



Project Information for: L250980

Lot : 3
Subdivision: Ruby Park
County: Columbia
Truss Count: 18
Design Program: MiTek 20/20 6.3
Building Code: FBC2004/TPI2002

August 27, 2007

Truss Design Load Information:
Gravity: **Wind:**

Roof (psf): 42.0 Wind Standard: ASCE 7-02 Wind Exposure: B
Floor (psf): N/A Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

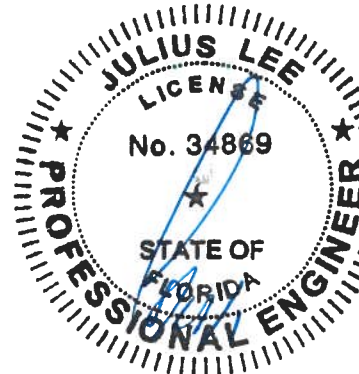
Contractor of Record, responsible for structural engineering:

Robert Stewart Florida License No. CBC1252898

Address: P.O. Box 3001 Lake City, Florida

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



Notes:

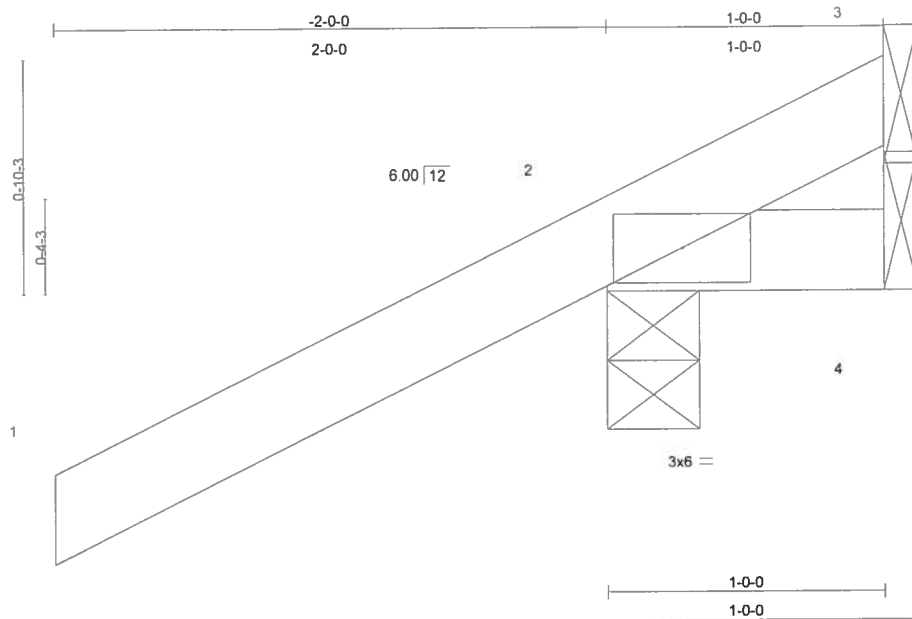
1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elements in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

No.	Drwg. #	Truss ID	Date
1	J1883157	CJ1	8/27/07
2	J1883158	CJ3	8/27/07
3	J1883159	CJ5	8/27/07
4	J1883160	EJ5	8/27/07
5	J1883161	EJ7	8/27/07
6	J1883162	HJ7	8/27/07
7	J1883163	HJ9	8/27/07
8	J1883164	T01	8/27/07
9	J1883165	T02	8/27/07
10	J1883166	T03	8/27/07
11	J1883167	T04	8/27/07
12	J1883168	T05	8/27/07
13	J1883169	T06	8/27/07
14	J1883170	T07	8/27/07
15	J1883171	T07G	8/27/07
16	J1883172	T08	8/27/07
17	J1883173	T09	8/27/07
18	J1883174	T10	8/27/07

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	CJ1	JACK	6	1	J1883157
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:16 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.28	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.01	Vert(TL)	-0.00	2	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=257/0-4-0, 4=5/Mechanical, 3=-91/Mechanical
Max Horz 2=87(load case 6)
Max Uplift 2=-287(load case 6), 4=-9(load case 4), 3=-91(load case 1)
Max Grav 2=257(load case 1), 4=14(load case 2), 3=128(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-69/76
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.14

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 287 lb uplift at joint 2, 9 lb uplift at joint 4 and 91 lb uplift at joint 3.
- Continued on page 2

Julius Lee
Truss Design Engineer
Florida Professional Engineer
1100 Coastal Bay Blvd
Gwynneth Beach, FL 32438

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	CJ1	JACK	6	1	J1883157
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:16 2007 Page 2

LOAD CASE(S) Standard

Julian Lee
Truss Design Engineer
Florida PE No. 37888
1100 Coastal Bay Blvd
Daytona Beach, FL 32138

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:17 2007 Page 1



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	CJ3	JACK	6	1	J1883158
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:17 2007 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
FirstSource PB No. 2-1000
1100 Coastal Bay Blvd
Daytona Beach, FL 32118

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

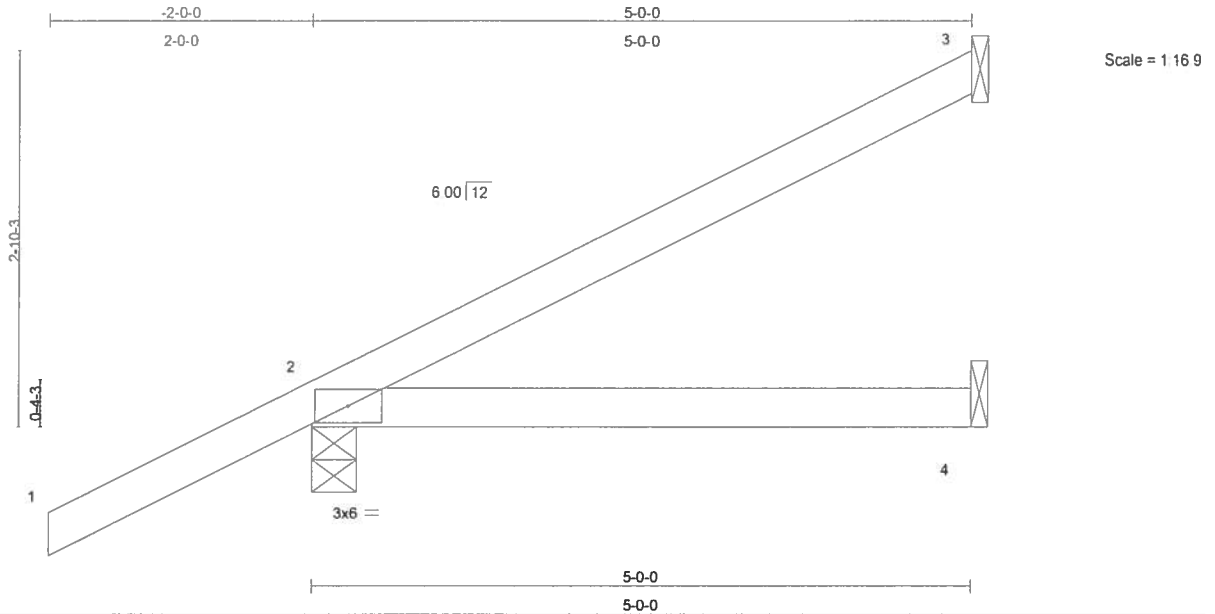
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	CJ5	JACK	6	1	J1883159
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:17 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	0.09	2-4	>671	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.24	Vert(TL)	-0.05	2-4	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=102/Mechanical, 2=296/0-4-0, 4=24/Mechanical

Max Horz 2=178(load case 6)

Max Uplift 3=-86(load case 6), 2=-261(load case 6), 4=-46(load case 4)

Max Grav 3=102(load case 1), 2=296(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-87/36

BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.15

NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 86 lb uplift at joint 3, 261 lb uplift at joint 2 and 46 lb uplift at joint 4.

Continued on page 2

Julius Lee
Truss Design Engineer
Phone: 813 710-3100
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	CJ5	JACK	6	1	J1883159
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:17 2007 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 21088
1400 Coastal Bay Blvd
Boynton Beach, FL 33435

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

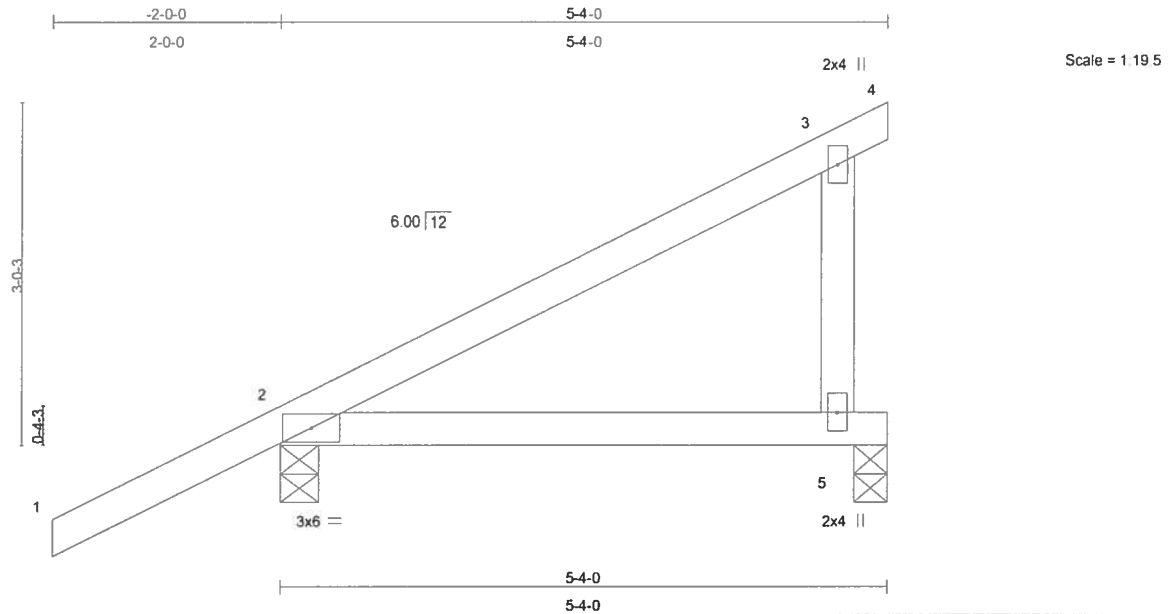
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	EJ5	MONO TRUSS	4	1	J1883160
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:18 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	0.08	2-5	>691	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.23	Vert(TL)	-0.05	2-5	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.05	Horz(TL)	0.00		n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 24 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=294/0-4-0, 5=149/0-3-8
Max Horz 2=185(load case 6)
Max Uplift 2=-255(load case 6), 5=-149(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-106/36, 3-4=-11/0
BOT CHORD 2-5=0/0
WEBS 3-5=-126/187

JOINT STRESS INDEX

2 = 0.15, 3 = 0.10 and 5 = 0.10

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 255 lb uplift at joint 2 and 149 lb uplift at joint 5.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 21888
1100 Central Bay Blvd
Daytona Beach, FL 32135

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	EJ5	MONO TRUSS	4	1	J1883160
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:18 2007 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida P.E. No. 31885
1100 Coastal Bay Blvd
Boynton Beach, FL 33436

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	EJ7	MONO TRUSS	6	1	J1883161
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:18 2007 Page 1

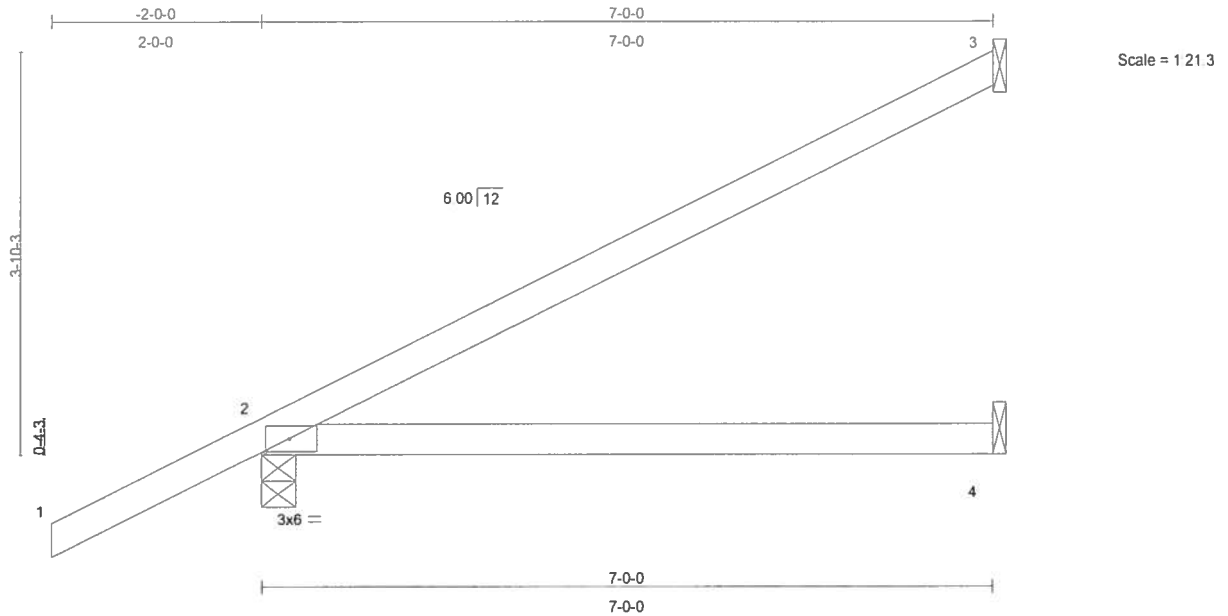


Plate Offsets (X,Y): [2-0-2-12,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.48	Vert(LL)	-0.08	2-4	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.28	Vert(TL)	-0.16	2-4	>506	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 26 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or
6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
bracing.

