Florida

Mark Disosway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
August 16, 2006

DRAWN BY: David Disosway      CHECKED BY:

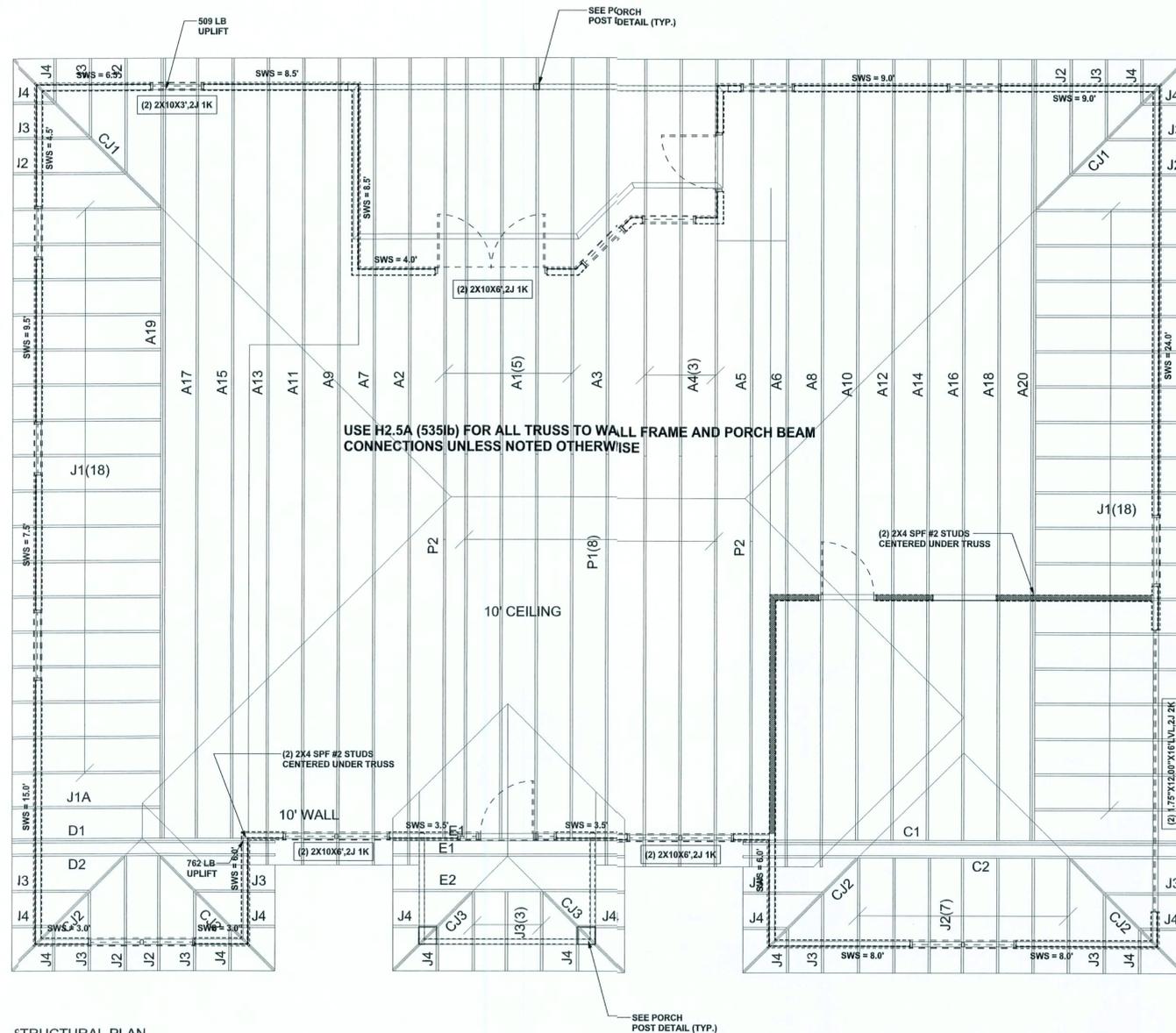
FINAL DATE:  
16 / Aug / 06

JOB NUMBER:  
603202a

DRAWING NUMBER  
**S-2**  
OF 3 SHEETS

REVISIONS	

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE



STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

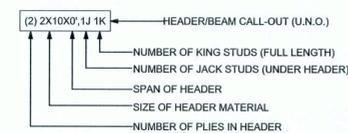
STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	36.0'	81.0'
LONGITUDINAL	32.5'	66.0'

