

DATE 01/16/2008

Columbia County Building Permit

PERMIT

This Permit Must Be Prominently Posted on Premises During Construction

000026620

APPLICANT PATRICK WILSON PHONE 352 206-5459
ADDRESS 6800 SOUTHPOINT PARKWAY JACKSONVILLE FL 32216
OWNER MARONDA HOMES PHONE 407 227-3504
ADDRESS 165 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024
CONTRACTOR THEODORE BROCK PHONE 407 227-3504
LOCATION OF PROPERTY 90W, TL ON 247S, TR ON 252B, TL ON TIMBER RIDGE,
3RD LOT ON RIGHT
TYPE DEVELOPMENT SFD,UTILITY ESTIMATED COST OF CONSTRUCTION 138550.00
HEATED FLOOR AREA 2223.00 TOTAL AREA 2771.00 HEIGHT STORIES 1
FOUNDATION CONC WALLS FRAMED ROOF PITCH 6/12 FLOOR SLAB
LAND USE & ZONING RSF-2 MAX. HEIGHT
Minimum Set Back Requirments: STREET-FRONT 25.00 REAR 15.00 SIDE 10.00
NO. EX.D.U. 0 FLOOD ZONE X PP DEVELOPMENT PERMIT NO.

PARCEL ID 10-4S-16-02856-103 SUBDIVISION TIMBERLANDS
LOT 3 BLOCK PHASE UNIT TOTAL ACRES 0.50
000001521 CRC1326913
Culvert Permit No. Culvert Waiver Contractor's License Number Applicant/Owner/Contractor
CULVERT 07-996 BK JH Y
Driveway Connection Septic Tank Number LU & Zoning checked by Approved for Issuance New Resident
COMMENTS: MFE: 99' PER PLAT, ELEVATION CONFIRMATION LETTER NEEDED AT SLAB
 Check # or Cash 861759

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power Foundation Monolithic
 date/app. by date/app. by date/app. by
Under slab rough-in plumbing Slab Sheathing/Nailing
 date/app. by date/app. by date/app. by
Framing Rough-in plumbing above slab and below wood floor
 date/app. by date/app. by
Electrical rough-in Heat & Air Duct Peri. beam (Lintel)
 date/app. by date/app. by date/app. by
Permanent power C.O. Final Culvert
 date/app. by date/app. by date/app. by
M/H tie downs, blocking, electricity and plumbing Pool
 date/app. by date/app. by
Reconnection Pump pole Utility Pole
 date/app. by date/app. by date/app. by
M/H Pole Travel Trailer Re-roof
 date/app. by date/app. by date/app. by

BUILDING PERMIT FEE \$ 659.00 CERTIFICATION FEE \$ 13.86 SURCHARGE FEE \$ 13.86
MISC. FEES \$ 0.00 ZONING CERT. FEE \$ 50.00 FIRE FEE \$ 0.00 WASTE FEE \$
FLOOD DEVELOPMENT FEE \$ FLOOD ZONE FEE \$ 25.00 CULVERT FEE \$ 25.00 TOTAL FEE 786.72
INSPECTORS OFFICE CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

For Office Use Only Application # 0801-07 Date Received 1/3/08 By TW Permit # 1521/26620
Zoning Official BLK Date 08.01.08 Flood Zone Xprph FEMA Map # N/A Zoning RSF-2
Land Use RES Low Den Elevation N/A MFE 99' plat River N/A Plans Examiner OK JTH Date 1-7-08
Comments Elevation Confirmation Letter Required
☒ NOC ☒ EH ☒ Deed or PA ☒ Site Plan ☐ State Road Info ☐ Parent Parcel #
☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor
☐ Unincorporated area ☒ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 07-996 Fax (904)-332-6367

Name Authorized Person Signing Permit Theodore C. Brock/Melissa McKague Phone (904)-296-1490

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490

911 Address 165 SW Timber Ridge Drive, L.P., 71 32024

Contractors Name Theodore C. Brock Phone (407)-227-3504

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771

Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02856-103 Estimated Cost of Construction 77,722

Subdivision Name Timberlands Lot 3 Block - Unit - Phase 1

Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge 3rd Lot

on Right

Number of Existing Dwellings on Property 0

Construction of Residential Single Family Dwelling Total Acreage .50 Lot Size -

Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height -

Actual Distance of Structure from Property Lines - Front 50 Side 39.6 Side 39.2 Rear 84.6

Number of Stories 2 Heated Floor Area 2223 Total Floor Area 2711 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.



Greater Lake City Regional Utility Authority

205 North Marion Avenue
LAKE CITY, FLORIDA 32055-3918
TELEPHONE: 386.719.5778 FAX: 386.719.5837
E-mail: customer.service@ci.lake-city.fl.us

APPLICATION FOR WATER /SEWER/GAS TAP-CAPACITY COMMITMENT

Project Name: Timberlands Date/Time: 12/10/07
Service Address: 105 SW Timber Ridge Dr
Applicant Name: Maronda Homes Telephone Number: (904)-296-1490
Applicant's Agent: Melissa McKague Telephone Number: (904)-296-1490 ext. 117
Business Name: Maronda Homes Inc. of Telephone Number: (904)-296-1490
Parcel ID #'s: _____
Mailing Address: 6800 Southpoint Pkwy #300 Jacksonville, FL 32216

Requested Water Capacity: ☐ No ☒ Yes, in the amount of _____ gpd/gph
Requested Sewer Capacity: ☐ No ☐ Yes, in the amount of _____ gpd/gph
Requested Gas Capacity: ☐ No ☐ Yes, in the amount of _____ btu/unit

Application For: ☒ Water/Tap Size 3/4 ☐ Sewer/Tap Size _____
☐ Irrigation/Tap Size _____ ☐ Gas/Tap Size _____

Meter Size /Quantity: ☒ 3/4" ☐ 1" ☐ 1 1/2" ☐ 2" ☐ 6" ☐ Other Specify _____

Within City limits: ☐ Yes ☒ No

Fire Protection: ☒ No ☐ Yes, Diameter of new mainline _____
Quantity of new Hydrants: _____

Growth Management Zoned: _____ ☒ Residential ☐ Commercial ☐ Industrial

The above named applicant request that an inspection be made by the Greater Lake City Regional Utility Authority for verification of available services based on address, parcel numbers, lot numbers, etc. Upon confirmation of both capacity and or availability, the applicant will be notified and provided a "Cost Estimate/Tap & Impact Fees" summary.

NOTE: This is only an estimate, fees are subject to change.

Applicant: Melissa McKague

Date: 12/10/07

Service Available Date Requested

Date: ASAP

Billing will begin upon completion of tap.



Greater Lake City Regional Utility Authority

205 North Marion Avenue
LAKE CITY, FLORIDA 32055-3918
TELEPHONE: 386.719.5778 FAX: 386.719.5837
E-mail: customer.service@ci.lake-city.fl.us

OFFICE USE ONLY:

Wastewater Capacity Available

Director _____

Date _____

Collections/Distribution Available

Director _____

Date _____

Water Capacity Available

Director _____

Date _____

Natural Gas Available

Director _____

Date _____

The Greater Lake City Regional Utility Authority has reviewed the applicants request for availability/reserved capacity. Upon this evaluation service connection has been

☐ Accepted

☐ Declined due to _____

Greater Lake City Regional Utility Authority - Customer Service

Application Fee Paid

Application Number: _____

Amount: \$ _____ Check # _____ Type of Establishment _____

Additional Information:

Tap and Impact

Representative _____

Date _____



STATE OF FLORIDA
DEPARTMENT OF HEALTH

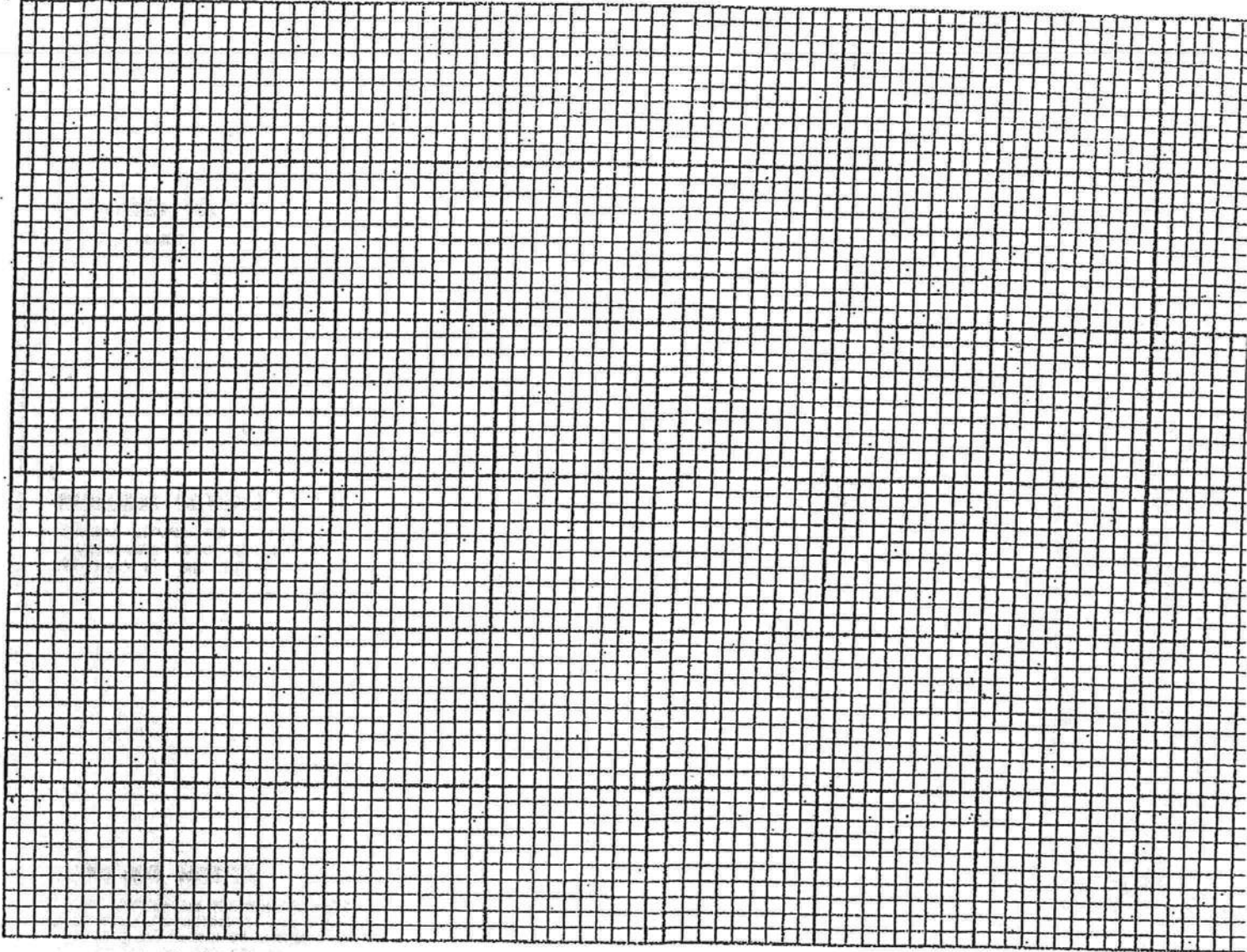
See attached.

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0996

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

Site Plan submitted by:

Jessie

Abner
Signature

Agent
Title

Plan Approved ☒

Not Approved ☐

12-28-07 Date 2007

By Mr. O. L.

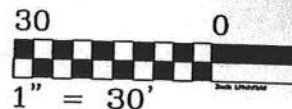
Columbia

County Health Department

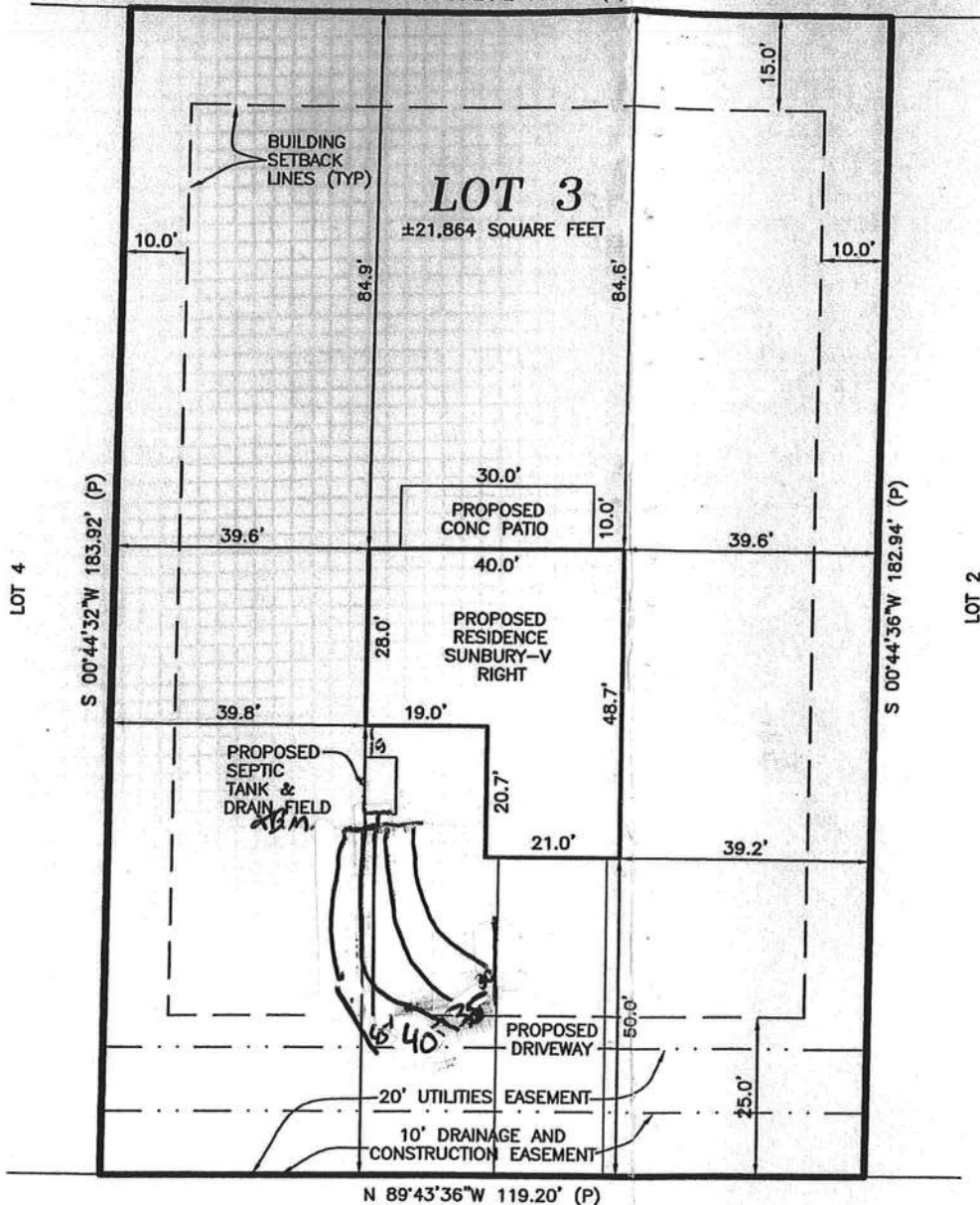
ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA



S 89°15'24"E 119.20' (P)



N 89°43'36"W 119.20' (P)

SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- ⊙ = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
- = SET 4" X 4" CONC. MON. P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M. 5582
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
- ⊞ = CATV RISER
- ⊞ = TELEPHONE PEDESTAL
- ⊞ = WOOD POWER POLE

**THIS IS NO
CERTIFICATE**

NOT VALID WITHOUT THE
A FLORIDA LICENSED
TO THIS MAP BY ANY

I HEREBY CERTIFY THAT
TRUE AND CORRECT
MY SUPERVISION OF
THE MINIMUM TECHNICAL
BOARD OF LAND SURVEYING
STATUTES, AND CHAPTER

BY: *James E. Brinkman*
JAMES E. BRINKMAN,

DATE: 10/13/07



BRINKMAN

4607 NW 6th

PHONE: (352)

SCALE: 1" = 30'

DATE: 10/15/07

"THE

THIS IS NOT A BOUNDARY SURVEY

PREPARED FOR: MARONDA HOLT

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

THIS SURVEYOR, ACCORDING TO THE
INSURANCE PROGRAM, FLOOD INSURANCE RATE
ANAL NO. 120070-0175-B, DATED 1-6-88,
IN FLOOD ZONE "X" WHICH IS AN AREA
OUTSIDE 500-YEAR FLOOD PLAIN, AS
MAP. INFORMATION FROM THE FEDERAL
EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE
MAP ON THIS MAP, WAS CURRENT AS OF THE
MAP REVISIONS AND AMENDMENTS ARE
BY LETTER AND MAY NOT BE REFLECTED
CURRENT MAP.

This instrument prepared by:
William J. Haley, Esquire
Brannon, Brown,
Haley & Bullock, P. A.
P. O. Box 1029
Lake City, FL 32056-1029

Inst:200712024085 Date:10/26/2007 Time:4:45 PM
Doc Stamp-Deed:3276.00
22 DC,P.DeWitt Cason,Columbia County Page 1 of 2

SPECIAL WARRANTY DEED

THIS INDENTURE, made this 10th day of October, 2007, between **RML HOLDINGS, INC.**, a Florida corporation, having a mailing address of 703 NW Blackberry Circle, Lake City, Florida 32055, hereinafter referred to as Grantor, and **MARONDA HOMES, INC. OF FLORIDA**, a Florida corporation, having a mailing address of 3993 West First Street, Sanford, Florida 32771, hereinafter referred to as Grantee.

WITNESSETH: That said Grantor, for and in consideration of the sum of \$10.00 and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt and sufficiency of which are hereby acknowledged, have granted, bargained and sold to the said Grantee, and Grantee's successors and assigns forever, the following described land, situate, lying and being in **Columbia** County, Florida, to-wit:

Lot(s) 2, 3, 4, 5, 6, 7, 8, 21, 22, 23, 25, and 26, **TIMBERLANDS, PHASE 1**, according to the plat thereof, as recorded in Plat Book 9, pages 26-27, public records of Columbia County, Florida.

PARCEL NO. Part of 10-4S-16-02856-000 and 15-4S-16-02990-000

SUBJECT TO: Taxes and special assessments for the year 2007 and subsequent years; restrictions, reservations, rights of way for public roads, easements of record, if any; and zoning and any other governmental restrictions regulating the use of the lands.

and said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons claiming by, through or under said Grantor.

IN WITNESS WHEREOF, Grantor has hereunto set its hand and seal the day and year first above written.

Signed, sealed and delivered
in the presence of:

RML HOLDINGS, INC., a Florida
corporation

Linda Brown
Print Name: Linda Brown

By: Margaret Lardizabal
Margaret Lardizabal
Vice President

Debbie G. Moore
Print Name: Debbie G. Moore

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 10th day of October, 2007, by Margaret Lardizabal, as Vice President of RML Holdings, Inc., a Florida corporation, on behalf of said corporation, who is personally known to me.

Debbie G. Moore
Notary Public, State of Florida



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

3/1 TM

Project Name: SUNBURY GAINESVILLE Address: 1105 SW Timber Ridge Dr City, State: Lake City, FL 32055 Owner: ELECTRIC Climate Zone: North	Builder: MARONDA HOMES Permitting Office: Columbia Permit Number: 26620 Jurisdiction Number: 221000
--	--

1. New construction or existing New ☐ 2. Single family or multi-family Single family ☐ 3. Number of units, if multi-family 1 ☐ 4. Number of Bedrooms 4 ☐ 5. Is this a worst case? Yes ☐ 6. Conditioned floor area (ft²) 2223 ft² ☐ 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) <table style="width: 100%;"> <tr> <td style="width: 30%;">a. U-factor:</td> <td style="width: 30%;">Description</td> <td style="width: 40%;">Area</td> </tr> <tr> <td>(or Single or Double DEFAULT)</td> <td>7a (Single Default)</td> <td>263.0 ft²</td> </tr> <tr> <td>b. SHGC:</td> <td></td> <td></td> </tr> <tr> <td>(or Clear or Tint DEFAULT)</td> <td>7b. (Clear)</td> <td>263.0 ft²</td> </tr> </table> 8. Floor types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Slab-On-Grade Edge Insulation</td> <td style="width: 30%;">R=0.0, 136.0(p) ft</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 9. Wall types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Concrete, Int Insul, Exterior</td> <td style="width: 30%;">R=4.1, 720.0 ft²</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. Frame, Wood, Exterior</td> <td>R=11.0, 992.0 ft²</td> <td></td> </tr> <tr> <td>c. Frame, Steel, Adjacent</td> <td>R=11.0, 159.0 ft²</td> <td></td> </tr> <tr> <td>d. N/A</td> <td></td> <td></td> </tr> <tr> <td>e. N/A</td> <td></td> <td></td> </tr> </table> 10. Ceiling types <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Under Attic</td> <td style="width: 30%;">R=19.0, 1120.0 ft²</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 11. Ducts <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Sup: Unc. Ret: Con. AH(Sealed):Interior</td> <td style="width: 30%;">Sup. R=6.0, 150.0 ft</td> <td style="width: 40%;"></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> </table>	a. U-factor:	Description	Area	(or Single or Double DEFAULT)	7a (Single Default)	263.0 ft²	b. SHGC:			(or Clear or Tint DEFAULT)	7b. (Clear)	263.0 ft²	a. Slab-On-Grade Edge Insulation	R=0.0, 136.0(p) ft		b. N/A			c. N/A			a. Concrete, Int Insul, Exterior	R=4.1, 720.0 ft²		b. Frame, Wood, Exterior	R=11.0, 992.0 ft²		c. Frame, Steel, Adjacent	R=11.0, 159.0 ft²		d. N/A			e. N/A			a. Under Attic	R=19.0, 1120.0 ft²		b. N/A			c. N/A			a. Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 150.0 ft		b. N/A			12. Cooling systems <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Central Unit</td> <td style="width: 30%;">Cap: 42.5 kBtu/hr</td> <td style="width: 40%;"></td> </tr> <tr> <td></td> <td>SEER: 14.00</td> <td></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 13. Heating systems <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Electric Heat Pump</td> <td style="width: 30%;">Cap: 42.5 kBtu/hr</td> <td style="width: 40%;"></td> </tr> <tr> <td></td> <td>HSPF: 8.70</td> <td></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. N/A</td> <td></td> <td></td> </tr> </table> 14. Hot water systems <table style="width: 100%;"> <tr> <td style="width: 30%;">a. Electric Resistance</td> <td style="width: 30%;">Cap: 50.0 gallons</td> <td style="width: 40%;"></td> </tr> <tr> <td></td> <td>EF: 0.90</td> <td></td> </tr> <tr> <td>b. N/A</td> <td></td> <td></td> </tr> <tr> <td>c. Conservation credits</td> <td></td> <td></td> </tr> <tr> <td colspan="3">(HR-Heat recovery, Solar DHP-Dedicated heat pump)</td> </tr> </table> 15. HVAC credits PT, ☐ <table style="width: 100%;"> <tr> <td style="width: 30%;">(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)</td> <td style="width: 70%;"></td> </tr> </table>	a. Central Unit	Cap: 42.5 kBtu/hr			SEER: 14.00		b. N/A			c. N/A			a. Electric Heat Pump	Cap: 42.5 kBtu/hr			HSPF: 8.70		b. N/A			c. N/A			a. Electric Resistance	Cap: 50.0 gallons			EF: 0.90		b. N/A			c. Conservation credits			(HR-Heat recovery, Solar DHP-Dedicated heat pump)			(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
a. U-factor:	Description	Area																																																																																											
(or Single or Double DEFAULT)	7a (Single Default)	263.0 ft²																																																																																											
b. SHGC:																																																																																													
(or Clear or Tint DEFAULT)	7b. (Clear)	263.0 ft²																																																																																											
a. Slab-On-Grade Edge Insulation	R=0.0, 136.0(p) ft																																																																																												
b. N/A																																																																																													
c. N/A																																																																																													
a. Concrete, Int Insul, Exterior	R=4.1, 720.0 ft²																																																																																												
b. Frame, Wood, Exterior	R=11.0, 992.0 ft²																																																																																												
c. Frame, Steel, Adjacent	R=11.0, 159.0 ft²																																																																																												
d. N/A																																																																																													
e. N/A																																																																																													
a. Under Attic	R=19.0, 1120.0 ft²																																																																																												
b. N/A																																																																																													
c. N/A																																																																																													
a. Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 150.0 ft																																																																																												
b. N/A																																																																																													
a. Central Unit	Cap: 42.5 kBtu/hr																																																																																												
	SEER: 14.00																																																																																												
b. N/A																																																																																													
c. N/A																																																																																													
a. Electric Heat Pump	Cap: 42.5 kBtu/hr																																																																																												
	HSPF: 8.70																																																																																												
b. N/A																																																																																													
c. N/A																																																																																													
a. Electric Resistance	Cap: 50.0 gallons																																																																																												
	EF: 0.90																																																																																												
b. N/A																																																																																													
c. Conservation credits																																																																																													
(HR-Heat recovery, Solar DHP-Dedicated heat pump)																																																																																													
(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)																																																																																													