REACTIONS (lb/size) 3=154/Mechanical, 2=352/0-4-0, 4=44/Mechanical
Max Horz 2=161(load case 6)
Max Uplift 3=-84(load case 6), 2=-140(load case 6)
Max Grav 3=154(load case 1), 2=352(load case 1), 4=93(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-119/54
BOT CHORD 2-4=0/0

JOINT STRESS INDEX

2 = 0.70

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 3 and 140 lb uplift at joint 2.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 21888
1400 Coastal Bay Blvd
Daytona Beach, FL 32118

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	EJ7	MONO TRUSS	6	1	J1883161
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:18 2007 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 3-1888
1100 Coastal Bay Blvd
Boynton Beach, FL 33438

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

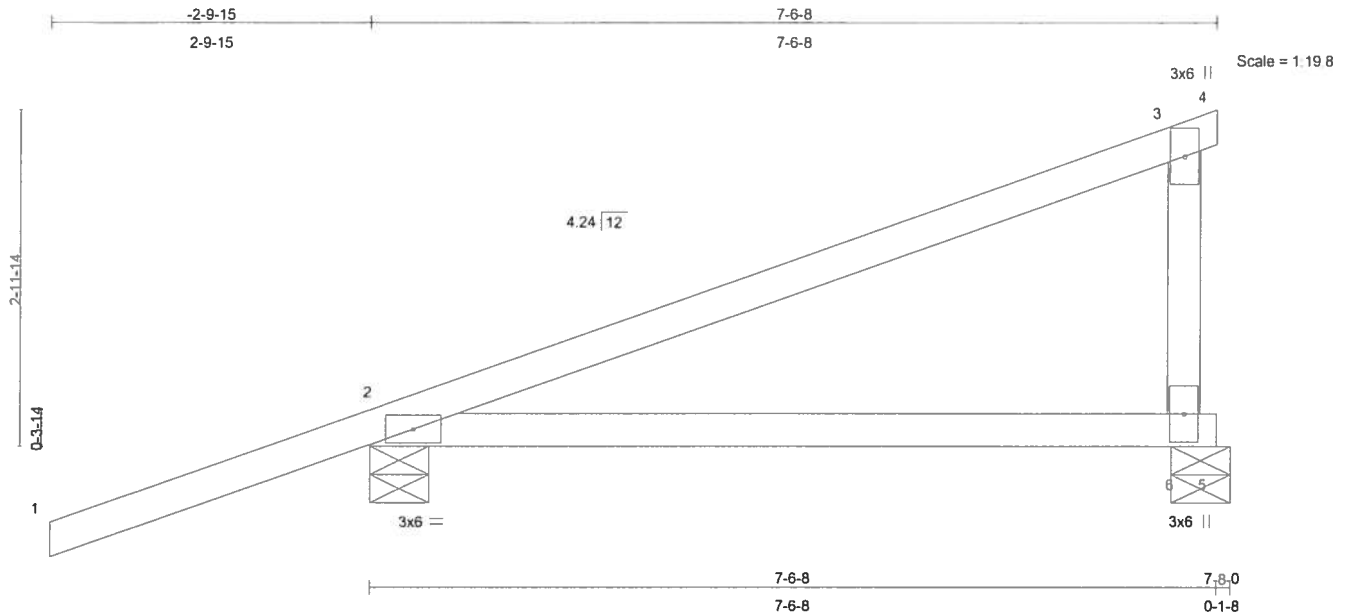
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	HJ7	MONO TRUSS	1	1	J1883162
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:19 2007 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0	TC 0.61	Vert(LL)	0.06	2-6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.18	Vert(TL)	-0.08	2-6	>999	240		
BCLL 10.0	* Rep Stress Incr NO	WB 0.00	Horz(TL)	0.00	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)							
									Weight: 31 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=266/0-6-7, 2=349/0-6-7
Max Horz 2=185(load case 3)
Max Uplift 6=-238(load case 3), 2=-340(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-135/22, 3-4=-6/0, 3-6=-215/186
BOT CHORD 2-6=-78/84, 5-6=0/0

JOINT STRESS INDEX
2 = 0.53, 3 = 0.52 and 6 = 0.33

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 6 and 340 lb uplift at joint 2.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Law
Truss Design Engineer
Florida P.E. No. 31888
1100 Coastal Bay Blvd
Boynton Beach, FL 33438

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	HJ7	MONO TRUSS	1	1	J1883162
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:19 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-3=-100(F=-23, B=-23), 3=-60(F=-23, B=-23)-to-4=-64(F=-25, B=-25), 2=0(F=5, B=5)-to-5=-19(F=-5, B=-5)

Julius Lee
Truss Design Engineer
Florida PB No. 3-1888
1100 Coastal Bay Blvd
Weynton Beach, FL 33435

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

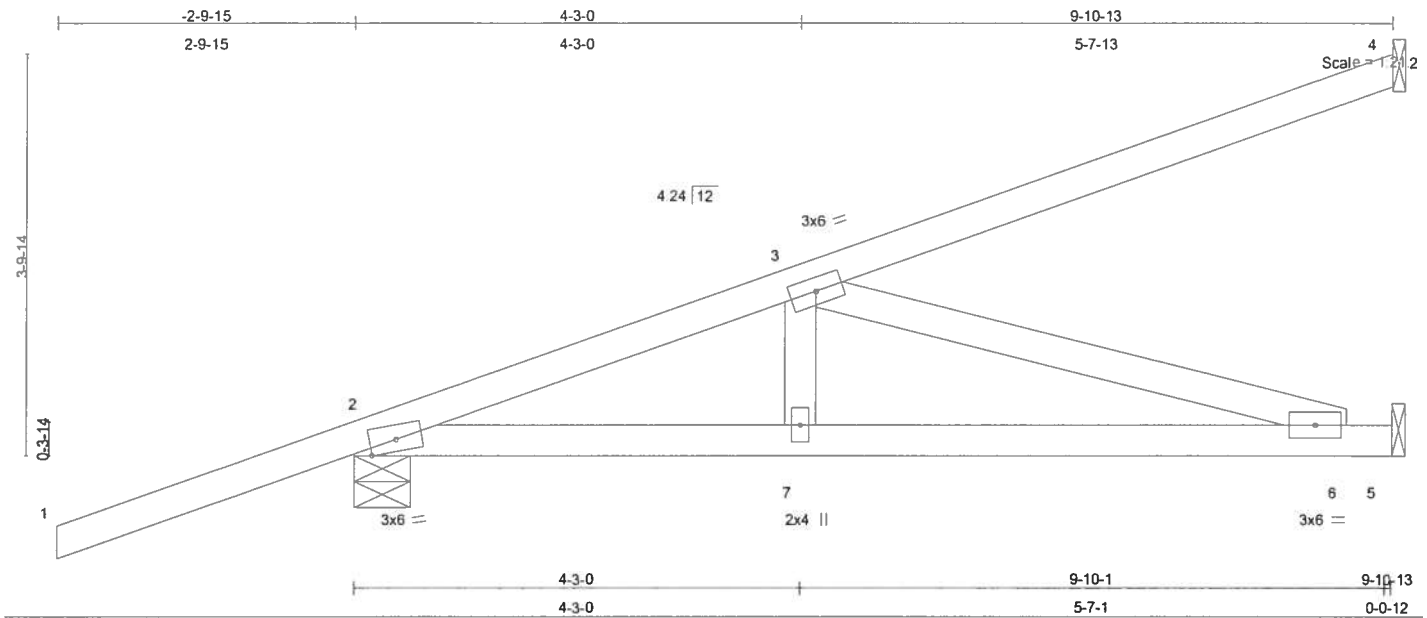
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	HJ9	MONO TRUSS	2	1	J1883163
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:19 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.61	Vert(LL)	0.05 6-7	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.12 6-7	>986	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.01 5	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
Weight: 45 lb									

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=268/Mechanical, 2=458/0-6-7, 5=217/Mechanical
Max Horz 2=270(load case 3)
Max Uplift 4=-232(load case 3), 2=-284(load case 3), 5=-61(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/50, 2-3=-642/116, 3-4=-105/65
BOT CHORD 2-7=-305/593, 6-7=-305/593, 5-6=0/0
WEBS 3-7=0/189, 3-6=-618/317

JOINT STRESS INDEX

2 = 0.78, 3 = 0.16, 6 = 0.17 and 7 = 0.13

NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 232 lb uplift at joint 4, 284 lb uplift at joint 2 and 61 lb uplift at joint 5.
- 5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 31888
1400 Coastal Bay Blvd
Waynton Beach, FL 33436

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 O'Donofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	HJ9	MONO TRUSS	2	1	J1883163
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:19 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54

Trapezoidal Loads (plf)

Vert: 2=-4(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Lee
Truss Design Engineer
Florida, PE No. 31888
1100 Coastal Bay Blvd
Boynton Beach, FL 33438

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T01	HIP	1	1	J1883164
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:20 2007 Page 1

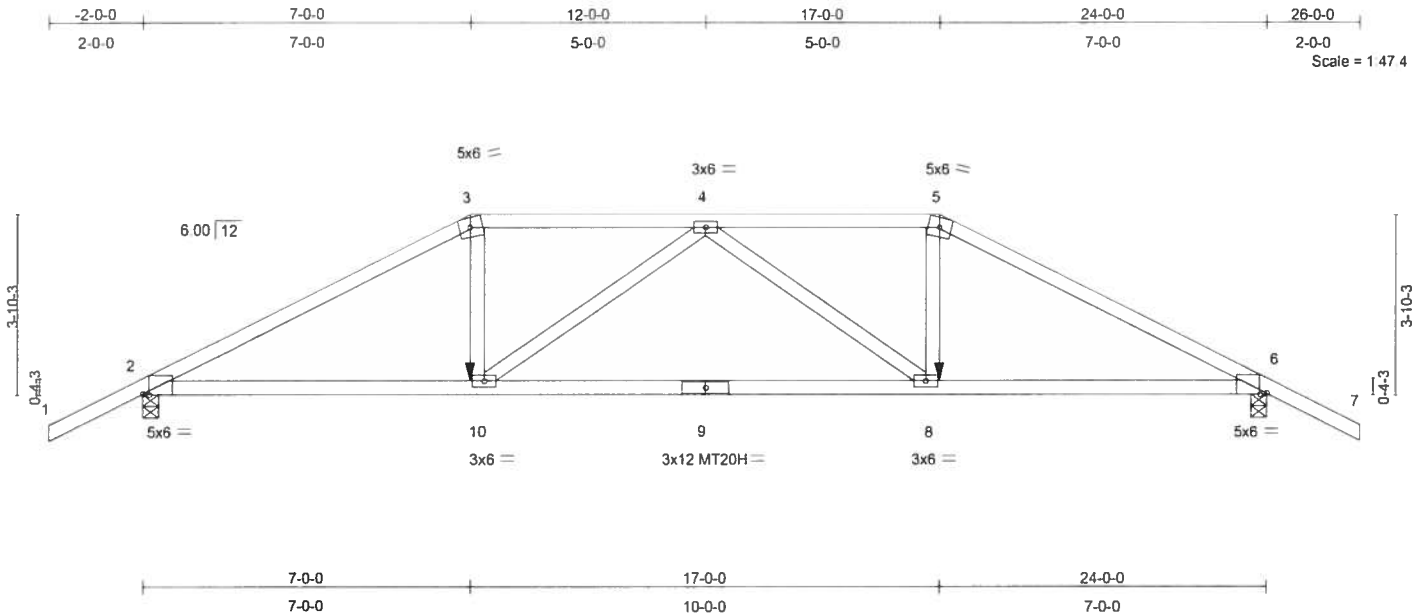


Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-1-11,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.44	Vert(LL)	-0.17	8-10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.81	Vert(TL)	-0.58	8-10	>491	240	MT20H	187/143
BCLL 10.0	* Rep Stress Incr	NO	WB 0.39	Horz(TL)	0.11	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 108 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-3-9 oc bracing.