WINDLOAD ENGINEER: Mark Disosway,  
P.E. No. 53915, PCB 868, Lake City, FL  
32056; 386-754-5419

DIMENSIONS:  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 53915

*Mark Disosway*  
16 AUG 2006  
SEAL

Shirley Hitton

Spec House  
Lot 185  
Emerald Cove S/D

ADDRESS:  
Lot 185 Emerald Cove S/D  
Columbia County, Florida

Mark Disosway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE:  
August 16, 2006

DRAWN BY: David Disosway  
CHECKED BY:

FINALS DATE:  
16 / Aug / 06

JOB NUMBER:  
603202a

DRAWING NUMBER

S-3

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, MAYO TRUSS CO.  
JOB #HITSONLOT18

**REVISIONS**




**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS ENGINEERING FULLY SATISFIES ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $F_c = 3000$  PSI.

WELODED WIRE REINFORCED SLAB: 4" x 6" W/ 4" x W/ 4, FB = 89KSI, WELDED WIRE REINFORCEMENT FABRIC (W/ 1" x 1" CONCRETE TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 12 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1118. SUPPLIER TO PROVIDE ASTM C 1118 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12 FT. DO NOT CUT W/M OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCLOSE THE SLAB TO CRACK ON A GIVEN LINE.

REBAR: ASTM A615, GRADE 60, DEFORMED BARS,  $F_y = 60$  KSI. ALL LAP SPLICES @ 1'0" (25" FOR BAR BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP,  $F_b = 2460$  PSI,  $E = 1800000$  UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCULATIONS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURER'S AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE MINIMUM LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE OR 15" IN GROUDED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 3/16"; WITH 5/8" BOLTS TO BE 3" x 3" x 3/16"; WITH 3/4" BOLTS TO BE 3" x 3" x 3/16".

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

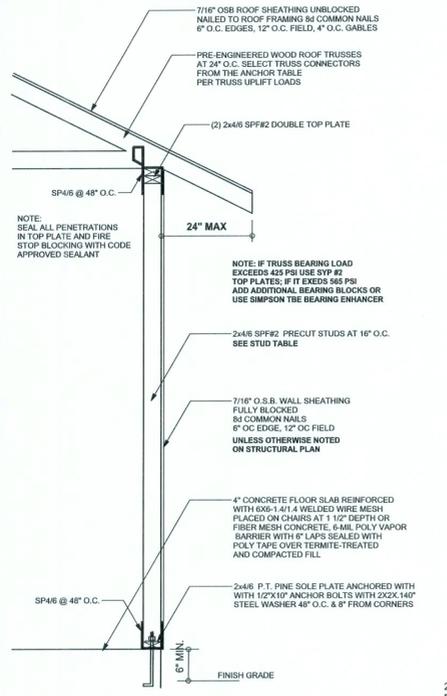
**ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

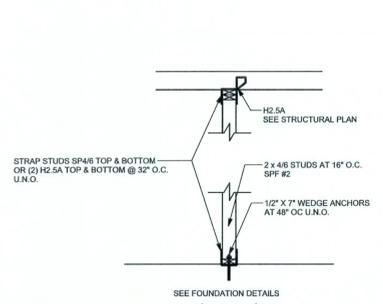
UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 650	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H18-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H18-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 660	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LG2	14-16d	14-16d	
		<b>HEAVY GIRDER TIEDOWNS*</b>			<b>TO FOUNDATION</b>
< 3965	< 3330	MGT		22-10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16-10d	2-5/8" THREADED ROD 12" EMBEDMENT
		<b>STUD STRAP CONNECTOR*</b>			<b>TO STUDS</b>
< 435	< 435	SSP DOUBLE TOP PLATE	3-10d		4-10d
< 455	< 420	SSP SINGLE SILL PLATE	1-10d		4-10d
< 825	< 825	DSP DOUBLE TOP PLATE	8-10d		8-10d
< 825	< 600	DSP SINGLE SILL PLATE	2-10d		8-10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	C20	18-8d		
< 1705	< 1705	CS16	28-8d		
		<b>STUD ANCHORS*</b>			<b>TO FOUNDATION</b>
< 1350	< 1305	LTT19	8-16d		12" AB
< 2310	< 2310	LTT01	18-10d, 1 1/2"		12" AB
< 2775	< 2570	H02A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18-16d		5/8" AB
< 1400	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		12" AB
< 2300	< 2300	ABU66	12-16d		12" AB
< 2320	< 2320	ABU88	18-16d		2-5/8" AB