Glass/Floor Area: 0.12 Total as-built points: 27736
Total base points: 30122

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [Signature]

DATE: Gary Carmack 12/13/10

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: Melissa McNamee

DATE: Melissa McNamee 12/13/10

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	2223.0	18.59	7439.0	1.Single, Clear	E	1.0	6.0	30.0	47.92	0.97	1394.0
				2.Single, Clear	E	1.0	15.0	30.0	47.92	1.00	1432.0
				3.Single, Clear	E	1.0	4.0	9.0	47.92	0.91	393.0
				4.Single, Clear	E	1.0	4.0	6.0	47.92	0.91	262.0
				5.Single, Clear	E	1.0	4.0	6.0	47.92	0.91	262.0
				6.Single, Clear	N	1.0	15.0	30.0	21.73	1.00	649.0
				7.Single, Clear	W	1.0	16.0	40.0	43.84	1.00	1748.0
				8.Single, Clear	W	1.0	15.0	30.0	43.84	1.00	1311.0
				9.Single, Clear	W	1.0	13.0	9.0	43.84	1.00	393.0
				10.Single, Clear	W	1.0	5.5	15.0	43.84	0.96	632.0
				11.Single, Clear	W	1.0	5.5	15.0	43.84	0.96	632.0
				12.Single, Clear	W	1.0	5.5	30.0	43.84	0.96	1265.0
				13.Single, Clear	E	4.0	2.5	8.0	47.92	0.42	161.0
				14.Single, Clear	E	4.0	7.0	5.0	47.92	0.68	162.0
				As-Built Total:				263.0	10696.0		
WALL TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM = Points	
Adjacent	159.0	0.70	111.3	1. Concrete, Int Insul, Exterior		4.1		720.0	1.13	817.2	
Exterior	1712.0	1.70	2910.4	2. Frame, Wood, Exterior		11.0		992.0	1.70	1686.4	
				3. Frame, Steel, Adjacent		11.0		159.0	1.00	159.0	
Base Total:	1871.0		3021.7	As-Built Total:				1871.0		2662.6	
DOOR TYPES				Area X BSPM = Points		Type		Area X SPM = Points			
Adjacent	17.0	2.40	40.8	1.Exterior Insulated				20.0	4.10	82.0	
Exterior	20.0	6.10	122.0	2.Adjacent Wood				17.0	2.40	40.8	
Base Total:	37.0		162.8	As-Built Total:				37.0		122.8	
CEILING TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM X SCM = Points	
Under Attic	1120.0	1.73	1937.6	1. Under Attic		19.0		1120.0	2.34 X 1.00	2620.8	
Base Total:	1120.0		1937.6	As-Built Total:				1120.0		2620.8	
FLOOR TYPES				Area X BSPM = Points		Type		R-Value		Area X SPM = Points	
Slab	136.0(p)	-37.0	-5032.0	1. Slab-On-Grade Edge Insulation		0.0		136.0(p)	-41.20	-5603.2	
Raised	0.0	0.00	0.0								
Base Total:			-5032.0	As-Built Total:				136.0		-5603.2	

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE			AS-BUILT			
INFILTRATION	Area X	BSPM = Points	Area X SPM = Points			
	2223.0	10.21 22696.8	2223.0 10.21 22696.8			
Summer Base Points: 30225.9			Summer As-Built Points: 33195.8			
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier	X System Multiplier X Credit Multiplier = Cooling Points
30225.9	0.3250	9823.4	(sys 1: Central Unit 42500btuh ,SEER/EFF(14.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 33196 1.00 (1.08 x 1.147 x 0.86) 0.244 0.950 8240.8 33195.8 1.00 1.072 0.244 0.950 8240.8			

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	2223.0	20.17	8071.0	1.Single, Clear	E	1.0	6.0	30.0	26.41	1.02	804.0
				2.Single, Clear	E	1.0	15.0	30.0	26.41	1.00	795.0
				3.Single, Clear	E	1.0	4.0	9.0	26.41	1.04	246.0
				4.Single, Clear	E	1.0	4.0	6.0	26.41	1.04	164.0
				5.Single, Clear	E	1.0	4.0	6.0	26.41	1.04	164.0
				6.Single, Clear	N	1.0	15.0	30.0	33.22	1.00	996.0
				7.Single, Clear	W	1.0	16.0	40.0	28.84	1.00	1154.0
				8.Single, Clear	W	1.0	15.0	30.0	28.84	1.00	866.0
				9.Single, Clear	W	1.0	13.0	9.0	28.84	1.00	259.0
				10.Single, Clear	W	1.0	5.5	15.0	28.84	1.01	436.0
				11.Single, Clear	W	1.0	5.5	15.0	28.84	1.01	436.0
				12.Single, Clear	W	1.0	5.5	30.0	28.84	1.01	873.0
				13.Single, Clear	E	4.0	2.5	8.0	26.41	1.41	297.0
				14.Single, Clear	E	4.0	7.0	5.0	26.41	1.15	151.0
				As-Built Total:				263.0	7641.0		
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	159.0	3.60	572.4	1. Concrete, Int Insul, Exterior	4.1		720.0	6.42		4622.4	
Exterior	1712.0	3.70	6334.4	2. Frame, Wood, Exterior	11.0		992.0	3.70		3670.4	
				3. Frame, Steel, Adjacent	11.0		159.0	5.20		826.8	
Base Total:	1871.0		6906.8	As-Built Total:		1871.0		9119.6			
DOOR TYPES Area X BWPM = Points				Type			Area X WPM = Points				
Adjacent	17.0	11.50	195.5	1.Exterior Insulated			20.0	8.40		168.0	
Exterior	20.0	12.30	246.0	2.Adjacent Wood			17.0	11.50		195.5	
Base Total:	37.0		441.5	As-Built Total:		37.0		363.5			
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1120.0	2.05	2296.0	1. Under Attic	19.0		1120.0	2.70 X 1.00		3024.0	
Base Total:	1120.0		2296.0	As-Built Total:		1120.0		3024.0			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	136.0(p)	8.9	1210.4	1. Slab-On-Grade Edge Insulation	0.0		136.0(p)	18.80		2556.8	
Raised	0.0	0.00	0.0								
Base Total:			1210.4	As-Built Total:		136.0		2556.8			

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT				
INFILTRATION Area X BWPM = Points				Area X WPM = Points				
2223.0 -0.59 -1311.6				2223.0 -0.59 -1311.6				
Winter Base Points: 17614.1				Winter As-Built Points: 21393.3				
Total Winter X System = Heating Points Multiplier Points				Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)				
17614.1 0.5540 9758.2				(sys 1: Electric Heat Pump 42500 btuh ,EFF(8.7) Ducts:Unc(S),Con(R),Int(AH),R6.0 21393.3 1.000 (1.060 x 1.169 x 0.88) 0.392 0.950 8721.0 21393.3 1.00 1.095 0.392 0.950 8721.0				

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE					AS-BUILT								
WATER HEATING													
Number of Bedrooms	X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	X	Tank X Ratio	Multiplier X	Credit	=	Total
4		2635.00		10540.0	50.0	0.90	4		1.00	2693.56	1.00		10774.2
					As-Built Total:								10774.2

CODE COMPLIANCE STATUS											
BASE						AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
9823		9758		10540	30122	8241		8721		10774	27736

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	✓
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.0

The higher the score, the more efficient the home.

ELECTRIC, Lot: , Sub: , Plat: , , ,

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 42.5 kBtu/hr
3. Number of units, if multi-family	1		SEER: 14.00
4. Number of Bedrooms	4	b. N/A	
5. Is this a worst case?	Yes	c. N/A	
6. Conditioned floor area (ft ²)	2223 ft ²		
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		13. Heating systems	
a. U-factor:	Description Area	a. Electric Heat Pump	Cap: 42.5 kBtu/hr
(or Single or Double DEFAULT) 7a(Sngle Default) 263.0 ft ²			HSPF: 8.70
b. SHGC:		b. N/A	
(or Clear or Tint DEFAULT) 7b. (Clear) 263.0 ft ²		c. N/A	
8. Floor types		14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 136.0(p) ft	a. Electric Resistance	Cap: 50.0 gallons
b. N/A			EF: 0.90
c. N/A		b. N/A	
9. Wall types		c. Conservation credits	
a. Concrete, Int Insul, Exterior	R=4.1, 720.0 ft ²	(HR-Heat recovery, Solar	
b. Frame, Wood, Exterior	R=11.0, 992.0 ft ²	DHP-Dedicated heat pump)	
c. Frame, Steel, Adjacent	R=11.0, 159.0 ft ²	15. HVAC credits	PT,
d. N/A		(CF-Ceiling fan, CV-Cross ventilation,	
e. N/A		HF-Whole house fan,	
10. Ceiling types		PT-Programmable Thermostat,	
a. Under Attic	R=19.0, 1120.0 ft ²	MZ-C-Multizone cooling,	
b. N/A		MZ-H-Multizone heating)	
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Con. AH(Sealed):Interior	Sup. R=6.0, 150.0 ft		
b. N/A			

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Melina Mergane Date: 12/13/10

Address of New Home: 1105 SW Timber Ridge Dr City/FL Zip: Lake City, FL 32055



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5)



Project Summary
Entire House
MARONDA HOMES

Job: SUNBURY
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: SUNBURY

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	39178 Btuh
Ducts	4273 Btuh
Central vent (100 cfm)	4048 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	47499 Btuh

Sensible Cooling Equipment Load Sizing

Structure	19600 Btuh
Ducts	9255 Btuh
Central vent (100 cfm)	1860 Btuh
Blower	0 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft²)	2223	2223
Volume (ft³)	17517	17517
Air changes/hour	0.32	0.16
Equiv. AVF (cfm)	93	47

Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	29794 Btuh

Latent Cooling Equipment Load Sizing

Structure	2842 Btuh
Ducts	2144 Btuh
Central vent (100 cfm)	3516 Btuh
Equipment latent load	8502 Btuh

Equipment total load	38295 Btuh
Req. total capacity at 0.76 SHR	3.3 ton

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H342AKA

Efficiency	8.7 HSPF
Heating input	
Heating output	0 Btuh @ 47°F
Temperature rise	0 °F
Actual air flow	0 cfm
Air flow factor	0.000 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Cooling Equipment Summary

Make	TEMPSTAR
Trade	HEAT PUMP
Cond	N4H342AKA
Coil	FEM4X4800
Efficiency	14 SEER
Sensible cooling	32300 Btuh
Latent cooling	10200 Btuh
Total cooling	42500 Btuh
Actual air flow	1240 cfm
Air flow factor	0.043 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.78

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



wrightsoft Right-Suite Residential 6.0.90 RSR21115

C:\Documents and Settings\gcarmack\My Documents\Man J Calcs 2007\GAINESVILLE\SUNBURY .rrp Calc =

2007-Sep-12 13:06:05

Page 1

Project Information

For: SUNBURY

Design Conditions

Location:

Gainesville, FL, US
Elevation: 151 ft
Latitude: 30°N

Outdoor:

Dry bulb (°F)
Daily range (°F)
Wet bulb (°F)
Wind speed (mph)

Heating

33
-
-
15.0

Cooling

92
19 (M)
77
7.5

Indoor:

Indoor temperature (°F)
Design TD (°F)
Relative humidity (%)
Moisture difference (gr/lb)

Heating

70
37
30
10.6

Cooling

75
17
50
52.0

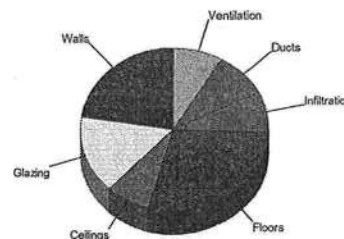
Infiltration:

Method
Construction quality
Fireplaces

Simplified
Average
0

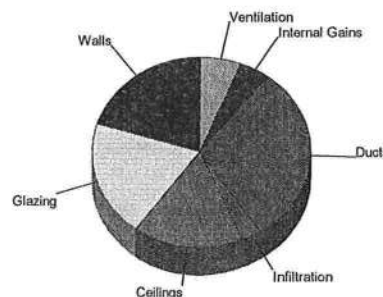
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.1	10723	22.6
Glazing	47.0	7283	15.3
Doors	0.0	0	0.0
Ceilings	1.3	3724	7.8
Floors	28.1	13667	28.8
Infiltration	1.4	3782	8.0
Ducts		4273	9.0
Piping		0	0.0
Humidification		0	0.0
Ventilation		4048	8.5
Adjustments		0	
Total		47499	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.4	6218	20.2
Glazing	38.9	6036	19.7
Doors	0.0	0	0.0
Ceilings	1.8	5097	16.6
Floors	0.0	0	0.0
Infiltration	0.3	869	2.8
Ducts		9255	30.1
Ventilation		1860	6.1
Internal gains		1380	4.5
Blower		0	0.0
Adjustments		0	
Total		30715	100.0



Overall U-value = 0.160 Btuh/ft²-°F

Data entries checked.

Duct System Summary

Entire House

MARONDA HOMES

Job: SUNBURY
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: (407) 321-0064

Project Information

For: SUNBURY

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.48 / 0.12 in H2O	0.48 / 0.12 in H2O
Lowest friction rate	1.935 in/100ft	1.935 in/100ft
Actual air flow	1240 cfm	1240 cfm
Total effective length (TEL)	31 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
KITCHEN	h 423 5	150	150	1.93 5	6	x0	VIFx	25.0	0.0	st1
FAMILY ROOM	h 424 1	140	140	1.93 5	6	x0	VIFx	25.0	0.0	ST2
DINING ROOM	h 399 1	150	150	1.93 5	6	x0	VIFx	25.0	0.0	st1
LIVING ROOM	c 332 9	140	140	9.677	6	x0	VIFx	5.0	0.0	ST3
UTILITY ROOM	h 829	25	25	9.677	4	x0	VIFx	5.0	0.0	ST3
BATHROOM	c 143 2	25	25	9.677	4	x0	VIFx	5.0	0.0	ST3
MASTER BEDROOM	c 330 3	140	140	3.226	6	x0	VIFx	15.0	0.0	ST2
MASTER BATHROOM	c 251 6	100	100	3.226	5	x0	VIFx	15.0	0.0	st5
HALL BATHROOM	c 110 9	25	25	4.839	4	x0	VIFx	10.0	0.0	ST2
TOILET	c 102 3	25	25	3.226	4	x0	VIFx	15.0	0.0	st5
BEDROOM #3	h 451 1	110	110	4.839	5	x0	VIFx	10.0	0.0	st1
BEDROOM #2	c 242 2	100	100	4.839	5	x0	VIFx	10.0	0.0	ST4
BEDROOM #4	c 268 1	110	110	4.839	5	x0	VIFx	10.0	0.0	st1

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	520	520	1.935	515	12	0 x 0	VinFlx	st5
ST2	Peak AVF	620	620	1.935	613	12	0 x 0	VinFlx	
ST3	Peak AVF	190	190	9.677	569	7	0 x 0	VinFlx	
ST4	Peak AVF	100	100	4.839	510	5	0 x 0	VinFlx	
st5	Peak AVF	315	315	3.226	632	10	0 x 0	VinFlx	st2

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	1240	1240	6.0	1.935	568	18	x0 0		VIFx	

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001521

DATE 01/16/2008 PARCEL ID # 10-4S-16-02856-103
APPLICANT PATRICK WILSON PHONE 352 206-5459
ADDRESS 6800 SOUTHPPOINT PARKWAY JACKSONVILLE FL 32216
OWNER MARONDA HOMES PHONE 904 296-1490
ADDRESS 165 SW TIMBER RIDGE DR LAKE CITY FL 32024
CONTRACTOR THEODORE C. BROCK PHONE 407 227-3504
LOCATION OF PROPERTY 90W, TL ON 247S, TR ON 252B, TL ON TIMBER RIDGE, 3RD
LOT ON RIGHT _____

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBERLANDS 3

SIGNATURE _____

INSTALLATION REQUIREMENTS



Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.



Culvert installation shall conform to the approved site plan standards.



Department of Transportation Permit installation approved standards.



Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALLATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



NOTICE OF COMMENCEMENT

26620

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number _____

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT:

1. Description of property (legal description): 3/1 Timberlands
a) Street (job) Address: 1105 SW Timber Ridge Dr.
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc. of FL 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Name and address of fee simple titleholder (if other than owner) _____
c) Interest in property _____
4. Contractor Information
a) Name and address: Theodore C. Brock 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Telephone No.: (407)-227-3504 Fax No. (Opt.) _____
5. Surety Information
a) Name and address: _____
b) Amount of Bond: _____
c) Telephone No.: _____ Fax No. (Opt.) _____
6. Lender
a) Name and address: Bank of America 250 Park Ave S. #400 Winter Park, FL 32789
b) Phone No. _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Kampf Title and Guaranty Corp. P.O. Box 1359 200 W. 1st St.
b) Telephone No.: _____ Fax No. (Opt.) Winter Park, FL 32771
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
a) Name and address: Staff at Maronda Homes Inc. of FL
b) Telephone No.: (904)-296-1490 Fax No. (Opt.) _____
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

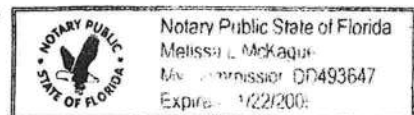
STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 3 day of January, 2008, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
fact) for Maronda Homes Inc. of FL (name of party on behalf of whom instrument was executed).

Personally Known XX OR Produced Identification _____ Type _____

Notary Signature [Signature] Notary Stamp or Seal:



11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
Signature of Natural Person Signing (in line #10 above.)

26620

FIELD DENSITY WORKSHEET

CLIENT MARIONA HOMES DATE 15 APR 08
Lot # 3
 PROJECT NAME TIMBERLANDS LAKE CITY PROJECT NO. 26620
 EARTH-CONTRACTOR 165 SW TIMBER RIDGE DR. PERMIT NO. #000026620
 COMPACTION REQUIREMENT (%) 95% ☐ Standard Proctor TESTED BY JVL
☒ Modified Proctor Patricia FIELD CONTACT
 TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☐ Building Footing ☐ Other _____

TEST LOCATION	LAB PROCTOR		TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
	DENS.	OMC						
CIR. OF POP	103.7	120	1Ft	12"	5.6	108.5	102.8	99.1
CIR. OF E. FTH	2	2	L	L	8.9	108.0	99.1	95.6
CIR. OF W. FTH	2	2	L	L	8.6	108.0	99.5	95.9

REMARKS _____

* Density failed to meet minimum project requirement
 ** Retest indicates minimum density requirement was obtained.
 () Client is aware of unsatisfactory test results.

COLUMBIA COUNTY OFFICE OF CIVIL ENGINEERING

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-103

Building permit No. 000026620

Use Classification SFD, UTILITY

Fire: 12.84

Permit Holder THEODORE BROCK

Waste: 33.50

Owner of Building MARONDA HOMES

Total: 46.34

Location: 165 SW TIMBER RIDGE DRIVE, LAKE CITY, FL

Date: 08/13/2008

Wayne A. Ruess

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

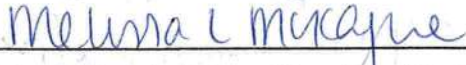

 Owners Signature Theodore C. Brock

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

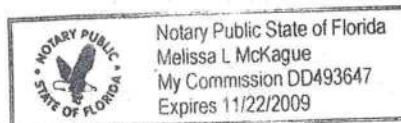

 Contractor's Signature (Permitee) Theodore C. Brock

Contractor's License Number CRC 1326913
 Columbia County
 Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10 day of December 2007.
 Personally known XXX or Produced Identification _____


 State of Florida Notary Signature (For the Contractor)
 Melissa L. McKague

SEAL:

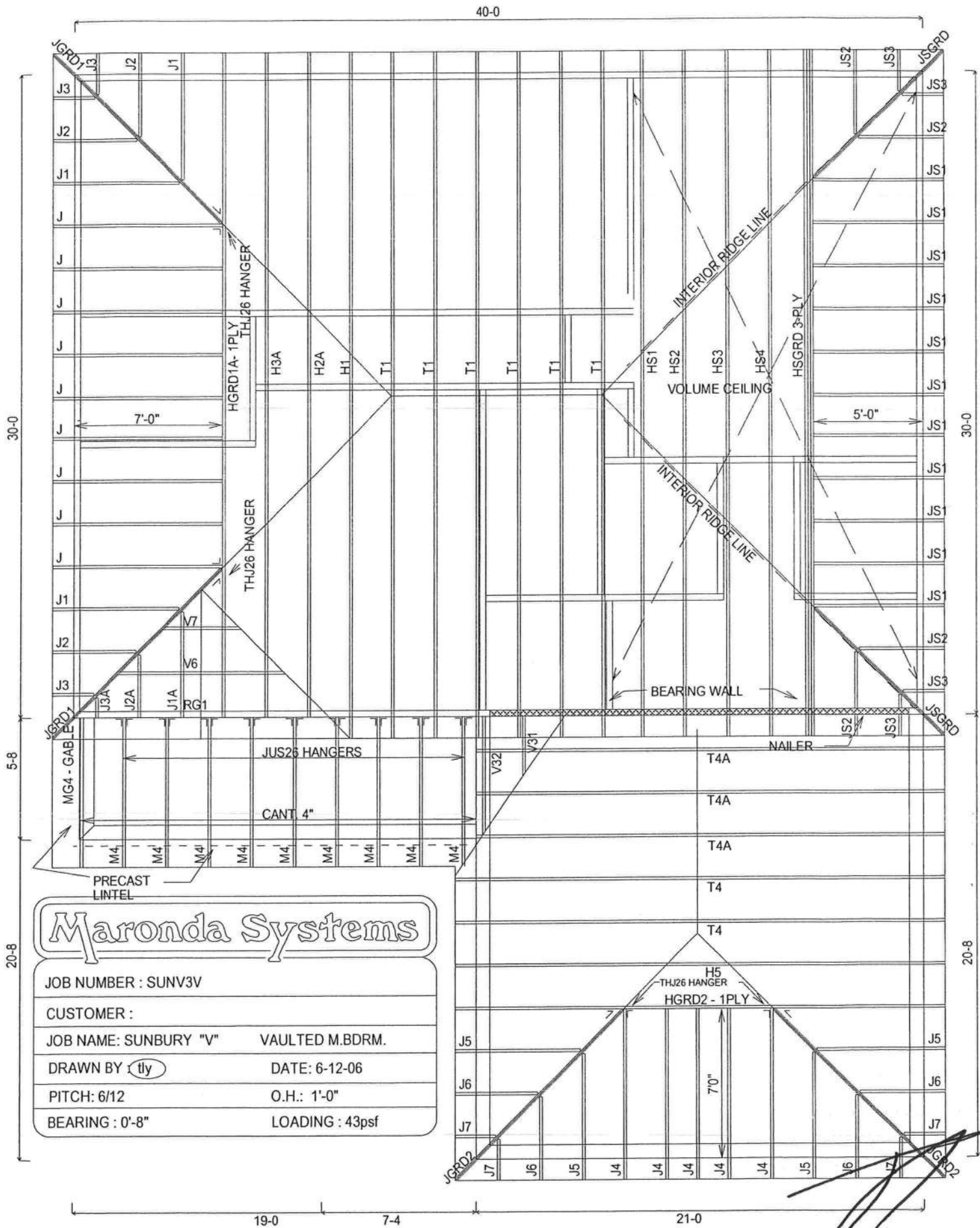


PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL
1TM00301	3-1		ORA-1TM	SUNV3 RIGHT