REACTIONS (lb/size) 2=1657/0-4-0, 6=1657/0-4-0
Max Horz 2=77(load case 5)
Max Uplift 2=-547(load case 5), 6=-547(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-3014/914, 3-4=-2639/853, 4-5=-2639/853, 5-6=-3014/914, 6-7=0/47
BOT CHORD 2-10=-779/2603, 9-10=-979/3035, 8-9=-979/3035, 6-8=-746/2603
WEBS 3-10=-248/918, 4-10=-598/318, 4-8=-598/318, 5-8=-248/918

JOINT STRESS INDEX

2 = 0.74, 3 = 0.75, 4 = 0.34, 5 = 0.75, 6 = 0.74, 8 = 0.58, 9 = 0.83 and 10 = 0.58

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Julius Lee
Truss Design Engineer
Florida, P.E. No. 21888
1100 Coastal Bay Blvd
Gwynn Beach, FL 33436

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T01	HIP	1	1	J1883164
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:20 2007 Page 2

NOTES

- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 547 lb uplift at joint 2 and 547 lb uplift at joint 6.
- 8) Girder carries hip end with 7'-0" end setback.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-5=-117(F=-63), 5-7=-54, 2-10=-10, 8-10=-22(F=-12), 6-8=-10

Concentrated Loads (lb)

Vert: 10=-411(F) 8=-411(F)

Julian Lee
Truss Design Engineer
Phone: 813-318-8888
1800 Coastal Bay Blvd
Boynton Beach, FL 33426

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T02	HIP	1	1	J1883165
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:21 2007 Page 1

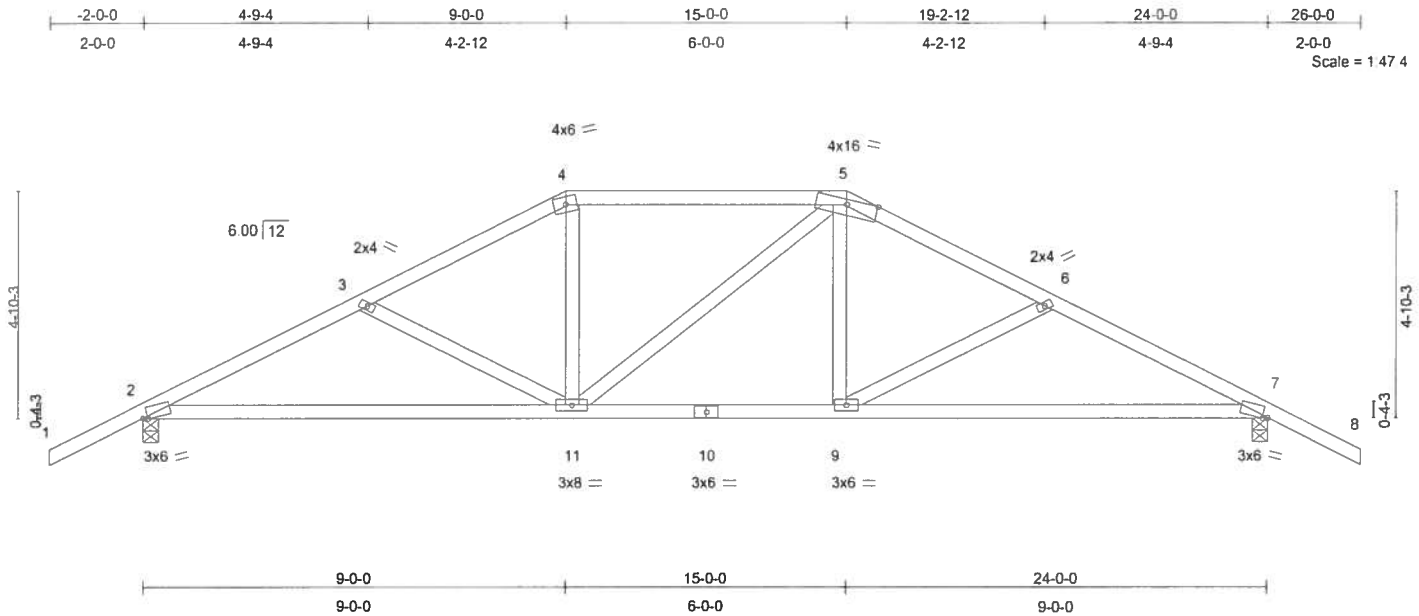


Plate Offsets (X,Y): [2:0-1-5,0-0-7], [7:0-1-5,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.30	Vert(LL)	-0.15	7-9	>999	360	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.28	7-9	>999	240	244/190
BCLL 10.0	* Rep Stress Incr	YES	WB 0.10	Horz(TL)	0.04	7	n/a	n/a	
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 119 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-5 oc bracing.

REACTIONS (lb/size) 2=874/0-4-0, 7=874/0-4-0
Max Horz 2=-89(load case 7)
Max Uplift 2=-245(load case 6), 7=-245(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1320/697, 3-4=-1086/596, 4-5=-935/589, 5-6=-1086/597,
6-7=-1320/697, 7-8=0/47
BOT CHORD 2-11=-457/1117, 10-11=-280/935, 9-10=-280/935, 7-9=-457/1117
WEBS 3-11=-212/201, 4-11=-43/262, 5-11=-109/110, 5-9=-43/263, 6-9=-212/201

JOINT STRESS INDEX

2 = 0.87, 3 = 0.33, 4 = 0.57, 5 = 0.91, 6 = 0.33, 7 = 0.87, 9 = 0.34, 10 = 0.30 and 11 = 0.56

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida P.E. No. 21889
1100 Coastal Bay Blvd
Daytona Beach, FL 32118

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oroffio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T02	HIP	1	1	J1883165
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:21 2007 Page 2

NOTES

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 2 and 245 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 31888
1100 Crystal Bay Blvd
Weynton Beach, FL 33436

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T03	HIP	1	1	J1883166
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:21 2007 Page 1

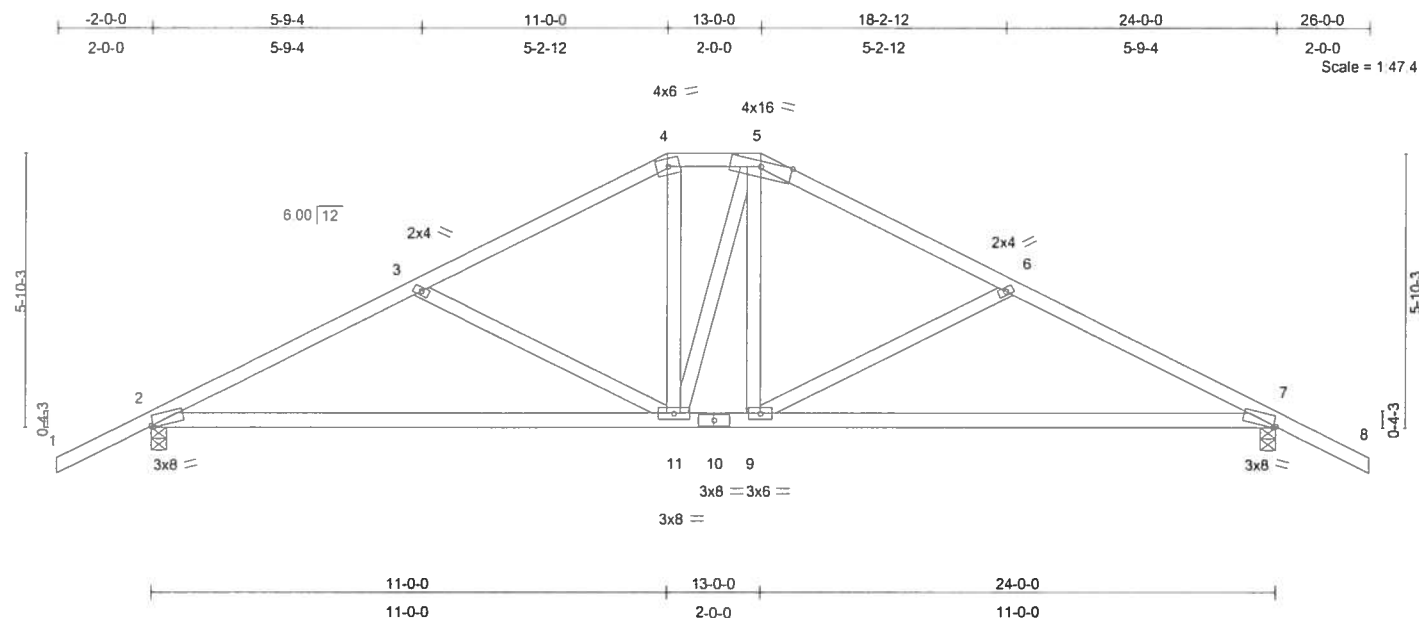


Plate Offsets (X,Y): [2:0-0-10,Edge], [7:0-0-10,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	-0.28	7-9	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.63	Vert(TL)	-0.51	7-9	>553	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.22	Horz(TL)	0.04	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 124 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-2-1 oc bracing.

REACTIONS

(lb/size) 2=874/0-4-0, 7=874/0-4-0
Max Horz 2=-101(load case 7)
Max Uplift 2=-256(load case 6), 7=-256(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/47, 2-3=-1299/712, 3-4=-972/557, 4-5=-812/559, 5-6=-971/557,
6-7=-1299/712, 7-8=0/47
BOT CHORD 2-11=-463/1097, 10-11=-198/810, 9-10=-198/810, 7-9=-463/1097
WEBS 3-11=-330/300, 4-11=-93/256, 5-11=-136/143, 5-9=-94/257, 6-9=-332/301

JOINT STRESS INDEX

2 = 0.84, 3 = 0.33, 4 = 0.51, 5 = 0.58, 6 = 0.33, 7 = 0.85, 9 = 0.34, 10 = 0.78 and 11 = 0.66

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 21008
1100 Coastal Bay Blvd
Boynton Beach, FL 33436

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T03	HIP	1	1	J1883166
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:21 2007 Page 2

NOTES

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 2 and 256 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 3-19938
1100 Coastal Bay Blvd
Weynton Beach, FL 33436

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T04	COMMON	5	1	J1883167
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Aug 27 14:02:45 2007 Page 1

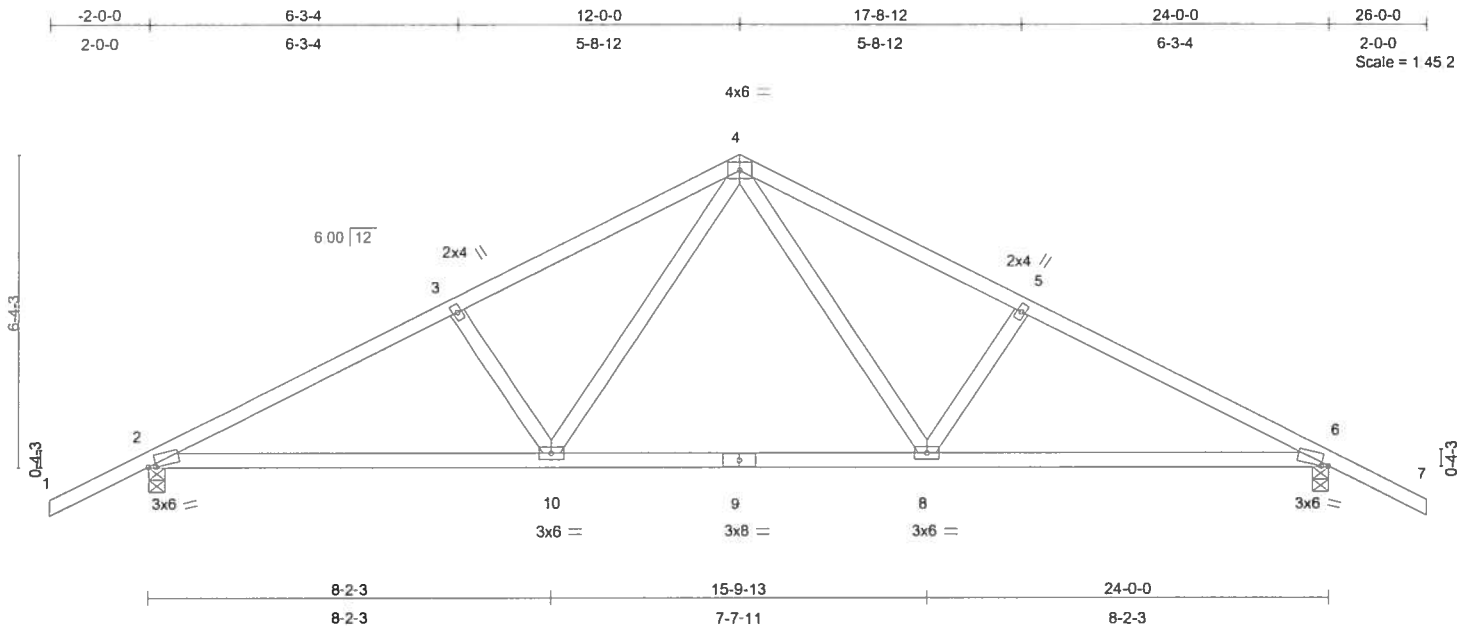


Plate Offsets (X,Y): [2:0-1-13,0-0-7], [6:0-1-13,0-0-7]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	0.32	8-10	>888	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.79	Vert(TL)	-0.49	8-10	>585	240		
BCLL 10.0	* Rep Stress Incr	NO	WB 0.34	Horz(TL)	0.06	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 113 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-3-4 oc bracing.

