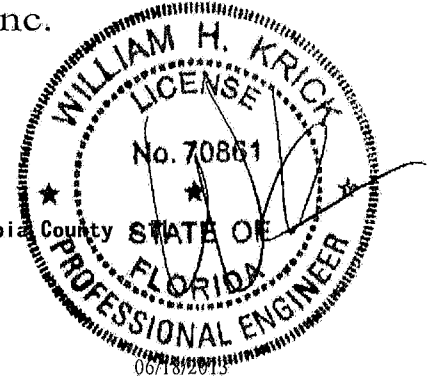


ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1UX6487-Z0218152810



Truss Fabricator **Anderson Truss Company**
Job Identification **13-190--0'Neal Roofing Rick Parker Res. Garage -- Columbia County**
Truss Count **3**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

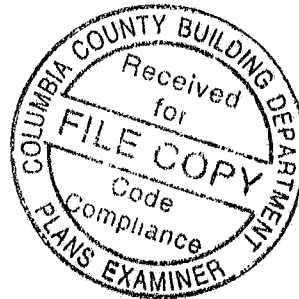
William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref	Description	Drawing#	Date
1	67742--A1	40' Common	13169009	06/18/13
2	67743-A2	25' 1" 8' Common	13169010	06/18/13
3	67744--DGA1	40' Gable	13169011	06/18/13

ALPINE



Top chord 2x4 SP #1_12A
Bot chord 2x4 SP #1_12A
Webs 2x4 SP #3_12A

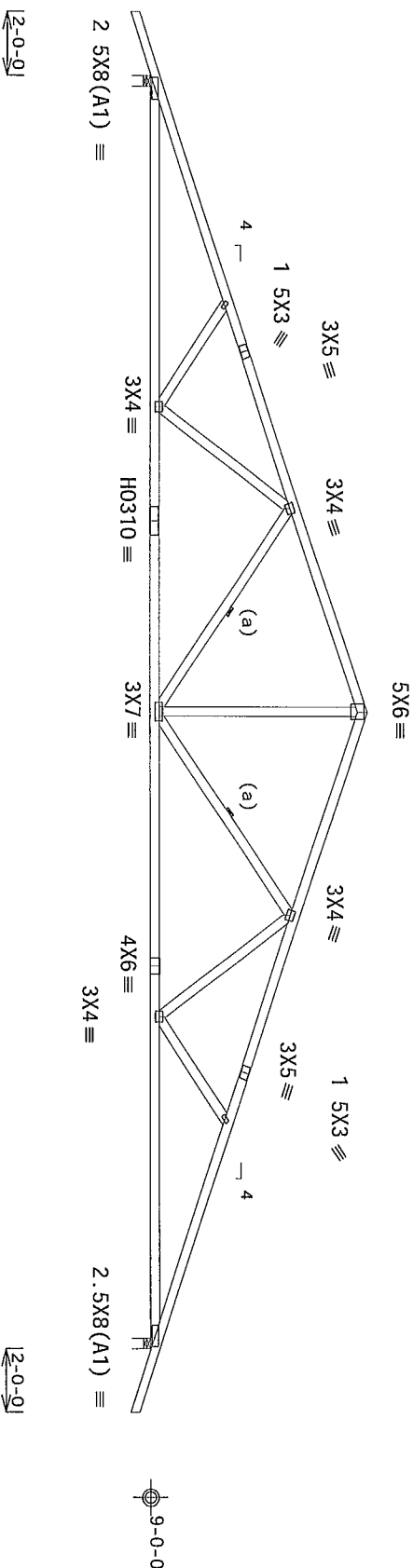
Lumber grades designated with '12A' use design values approved 1/5/2012 by ALSC

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design

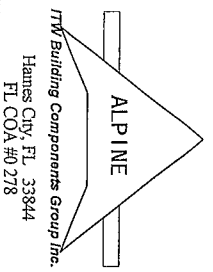
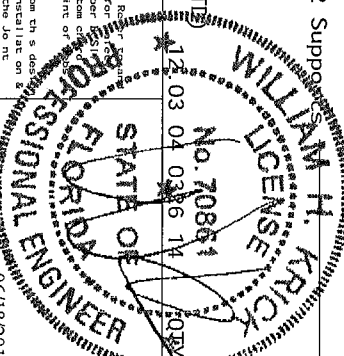
(a) Continuous lateral bracing equally spaced on member
Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