**GRADE & SPECIES TABLE**

	Fb (psi)	E (10 <sup>6</sup> psi)	
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

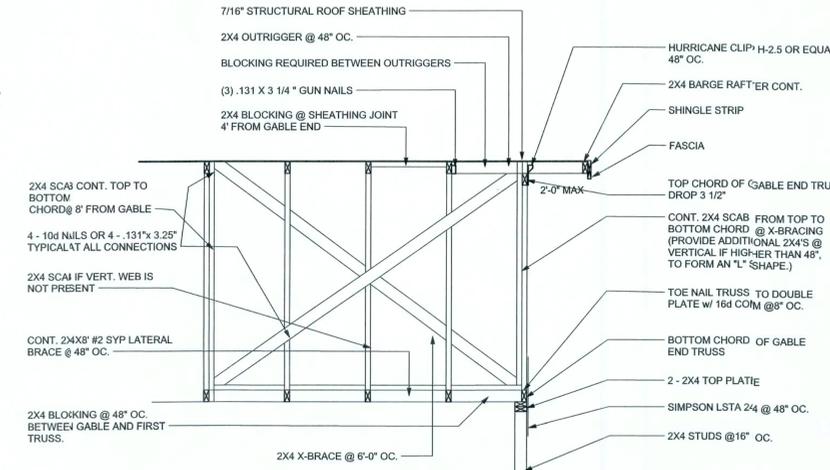


**INTERIOR BEARING WALL**  
SCALE: 1/2" = 1'-0"

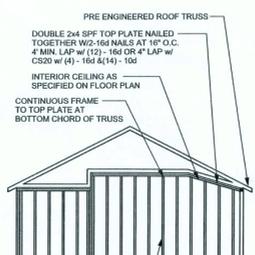
**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

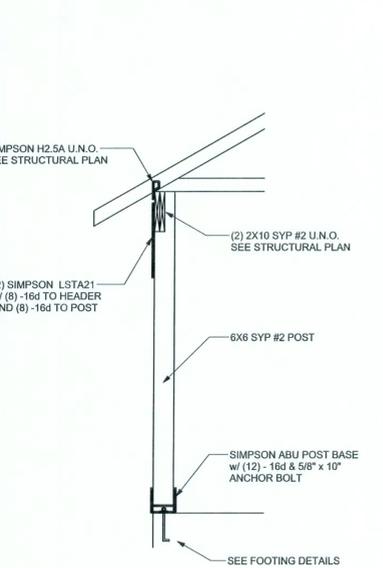
THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR WIND LOADS (12" MIN EXPOSURE) STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE: 18" O.C. x 0.85 = 15" O.C.



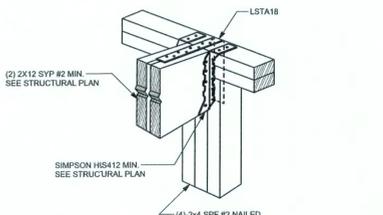
**TYPICAL GABLE END (X-BRACING)**  
ALL MEMBERS SHALL BE SYP



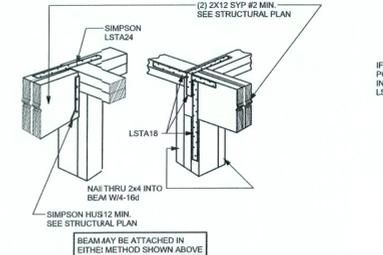
**CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL**  
SCALE: N.T.S.