VAULTED

Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

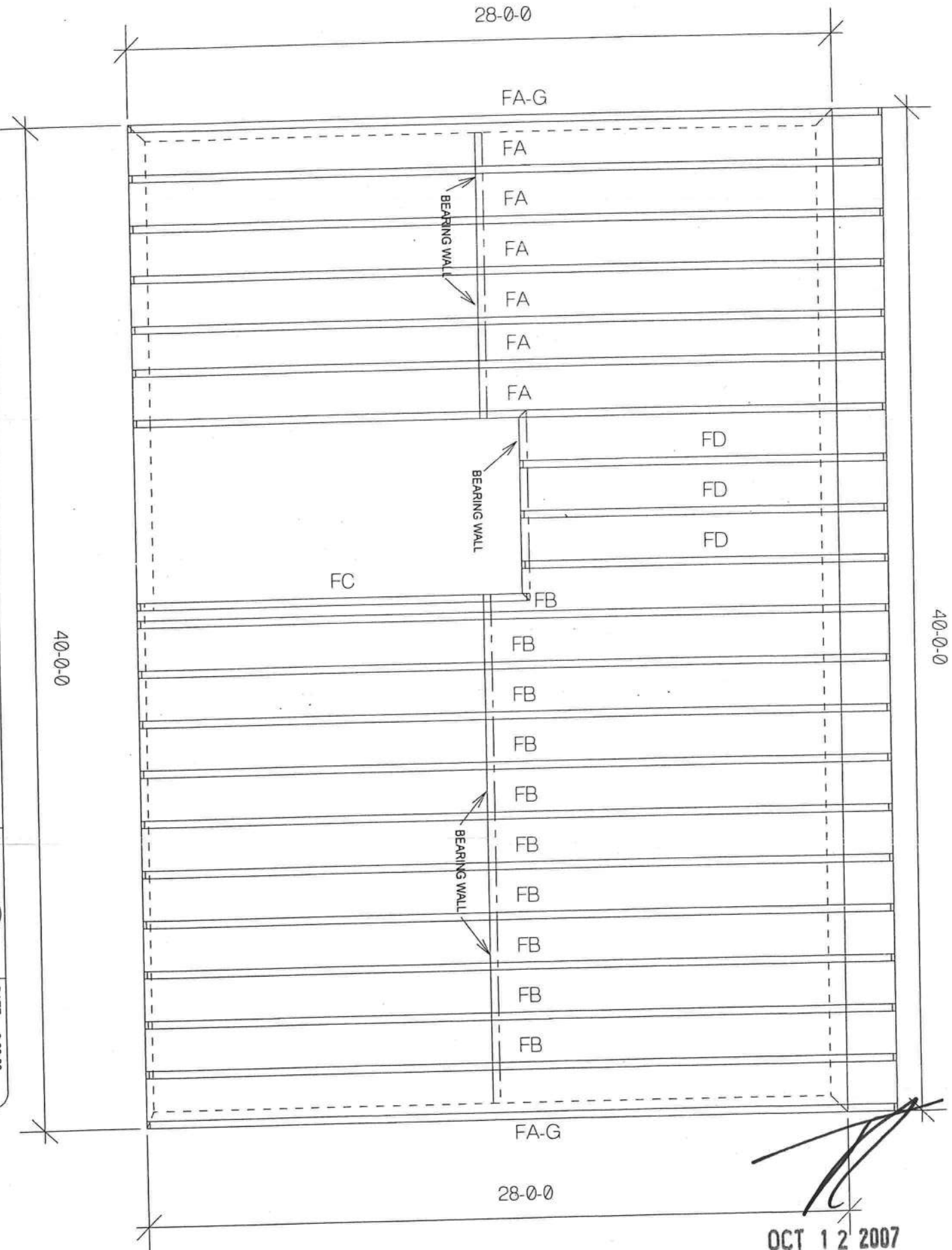
DATE: OCT 12 2007



OCT 12 2007

Maronda Systems

JOB NUMBER: SUNF	DRAWN BY: 	DATE: 2-20-06
CUSTOMER:	PITCH:	OH:
JOB NAME: SUNBURY FLOOR	BEARING: 8" & 3.5"	LOAD: 55#



OCT 12 2007

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

⚠ The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arriostre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

⚠ Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use guantes y lentes protectores cuando corte los empaques.

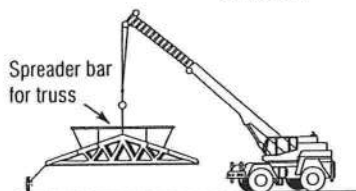
HANDLING — MANEJO

⚠ Avoid lateral bending. — Evite la flexión lateral.



⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.

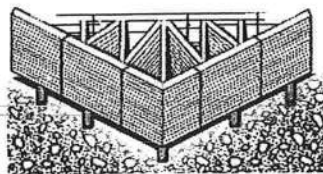


✓ Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.

⚠ The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



✓ If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

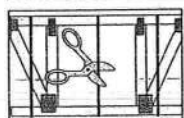
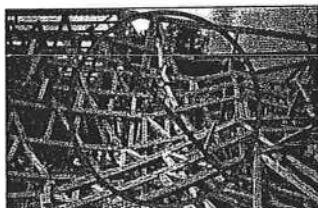
Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenamiento de los trusses en área de trabajo.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

⚠ Warning! Don't overload the crane.

¡Advertencia! ¡No sobrecarga la grúa!

⊘ Never use banding alone to lift a bundle.

Do not lift a group of individually banded bundles.

Nunca use sólo los empaques para levantar un paquete. No levante un grupo de empaques individuales.

✓ A single lift point may be used for bundles with trusses up to 45'.

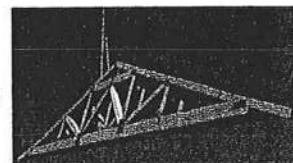
Two lift points may be used for bundles with trusses up to 60'.

Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.

Puede usar dos puntos de levantar para paquetes más de 60 pies.

Use por lo menos tres puntos de levantar para paquetes más de 60 pies.



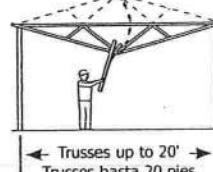
⚠ Warning! Do not over load supporting structure with truss bundle. ¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

✓ Place truss bundles in stable position. Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

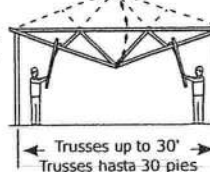
✓ Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



✓ Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUAL

✓ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

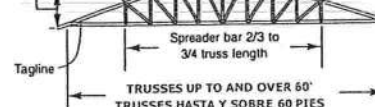
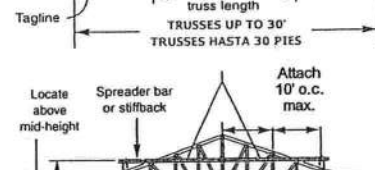
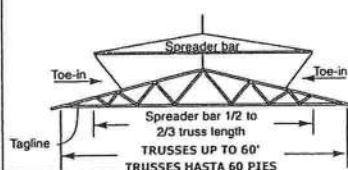
⚠ Warning! Using a single pick-point at the peak can damage the truss.

¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

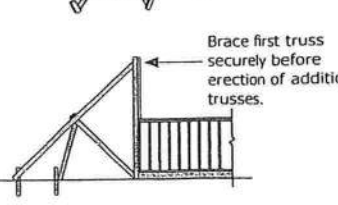
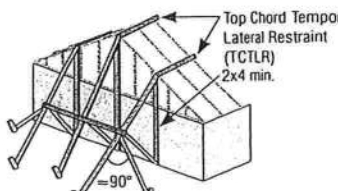
Vea el resumen BCSI B2 - Restricción/Arriostre Temporal y Instalación de los Trusses para más información.

✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).

⚠ Do not walk on unbraced trusses.

No camine en trusses sueltos.



STEPS TO SETTING TRUSSES
LAS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES
RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCTLR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un Ingeniero para trusses de mas de 60 pies.

See BCSI-B2 for TCTLR options.
Vea el BCSI-B2 para las opciones de TCTLR.

Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for Gable End Frame restraint/bracing/reinforcement information.

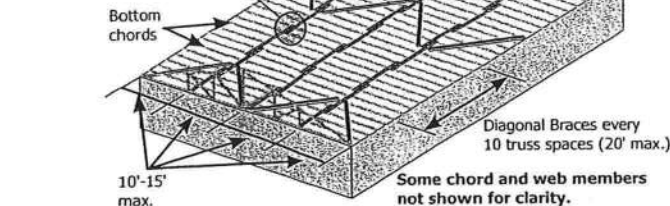
Para información sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.

WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT
LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!

3) BOTTOM CHORD — CUERDA INFERIOR

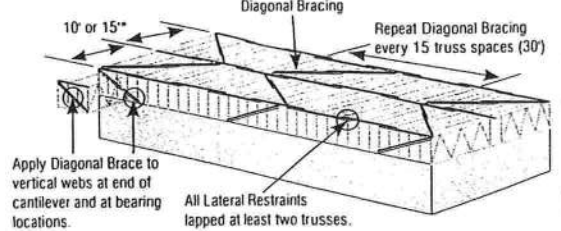
Lateral Restraints - 2x4x12' or greater lapped over two trusses.



RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

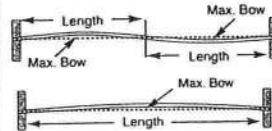
Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.

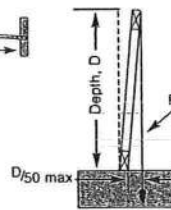


INSTALLING — INSTALACION

Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.



D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥8'

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.



Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.

Never stack materials near a peak.
Nunca amontone material cerca del pico.

Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.

Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.
Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.
No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.

Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WTCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbcindustry.com



TRUSS PLATE INSTITUTE
218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinst.org

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

From: Maronda Systems

Tomas Ponce

Professional Engineer

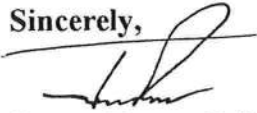
State of Florida #0050068

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

Date: 11/1/06

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design limitations to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed in accordance with BS EN 1995-1-1 and BS EN 1995-2-1 in conjunction with BS EN 1090-1 and BS EN 1090-2. It is recommended that all parts of the building design are fabricated in accordance with BS EN 1090-1 and BS EN 1090-2 by a professional engineer (registered architect or structural engineer).

When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special requirements apply. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced against buckling.

The bottom chords in tension are not fully braced laterally by a proprietary applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be installed in full depth and welded to the web of the member connecting ASTM A 653, Grade 40, unless otherwise noted.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over boltlines, bolts or distorted grain. Members shall be cut following the grain to avoid splitting. Members shall be fully embedded and shall be symmetrical about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4' long. A 6x8 plate is 6" wide x 8" long. Truss (holes) shall be parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with tie remnant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-

TOP CHORDS: 2X4 SP #2
BOT CHORDS: 2X4 SP #2
WEBS: 2X4 SP #3

T/C MUST BE CONTINUOUSLY BRACED
BY ROOF SHEATHING UNLESS NOTED
OTHERWISE.

VALLEY MEMBERS TO BE SET
PERPENDICULAR TO TRUSSES BELOW.

IT IS NOT REQUIRED TO SHEATH TRUSSES
BELOW VALLEY SET. VALLEY MEMBERS
PROVIDE ALL NECESSARY TOP CHORD
BRACING.

WHEN VALLEY MEMBERS ARE NAILED DIRECT
TO TRUSS TOP CHORDS USE (2) 16d NAILS
INTERSECTION, OR TWO FEET ON CENTER
WHEN BELOW TRUSSES ARE SHEATHED FLOOR
USE (2) 10d NAILS OR (1) 16d NAIL PER
INTERSECTION, OR TWO FEET ON CENTER

VARIES
UP TO 7/12

DIAGONAL WEBS REQUIRED WITH
CREATED WHEN 2S 0

2x4 TRANSVERSE BRACING NAILED FLAT TO EDGE
OF WEB WITH 12d NAILS AT 8" O.C. OR A SCAB OF
THE SAME DIMENSION AND GRADE AS WEB; NAILED
TO FACE (S) OF WEB WITH 10d NAILS STAGGERED
8" O.C. SCAB OR TRANSVERSE BRACKING TO EXTEND
FOR 90% OF WEB LENGTH. 2x6 BRACE REQUIRED
ON ANY WEB EXCEEDING 14'.

... denotes ONE edge or one face
*** denotes BOTH edges or Both faces.

(1) ONE BRACE REQUIRED ON WEBS $> = 94.0'$ LONG
(2) TWO BRACES REQ'D ON WEBS $> = 126.0'$ LONG **

VALLEY TRUSSES ARE ACCEPTABLE TO BE
CANTILEVERED UP TO TWO FEET.

MONO

VALLEY TRUSSES (VTF)

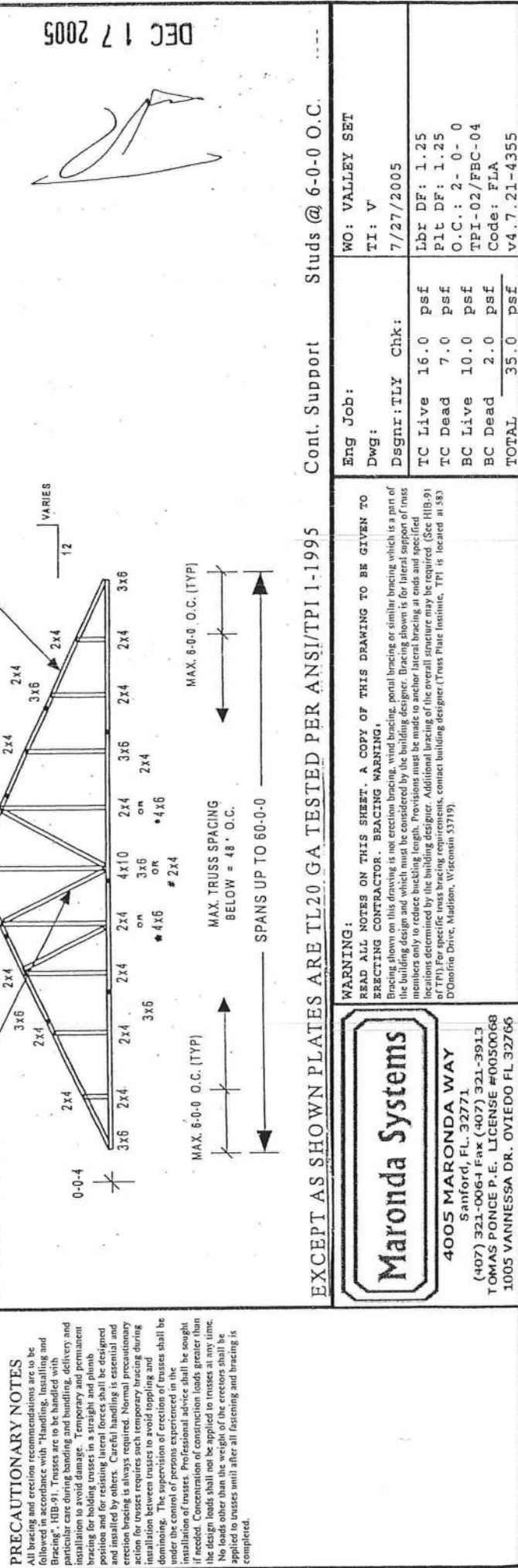
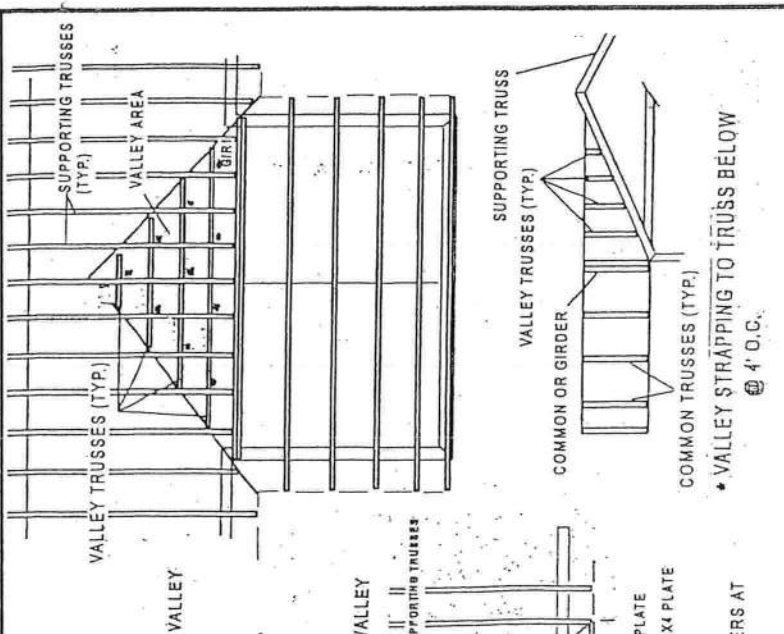
(1) WHEN DIAGONAL WEB PRESENT USE (X)
(2) WHEN NO DIAGONAL WEB PRESENT USE

IF LESS THEN 3-0-0 THEN MOVE
DIAGONAL WEB TO NEXT PANEL.

2x4 OR 4x6 2x6
OR 5x6 3x6 4x6 3x6

VALLEY MEMB

24" O.C. (TYP)



DEC 17 2005

Cont. Support		Studs @ 6-0-0 O.C.	
Eng Job:	WO: VALLEY SET		
Dwg:	TI: V		
Dsgnr: TLY	7/27/2005		
TC Live	16.0	psf	Lbr DF: 1.25
TC Dead	7.0	psf	Plt DF: 1.25
BC Live	10.0	psf	O.C.: 2 - 0 - 0
BC Dead	2.0	psf	TPI-02/FBC-04
			Code: FLA
TOTAL	35.0	psf	v4.7.21-4355

ARE TL20 GA TESTED PER ANSI/TPI 1-1995

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE CONTRACTING CONTRACTOR. BRACING WARNING.

Bracing shown on this drawing is an erection bracing. Wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of the members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See H19.1 of TPI) For specific mass bracing requirements, contact building designer. (Trust Plate Institute, TPI is located at 5120 Onofre Drive, Madison, Wisconsin 53719).

EXCEPT AS SHOWN PLATE

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P. E. LICENSE #0030068
1005 VANNESA DR. OVIEDO FL 32766

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the applicable code and all applicable standards. The fabricator's plans and to realize a continuing responsibility for each project. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over bobbles, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x4 plate is 6" wide x 4" long. Double (sloes) are parallel to the plate length specified. Slobs (sloes) with members shall meet at the center of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSIT/PTP1 1:1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be braced with particular care during bundling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and trusses are to be braced in accordance with the erection bracing is always required. Normal preassembly action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No dead loads shall be applied to the trusses at any time. The design loads shall not be applied to trusses until after all fastening and bracing is completed.

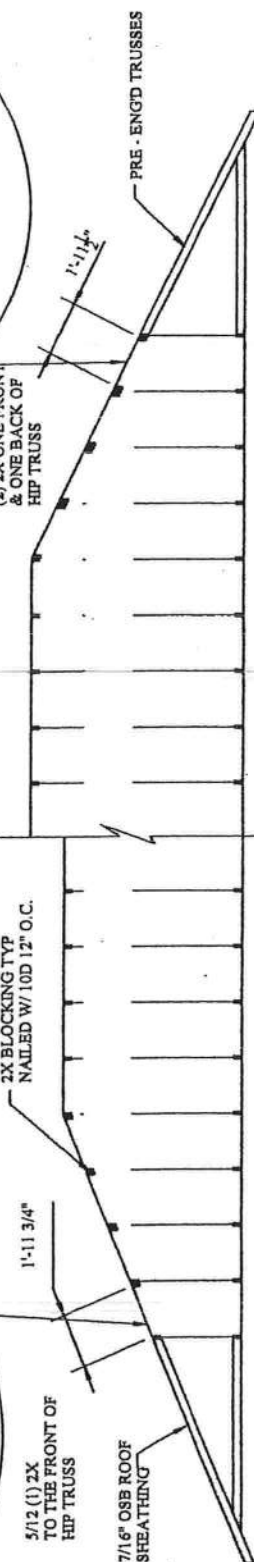
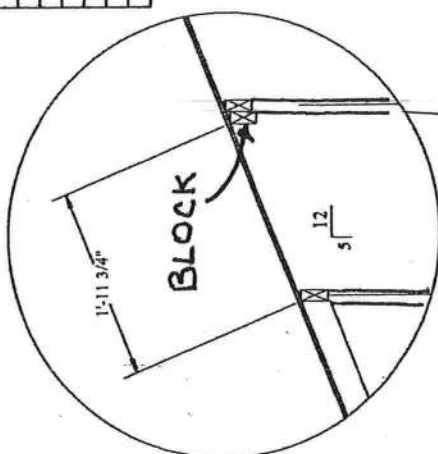
HIP TRUSS BLOCKING REQUIREMENTS

APA-FORM NO. TT-083

SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 24" O.C

Roof Slope	24/16	Panel Span Rating	48/24
≤ 1:12	OK	OK	OK
≤ 2:12	OK	OK	OK
≤ 3:12	OK	OK	OK
≤ 4:12	OK	OK	OK
≤ 5:12	OK	OK	OK
≤ 6:12	OK	OK	OK
≤ 7:12	OK	OK	OK
≤ 8:12	OK	OK	OK
≤ 9:12	OK	OK	OK
≤ 10:12	OK	OK	OK
≤ 11:12	OK	OK	OK
≤ 12:12	OK	OK	OK
≤ 13:12	OK	OK	OK
≤ 14:12	OK	OK	OK

NOTE: SHADED AREA REQUIRE 2X LUMBER BLOCKING BETWEEN TRUSS AT UN SUPPORTED PANEL EDGES IN HIP ROOF AREAS



NOTE: NO BLOCKING IS REQUIRED ON 4/12 AND BELOW PITCHED ROOFS

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE EFFECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified members only as determined by the building designer. Additional bracing of the overall structure may be required. (See HHS-91 for TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Orofino Drive, Madison, Wisconsin 53719).