20'-0"-0
40'-0"-0 Over 2 Supports
20'-0"-0
R=1613 U=78 W=4" (4" min)
RL=117/-117
R=1613 U=78 W=4" (4" min)

PLT TYP. 20 Gauge HS. Wave

Design C-11: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)



IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, and installing. Follow the latest edition of BCSI (Building Components Safety Information by TPI and WTC) for details on proper installation. Installers shall provide temporary bracing and bracing prior to performing these functions. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of any truss shall be installed per BCSI sheet one B3 B7 or B10 as applicable.
The Building Components Group Inc. (ITBCCI) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing, or any other use of the truss. The user shall be responsible for the design and use of the truss on the drawing or cover page listing this design. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec 2. For more information see "This Job's BCSI" www.bcsi.org

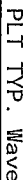
TC LL	20 0 PSF	REF	R487-- 67742
TC DL	7.0 PSF	DATE	06/18/13
BC DL	10 0 PSF	DRW	HCSR487 13169009
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	303948
DUR FAC	1 25		
SPACING	24.0"	JREF	1UX6487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCP1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increases
factor for dead load is 1 50



No. 7086
12.03.04.03P6 14

Scale = .25"/Ft

Haines City, FL 33844
FL COA #0278

The seal is circular with the text "STATE OF FLORIDA" and "PROFESSIONAL ENGINEER" around the perimeter. In the center, it reads "No. 70861" and "12.03.04". The name "WILLIAM H. KRICK" is written in a larger arc across the middle. There is a star at the top and a signature across the bottom.

TC LL	20 0 PSF	REF R487-- 67743
TC DL	7 0 PSF	DATE 06/18/13
BC DL	10 0 PSF	DRW HCUR487 13169010
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT LD.	37.0 PSF	SEQN- 303966
DUR FAC	1 25	
SPACING	24.0"	JREF- 1UX6487_Z02

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

This design is based on lumber values in effect prior to June 1, 2013 and shall only be used on projects designed and permitted prior to this date unless specifically approved in writing by the building authority having jurisdiction, the building designer and the project owner.

Stacked top chord must NOT be notched or cut in area (NLI). Dropped top chord braced at 24" o c intervals. Attach stacked top chord (SC) to dropped top chord in noticable area using 3x4 tie-plates 24" o c. Center plate on stacked/dropped chord interface. Plate length perpendicular to chord length. Splice top chord in noticable area using 3x6.

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCP(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

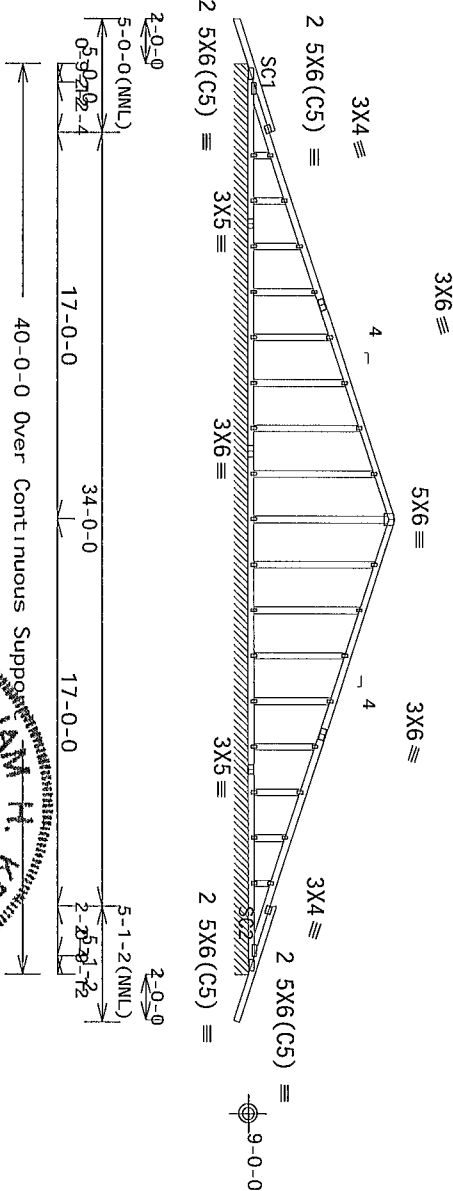
Truss spaced at 24 0 0C designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10 00 PSF Top chord must not be cut or notched

See DWGS A12015ENC100212, GBLLET1N0212, & GABRS1T00212 for more requirements

In lieu of structural panels use purlins to brace TC @ 24' 0C

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



Note	All Plates Are 1 5X3 Except As Shown
PLT TYP.	Wave Design

BC2010Res/TP1-2007(STB)
FT/RT=10%(0%)/0(0)

12 03.04.0326 14

QTY 2 FL/-/3/-/-/R/-

Scale = .125"/Ft

ALPINE

ITW Building Components Group Inc.