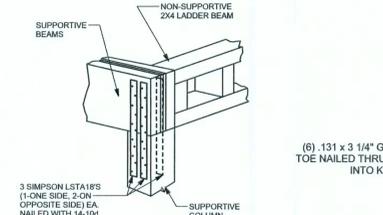
**TYPICAL PORCH POST DETAIL**  
SCALE: 1/2" = 1'-0"



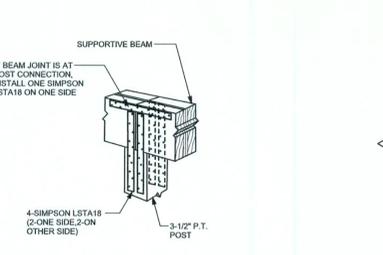
**BEAM-WALL CONNECTION DETAIL**  
SCALE: N.T.S.



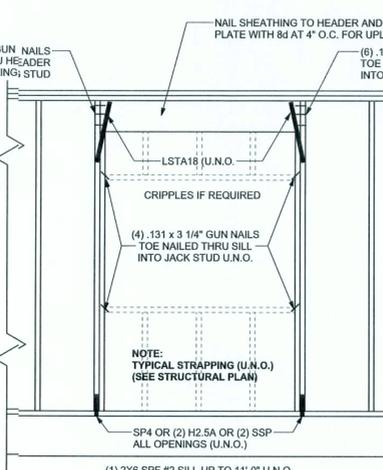
**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.



**SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.



**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.



**TYPICAL HEADER STRAPPING DETAIL**  
SCALE: 1/2" = 1'-0"

**MASONRY NOTES:**

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1-02 AND ACI 530.2-02). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI 530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls $F_m = 1500$ psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, $F_y = 60$ ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/R2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wet soils, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/R2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

**DESIGN DATA**

**WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1**  
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT. NOT ON UPPER HALF OF HILL OR ESCARPMENT 60 FT IN EXP. B, 30 FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)  
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE  
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION  
BASIC WIND SPEED = 110 MPH  
WIND EXPOSURE = B  
WIND IMPORTANCE FACTOR = 1.0  
BUILDING CATEGORY = II  
ROOF ANGLE = 10-45 DEGREES  
MEAN ROOF HEIGHT = <30 FT  
INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)  
COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft <sup>2</sup> )	10	100
1	19.9	21.8	18.1
2	19.9	25.5	18.1
2.0th		40.6	40.6
3	19.9	25.5	18.1
3.0th		68.3	42.4
4	21.8	23.6	18.5
5	21.8	29.1	18.5
Doors & Windows		21.8	29.1
Worst Case (Zone 5, 10 ft <sup>2</sup> )			
8x7 Garage Door		19.5	22.9
16x7 Garage Door		18.5	21.0

**DESIGN LOADS**  
FLOOR: 40 PSF (ALL OTHER DWELLING ROOMS)  
30 PSF (SLEEPING ROOMS)  
30 PSF (ATTICS WITH STORAGE)  
10 PSF (ATTICS WITHOUT STORAGE, <3-12)  
ROOF: 20 PSF (FLAT OR <4-12)  
16 PSF (4-12 TO <12-12)  
12 PSF (12-12 AND GREATER)  
STAIRS: 40 PSF (ONE & TWO FAMILY DWELLINGS)  
SOIL BEARING CAPACITY: 1000 PSF  
NOT IN FLOOD ZONE (BUILDER TO VERIFY)

WINDLOAD ENGINEER: Mark Disoway, PE No. 53915, P.O. Box 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