Commercial Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd., Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Eng Job:

Dwg :

Dsqnr:TLY Chk:

TTC Live	16.0	psf
TTC Dead	7.0	psf
BC Live	10.0	psf
BC Dead	10.0	psf
TOTAL	43.0	psf

WO: HIPDETAIL

TI: HIP

11/2/2006

Lbr DF: 1.25
 Plt DF: 1.25
 O.C.: 2- 0- 0
 TPI-02/FBC-04
 Code: FLA
 v4.7.32-0

DESIGN INFORMATION

This design is for an individual building component and is not based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ASTM A 633-1995 and AISI S 308-97 in accordance with AISI S 308-97 and is designed for a building designer. When reviewed for approval by a building designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building orders, local climatic records for wind or snow loads, project specifications or special wind or snow loads, project specifications or special uplift loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a property applied right away, they should be braced at a maximum of 10 ft. The truss is designed to be manufactured from 20 gauge low dipped galvanized steel, meeting ASTM A 653, Grade 30, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to confirm a continuing responsibility for each callout. Any discrepancies are to be noted before cutting or fabrication. Plates shall be installed over boltnuts, known or suspected, and not be installed over bolthead, known or suspected. Connector plates shall be cut for tight fitting wood to wood joining. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Double (2x) sym. parallel to the plate length specified. Double ends of web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with applicable industry specifications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing for trusses requires such temporary bracing during installation between trusses to avoid toppling and dismounting. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than those specified shall be avoided.

No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

EXCEPT AS SHOWN PLATES ARE MITek MT20

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR~1\H2A.prx

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ALL SUBMITTING CONTRACTOR. BRACING WARNING:
The bracing shown on this drawing is not critical bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified intermediate points as determined by the building designer. Additional bracing of the entire structure may be required. (See AISC 360-10, Table 11.2-2, F. 1.2.2.1) For more information, contact Harsco Products Institute, TPI is located at 583 O'Donnell Drive, Madison, Wisconsin 53719. Component Engineering by: Truss Institute, Inc. P. A., 818 Soundside Rd. Edison, NC 27932

Eng Job:

2

Dwg: Dsgnr: TLY Chk:

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
---	---	---	---	---	---	---	---	---	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	-----

TC Live 16.0 psi

TC Dead 7.0 psf

ABC Live 10.0 psf

BC Dead 10.0 psf

TOTAL	43.0	psf
-------	------	-----

W0: SUNTV3V

ST. J. C.

DATE: 5/7/2006

195

TOTAL DE: 1.23
DIT+ DE: 1 25

CIT DE: 1:23
C C . 2-0-0

TPI-02/FBC-04

Code: FLA

4. 7. 32-47152

TC:	2x 4	SP #2	4-11, 4-10
BC:	2x 4	SP #2	
WB:	2x 4	SP #3	
	2x 4	SP #2	

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

In-Plant Quality Assurance with Cq= 1

In-Plant Quality Assurance, per

sec. 3.2.4 of TPI-102, for joint(s):

17	TC	FORCE	CSI	Cr	BC	FORCE	CSI
1	2	-2395	0.43	A 13-12	2000	0.54	
1	2	-2395	0.43	A 13-12	2000	0.54	
2	3	-1863	0.40	A 12-11	1996	0.64	
2	3	-1863	0.40	A 12-11	1996	0.64	
3	4	-1825	0.23	A 11-10	1691	0.59	
3	4	-1825	0.23	A 11-10	1691	0.59	
4	5	-1825	0.23	A 10-9	1996	0.64	
4	5	-1825	0.23	A 10-9	1996	0.64	
5	6	-1863	0.38	A 9-8	2000	0.54	
5	6	-1863	0.38	A 9-8	2000	0.54	
6	7	-2395	0.40	A			
6	7	-2395	0.40	A			

MULTIPLE LOADS -- This design is the composite result of multiple loads.

Windload per ASCE 7-02, CSC, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld type= encl I= 4.0, ft W= 30.0 ft Truss in INT zone, TCDL= 4.2 psf, BCCL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

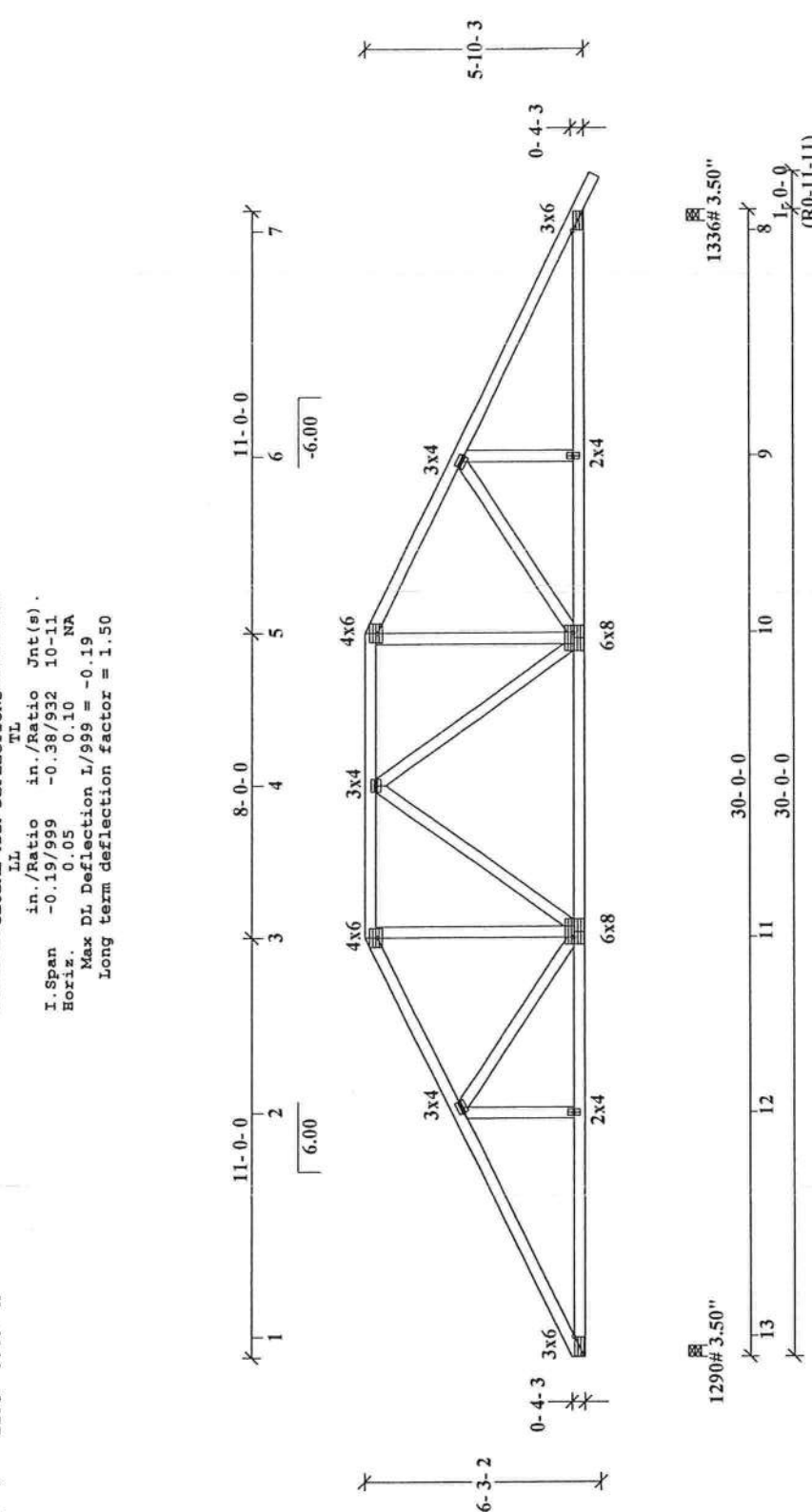
FORCES (lb) - Max Compression/Max Tension

3-TO-4 CHORD 1-2=-2295/1065,2-3=-1863/943,
3-4=-1625/902,4-5=-1625/898,5-6=-1863/938,
6-7=-2295/1046,
BUT CHORD 13-13=-864/2000,12-11=-864/1996,
11-11=-677/1691,10-9=-841/1996,9-8=-841/2000,
Webs 12-2=0/195,2-1=-402/295,
3-11=-220/630,11-4=-115/97,4-10=-115/101,
10-5=-214/630,10-6=-462/212,6-9=0/195,
-----GLOBAL MAX DEFLECTIONS-----

	LL	in./Ratio	in./Ratio	Int(s)
I Span	-0.19/999	-0.38/932	10-11	
Horiz.	0.05	0.10	NA	
Max DL Deflection L/999 = -0.19				
Long term deflection factor = 1.50				

0-0
4-12
0-0

Type	Pir
H Roll	
CSI	
5	0.0
10	0.2
32	0.2
65	0.0



Scale = 0.2070

Job: SUNBURY V 3 V

Customer:

WO: SUNV3V

TI: H3

Qty: 1

DESIGN INFORMATION

his design is for an individual building component and has been based on information provided by the client. The client is responsible for providing accurate information as a basis for any design. The design is based on the information and design furnished to the client by the client and the client is responsible for providing accurate information as a basis for any design. The design is based on the information and design furnished to the client by the client and the client is responsible for providing accurate information as a basis for any design.

FABRICATION NOTES

For fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood joining. Connector plates shall be located on both faces of the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x3 plate is 6" wide x 3" long. Slots (holes) in parallel to the plate length specified. Double cut on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for other loading and design conditions. This truss is to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and true position and for reducing stresses during erection shall be provided by the erector. Careful handling is essential and tie bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, C6C, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.10

Bld Type= encl I= 32.0 ft W= 5.0 ft Truss

in INT zone, TCDF= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 -52 0.66 A 1- 3 0 0.53

-----GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio in./Ratio Jnt(s)

I. Span 0.14/xxx 0.05/xxx 3-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/xxx = -0.09

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, C6C, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.10

Bld Type= encl I= 32.0 ft W= 5.0 ft Truss

in INT zone, TCDF= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 -52 0.66 A 1- 3 0 0.53

-----GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio in./Ratio Jnt(s)

I. Span 0.14/xxx 0.05/xxx 3-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/xxx = -0.09

Long term deflection factor = 1.50

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-52/9,

BOT CHORD 1-3=0/0,

Joint Locations

1) 0- 6- 8 3) 5- 0- 0

2) 5- 0- 0 4) 0- 6- 8

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 214 198 -305 B Pin

4-11- 4 1 112 152 -281 T Pin

4-11- 4 1 103 0 -46 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

5-0-0

2

6.00

C/L: 4-11-4

112# 1.50"

2-10-3

0-4-3

3x4

2-10-3

Attach with (3)

16d Toe-Nails

5-0-0

3

214# 3.50"

103# 1.50"

Attach with (2)

16d Toe-Nails

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Over 3 supports Scale = 0.4660

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See Hilti 91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI) is located at 583 D'Oroff Drive, Madison, Wisconsin 53719.

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR-1\H3.prx

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNV3V

TI: H3

6/7/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-47167

Job: SUNBURY V 3 V

Customer: WO: SUNV3V

TI: H3A

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or warranty of any kind for the design of the building, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, proper specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are steel members. Where needed to achieve the design strength, the top chord should be properly applied rigid ceiling, they should be braced as a maximum spacing of 10' 0" c/c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Knots and knots shall be located on the bottom chord. Connector plates shall be located on both faces of the joint unless otherwise shown. A 5/8" plate is 5" wide x 1' long. A 6/8" plate is 6" wide x 8" long. Slots (holes) in web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADS -- This design is the composite result of multiple loads.

Windload per ASCE 7-02, C6C, V=125mph, H=21.0 ft, I=1.00, Exp.Cat. B, Kzt=1.0
Bld Type= encl L= 40.0 ft W= 30.0 ft Truss in INT zone, TCDF= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-2288/1130, 2-3=-2065/1000, 3-4=-2128/1092, 4-5=-2128/1092, 5-6=-2065/989, 6-7=-2288/1105,
BOT CHORD 12-11=-942/2016, 11-10=-713/1799, C10-9=-704/1799, 9-8=-912/2016,
CWebs 2-11=-243/262, 11-3=-105/439, C3-10=-179/409, 10-4=-291/235, 10-5=-186/409, 5-9=-91/439, 9-6=-243/238,
5-9=-91/439, 9-6=-243/238.

COMPRESSION CHORDS are assumed to be continuously braced unless noted otherwise.

---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0-1-12 1 1290 14 -527 B Pin
29-10-4 1 1336 0 -701 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
WEB, . . . FORCE, . . . CSI, Cr . . . FORCE, . . . CSI, Cr
2-11 262 0.12 C 10-5 409 0.17
11-3 439 0.14 C 5-9 439 0.14
3-10 409 0.16 C 9-6 -243 0.12
10-4 -291 0.12 C
-----GLOBAL MAX DEFLECTIONS-----
LL TL
in./Ratio in./Ratio Jnt(s)
I.Span -0.18/999 -0.36/993 8-9
Horiz. 0.05 0.10 NA
Max DL Deflection L/999 = -0.18
Long term deflection factor = 1.50

Joint Locations

1)	0-0-0	5)	21-0-0	9)	21-0-0
2)	4-9-4	6)	25-2-12	10)	15-0-0
3)	9-0-0	7)	30-0-0	11)	9-0-0
4)	15-0-0	8)	30-0-0	12)	0-0-0

In-Plant Quality Assurance with Cq= 1
In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s):
1,7 . . . TC, . . . FORCE, . . . CSI, Cr . . . BC, . . . FORCE, . . . CSI,
1-2 -2288 0.33 A 12-11 2016 0.73
2-3 -2065 0.31 A 11-10 1799 0.70
3-4 -2128 0.37 A 10-9 1799 0.70
4-5 -2128 0.37 A 9-8 2016 0.73
5-6 -2065 0.30 A
6-7 -2288 0.31 A

Diagram

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See TPI-01 of TPI) For specific truss bracing requirements, contact Building Designer (Truss Plate Institute, TPI is located at 581 D'Ondra Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR-1\H3A.prx

Eng Job:

WO: SUNV3V
TI: H3A
Dsgnr: TLY Chk: 6/7/2006

TC Live 16.0 psf

TC Dead 7.0 psf
BC Live 10.0 psf
BC Dead 10.0 psf
TOTAL 43.0 psf

Scale = 0.2118

EXCEPT AS SHOWN PLATES ARE Mitek MT20

Job: SUNBURY V 3 V

Customer:

WO: SUNV3V

TI: HSI

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and is not to be used for any other purpose. The designer declines any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 3" wide x 4' long. A 6x6 plate is 6" wide x 8' long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may vary. Trusses are to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and parallel position for unloading should be designed and installed by the erector. Careful handling is essential and bracing is always required. No erection or installation action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

MULTIPLE LOADS -- This design is the composite result of multiple loads. Windload per ASCE 7-02, C6C, V= 125mph, E= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= encl I= 40.0 ft W= 30.0 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-4196/1752, 2-3=-3180/1376, 3-4=-2813/1311, 4-5=-3180/1373, 5-6=-4196/1753, BOT CHORD 12-11=-1506/3828, 11-10=-1500/3819, 10-9=-968/2814, 9-8=-1503/3820, 8-7=-1507/3828, Webs 11-2=0/217, 2-10=-883/476, 3-10=-376/1165, 3-9=-2/4, 9-4=-375/1165, 9-5=-884/481, 5-8=0/217, 3-9

Joint Locations

1) 0-0-0 5) 23-1-1 9) 16-10-8
2) 6-10-15 6) 30-0-0 10) 13-1-8
3) 13-0-0 7) 30-0-0 11) 6-10-15
4) 17-0-0 8) 23-1-1 12) 0-0-0
---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0-1-12 1 1342 0 -671 B H Roll
29-10-4 1 1342 0 -671 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
WEB...FORCE...CSI...CF...WEB...FORCE...CSI...CF...
11-2 217 0.07 C 9-4 1165 0.37 C
2-10 -883 0.64 C 9-5 -884 0.64 C
3-10 1165 0.37 C 5-8 217 0.07 C
3-9 4 0.00 C

GLOBAL MAX DEFLECTIONS

LL TL
in./Ratio in./Ratio Jnt(s)
I.Span -0.42/833 -0.83/423 5-4
Horiz. 0.30 0.58 NA
Max DL Deflection L/862 = -0.41
Long term deflection factor = 1.50

Diagram

Diagram of a truss structure with dimensions and member sizes. Key dimensions include: 7-3-15, 13-0-0, 4-0-0, 13-1-8, 3-9-0, 30-0-0, 0-4-3, 6-10-3, 0-3-8, 1342# 3.50", 1-0-0, (R1-1-7), 3-0-0, 3-2-8, 3x4, 2x4, 5x6, 6x12, 3x8. Member sizes are indicated on the diagram.

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.2038

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI) is located at 583 O'Donoghue Drive, Madison, Wisconsin 53719. Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenport, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR-1\ESI.prx

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNV3V

TI: HSI

6/7/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.32-47171

DESIGN INFORMATION

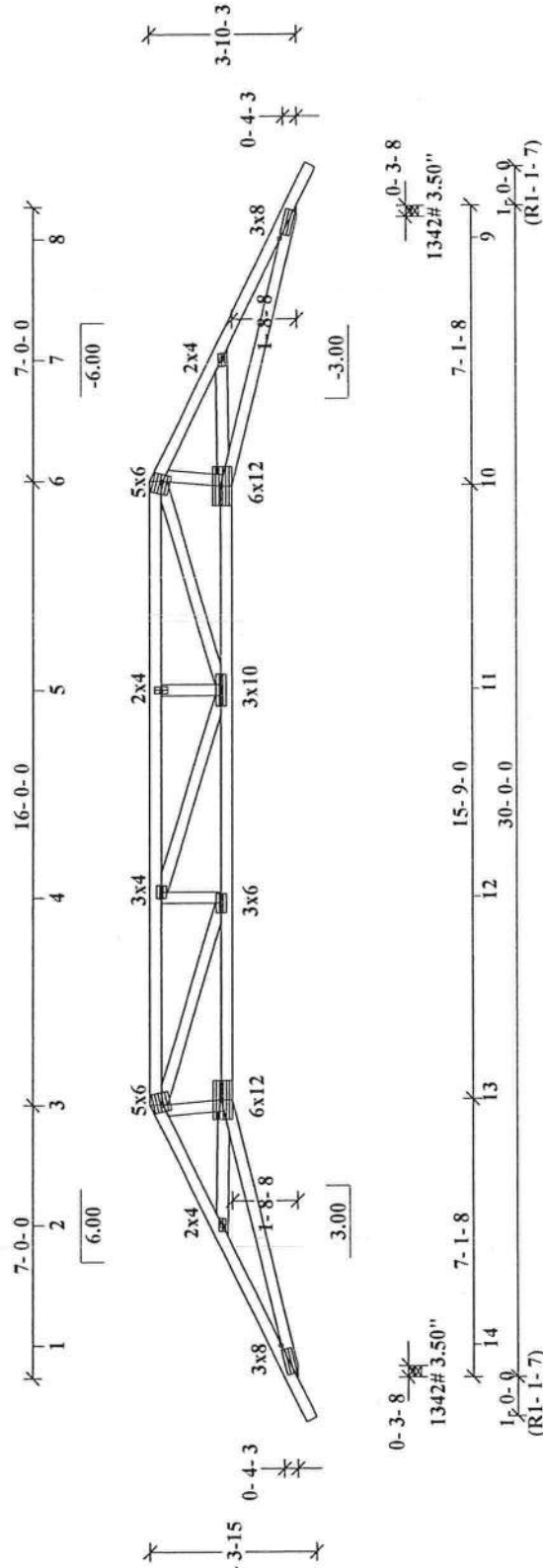
This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or specific nor control with regard to construction, handling, shipment and installation of trusses. This disclaimer is in accordance with ANSI/PCT-1:1995 and NDS-97 to be incorporated as part of the building design by the building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown is in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where a truss is used for a ceiling, it must be designed for a uniformly applied ceiling loading; they should be braced at a maximum spacing of 10'-0". Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to utilize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plans shall not be installed over knuckles, toes or distorted areas. Members shall be cut at right fitting ends to avoid tearing. Connections shall be made at least 6 inches from the joint unless otherwise shown. A 5/8" plate, 3" wide x 4" long, a 6/8" plate is 6" wide, 8" long. Double (holes) are parallel to the plate length specified. Scribe (holes) web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This member is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSII/TPI-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses in void topping and dismantling. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.



EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.2038

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUN3V\SUNBUR~1\HS4.prx

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design. Erection bracing is for lateral support of truss like members and which must be considered by the building designer. Bracing shown is for lateral support of truss like members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91-107 for specific rafter bracing requirements, connect building designer (Truss Plus Institute, TPI) is located at 583 O'Neiro Drive, Madison, Wisconsin 53719.)

Component Engineering Inc., P.A., 818 Soundside Rd, Edenboro, NC 27932

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd., Edenton, NC 27932

Eng Job:

Dsgnr:	TLY	Chk:
TC Live	16.0	psf
TC Dead	7.0	psf
BC Live	10.0	psf
BC Dead	10.0	psf
TOTAL	43.0	psf

WO: SUNTV3V

TI: HS4
6/7/2006
Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2- 0- 0
TPI-02/FBC-04
Code: FLA
v4.7.32-47182

v4.7.32-47182

wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed.

for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously

braced by sheathing unless otherwise specified. Where bottom chords in truss are not fully braced laterally by a

properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be

1
2
3
4
5
6
7
8
9
10
11
12
13
14
15
16
17
18
19
20
21
22
23
24
25
26
27
28
29
30
31
32
33
34
35
36
37
38
39
40
41
42
43
44
45
46
47
48
49
50
51
52
53
54
55
56
57
58
59
60
61
62
63
64
65
66
67
68
69
70
71
72
73
74
75
76
77
78
79
80
81
82
83
84
85
86
87
88
89
90
91
92
93
94
95
96
97
98
99
100
101
102
103
104
105
106
107
108
109
110
111
112
113
114
115
116
117
118
119
120
121
122
123
124
125
126
127
128
129
130
131
132
133
134
135
136
137
138
139
140
141
142
143
144
145
146
147
148
149
150
151
152
153
154
155
156
157
158
159
160
161
162
163
164
165
166
167
168
169
170
171
172
173
174
175
176
177
178
179
180
181
182
183
184
185
186
187
188
189
190
191
192
193
194
195
196
197
198
199
200
201
202
203
204
205
206
207
208
209
210
211
212
213
214
215
216
217
218
219
220
221
222
223
224
225
226
227
228
229
230
231
232
233
234
235
236
237
238
239
240
241
242
243
244
245
246
247
248
249
250
251
252
253
254
255
256
257
258
259
260
261
262
263
264
265
266
267
268
269
270
271
272
273
274
275
276
277
278
279
280
281
282
283
284
285
286
287
288
289
290
291
292
293
294
295
296
297
298
299
300
301
302
303
304
305
306
307
308
309
310
311
312
313
314
315
316
317
318
319
320
321
322
323
324
325
326
327
328
329
330
331
332
333
334
335
336
337
338
339
340
341
342
343
344
345
346
347
348
349
350
351
352
353
354
355
356
357
358
359
360
361
362
363
364
365
366
367
368
369
370
371
372
373
374
375
376
377
378
379
380
381
382
383
384
385
386
387
388
389
390
391
392
393
394
395
396
397
398
399
400
401
402
403
404
405
406
407
408
409
410
411
412
413
414
415
416
417
418
419
420
421
422
423
424
425
426
427
428
429
430
431
432
433
434
435
436
437
438
439
440
441
442
443
444
445
446
447
448
449
450
451
452
453
454
455
456
457
458
459
460
461
462
463
464
465
466
467
468
469
470
471
472
473
474
475
476
477
478
479
480
481
482
483
484
485
486
487
488
489
490
491
492
493
494
495
496
497
498
499
500
501
502
503
504
505
506
507
508
509
510
511
512
513
514
515
516
517
518
519
520
521
522
523
524
525
526
527
528
529
530
531
532
533
534
535
536
537
538
539
540
541
542
543
544
545
546
547
548
549
550
551
552
553
554
555
556
557
558
559
560
561
562
563
564
565
566
567
568
569
570
571
572
573
574
575
576
577
578
579
580
581
582
583
584
585
586
587
588
589
590
591
592
593
594
595
596
597
598
599
600
601
602
603
604
605
606
607
608
609
610
611
612
613
614
615
616
617
618
619
620
621
622
623
624
625
626
627
628
629
630
631
632
633
634
635
636
637
638
639
640
641
642
643
644
645
646
647
648
649
650
651
652
653
654
655
656
657
658
659
660
661
662
663
664
665
666
667
668
669
670
671
672
673
674
675
676
677
678
679
680
681
682
683
684
685
686
687
688
689
690
691
692
693
694
695
696
697
698
699
700
701
702
703
704
705
706
707
708
709
710
711
712
713
714
715
716
717
718
719
720
721
722
723
724
725
726
727
728
729
730
731
732
733
734
735
736
737
738
739
740
741
742
743
744
745
746
747
748
749
750
751
752
753
754
755
756
757
758
759
760
761
762
763
764
765
766
767
768
769
770
771
772
773
774
775
776
777
778
779
780
781
782
783
784
785
786
787
788
789
790
791
792
793
794
795
796
797
798
799
800
801
802
803
804
805
806
807
808
809
810
811
812
813
814
815
816
817
818
819
820
821
822
823
824
825
826
827
828
829
830
831
832
833
834
835
836
837
838
839
840
84

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with

the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are

to be put in writing before cutting or fabrication. Plastics shall not be installed over knotholes, knots or distorted

Members shall be cut of tight fitting wood to wood grain. Connector plates shall be located on both faces of boaring.