REACTIONS (lb/size) 2=1103/0-4-0, 6=1103/0-4-0
Max Horz 2=-107(load case 7)
Max Uplift 2=-324(load case 6), 6=-324(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1838/1000, 3-4=-1664/998, 4-5=-1664/998, 5-6=-1838/1000, 6-7=0/47
BOT CHORD 2-10=-711/1561, 9-10=-376/1078, 8-9=-376/1078, 6-8=-711/1561
WEBS 3-10=-268/257, 4-10=-370/671, 4-8=-370/671, 5-8=-268/257

JOINT STRESS INDEX

2 = 0.79, 3 = 0.34, 4 = 0.71, 5 = 0.34, 6 = 0.79, 8 = 0.51, 9 = 0.97 and 10 = 0.51

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 324 lb uplift at joint 2 and 324 lb uplift at joint 6.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Julius Lee
Truss Design Engineer
Florida PE No. 31888
1100 Coastal Bay Blvd
Daytona Beach, FL 32110

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSE-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T04	COMMON	5	1	J1883167
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2006 MiTek Industries, Inc. Mon Aug 27 14:02:46 2007 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1400 Coastal Bay Blvd
Weymouth Beach, FL 33450

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

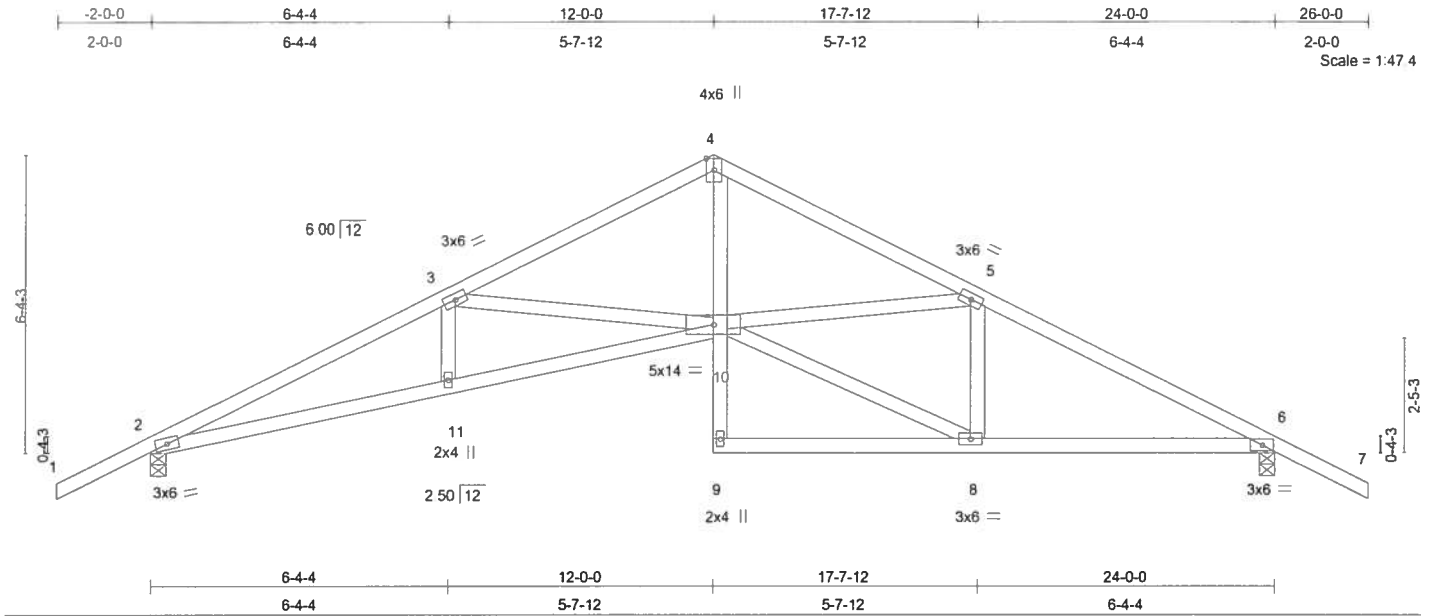
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T05	SPECIAL	4	1	J1883168
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:23 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.33	Vert(LL)	0.13 10-11	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.44	Vert(TL)	-0.24 10-11	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.38	Horz(TL)	0.14 6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
									Weight: 123 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2 *Except*
 4-9 2 X 4 SYP No.3
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-1-10 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 7-5-11 oc bracing.

REACTIONS (lb/size) 2=874/0-4-0, 6=874/0-4-0
 Max Horz 2=-107(load case 7)
 Max Uplift 2=-261(load case 6), 6=-261(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2082/977, 3-4=-1445/689, 4-5=-1473/704, 5-6=-1308/676, 6-7=0/47
 BOT CHORD 2-11=-707/1818, 10-11=-709/1817, 9-10=0/74, 4-10=-374/941, 8-9=-8/25,
 6-8=-426/1094
 WEBS 3-11=0/201, 3-10=-573/401, 8-10=-457/1169, 5-10=-16/222, 5-8=-410/232

JOINT STRESS INDEX

2 = 0.68, 3 = 0.39, 4 = 0.68, 5 = 0.39, 6 = 0.58, 8 = 0.65, 9 = 0.60, 10 = 0.92 and 11 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida PE No. 3-1888
 1100 Coastal Bay Blvd
 Weymouth Beach, FL 33456

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T05	SPECIAL	4	1	J1883168
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:23 2007 Page 2

NOTES

- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 261 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 31888
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

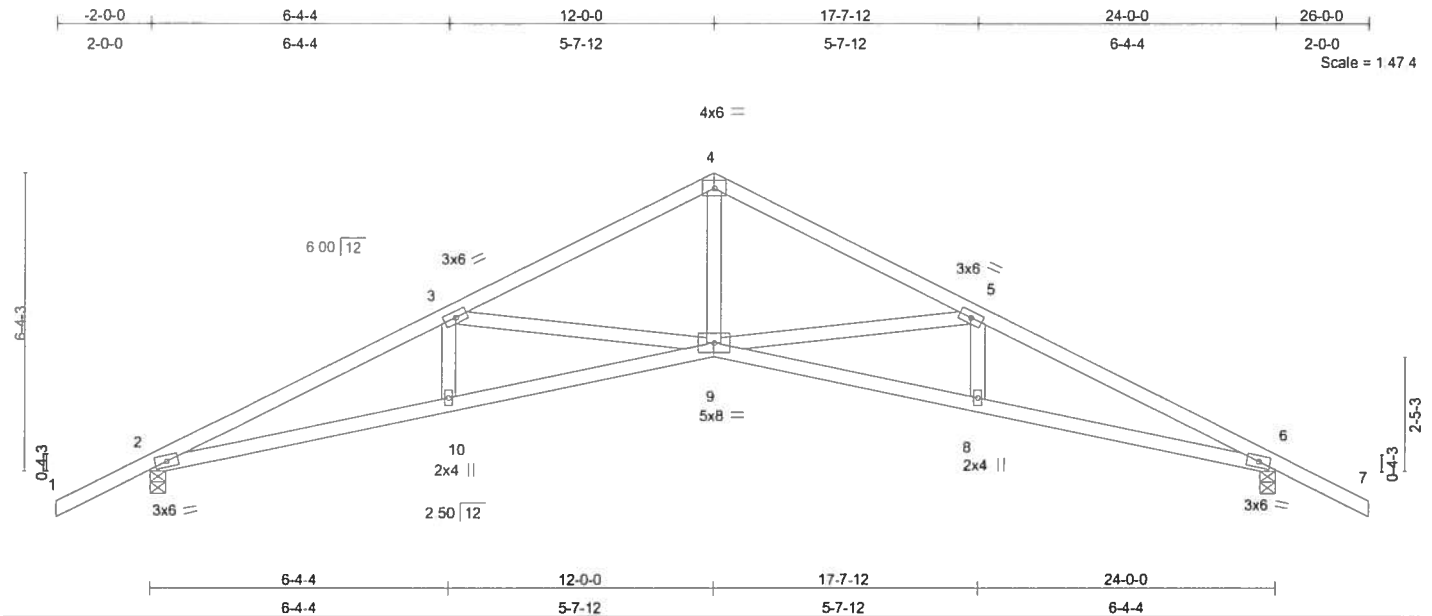
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T06	SCISSOR	4	1	J1883169
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:24 2007 Page 1



LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	2-0-0	TC 0.33	Vert(LL)	0.15	9-10	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.40	Vert(TL)	-0.28	9-10	>999	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.33	Horz(TL)	0.19	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 110 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-1-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-5-7 oc bracing.

REACTIONS (lb/size) 2=874/0-4-0, 6=874/0-4-0
Max Horz 2=106(load case 6)
Max Uplift 2=-261(load case 6), 6=-261(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2079/977, 3-4=-1472/700, 4-5=-1472/700, 5-6=-2079/977, 6-7=0/46
BOT CHORD 2-10=-708/1816, 9-10=-712/1817, 8-9=-712/1817, 6-8=-708/1816
WEBS 3-10=0/184, 3-9=-564/392, 4-9=-375/947, 5-9=-564/392, 5-8=0/184

JOINT STRESS INDEX

2 = 0.68, 3 = 0.39, 4 = 0.60, 5 = 0.39, 6 = 0.68, 8 = 0.33, 9 = 0.52 and 10 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 - *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 - Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula.
- Building designer should verify capacity of bearing surface.

Continued on page 2

Julius Lee
Truss Design Engineer
Phone: 813-210-3188
1100 Coastal Bay Blvd
Maitland, FL 32751

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T06	SCISSOR	4	1	J1883169
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:24 2007 Page 2

NOTES

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 261 lb uplift at joint 2 and 261 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lane
Truss Design Engineer
Florida, FL 32055
1100 Coastal Bay Blvd
Gwynn Beach, FL 32055

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

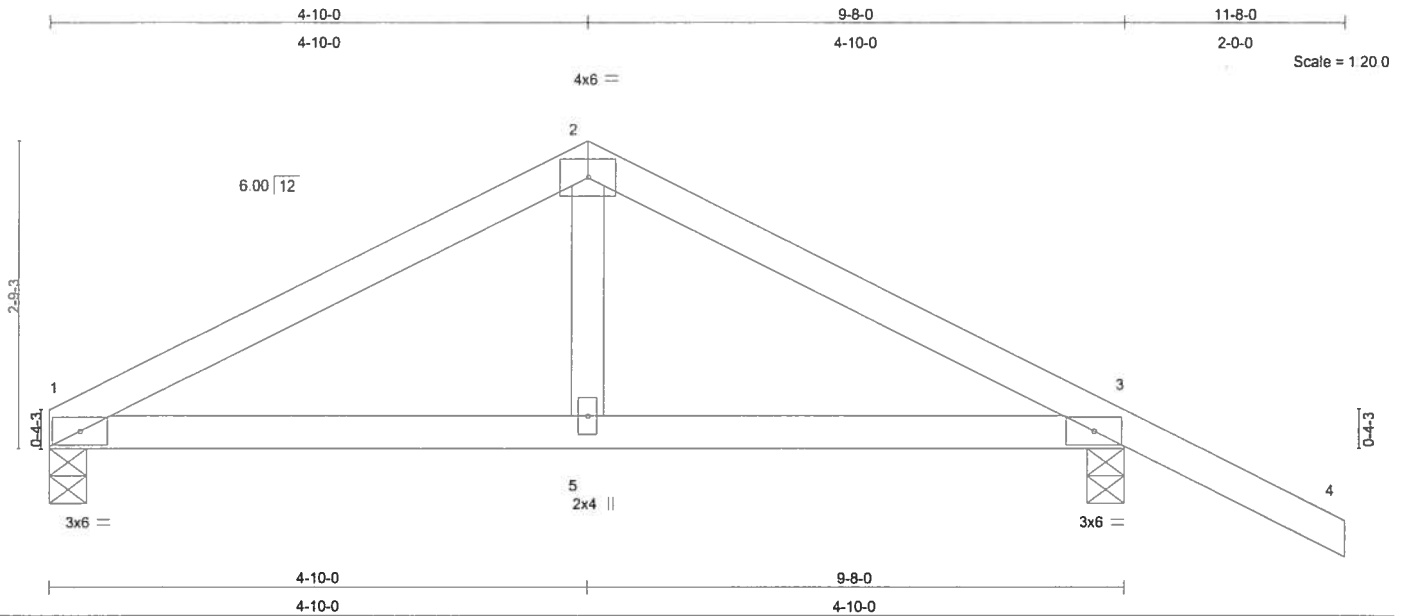
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T07	COMMON	3	1	J1883170
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:24 2007 Page 1



LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 2-0-0		TC 0.30	Vert(LL)	0.02	1-5	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.14	Vert(TL)	-0.03	1-5	>999	240		
BCLL 10.0	* Rep Stress Incr YES		WB 0.05	Horz(TL)	0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 37 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=285/0-4-0, 3=429/0-4-0
 Max Horz 1=-78(load case 7)
 Max Uplift 1=-63(load case 6), 3=-169(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-404/238, 2-3=-410/247, 3-4=0/47
 BOT CHORD 1-5=-55/311, 3-5=-55/311
 WEBS 2-5=0/159

JOINT STRESS INDEX

1 = 0.41, 2 = 0.49, 3 = 0.41 and 5 = 0.11

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 1 and 169 lb uplift at joint 3.