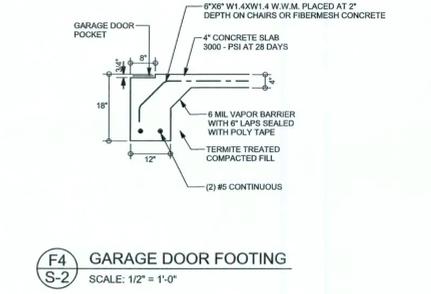
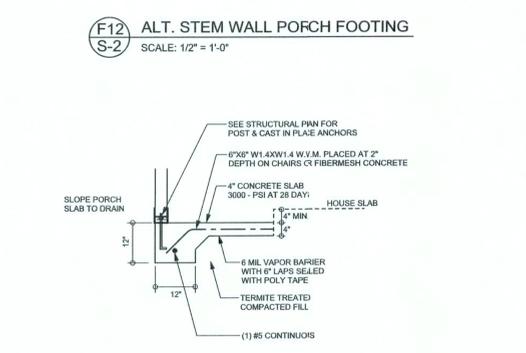
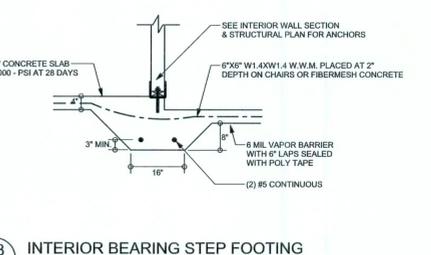
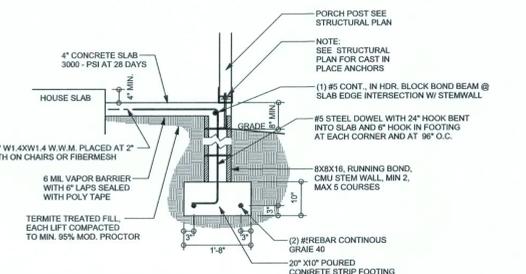
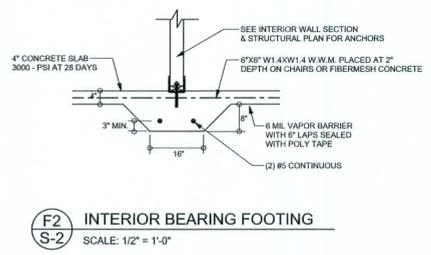
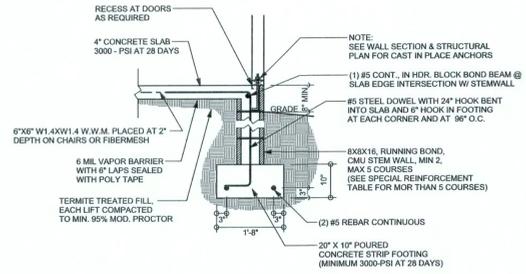
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CERTIFICATION: I hereby certify that I have examined this plan, relating to wind engineering, and that the applicable portions of the plan, relating to wind engineering, comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.  
LIMITATION: This design is valid for one building, at specified location.  
MARK DISOWAY  
P.E. 53915  
10/18/06  
SEAL

**Shirley Hitson**  
Spec House  
Emerald Cove S/D  
ADDRESS:  
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Columbia County, Florida  
Mark Disoway P.E.  
P.O. Box 868  
Lake City, Florida 32056  
Phone: (386) 754 - 5419  
Fax: (386) 269 - 4871

PRINTED DATE: August 16, 2006  
DRAWN BY: David Disoway  
CHECKED BY:  
FINAL DATE: 16 / Aug / 06  
JOB NUMBER: 603202a  
DRAWING NUMBER  
**S-1**  
OF 3 SHEETS