[illegible]

run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless

2-10-3

increased for certain handling and/or erection stresses. This stress is not to be fabricated with fire retardant treated

0-4-3
3x5
ANSI/TP1
information on Quality Control refer to
lumber unless otherwise shown. For additional

Attach with (2)

16d Toe-Nails

PRECAUTIONARY NOTES

All fracture and erection recommendations are to be followed.

followed in accordance with accepted industry publications. Trusses are to be handled with particular

care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent

bracing for holding masses in a straight and plumb position and for resisting lateral forces shall be designed

and installed by others. Careful handling is essential and crection bracing is always required. Normal precautionary

action for trusses requires such temporary bracing during installation between trusses to avoid slopping, and

dominating. The supervision of erection of masts shall be under the control of persons experienced in the

installation or misses. Professional advice shall be sought if needed. Concentration of construction loads greater than 266# 3.50" 103# 1.50"

[illegible]

complicated

[illegible]
$$\overbrace{\quad\quad\quad}^{(R1-1-\gamma)}$$

EXCEPT AS SHOWN PLATES ARE MITEK MT20
Over 3 Supports Scale = 0.5099

[illegible]

REMARKS:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO

JOB NO: SUNV3V
WO: 71
DT: 71

Maranda Systems
EFFECTING CONTRACTOR. BRACING WARNING:
 Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
 DWG: 9/19/2007
 Descri: 9/19/2007
 CMT:

the building design and the building designer. Bracing shows is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified members only to reduce buckling length.	design: 1st wk.	3/12/2007
the building design and the building designer. Bracing shows is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified members only to reduce buckling length.	design: 1st wk.	3/12/2007

4005 MARONDA WAY

IC	LIVE	10.0	PST
TD	Dead	7.0	psf
PLT	DF:	1.25	
AOL	DZ:	1.25	

4005 MARIONDA WAY
Sanford, FL 32771
Component Engineering by: Trust Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932
D'Onofrio Drive, Madison, Wisconsin 53719
BC Live 10.0 psf
O.C. : 2 - 0 - 0
S.C. : 0 - 0 - 0

(407) 321-0064 Fax (407) 321-3913
 TOMAS PONCE P.E. LICENSE #0050068
 BC Dead 10.0 psf
 Code: FIA
 TPI-UZ/FBC-U4

[illegible]

Design: Matrix Analysis
Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\J1.PRX

Job: SUNBURY V 3 V

Customer: WO: SUNV3V

TI: JLA

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

[] or [] : Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 214 198 -305 B Pin

4-11- 4 1 112 152 -281 T Pin

4-11- 4 1 103 0 -46 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02, C_sC_e V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, K_{zt}= 1.0 Bld Type= encl L= 32.0 ft W= 5.0 ft Truss in INT zone, FCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req

1) 0- 6- 8 3) 5- 0- 0

2) 5- 0- 0 4) 0- 6- 8

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.14/xxx 0.05/xxx 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of that information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSITPP 1-1995 and NDS-97 and shall be a part of the building design by a Building Designer. When the design is approved by the building designer, the design loadings shall be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knots, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSITPP 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding (trusses) in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

2x 4 SP #2

2x 4 SP #2

5-0-0

6.00

2-10-3

0-4-3

214# 3.50"

103# 1.50"

5-0-0

3

4

2-10-3

Attach with (3) 16d Toe-Nails

Attach with (2) 16d Toe-Nails

C/L: 4-11-4 112# 1.50"

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.5520

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIL-9) of (TPI) Engineering, Madison, Wisconsin 53719.

D'Oroffo Dring, Madison, Wisconsin 53719.

Component Engineering by: Trust Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00500068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\JLA.prx

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNV3V

TI: JLA

9/19/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-9937

Job: SUNBURY V 3 V

Customer:

WO: SUNV3V

TI: J2

Qty: 3

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or liability for control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed in accordance with the provisions of the American Institute of Steel Construction, Inc. (AISC) 1989 Specification for Structural Steel Buildings and the American National Standards Institute (ANSI) 1995 Standard for the Design and Construction of Steel Trusses and Joists. The truss shall be designed as part of the building design by a Building Designer. When required for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing and installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Precautionary advice shall be sought if needed. Concentration of construction loads greater than 15 psf (dead plus live) shall not be applied to trusses in any one place. No load shall be applied to the height of erection until the applied in trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

[] or []: Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 181 119 -279 B Pin

2-11- 4 1 67 92 -169 T Pin

2-11- 4 1 62 0 -27 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02, C6C, V= 125mph, B= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= encl I= 32.0 ft W= 3.0 ft Truss in END zone, TCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req

1

2

3-0-0

6.00

CL: 2-11-4

67# 1.50"

(3x5)

2-3-15

0-4-3

1-10-3

Attach with (2)

16d Toe-Nails

181# 3.50"

62# 1.50"

3-0-0

3

1-0-0

(R1-1-7)

3-0-0

Joint Locations

1) 0- 6- 8 3) 3- 0- 0

2) 3- 0- 0 4) 0- 6- 8

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.02/xxx 0.01/xxx 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

EXCEPT AS SHOWN PLATES ARE Mitek MT20

Over 3 Supports Scale = 0.6060

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluoth, FL 32766

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-9) of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Onefrino Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Eng Job:

Dwg:

Dsgnr: TLY

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNV3V

TI: J2

9/19/2007

Ibr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-9938

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\J2.PRX

Job: SUNBURY V 3 V

Customer:

WO: SUNV3V

TI: J2A

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

[] or []: Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

---MAX. REACTIONS PER BEARING LOCATION---

0- 1-12 1 129 119 -183 B Pin

2-11- 4 1 67 92 -169 T Pin

2-11- 4 1 62 0 -27 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trust designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or expertise to control with regard to the design, shipment and handling of the building component in accordance with ANSI/TPI 1-1995 and NDS-07 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, trust has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slot (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This trust is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling. Delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses when time. No member shall be subjected to erection stresses shall be applied to trusses until after all fasciing and bracing is completed.

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02, C6C, V= 125mph, H= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= encl I= 32.0 ft W= 3.0 ft Truss in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req

Joint Locations

1) 0- 6- 8 3) 3- 0- 0

2) 3- 0- 0 4) 0- 6- 8

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I. Span 0.02/xxx 0.01/xxx 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Attach with (2)

16d Toe-Nails

1-10- 3

Attach with (2)

16d Toe-Nails

0- 4- 3

1-10- 3

67# 1.50"

62# 1.50"

129# 3.50"

3- 0- 0

3

4

3- 0- 0

6.00

3- 0- 0

2

1

3x5

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.6655

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at each end of the truss. Locations determined by the building designer. Additional bracing of the overall structure shall be required (See HUB-01 of TPI) For specific truss bracing requirements, contact building designer (Trust Plate Institute, TPI is located at 583 D'Oonofrio Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\J2A.PRX

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNV3V

TI: J2A

9/19/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-9939

Job: SUNBURY V 3 V		Customer:		WO: SUNV3V		TI: J3		Qty: 3																									
TC: 2x 4 SP #2 BC: 2x 4 SP #2 [] or []: Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation. User defined loadcases have been considered in this design. ---MAX. REACTIONS PER BEARING LOCATION--- <table><tr><th>X-Loc</th><th>Bset</th><th>Vert</th><th>Horiz</th><th>Uplift</th><th>Y Type</th></tr><tr><td>0-1-12</td><td>1</td><td>95</td><td>11</td><td>-70</td><td>B Pin</td></tr><tr><td>0-11-4</td><td>1</td><td>23</td><td>9</td><td>-13</td><td>T Pin</td></tr><tr><td>0-11-4</td><td>1</td><td>21</td><td>0</td><td>0</td><td>B H Roll</td></tr></table> PROVIDE UPLIFT CONNECTION PER SCHEDULE										X-Loc	Bset	Vert	Horiz	Uplift	Y Type	0-1-12	1	95	11	-70	B Pin	0-11-4	1	23	9	-13	T Pin	0-11-4	1	21	0	0	B H Roll
X-Loc	Bset	Vert	Horiz	Uplift	Y Type																												
0-1-12	1	95	11	-70	B Pin																												
0-11-4	1	23	9	-13	T Pin																												
0-11-4	1	21	0	0	B H Roll																												
Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. W/Load per ASCE 7-02, MWFRS, V= 125mph, B= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0, Bld Type= encl L= 32.0 ft W= 1.0 ft Truss in END zone, TCDF= 4.2 psf, BCDF= 6.0 psf Impact Resistant Covering and/or Glazing Req 1) 0-6-8 2) 1-0-0 3) 1-0-0 4) 0-6-8 This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing. In-Plant Quality Assurance with Cq= 1 -----GLOBAL MAX DEFLECTIONS----- <table><tr><th>LL</th><th>TL</th><th>in./Ratio</th><th>in./Ratio</th><th>Jnt(s)</th></tr><tr><td>I Span</td><td>0.00/999</td><td>0.00</td><td>0.00/999</td><td>2-1</td></tr><tr><td>Horiz.</td><td>0.00</td><td>0.00</td><td>0.00</td><td>NA</td></tr></table> Long term deflection factor = 1.50										LL	TL	in./Ratio	in./Ratio	Jnt(s)	I Span	0.00/999	0.00	0.00/999	2-1	Horiz.	0.00	0.00	0.00	NA									
LL	TL	in./Ratio	in./Ratio	Jnt(s)																													
I Span	0.00/999	0.00	0.00/999	2-1																													
Horiz.	0.00	0.00	0.00	NA																													
Attach with (2) 16d Toe-Nails 0-10-3 0-4-3 1-3-15 0-4-3 1-0-0 1 2 6.00 C/L: 0-11-4 23# 1.50" (3x5) 95# 3.50" 21# 1.50" 1-0-0 4 3 1-0-0 (RI-1-7) Attach with (2) 16d Toe-Nails																																	
Over 3 Supports Scale = 0.7440																																	
EXCEPT AS SHOWN PLATES ARE MITAK MT20																																	
Maronda Systems 4005 MARONDA WAY Sanford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Medallion Pl. Chuluota, FL 32766																																	
DESIGN INFORMATION This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of failure or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design drawings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records, for wind or snow loads, proper specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-4" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown. FABRICATION NOTES Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, joints or distorted grain. Webbers shall be cut and fitted to fit snugly in wood grain. Connector plates shall be located on both sides of the joint with nail fully imbedded and shall be sym about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) min parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate areas are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995. PRECAUTIONARY NOTES All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.																																	
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Seaside Rd. Eden, NC 27932																																	
Eng Job: Dwg: Dsgnr: TLY Chk: TC Live 16.0 psf TC Dead 7.0 psf BC Live 10.0 psf BC Dead 10.0 psf TOTAL 43.0 psf WO: SUNV3V TI: J3 9/19/2007 Lbr DF: 1.25 Plt DF: 1.25 O.C.: 2-0-0 TPI-02/FBC-04 Code: FTA v4.7.3505-9940																																	

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the trade designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies to be put in writing before cutting of connector plates shall not be installed over twisted, bent or distorted bars. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with male flange imbedded and shall be sym about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Double cut on nail parallel to the plate length specified. Double cut on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1995.

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a temporary and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and trusses are to be braced during erection. Normal precautionary action for trusses requires such temporary bracing during erection as is required to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. No design loads shall be applied to trusses until they are applied to trusses until after full fastening and bracing is completed.

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluotla, FL 32766

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\J3A.prx

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO THE ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and required locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 for specifications for bracing of trusses.)

For special conditions, contact building designer. Truss Plastic fastening, TPI is located at 583 D'Oroville Industrial Drive, Waco, TX 76710.

Component Erecting by: Waco Erecting, Inc. P. A. 818 Sandeagle Drive, Edenburg, NC 27932

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Eng Job:
Dwg:
Dsgnr: TLY Chk:

TC Live	16.0	psf	Lbr DF: 1.25
TC Dead	7.0	psf	Plt DF: 1.25
BC Live	10.0	psf	O.C.: 2- 0- 0
BC Dead	10.0	psf	TPI-02/FBC-04
TOTAL	43.0	psf	Code: FLA
			v4 7 305-994

Over 3 Supports
Scale = 0 8794

Customer:

WO: SUNV3V

TI: J3A

Oty:1

TC: 2x 4 SP #2
BC: 2x 4 SP #2
[] or {}: Plate(s)
The Joint Detail p
with any submittal
fabrication docum
User defined load
in this design

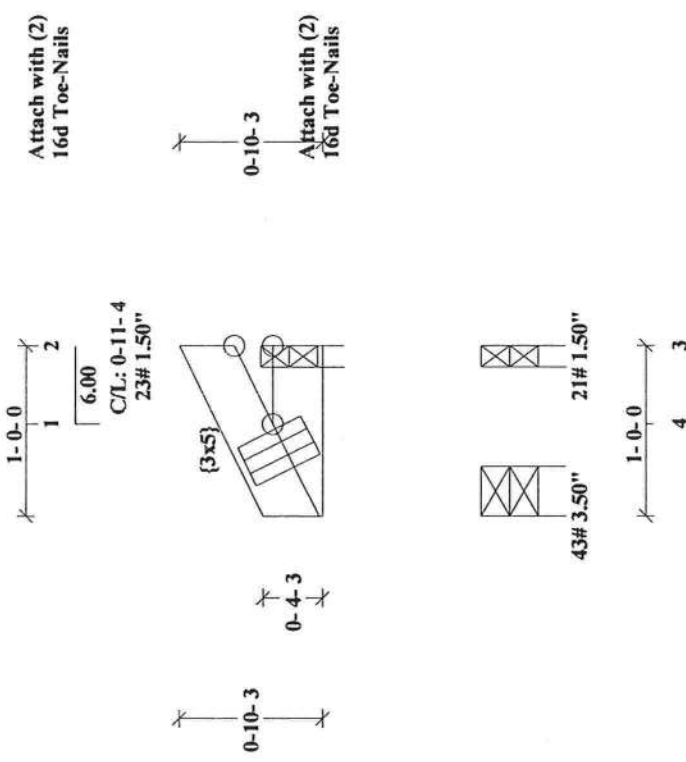
-----MAX. REACTIONS PER BEARING LOCATION-----				
X-Loc	Bset	Vert	Horiz	Uplift Y Type
0-1-12	1	43	40	-61 B Pin
0-11-4	1	23	32	-58 T Pin
0-11-4	1	21	0	-7 B Roll

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WndLoad per ASCE 7-02, CSc, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

BldType= encl L= 32.0 ft W= 1.0 ft Truss in END zone, TCdL= 4.2 psf, BCdL= 6.0 psf Impact Resistant Covering and/or Glazing Req

I. Span	0.00/999	0.00/999	2-1
Horiz.	0.00	0.00	N/A
Long term deflection factor =			1.50



Joint Locations

1)	0-6-8	3)	5-0-0
2)	5-0-0	4)	0-6-8

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if

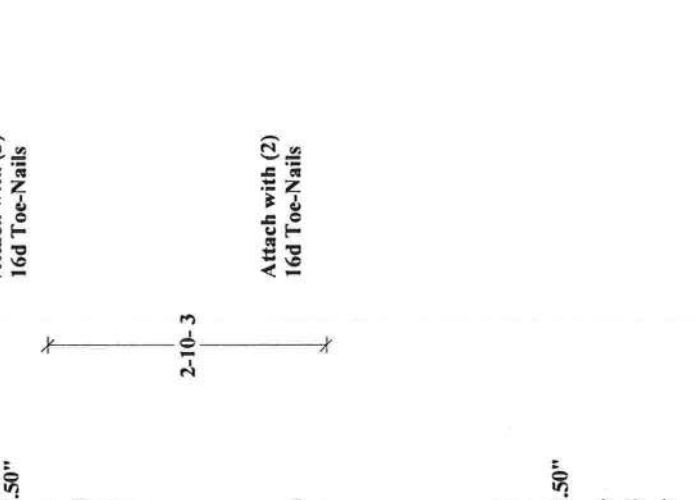
necessary to achieve full bearing.
In-Plant Quality Assurance with Cq= 1
-----GLOBAL MAX DEFLECTIONS-----

	LL	TL	
	in./Ratio	in./Ratio	Jnt(s).

-----GLOBAL MAX DEFLECTIONS-----			
	LL	TL	
	in./Ratio	in./Ratio	Jnt(s)

Long term deflection factor = 1.50

Attach with (3)



Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #00500068
367 Medallion Pl. Chuluota, FL 32766

Eng Job:	WO: SUNV3V
Dwg:	TI: J5

Dsgnr: TLY	Chk:	9/19/2007
TC Live	16.0	psf
TC Dead	7.0	psf
BC Live	10.0	psf
BC Dead	10.0	psf
TOTAL	43.0	psf

Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\J5.PRX

Job: SUNBURY V 3 VCustomer: WO: SUNV3V

TI:J6Qty: 4

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind and snow loads, and other applicable loads. The design assumes for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and practices and is consistent with the building code requirements. A fabrication license shall not be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

[] or []: Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0-4-0 1 181 119 -279 B Pin

2-11-4 1 67 92 -169 T Pin

2-11-4 1 62 0 -27 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02, Cfc, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl L= 32.0 ft W= 3.0 ft Truss in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req

1 0-6-8 3) 3-0-0

2) 3-0-0 4) 0-6-8

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.02/xxx 0.01/xxx 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

2-3-15 0-4-3

2-3-0 0

6.00

C/L: 2-11-4

67# 1.50"

1-10-3

Attach with (2)

16d Toe-Nails

181# 8.00"

62# 1.50"

3-0-0

3-0-0

1-0-0

(R1-1-7)

Over 3 Supports Scale = 0.6060

Eng Job: WO: SUNV3V

Dwg: TI: J6

Dsgnr: TLY Chk: 9/19/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.3505-9943

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-9) of TPI. For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 381 D'Ondeno Drive, Madison, Wisconsin 53717).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\J6.PRK

Job: SUNBURY V 3 V

Customer:

WO: SUNV3V

TI: J7

Qty: 4

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and designs furnished to the engineer, designer by the client and the design agency. This information as it may relate to a specific project and accept no responsibility, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, proper specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2

BC: 2x 4 SP #2

() or (): Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0-4-0 1 95 40 -158 B Pin

0-11-4 1 23 32 -58 T Pin

0-11-4 1 21 0 -7 B E Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02, C&C, V= 125mph, B= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl I= 32.0 ft W= 1.0 ft Truss in END zone, TCOL= 4.2 psf, BCOL= 6.0 psf Impact Resistant Covering and/or Glazing Req

1) 0- 6- 8 3) 1- 0- 0

2) 1- 0- 0 4) 0- 6- 8

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I.Span 0.00/999 0.00/999 2-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Attach with (2)

16d Toe-Nails

0-10-3

Attach with (2)

16d Toe-Nails

1-0-0 1 2

6.00

CL: 0-11-4

23# 1.50"

(3x5)

1-3-15 0-4-3

95# 8.00" 21# 1.50"

1-0-0 4 3

1-0-0 1-0-0

(R1-1-7)

Over 3 Supports

Scale = 0.7440

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00050068

387 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\SUNV3V\SUNV3V.J7.PRX

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to replace missing length. Provisions must be made to install lateral bracing at ends and specified locations during erection. The building designer, fabricator and erector are responsible for the design and installation of TPI. For specific riss bracing, consult building designer (Treas. Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Eng Job:

WO: SUNV3V

TI: J7

9/19/2007

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Job: SUNBURY V 3 V

Customer:

WO: SUNV3V

TI: JS2

Qty: 4

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer declines any responsibility for damages as a result of faulty or incorrect information, specifications, and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for an exercise project with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased to meet the requirements of the building designer. This truss is not to be fabricated with fire-retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and section for bracing is always required. When preliminary erection for bracing is required, temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

0-1-12 1 222 102 -233 B Fin

2-11-4 1 112 74 -131 T Fin

2-11-4 1 89 0 -20 B E Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Shim or wedge if necessary to achieve full bearing.

WindLoad per ASCE 7-02, C&C, V= 125mph, H= 21.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl I= 32.0 ft W= 3.0 ft Truss

in END zone, TCCL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1-2 -49 0.29 C 1-3 -16 0.15

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I Span -0.01/511 -0.02/999 3-1

Horiz. 0.00 0.00 NA

Max DL Deflection L/681 = -0.01

Long term deflection factor = 1.50

Joint Locations

1) 0-9-13 3) 3-0-0

2) 3-0-0 4) 0-9-13

----- TOTAL DESIGN LOADS -----

Uniform PLF From PLF To

TC Vert L+D -46 -1-1-9 -46 0-10-11

TC Vert L+D -82 0-10-11 -82 3-0-0

BC Vert L+D -40 0-0-0 -40 0-10-11

BC Vert L+D -58 0-10-11 -58 3-0-0

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-49/0,

BOT CHORD 1-3=-16/10,

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.4664

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroff Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR-1\JS2.prx

Eng Job:

Dwg:

Dsgnr: TLY Chk:

WO: SUNV3V

TI: JS2

6/7/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FEC-04

Code: FLA

v4.7.32-47180

Job: SUNBURY V 3 VCustomer: WO: SUNV3V

TI:MG4Qty:1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1993 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design engineer shown must be checked to ensure that the data and design are in agreement with the local building codes, local climate or special wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knots, holes or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. WindLoad per ASCE 7-02, C6C, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= part L= 40.0 ft W= 6.0 ft Truss in END zone, TCDL= 0.0 psf, BCDL= 0.0 psf ..TC..FORCE..CSI..Cr..RC..FORCE..CSI.. 1- 2 -115 0.25 A 1- 5 0 0.19 2- 3 59 0.26 A 5- 4 0 0.11 -----GLOBAL MAX DEFLECTIONS-----

LL	TL	in./Ratio	in./Ratio	Jnt(s)
I Span	0.00/999	0.00/999	3-2	NA
Horiz.	0.00	0.00		

Max DL Deflection L/999 = 0.00
Long term deflection factor = 1.50

Multiple Loads -- This design is the composite result of multiple loads.