Haines City, FL 33844
FL COA #0278

[illegible]

06/18/2013

TC LL	20.0 PSF	REF	R487-- 67744
TC DL	7 0 PSF	DATE	06/18/13
BC DL	10 0 PSF	DRW	HOURS487 13168011
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT LD.	37.0 PSF	SEQN-	303953
DUR.FAC	1.25		
SPACING	24.0"	JREF-	1UX6487_Z02

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED
CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T BRACING OR SCAB
BRACING

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE FOR MINIMUM ALTERNATIVE BRACING RE RUN DESIGN WITH APPROPRIATE BRACING

WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE T OR L-BRACE	BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1 2X4
2X3 OR 2X4	2 ROWS	2X6	2 2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2 2X4(*)
2X8	1 ROW	2X6	1 2X8
2X8	2 ROW*	2X6	2 2X6(*)

T BRACE L BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN

1) CENTER SCAB ON WIDE FACE OF WEB APPLY (1) \$ AB TO EACH
FACE OF WEB



Building Components Group Inc.

Building Components Group Inc.

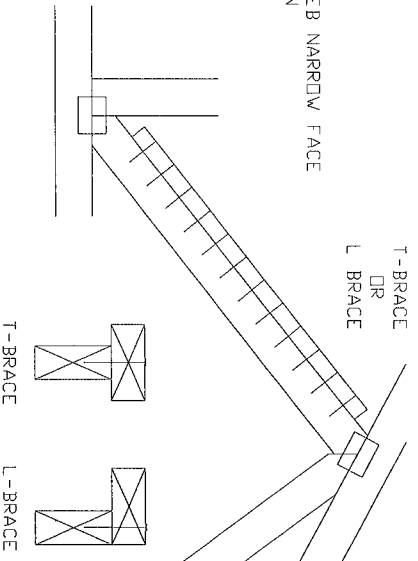
Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON SHEET
 RESUME extr m core n decreasing handling shipping mite and bending
 follow BCS1 bending Component Safety Instruction by TPI and VIDA for safety practices for to
 to enhance top chord steel have property at steel structure panels and control Shm
 properly attaching rigid c/mg. Location shown for permanent lateral restraint of webs slp
 bending initiated per BCS1 sections 33 & 37 See this page's general notes page for more info
 INSTRUMENTS FINISH COPY DR #1 DESIGN 10 INSTALLATION CONTRAC DR
 ***BUILDING COMPONENTS GROUP INC (VIBCO) shall not be responsible for any deviation from this
 design or any failure to build the truss in conformance with TPI or fabricating handling, shipping,
 gross 37/40/60 (60/60/35) gals. plates to each face of webs positioned as shown below
 on or Joint BCS1
 A steel on this drawing or cover page indicates acceptance and professional engineering responsi
 with the responsible for the Building Designer per ANSI/TPI 1 Sec 2
 building was the company VIDA www.vidacorp.com VIDA www.vidacorp.com, ITC www.itccorp.org
 building was the company TPI www.tpiinc.com VIDA www.vidacorp.com, ITC www.itccorp.org

T-BRACING
OR
L-BRACING

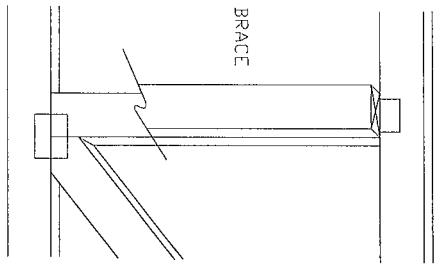
APPLY TO EITHER SIDE OF WEB NARROW FACE
ATTACH WITH 10d BOX OR GUN
10128 x 3 MINd NAILS

BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH



SCAB BRACING

APPLY(SCABS) TO WIDE FACE OF WEB
NOT MORE THAN (1) SCAB PER FACE
ATTACH WITH 10d BOX DR GUN
(0.128 x 3 MIN) NAILS
AT 6 OC
BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