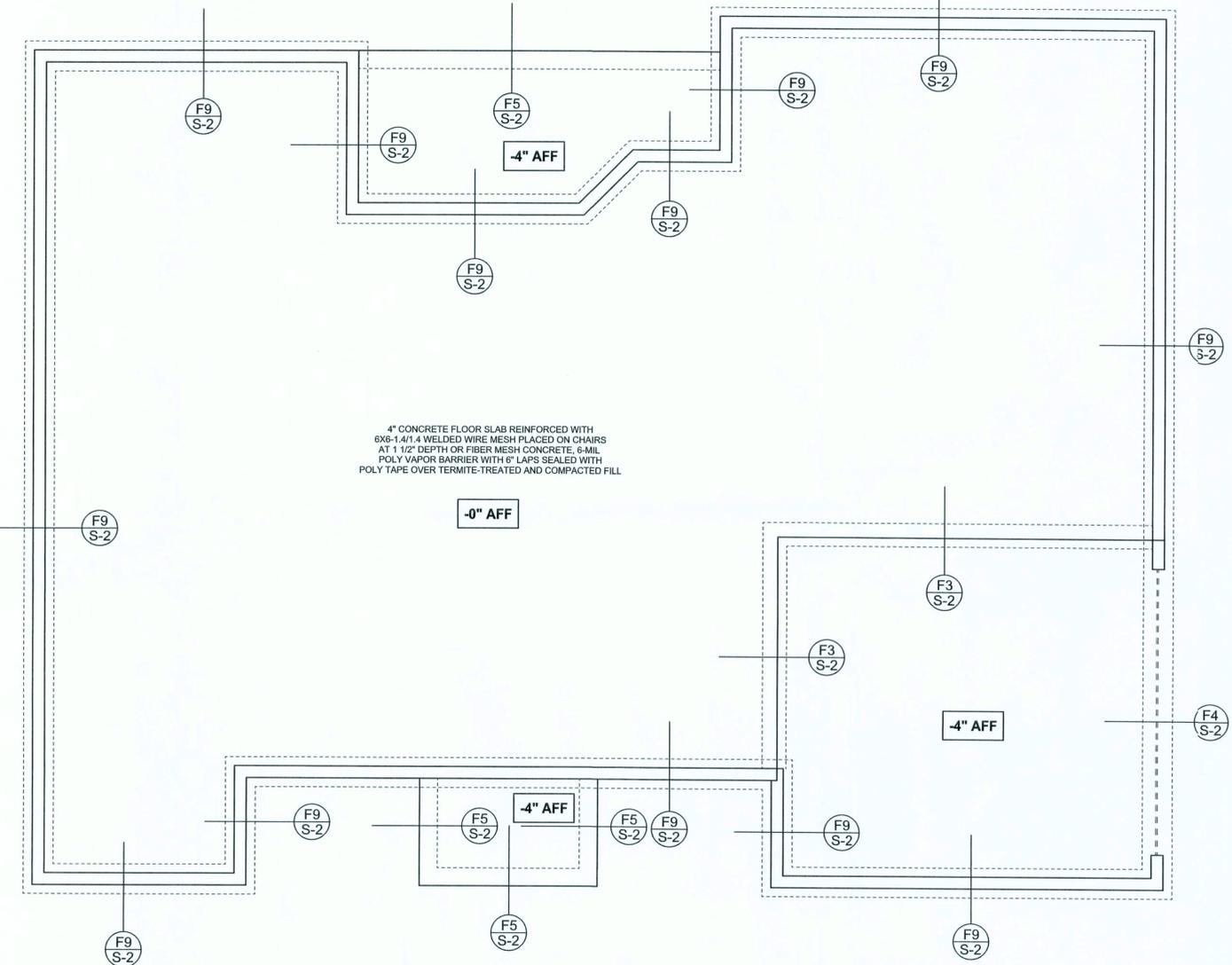
REVISIONS	



**TALL STEM WALL TABLE**

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the exterior side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Darnwall ladder reinforcement at 1800 vertically or a horizontal bond beam with #8 continuous at mid height. For higher parts of the wall 1" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, PCB 866, Lake City, FL 32056, 386-754-5419

**MARK DISOSWAY**  
P.E. 53915  
*Mark Disosway*  
10/16/06  
SEAL

**Shirley Hitson**  
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Emerald Cove S/D

ADDRESS:  
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Columbia County, Florida

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PRINTED DATE:  
August 16, 2006