Right End vertical designed for wind.
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-115/0, 2-3=-53/59,
BOT CHORD 1-5=0/0, 5-4=0/0,
Webs 5-2=-179/538, 4-3=-11/28,

Joint Locations

1)	0- 0- 0	3)	6- 0- 0	5)	4- 0- 0
2)	4- 0- 0	4)	6- 0- 0	6)	0- 0- 0

In-Plant Quality Assurance with Cq= 1
----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc BSet Vert Horiz Uplift Y Type
0- 7-10 1 189 -315 B Pin
5-10-12 1 14 -71 -19 B Pin
PROVIDE UPLIFT CONNECTION PER SCHEDULE
..WEB..FORCE..CSI..Cr..WEB..FORCE..CSI..Cr
5- 2 538 0.13 C 4- 3 28 0.12

Diagram

Scale = 0.4619

CANT: 0- 4- 0
EXCEPT AS SHOWN PLATES ARE MiTek MT20
(R0-11-8)

Warnings

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D Onofrio Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNV\MG4.prx

Eng Job:

WO: SUNV3V
TI: MG4
Dsgnr: TLY Chk:
9/20/2006

TC Live 16.0 psf
TC Dead 7.0 psf
BC Live 10.0 psf
BC Dead 10.0 psf
TOTAL 43.0 psf

Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2- 0- 0
TPI-02/FBC-04
Code: FLA
v4.7.32-47147

Job: SUNBURY V 3 V

Customer: WO: SUNV3V

TI: RG1

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

GBL: 2x 4 SP #3

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or expertise in control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuous braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 1/4" gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x6 plate is 6" wide x 8" long. (Shots tholes) with members shall meet at the centroid of the web or face unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry practice and are to be followed with particular care during handling and building, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vart Horiz Uplift Y Type

0- 4- 0 1 95 -120 -83 B Pin

11- 0- 0 1 95 -120 -83 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..TC.. FORCE.. CSI.. Cr ..BC.. FORCE.. CSI..

1- 2 134 0.26 A 1- 9 0 0.07

2- 3 86 0.13 A 9- 8 0 0.04

3- 4 86 0.19 A 8- 7 0 0.04

4- 5 -117 0.18 A 7- 5 0 0.07

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-44/134, 2-3=-37/86, 3-4=-48/86, 4-5=-117/10,

BOT CHORD 1-9=0/0.9-8=0/0.8-7=0/0.7-5=0/0.0,

Webs 9-2=-134/272, 8-3=-62/53, 7-4=-134/272,

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.02/999 0.02/999 5-4

Horiz. 0.00 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

Joint Locations

1) 0- 0- 0 5) 11- 4- 0 9) 3- 8- 0

2) 3- 8- 0 6) 11- 4- 0 10) 0- 0- 0

3) 5- 8- 0 7) 7- 8- 0

4) 7- 8- 0 8) 5- 8- 0

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vart Horiz Uplift Y Type

0- 4- 0 1 95 -120 -83 B Pin

11- 0- 0 1 95 -120 -83 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..TC.. FORCE.. CSI.. Cr ..BC.. FORCE.. CSI..

1- 2 134 0.26 A 1- 9 0 0.07

2- 3 86 0.13 A 9- 8 0 0.04

3- 4 86 0.19 A 8- 7 0 0.04

4- 5 -117 0.18 A 7- 5 0 0.07

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-44/134, 2-3=-37/86, 3-4=-48/86, 4-5=-117/10,

BOT CHORD 1-9=0/0.9-8=0/0.8-7=0/0.7-5=0/0.0,

Webs 9-2=-134/272, 8-3=-62/53, 7-4=-134/272,

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.02/999 0.02/999 5-4

Horiz. 0.00 0.00 0.00 NA

Max DL Deflection L/999 = 0.00

Long term deflection factor = 1.50

Joint Locations

1) 0- 0- 0 5) 11- 4- 0 9) 3- 8- 0

2) 3- 8- 0 6) 11- 4- 0 10) 0- 0- 0

3) 5- 8- 0 7) 7- 8- 0

4) 7- 8- 0 8) 5- 8- 0

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.5314

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oroff Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3513

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR-1\RG1.prx

Eng Job:

DWG:

Dsgnr: ECH

Chk:

WO: SUNV3V

TI: RG1

6/7/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 2.0 psf

TOTAL 35.0 psf

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-47179

Job: SUNBURY V 3 V

Customer:

WJ: SUNV3V

TI: T4

Qty: 2

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSIRUP 1-1993 and NDS-97 to be incorporated as part of the building design by a building designer. The design loadings shown must be checked with the building designer. The design is in agreement with the local building code. The design is for a wind or snow load. The design is for a specific applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall be installed over knotholes, knots or distorted members. The truss shall be fabricated in accordance with the design. Connector plates shall be located on both ends of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSIRUP 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design. In-Plant Quality Assurance with Cq= 1 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-1357/670, 2-3=-1088/546, 3-4=-1088/546 4-5=-1357/670, 5-6=-1088/546, 6-7=-1357/670, 7-8=-1088/546 BOT CHORD 8-7=-513/1179, 7-6=-513/1179, 6-5=-513/1179, 5-4=-513/1179, 4-3=-513/1179, 3-2=-513/1179, 2-1=-513/1179 Webs 2-7=-294/255, 7-3=-212/718, 3-4=-212/718, 4-5=-212/718, 5-6=-212/718, 6-7=-212/718, 7-8=-212/718, 8-9=-212/718, 9-10=-212/718, 10-11=-212/718, 11-12=-212/718, 12-13=-212/718, 13-14=-212/718, 14-15=-212/718, 15-16=-212/718, 16-17=-212/718, 17-18=-212/718, 18-19=-212/718, 19-20=-212/718, 20-21=-212/718, 21-22=-212/718, 22-23=-212/718, 23-24=-212/718, 24-25=-212/718, 25-26=-212/718, 26-27=-212/718, 27-28=-212/718, 28-29=-212/718, 29-30=-212/718, 30-31=-212/718, 31-32=-212/718, 32-33=-212/718, 33-34=-212/718, 34-35=-212/718, 35-36=-212/718, 36-37=-212/718, 37-38=-212/718, 38-39=-212/718, 39-40=-212/718, 40-41=-212/718, 41-42=-212/718, 42-43=-212/718, 43-44=-212/718, 44-45=-212/718, 45-46=-212/718, 46-47=-212/718, 47-48=-212/718, 48-49=-212/718, 49-50=-212/718, 50-51=-212/718, 51-52=-212/718, 52-53=-212/718, 53-54=-212/718, 54-55=-212/718, 55-56=-212/718, 56-57=-212/718, 57-58=-212/718, 58-59=-212/718, 59-60=-212/718, 60-61=-212/718, 61-62=-212/718, 62-63=-212/718, 63-64=-212/718, 64-65=-212/718, 65-66=-212/718, 66-67=-212/718, 67-68=-212/718, 68-69=-212/718, 69-70=-212/718, 70-71=-212/718, 71-72=-212/718, 72-73=-212/718, 73-74=-212/718, 74-75=-212/718, 75-76=-212/718, 76-77=-212/718, 77-78=-212/718, 78-79=-212/718, 79-80=-212/718, 80-81=-212/718, 81-82=-212/718, 82-83=-212/718, 83-84=-212/718, 84-85=-212/718, 85-86=-212/718, 86-87=-212/718, 87-88=-212/718, 88-89=-212/718, 89-90=-212/718, 90-91=-212/718, 91-92=-212/718, 92-93=-212/718, 93-94=-212/718, 94-95=-212/718, 95-96=-212/718, 96-97=-212/718, 97-98=-212/718, 98-99=-212/718, 99-100=-212/718, 100-101=-212/718, 101-102=-212/718, 102-103=-212/718, 103-104=-212/718, 104-105=-212/718, 105-106=-212/718, 106-107=-212/718, 107-108=-212/718, 108-109=-212/718, 109-110=-212/718, 110-111=-212/718, 111-112=-212/718, 112-113=-212/718, 113-114=-212/718, 114-115=-212/718, 115-116=-212/718, 116-117=-212/718, 117-118=-212/718, 118-119=-212/718, 119-120=-212/718, 120-121=-212/718, 121-122=-212/718, 122-123=-212/718, 123-124=-212/718, 124-125=-212/718, 125-126=-212/718, 126-127=-212/718, 127-128=-212/718, 128-129=-212/718, 129-130=-212/718, 130-131=-212/718, 131-132=-212/718, 132-133=-212/718, 133-134=-212/718, 134-135=-212/718, 135-136=-212/718, 136-137=-212/718, 137-138=-212/718, 138-139=-212/718, 139-140=-212/718, 140-141=-212/718, 141-142=-212/718, 142-143=-212/718, 143-144=-212/718, 144-145=-212/718, 145-146=-212/718, 146-147=-212/718, 147-148=-212/718, 148-149=-212/718, 149-150=-212/718, 150-151=-212/718, 151-152=-212/718, 152-153=-212/718, 153-154=-212/718, 154-155=-212/718, 155-156=-212/718, 156-157=-212/718, 157-158=-212/718, 158-159=-212/718, 159-160=-212/718, 160-161=-212/718, 161-162=-212/718, 162-163=-212/718, 163-164=-212/718, 164-165=-212/718, 165-166=-212/718, 166-167=-212/718, 167-168=-212/718, 168-169=-212/718, 169-170=-212/718, 170-171=-212/718, 171-172=-212/718, 172-173=-212/718, 173-174=-212/718, 174-175=-212/718, 175-176=-212/718, 176-177=-212/718, 177-178=-212/718, 178-179=-212/718, 179-180=-212/718, 180-181=-212/718, 181-182=-212/718, 182-183=-212/718, 183-184=-212/718, 184-185=-212/718, 185-186=-212/718, 186-187=-212/718, 187-188=-212/718, 188-189=-212/718, 189-190=-212/718, 190-191=-212/718, 191-192=-212/718, 192-193=-212/718, 193-194=-212/718, 194-195=-212/718, 195-196=-212/718, 196-197=-212/718, 197-198=-212/718, 198-199=-212/718, 199-200=-212/718, 200-201=-212/718, 201-202=-212/718, 202-203=-212/718, 203-204=-212/718, 204-205=-212/718, 205-206=-212/718, 206-207=-212/718, 207-208=-212/718, 208-209=-212/718, 209-210=-212/718, 210-211=-212/718, 211-212=-212/718, 212-213=-212/718, 213-214=-212/718, 214-215=-212/718, 215-216=-212/718, 216-217=-212/718, 217-218=-212/718, 218-219=-212/718, 219-220=-212/718, 220-221=-212/718, 221-222=-212/718, 222-223=-212/718, 223-224=-212/718, 224-225=-212/718, 225-226=-212/718, 226-227=-212/718, 227-228=-212/718, 228-229=-212/718, 229-230=-212/718, 230-231=-212/718, 231-232=-212/718, 232-233=-212/718, 233-234=-212/718, 234-235=-212/718, 235-236=-212/718, 236-237=-212/718, 237-238=-212/718, 238-239=-212/718, 239-240=-212/718, 240-241=-212/718, 241-242=-212/718, 242-243=-212/718, 243-244=-212/718, 244-245=-212/718, 245-246=-212/718, 246-247=-212/718, 247-248=-212/718, 248-249=-212/718, 249-250=-212/718, 250-251=-212/718, 251-252=-212/718, 252-253=-212/718, 253-254=-212/718, 254-255=-212/718, 255-256=-212/718, 256-257=-212/718, 257-258=-212/718, 258-259=-212/718, 259-260=-212/718, 260-261=-212/718, 261-262=-212/718, 262-263=-212/718, 263-264=-212/718, 264-265=-212/718, 265-266=-212/718, 266-267=-212/718, 267-268=-212/718, 268-269=-212/718, 269-270=-212/718, 270-271=-212/718, 271-272=-212/718, 272-273=-212/718, 273-274=-212/718, 274-275=-212/718, 275-276=-212/718, 276-277=-212/718, 277-278=-212/718, 278-279=-212/718, 279-280=-212/718, 280-281=-212/718, 281-282=-212/718, 282-283=-212/718, 283-284=-212/718, 284-285=-212/718, 285-286=-212/718, 286-287=-212/718, 287-288=-212/718, 288-289=-212/718, 289-290=-212/718, 290-291=-212/718, 291-292=-212/718, 292-293=-212/718, 293-294=-212/718, 294-295=-212/718, 295-296=-212/718, 296-297=-212/718, 297-298=-212/718, 298-299=-212/718, 299-300=-212/718, 300-301=-212/718, 301-302=-212/718, 302-303=-212/718, 303-304=-212/718, 304-305=-212/718, 305-306=-212/718, 306-307=-212/718, 307-308=-212/718, 308-309=-212/718, 309-310=-212/718, 310-311=-212/718, 311-312=-212/718, 312-313=-212/718, 313-314=-212/718, 314-315=-212/718, 315-316=-212/718, 316-317=-212/718, 317-318=-212/718, 318-319=-212/718, 319-320=-212/718, 320-321=-212/718, 321-322=-212/718, 322-323=-212/718, 323-324=-212/718, 324-325=-212/718, 325-326=-212/718, 326-327=-212/718, 327-328=-212/718, 328-329=-212/718, 329-330=-212/718, 330-331=-212/718, 331-332=-212/718, 332-333=-212/718, 333-334=-212/718, 334-335=-212/718, 335-336=-212/718, 336-337=-212/718, 337-338=-212/718, 338-339=-212/718, 339-340=-212/718, 340-341=-212/718, 341-342=-212/718, 342-343=-212/718, 343-344=-212/718, 344-345=-212/718, 345-346=-212/718, 346-347=-212/718, 347-348=-212/718, 348-349=-212/718, 349-350=-212/718, 350-351=-212/718, 351-352=-212/718, 352-353=-212/718, 353-354=-212/718, 354-355=-212/718, 355-356=-212/718, 356-357=-212/718, 357-358=-212/718, 358-359=-212/718, 359-360=-212/718, 360-361=-212/718, 361-362=-212/718, 362-363=-212/718, 363-364=-212/718, 364-365=-212/718, 365-366=-212/718, 366-367=-212/718, 367-368=-212/718, 368-369=-212/718, 369-370=-212/718, 370-371=-212/718, 371-372=-212/718, 372-373=-212/718, 373-374=-212/718, 374-375=-212/718, 375-376=-212/718, 376-377=-212/718, 377-378=-212/718, 378-379=-212/718, 379-380=-212/718, 380-381=-212/718, 381-382=-212/718, 382-383=-212/718, 383-384=-212/718, 384-385=-212/718, 385-386=-212/718, 386-387=-212/718, 387-388=-212/718, 388-389=-212/718, 389-390=-212/718, 390-391=-212/718, 391-392=-212/718, 392-393=-212/718, 393-394=-212/718, 394-395=-212/718, 395-396=-212/718, 396-397=-212/718, 397-398=-212/718, 398-399=-212/718, 399-400=-212/718, 400-401=-212/718, 401-402=-212/718, 402-403=-212/718, 403-404=-212/718, 404-405=-212/718, 405-406=-212/718, 406-407=-212/718, 407-408=-212/718, 408-409=-212/718, 409-410=-212/718, 410-411=-212/718, 411-412=-212/718, 412-413=-212/718, 413-414=-212/718, 414-415=-212/718, 415-416=-212/718, 416-417=-212/718, 417-418=-212/718, 418-419=-212/718, 419-420=-212/718, 420-421=-212/718, 421-422=-212/718, 422-423=-212/718, 423-424=-212/718, 424-425=-212/718, 425-426=-212/718, 426-427=-212/718, 427-428=-212/718, 428-429=-212/718, 429-430=-212/718, 430-431=-212/718, 431-432=-212/718, 432-433=-212/718, 433-434=-212/718, 434-435=-212/718, 435-436=-212/718, 436-437=-212/718, 437-438=-212/718, 438-439=-212/718, 439-440=-212/718, 440-441=-212/718, 441-442=-212/718, 442-443=-212/718, 443-444=-212/718, 444-445=-212/718, 445-446=-212/718, 446-447=-212/718, 447-448=-212/718, 448-449=-212/718, 449-450=-212/718, 450-451=-212/718, 451-452=-212/718, 452-453=-212/718, 453-454=-212/718, 454-455=-212/718, 455-456=-212/718, 456-457=-212/718, 457-458=-212/718, 458-459=-212/718, 459-460=-212/718, 460-461=-212/718, 461-462=-212/718, 462-463=-212/718, 463-464=-212/718, 464-465=-212/718, 465-466=-212/718, 466-467=-212/718, 467-468=-212/718, 468-469=-212/718, 469-470=-212/718, 470-471=-212/718, 471-472=-212/718, 472-473=-212/718, 473-474=-212/718, 474-475=-212/718, 475-476=-212/718, 476-477=-212/718, 477-478=-212/718, 478-479=-212/718, 479-480=-212/718, 480-481=-212/718, 481-482=-212/718, 482-483=-212/718, 483-484=-212/718, 484-485=-212/718, 485-486=-212/718, 486-487=-212/718, 487-488=-212/718, 488-489=-212/718, 489-490=-212/718, 490-491=-212/718, 491-492=-212/718, 492-493=-212/718, 493-494=-212/718, 494-495=-212/718, 495-496=-212/718, 496-497=-212/718, 497-498=-212/718, 498-499=-212/718, 499-500=-212/718, 500-501=-212/718, 501-502=-212/718, 502-503=-212/718, 503-504=-212/718, 504-505=-212/718, 505-506=-212/718, 506-507=-212/718, 507-508=-212/718, 508-509=-212/718, 509-510=-212/718, 510-511=-212/718, 511-512=-212/718, 512-513=-212/718, 513-514=-212/718, 514-515=-212/718, 515-516=-212/718, 516-517=-212/718, 517-518=-212/718, 518-519=-212/718, 519-520=-212/718, 520-521=-212/718, 521-522=-212/718, 522-523=-212/718, 523-524=-212/718, 524-525=-212/718, 525-526=-212/718, 526-527=-212/718, 527-528=-212/718, 528-529=-212/718, 529-530=-212/718, 530-531=-212/718, 531-532=-212/718, 532-533=-212/718, 533-534=-212/718, 534-535=-212/718, 535-536=-212/718, 536-537=-212/718, 537-538=-212/718, 538-539=-212/718, 539-540=-212/718, 540-541=-212/718, 541-542=-212/718, 542-543=-212/718, 543-544=-212/718, 544-545=-212/718, 545-546=-212/718, 546-547=-212/718, 547-548=-212/718, 548-549=-212/718, 549-550=-212/718, 550-551=-212/718, 551-552=-212/718, 552-553=-212/718, 553-554=-212/718, 554-555=-212/718, 555-556=-212/718, 556-557=-212/718, 557-558=-212/718, 558-559=-212/718, 559-560=-212/718, 560-561=-212/718, 561-562=-212/718, 562-563=-212/718, 563-564=-212/718, 564-565=-212/718, 565-566=-212/718, 566-567=-212/718, 567-568=-212/718, 568-569=-212/718, 569-570=-212/718, 570-571=-212/718, 571-572=-212/718, 572-573=-212/718, 573-574=-212/718, 574-575=-212/718, 575-576=-212/718, 576-577=-212/718, 577-578=-212/718, 578-579=-212/718, 579-580=-212/718, 580-581=-212/718, 581-582=-212/718, 582-583=-212/718, 583-584=-212/718, 584-585=-212/718, 585-586=-212/718, 586-587=-212/718, 587-588=-212/718, 588-589=-212/718, 589-590=-212/718, 590-591=-212/718, 591-592=-212/718, 592-593=-212/718, 593-594=-212/718, 594-595=-212/718, 595-596=-212/718, 596-597=-212/718, 597-598=-212/718, 598-599=-212/718, 599-600=-212/718, 600-601=-212/718, 601-602=-212/718, 602-603=-212/718, 603-604=-212/718, 604-605=-212/718, 605-606=-212/718, 606-607=-212/718, 607-608=-212/718, 608-609=-212/718, 609-610=-212/718, 610-611=-212/718, 611-612=-212/718, 612-613=-212/718, 613-614=-212/718, 614-615=-212/718, 615-616=-212/718, 616-617=-212/718, 617-618=-212/718, 618-619=-212/718, 619-620=-212/718, 620-621=-212/718, 621-622=-212/718, 622-623=-212/718, 623-624=-212/718, 624-625=-212/718, 625-626=-212/718, 626-627=-212/718, 627-628=-212/718, 628-629=-212/718, 629-630=-212/718, 630-631=-212/718, 631-632=-212/718, 632-633=-212/718, 633-634=-212/718, 634-635=-212/718, 635-636=-212/718, 636-637=-212/718, 637-638=-212/718, 638-639=-212/718, 639-640=-212/718, 640-641=-212/718, 641-642=-212/718, 642-643=-212/718, 643-644=-212/718, 644-645=-212/718, 645-646=-212/718, 646-647=-212/718, 647-648=-212/718, 648-649=-212/718, 649-650=-212/718, 650-651=-212/718, 651-652=-212/718, 652-653=-212/718, 653-654=-212/718, 654-655=-212/718, 655-656=-212/718, 656-657=-212/718, 657-658=-212/718, 658-659=-212/718, 659-660=-212/718, 660-661=-212/718, 661-662=-212/718, 662-663=-212/718, 663-664=-212/718, 664-665=-212/718, 665-666=-212/718, 666-667=-212/718, 667-668=-212/718, 668-669=-212/718, 669-670=-212/718, 670-671=-212/718, 671-672=-212/718, 672-673=-212/718, 673-674=-212/718, 674-675=-212/718, 675-676=-212/718, 676-677=-212/718, 677-678=-212/

Job: SUNBURY V 3 V

Customer: WO: SUNV3V

TI: V31

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or warranty of control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid end blocking, they shall be braced to a maximum spacing of 10'-0". Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully embedded and shall be symmetrical to the bottom chord. A 6x8 plate is 6" wide x 8" long. A 6x4 plate is 6" wide x 4" long. A 6x6 plate is 6" wide x 6" long. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during loading and unloading, rigging, delivery and erection. Trusses are to be erected using temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

MULTIPLE LOADS -- This design is the composite result of multiple loads. Right End vertical designed for wind. In-Plant Quality Assurance with Cq= 1 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=11/40, C&C, V= 125mph, E= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl L= 40.0 ft W= 2.7 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 1.2 psf Impact Resistant Covering and/or Glazing Req

-----GLOBAL MAX DEFLECTIONS-----

LL TL in./Ratio in./Ratio Jnt(s) I. Span 0.01/xxx 0.00/xxx 4 Horiz. 0.00 0.00 NA Max DL Deflection L/xxx = 0.00 Long term deflection factor = 1.50

-----MAX. REACTIONS PER BEARING LOCATION----- X-Loc BSet Vert Horiz Uplift Y Type 0- 0-12 1 90 -121 -92 B Pin 2- 7- 4 1 96 0 -106 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE .WEB...FORCE..CSI..Cz .WEB...FORCE..CSI..Cz 3- 2 68 0.02 C

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.8648

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing. Wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 581 D'Ondra Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Saunders Rd, Eden, NC 27332

Maronda Systems

4005 MARONDA WAY Sanford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Medallion Pl. Chuluota, FL 32766

Eng Job: WO: SUNV3V

Dwg: TI: V31

Dsgnr: ECH Chk: 6/7/2006

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 2.0 psf

TOTAL 35.0 psf

Profile Path: C:\TEE-LOK\work\Jobs\SUNV3V\SUNBUR-1\V31.prx

Design: Matrix Analysis

Job: SUNBURY V 3 V

Customer: WO: SUNV3V

TI: V32

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for or damages resulting from reliance on this information.