SCAB BRACE

No. 70864

06/18/2013

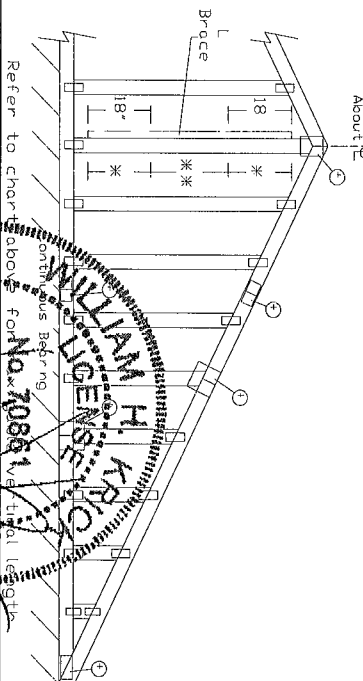
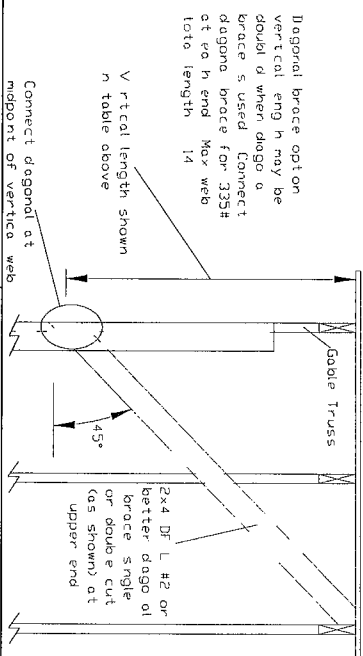
TC LL	PSF	REF	CLB	SUBST
TC DL	PSF	DATE	1/1/09	
BC DL	PSF	DRWG	BRC	BSUB0109
BC LL	PSF			
TOT LD	PSF			
DUR	FAC			
SPACING				

ASCE 7 10 120 mph Wind Speed 15 Mean Height, Enclosed Exposure C, Kzt = 100

100 mph Wind Speed 15 Mean Height, Enclosed Exposure C, Kzt = 100
 100 mph Wind Speed 15 Mean Height, Enclosed Exposure D, Kzt = 100

Max Gable Vertical Length

Gable Vertical Spacing	2x4 Species	Brace Grade	No Braces	(1) 1x4 L Brace				(2) 2x4 L Brace				(3) 2x6 L Brace			
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF	#1 / #2	4 10	8 2	8 6	9 8	10 1*	11 6	12 0	14 0	14 0*	14 0	14 0*	14 0	14 0*
		#3	4 7*	7 9	8 3	9 7	9 11*	11 5	11 10	14 0	14 0	14 0	14 0	14 0	14 0
		Stud	4 7*	8 1	8 4	9 7	9 11*	11 5	11 10	14 0	14 0	14 0	14 0	14 0	14 0
		Standard	4 11	8 3	8 7	9 9	10 1*	11 7	12 1*	14 0	14 0	14 0	14 0	14 0	14 0
16" o.c.	SPF	#1	4 10	8 2*	8 6*	9 8*	10 1*	11 6*	12 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		#2	4 7	6 11*	7 4*	8 0*	9 10*	11 5*	11 10*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		#3	4 7	6 11*	7 4*	8 0*	9 10*	11 5*	11 10*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		Standard	4 7	6 11	7 4	8 0*	9 10*	11 5*	11 10*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
12" o.c.	SPF	#1 / #2	5 6	9 5	9 9*	11 1*	11 6*	13 2*	13 7*	14 0	14 0*	14 0*	14 0*	14 0*	14 0*
		#3	5 3*	9 3	9 9*	10 11*	11 4*	13 0*	13 7*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		Stud	5 3*	9 3	9 9*	10 11*	11 4*	13 0*	13 7*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		Standard	5 3	9 3*	9 7*	10 11	11 4	13 0*	13 7*	14 0	14 0*	14 0*	14 0*	14 0*	14 0*
16" o.c.	SPF	#1	5 8*	9 5	9 10	11 2*	11 7	13 3*	13 10*	14 0	14 0*	14 0*	14 0*	14 0*	14 0*
		#2	5 6	9 5	9 9*	11 1*	11 6	13 2*	13 9*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		#3	5 3	8 6*	9 0*	10 11*	11 4	13 0*	13 7*	14 0	14 0*	14 0*	14 0*	14 0*	14 0*
		Stud	5 3	8 6	9 0*	10 11*	11 4	13 0*	13 7*	14 0	14 0*	14 0*	14 0*	14 0*	14 0*
12" o.c.	SPF	Standard	5 3	7 4	7 10	9 9	10 5*	12 8*	13 2	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		#1 / #2	6 1	10 4	10 8	12 2	12 8*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		#3	5 9*	10 2	10 7	12 0	12 6	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		Stud	5 9	10 2	10 7	12 0	12 6	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
16" o.c.	SPF	Standard	5 9	10 2	10 7	12 0*	12 6	14 0	14 0	14 0	14 0*	14 0	14 0	14 0	14 0
		#1	6 2	10 5	10 9	12 3*	12 9*	14 0	14 0*	14 0	14 0	14 0	14 0	14 0	14 0
		#2	6 1	10 4	10 8	12 2	12 8	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0
		#3	5 9	9 9	10 5	12 0	12 6	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0
12" o.c.	SPF	Stud	5 9	9 9	10 5	12 0*	12 6	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0
		Standard	5 9*	9 9*	10 5	11 3*	12 1	14 0	14 0	14 0	14 0	14 0	14 0	14 0	14 0
		#1	6 1	10 4	10 8	12 2	12 8*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*
		#2	6 1	10 4	10 8	12 2	12 8*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*	14 0*