Continued on page 2

Julius Lee
 Truss Design Engineer
 Florida PE No. 31088
 1100 Coastal Bay Blvd
 Daytona Beach, FL 32118

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T07	COMMON	3	1	J1883170
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:24 2007 Page 2

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34888
1100 Coastal Bay Blvd
Boynton Beach, FL 33438

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 8300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T07G	GABLE	1	1	J1883171
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MITek Industries, Inc. Wed Aug 22 09:15:25 2007 Page 1

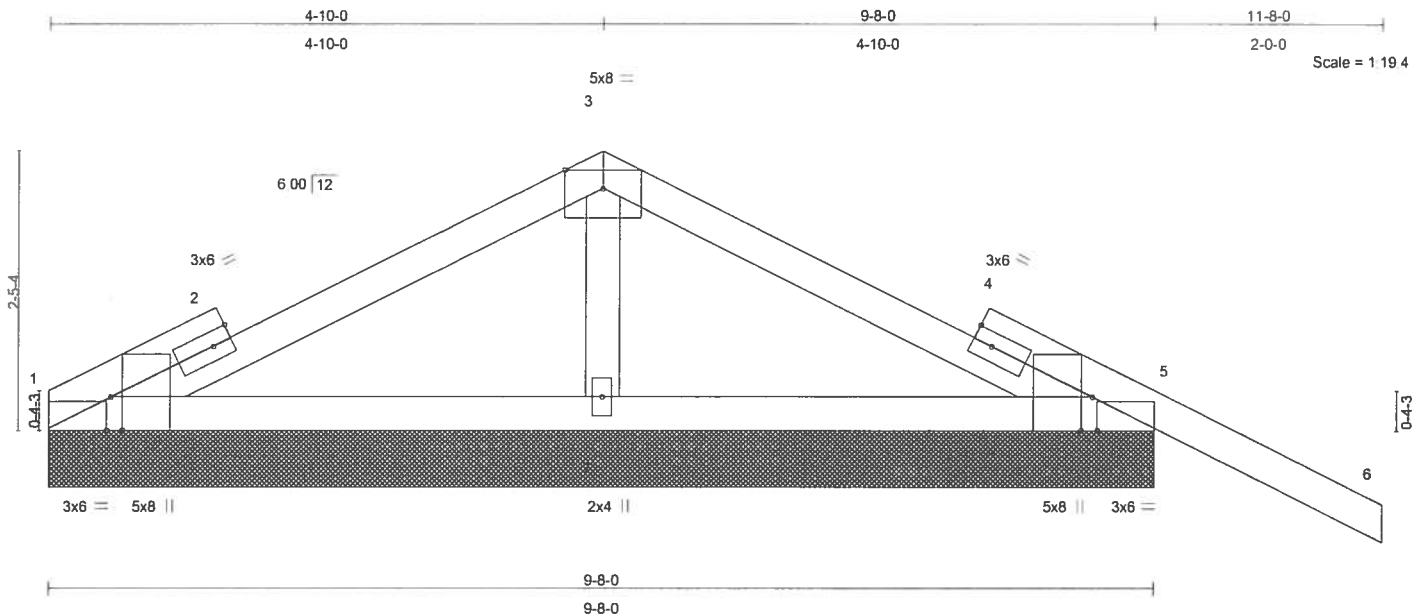


Plate Offsets (X,Y): [1:0-3-8,Edge], [1:0-0-8,Edge], [5:0-3-8,Edge], [5:0-0-8,Edge]

LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	2-0-0	TC 0.49	Vert(LL) -0.02	6	n/r	120		MT20	244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.19	Vert(TL) -0.04	6	n/r	90			
BCLL 10.0	* Rep Stress Incr NO		WB 0.15	Horz(TL) 0.00	5	n/a	n/a			
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						Weight: 40 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 9-8-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 1=133/9-8-0, 5=408/9-8-0, 7=886/9-8-0

Max Horz 1=-92(load case 7)
Max Uplift 1=-46(load case 6), 5=-255(load case 7), 7=-318(load case 6)
Max Grav 1=153(load case 10), 5=442(load case 11), 7=886(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-179/257, 2-3=-220/412, 3-4=-203/398, 4-5=-155/207, 5-6=-30/99
BOT CHORD 1-7=-254/294, 5-7=-254/294
WEBS 3-7=-797/592

JOINT STRESS INDEX

1 = 0.63, 1 = 0.00, 2 = 0.00, 2 = 0.36, 3 = 0.68, 4 = 0.00, 4 = 0.36, 5 = 0.63, 5 = 0.00 and 7 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face) see MITek "Standard Gable End Detail"

Continued on page 2

Julius Lee
Truss Design Engineer
Florida PE No. 3-38835
1106 Coastal Bay Blvd
Gwynn Beach, FL 32438

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T07G	GABLE	1	1	J1883171
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:25 2007 Page 2

NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2'-0" oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1, 255 lb uplift at joint 5 and 318 lb uplift at joint 7.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-114(F=-60), 3-6=-114(F=-60), 1-5=-10

Julius Lee
Truss Design Engineer
Florida PB No. 31888
1400 Coastal Bay Blvd
Boynton Beach, FL 33438

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

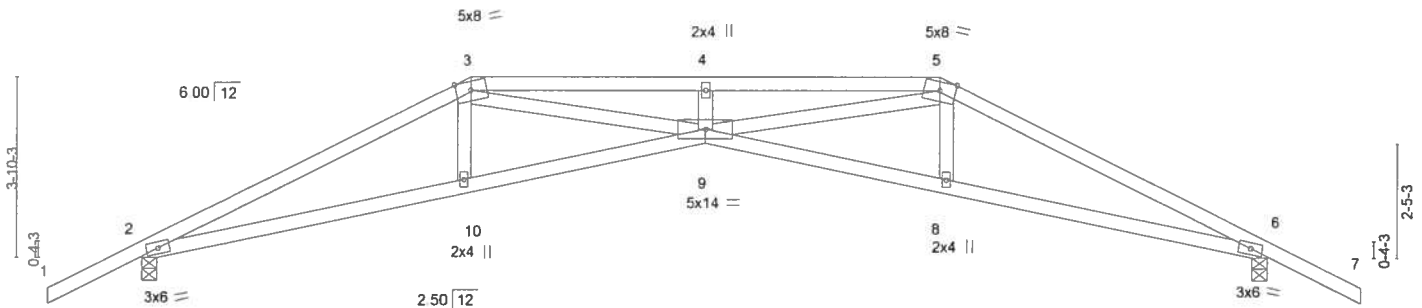
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 8300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T08	SPECIAL	1	1	J1883172
Job Reference (optional)					

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:26 2007 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.46	Vert(LL)	0.37	9	>763	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.50	Vert(TL)	-0.70	9	>405	240		
BCLL 10.0	* Rep Stress Incr	YES	WB 0.65	Horz(TL)	0.37	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 103 lb										

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-11-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-10-7 oc bracing.

REACTIONS (lb/size) 2=874/0-4-0, 6=874/0-4-0
Max Horz 2=77(load case 6)
Max Uplift 2=-231(load case 6), 6=-231(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2075/897, 3-4=-3743/1605, 4-5=-3743/1605, 5-6=-2075/897, 6-7=0/46
BOT CHORD 2-10=-629/1811, 9-10=-628/1794, 8-9=-628/1794, 6-8=-629/1811
WEBS 3-10=0/221, 3-9=-752/2035, 4-9=-206/121, 5-9=-752/2035, 5-8=0/221

JOINT STRESS INDEX

2 = 0.68, 3 = 0.71, 4 = 0.33, 5 = 0.71, 6 = 0.68, 8 = 0.33, 9 = 0.69 and 10 = 0.33

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

Julius Lee
Truss Design Engineer
Florida Reg No. 3-1888
1100 Coastal Bay Blvd
Boynton Beach, FL 33435

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITTEK connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Oonofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T08	SPECIAL	1	1	J1883172
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:26 2007 Page 2

NOTES

- 6) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 2 and 231 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida Reg No. 31888
1100 Coastal Bay Blvd
Boynton Beach, FL 33438

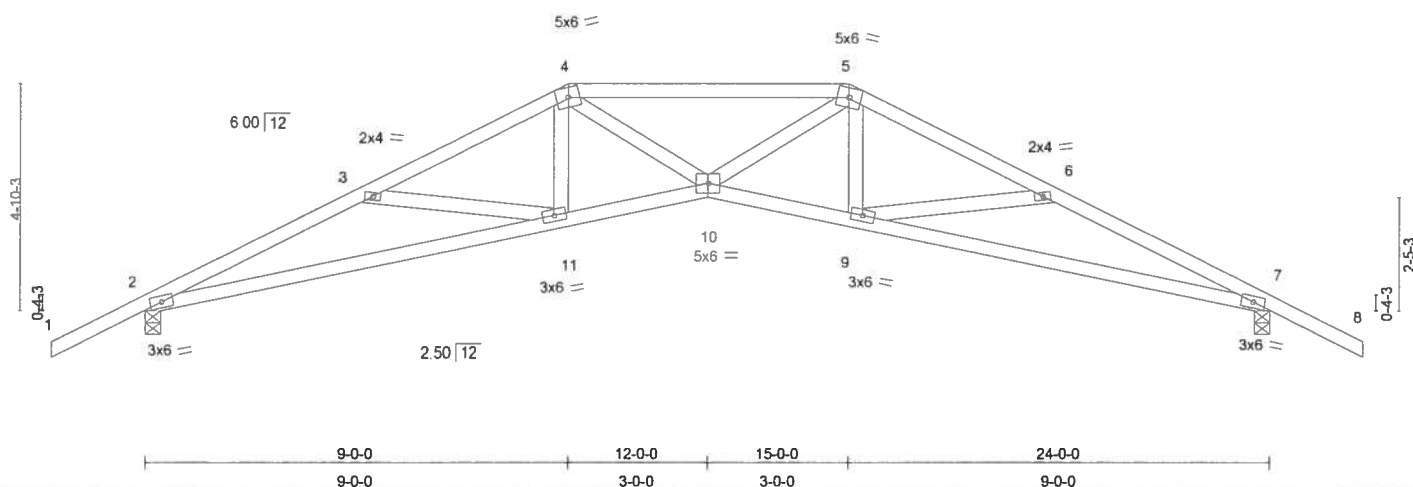
August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:27 2007 Page 1



Builders
FirstSource

Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T09	SPECIAL	1	1	J1883173
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:27 2007 Page 2

NOTES

- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 245 lb uplift at joint 2 and 245 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PE No. 34889
1100 Coastal Bay Blvd
Weynton Beach, FL 33438

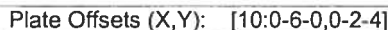
August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:28 2007 Page 1



LUMBER

BRACING

REACTIONS (lb/size) 2=874/0-4-0, 7=874/0-4-0
Max Horz 2=-100(load case 7)
Max Uplift 2=-256(load case 6), 7=-256(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/46, 2-3=-2074/959, 3-4=-1480/691, 4-5=-1418/721, 5-6=-1480/691,
6-7=-2074/959, 7-8=0/46

BOT CHORD 2-11=-692/1810, 10-11=-695/1812, 9-10=-695/1812, 7-9=-692/1810

WEBS 3-11=0/184, 3-10=-520/375, 4-10=-190/494, 5-10=-190/494, 6-10=-520/375,
6-9=0/184

JOINT STRESS INDEX

2 = 0.68, 3 = 0.39, 4 = 0.37, 5 = 0.37, 6 = 0.39, 7 = 0.68, 9 = 0.33, 10 = 0.56 and 11 = 0.33

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDF=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.

Continued on page 2

Julius Lee
True Design Engineer
Florida FE No. 21888
1100 Crystal Bay Blvd
Gwynn Beach, FL 33426

August 27, 2007

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BC51-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	CASH ACCOUNT - RUBY PARK LOT 3
L250980	T10	SPECIAL	1	1	J1883174
					Job Reference (optional)

Builders FirstSource, Lake City, FL 32055

6.300 s Feb 15 2006 MiTek Industries, Inc. Wed Aug 22 09:15:28 2007 Page 2

NOTES

- 4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Bearing at joint(s) 2, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 2 and 256 lb uplift at joint 7.