This truss has been designed as an individual building component in accordance with ANS/ITPI 1-1993 and NDG-97 to be incorporated as part of the building design by the Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by shanking unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/ITPI 1-1993.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

MULTIPLE LOADS -- This design is the composite result of multiple loads. Right End vertical designed for wind. In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2--26/40,

BOT CHORD 4-3--39/43,

Wabs 3-2--120/172,

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 40 0.25 A 4- 3 43 0.27

Joint Locations

1) 0- 0- 0 3) 5- 4- 0

2) 5- 4- 0 4) 0- 0- 0

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0- 0-12 1 226 68 -213 B Pin

5- 3- 4 1 233 0 -237 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..WEB..FORCE..CSI..Cr..WEB..FORCE..CSI..Cr

3- 2 172 0.09 C

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.6911

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Orofino Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00500669

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR-1\V32.Prx

Eng Job:

Dwg:

Dsgnr: TLY Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: SUNV3V

TI: V32

6/7/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

V4.7.32-47145

Job: SUNBURY V 3 V

Customer: WO: SUNV3V

TI: V6

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for errors or omissions in the design.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by the Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuous braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on members shall be in the center of the webs unless otherwise shown. Connectors shall be sized in accordance with sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing the leading trusses in a rough build up shall be provided and maintained until erection forces shall be applied and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. In-Plant Quality Assurance with Cq= 1 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=69/190, 2-3=-135/66, BOT CHORD 1-5=0/0.5-3=0/0, Webs 5-2=-149/266, ..TC..FORCE..CSI..Cr..BC..FORCE..CSI.. 1- 2 190 0.42 A 1- 5 0 0.09 2- 3 -135 0.25 A 5- 3 0 0.09 ..WEB..FORCE..CSI..Cr..WEB..FORCE..CSI..Cr 5- 2 266 0.25 C

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. In-Plant Quality Assurance with Cq= 1 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=69/190, 2-3=-135/66, BOT CHORD 1-5=0/0.5-3=0/0, Webs 5-2=-149/266, ..TC..FORCE..CSI..Cr..BC..FORCE..CSI.. 1- 2 190 0.42 A 1- 5 0 0.09 2- 3 -135 0.25 A 5- 3 0 0.09 ..WEB..FORCE..CSI..Cr..WEB..FORCE..CSI..Cr 5- 2 266 0.25 C

Joint Locations

1) 0-0-0 3) 8-0-0 5) 4-0-0 2) 4-0-0 4) 8-0-0 6) 0-0-0

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 108 -170 -138 B Pin

7- 8- 0 1 108 -170 -138 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

..WEB..FORCE..CSI..Cr..WEB..FORCE..CSI..Cr 5- 2 266 0.25 C



Job: SUNBURY V 3 V

Customer: WO: SUNV3V

TI: V7

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be continuously braced with 2x4 gage and dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood joining. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 3' wide x 4' long. A 4x6 plate is 6' wide x 8' long. Slots (holes) are permitted in plate length specified. Double cuts on web members shall be indicated. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry practices. Trusses are to be installed with particular care during handling and erection. Temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires that temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADS -- This design is the composite result of multiple loads.

WindLoad per ASCE 7-02, Cfc, V= 125mph, H= 21.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 30.0 ft W= 4.0 ft Truss in END zone, TCDL= 4.2 psf, BCDL= 1.2 psf

Impact Resistant Covering and/or Glazing Req

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-3/71, 2-3=-3/71,

BOT CHORD 1-3=0/0, 2-3=-3/71,

..TC..FORCE..CSI..Cr..BC..FORCE..CSI..

1- 2 71 0.04 A 1- 3 0 0.03

2- 3 71 0.04 A

Joint Locations

1) 0- 0- 0 3) 4- 0- 0 5) 0- 0- 0

2) 2- 0- 0 4) 4- 0- 0

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Eset Vert Horiz Uplift Y Type

0- 4- 0 1 54 -79 -76 B Pin

3- 8- 0 1 54 -79 -76 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I. Span 0.00/999 0.00/999 3-1

Horiz. 0.00 0.00

Max DL Deflection L/999 = 0.00 NA

Long term deflection factor = 1.50

2-0-0 1 2 3

2-0-0 3

6.00 6.00 -6.00

4x6

3x4

3x4

1-0-0

54# 8.00"

Stud @ 4-0-0

54# 8.00"

4-0-0 5 4

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.8648

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\SUNV3V\SUNBUR-1\V7.prx

Eng Job:

Dwg:

Dsgnr:ECH Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 2.0 psf

TOTAL 35.0 psf

WO: SUNV3V

TI: V7

6/7/2006

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.32-47170

Job: SUNF		Customer:		WO: SUNF		TI: PA-G		Qty: 2	
TC: 4x 2 SP #2 BC: 4x 2 SP #2 WB: 4x 2 SP #3 GB: 4x 2 SP #3 All COMPRESSION Chords are assumed to be continuous unless noted otherwise. ---MAX. REACTIONS PER BEARING LOCATION--- X-LOC Bset Vert Horiz Uplift Y Type 0- 0-12 1 4 0 0 0 B H Roll 1- 1- 0 1 13 0 0 0 B Pin 13-10- 8 1 5 0 0 0 B Pin 15- 2- 8 1 5 0 0 0 B Pin 27- 9-12 1 20 0 0 0 B Pin PROVIDE UPLIFT CONNECTION PER SCHEDULE ---TC. ---FORCE..CSI..CR ---BC. ---FORCE..CSI..CR 1- 2 0 0.07 C 50-49 2- 3 0 0.09 C 49-48 3- 4 0 0.09 C 48-47 4- 5 0 0.08 C 47-46 5- 6 0 0.08 C 46-45 6- 7 0 0.08 C 45-44 7- 8 0 0.08 C 44-43 8- 9 0 0.09 C 43-42 9-10 0 0.09 C 42-41 10-11 0 0.06 C 41-40 11-12 0 0.07 C 40-39 12-13 0 0.08 C 39-38 13-14 0 0.08 C 38-37 14-15 0 0.08 C 37-36 15-16 0 0.08 C 36-35 16-17 0 0.09 C 35-34 17-18 0 0.06 C 34-33 18-19 0 0.06 C 33-32 20-21 0 0.10 C 32-31 21-22 0 0.10 C 31-30 22-23 0 0.19 C 30-29 23-24 0 0.19 C 29-28 24-25 0 0.02 Analysis based on Simplified Analog Model. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing. In-Plant Quality Assurance with Cg= 1.11 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=0/0, 2-3=0/0, 3-4=0/0, 4-5=0/0, 5-6=0/0, 6-7=0/0, 7-8=0/0, 8-9=0/0, 9-10=0/0, 10-11=0/0, 11-12=0/0, 12-13=0/0, 13-14=0/0, 14-15=0/0, 15-16=0/0, 16-17=0/0, 17-18=0/0, 18-19=0/0, 19-20=0/0, 20-21=0/0, 21-22=0/0, 22-23=0/0, 23-24=0/0, 24-25=0/0. BOT CHORD 30-49=0/0, 49-48=0/0, 48-47=0/0, 47-46=0/0, 46-45=0/0, 45-44=0/0, 44-43=0/0, 43-42=0/0, 42-41=0/0, 41-40=0/0, 40-39=0/0, 39-38=0/0, 38-37=0/0, 37-36=0/0, 36-35=0/0, 35-34=0/0, 34-33=0/0, 33-32=0/0, 32-31=0/0, 31-30=0/0, 30-29=0/0, 29-28=0/0, 28-27=0/0, 27-26=0/0, 26-25=0/0. Web 50-1=0/0, 49-2=0/0, 48-3=0/0, 47-4=0/0, 46-5=0/0, 45-6=0/0, 44-7=0/0, 43-8=0/0, 42-9=0/0, 41-10=0/0, 40-11=0/0, 39-12=0/0, 38-13=0/0, 37-14=0/0, 36-15=0/0, 35-16=0/0, 34-17=0/0, 33-18=0/0, 32-19=0/0, 31-20=0/0, 30-21=0/0, 29-22=0/0, 28-23=0/0, 27-24=0/0, 26-25=0/0. -----GLOBAL MAX DEFLECTIONS----- LL TL In./Ratio In./Ratio Jnt(s) I-Span 0.00/999 0.00/999 24-23 Horiz. 0.00 0.00 NA Max DL Deflection L/999 = 0.00 Long term deflection factor = 1.50 0-9-4 2x4 2									

DESIGN INFORMATION

This design is for an individual building component and has been prepared by Sunbury Engineering, Inc. for the use of the client. The client is responsible for the design and construction of the building and for the safety of the building and its occupants. The client is also responsible for the design and construction of the building and for the safety of the building and its occupants.

This truss has been designed as an individual building component in accordance with ANS/PTI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0". Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum street based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/PTI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

Job: SUNF

Customer:

WO: SUNF

TI: FB

Qty: 10

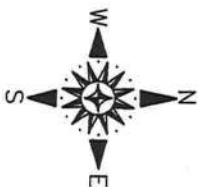
TC: 4x 2 SP #2 D 8-17, 17-21
BC: 4x 2 SP #2 D
WB: 4x 2 SP #1 D 31-34, 22-31
4x 2 SP #2 0-0

MULTIPLE LOADS -- This design is the composite result of multiple loads. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. **CUT TOP CHORD OVER INTERIOR SUPPORT ***** This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2 = 968/0.2 = 968/0.3 = 4-2568/0.4 = 5-3710/0.5 = 6-3868/0.6 = 7-3968/0.7 = 8-4145/0.8 = 9-4250/0.9 = 10-4350/1.0 = 11-4450/1.1 = 12-4550/1.2 = 13-4650/1.3 = 14-4750/1.4 = 15-4850/1.5 = 16-4950/1.6 = 17-5050/1.7 = 18-5150/1.8 = 19-5250/1.9 = 20-5350/2.0 = 21-5450/2.1 = 22-5550/2.2 = 23-5650/2.3 = 24-5750/2.4 = 25-5850/2.5 = 26-5950/2.6 = 27-6050/2.7 = 28-6150/2.8 = 29-6250/2.9 = 30-6350/3.0 = 31-6450/3.1 = 32-6550/3.2 = 33-6650/3.3 = 34-6750/3.4 = 35-6850/3.5 = 36-6950/3.6 = 37-7050/3.7 = 38-7150/3.8 = 39-7250/3.9 = 40-7350/4.0 = 41-7450/4.1 = 42-7550/4.2 = 43-7650/4.3 = 44-7750/4.4 = 45-7850/4.5 = 46-7950/4.6 = 47-8050/4.7 = 48-8150/4.8 = 49-8250/4.9 = 50-8350/5.0 = 51-8450/5.1 = 52-8550/5.2 = 53-8650/5.3 = 54-8750/5.4 = 55-8850/5.5 = 56-8950/5.6 = 57-9050/5.7 = 58-9150/5.8 = 59-9250/5.9 = 60-9350/6.0 = 61-9450/6.1 = 62-9550/6.2 = 63-9650/6.3 = 64-9750/6.4 = 65-9850/6.5 = 66-9950/6.6 = 70-10050/7.0 = 71-10150/7.1 = 72-10250/7.2 = 73-10350/7.3 = 74-10450/7.4 = 75-10550/7.5 = 76-10650/7.6 = 77-10750/7.7 = 78-10850/7.8 = 79-10950/7.9 = 80-11050/8.0 = 81-11150/8.1 = 82-11250/8.2 = 83-11350/8.3 = 84-11450/8.4 = 85-11550/8.5 = 86-11650/8.6 = 87-11750/8.7 = 88-11850/8.8 = 89-11950/8.9 = 90-12050/9.0 = 91-12150/9.1 = 92-12250/9.2 = 93-12350/9.3 = 94-12450/9.4 = 95-12550/9.5 = 96-12650/9.6 = 97-12750/9.7 = 98-12850/9.8 = 99-12950/9.9 = 100-13050/10.0 = 101-13150/10.1 = 102-13250/10.2 = 103-13350/10.3 = 104-13450/10.4 = 105-13550/10.5 = 106-13650/10.6 = 107-13750/10.7 = 108-13850/10.8 = 109-13950/10.9 = 110-14050/11.0 = 111-14150/11.1 = 112-14250/11.2 = 113-14350/11.3 = 114-14450/11.4 = 115-14550/11.5 = 116-14650/11.6 = 117-14750/11.7 = 118-14850/11.8 = 119-14950/11.9 = 120-15050/12.0 = 121-15150/12.1 = 122-15250/12.2 = 123-15350/12.3 = 124-15450/12.4 = 125-15550/12.5 = 126-15650/12.6 = 127-15750/12.7 = 128-15850/12.8 = 129-15950/12.9 = 130-16050/13.0 = 131-16150/13.1 = 132-16250/13.2 = 133-16350/13.3 = 134-16450/13.4 = 135-16550/13.5 = 136-16650/13.6 = 137-16750/13.7 = 138-16850/13.8 = 139-16950/13.9 = 140-17050/14.0 = 141-17150/14.1 = 142-17250/14.2 = 143-17350/14.3 = 144-17450/14.4 = 145-17550/14.5 = 146-17650/14.6 = 147-17750/14.7 = 148-17850/14.8 = 149-17950/14.9 = 150-18050/15.0 = 151-18150/15.1 = 152-18250/15.2 = 153-18350/15.3 = 154-18450/15.4 = 155-18550/15.5 = 156-18650/15.6 = 157-18750/15.7 = 158-18850/15.8 = 159-18950/15.9 = 160-19050/16.0 = 161-19150/16.1 = 162-19250/16.2 = 163-19350/16.3 = 164-19450/16.4 = 165-19550/16.5 = 166-19650/16.6 = 167-19750/16.7 = 168-19850/16.8 = 169-19950/16.9 = 170-20050/17.0 = 171-20150/17.1 = 172-20250/17.2 = 173-20350/17.3 = 174-20450/17.4 = 175-20550/17.5 = 176-20650/17.6 = 177-20750/17.7 = 178-20850/17.8 = 179-20950/17.9 = 180-21050/18.0 = 181-21150/18.1 = 182-21250/18.2 = 183-21350/18.3 = 184-21450/18.4 = 185-21550/18.5 = 186-21650/18.6 = 187-21750/18.7 = 188-21850/18.8 = 189-21950/18.9 = 190-22050/19.0 = 191-22150/19.1 = 192-22250/19.2 = 193-22350/19.3 = 194-22450/19.4 = 195-22550/19.5 = 196-22650/19.6 = 197-22750/19.7 = 198-22850/19.8 = 199-22950/19.9 = 200-23050/20.0 = 201-23150/20.1 = 202-23250/20.2 = 203-23350/20.3 = 204-23450/20.4 = 205-23550/20.5 = 206-23650/20.6 = 207-23750/20.7 = 208-23850/20.8 = 209-23950/20.9 = 210-24050/21.0 = 211-24150/21.1 = 212-24250/21.2 = 213-24350/21.3 = 214-24450/21.4 = 215-24550/21.5 = 216-24650/21.6 = 217-24750/21.7 = 218-24850/21.8 = 219-24950/21.9 = 220-25050/22.0 = 221-25150/22.1 = 222-25250/22.2 = 223-25350/22.3 = 224-25450/22.4 = 225-25550/22.5 = 226-25650/22.6 = 227-25750/22.7 = 228-25850/22.8 = 229-25950/22.9 = 230-26050/23.0 = 231-26150/23.1 = 232-26250/23.2 = 233-26350/23.3 = 234-26450/23.4 = 235-26550/23.5 = 236-26650/23.6 = 237-26750/23.7 = 238-26850/23.8 = 239-26950/23.9 = 240-27050/24.0 = 241-27150/24.1 = 242-27250/24.2 = 243-27350/24.3 = 244-27450/24.4 = 245-27550/24.5 = 246-27650/24.6 = 247-27750/24.7 = 248-27850/24.8 = 249-27950/24.9 = 250-28050/25.0 = 251-28150/25.1 = 252-28250/25.2 = 253-28350/25.3 = 254-28450/25.4 = 255-28550/25.5 = 256-28650/25.6 = 257-28750/25.7 = 258-28850/25.8 = 259-28950/25.9 = 260-29050/26.0 = 261-29150/26.1 = 262-29250/26.2 = 263-29350/26.3 = 264-29450/26.4 = 265-29550/26.5 = 266-29650/26.6 = 267-29750/26.7 = 268-29850/26.8 = 269-29950/26.9 = 270-30050/27.0 = 271-30150/27.1 = 272-30250/27.2 = 273-30350/27.3 = 274-30450/27.4 = 275-30550/27.5 = 276-30650/27.6 = 277-30750/27.7 = 278-30850/27.8 = 279-30950/27.9 = 280-31050/28.0 = 281-31150/28.1 = 282-31250/28.2 = 283-31350/28.3 = 284-31450/28.4 = 285-31550/28.5 = 286-31650/28.6 = 287-31750/28.7 = 288-31850/28.8 = 289-31950/28.9 = 290-32050/29.0 = 291-32150/29.1 = 292-32250/29.2 = 293-32350/29.3 = 294-32450/29.4 = 295-32550/29.5 = 296-32650/29.6 = 297-32750/29.7 = 298-32850/29.8 = 299-32950/29.9 = 300-33050/30.0 = 301-33150/30.1 = 302-33250/30.2 = 303-33350/30.3 = 304-33450/30.4 = 305-33550/30.5 = 306-33650/30.6 = 307-33750/30.7 = 308-33850/30.8 = 309-33950/30.9 = 310-34050/31.0 = 311-34150/31.1 = 312-34250/31.2 = 313-34350/31.3 = 314-34450/31.4 = 315-34550/31.5 = 316-34650/31.6 = 317-34750/31.7 = 318-34850/31.8 = 319-34950/31.9 = 320-35050/32.0 = 321-35150/32.1 = 322-35250/32.2 = 323-35350/32.3 = 324-35450/32.4 = 325-35550/32.5 = 326-35650/32.6 = 327-35750/32.7 = 328-35850/32.8 = 329-35950/32.9 = 330-36050/33.0 = 331-36150/33.1 = 332-36250/33.2 = 333-36350/33.3 = 334-36450/33.4 = 335-36550/33.5 = 336-36650/33.6 = 337-36750/33.7 = 338-36850/33.8 = 339-36950/33.9 = 340-37050/34.0 = 341-37150/34.1 = 342-37250/34.2 = 343-37350/34.3 = 344-37450/34.4 = 345-37550/34.5 = 346-37650/34.6 = 347-37750/34.7 = 348-37850/34.8 = 349-37950/34.9 = 350-38050/35.0 = 351-38150/35.1 = 352-38250/35.2 = 353-38350/35.3 = 354-38450/35.4 = 355-38550/35.5 = 356-38650/35.6 = 357-38750/35.7 = 358-38850/35.8 = 359-38950/35.9 = 360-39050/36.0 = 361-39150/36.1 = 362-39250/36.2 = 363-39350/36.3 = 364-39450/36.4 = 365-39550/36.5 = 366-39650/36.6 = 367-39750/36.7 = 368-39850/36.8 = 369-39950/36.9 = 370-40050/37.0 = 371-40150/37.1 = 372-40250/37.2 = 373-40350/37.3 = 374-40450/37.4 = 375-40550/37.5 = 376-40650/37.6 = 377-40750/37.7 = 378-40850/37.8 = 379-40950/37.9 = 380-41050/38.0 = 381-41150/38.1 = 382-41250/38.2 = 383-41350/38.3 = 384-41450/38.4 = 385-41550/38.5 = 386-41650/38.6 = 387-41750/38.7 = 388-41850/38.8 = 389-41950/38.9 = 390-42050/39.0 = 391-42150/39.1 = 392-42250/39.2 = 393-42350/39.3 = 394-42450/39.4 = 395-42550/39.5 = 396-42650/39.6 = 397-42750/39.7 = 398-42850/39.8 = 399-42950/39.9 = 400-43050/40.0 = 401-43150/40.1 = 402-43250/40.2 = 403-43350/40.3 = 404-43450/40.4 = 405-43550/40.5 = 406-43650/40.6 = 407-43750/40.7 = 408-43850/40.8 = 409-43950/40.9 = 410-44050/41.0 = 411-44150/41.1 = 412-44250/41.2 = 413-44350/41.3 = 414-44450/41.4 = 415-44550/41.5 = 416-44650/41.6 = 417-44750/41.7 = 418-44850/41.8 = 419-44950/41.9 = 420-45050/42.0 = 421-45150/42.1 = 422-45250/42.2 = 423-45350/42.3 = 424-45450/42.4 = 425-45550/42.5 = 426-45650/42.6 = 427-45750/42.7 = 428-45850/42.8 = 429-45950/42.9 = 430-46050/43.0 = 431-46150/43.1 = 432-46250/43.2 = 433-46350/43.3 = 434-46450/43.4 = 435-46550/43.5 = 436-46650/43.6 = 437-46750/43.7 = 438-46850/43.8 = 439-46950/43.9 = 440-47050/44.0 = 441-47150/44.1 = 442-47250/44.2 = 443-47350/44.3 = 444-47450/44.4 = 445-47550/44.5 = 446-47650/44.6 = 447-47750/44.7 = 448-47850/44.8 = 449-47950/44.9 = 450-48050/45.0 = 451-48150/45.1 = 452-48250/45.2 = 453-48350/45.3 = 454-48450/45.4 = 455-48550/45.5 = 456-48650/45.6 = 457-48750/45.7 = 458-48850/45.8 = 459-48950/45.9 = 460-49050/46.0 = 461-49150/46.1 = 462-49250/46.2 = 463-49350/46.3 = 464-49450/46.4 = 465-49550/46.5 = 466-49650/46.6 = 467-49750/46.7 = 468-49850/46.8 = 469-49950/46.9 = 470-50050/47.0 = 471-50150/47.1 = 472-50250/47.2 = 473-50350/47.3 = 474-50450/47.4 = 475-50550/47.5 = 476-50650/47.6 = 477-50750/47.7 = 478-50850/47.8 = 479-50950/47.9 = 480-51050/48.0 = 481-51150/48.1 = 482-51250/48.2 = 483-51350/48.3 = 484-51450/48.4 = 485-51550/48.5 = 486-51650/48.6 = 487-51750/48.7 = 488-51850/48.8 = 489-51950/48.9 = 490-52050/49.0 = 491-52150/49.1 = 492-52250/49.2 = 493-52350/49.3 = 494-52450/49.4 = 495-52550/49.5 = 496-52650/49.6 = 497-52750/49.7 = 498-52850/49.8 = 499-52950/49.9 = 500-53050/50.0 = 501-53150/50.1 = 502-53250/50.2 = 503-53350/50.3 = 504-53450/50.4 = 505-53550/50.5 = 506-53650/50.6 = 507-53750/50.7 = 508-53850/50.8 = 509-53950/50.9 = 510-54050/51.0 = 511-54150/51.1 = 512-54250/51.2 = 513-54350/51.3 = 514-54450/51.4 = 515-54550/51.5 = 516-54650/51.6 = 517-54750/51.7 = 518-54850/51.8 = 519-54950/51.9 = 520-55050/52.0 = 521-55150/52.1 = 522-55250/52.2 = 523-55350/52.3 = 524-55450/52.4 = 525-55550/52.5 = 526-55650/52.6 = 527-55750/52.7 = 528-55850/52.8 = 529-55950/52.9 = 530-56050/53.0 = 531-56150/53.1 = 532-56250/53.2 = 533-56350/53.3 = 534-56450/53.4 = 535-56550/53.5 = 536-56650/53.6 = 537-56750/53.7 = 538-56850/53.8 = 539-56950/53.9 = 540-57050/54.0 = 541-57150/54.1 = 542-57250/54.2 = 543-57350/54.3 = 544-57450/54.4 = 545-57550/54.5 = 546-57650/54.6 = 547-57750/54.7 = 548-57850/54.8 = 549-57950/54.9 = 550-58050/55.0 = 551-58150/55.1 = 552-58250/55.2 = 553-58350/55.3 = 554-58450/55.4 = 555-58550/55.5 = 556-58650/55.6 = 557-58750/55.7 = 558-58850/55.8 = 559-58950/55.9 = 560-59050/56.0 = 561-59150/56.1 = 562-59250/56.2 = 563-59350/56.3 = 564-59450/56.4 = 565-59550/56.5 = 566-59650/56.6 = 567-59750/56.7 = 568-59850/56.8 = 569-59950/56.9 = 570-60050/57.0 = 571-60150/57.1 = 572-60250/57.2 = 573-60350/57.3 = 574-60450/57.4 = 575-60550/57.5 = 576-60650/57.6 = 577-60750/57.7 = 578-60850/57.8 = 579-60950/57.9 = 580-61050/58.0 = 581-61150/58.1 = 582-61250/58.2 = 583-61350/58.3 = 584-61450/58.4 = 585-61550/58.5 = 586-61650/58.6 = 587-61750/58.7 = 588-61850/58.8 = 589-61950/58.9 = 590-62050/59.0 = 591-62150/59.1 = 592-62250/59.2 = 593-62350/59.3 = 594-62450/59.4 = 595-62550/59.5 = 596-62650/59.6 = 597-62750/59.7 = 598-62850/59.8 = 599-62950/59.9 = 600-63050/60.0 = 601-63150/60.1 = 602-63250/60.2 = 603-63350/60.3 = 604-63450/60.4 = 605-63550/60.5 = 606-63650/60.6 = 607-63750/60.7 = 608-63850/60.8 = 609-63950/60.9 = 610-64050/61.0 = 611-64150/61.1 = 612-64250/61.2 = 613-64350/61.3 = 614-64450/61.4 = 615-64550/61.5 = 616-64650/61.6 = 617-64750/61.7 = 618-64850/61.8 = 619-64950/61.9 = 620-65050/62.0 = 621-65150/62.1 = 622-65250/62.2 = 623-65350/62.3 = 624-65450/62.4 = 625-65550/62.5 = 626-65650/62.6 = 627-65750/62.7 = 628-65850/62.8 = 629-65950/62.9 = 630-66050/63.0 = 631-66150/63.1 = 632-66250/63.2 = 633-66350/63.3 = 634-66450/63.4 = 635-66550/63.5 = 636-66650/63.6 = 637-66750/63.7 = 638-66850/63.8 = 639-66950/63.9 = 640-67050/64.0 = 641-67150/64.1 = 642-67250/64.2 = 643-67350/64.3 = 644-67450/64.4 = 645-67550/64.5 = 646-67650/64.6 = 647-67750/64.7 = 648-67850/64.8 = 649-67950/64.9 = 650-68050/65.0 = 651-68150/65.1 = 652-68250/65.2 = 653-68350/65.3 = 654-68450/65.4 = 655-68550/65.5 = 656-68650/65.6 = 657-68750/65.7 = 658-68850/65.8 = 659-68950/65.9 = 660-69050/66.0 = 661-69150/66.1 = 662-69250/66.2 = 663-69350/66.3 = 664-69450/66.4 = 665-69550/66.5 = 666-69650/66.6 = 667-69750/66.7 = 668-69850/66.8 = 669-69950/66.9 = 670-70050/67.0 = 671-70150/67.1 = 672-70250/67.2 = 673-70350/67.3 = 674-70450/67.4 = 675-70550/67.5 = 676-70650/67.6 = 677-70750/67.7 = 678-70850/67.8 = 679-70950/67.9 = 680-71050/68.0 = 681-71150/68.1 = 682-71250/68.2 = 683-71350/68.3 = 684-71450/68.4 = 685-71550/68.5 = 686-71650/68.6 = 687-71750/68.7 = 688-71850/68.8 = 689-71950/68.9 = 690-72050/69.0 = 691-72150/69.1 = 692-72250/69.2 = 693-72350/69.3 = 694-72450/69.4 = 695-72550/69.5 = 696-72650/69.6 = 697-72750/69.7 = 698-72850/69.8 = 699-72950/69.9 = 700-73050/70.0 = 701-73150/70.1 = 702-73250/70.2 = 703-73350/70.3 = 704-73450/70.4 = 705-73550/70.5 = 706-73650/70.6 = 707-73750/70.7 = 708-73850/70.8 = 709-73950/70