Bracing Group Species and Grades			
Group A		Group B	
Species	Grade	Species	Grade
SPF	#1 / #2	SPF	#1 / #2
Stud	#3	Stud	#3
Standard	Standard	Standard	Standard



Building Components Group Inc.
 114 Building Components Group Inc. shall not be responsible for any deviation from this drawing and failure to build the truss in conformance with ANSI/APA 100 for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility by the design shown. The suitability and use of this drawing for any other purpose is the responsibility of the user. For more information see the Building Components Group Inc. website at www.bcginc.com
 114 Building Components Group Inc. shall not be responsible for any deviation from this drawing and failure to build the truss in conformance with ANSI/APA 100 for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility by the design shown. The suitability and use of this drawing for any other purpose is the responsibility of the user. For more information see the Building Components Group Inc. website at www.bcginc.com

Professional Engineer
 STATE OF FLORIDA
 JUN 18 '13
 06/18/2013

MAX TDT LD 60 PSF
 MAX SPACING 24 0

REF ASCE7 10-GAB12015
 DATE 2/14/12
 DRWG A12015ENC100212

Earth City MO 63045

ASCE 7-10 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

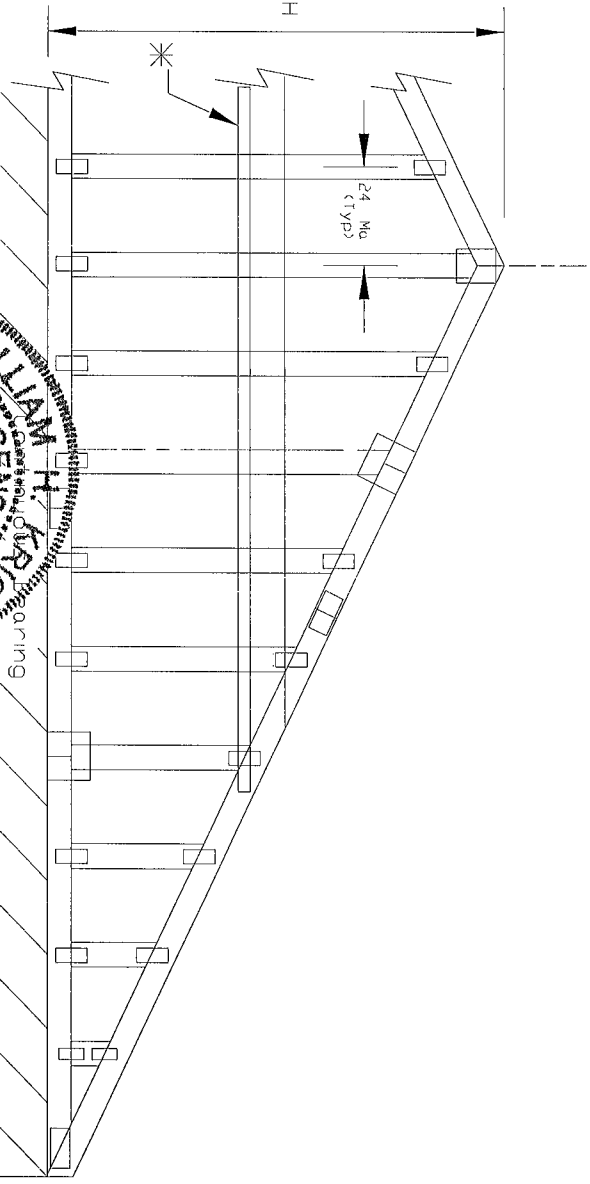
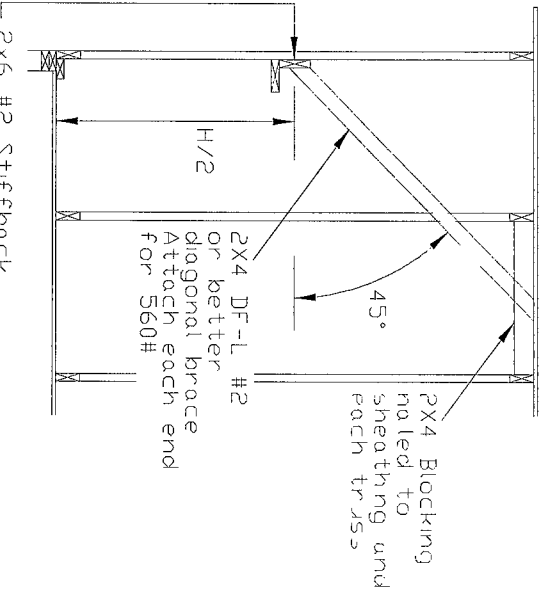
120 mph Mean Hgt ASCE 7-10 Enclosed Exp C or
100 mph 30ft Mean Hgt ASCE 7-10 Enclosed Exp D or
100 mph 30ft Mean Hgt ASCE 7-10 Part Enclosed Exp C
Kzt = 1.00 Wind TC DL=50 psf, Wind BC DL=50 psf

Lateral chord bracing requirements
Top Continuous roof sheathing
Bot Continuous ceiling diaphragm

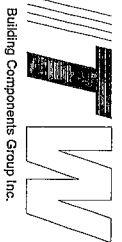
See Engineer's sealed design referencing this detail
for lumber plates and other information not shown
on this detail

Nails 10d box or gun (0.128 x 3 min) nails

2x6 #2 Stiffback
attached to each
Stud w/ (4) 10d box or gun (0.128 x 3 min) nails



H Less than 4.6 - no stud bracing required
H Greater than 4.6 to 7.6 in length
provide a 2x6 stiffback at mid-height and brace stiffback
to roof diaphragm every 6.0 (see detail below or
refer to DRWG A12030ENC10)
H Greater than 7.6 to 12.0 max
provide a 2x6 stiffback at mid height and brace
to roof diaphragm every 4.0 (see detail below or
refer to DRWG A12030ENC10)
* Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128 x 3 min) nails @ 6 oc



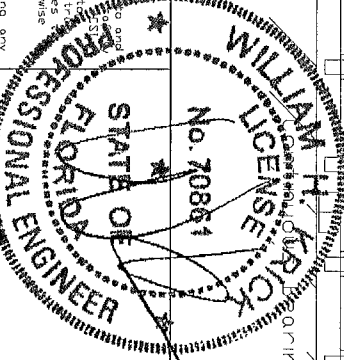
Building Components Group Inc.

Earth City MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

These drawings are in final stage. No changes or modifications should be made without the written approval of the designer. The designer shall be responsible for the accuracy of the information provided. The user of these drawings shall be responsible for the accuracy of the information provided. The user of these drawings shall be responsible for the accuracy of the information provided.

Building Components Group Inc. shall not be responsible for any deviation from this drawing. The user of these drawings shall be responsible for the accuracy of the information provided. The user of these drawings shall be responsible for the accuracy of the information provided.



Jun 18 '13

06/18/2013

MAX TOT LD 60 PSF
MAX SPACING

REF	GE	WHALE
DATE	2/14/12	
DRWG	GABRST100212	

Rick Parker Garage