LOAD CASE(S) Standard

Julius Lee
Truss Design Engineer
Florida PB No. 21888
1100 Coastal Bay Blvd
Daytona Beach, FL 32118

August 27, 2007

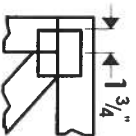
Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling, Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719

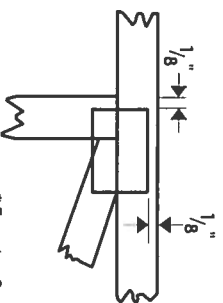


Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



*This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

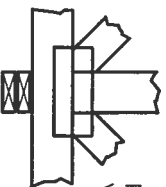
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



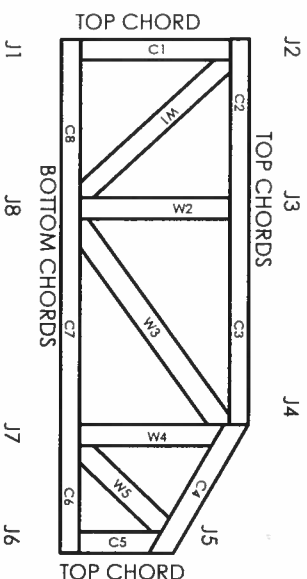
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILHR	960022-W, 970036-N
NER	561



MITek Engineering Reference Sheet: MIT-7473



General Safety Notes

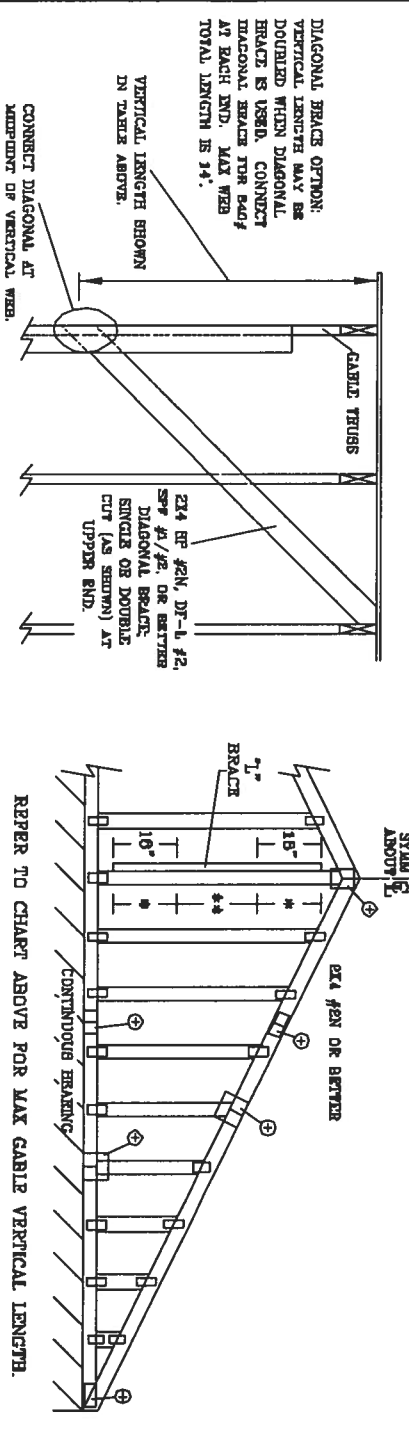
Failure to Follow Could Cause Property Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length ($\pm 6"$ from adjacent joint.)
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or purlins provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

© 1993 MITek® Holdings, Inc.

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		BRACE		NO		(1) 1X4 T" BRACE *		(1) 2X4 T" BRACE *		(2) 2X4 T" BRACE **		(1) 2X6 T" BRACE *		(2) 2X6 T" BRACE *		(2) 2X8 T" BRACE **	
GABLE VERTICAL SPACING	2X4 SPECIES	GRADE	BRACES	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 6"	10' 10"	11' 2"	12' 11"	13' 3"				
		#3	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"	12' 11"				
		STUD	3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"	12' 11"				
		STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	9' 5"	9' 5"	11' 8"	11' 8"	12' 11"	13' 11"		
16" O.C.	SPF	#1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 3"	10' 10"	11' 8"	12' 11"	13' 7"				
		#2	3' 7"	6' 10"	6' 0"	6' 8"	6' 8"	8' 3"	8' 3"	10' 4"	10' 4"	12' 11"	13' 7"				
		#3	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 3"	10' 3"	10' 3"	12' 11"	13' 7"				
		STUD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	9' 8"	9' 8"	11' 8"	11' 8"	12' 11"	13' 7"		
24" O.C.	SPF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
		#3	3' 8"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
		STUD	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
		STANDARD	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DISTRIBUTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS FOR 136 T/L OVER CONTINUOUS BRACING (6 PSF TO DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLEAKS WITH 2' 0" OVERHANG, OR 12" PLATEWOOD OVERHANG.	
ATTACH EACH T" BRACE WITH 10d NAILS.	
* FOR (1) T" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) T" BRACES, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.	
T" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.	

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO BRACE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2X6
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND BEEL PLATES.	

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR EACH AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

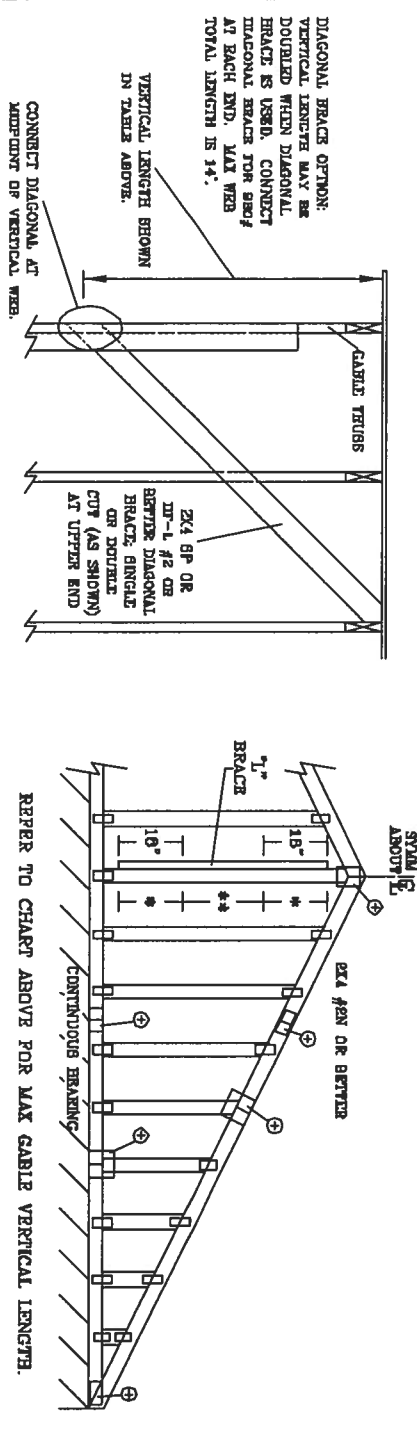
WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO ISS-1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS OF AMERICA, 1000 N. 10TH ST., SUITE 100, DENVER, CO 80202, FOR ADDITIONAL INFORMATION. THESE FUNCTIONAL, UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

REF ASCE 7-02-CAB10015
DATE 11/26/03
DRWG MTRX STD CABLE IS E HT
-ENG

JULIUS LEE'S
CONS. ENGINEERS P.A.
1465 67th AVE. NORTH
DEERBEEK, FL 33441-2161
No. 34869
STATE OF FLORIDA
MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH		2x4		BRACE		NO		(1) 1x4 L" BRACE *		(1) 2x4 L" BRACE *		(2) 2x4 L" BRACE **		(1) 2x6 L" BRACE *		(2) 2x6 L" BRACE *		(2) 2x8 L" BRACE **	
GABLE VERTICAL SPACING	SPECIES	GRADE	BRACES	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE	BRACE
12" O.C.	SPF	#1 / #2	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD
16" O.C.	SPF	#1 / #2	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD
24" O.C.	SPF	#1 / #2	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD	STUD



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS FOR 160 PSF OVER CONTINUOUS BRACING (6 PSF VC DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4" O" OUTLINES WITH 2" O" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH L" BRACE WITH 10d NAILS.	
* FOR (1) L" BRACE, SPACE NAILS AT 8" O.C.	
IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) L" BRACES, SPACE NAILS AT 3" O.C.	
IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.	
L" BRACING MUST BE A MINIMUM OF 60% OF WEB MEMBER LENGTH.	

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 80% AT EACH END. MAX WEB TOTAL LENGTH IS 14'.

VERTICAL LENGTH SHOWN IN TABLE ABOVE.

CONNECT DIAGONAL AT MIDPOINT OF VERTICAL WEB.

2x4 8P OR 2x4 12P OR 2x4 16P BRACE DIAGONAL OR DOUBLE COT (AS SHOWN) AT UPPER END

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

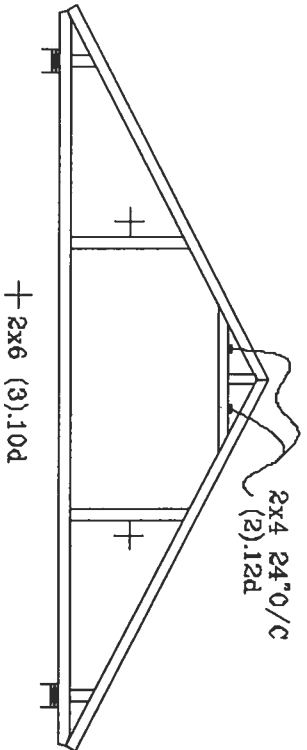
MAXIMUM TRUSSES REQUIRE EXTREME CARE IN MANUFACTURING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSTON 1-800-451-0000 FOR TRUSS INFORMATION. PUBLISHED BY THE TRUSS AND BRACE INSTITUTE, 383 BROADWAY, 2ND FLOOR, NEW YORK, NY 10013-2400. (212) 677-1100. FAX (212) 677-1101. E-MAIL: TRUSS@TRUSSINSTITUTE.COM. WWW.TRUSSINSTITUTE.COM. THESE TRUSSES ARE DESIGNED FOR 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C. STRUCTURAL PILES AND BOLTS OR OTHER ANCHORS SHALL HAVE A MINIMUM OF 60% OF WEB MEMBER LENGTH.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1466 NW 4th AVENUE
DELRAY BEACH, FL 33444-0101

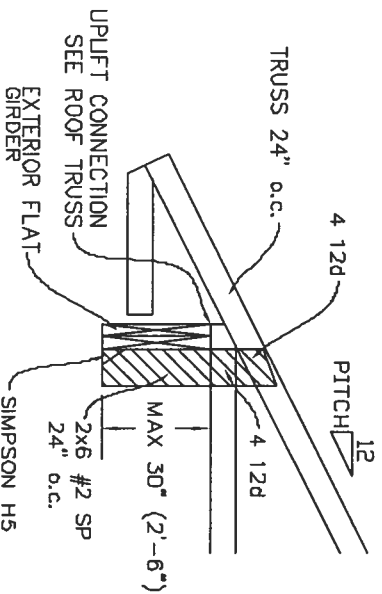
REF ASCE 7-02-GAB130390
DATE 11/26/03
DWG. LATEC STD GABLE 90' 2' 11"
-ENG

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

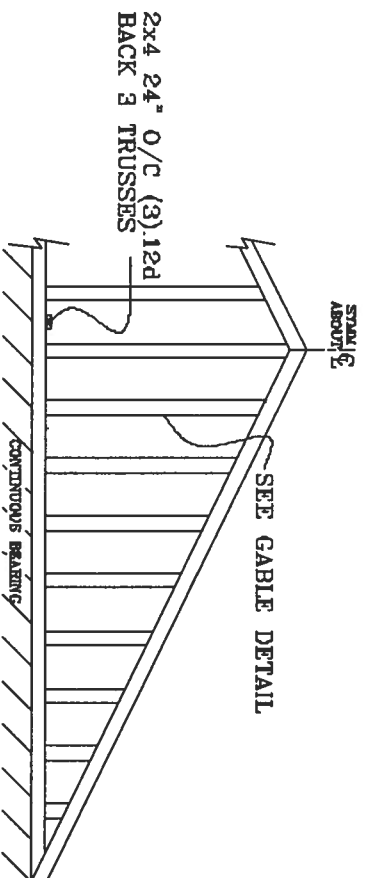
TYPICAL ATTIC TRUSS BRACING



TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS

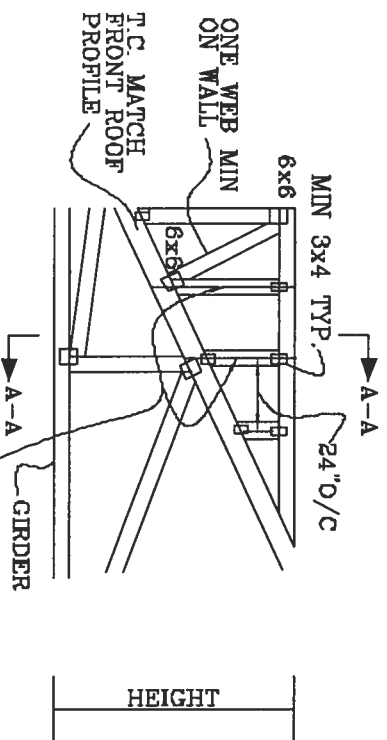


GABLE END TRUSS DETAIL



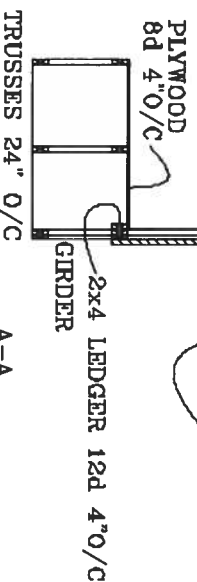
MINIMUM BRACING ON GABLE TRUSS, OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR BOB

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT
ROOF 24" O/C

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL



A-A

JULIUS LEE'S
CONS. ENGINEERS P.A.