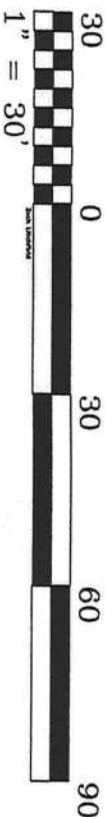
Maronda Systems 4005 MARONDA WAY Sanford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Medallion Pl, Chuluota, FL 32766	Design: Matrix Analysis WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing. Wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only and is not erection bracing. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. The engineering of the overall structure may be required. (See HIB-91 of TPI) For specific post bracing requirements, contact building designer. (Truss Pile Institute, TPI) is located at 583 D'Ondra Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Saunders Rd, Edenboro, NC 27632 Profile Path: C:\TEE-LOK\Work\Jobs\SUNF\SUNBURY FLOOR\SUNF\FC.prx <table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="width: 30%;">Eng Job: SUNBURY</td> <td style="width: 30%;">WO: SUNF</td> </tr> <tr> <td>Dwg:</td> <td>TI: FC</td> </tr> <tr> <td>Dsgnr: TLY</td> <td>Chk: 4/4/2006</td> </tr> <tr> <td>TC Live 40.0</td> <td>psf Lbr DF: 1.00</td> </tr> <tr> <td>TC Dead 10.0</td> <td>psf Plt DF: 1.00</td> </tr> <tr> <td>BC Live 0.0</td> <td>psf O.C.: 2- 0- 0</td> </tr> <tr> <td>BC Dead 5.0</td> <td>psf TFI-02/FBC-04</td> </tr> <tr> <td></td> <td>Code: FLA</td> </tr> <tr> <td>TOTAL 55.0</td> <td>psf v4.7.32-48969</td> </tr> </table>	Eng Job: SUNBURY	WO: SUNF	Dwg:	TI: FC	Dsgnr: TLY	Chk: 4/4/2006	TC Live 40.0	psf Lbr DF: 1.00	TC Dead 10.0	psf Plt DF: 1.00	BC Live 0.0	psf O.C.: 2- 0- 0	BC Dead 5.0	psf TFI-02/FBC-04		Code: FLA	TOTAL 55.0	psf v4.7.32-48969
Eng Job: SUNBURY	WO: SUNF																		
Dwg:	TI: FC																		
Dsgnr: TLY	Chk: 4/4/2006																		
TC Live 40.0	psf Lbr DF: 1.00																		
TC Dead 10.0	psf Plt DF: 1.00																		
BC Live 0.0	psf O.C.: 2- 0- 0																		
BC Dead 5.0	psf TFI-02/FBC-04																		
	Code: FLA																		
TOTAL 55.0	psf v4.7.32-48969																		

LEGAL DESCRIPTION:
LOT THREE (3) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



CERTIFIED TO:

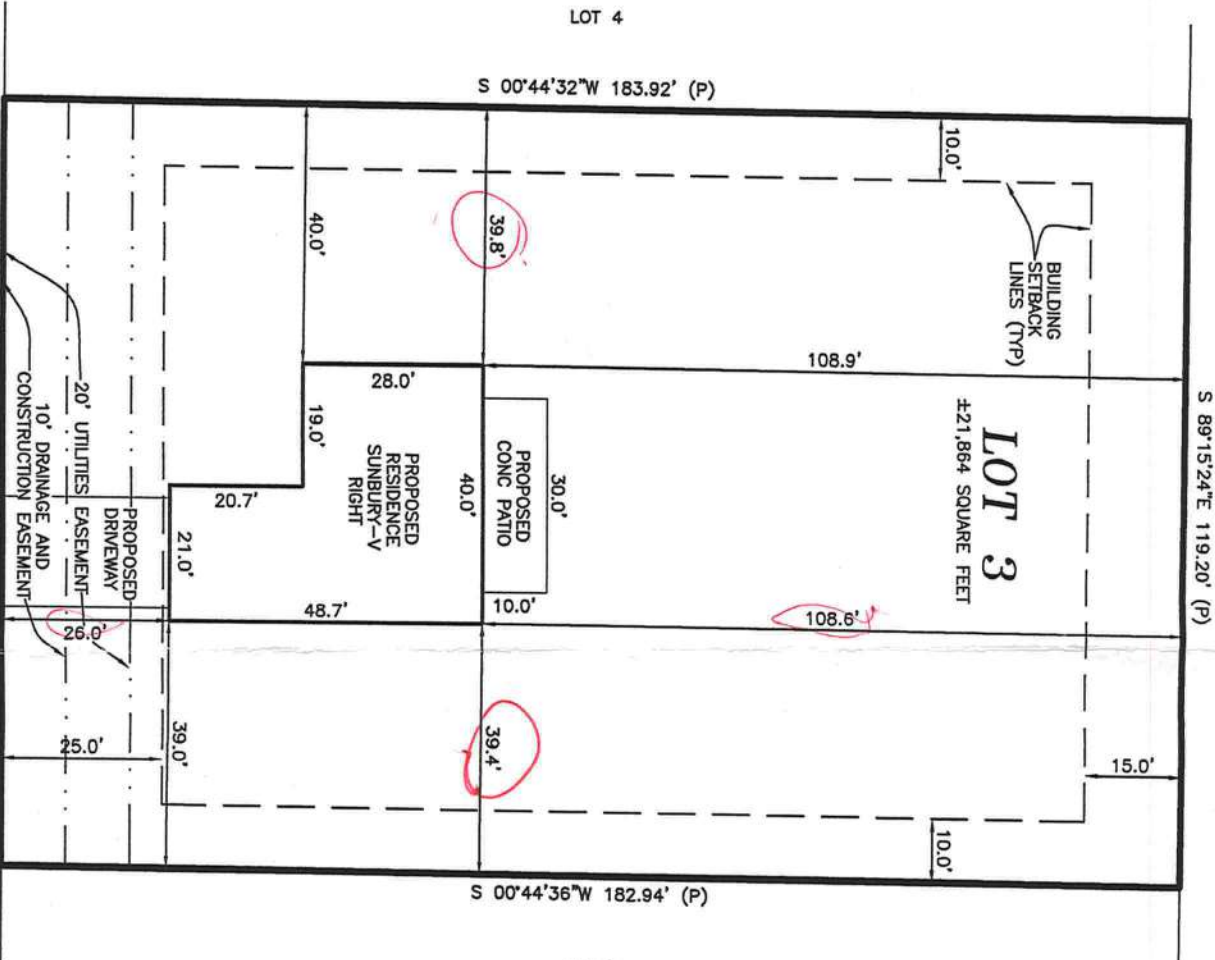
- 1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) NO BUILDING SETBACK INFORMATION HAS BEEN PROVIDED TO THIS SURVEYOR. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM S&M MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- ⊙ = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- ⊖ = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION
- = SET 4" X 4" CONC. MON. P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M. 5582
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
- ⊞ = CATV RISER
- ⊡ = TELEPHONE PEDESTAL
- ⦿ = WOOD POWER POLE

ABBREVIATIONS:

- A = ARC LENGTH
- A/C = AIR CONDITIONER
- ALUM = ALUMINUM
- ASPH = ASPHALT
- AVE = AVENUE
- BLK = BLOCK
- BLVD = BOULEVARD
- BM = BENCHMARK
- C = CALCULATED FROM MEASURED
- CATV = CABLE TELEVISION
- C/B = CONCRETE BLOCK
- CL = CENTER LINE
- CLF = CHAIN LINK FENCE
- CM = CONCRETE MONUMENT
- CMP = CORRUGATED METAL PIPE
- CONC = CONCRETE
- DOT = DEPARTMENT OF TRANSPORTATION
- ELEC = ELECTRIC
- ELEV = ELEVATION
- FND = FOUND
- FNC = FENCE
- (I) = PER LEGAL DESCRIPTION
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MH = MANHOLE
- P = PLAT
- PB = PLAT BOOK
- P.U.E. = PUBLIC UTILITIES EASEMENT
- R/W = RIGHT OF WAY

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE.

BY: *James E. Brinkman* JAMES E. BRINKMAN, PSM - FLA. CERT# 5562

DATE: 10/17/07



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 8th STREET SUITE C, GAINESVILLE, FL. 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

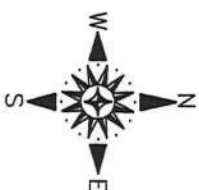
DATE: 10/15/07 "THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL
CHECKED BY: J.B.

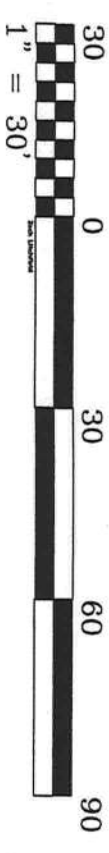
THIS IS NOT A BOUNDARY SURVEY

PREPARED FOR: MARONDA HOMES
DRAWING NUMBER 299-07

LOT THREE (3) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK 'D', PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA



1) MARONDA HOMES

BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT
25', REAR 15', SIDE 10'

1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY

THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

4) NO BUILDING SETBACK INFORMATION HAS BEEN PROVIDED TO THIS SURETOR. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).

6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88,

60' RIGHT-OF-WAY

THIS SUREY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SUREYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

- = FOUND 1/2" REBAR NO IDENTIFICATION
- ⊙ = FOUND 1/2" REBAR & CAP
L.B. 6894
- = SET 1/2" REBAR & CAP
L.B. 6894
- ⊘ = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON.
NO IDENTIFICATION
- = SET 4" X 4" CONC. MON.
P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M. 5582
- ✕ = FOUND NAIL & DISK
- ⊗ = FOUND 6" X 6" S.R.D.
R/W MON.
- ⊖ = CATV RISER
- ⊞ = TELEPHONE PEDESTAL
- ⊟ = WOOD POWER POLE

A = ARC LENGTH
A/C = AIR CONDITIONER
ALUM = ALUMINUM
ASPH = ASPHALT
AVE = AVENUE
BLK = BLOCK
BLVD = BOULEVARD
BM = BENCHMARK
C = CALCULATED FROM MEASURED
CATV = CABLE TELEVISION
C/B = CONCRETE BLOCK
CL = CENTER LINE
CLF = CHAIN LINK FENCE
CM = CONCRETE MONUMENT
CMP = CORRUGATED METAL PIPE
CONC = CONCRETE
DOT = DEPARTMENT OF TRANSPORTATION
ELEC = ELECTRIC
ELEV = ELEVATION
FND = FOUND
FNC = FENCE
(L) = PER LEGAL DESCRIPTION
LB = LICENSED SURVEYOR BUSINESS
(M) = FIELD MEASURED
MH = MANHOLE
P = PLAT
PB = PLAT BOOK
P.U.E. = PUBLIC UTILITIES EASEMENT
R/W = RIGHT OF WAY

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HERON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 61G12-6, FLORIDA ADMINISTRATIVE CODE.

JAMES E. BRINKMAN, PSM - FLA. CERT# 5582

DATE: 6/1/00



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL. 32609

PHONE: (352) 374-7707 FAX: (352) 374-8757

FAX: (352) 374-8757

SCALE: 1" = 30'

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL

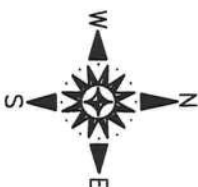
DATE: 10/15/07

THIS IS NOT A BOUNDARY SURVEY

PREPARED FOR: MARONDA HOMES

DRAWING NUMBER

LEGAL DESCRIPTION:
LOT THREE (3) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

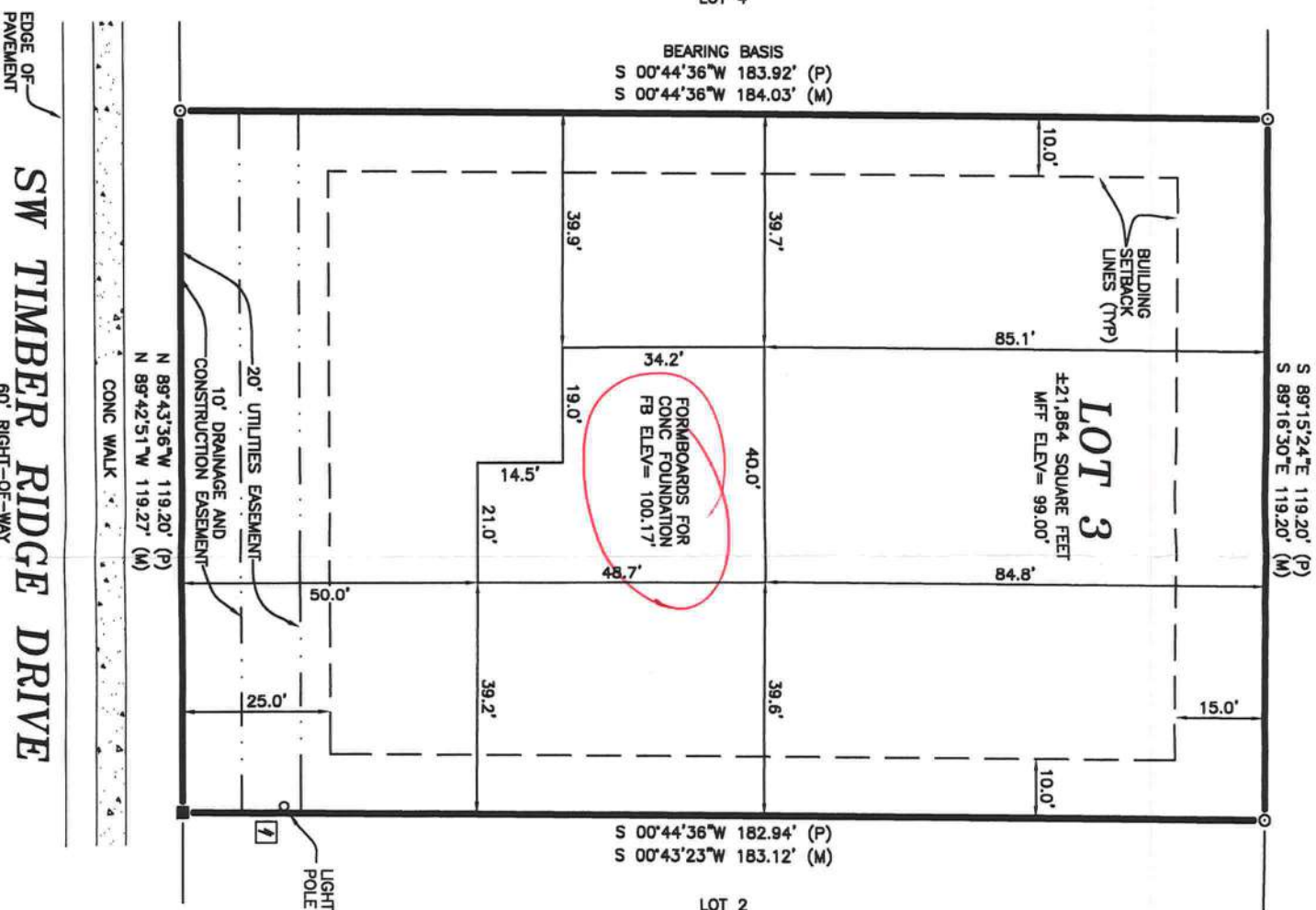


CERTIFIED TO:
1) MARONDA HOMES

BUILDING SETBACK NOTE:
BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

BEARING NOTE:
BEARINGS SHOWN HEREON ARE REFERRED TO AN ASSUMED VALUE OF N. 00°44'36" W., FOR THE WEST PROPERTY LINE OF LOT 3.

- SURVEYOR NOTES:
- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
 - 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
 - 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
 - 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
 - 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
 - 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



FLOOD NOTE:
IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0173-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:
THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE OF PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.



- LEGEND:
- = FOUND 1/2" REBAR & CAP
 - LB. 6894
 - = FOUND 4" X 4" CONC. MON.
 - = NO IDENTIFICATION
 - [7] = ELEC TRANSFORMER

- ABBREVIATIONS:
- A = ARC LENGTH
 - A/C = AIR CONDITIONER
 - ALUM = ALUMINUM
 - ASPH = ASPHALT
 - AVE = AVENUE
 - BLK = BLOCK
 - BLVD = BOULEVARD
 - BM = BENCHMARK
 - C = CALCULATED FROM MEASURED
 - CATV = CABLE TELEVISION
 - C/B = CONCRETE BLOCK
 - CL = CENTER LINE
 - CLF = CHAIN LINK FENCE
 - CM = CONCRETE MONUMENT
 - CMP = CORRUGATED METAL PIPE
 - CONC = CONCRETE
 - DOT = DEPARTMENT OF TRANSPORTATION
 - ELEC = ELECTRIC
 - ELEV = ELEVATION
 - FB = FORMBOARD
 - FND = FOUND
 - FNC = FENCE
 - (L) = PER LEGAL DESCRIPTION
 - LB = LICENSED SURVEYOR BUSINESS
 - (M) = FIELD MEASURED
 - MH = MANHOLE
 - MFT = MINIMUM FINISHED FLOOR
 - P = PLAT
 - PB = PLAT BOOK
 - P.U.E. = PUBLIC UTILITIES EASEMENT
 - R/W = RIGHT OF WAY

CERTIFICATE OF SURVEYOR:
NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 6197-6, FLORIDA ADMINISTRATIVE CODE.

BY: *James E. Brinkman*
JAMES E. BRINKMAN, PSN - FLA. CERT# 5582
DATE: 4/19/08



BRINKMAN SURVEYING & MAPPING INC.
4607 NW 6th STREET SUITE C, GAINESVILLE, FL. 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'	THE BENCHMARK IN QUALITY SERVICE"	DRAWN BY: ZL
DATE: 4/16/08		CHECKED BY: J.B.
FIELDWORK COMPLETED: 4/15/08	FIELDBOOK 87	PAGE 36
PREPARED FOR: MARONDA HOMES		DRAWING NUMBER 299-07

**HOMETEAM**

PEST DEFENSE®

26620

TREATMENT WORKORDER

- ☐ Termite Baiting System w/Tubes-under-the slab
☐ Treat Only ☒ Tubes-under-the slab and Treat ☐ Bora-Care

DATE CALLED IN:	4-21	DATE OF SCHEDULE:	4-23
TIME CALLED IN:		TIME SCHEDULE:	10:00

JOB NAME:		SUBDIVISION:	
MARONDA		Timberland	
JOB ADDRESS:			
165 Timber Ridge Dr			
BILLING NAME:		BILLING PHONE:	
BILLING ADDRESS:			
CALLER BY:	PHONE:	PERMIT NUMBER:	
Patrick	352-206-5459	000026620	

LOT & MODEL NUMBER: Lot 3

DATE & TIME COMPLETED: 4-23-08

SQUARE FOOT: 1848 LINEAR FOOT: BLOCKVOIDS:

SLAB TYPE: MONO TYPE OF FILL: Sand

APPROX. DEPTH OF FOOTING: Outside: Inside:

☐ Addition ☐ Spot Treat ☐ Pool Addition ☐ Driveway☐ Final/Completion ☐ Other

PESTICIDE USED: Imaxx Pro TOTAL APPLIED: 100gal

PERCENT (%) USED: 1% STICKER POSTED: yes

PRICE PER SQ. FT. =	TOTAL FOR P.T.		
	ADDITIONAL		
	TAX:		
/ /	TOTAL AMOUNT	\$	

X X TECHNICIAN:

I hereby acknowledge the satisfactory completion of the above described work.

GT 23 / TCI

12/05