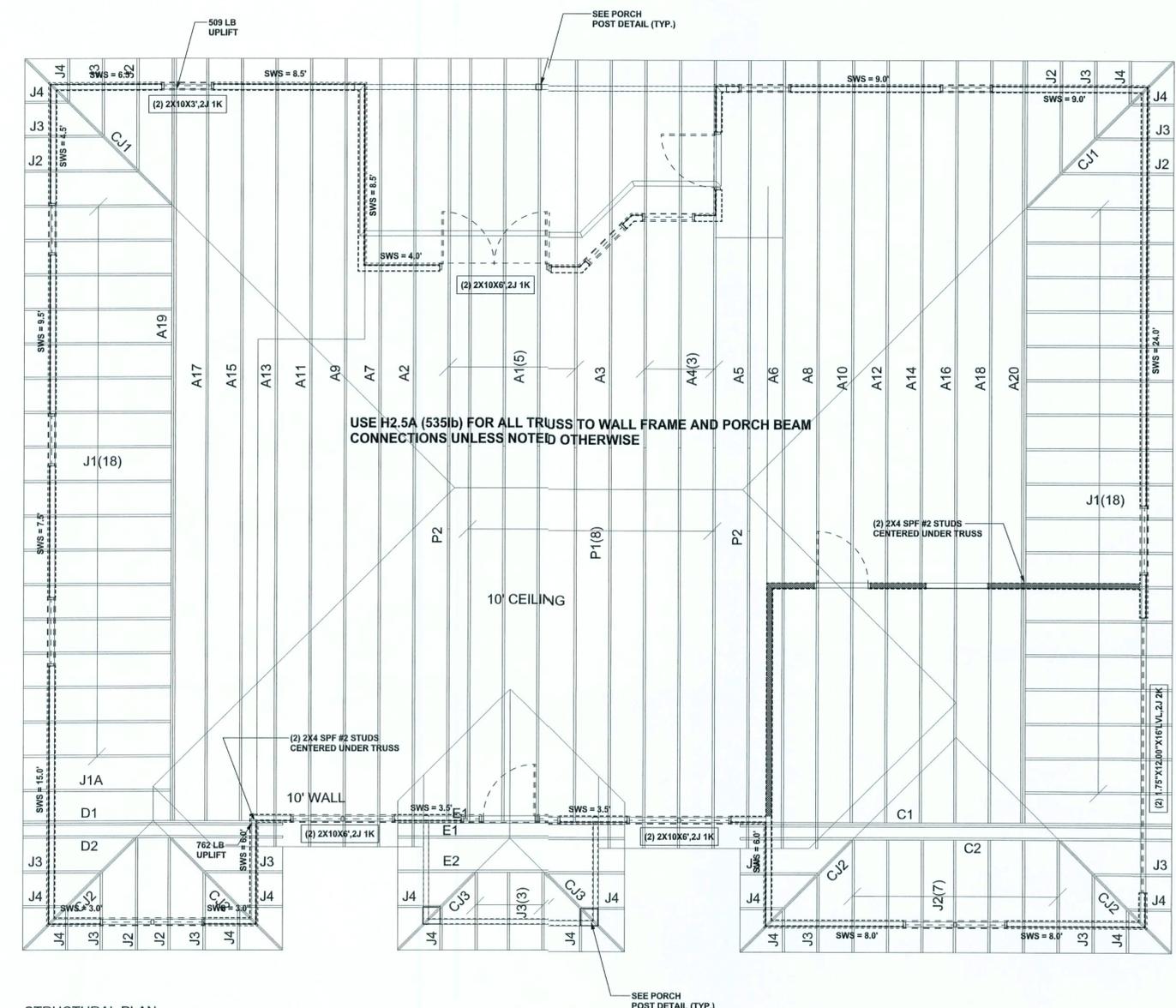
DRAWN BY: David Disosway  
CHECKED BY:

FINALS DATE:  
16 / Aug / 06

JOB NUMBER:  
603202a

DRAWING NUMBER:  
**S-2**  
OF 3 SHEETS

REVISIONS



**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

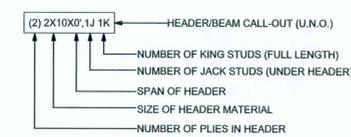
**STRUCTURAL PLAN NOTES**

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

**WALL LEGEND**

	1ST FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	2ND FLOOR EXTERIOR WALL WITH 7/16" O.S.B. WALL SHEATHING FULLY BLOCKED 8d COMMON NAILS 6" O.C. EDGE, 12" O.C. FIELD (U.N.O.)
	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

**HEADER LEGEND**



**TOTAL SHEAR WALL SEGMENTS**

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	36.0'	81.0'
LONGITUDINAL	32.5'	66.0'

WINDLOAD ENGINEER: Mark Disosway, P.E. No. 53915, POB 868, Lake City, FL 32056, 386-754-5419

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 6301.2.1, Florida building code residential 2004, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specified location.

MARK DISOSWAY  
P.E. 53915  
*Mark Disosway*  
16AUG06  
SEAL

**Shirley Hitson**

Spec House  
Lot 185  
Emerald Cove S/D

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PRINTED DATE:  
August 16, 2006

DRAWN BY: David Disosway      CHECKED BY:

FINALS DATE:  
16 / Aug / 06

JOB NUMBER:  
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DRAWING NUMBER  
**S-3**

OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER, MAYO TRUSS CO. JOB #HITSONLOT18