1405 SW 4TH AVENUE
SUITE 200, FT. LAUDERDALE, FL 33444-2001

No. 34489
STATE OF FLORIDA

PIGGYBACK DETAIL

PIGgyBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

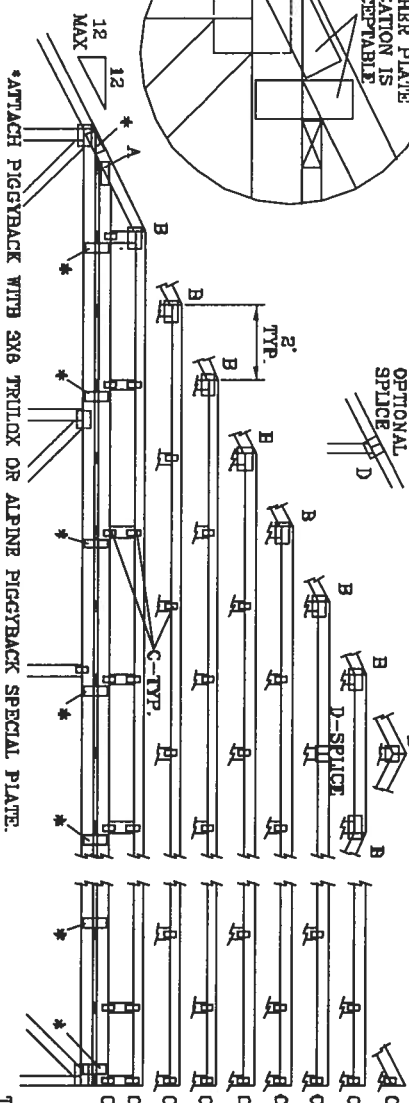
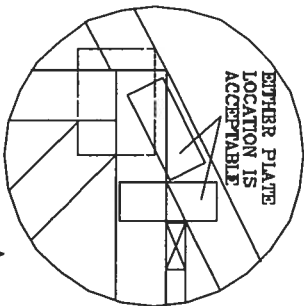
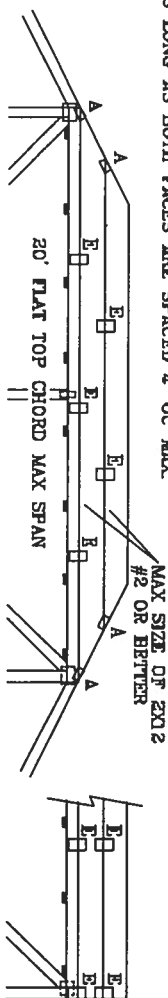
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

310 MFB WIND, 30 MEAN HGT, ASCE 7-03, CLOSED BLDG,
LOCATED ANYWHERE IN BOOE 1 MI FROM COAST

CAT 1, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, SEC
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF
WIND TC DL-5 PSF, WIND BC DL-5 PSF

FRONT FACE (B,*) PLATES MAY BE OFFSET FROM BACK FACE
PLATES AS LONG AS BOTH FACES ARE SPACED 4" OC MAX.



*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE

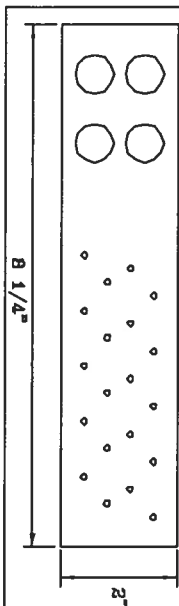
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2K4	2.6X4	2.6X4	3X5
B	4X6	6X6	6X6	6X6
C	1.5X3	1.6X4	1.6X4	1.5X4
D	5X4	6X5	6X5	6X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (6) 0.120" X 1.375" NAILS OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0 TO 7' 0"	NO BRACING
7' 0" TO 10'	1 1/4" BRCE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND BOX LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4" O.C.
10' TO 14'	2x4 1/2" BRCE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND BOX LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4" O.C.

*** PIGGYBACK SPECIAL PLATE**

ATTACH TEETH TO THE PIGBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER PLY. APPLY PIGBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4" OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 647,045

THESE READER EXTREME CARE FABRICATING, HANDLING, SHIPPING, INSTALLING AND REMOVING. REFER TO BEST-10 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE GLASS PLATE MANUFACTURERS OF NORTH CAROLINA, SUITE 200, WATKINS VA 23179 AND VITA CYCLES TRUCK COUNCIL OF AMERICA, 6300 WEST 10TH AVENUE, DENVER CO 80202 FOR SAFETY PRACTICES PRIOR TO CROPPING THE FUNCTION. THESE OTHERWISE INDICATED, THE CHASE SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED PROXY CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.

1469 SW 4th AVENUE
DIERAY BEACH, FL 33444-2181

[illegible]

DATE 11/26/03

DRWGMITEK STD PIGGY

50 PST AT

1.25 DUR. FAC.

47 PSF AT

1.15 DUR. FAC.

-ENG JL

--	--

No: 94B68
STATE OF FLORIDA

SPACING	24.0"
---------	-------

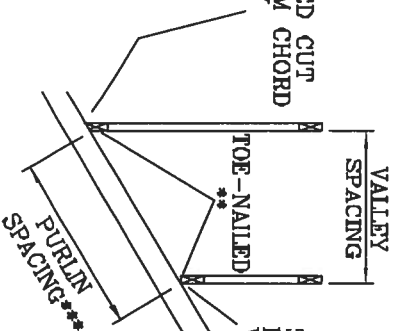
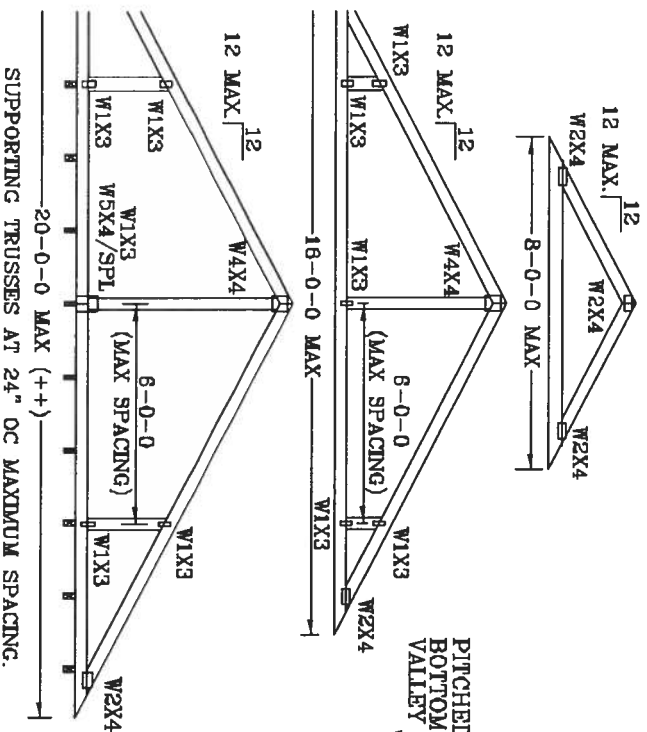
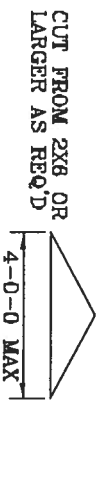
VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:

(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SEC 110 MPH. ASCE 7-83 110 MPH WIND OR (3) 16d FOR ASCE 7-98 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING. EXP. C. RESIDENTIAL. WIND TC D1=5 PSF.



UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "I"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 8" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

OR

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

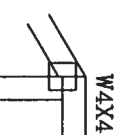
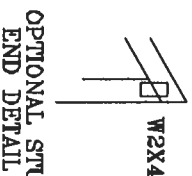
OR

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS

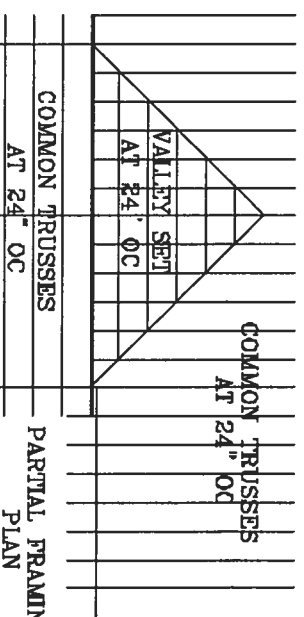
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



OPTIONAL STUB
END DETAIL

OPTIONAL HIP
JOINT DETAIL



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.

THIS DRAWING REPLACES DRAWING A105

[illegible]

**JULIUS LEE'S
CONS. ENGINEERS P.A.**

TC IL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC IL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		

No. 34869
STATE OF FLORIDA

DURFAC 1.2
SPACING1.25
24"

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-1997 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

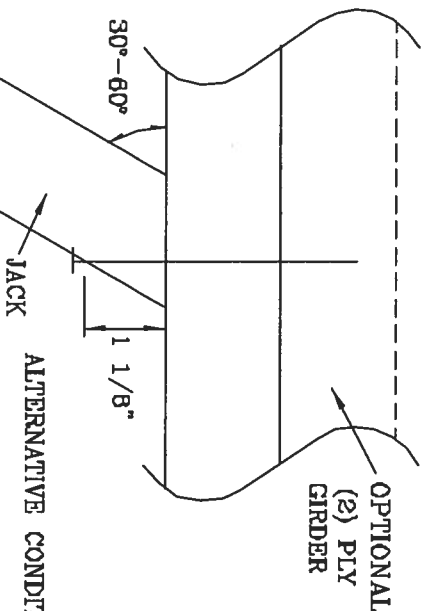
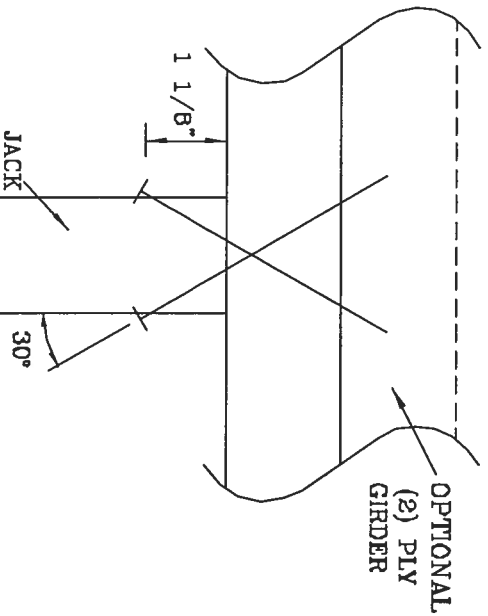
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

MAXIMUM LATERAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS	1 PLY	2 PLYS
2	197#	256#	181#	234#	156#	203#	154#	189#
3	296#	383#	271#	351#	234#	304#	230#	288#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	486#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 588 DUMFRIES DR., SUITE 200, NASHVILLE, TN 37219 AND APCA (AMERICAN WOOD PRESERVATION) TRUSS PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED. STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1468 ST 4TH AVENUE
DECATUR BRANCH, TN 35611-2101

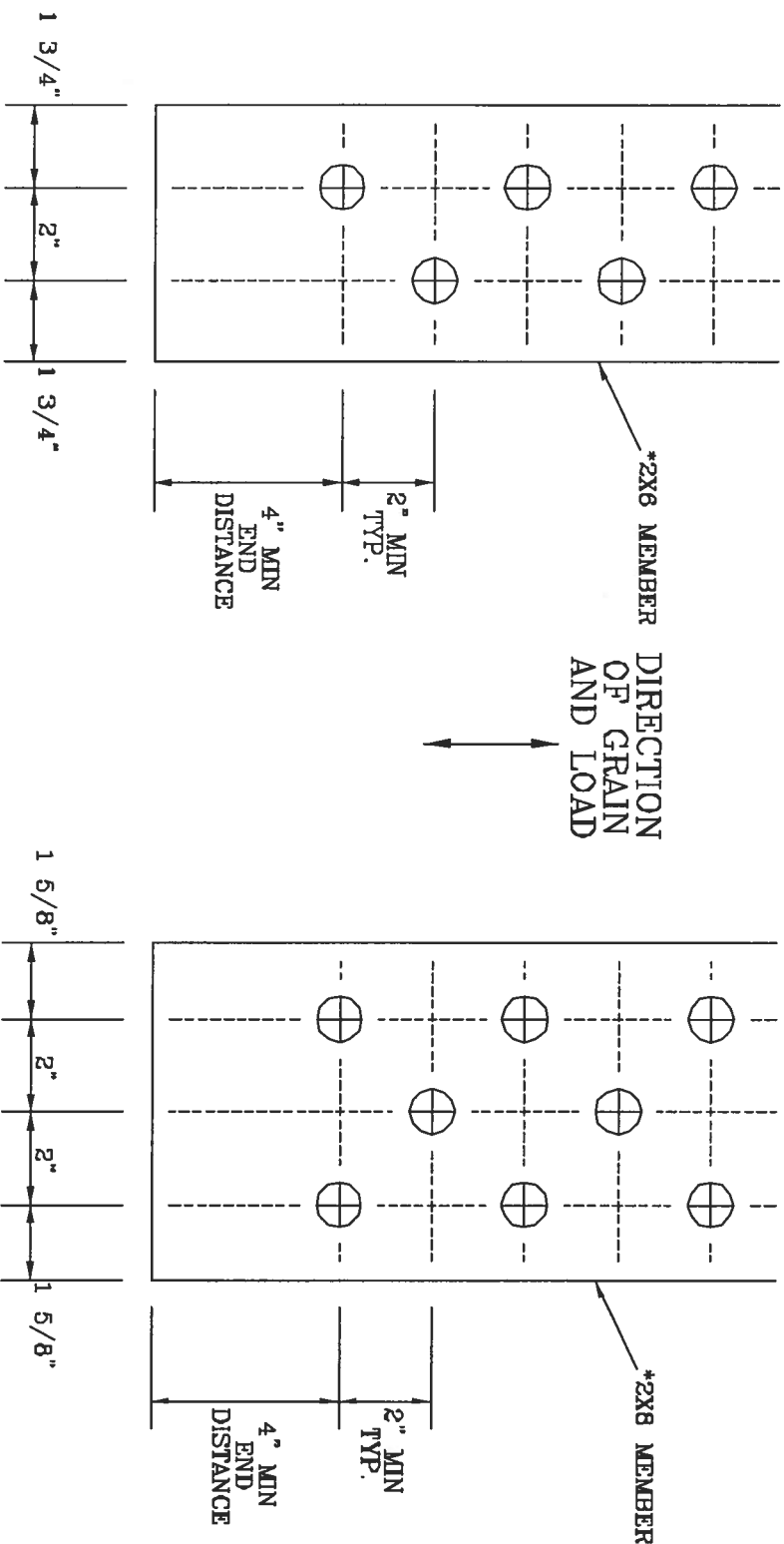
Not 34889
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNTONAIL103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES FOR BUILDING CODES AND SAFETY REGULATIONS, PUBLISHED BY THE TRUSS ASSOCIATION OF AMERICA, 6300 ENTERPRISE LN, HANSON, MA 01920 FOR SAFETY AND PROPERLY ATTACHED STRUCTURAL PANELS AND JOINTS. CHORD SHALL HAVE A PROPERLY ATTACHED RIBBON CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 BY 4TH AVENUE
DELRAY BEACH, FL 33444-2101

No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLSP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		

DUR. FAC.

SPACING

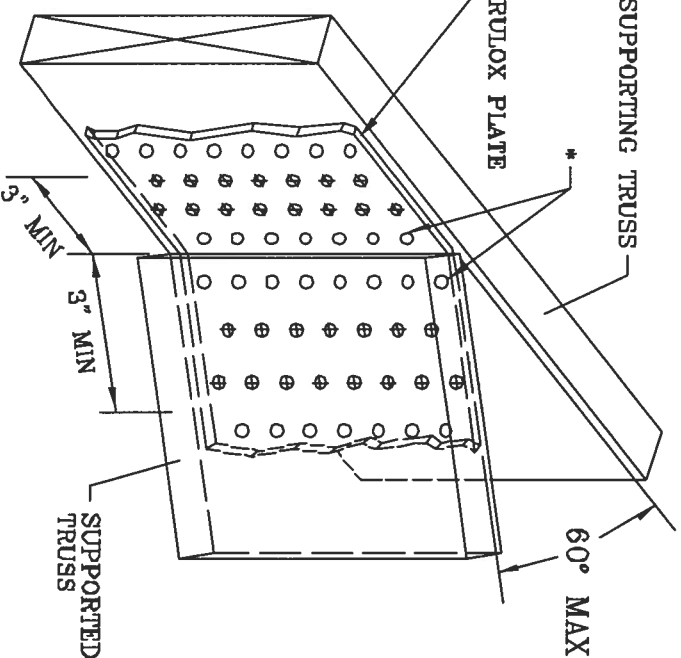
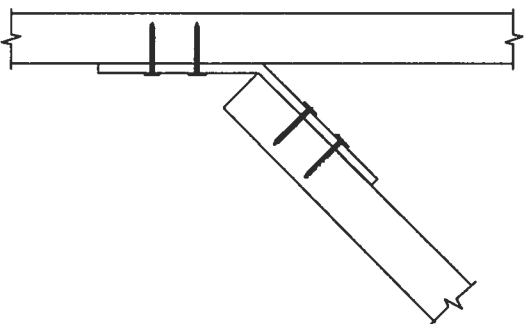
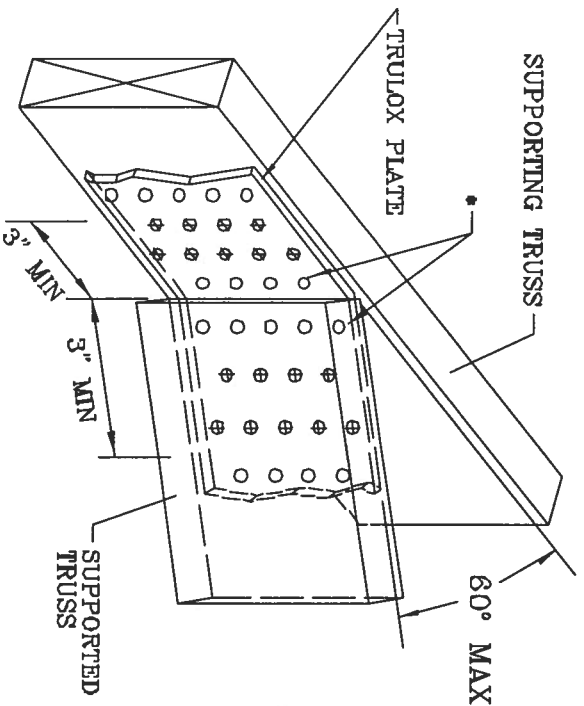
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
5X6	16	990#

THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

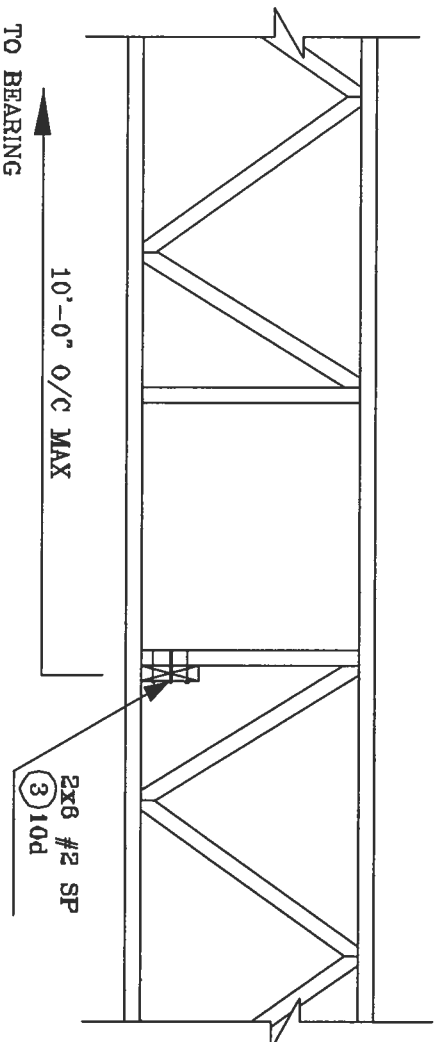
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO ACES 1-60 (BUILDING DEPARTMENT SAFETY DEPARTMENT, PUBLISHED BY THE TRUSS PLATE INSTITUTE, 583 DUNDRETT DR., SUITE 200, WOODEN, VA 23790) AND VITA (VEDO TRUSS CONSTRUCTION) FOR ADDITIONAL INFORMATION. THESE INSTRUCTIONS MUST BE OBSERVED PRIOR TO PERFORMING STRUCTURAL PANELS AND JOINTS. THESE PANELS MUST HAVE A PROPERLY ATTACHED ROOF CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DEALAT BEACH, FL 33444-2201

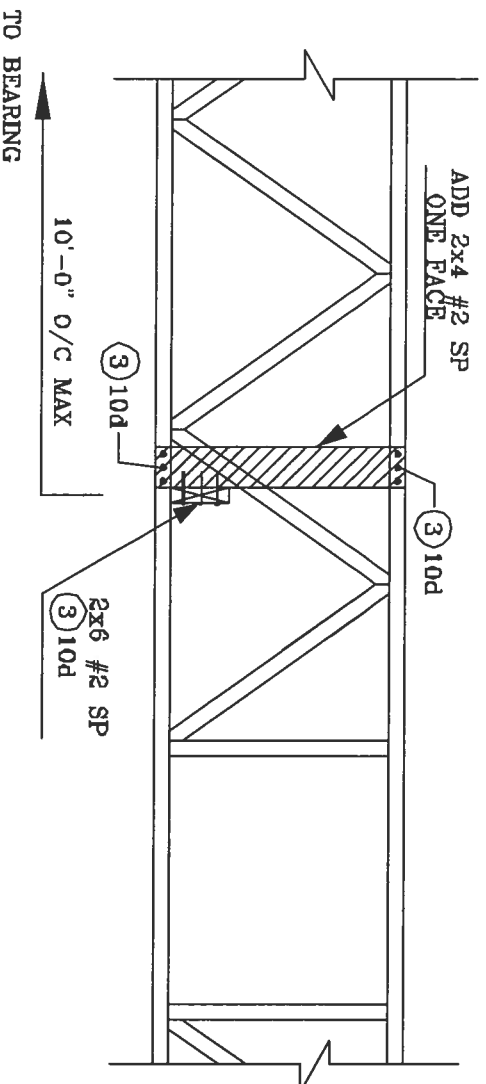
Reg. 34859
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



JULIUS LEE'S
CONS. ENGINEERS P.A.

1426 SW 4TH AVENUE
DIERFLY BRIDGE, FL 33444-2101

No. 34869
STATE OF FLORIDA

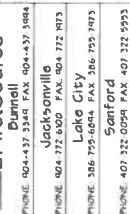
8'-0"
OVERHANG
2'-0"
ROOF PITCH(S)
6/12

[illegible]

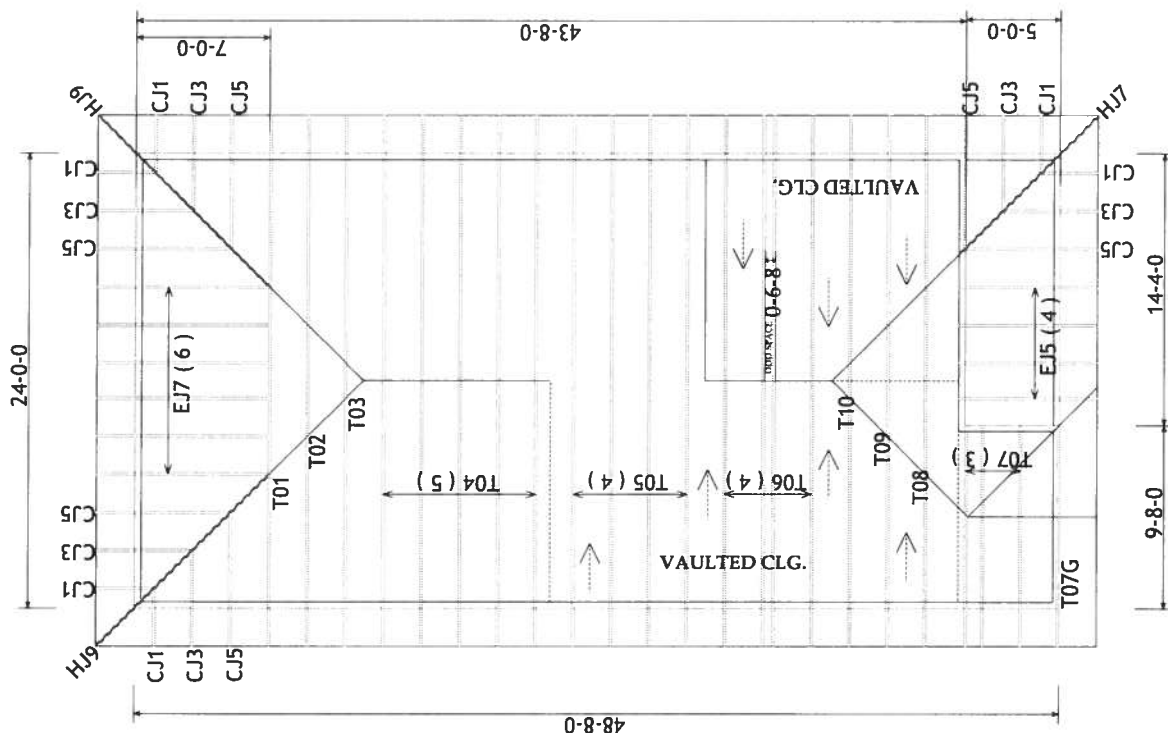
TOA C1 SYSTEM VALUES IN
THIS TABLE ARE APPROXIMATE
VALUES BASED ON THE FOLLOWING
ASSUMPTIONS:

Approved February 1964

Approved by _____ Field _____



BUILDER	RUBY PARK LOT 3
LEGAL ADDRESS	COLUMBIA, FL
MODEL	CUSTOM
DATE	12/15/07
REVISION	SCALE NTS
	DRAWN BY JOB #
	A L250980
	MONDRAGON



NOTE:
VERIFY VAULTED PITCH:

