Schafer Engineering, LLC

14705 Main St. Alachua FL 32615

E

Prepared for:

ONEAL CONSTRUCTION
THE NELSON RESIDENCE
COLUMBIA COUNTY, FLORIDA

By:

Schafer Engineering, LLC

386-462-1340 / 352-375-6329

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SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340 \ 352-375-6329

Trusses: Pre-engineered, pre-fabracated with the manufacturers required bracing system installed.

Roof Sheathing: Type: CDX plywood Size: 1/2 Fastener type nails: 8d / .113 Ring Shank
Interior zone spacing: Interior: 6" Periphery: 4
Edge and end zone spacing: Interior: 6" Periphery: 4

Double Top Plate: Type: Spruce Grade: #1 #2 Size: 2 x 6 Nail Spacing: 8 in

Stud Type: Spruce Grade: #1 #2 Size: 2 x 6
Interior stud spacing: 16" End stud spacing: 16"

Shear Wall Siding: Type: OSB Thickness: 7/16
73 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4
53 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4

Allowable Unit Shear on Shear Walls: 314 pounds per linear foot
Unit Shear Transferred from Diaphragm: Trans: 84 Long: 195

Wall Tension Transferred by: Siding Nails: 8d/131 @ 4 O.C. Edges

Foundation Anchor Bolts: Concrete Strength: 3000 psi Size: 1/2"
Washer: 2" Embedment: 7" Location of first anchor bolt from corner: 8"

Anchor Bolts @ 48" o.c. Model: A307 Loc. from corner: 8"
Type of Foundation: (1) - #5 rebar continuous required in bond beam.
Floor Slab: 4" Cmu size: 8" x 16" Height: 24" Rein.: #5 at 72" o.c.

Monolithic Footing: Depth: 20" Bottom Width: 12 Rein.: 2 #5 rebars

Stemwall Footing: Width: 20 Depth: 10 Rein.: 2 #5 rebar
Interior Footings 16" Wide X 10" Deep with 2-#5 rebar continuous

Porch Columns: 6X6X10 SYP PTD @ 84" o.c. MAX Column Fasteners: Simpson CBSQ66/CC66ORE equiv

Special Comments: Install ceiling diaphragm on screen porch ceiling using same nail pattern, nail size, & same grade material as roof sheathing.
Install 2 ply 2X12 SYP HR w/ 7/16" OSB filler for all window & door headers.
Install 2 ply 2X12 SYP HR with 7/16" OSB filler for screen porch headers.
Garage door header to be sized by others.

Notes:
1. Balloon frame all gable ends unless accompanied by gable end detail
2. All trusses must bear on exterior walls and porch beams.
3. All wall and roof sheathing to be nailed with same nail pattern as the shear walls.
4. This wind load is not valid without a raised, embossed seal.
5. It is assumed that ideal soil conditions and pad preparations are provided.
6. Fiber mesh or WWM may be used in concrete slab.
7. Trusses must be installed and anchored in accordance to the truss engineering.
8. All headers spanning over 12' must be pre-engineered.
9. The foundatioin and walls are minimum design use, and may be increased.
10. Wind load is for one use only \ FBC-2007 \ No copies permitted

6-21-11

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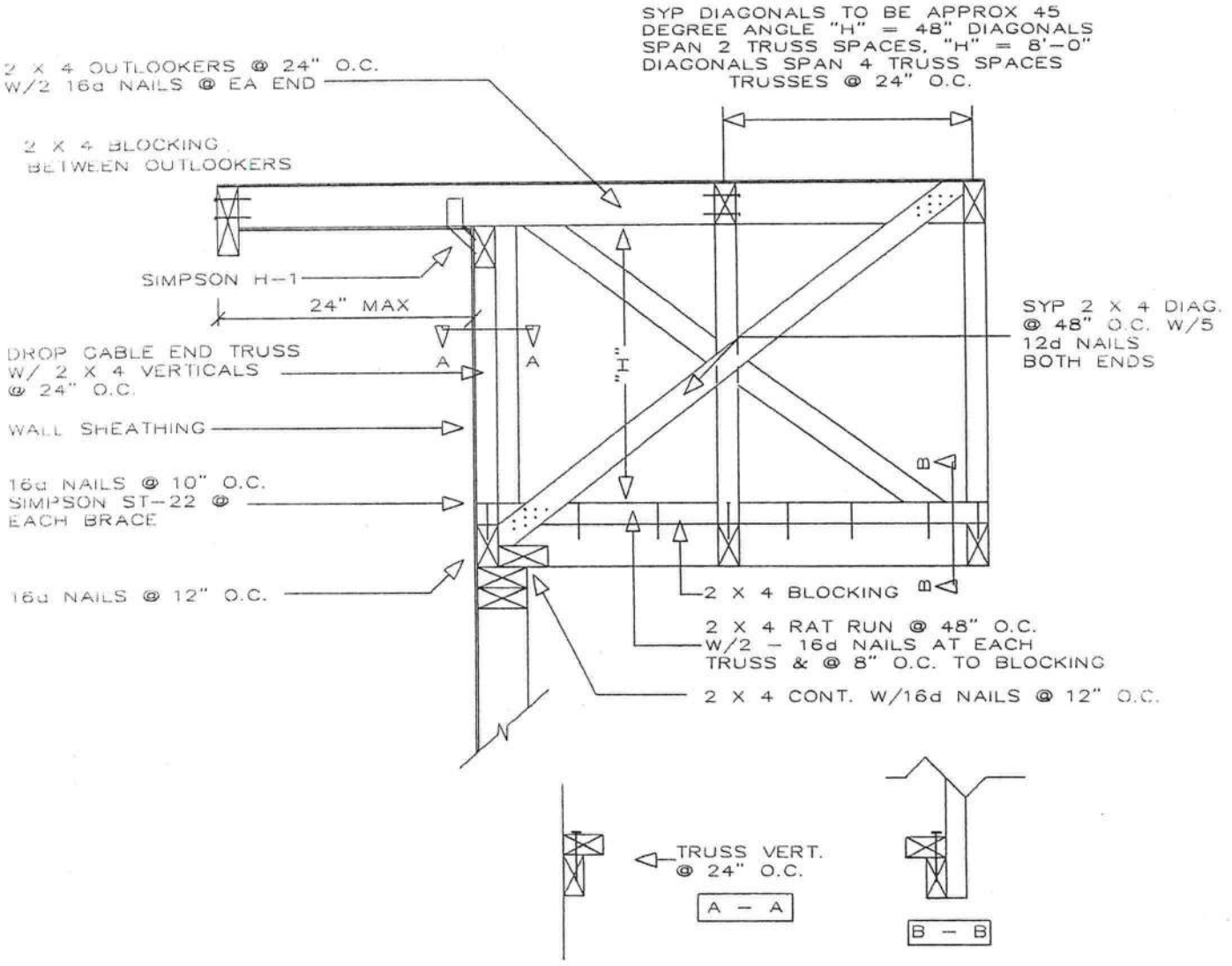

TYPICAL GABLE END BRACING

6-21-11

DETAIL MAY BE USED WITH INTERIOR CATH. CEILING BY INSTALLING A SYP 2 X 4 LEDGER IN PLANE WITH THE INTERIOR CEILING USING 2 - 16d NAILS ON EACH POINT WHERE THE LEDGER CROSSES THE GABLE END TRUSS VERTICALS

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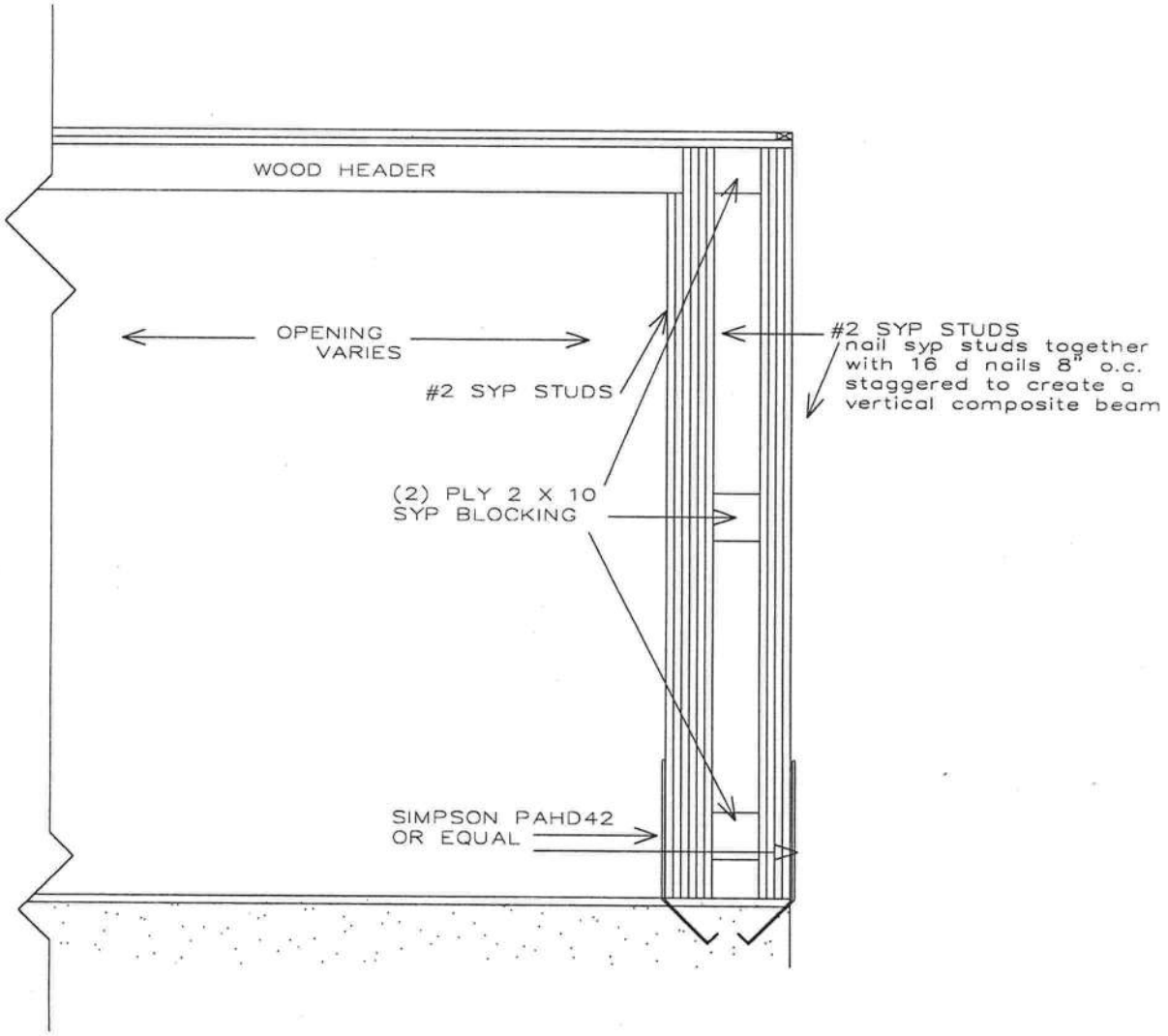

EQUIVALENT 3'-0" SHEAR WALL SEGMENT

6-21-11

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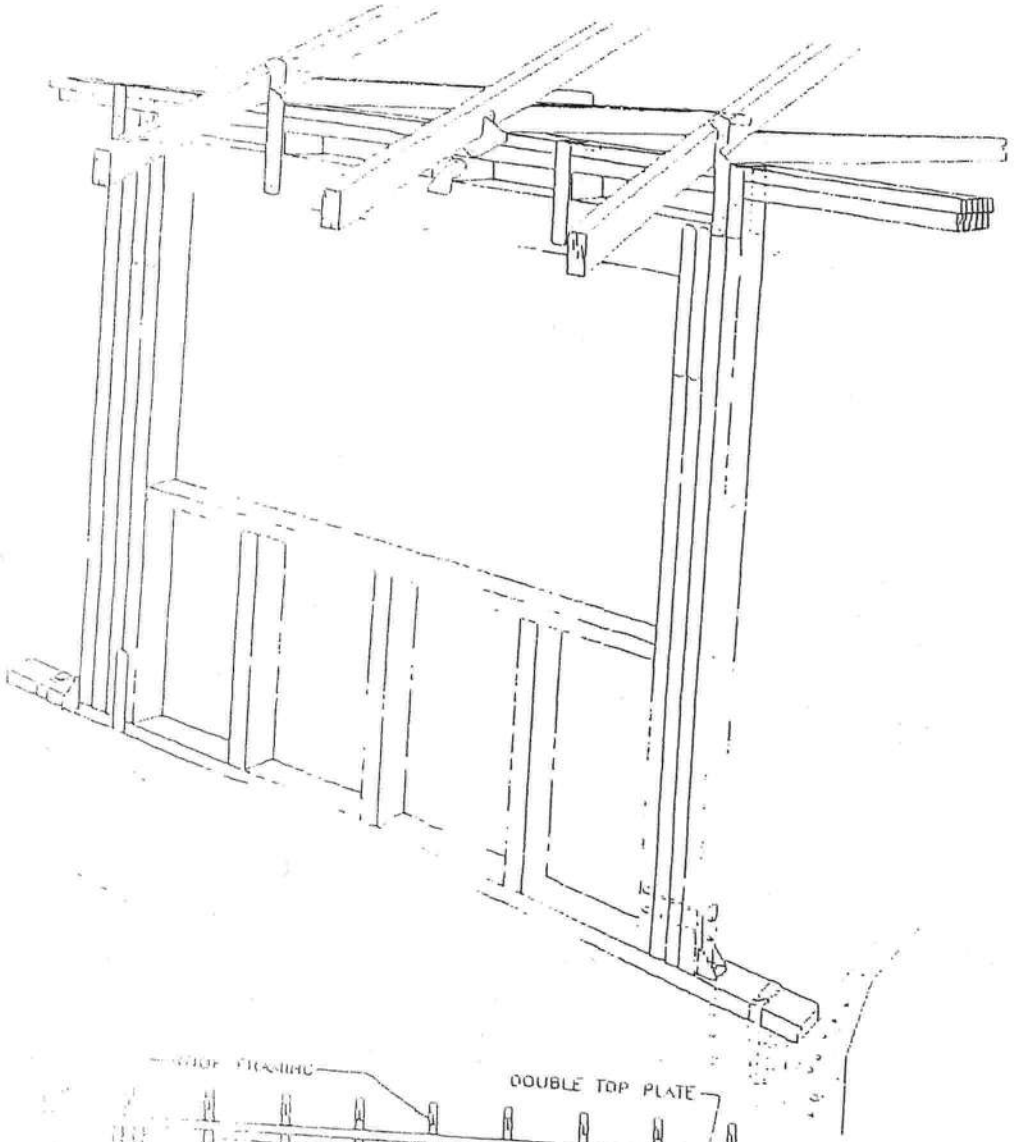

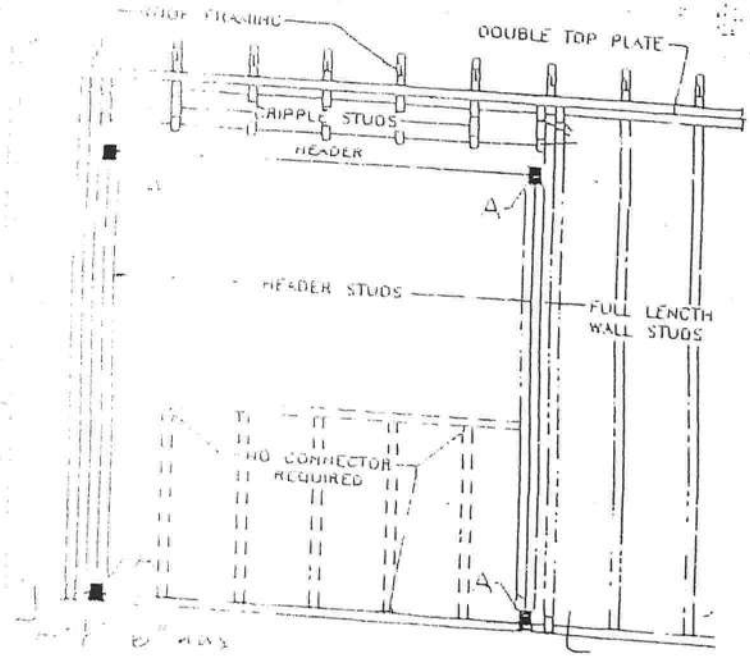

Total each truss uplift on the header divide by 2 for header anchorage

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HEADER STRAPPING				
Uplift Lbs	Top Connector	Rating Lbs	Bottom Connector	Rating Lbs
to 455	LSTA19	635	H3	320
to 910	LSTA12	795	2-H3	640
to 1265	LSTA18	1110	LTT19	1305
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2165
to 2865	3-LSTA18	3255	HD2A-3.5	2865
to 3700	3-LSTA24	3880	HD5A-3	3130
Total the uplift for each truss sitting on the header and divide by 2 to determine the uplift on the header. Use proper bolt anchors sufficient to support required uplift loads.				

TRUSSES \ GIRDERS			
Uplift Lbs	Top Connector	Bottom Connector	Rating Lbs
to 535	H2.5A	NA	
to 1015	H10A	NA	
to 1215	TS22	LTT19	1305
to 1750	2-TS22	LTT20	1750
to 2570	2-TS22	HD2A	2775
to 3665	3-TS22	HD5A	4010
to 5420	2-MST37	HTT22	5250
to 9660	2-MST60	HD10A	9540
Two 12d common toenials are required per truss for each bearing point into top plate. It is the contractors responsibility to provide a continuous load path from truss to foundation.			

	TOP CONNECTOR	RATING LBS	BOTTOM CONNECTOR	RATING LBS
BEAM SEATS	LSTA18	1110	LTT19	1305
POSTS	2-LSTA18	2220	ABU44	2300

1. Simpson or equivient hardware may be used. For nailing into spruce members, multiply table values by .86
2. See truss engineering for anchor uplift values.
3. This schedule is not meant to be a replacement to the specified values of any manufactures values.

ASCE 7-05

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	110	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	26.6	Deg
Type of Roof	Hipped	
Eave Height (Eht)	10.00	ft
Ridge Height (RHt)	24.67	ft
Mean Roof Height (Ht)	18.33	ft
Width Perp. to Wind (B)	72.00	ft
Width Parallel to Wind (L)	54.00	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters	
Type of Structure	
Height/Least Horizontal Dim	0.34
Flexible Structure	No

Calculated Parameters		
Importance Factor	1	
Hurricane Prone Region (V>100 mph)		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
Izm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8806	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8545	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	161.33	ft/s
Vzm	bm*(zm/33)^Am*Vhref	70.89	ft/s
NF1	NatFreq*Lzm/Vzm	4.37	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0552	
Nh	4.6*NatFreq*Ht/Vzm	1.19	
Nb	4.6*NatFreq*B/Vzm	4.67	
Nd	15.4*NatFreq*Depth/Vzm	11.73	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.5201	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.1911	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0816	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.5583	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	0.99	

Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.85	Gust Factor (G)	0.85

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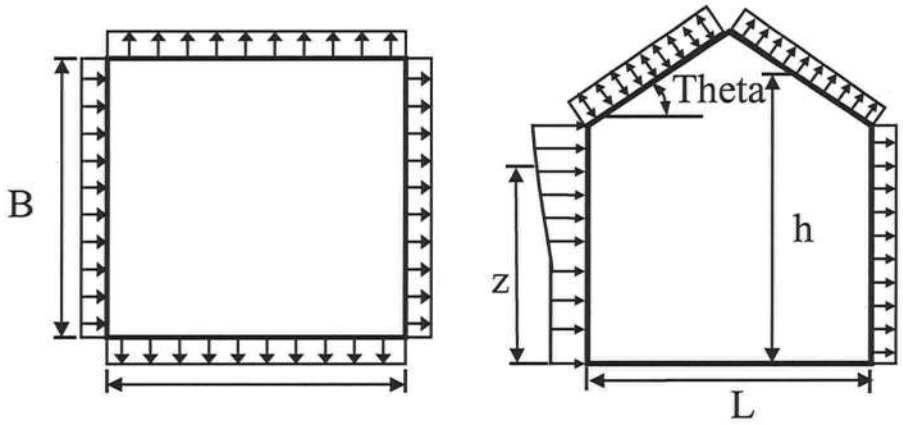
6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2)	
					Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
24.67	0.70	1.00	1.00	21.70	11.44	18.23
20	0.70	1.00	1.00	21.70	11.44	18.23
18.33	0.70	1.00	1.00	21.70	11.44	18.23
15	0.70	1.00	1.00	21.70	11.44	18.23

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	0.61	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	18.85	psf

Wall Pressure Coefficients, Cp	
Surface	**Cp**
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf)	
		+GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 72 ft wall)	-0.50	-11.45	-4.66
Leeward Walls (Wind Dir Parallel to 54 ft wall)	-0.43	-10.37	-3.59
Side Walls	-0.70	-14.67	-7.88
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.22	-7.01	-0.22
Windward - Max Positive	0.26	0.86	7.65
Leeward Normal to Ridge	-0.60	-13.06	-6.27
Overhang Top	-0.22	-3.61	-3.61
Overhang Bottom	0.80	0.68	0.68
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 9.165 ft	-0.90	-17.89	-11.11

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Dist from Windward Edge: 9.165 ft to 18.33 ft	-0.90	-17.89	-11.11
Dist from Windward Edge: 18.33 ft to 36.66 ft	-0.50	-11.45	-4.66
Dist from Windward Edge: > 36.66 ft	-0.30	-8.23	-1.44

* Horizontal distance from windward edge

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.61
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.85

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.55	0.18	-0.18	21.70	8.03	15.84
2	-0.10	0.18	-0.18	21.70	-5.99	1.82
3	-0.45	0.18	-0.18	21.70	-13.61	-5.79
4	-0.39	0.18	-0.18	21.70	-12.38	-4.57
5	0.00	0.18	-0.18	21.70	-3.91	3.91
6	0.00	0.18	-0.18	21.70	-3.91	3.91
1E	0.73	0.18	-0.18	21.70	11.88	19.69
2E	-0.19	0.18	-0.18	21.70	-7.93	-0.12
3E	-0.58	0.18	-0.18	21.70	-16.59	-8.78
4E	-0.53	0.18	-0.18	21.70	-15.50	-7.69
5E	0.00	0.18	-0.18	21.70	-3.91	3.91
6E	0.00	0.18	-0.18	21.70	-3.91	3.91

* p = qh * (GCpf - GCpi)

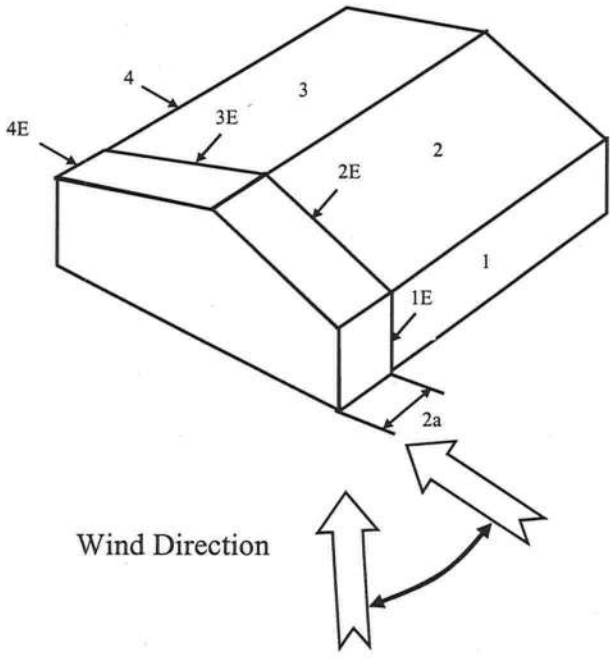

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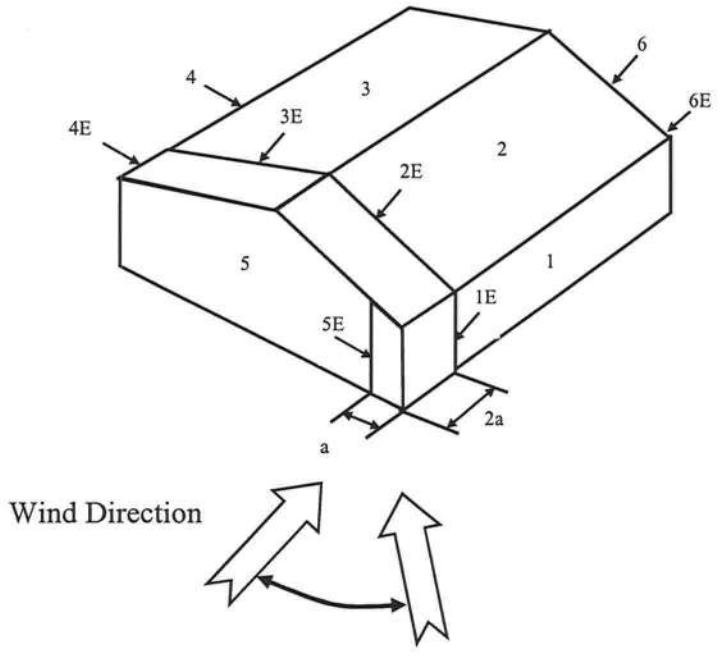
Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.61
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	18.85

Case B						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	-0.45	0.18	-0.18	21.70	-13.67	-5.86
2	-0.69	0.18	-0.18	21.70	-18.88	-11.07
3	-0.37	0.18	-0.18	21.70	-11.94	-4.12
4	-0.45	0.18	-0.18	21.70	-13.67	-5.86
5	0.40	0.18	-0.18	21.70	4.77	12.59
6	-0.29	0.18	-0.18	21.70	-10.20	-2.39
1E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
2E	-1.07	0.18	-0.18	21.70	-27.13	-19.31
3E	-0.53	0.18	-0.18	21.70	-15.41	-7.60
4E	-0.48	0.18	-0.18	21.70	-14.32	-6.51
5E	0.61	0.18	-0.18	21.70	9.33	17.14
6E	-0.43	0.18	-0.18	21.70	-13.24	-5.43

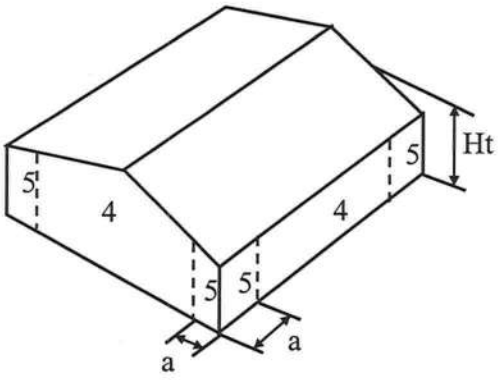
* p = qh * (GCpf - GCpi)

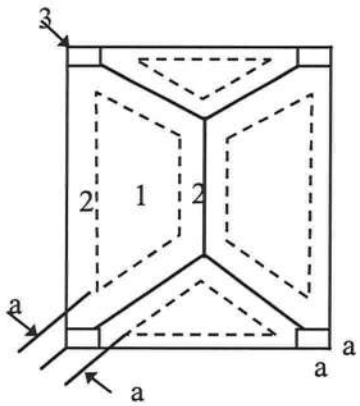


ASCE 7-05

Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft





Hipped Roof

10 < Theta <= 30

a = 5.4 ==> 5.40 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp		Wind Press (lb/ft^2)	
					Max	Min	Max	Min
	16	7	112.00	5	0.81	-1.03	18.75	-22.80
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

ASCE 7-05

Condition	Gcpi	
	Max +	Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**

Table 6-8 External Pressure Coefficients for Arched Roofs, Cp

r (Rise-to-Span Ratio) = 0.3

Condition	Variable	Cp		
		Windward Quarter	Center Half	Leeward Quarter
Roof on Elevated Structure	Cp	0.13	-1	-0.5
	P (+GCpi) - psf	-1.38	-19.50	-11.45
	P (-GCpi) -psf	5.41	-12.72	-4.66
Roof Springing from Ground	Cp	0.42	-1	-0.5
	P (+GCpi) - psf	3.37	-19.50	-11.45
	P (-GCpi) -psf	3.37	-19.50	-11.45

Table 6-9 Force Coefficients for Monoslope Roofs over Open Buildings, Cf

Variable	Description	Value	
L	Roof dimension normal to wind direction	54.00	ft
B	Roof dimension parallel to wind direction	72.00	ft
L/B	Ratio of L to B	0.750	
Theta	Slope of Roof	26.6	Deg
Cf	Force Coefficient	1.17	
X	Distance to center of pressure from windward edge	0.42	ft

Julius Lee

RE: 361993 - O'NEIL CONST. - NELSON RES.

1109 Coastal Bay Blvd.
Boynton Beach, FL 33435

Site Information:

Project Customer: O'NEIL CONST. Project Name: 361993 Model: NELSON RES.
Lot/Block: Subdivision:
Address: 1136 MARYNIK DR.
City: ALACHUA CTY State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: O'NEIL CONST. License #: QB0010656
Address: 110 NE 1ST AVE
City: HIGH SPRINGS, State: FL

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.1
Wind Code: ASCE 7-05 Wind Speed: 110 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 32 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4799420	EJ7	6/24/011	18	I4799437	T08G	6/24/011
2	I4799421	EJ7A	6/24/011	19	I4799438	T09	6/24/011
3	I4799422	PB01	6/24/011	20	I4799439	T09G	6/24/011
4	I4799423	PB02	6/24/011	21	I4799440	T10	6/24/011
5	I4799424	PB02G	6/24/011	22	I4799441	T11	6/24/011
6	I4799425	T01	6/24/011	23	I4799442	T17	6/24/011
7	I4799426	T01G	6/24/011	24	I4799443	T18	6/24/011
8	I4799427	T02	6/24/011	25	I4799444	T19	6/24/011
9	I4799428	T03	6/24/011	26	I4799445	T20	6/24/011
10	I4799429	T04	6/24/011	27	I4799446	T21	6/24/011
11	I4799430	T05	6/24/011	28	I4799447	T22	6/24/011
12	I4799431	T05G	6/24/011	29	I4799448	T23	6/24/011
13	I4799432	T06	6/24/011	30	I4799449	T24	6/24/011
14	I4799433	T06A	6/24/011	31	I4799450	T25	6/24/011
15	I4799434	T07	6/24/011	32	I4799451	T26	6/24/011
16	I4799435	T07G	6/24/011				
17	I4799436	T08	6/24/011				

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Lake City).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2013.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.

June 24,2011

Julius Lee

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799420
361993	EJ7	MONO TRUSS	12	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Jun 24 14:58:08 2011 Page 1

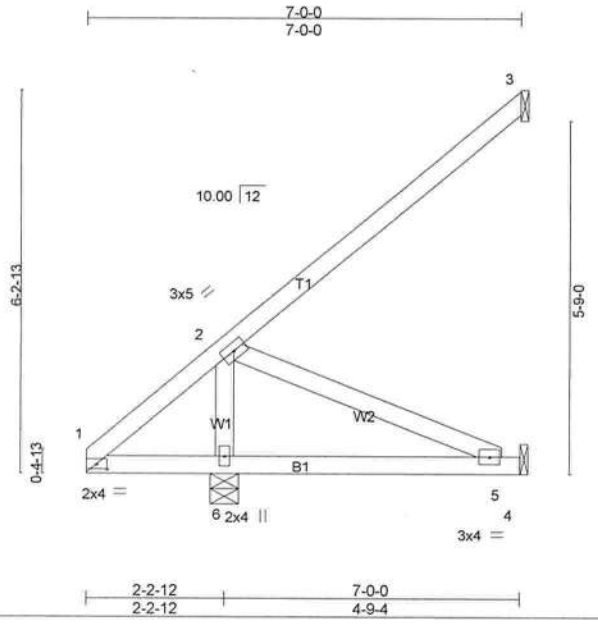


Plate Offsets (X,Y): [1:0-2-1,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.01	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.12	Vert(TL)	-0.03	5-6	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.00	5-6	>999	240	Weight: 34 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

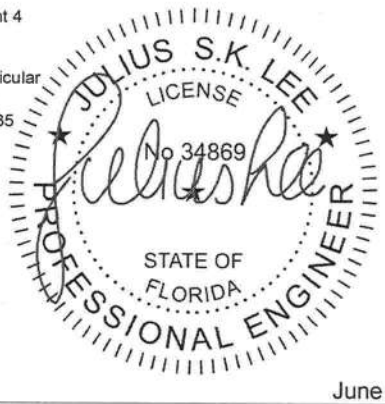
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=103/Mechanical, 4=14/Mechanical, 6=327/0-5-8
Max Horz 6=236(LC 6)
Max Uplift 3=-115(LC 6), 4=-63(LC 6), 6=-73(LC 6)
Max Grav 3=103(LC 1), 4=53(LC 2), 6=327(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-5=-53/271, 2-6=-281/169

NOTES (8-9)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2 .
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 3, 63 lb uplift at joint 4 and 73 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	EJ7A	MONO TRUSS	10	1	I4799421
					Job Reference (optional)

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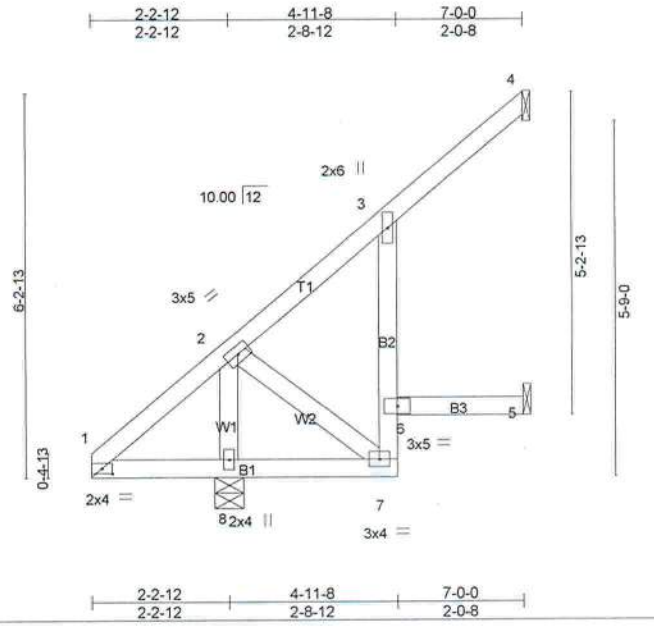


Plate Offsets (X,Y): [1:0-2-1,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.24	Vert(LL)	-0.02	6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.03	6	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.06	4	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.08	6	>696	240	Weight: 37 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
10-0-0 oc bracing: 5-6.

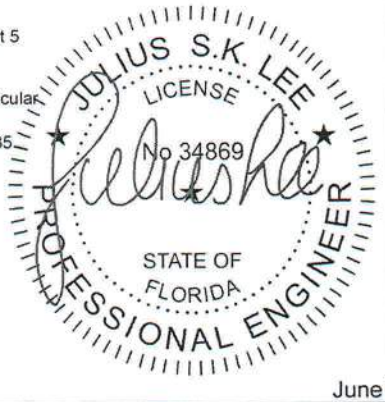
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=77/Mechanical, 5=40/Mechanical, 8=327/0-5-8
Max Horz 8=236(LC 6)
Max Uplift 4=-118(LC 6), 5=-59(LC 6), 8=-73(LC 6)
Max Grav 4=77(LC 1), 5=48(LC 2), 8=327(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-288/138

NOTES (8-9)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2 .
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 4, 59 lb uplift at joint 5 and 73 lb uplift at joint 8.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799422
361993	PB01	PIGGYBACK	10	1	Job Reference (optional)	

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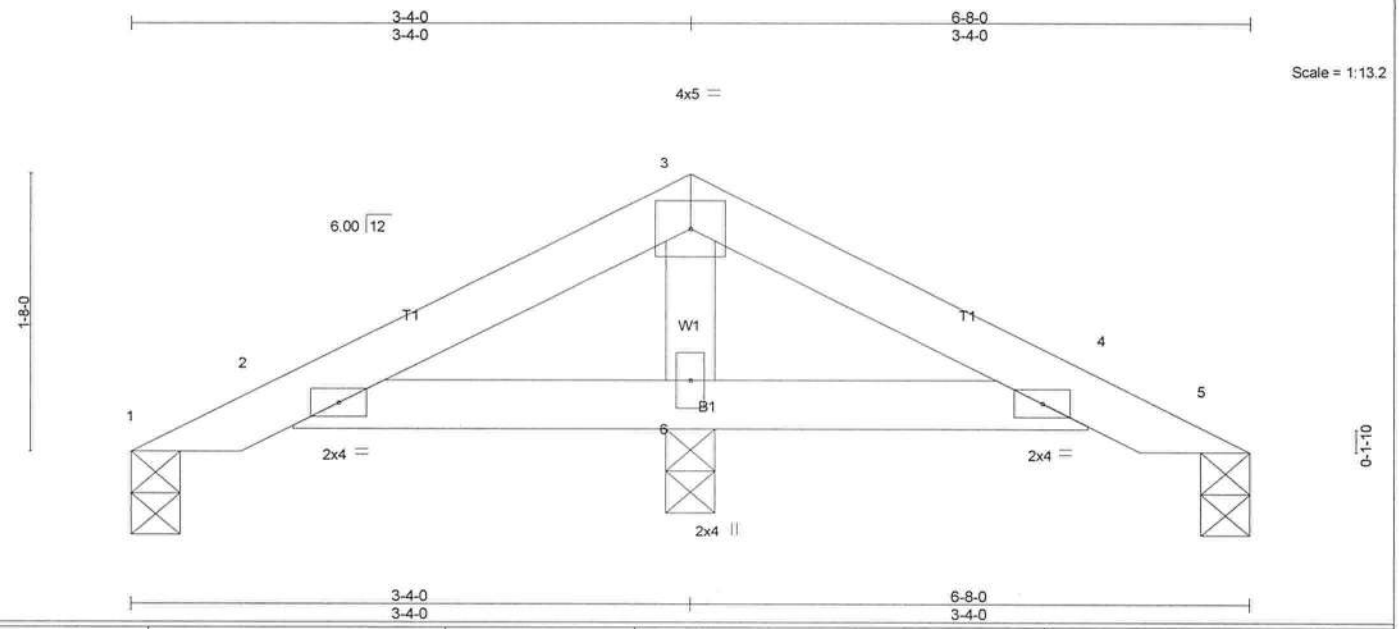

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.08	Vert(LL)	-0.00	4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	-0.01	4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(TL)	-0.00	5	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.00	4	>999	240	Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=36/0-3-8, 5=36/0-3-8, 6=337/0-3-8
Max Horz 1=26(LC 5)
Max Uplift 1=-14(LC 6), 5=-23(LC 7), 6=-107(LC 6)
Max Grav 1=57(LC 10), 5=57(LC 11), 6=337(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-6=-281/357

NOTES (10-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1, 23 lb uplift at joint 5 and 107 lb uplift at joint 6.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799423
361993	PB02	PIGGYBACK	9	1	Job Reference (optional)	

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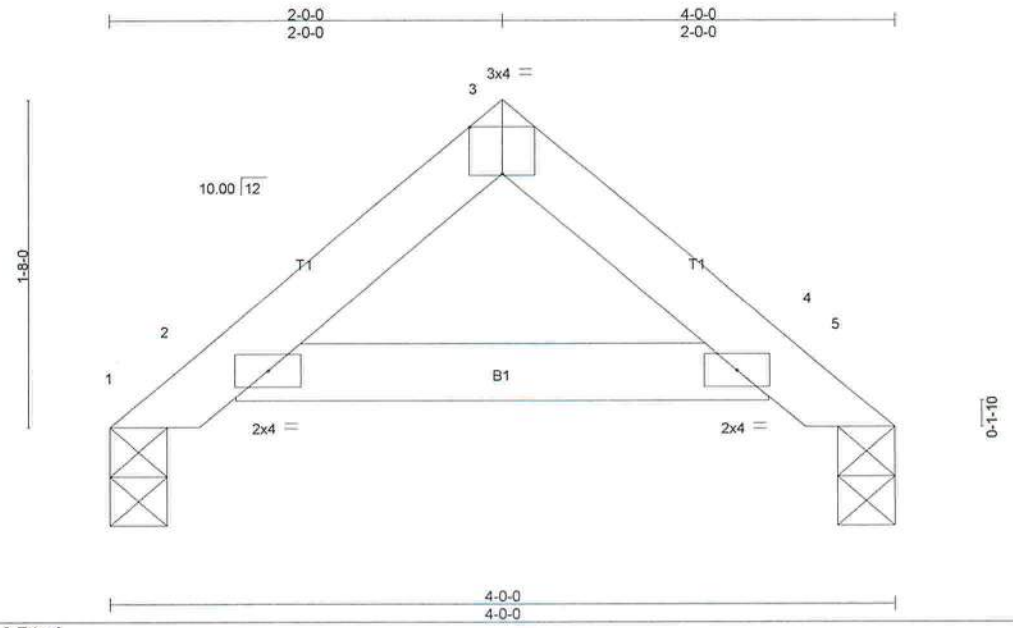


Plate Offsets (X,Y): [3:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.01	2-4	>999	240	Weight: 12 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

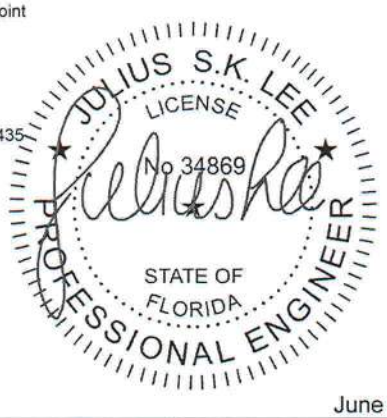
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=120/0-3-8, 5=120/0-3-8
Max Horz 1=54(LC 5)
Max Uplift 1=-36(LC 6), 5=-36(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 36 lb uplift at joint 1 and 36 lb uplift at joint 5.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799424
361993	PB02G	PIGGYBACK	1	1	Job Reference (optional)	

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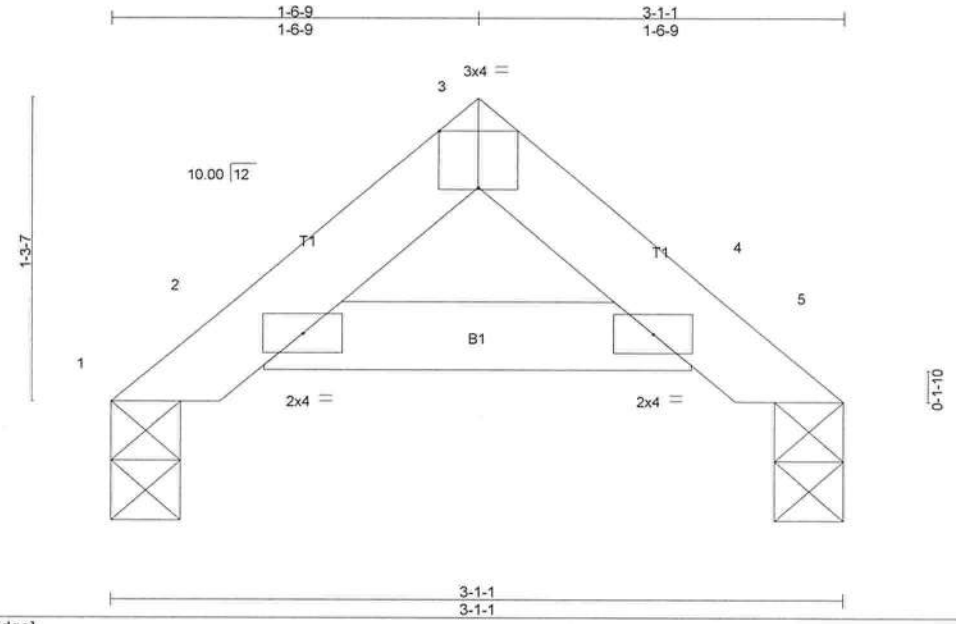

Plate Offsets (X,Y): [3:0-2-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.09	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	-0.01	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.00	2	>999	240	Weight: 9 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=98/0-3-8, 5=98/0-3-8
Max Horz 1=-55(LC 4)
Max Uplift 1=-54(LC 6), 5=-54(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (10-11)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Bearing at joint(s) 1, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 54 lb uplift at joint 1 and 54 lb uplift at joint 5.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) SEE MiTek STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799425
361993	T01	ATTIC	2	1	Job Reference (optional)	

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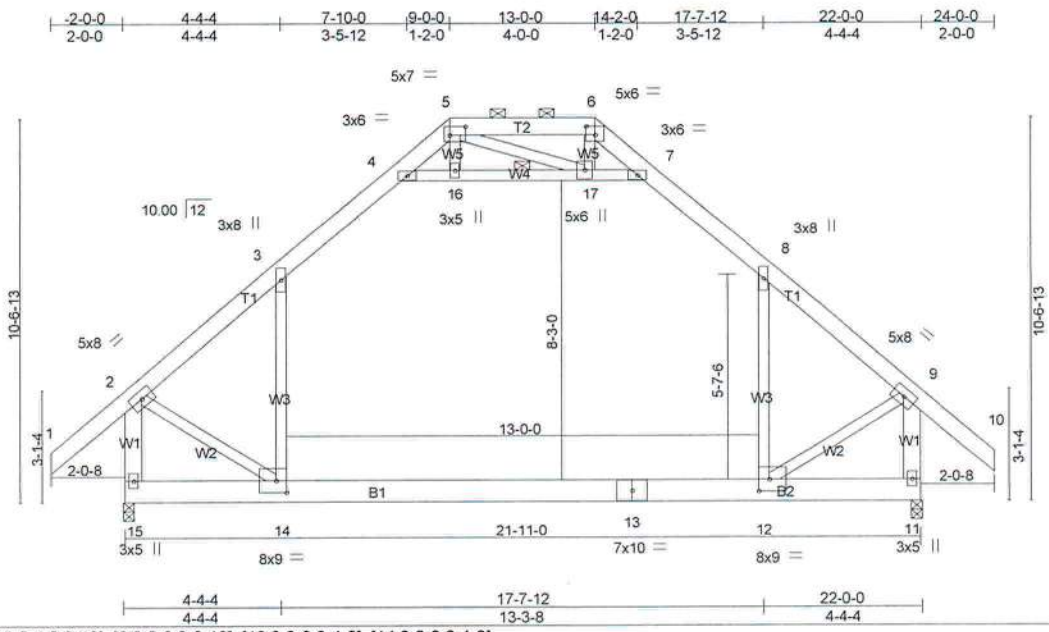

Plate Offsets (X,Y): [5:0-5-4,0-2-12], [6:0-3-0,0-2-12], [12:0-3-8,0-4-0], [14:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.33	12-14	>771	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.54	12-14	>480	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.01	11	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.10	12-14	>999	240	Weight: 204 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-9-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 15=1601/0-3-8, 11=1601/0-3-8
Max Horz 15=388(LC 5)
Max Uplift 15=-134(LC 6), 11=-134(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1472/264, 3-4=-1005/400, 4-5=0/314, 6-7=0/346, 7-8=-1005/401, 8-9=-1471/263, 2-15=-1874/388, 9-11=-1873/387, 5-6=0/524
BOT CHORD 14-15=-370/426, 13-14=-50/954, 12-13=-50/954
WEBS 4-16=-1297/362, 16-17=-1288/363, 7-17=-1337/377, 3-14=0/656, 8-12=0/654, 2-14=-105/1083, 9-12=-108/1084

NOTES (13-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-16, 16-17, 7-17; Wall dead load (5.0psf) on member(s).3-14, 8-12
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14
8) All bearings are assumed to be SYP No.2 .
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 15 and 134 lb uplift at joint 11.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
12) Attic room checked for L/360 deflection.
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T01G	GABLE	1	1		I4799426

Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

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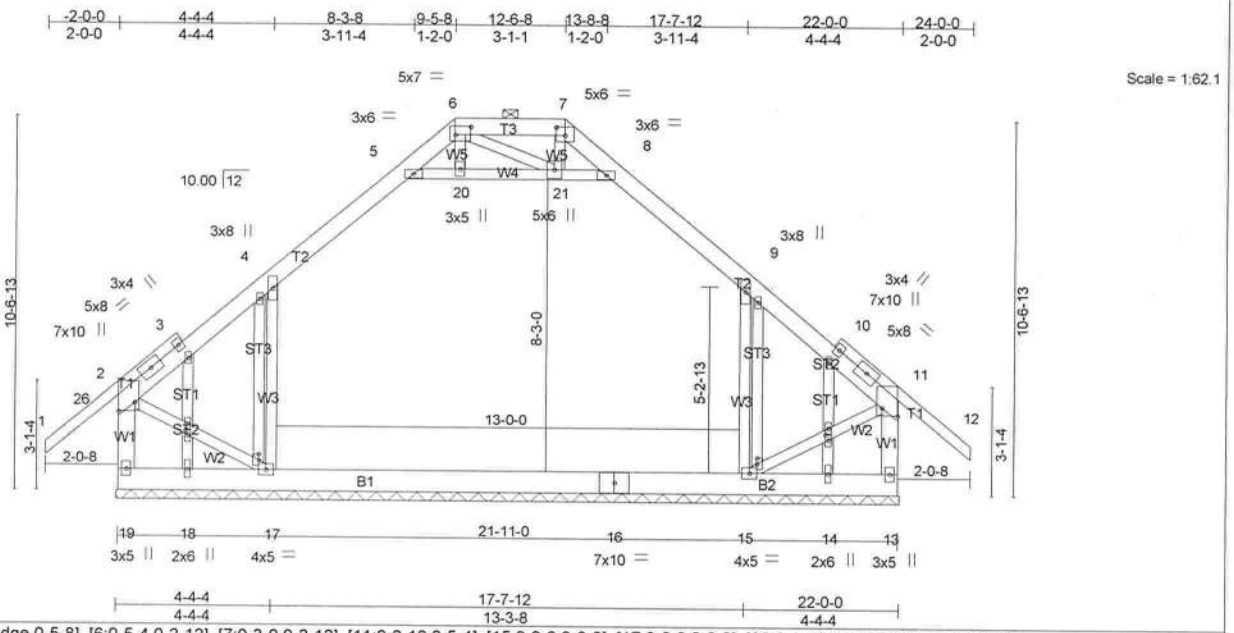

Plate Offsets (X,Y): [2:Edge,0-5-8], [6:0-5-4,0-2-12], [7:0-3-0,0-2-12], [11:0-2-12,0-5-4], [15:0-2-0,0-0-0], [17:0-2-0,0-0-0], [22:0-1-12,0-1-0], [24:0-1-12,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.07	11-12	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.10	11-12	n/r	90		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.24	Horz(TL)	0.01	13	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							Weight: 224 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except* T1: 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except* W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 22-0-0.
(lb) - Max Horz 19=500(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) except 19=-657(LC 4), 17=-374(LC 6), 15=-367(LC 7), 13=-663(LC 5), 14=-500(LC 1), 18=-503(LC 1)
Max Grav All reactions 250 lb or less at joint(s) except 19=1529(LC 1), 17=1006(LC 11), 15=995(LC 12), 13=1558(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-950/483, 3-4=-748/442, 4-5=-1024/512, 5-6=-535/286, 7-8=-522/278, 8-9=-1024/512, 9-10=-751/437, 10-11=-954/479, 2-19=-1366/645, 11-13=-1398/657, 6-7=-390/271
BOT CHORD 18-19=-434/457, 17-18=-434/457, 16-17=-383/646, 15-16=-383/646
WEBS 5-20=-266/321, 20-21=-262/321, 8-21=-280/334, 4-17=-640/445, 9-15=-634/440, 2-17=-404/747, 11-15=-411/756

NOTES (17-18)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) Provide adequate drainage to prevent water ponding.
5) All plates are 2x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
10) Ceiling dead load (5.0 psf) on member(s). 4-5, 8-9, 5-20, 20-21, 8-21; Wall dead load (5.0psf) on member(s).4-17, 9-15
11) All bearings are assumed to be SYP No.2 .
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 657 lb uplift at joint 19, 374 lb uplift at joint 17, 367 lb uplift at joint 15, 663 lb uplift at joint 13, 500 lb uplift at joint 14 and 503 lb uplift at joint 18.
13) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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NOTES (17-18)

14) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
15) Attic room checked for L/360 deflection.
16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
17) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
18) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 17-19=-10, 15-17=-50, 13-15=-10, 1-26=-54, 2-26=-114(F=-60), 2-4=-114(F=-60), 4-5=-124(F=-60), 5-6=-114(F=-60), 7-8=-114(F=-60), 8-9=-124(F=-60), 9-11=-114(F=-60), 11-12=-114(F=-60), 5-8=-10, 6-7=-114(F=-60)
   Drag: 4-17=-10, 9-15=-10

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T02	ATTIC	4	1	Job Reference (optional)	I4799427

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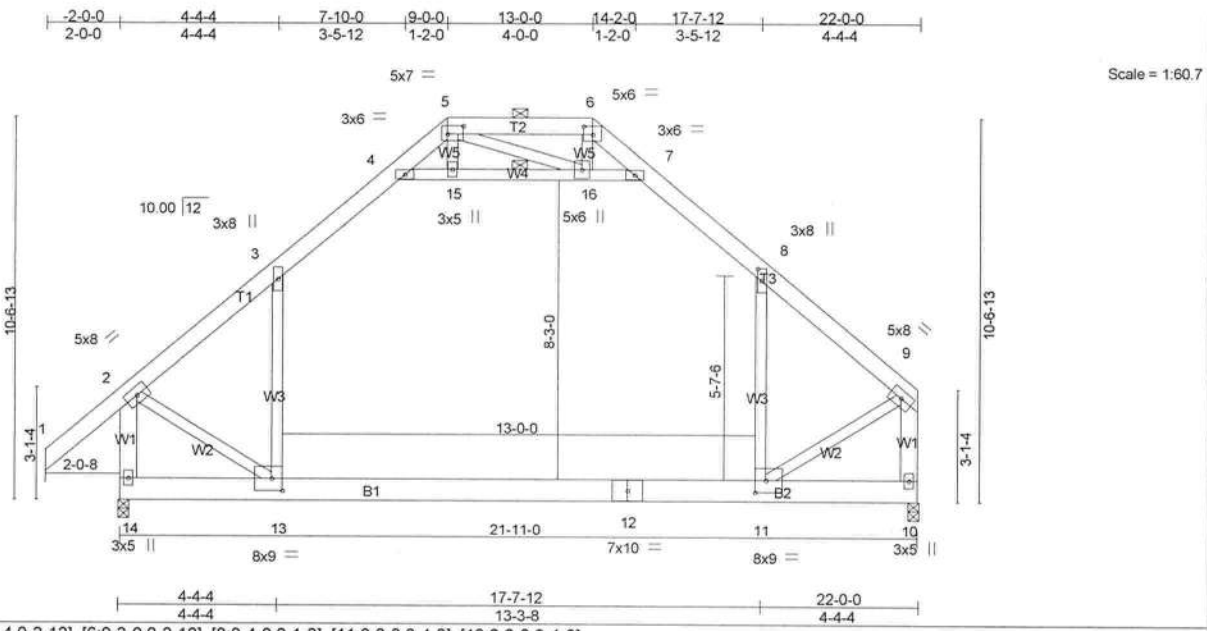


Plate Offsets (X,Y): [5:0-5-4,0-2-12], [6:0-3-0,0-2-12], [8:0-4-0,0-1-3], [11:0-3-8,0-4-0], [13:0-3-8,0-4-0]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.67	Vert(LL)	-0.34	11-13	>762	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.41	Vert(TL)	-0.54	11-13	>474	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.10	11-13	>999	240	Weight: 198 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-8-9 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-7

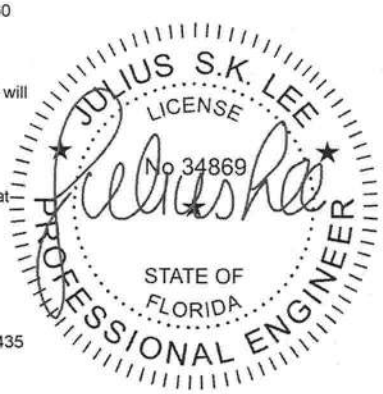
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=1608/0-3-8, 10=1472/0-3-8
Max Horz 14=437(LC 5)
Max Uplift 14=-133(LC 6), 10=-12(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1486/258, 3-4=-1012/395, 4-5=0/318, 6-7=0/351, 7-8=-1013/399, 8-9=-1463/221, 2-14=-1891/382, 9-10=-1730/213, 5-6=0/531
BOT CHORD 13-14=-419/382, 12-13=-96/963, 11-12=-96/963
WEBS 4-15=-1322/355, 15-16=-1313/356, 7-16=-1364/381, 3-13=0/660, 8-11=0/636, 2-13=-102/1096, 9-11=-126/1091

NOTES (13-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-15, 15-16, 7-16; Wall dead load (5.0psf) on member(s).3-13, 8-11
7) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
8) All bearings are assumed to be SYP No.2 .
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 14 and 12 lb uplift at joint 10.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
12) Attic room checked for L/360 deflection.
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799428
361993	T03	ATTIC	2	2	Job Reference (optional)	

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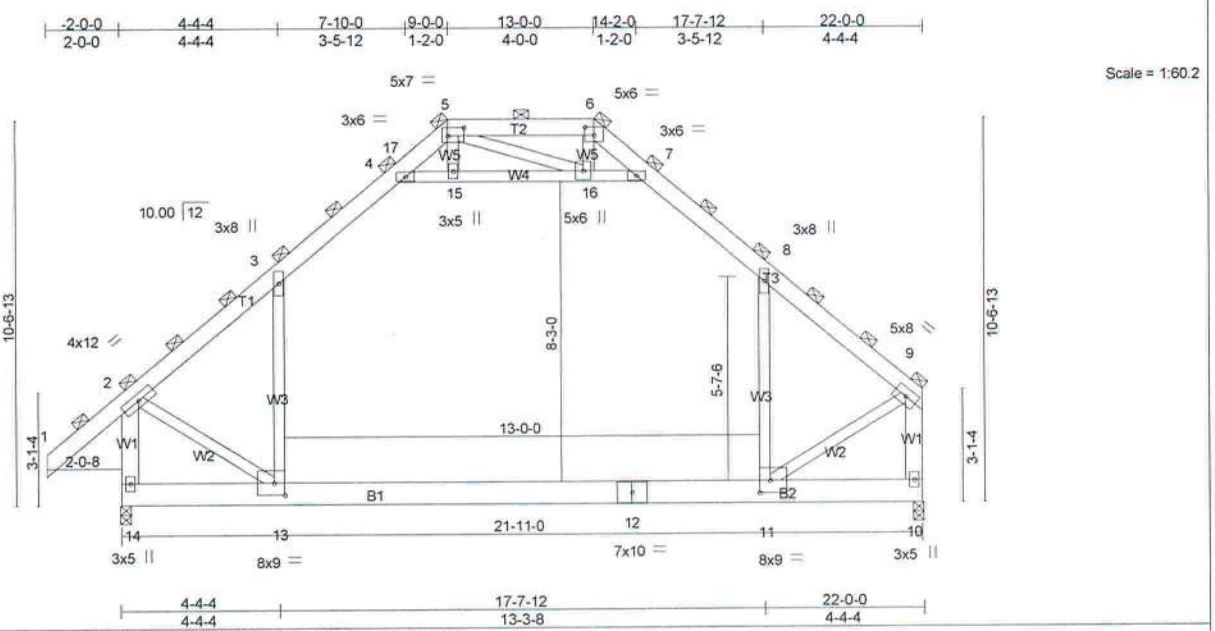

Plate Offsets (X,Y): [5:0-5-4,0-2-12], [6:0-3-0,0-2-12], [11:0-3-8,0-4-0], [13:0-3-8,0-4-0]

LOADING (psf)		SPACING	4-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.97	Vert(LL)	-0.38	11-13	>685	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.53	Vert(TL)	-0.60	11-13	>429	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.64	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.17	11-13	>999	240	Weight: 395 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 5, 9, 6, 2

REACTIONS (lb/size) 14=4940/0-3-8, 10=3810/0-3-8
Max Horz 14=874(LC 4)
Max Uplift 14=-701(LC 5), 10=-110(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3935/347, 3-4=-2659/447, 4-17=0/754, 5-17=0/786, 6-7=-15/897, 7-8=-2429/418, 8-9=-3562/240, 2-14=-4894/504, 9-10=-4192/239, 5-6=-105/1351
BOT CHORD 13-14=-920/681, 12-13=-335/2383, 11-12=-335/2383
WEBS 4-15=-3288/450, 15-16=-3269/452, 7-16=-3659/584, 3-13=0/1368, 8-11=-35/1609, 2-13=-278/2474, 9-11=-443/2702, 6-16=-156/334, 5-16=-596/545

NOTES (15-16)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-15, 15-16, 7-16; Wall dead load (5.0psf) on member(s).3-13, 8-11
9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
10) All bearings are assumed to be SYP No.2 .
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 701 lb uplift at joint 14 and 110 lb uplift at joint 10.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
14) Attic room checked for L/360 deflection.
15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2011

Your Company Name

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 13-14=-180(F=-160), 11-13=-220, 10-11=-180(F=-160), 1-2=-108, 5-17=-108, 6-7=-108, 7-8=-128, 8-9=-108, 4-7=-20, 5-6=-108

Drag: 3-13=-20, 8-11=-20

Trapezoidal Loads (plf)

Vert: 2=-315(F=-207)-to-3=-272(F=-164), 3=-292(F=-164)-to-4=-258(F=-130), 4=-238(F=-130)-to-17=-233(F=-125)

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799429
361993	T04	ATTIC	1	2	Job Reference (optional)	

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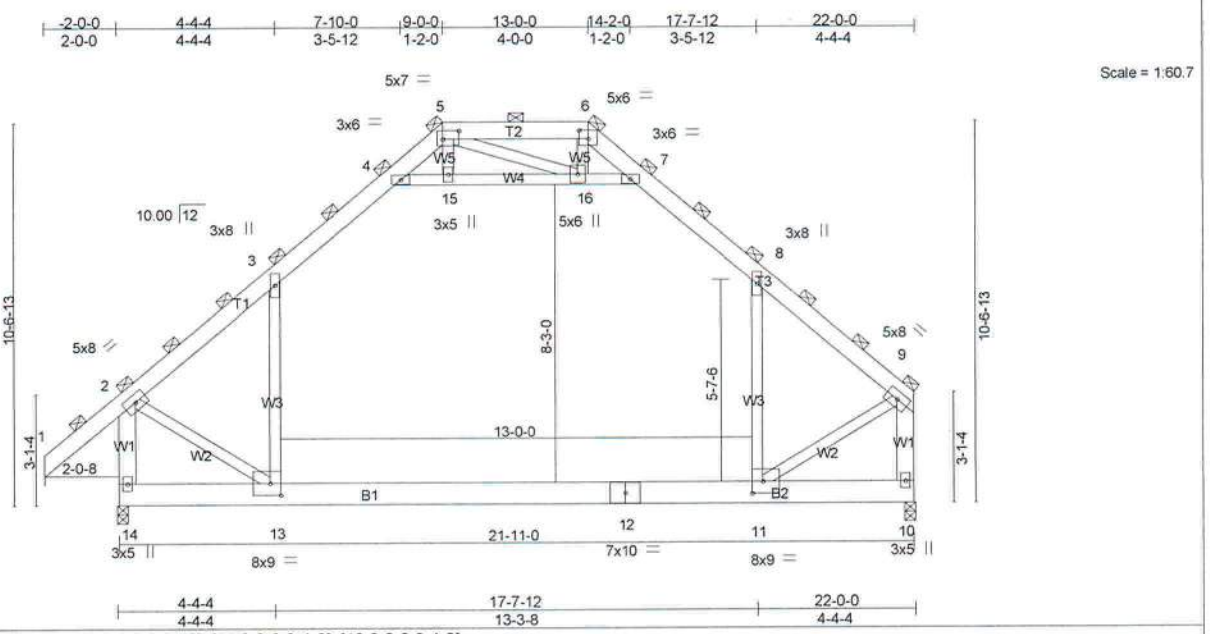

Plate Offsets (X,Y): [5:0-5-4,0-2-12], [6:0-3-0,0-2-12], [11:0-3-8,0-4-0], [13:0-3-8,0-4-0]

LOADING (psf)		SPACING	3-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.57	Vert(LL)	-0.25	11-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.41	11-13	>632	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.08	11-13	>999	240	Weight: 395 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 5, 9, 6, 2

REACTIONS (lb/size) 14=2412/0-3-8, 10=2208/0-3-8
Max Horz 14=655(LC 4)
Max Uplift 14=-199(LC 5), 10=-18(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2229/78, 3-4=-1518/228, 4-5=0/477, 6-7=0/526, 7-8=-1519/233, 8-9=-2195/71, 2-14=-2836/64, 9-10=-2596/25, 5-6=0/796
BOT CHORD 13-14=-628/573, 12-13=-144/1445, 11-12=-144/1445
WEBS 4-15=-1984/250, 15-16=-1970/251, 7-16=-2045/266, 3-13=0/989, 8-11=0/953, 2-13=-153/1644, 9-11=-189/1636, 5-16=-299/300

NOTES (15-16)
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Ceiling dead load (5.0 psf) on member(s). 3-4, 7-8, 4-15, 15-16, 7-16; Wall dead load (5.0psf) on member(s).3-13, 8-11
9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
10) All bearings are assumed to be SYP No.2 .
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 199 lb uplift at joint 14 and 18 lb uplift at joint 10.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
14) Attic room checked for L/360 deflection.
15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T04	ATTIC	1	2	I4799429

Builders FirstSource, Lake City, FL 32055

Job Reference (optional)

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LOAD CASE(S) Standard

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799430
361993	T05	ATTIC	2	1	Job Reference (optional)	

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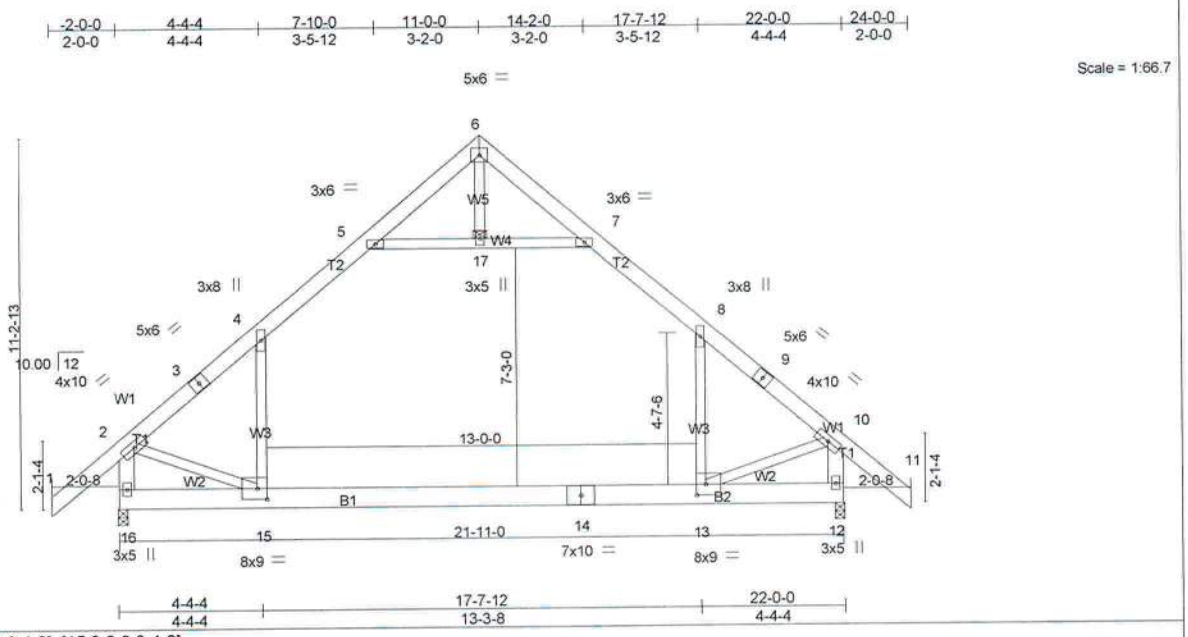

Plate Offsets (X,Y): [13:0-3-8,0-4-0], [15:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.40	13-15	>651	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.43	Vert(TL)	-0.64	13-15	>402	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.32	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.11	13-15	>999	240	Weight: 193 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-11-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 16=1591/0-3-8, 12=1591/0-3-8
Max Horz 16=395(LC 5)
Max Uplift 16=-158(LC 6), 12=-158(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1677/195, 3-4=-1550/212, 4-5=-1092/371, 7-8=-1092/371, 8-9=-1550/212, 9-10=-1677/195, 2-16=-1841/386, 10-12=-1841/386
BOT CHORD 15-16=-391/490, 14-15=0/1067, 13-14=0/1067
WEBS 5-17=-1261/388, 7-17=-1261/388, 4-15=-11/840, 8-13=-10/840, 2-15=-92/991, 10-13=-99/991

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-17, 7-17; Wall dead load (5.0psf) on member(s).4-15, 8-13
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
7) All bearings are assumed to be SYP No.2 .
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 16 and 158 lb uplift at joint 12.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) Attic room checked for L/360 deflection.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T05G	GABLE	1	1		I4799431
Builders FrstSource, Lake City, FL 32055					Job Reference (optional)	

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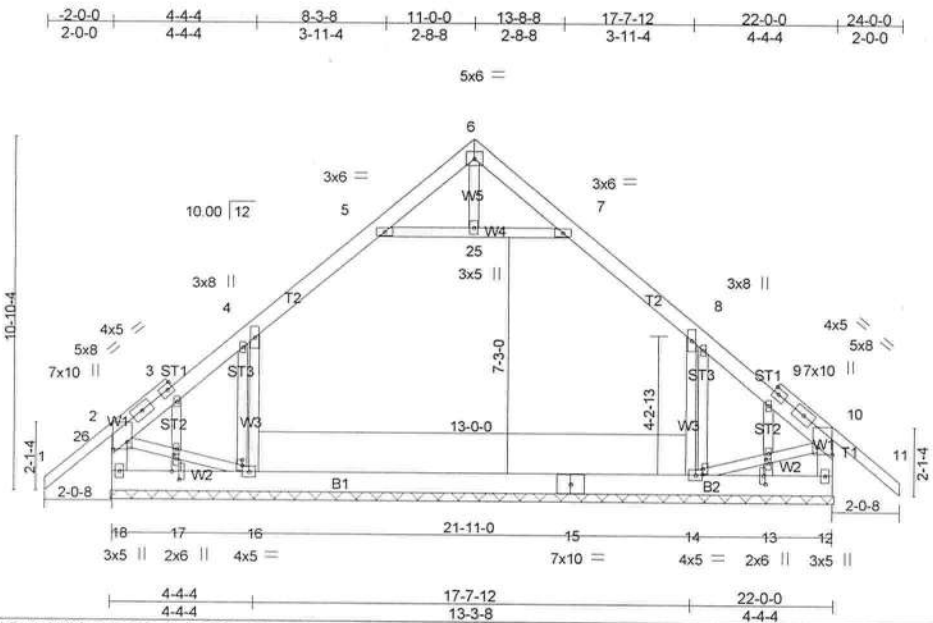


Plate Offsets (X,Y): [2:0-2-12,0-5-4], [10:0-2-12,0-5-4], [13:0-3-0,0-0-12], [14:0-2-0,0-0-0], [16:0-2-0,0-0-0], [17:0-3-0,0-2-12], [20:0-1-8,0-1-0], [23:0-1-8,0-1-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.07	10-11	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.10	10-11	n/r	90		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.21	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							Weight: 208 lb	

LUMBER

TOP CHORD 2 X 6 SYP No.1D *Except*
T1: 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

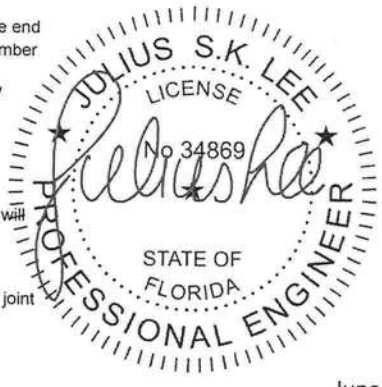
REACTIONS All bearings 22-0-0.
(lb) - Max Horz 18=-491(LC 4)
Max Uplift All uplift 100 lb or less at joint(s) except 18=-465(LC 7), 16=-338(LC 6), 14=-332(LC 7), 12=-471(LC 6), 13=-524(LC 1), 17=-526(LC 1)
Max Grav All reactions 250 lb or less at joint(s) except 18=1416(LC 1), 16=1135(LC 11), 14=1123(LC 12), 12=1446(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-986/341, 3-4=-737/325, 4-5=-1043/485, 5-6=-420/178, 6-7=-420/180, 7-8=-1042/485, 8-9=-740/322, 9-10=-990/338, 2-18=-1230/539, 10-12=-1263/551
BOT CHORD 17-18=-398/440, 16-17=-398/440, 15-16=-236/670, 14-15=-236/670
WEBS 5-25=-431/410, 7-25=-431/410, 4-16=-613/454, 8-14=-606/447, 2-16=-174/654, 10-14=-219/669

NOTES (15-16)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
9) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-25, 7-25; Wall dead load (5.0psf) on member(s).4-16, 8-14
10) All bearings are assumed to be SYP No.2 .
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 465 lb uplift at joint 18, 338 lb uplift at joint 16, 332 lb uplift at joint 14, 471 lb uplift at joint 12, 524 lb uplift at joint 13 and 526 lb uplift at joint 17.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) Attic room checked for L/360 deflection.
14) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

Continued on page 2



June 24,2011

Your Company Name

15) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

16) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 16-18=-10, 14-16=-50, 12-14=-10, 1-26=-54, 2-26=-114(F=-60), 2-4=-114(F=-60), 4-5=-124(F=-60), 5-6=-114(F=-60), 6-7=-114(F=-60), 7-8=-124(F=-60), 8-10=-114(F=-60), 10-11=-114(F=-60), 5-7=-10

Drag: 4-16=-10, 8-14=-10

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T06	ATTIC	10	1	Job Reference (optional)	I4799432

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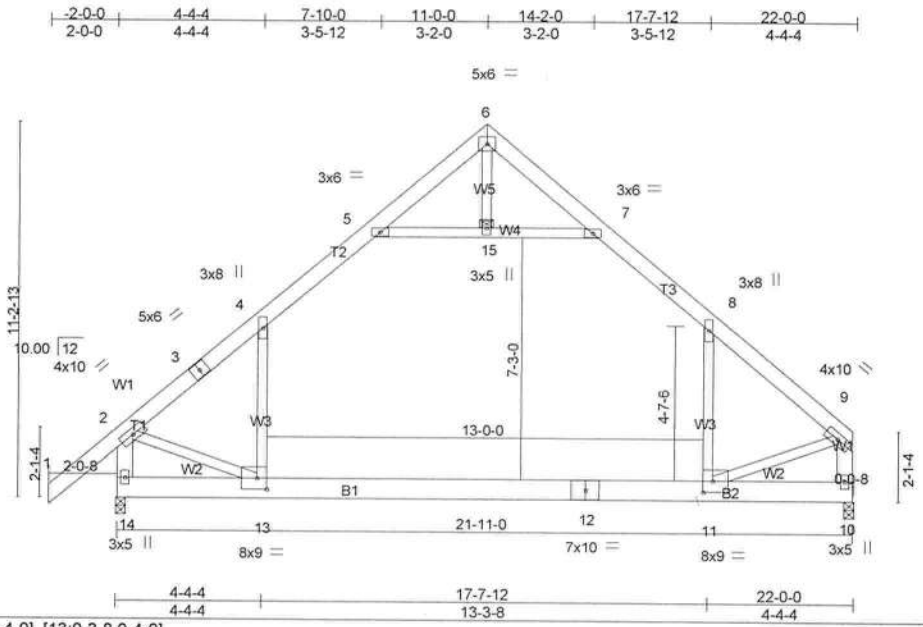


Plate Offsets (X,Y): [11:0-3-8,0-4-0], [13:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.40	11-13	>641	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.44	Vert(TL)	-0.65	11-13	>396	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.01	10	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.11	11-13	>999	240	Weight: 187 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP 2400F 2.0E
WEBS 2 X 4 SYP No.3 *Except*
W3: 2 X 4 SYP No.2, W1: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-7

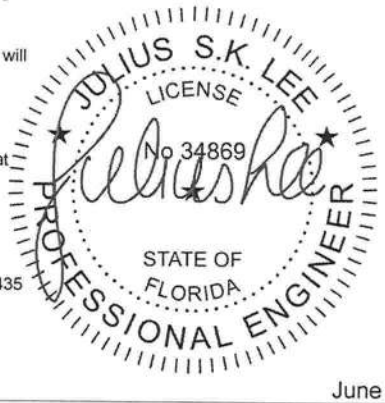
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=1598/0-3-8, 10=1462/0-3-8
Max Horz 14=443(LC 5)
Max Uplift 14=-158(LC 6), 10=-31(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1692/203, 3-4=-1565/221, 4-5=-1100/375, 7-8=-1101/376, 8-9=-1664/194, 2-14=-1857/396, 9-10=-1697/194
BOT CHORD 13-14=-439/446, 12-13=-39/1078, 11-12=-39/1078
WEBS 5-15=-1280/400, 7-15=-1280/400, 4-13=-12/852, 8-11=-9/811, 2-13=-93/1003, 9-11=-91/1022

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Ceiling dead load (5.0 psf) on member(s). 4-5, 7-8, 5-15, 7-15; Wall dead load (5.0psf) on member(s).4-13, 8-11
6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13
7) All bearings are assumed to be SYP No.2 .
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 14 and 31 lb uplift at joint 10.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) Attic room checked for L/360 deflection.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 24,2011

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799433
361993	T06A	ROOF TRUSS	5	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055

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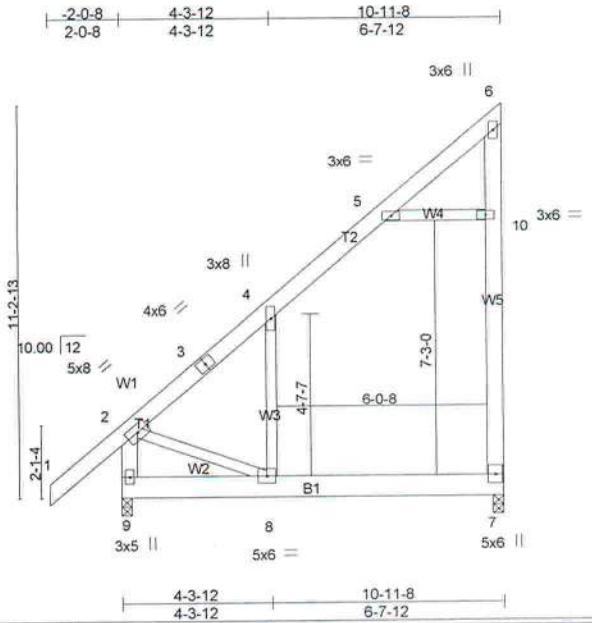


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.35	Vert(LL)	-0.12	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.38	Vert(TL)	-0.22	7-8	>577	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.24	Horz(TL)	-0.00	7	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.21	7-8	>589	240	Weight: 118 lb	

LUMBER
TOP CHORD 2 X 6 SYP No.1D
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W5,W1: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-10-11 oc bracing.

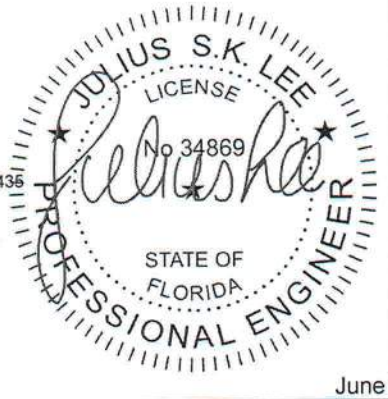
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=719/0-3-8, 7=833/0-3-8
Max Horz 9=524(LC 6)
Max Uplift 7=-240(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-723/0, 3-4=-707/0, 4-5=-482/0, 5-6=-484/323, 7-10=-290/456, 6-10=-271/465, 2-9=-493/0
BOT CHORD 8-9=-1188/137, 7-8=-308/128
WEBS 4-8=-45/333, 2-8=-45/953, 5-10=-337/141

NOTES (10-11)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Ceiling dead load (5.0 psf) on member(s). 4-5, 5-10; Wall dead load (5.0psf) on member(s).4-8
5) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 7-8
6) All bearings are assumed to be SYP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 240 lb uplift at joint 7.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Attic room checked for L/360 deflection.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T07	COMMON	6	1		I4799434
Builders FrstSource, Lake City, FL 32055					Job Reference (optional)	

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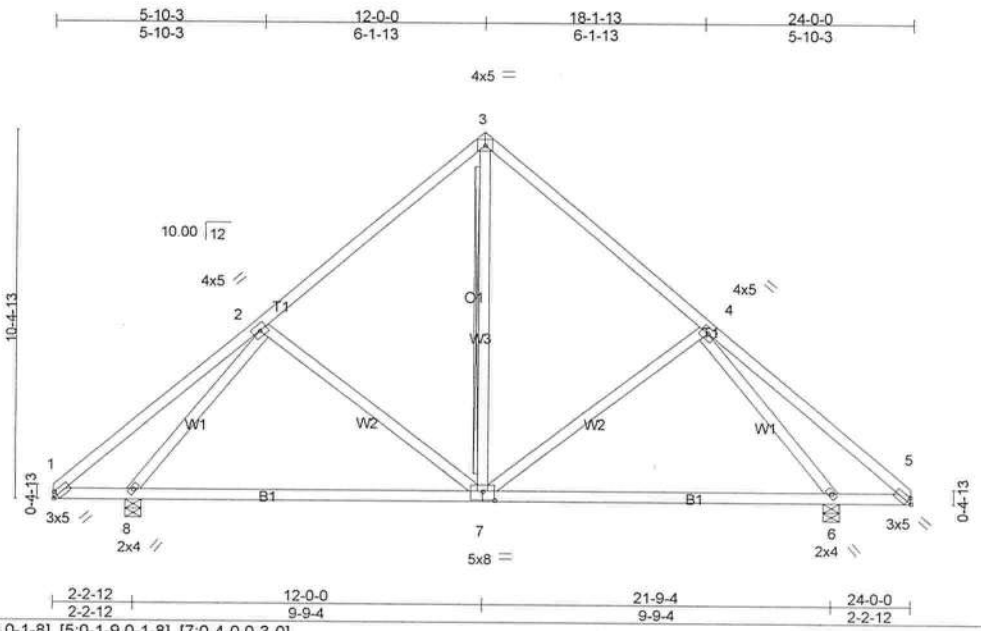


Plate Offsets (X,Y): [1:0-1-9,0-1-8], [5:0-1-9,0-1-8], [7:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.13	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.60	Vert(TL)	-0.22	6-7	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.52	Horz(TL)	-0.02	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.41	6-7	>579	240	Weight: 137 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

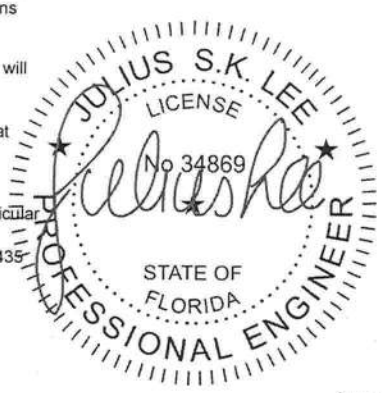
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 8=768/0-5-8, 6=768/0-5-8
Max Horz 8=-352(LC 4)
Max Uplift 8=-622(LC 6), 6=-622(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-528/928, 3-4=-528/928
BOT CHORD 7-8=-484/392, 6-7=-474/392
WEBS 3-7=-907/308, 2-8=-797/847, 4-6=-797/847

NOTES (9-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 622 lb uplift at joint 8 and 622 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799435
361993	T07G	GABLE	1	1	Job Reference (optional)	

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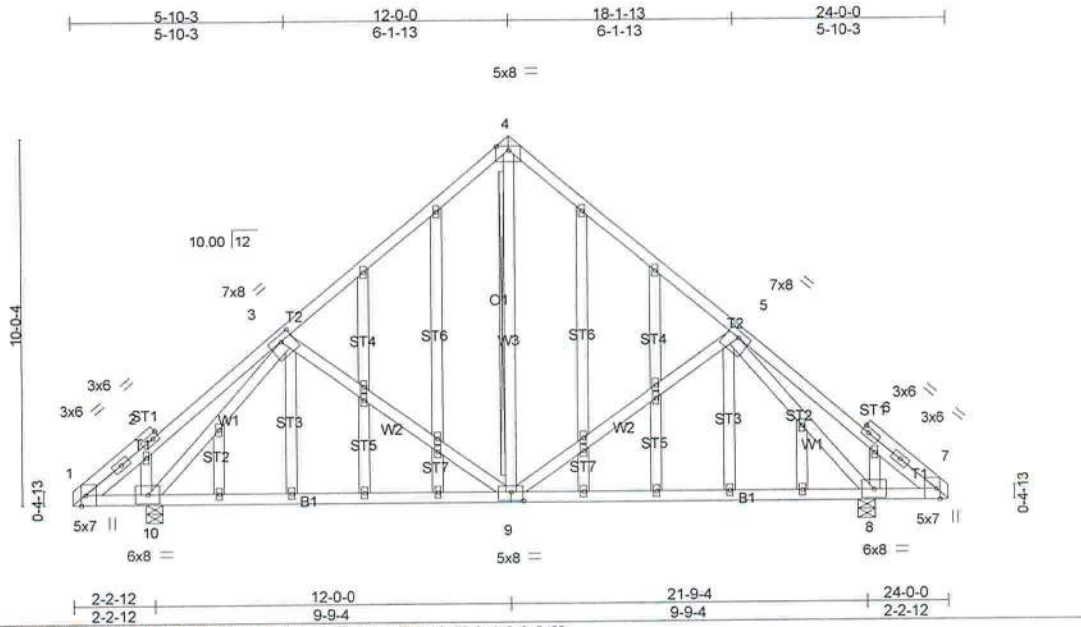

Plate Offsets (X,Y): [1:0-3-8,Edge], [3:0-4-0,0-2-0], [5:0-4-0,0-2-0], [7:0-3-8,Edge], [9:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	1.00	Vert(LL)	-0.13	8-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.23	8-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	1.00	Horz(TL)	-0.04	8	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.43	8-9	>556	240	Weight: 204 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-2-10 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-0-14 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 4-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=1488/0-5-8, 8=1488/0-5-8
Max Horz 10=-423(LC 4)
Max Uplift 10=-1670(LC 6), 8=-1670(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-2=-234/288, 2-3=-366/539, 3-4=-1066/1768, 4-5=-1066/1768, 5-6=-366/539, 6-7=-234/288
BOT CHORD	1-10=-236/289, 9-10=-1215/838, 8-9=-1215/838, 7-8=-236/288
WEBS	4-9=-1176/430, 5-9=-288/503, 3-9=-288/503, 3-10=-1699/2301, 5-8=-1699/2301

NOTES (13-14)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SYP No.2 .
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1670 lb uplift at joint 10 and 1670 lb uplift at joint 8.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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Continued on page 2

LOAD CASE(S) Standard

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T07G	GABLE	1	1	I4799435
					Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 1-7=-10

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799436
361993	T08	COMMON	2	1	Job Reference (optional)	

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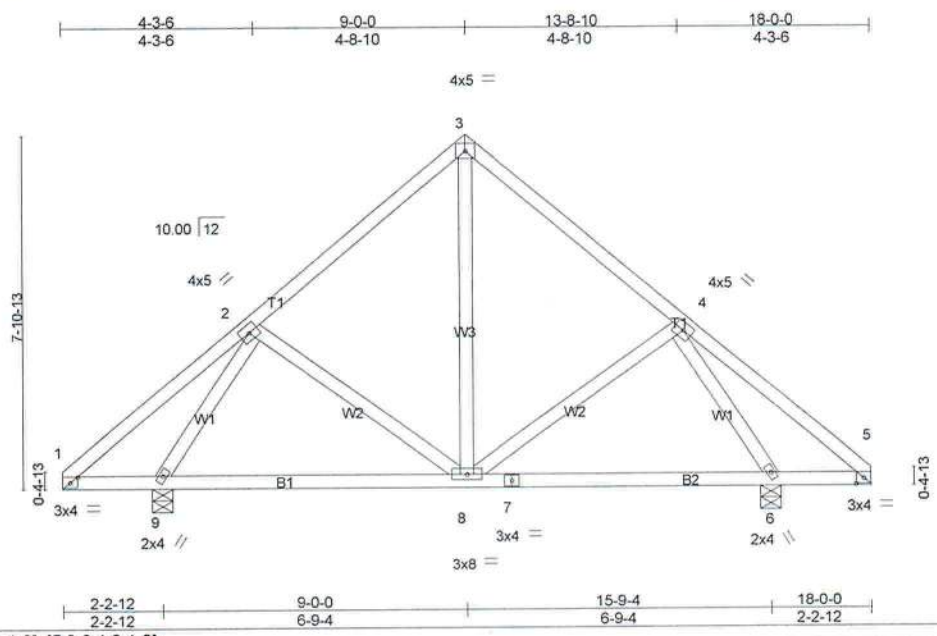


Plate Offsets (X,Y): [1:0-2-1,0-1-8], [5:0-2-1,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.22	Vert(LL)	-0.03	6-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.05	6-8	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.01	8	>999	240	Weight: 101 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

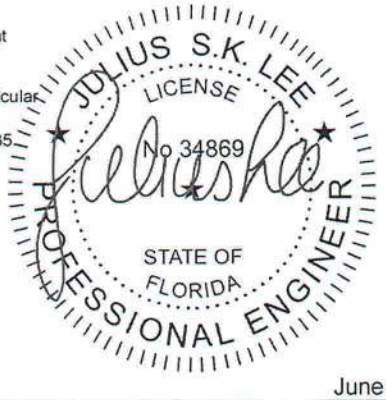
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 9=576/0-5-8, 6=576/0-5-8
Max Horz 9=-265(LC 4)
Max Uplift 9=-253(LC 6), 6=-253(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-342/279, 3-4=-342/279
BOT CHORD 8-9=-216/258
WEBS 2-9=-573/509, 4-6=-573/509

NOTES (8-9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 9 and 253 lb uplift at joint 6.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T08G	GABLE	1	1	Job Reference (optional)	I4799437

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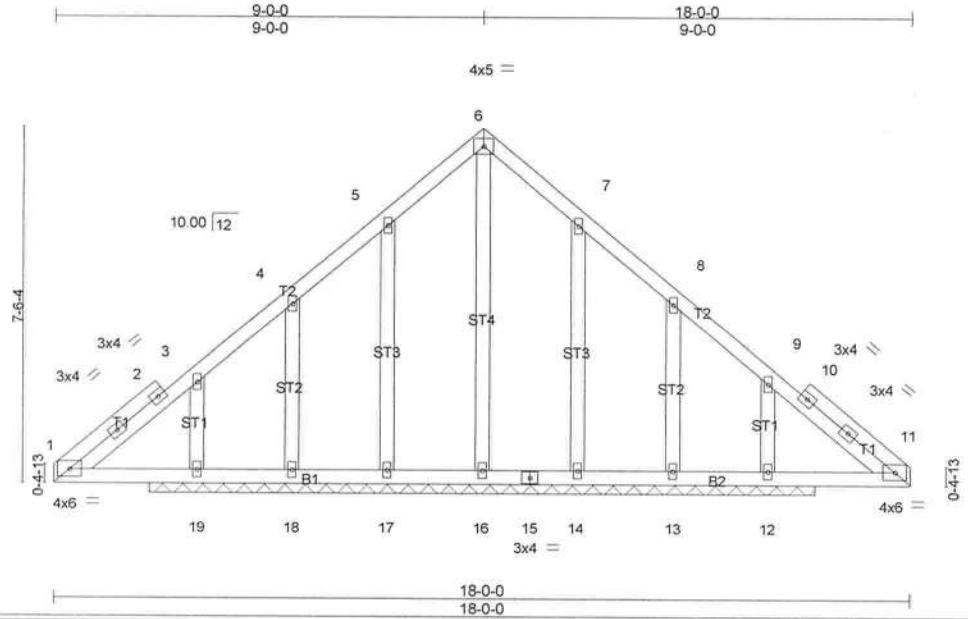


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.31	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	12	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							Weight: 112 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

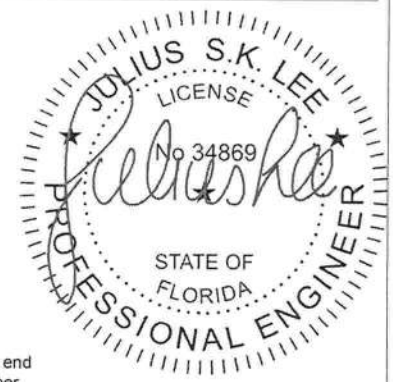
BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 14-0-0.
(lb) - Max Horz 19=-314(LC 4)
Max Uplift All uplift 100 lb or less at joint(s) 16 except 17=-261(LC 6), 18=-360(LC 5), 19=-364(LC 4), 14=-261(LC 7), 13=-354(LC 4), 12=-363(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 18, 13 except 16=553(LC 1), 17=300(LC 10), 19=434(LC 10), 14=300(LC 11), 12=434(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-331/256, 2-3=-370/380, 3-4=-237/333, 4-5=-101/353, 5-6=-13/333, 6-7=-9/333, 7-8=-101/353, 8-9=-231/333, 9-10=-370/380, 10-11=-331/256
BOT CHORD 1-19=-210/318, 18-19=-210/318, 17-18=-210/318, 16-17=-210/318, 15-16=-210/318, 14-15=-210/318, 13-14=-210/318, 12-13=-210/318, 11-12=-210/318
WEBS 6-16=-537/100, 5-17=-266/270, 4-18=-196/259, 3-19=-339/315, 7-14=-266/270, 8-13=-196/258, 9-12=-339/317

NOTES (13-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) All bearings are assumed to be SYP No.2 .
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16 except (jt=lb) 17=261, 18=360, 19=364, 14=261, 13=354, 12=363.
10) Non Standard bearing condition. Review required.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435



Continued on page 2

LOAD CASE(S) Standard

June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T08G	GABLE	1	1	I4799437
					Job Reference (optional)

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-6=-114(F=-60), 6-11=-114(F=-60), 1-11=-10

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T09	COMMON	2	1	Job Reference (optional)	I4799438

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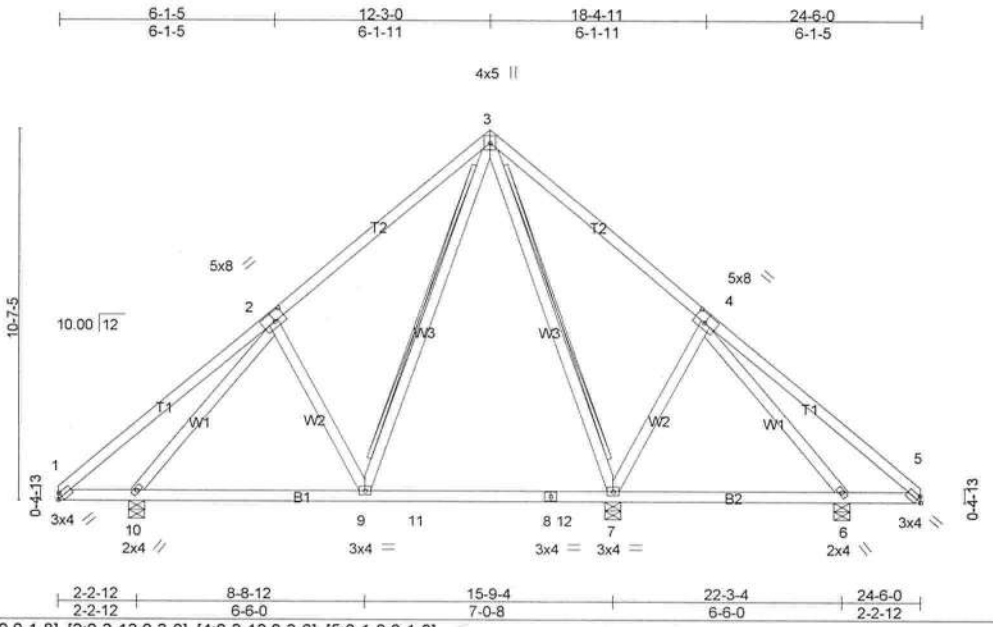


Plate Offsets (X,Y): [1:0-1-9,0-1-8], [2:0-3-12,0-3-0], [4:0-3-12,0-3-0], [5:0-1-9,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.11	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.14	7-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.48	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.11	7-9	>999	240	Weight: 151 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-7, 3-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

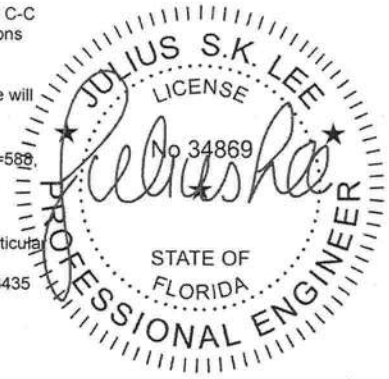
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 7=812/0-5-8, 10=604/0-5-8, 6=322/0-5-8
Max Horz 10=-359(LC 4)
Max Uplift 7=-588(LC 7), 10=-451(LC 6), 6=-238(LC 7)
Max Grav 7=812(LC 1), 10=604(LC 1), 6=324(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-75/289, 2-3=-400/686, 4-5=-88/298
BOT CHORD 9-10=-412/283
WEBS 3-7=-465/402, 4-7=-230/431, 3-9=-722/320, 2-9=-155/319, 2-10=-674/641, 4-6=-333/145

NOTES (9-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=588, 10=451, 6=238.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T09G	GABLE	1	1	I4799439
					Job Reference (optional)

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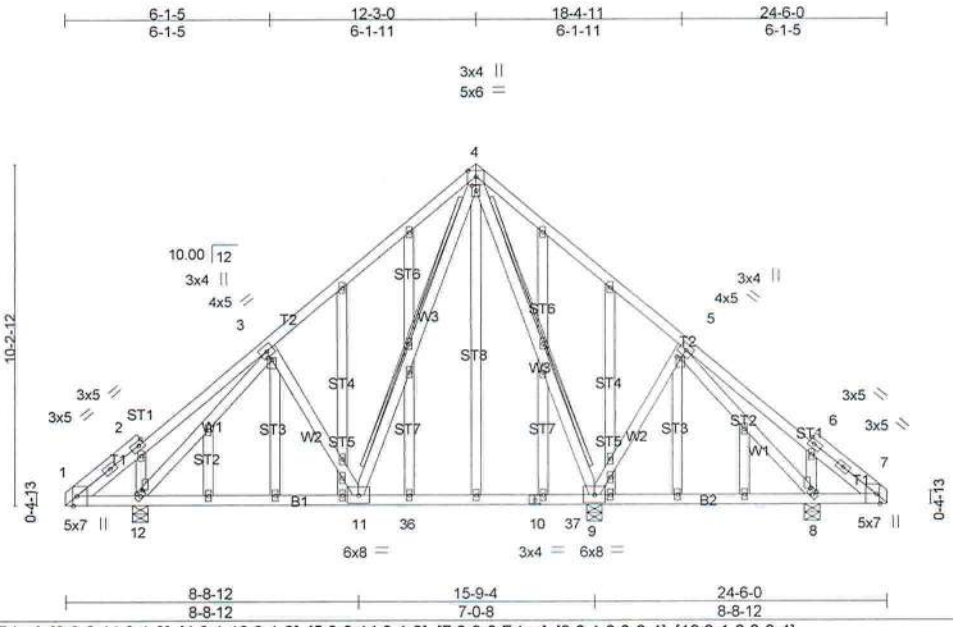


Plate Offsets (X,Y): [1:0-3-8,Edge], [3:0-0-14,0-1-8], [4:0-1-12,0-1-8], [5:0-0-14,0-1-8], [7:0-3-8,Edge], [8:0-1-3,0-2-4], [12:0-1-3,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.12	9-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.37	Vert(TL)	-0.15	9-11	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.85	Horz(TL)	-0.01	8	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.12	9-11	>999	240	Weight: 233 lb	

LUMBER

TOP CHORD	2 X 4 SYP No.2
BOT CHORD	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3
OTHERS	2 X 4 SYP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 4-9, 4-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

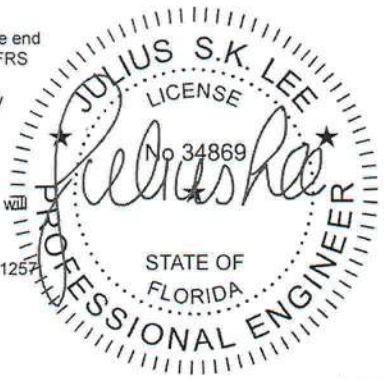
REACTIONS (lb/size) 9=1455/0-5-8, 12=1127/0-5-8, 8=622/0-5-8
Max Horz 12=433(LC 5)
Max Uplift 9=-1257(LC 7), 12=-970(LC 6), 8=-525(LC 7)
Max Grav 9=1455(LC 1), 12=1127(LC 1), 8=627(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-2=-255/354, 2-3=-374/618, 3-4=-742/986, 4-5=-131/331, 5-6=-403/640, 6-7=-284/375
BOT CHORD	1-12=-291/312, 11-12=-647/555, 11-36=-253/212, 10-36=-253/212, 10-37=-253/212, 9-37=-253/212, 7-8=-307/332
WEBS	4-9=-924/880, 5-9=-478/669, 4-11=-903/504, 3-11=-364/529, 3-12=-1318/1290, 5-8=-698/503

NOTES (13-14)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
8) All bearings are assumed to be SYP No.2 .
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=1257, 12=970, 8=525.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).



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Your Company Name

13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-4=-114(F=-60), 4-7=-114(F=-60), 1-36=-10, 36-37=-50, 7-37=-10

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799440
361993	T10	COMMON	1	1	Job Reference (optional)	

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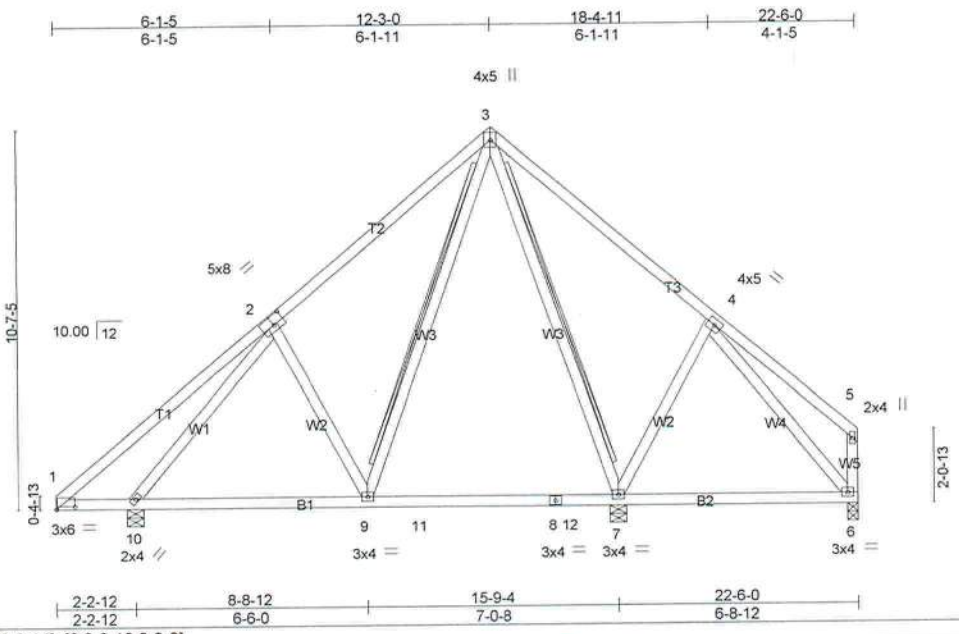

Plate Offsets (X,Y): [1:0-6-3,0-0-14], [2:0-3-12,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.11	7-9	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.45	Vert(TL)	-0.14	7-9	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.49	Horz(TL)	-0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.15	6-7	>516	240	Weight: 146 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-7, 3-9
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 7=771/0-5-8, 10=624/0-5-8, 6=206/0-3-8
Max Horz 10=356(LC 5)
Max Uplift 7=-587(LC 7), 10=-447(LC 6), 6=-120(LC 6)
Max Grav 7=771(LC 1), 10=624(LC 1), 6=207(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-70/289, 2-3=-423/697
BOT CHORD 9-10=-404/298
WEBS 3-7=-423/406, 4-7=-227/401, 3-9=-725/318, 2-9=-152/314, 2-10=-698/649

NOTES (9-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=587, 10=447, 6=120.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T11	COMMON	1	1	Job Reference (optional)	I4799441

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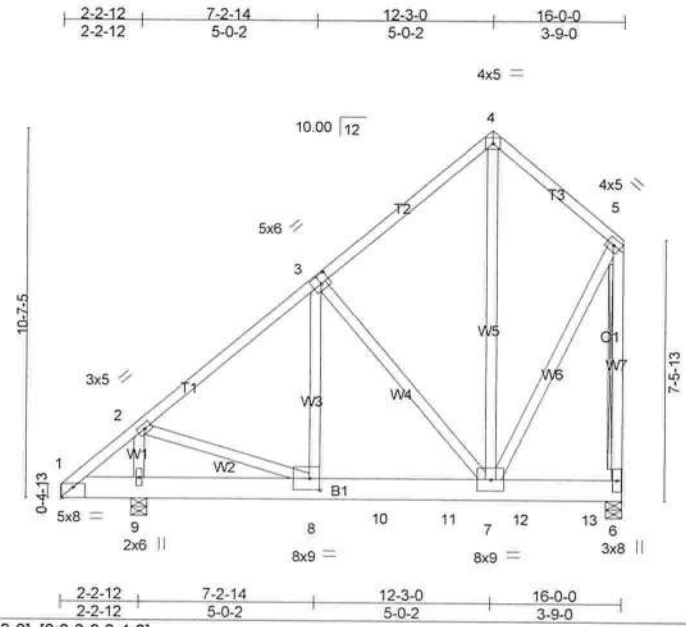

Plate Offsets (X,Y): [1:0-4-0,Edge], [3:0-3-0,0-3-0], [8:0-3-8,0-4-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.05	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.08	7-8	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.81	Horz(TL)	-0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.05	7-8	>999	240	Weight: 145 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W7: 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-1-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-6
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=2107/0-5-8, 9=1557/0-5-8
Max Horz 9=342(LC 4)
Max Uplift 6=-1132(LC 5), 9=-858(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1477/800, 3-4=-867/463, 4-5=-829/469, 5-6=-1743/944
BOT CHORD 8-9=-358/101, 8-10=-758/1065, 10-11=-758/1065, 7-11=-758/1065
WEBS 2-9=-1448/785, 2-8=-568/1086, 3-8=-597/737, 3-7=-744/687, 4-7=-463/805, 5-7=-707/1301

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); cantilever left exposed ; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=1132, 9=858.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 595 lb down and 657 lb up at 7-1-8, 541 lb down and 199 lb up at 9-0-12, 554 lb down and 203 lb up at 11-0-12, and 479 lb down and 250 lb up at 13-0-12, and 479 lb down and 250 lb up at 15-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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LOAD CASE(S) Standard

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Your Company Name

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
      Vert: 1-4=-54, 4-5=-54, 1-6=-10
   Concentrated Loads (lb)
      Vert: 8=-595(F) 10=-541(F) 11=-554(F) 12=-479(F) 13=-479(F)

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	
361993	T17	SPECIAL	1	1		I4799442

Builders FrstSource, Lake City, FL 32055

Job Reference (optional)

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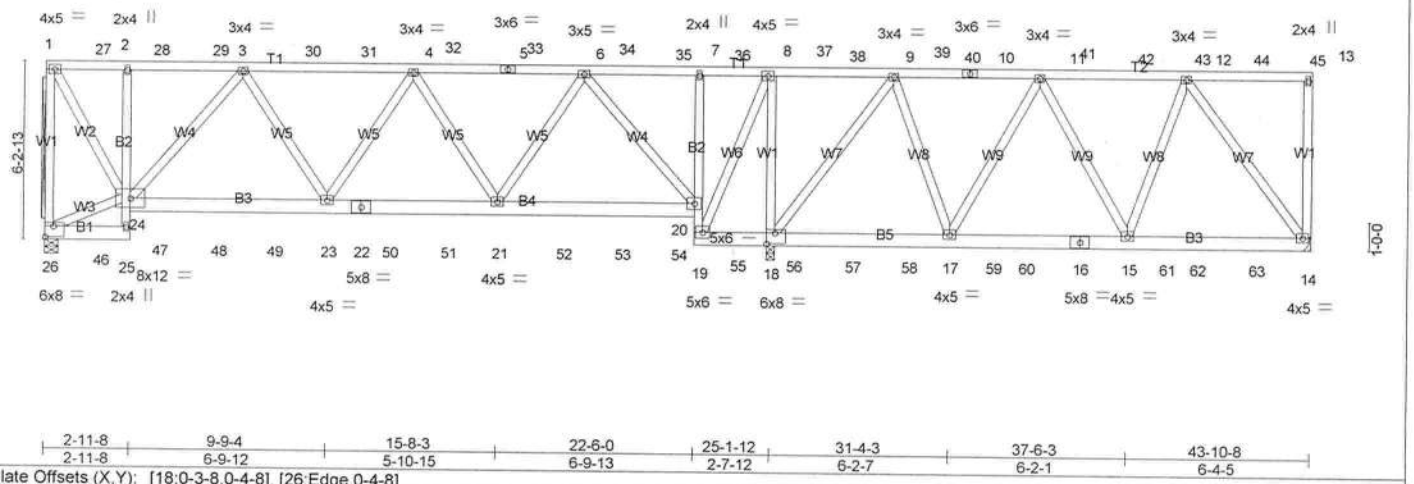
2-11-8 | 6-9-14 | 12-8-12 | 18-7-10 | 22-6-0 | 25-1-12 | 29-4-6 | 34-5-3 | 39-6-1 | 43-10-8

2-11-8 | 3-10-6 | 5-10-14 | 5-10-14 | 3-10-6 | 2-7-12 | 4-2-10 | 5-0-13 | 5-0-14 | 4-4-7

Scale = 1:76.6



2-11-8 | 9-9-4 | 15-8-3 | 22-6-0 | 25-1-12 | 31-4-3 | 37-6-3 | 43-10-8

2-11-8 | 6-9-12 | 5-10-15 | 6-9-13 | 2-7-12 | 6-2-7 | 6-2-1 | 6-4-5

Plate Offsets (X,Y): [18:0-3-8,0-4-8], [26:Edge,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.04	21-23	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.63	Vert(TL)	-0.09	23-24	>999	240		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.93	Horz(TL)	-0.03	14	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.10	23-24	>999	240	Weight: 334 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B2: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-10-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-1-4 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 1-26
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 26=1070/0-5-8, 14=609/Mechanical, 18=2338/0-3-8
Max Uplift 26=-1183(LC 3), 14=-650(LC 3), 18=-2628(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-26=-1039/1102, 1-27=-477/533, 2-27=-477/533, 2-28=-484/542, 28-29=-484/542, 3-29=-484/542, 3-30=-1061/1193, 30-31=-1061/1193, 31-32=-1061/1193, 4-32=-1061/1193, 4-33=-857/964, 5-33=-857/964, 5-34=-857/964, 6-34=-857/964, 8-38=-696/624, 38-39=-696/624, 9-39=-696/624, 11-42=-363/420, 42-43=-363/420, 12-43=-363/420

BOT CHORD 24-47=-1053/934, 47-48=-1053/934, 48-49=-1053/934, 23-49=-1053/934, 22-23=-1220/1082, 22-50=-1220/1082, 50-51=-1220/1082, 21-51=-1220/1082, 21-52=-609/538, 52-53=-609/538, 53-54=-609/538, 20-54=-609/538, 19-20=-1130/1268, 19-55=-624/696, 18-55=-624/696, 17-59=-390/354, 59-60=-390/354, 16-60=-390/354, 15-16=-390/354, 15-61=-373/340, 61-62=-373/340, 62-63=-373/340, 14-63=-373/340

WEBS 1-24=-1101/983, 3-24=-707/802, 3-23=-269/253, 4-21=-434/493, 6-21=-682/614, 6-20=-1124/1268, 8-19=-1261/1131, 8-18=-1320/1421, 9-18=-1026/1134, 9-17=-605/486, 11-17=-477/493, 12-14=-571/627

NOTES (12-14)

1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 26=1183, 14=650, 18=2628.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

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JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

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Your Company Name

NOTES (12-14)

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 103 lb down and 83 lb up at 0-1-12, 49 lb down and 83 lb up at 1-11-4, 23 lb down and 86 lb up at 3-11-4, 23 lb down and 86 lb up at 5-11-4, 23 lb down and 86 lb up at 7-11-4, 23 lb down and 86 lb up at 9-11-4, 23 lb down and 86 lb up at 11-11-4, 23 lb down and 86 lb up at 13-11-4, 23 lb down and 86 lb up at 15-11-4, 23 lb down and 86 lb up at 17-11-4, 23 lb down and 86 lb up at 19-11-4, 23 lb down and 86 lb up at 21-11-4, 49 lb down and 83 lb up at 23-11-4, 49 lb down and 83 lb up at 25-11-4, 49 lb down and 83 lb up at 27-11-4, 49 lb down and 83 lb up at 29-11-4, 49 lb down and 83 lb up at 31-11-4, 49 lb down and 83 lb up at 33-11-4, 49 lb down and 83 lb up at 35-11-4, 49 lb down and 83 lb up at 37-11-4, and 49 lb down and 83 lb up at 39-11-4, and 49 lb down and 83 lb up at 41-11-4 on top chord, and 53 lb down and 63 lb up at 0-1-12, 23 lb down and 69 lb up at 1-11-4, 30 lb down and 65 lb up at 3-11-4, 30 lb down and 65 lb up at 5-11-4, 30 lb down and 65 lb up at 7-11-4, 30 lb down and 65 lb up at 9-11-4, 30 lb down and 65 lb up at 11-11-4, 30 lb down and 65 lb up at 13-11-4, 30 lb down and 65 lb up at 15-11-4, 30 lb down and 65 lb up at 17-11-4, 30 lb down and 65 lb up at 19-11-4, 30 lb down and 65 lb up at 21-11-4, 23 lb down and 69 lb up at 23-11-4, 23 lb down and 69 lb up at 25-11-4, 23 lb down and 69 lb up at 27-11-4, 23 lb down and 69 lb up at 29-11-4, 23 lb down and 69 lb up at 31-11-4, 23 lb down and 69 lb up at 33-11-4, 23 lb down and 69 lb up at 35-11-4, 23 lb down and 69 lb up at 37-11-4, and 23 lb down and 69 lb up at 39-11-4, and 23 lb down and 69 lb up at 41-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-13=-54, 25-26=-10, 20-24=-10, 14-19=-10

Concentrated Loads (lb)

Vert: 26=-14(B) 1=-103(B) 5=-23(B) 16=-4(B) 23=-30(B) 21=-30(B) 10=-49(B) 27=-49(B) 28=-23(B) 29=-23(B) 30=-23(B) 31=-23(B) 32=-23(B) 33=-23(B) 34=-23(B) 35=-23(B) 36=-23(B) 37=-49(B) 38=-49(B) 39=-49(B) 40=-49(B) 41=-49(B) 42=-49(B) 43=-49(B) 44=-49(B) 45=-49(B) 46=-4(B) 47=-30(B) 48=-30(B) 49=-30(B) 50=-30(B) 51=-30(B) 52=-30(B) 53=-30(B) 54=-30(B) 55=-4(B) 56=-4(B) 57=-4(B) 58=-4(B) 59=-4(B) 60=-4(B) 61=-4(B) 62=-4(B) 63=-4(B)

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799443
361993	T18	SPECIAL	1	1	Job Reference (optional)	

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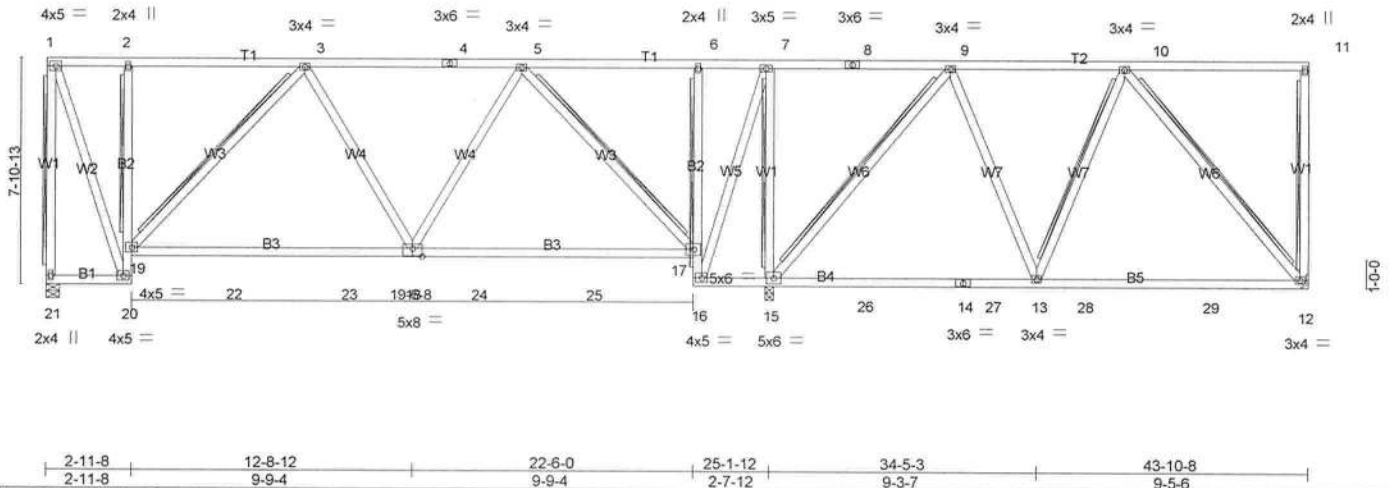
2-11-8 | 8-11-7 | 16-6-1 | 22-6-0 | 25-1-12 | 31-5-0 | 37-5-7 | 43-10-8
2-11-8 | 5-11-15 | 7-6-11 | 5-11-15 | 2-7-12 | 6-3-4 | 6-0-8 | 6-5-1

Scale = 1:76.8



2-11-8 | 12-8-12 | 22-6-0 | 25-1-12 | 34-5-3 | 43-10-8
2-11-8 | 9-9-4 | 9-9-4 | 2-7-12 | 9-3-7 | 9-5-6

Plate Offsets (X,Y): [18:0-4-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.28	18-19	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.69	Vert(TL)	-0.46	18-19	>656	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.11	12	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.09	18-19	>999	240	Weight: 307 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-4-10 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 2-19, 6-17
WEBS T-Brace: 2 X 4 SYP No.3 - 1-21, 11-12, 3-19, 5-17, 7-15, 9-15, 10-13, 10-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 21=799/0-5-8, 12=558/Mechanical, 15=2105/0-3-8
Max Uplift 21=-291(LC 4), 12=-195(LC 4), 15=-648(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-21=-842/528, 1-2=-274/170, 2-3=-312/185, 3-4=-685/379, 4-5=-685/379, 7-8=-197/478, 8-9=-197/478, 9-10=-264/134
BOT CHORD 19-20=-711/470, 19-22=-436/681, 22-23=-436/681, 18-23=-436/681, 18-24=-360/496, 24-25=-360/496, 17-25=-360/496, 16-17=-994/553, 15-16=-478/197, 13-28=-184/280, 28-29=-184/280, 12-29=-184/280
WEBS 1-20=-512/827, 3-19=-542/368, 5-18=-39/380, 5-17=-988/555, 7-16=-568/946, 7-15=-1165/785, 9-15=-928/483, 9-13=-63/414, 10-12=-415/283

NOTES (10-12)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
5) All bearings are assumed to be SYP No.2 .
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 21=291, 12=195, 15=648.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T18	SPECIAL	1	1	I4799443
					Job Reference (optional)

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LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799444
361993	T19	SPECIAL	1	1	Job Reference (optional)	

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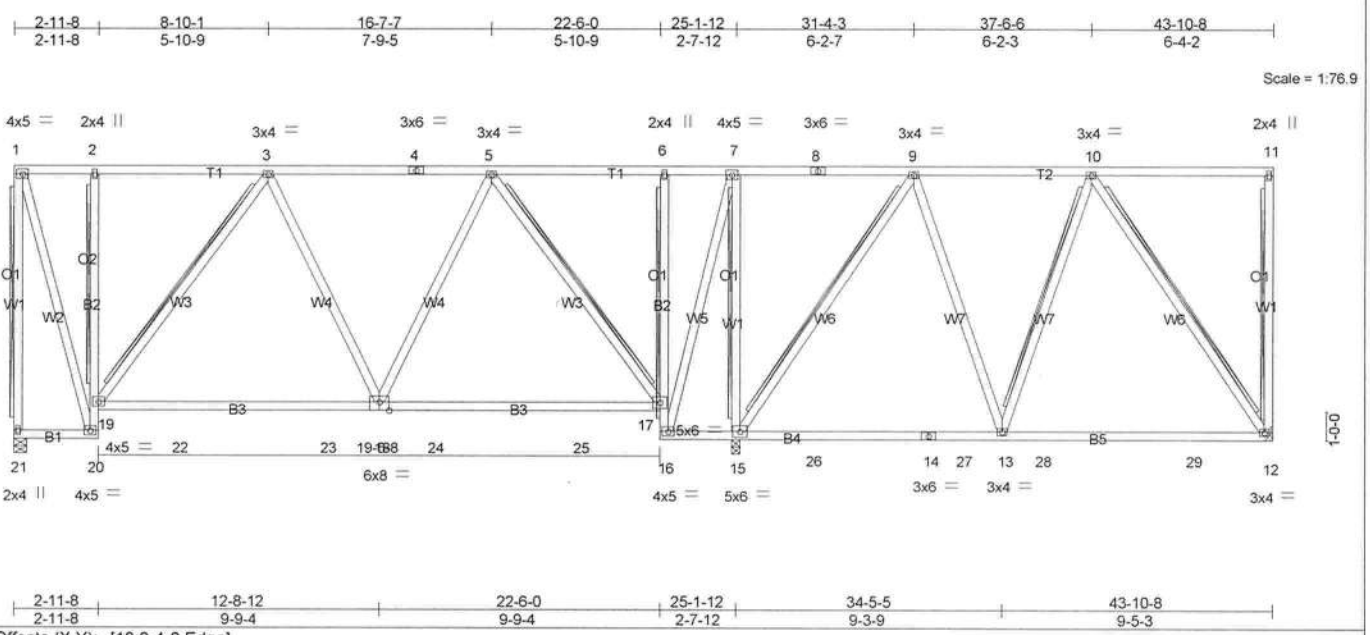


Plate Offsets (X,Y): [18:0-4-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.33	18-19	>908	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.66	Vert(TL)	-0.50	18-19	>594	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.89	Horz(TL)	0.12	12	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.08	18-19	>999	240	Weight: 341 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

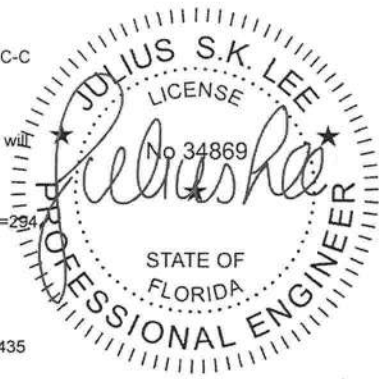
BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-2-9 oc bracing. Except:
T-Brace: 2 X 4 SYP No.3 - 2-19, 6-17
WEBS T-Brace: 2 X 4 SYP No.3 - 1-21, 11-12, 3-19, 5-17, 7-15, 9-15, 10-13, 10-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 21=829/0-5-8, 12=569/Mechanical, 15=2224/0-3-8
Max Uplift 21=-294(LC 4), 12=-199(LC 4), 15=-641(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-21=-871/528, 2-3=-259/148, 3-4=-569/308, 4-5=-569/308, 7-8=-150/435, 8-9=-150/435
BOT CHORD 19-20=-740/474, 19-22=-353/561, 22-23=-353/561, 18-23=-353/561, 18-24=-294/393, 24-25=-294/393, 17-25=-294/393, 16-17=-1052/546, 15-16=-435/150
WEBS 1-20=-502/842, 3-19=-512/347, 5-18=-33/413, 5-17=-960/507, 7-16=-553/983, 7-15=-1218/779, 9-15=-912/445, 9-13=-55/446, 10-12=-386/273

NOTES (10-12)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Provide adequate drainage to prevent water ponding.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
5) All bearings are assumed to be SYP No.2 .
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 21=294, 12=199, 15=641.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
12) Use Simpson HTU26 to attach Truss to Carrying member



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LOAD CASE(S) Standard

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T20	HIP	2	1	I4799445
					Job Reference (optional)

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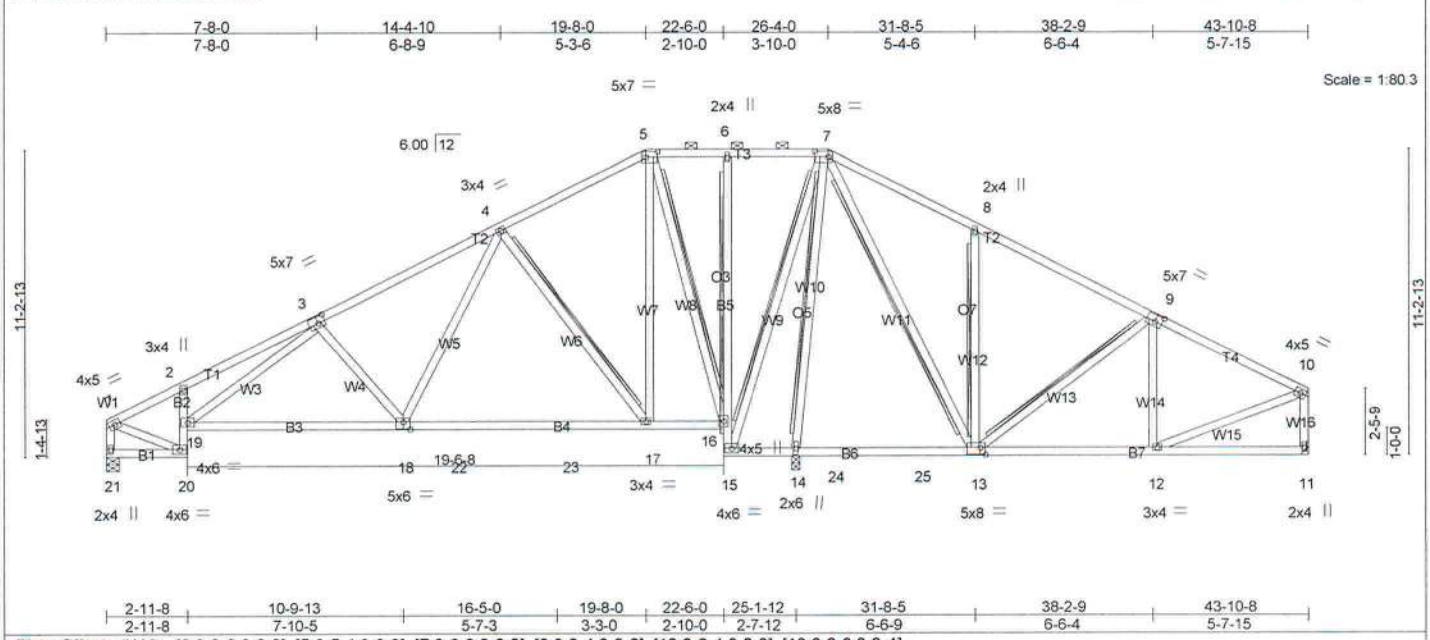

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-5-4,0-2-8], [7:0-6-0,0-2-8], [9:0-3-4,0-3-0], [13:0-2-4,0-3-0], [18:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.27	17-18	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.40	17-18	>754	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.10	14	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.16	18-19	>999	240	Weight: 323 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W1,W16: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied. Except:
T-Brace: 2 X 4 SYP No.3 - 6-16
WEBS T-Brace: 2 X 4 SYP No.3 - 4-17, 5-16, 7-15, 7-13, 8-13, 9-13
2 X 6 SYP No.2 - 7-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 21=690/0-5-8, 14=2043/0-3-8, 11=380/Mechanical
Max Horz 21=172(LC 6)
Max Uplift 21=-224(LC 6), 14=-523(LC 6), 11=-247(LC 7)
Max Grav 21=699(LC 10), 14=2043(LC 1), 11=493(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-802/614, 2-3=-1247/1097, 3-4=-869/759, 4-5=-164/380, 5-6=0/270, 6-7=0/276, 7-8=-286/499, 8-9=-291/272, 9-10=-496/354, 1-21=-741/583, 10-11=-465/359
BOT CHORD 2-19=-319/361, 18-19=-683/926, 18-22=-231/469, 22-23=-231/469, 17-23=-231/469, 15-16=-1012/665, 14-15=-484/481, 14-24=-314/349, 24-25=-314/349, 13-25=-314/349, 12-13=-232/394
WEBS 3-19=-270/234, 3-18=-353/469, 4-18=-358/538, 4-17=-654/650, 5-17=-424/706, 5-16=-903/478, 7-15=-621/950, 7-14=-1755/1213, 7-13=-683/715, 8-13=-346/463, 9-13=-309/349, 1-20=-505/691, 10-12=-226/397

NOTES (12-14)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SYP No.2 .
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 21=224, 14=523, 11=247.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

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NOTES (12-14)

10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T21	HIP	3	1	I4799446
					Job Reference (optional)

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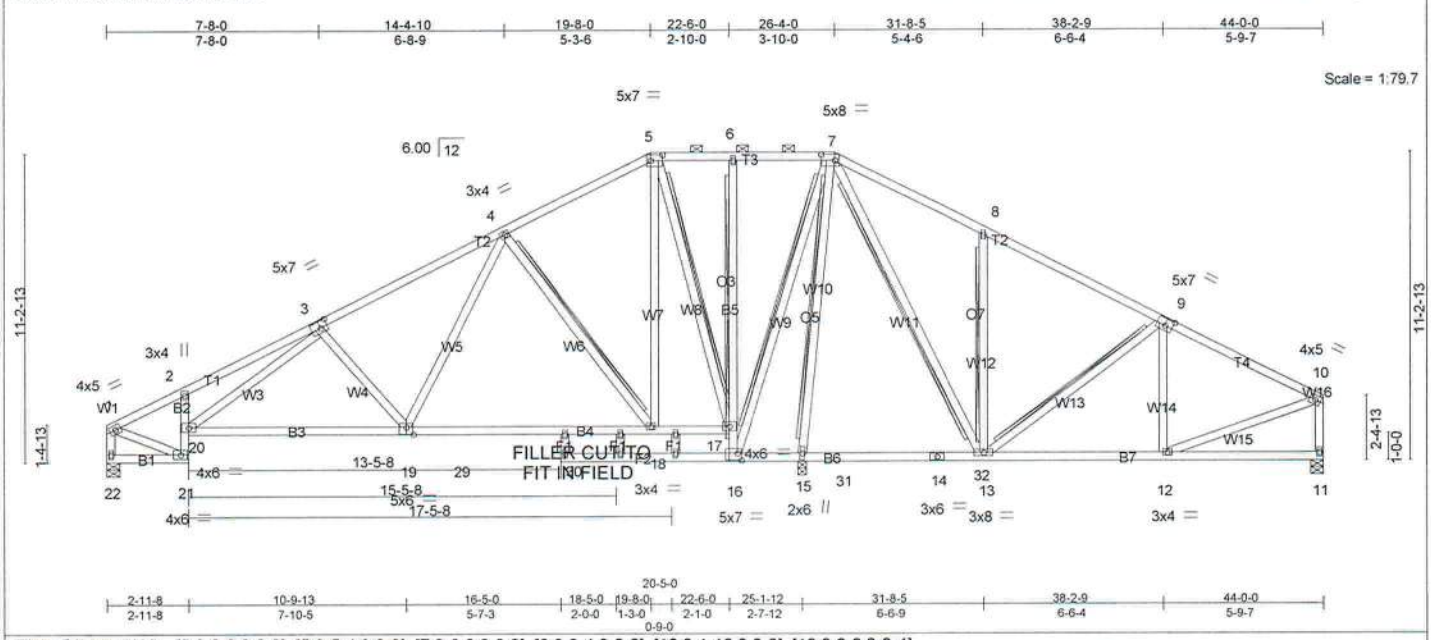

Plate Offsets (X,Y): [3:0-3-0,0-3-0], [5:0-5-4,0-2-8], [7:0-6-0,0-2-8], [9:0-3-4,0-3-0], [16:0-1-12,0-3-0], [19:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.47	Vert(LL)	-0.27	18-19	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.40	18-19	>754	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.76	Horz(TL)	0.10	15	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.16	19-20	>999	240	Weight: 336 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W1,W16: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-4-0 oc purlins, except end verticals, and 2-0-0 oc purlins (10-0-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied. Except:
T-Brace: 2 X 4 SYP No.3 - 6-17
5-2-0 oc bracing: 16-17
WEBS T-Brace: 2 X 4 SYP No.3 - 4-18, 5-17, 7-16, 7-13, 8-13, 9-13
2 X 6 SYP No.2 - 7-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 22=689/0-5-8, 15=2047/0-3-8, 11=384/0-5-8
Max Horz 22=171(LC 5)
Max Uplift 22=-224(LC 6), 15=-522(LC 6), 11=-249(LC 7)
Max Grav 22=698(LC 10), 15=2047(LC 1), 11=496(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-801/615, 2-3=-1247/1096, 3-4=-868/760, 4-5=-164/382, 5-6=0/272, 6-7=0/277, 7-8=-287/503, 8-9=-292/276, 9-10=-507/363, 1-22=-741/583, 10-11=-468/364
BOT CHORD 2-20=-319/360, 19-20=-680/926, 19-29=-228/469, 29-30=-228/469, 18-30=-228/469, 16-17=-1012/664, 15-16=-486/483, 15-31=-314/351, 31-32=-314/351, 14-32=-314/351, 13-14=-314/351, 12-13=-238/402
WEBS 3-20=-267/234, 3-19=-353/469, 4-19=-357/538, 4-18=-654/650, 5-18=-424/706, 5-17=-903/476, 7-16=-620/950, 7-15=-1759/1214, 7-13=-687/719, 8-13=-345/461, 9-13=-317/356, 1-21=-506/691, 10-12=-228/400

NOTES (12-13)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C&C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) All plates are 2x4 MT20 unless otherwise indicated.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
7) All bearings are assumed to be SYP No.2 .
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 22=224, 15=522, 11=249.

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Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T21	HIP	3	1	I4799446
					Job Reference (optional)

NOTES (12-13)

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799447
361993	T22	HIP	1	1	Job Reference (optional)	

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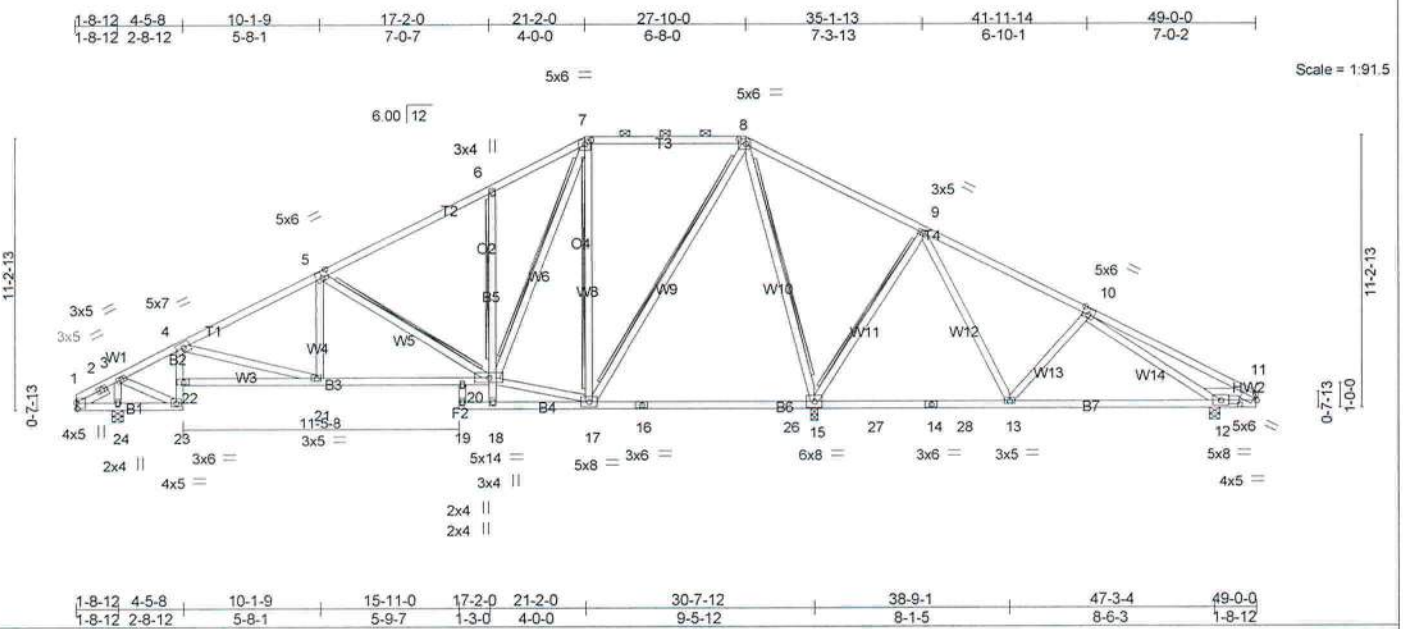

Plate Offsets (X,Y): [1:0-2-14,0-0-8], [5:0-3-0,0-3-4], [7:0-3-0,0-2-0], [8:0-3-8,0-2-4], [10:0-3-0,0-3-4], [11:0-1-10,0-2-6], [11:0-8-5,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.25	15-17	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.36	15-17	>964	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.12	15	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.12	21-22	>999	240	Weight: 324 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2: 2 X 4 SYP No.1D, B5: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W9: 2 X 4 SYP No.2
SLIDER Left 2 X 4 SYP No.2 1-5-5, Right 2 X 6 SYP No.1D 2-2-1

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-14 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 7-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
7-8-13 oc bracing: 21-22
9-3-13 oc bracing: 20-21
10-0-0 oc bracing: 18-19.
T-Brace: 2 X 4 SYP No.3 - 6-20
6-0-0 oc bracing: 18-20
WEBS T-Brace: 2 X 4 SYP No.3 - 5-20, 7-20, 7-17, 8-17, 9-15
2 X 6 SYP No.2 - 8-15
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 24=848/0-5-8, 15=2415/0-3-8, 12=278/0-5-8
Max Horz 24=177(LC 5)
Max Uplift 24=-339(LC 6), 15=-596(LC 6), 12=-243(LC 7)
Max Grav 24=877(LC 10), 15=2415(LC 1), 12=434(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-824/579, 4-5=-1165/798, 5-6=-685/534, 6-7=-639/728, 7-8=-262/455, 8-9=-278/853, 9-10=-174/374
BOT CHORD 22-23=-259/214, 21-22=-660/1136, 20-21=-461/986, 6-20=-303/417, 16-17=-290/566, 16-26=-290/566, 15-26=-290/566, 15-27=-438/475, 14-27=-438/475, 14-28=-438/475, 13-28=-438/475, 12-13=-226/273, 11-12=-112/314
WEBS 3-24=-927/724, 3-23=-552/780, 5-21=-14/256, 5-20=-534/541, 17-20=0/325, 7-20=-642/701, 7-17=-671/515, 8-17=-625/975, 8-15=-1604/1055, 9-15=-626/671, 9-13=-307/435, 10-13=-330/438, 10-12=-457/339

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SYP No.2 .

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Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T22	HIP	1	1	I4799447

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NOTES (11-12)

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 24=339, 15=596, 12=243.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799448
361993	T23	HIP	1	1	Job Reference (optional)	

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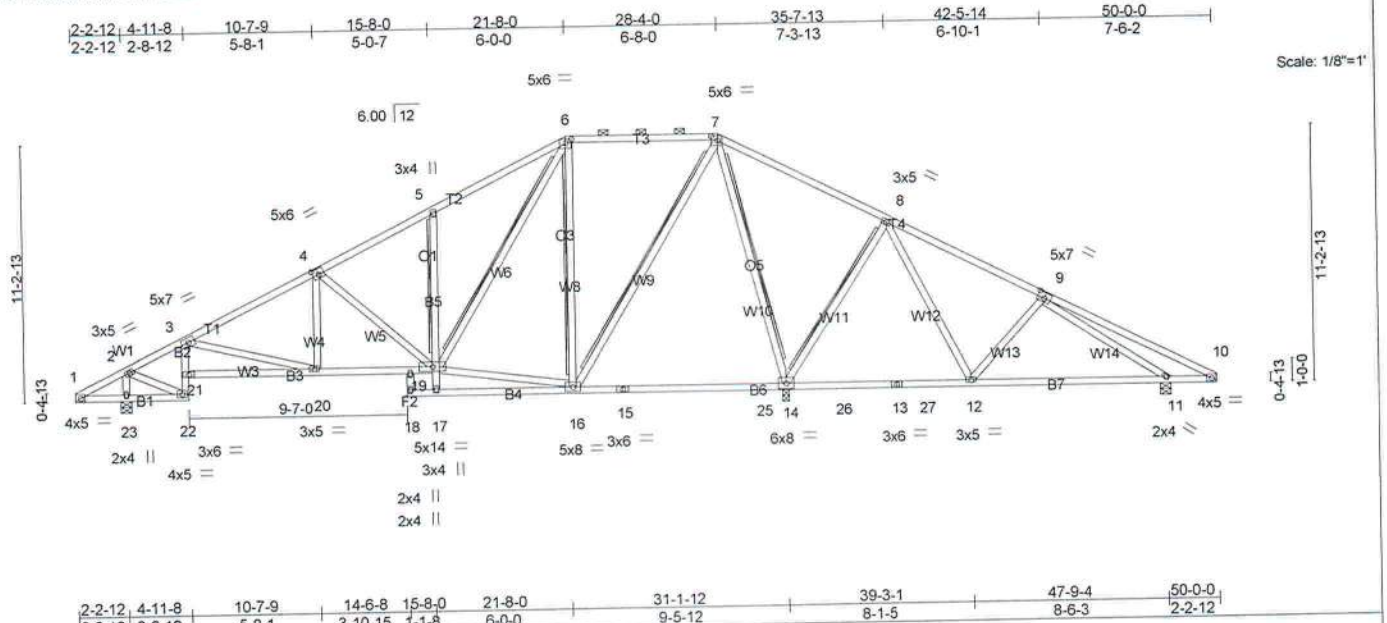

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [6:0-3-0,0-2-0], [7:0-3-8,0-2-4], [9:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.27	14-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.38	14-16	>923	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.12	14	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.13	18	>999	240	Weight: 320 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B5: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W9: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-8-7 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-7.

BOT CHORD Rigid ceiling directly applied. Except:
T-Brace: 2 X 4 SYP No.3 - 5-19
10-0-0 oc bracing: 17-19

WEBS T-Brace: 2 X 4 SYP No.3 - 6-19, 6-16, 7-16, 8-14
2 X 6 SYP No.2 - 7-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 23=878/0-5-8, 14=2418/0-3-8, 11=306/0-5-8
Max Horz 23=-177(LC 4)
Max Uplift 23=-370(LC 6), 14=-593(LC 6), 11=-274(LC 7)
Max Grav 23=909(LC 10), 14=2418(LC 1), 11=463(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-801/545, 3-4=-1135/767, 4-5=-778/617, 5-6=-787/826, 6-7=-255/455, 7-8=-279/867, 8-9=-161/382, 9-10=-345/324

BOT CHORD 21-22=-277/242, 3-21=-265/265, 20-21=-627/1114, 19-20=-423/952, 5-19=-301/411, 15-16=-301/568, 15-25=-301/568, 14-25=-301/568, 14-26=-445/481, 13-26=-445/481, 13-27=-445/481, 12-27=-445/481, 11-12=-234/262, 10-11=-200/406

WEBS 2-23=-949/751, 2-22=-603/820, 4-19=-414/408, 16-19=-3/294, 6-19=-697/737, 6-16=-668/551, 7-16=-629/977, 7-14=-1608/1053, 8-14=-621/663, 8-12=-303/436, 9-12=-334/440, 9-11=-556/427

NOTES (11-12)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SYP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 23=370, 14=593, 11=274.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

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June 24,2011

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.
361993	T23	HIP	1	1	I4799448
					Job Reference (optional)

NOTES (11-12)

9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Your Company Name

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799449
361993	T24	HIP	3	1	Job Reference (optional)	

Builders FrstSource, Lake City, FL 32055 — 7.140 s Oct 1 2009 MiTek Industries, Inc. Fri Jun 24 14:58:26 2011 Page 1

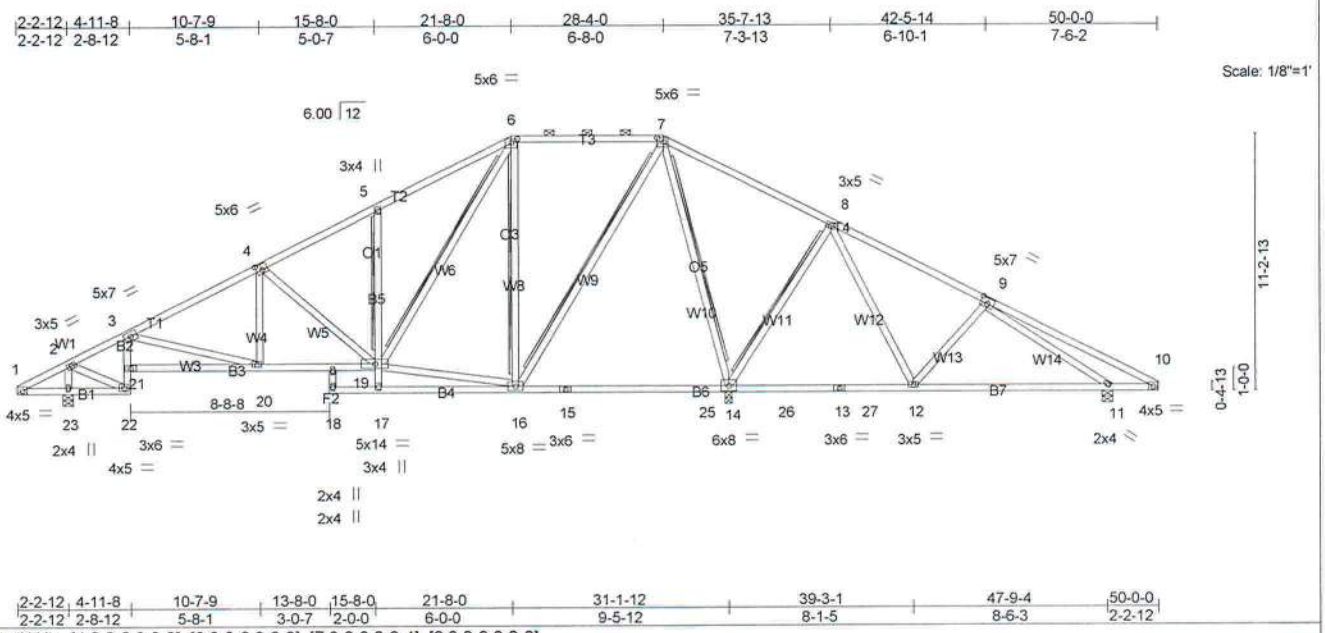

Plate Offsets (X,Y): [4:0-3-0,0-3-0], [6:0-3-0,0-2-0], [7:0-3-8,0-2-4], [9:0-3-8,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.27	14-16	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.78	Vert(TL)	-0.38	14-16	>921	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.73	Horz(TL)	0.12	14	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.13	18	>999	240	Weight: 322 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B2: 2 X 4 SYP No.1D, B5: 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3 *Except*
W9: 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-7-15 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 6-7.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
7-10-14 oc bracing: 20-21
9-9-2 oc bracing: 19-20
10-0-0 oc bracing: 17-18.
T-Brace: 2 X 4 SYP No.3 - 5-19
10-0-0 oc bracing: 17-19
WEBS T-Brace: 2 X 4 SYP No.3 - 6-19, 6-16, 7-16, 8-14
2 X 6 SYP No.2 - 7-14
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 23=887/0-5-8, 14=2411/0-3-8, 11=313/0-5-8
Max Horz 23=-177(LC 4)
Max Uplift 23=-371(LC 6), 14=-582(LC 6), 11=-275(LC 7)
Max Grav 23=918(LC 10), 14=2411(LC 1), 11=464(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-809/543, 3-4=-1155/768, 4-5=-798/619, 5-6=-807/827, 6-7=-266/461, 7-8=-265/854, 8-9=-163/369, 9-10=-345/324
BOT CHORD 21-22=-283/244, 3-21=-271/266, 20-21=-632/1135, 19-20=-424/970, 5-19=-302/411, 15-16=-291/559, 15-25=-291/559, 14-25=-291/559, 14-26=-433/469, 13-26=-433/469, 13-27=-433/469, 12-27=-433/469, 11-12=-224/264, 10-11=-200/406
WEBS 2-23=-953/748, 2-22=-600/827, 4-19=-413/408, 16-19=0/315, 6-19=-690/752, 6-16=-672/545, 7-16=-622/977, 7-14=-1602/1040, 8-14=-620/662, 8-12=-302/434, 9-12=-332/438, 9-11=-558/439

NOTES (11-12)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SYP No.2 .

Continued on page 2

June 24,2011

NOTES (11-12)

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 23=371, 14=582, 11=275.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799450
361993	T25	HIP	1	1	Job Reference (optional)	

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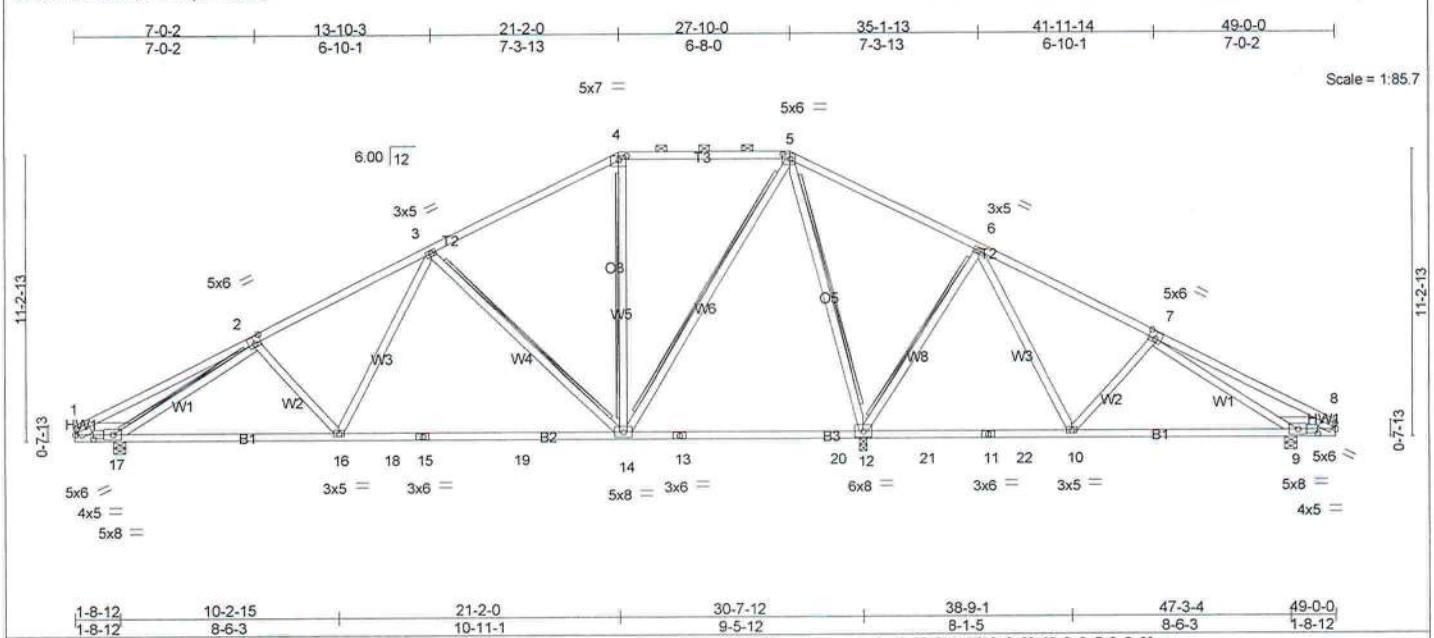

Plate Offsets (X,Y): [1:0-1-10,0-2-6], [1:0-8-5,0-2-0], [2:0-3-0,0-3-4], [4:0-3-8,0-1-12], [5:0-3-8,0-2-4], [7:0-3-0,0-3-4], [8:0-1-10,0-2-6], [8:0-8-5,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.39	14-16	>902	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.48	Vert(TL)	-0.58	14-16	>607	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.69	Horz(TL)	0.03	12	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.09	14-16	>999	240	Weight: 299 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3 *Except*
W6: 2 X 4 SYP No.2
SLIDER Left 2 X 6 SYP No.1D 2-2-1, Right 2 X 6 SYP No.1D 2-2-1

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-2-9 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
8-6-11 oc bracing: 16-17
10-0-0 oc bracing: 14-16.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-14, 4-14, 5-14, 6-12, 2-17
2 X 6 SYP No.2 - 5-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.
Brace must cover 90% of web length.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 12=2299/0-3-8, 17=1021/0-5-8, 9=409/0-5-8
Max Horz 17=177(LC 5)
Max Uplift 12=-518(LC 7), 17=-369(LC 6), 9=-226(LC 7)
Max Grav 12=2299(LC 1), 17=1053(LC 10), 9=483(LC 11)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1307/845, 3-4=-656/510, 4-5=-500/549, 5-6=-122/598, 6-7=-262/167
BOT CHORD 1-17=-131/265, 16-17=-602/1172, 16-18=-306/939, 15-18=-306/939, 15-19=-306/939, 14-19=-306/939, 13-14=-84/456, 13-20=-84/456, 12-20=-84/456, 12-21=-207/342, 11-21=-207/342, 11-22=-207/342, 10-22=-207/342, 9-10=-60/343, 8-9=-105/304
WEBS 2-16=-123/321, 3-16=-191/363, 3-14=-616/636, 5-14=-594/1023, 6-12=-613/661, 6-10=-287/412, 7-10=-295/418, 2-17=-1588/1059, 7-9=-541/445, 5-12=-1519/942

NOTES (11-12)

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) All bearings are assumed to be SYP No.2 .
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=518, 17=369, 9=226.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

Continued on page 2

JULIUS S.K. LEE LICENSE No 34869 STATE OF FLORIDA PROFESSIONAL ENGINEER

June 24,2011

Your Company Name

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	O'NEIL CONST. - NELSON RES.	I4799451
361993	T26	KINGPOST	4	1	Job Reference (optional)	

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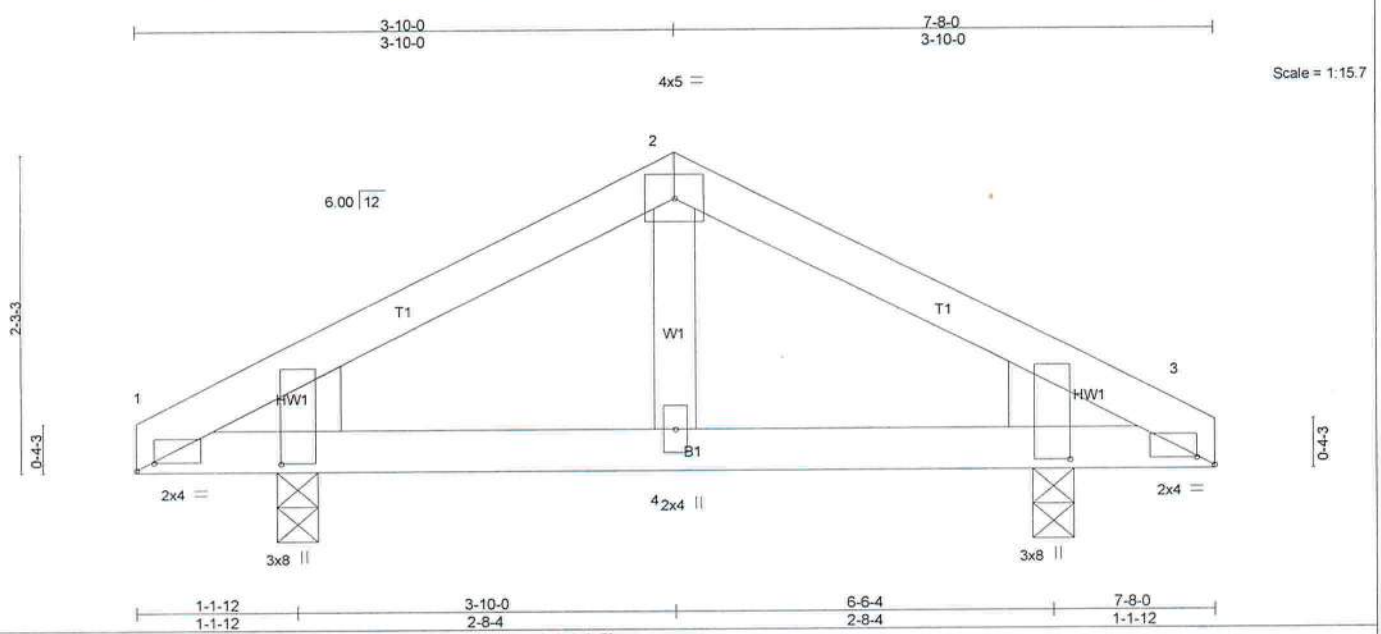


Plate Offsets (X,Y): [1:0-1-8,0-0-10], [1:0-0-8,1-0-5], [3:0-1-8,0-0-10], [3:0-0-8,1-0-5]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.55	Vert(LL)	-0.01	3-4	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.73	Vert(TL)	-0.01	3-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Wind(LL)	0.03	3-4	>999	240	Weight: 31 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 6 SYP No.1D, Right: 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-5-7 oc bracing.

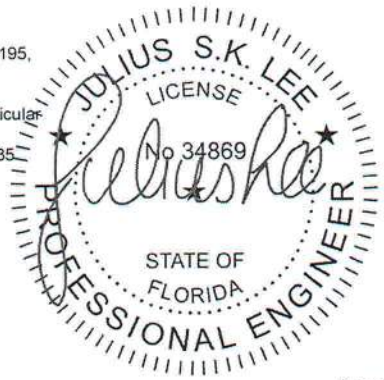
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=236/0-3-8, 3=236/0-3-8
Max Horz 1=-32(LC 4)
Max Uplift 1=-195(LC 6), 3=-195(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-332/678, 2-3=-332/678
BOT CHORD 1-4=-521/255, 3-4=-521/255
WEBS 2-4=-336/133

NOTES (8-9)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=18ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) All bearings are assumed to be SYP No.2 .
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=195, 3=195.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

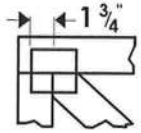


June 24,2011

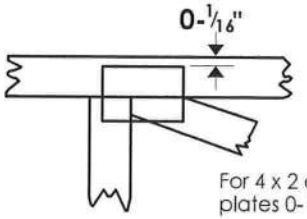
Your Company Name

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

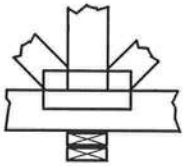
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

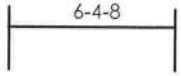


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

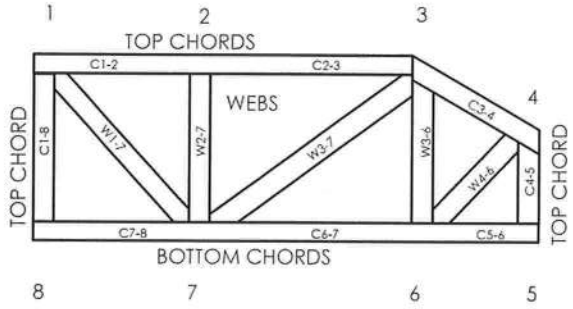
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

Your Company Information and logo

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

R

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

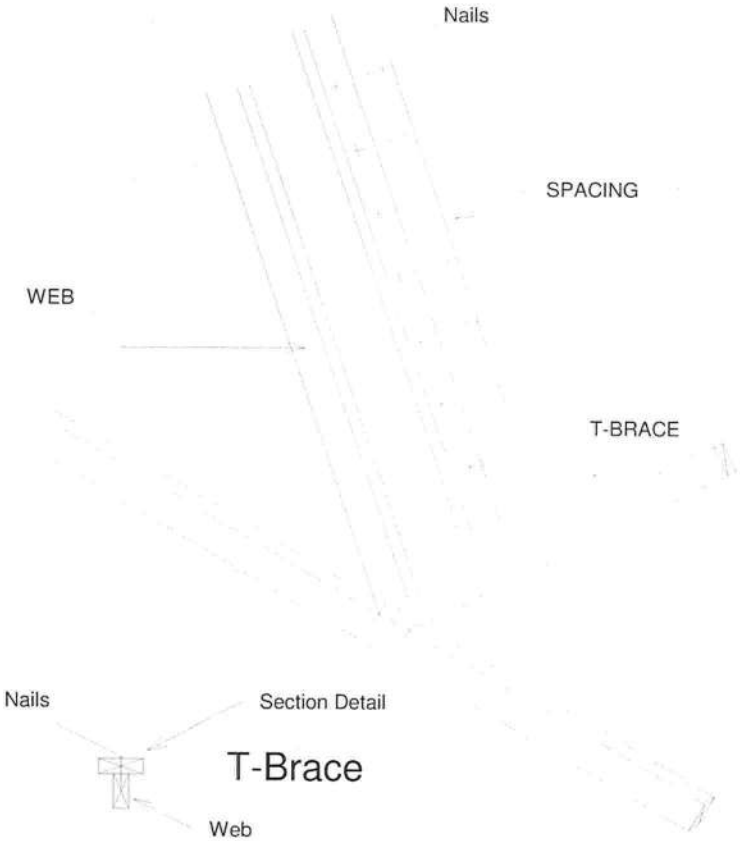
Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

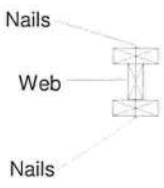
Nailing Pattern		
T-Brace size	Nail Size	Nail Spacing
2x4 or 2x6 or 2x8	10d	6" o.c.
Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)		

	Brace Size for One-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace Size for Two-Ply Truss	
	Specified Continuous Rows of Lateral Bracing	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

T-Brace / I-Brace must be same species and grade (or better) as web member.





I-Brace

JULIUS S.K. LEE LICENSE No 34869 6/22/11 STATE OF FLORIDA PROFESSIONAL ENGINEER

1109 COASTAL BAY
BOYNTON BC, FL 33435

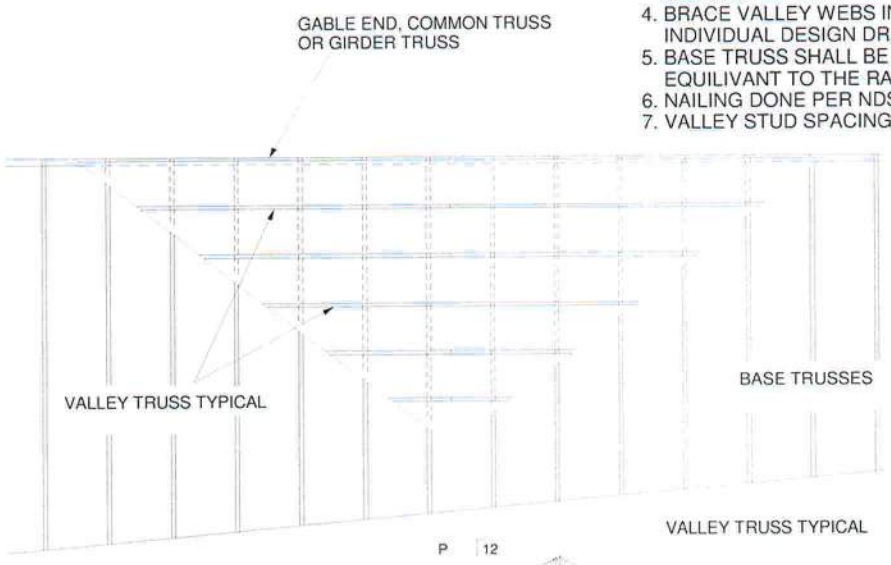
OCTOBER 25, 2010	TRUSSED VALLEY SET DETAIL	ST-VALLEY HIGH WIND1

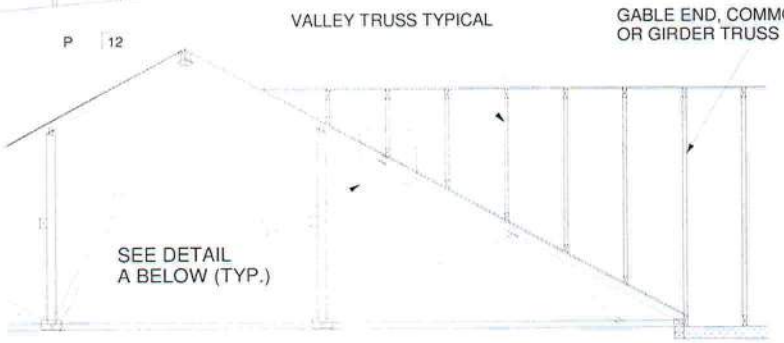
R

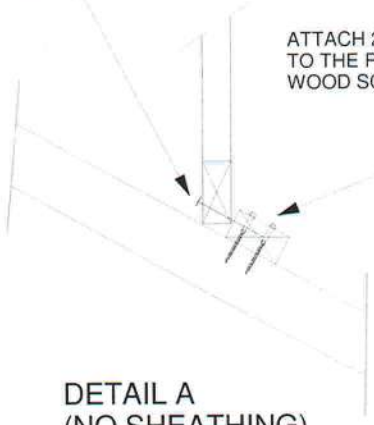
MiTek Industries, Inc.

GENERAL SPECIFICATIONS

1. NAIL SIZE = 3" X 0.131" = 10d
2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT
   DO NOT USE DRYWALL OR DECKING TYPE SCREW
3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND
   SECURE PER DETAIL A
4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE
   INDIVIDUAL DESIGN DRAWINGS.
5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING
   EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.
6. NAILING DONE PER NDS - 01
7. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.







DETAIL A
(NO SHEATHING)
N.T.S.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05
MAXIMUM WIND SPEED = 146 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12
CATEGORY II BUILDING
EXPOSURE C
WIND DURATION OF LOAD INCREASE : 1.60
MAX TOP CHORD TOTAL LOAD = 50 PSF
MAX SPACING = 24" O.C. (BASE AND VALLEY)
MINIMUM REDUCED DEAD LOAD OF 6 PSF
ON THE TRUSSES

1109 COASTAL BAY
BOYNTON BC, FL 33435

MiTek Industries, Inc.

NOTES:
1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2001 (lb/nail)					
	DIAM.	SYP	DF	HF	SPF	SPF-S
3.5" LONG	.131	88.0	80.6	69.9	68.4	59.7
	.135	93.5	85.6	74.2	72.6	63.4
	.162	108.8	99.6	86.4	84.5	73.8
3.25" LONG	.128	74.2	67.9	58.9	57.6	50.3
	.131	75.9	69.5	60.3	59.0	51.1
	.148	81.4	74.5	64.6	63.2	52.5

VALUES SHOWN ARE CAPACITY PER TOE-NAIL.
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

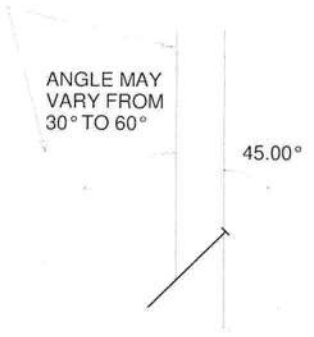
SIDE VIEW

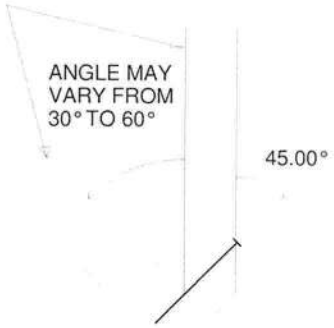
3 NAILS

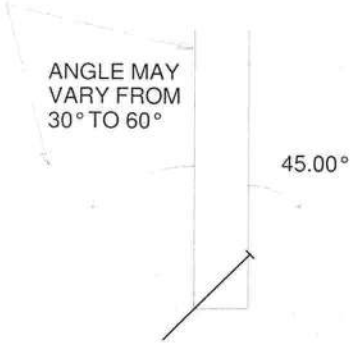
NEAR SIDE
NEAR SIDE
NEAR SIDE

EXAMPLE:
(3) - 16d NAILS (.162" diam. x 3.5") WITH SPF SPECIES BOTTOM CHORD

For load duration increase of 1.15:
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity







1109 COASTAL BAY
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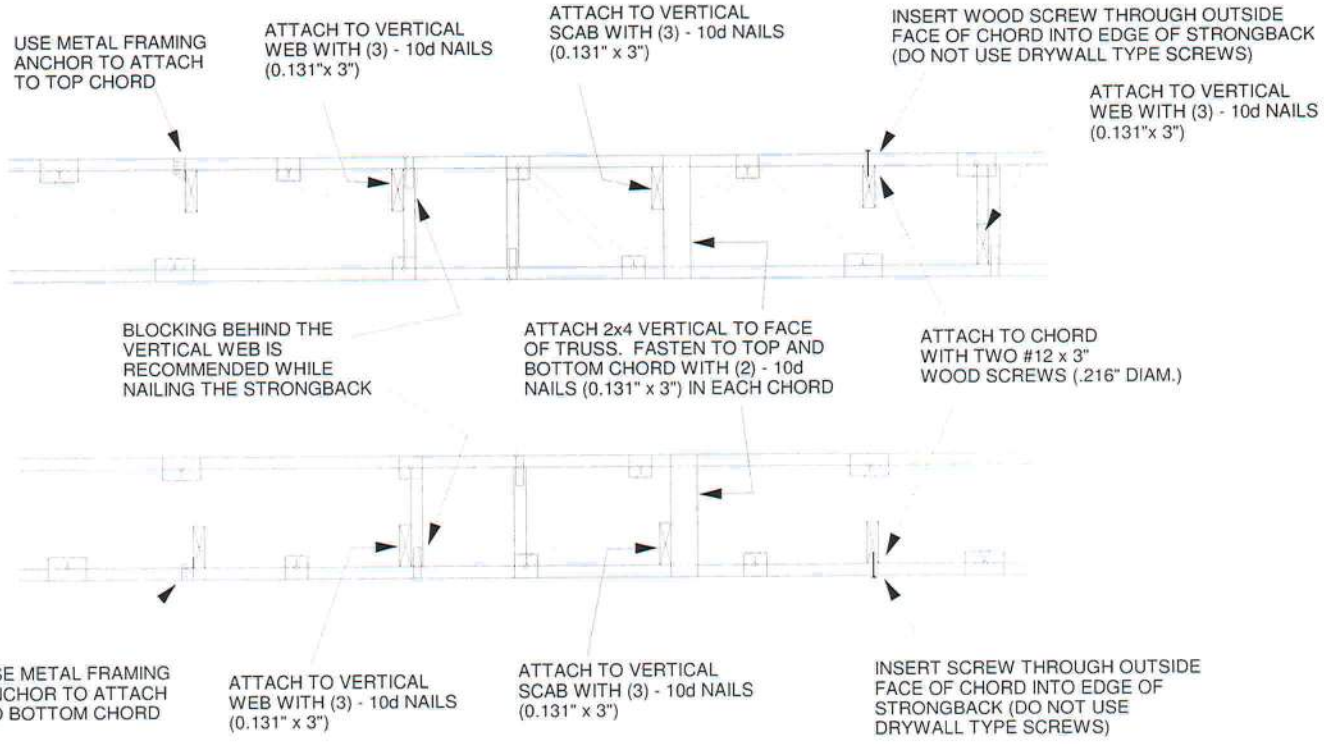
FEBRUARY 8, 2008	LATERAL BRACING RECOMMENDATIONS	ST-STRGBCK

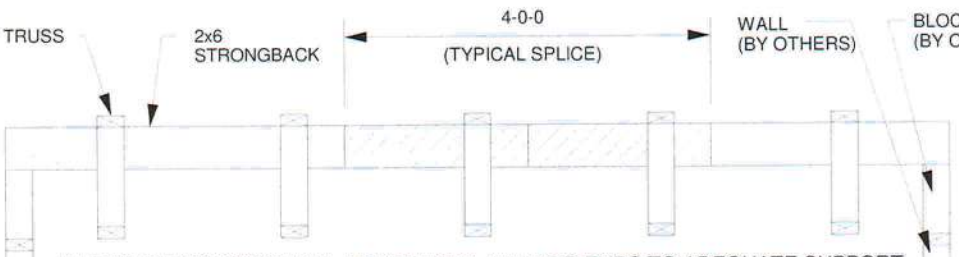
MiTek Industries, Inc.

TO MINIMIZE VIBRATION COMMON TO ALL SHALLOW FRAMING SYSTEMS, 2x6 "STRONGBACK" IS RECOMMENDED, LOCATED EVERY 8 TO 10 FEET ALONG A FLOOR TRUSS.

NOTE 1: 2X6 STRONGBACK ORIENTED VERTICALLY MAY BE POSITIONED DIRECTLY UNDER THE TOP CHORD OR DIRECTLY ABOVE THE BOTTOM CHORD. SECURELY FASTENED TO THE TRUSS USING ANY OF THE METHODS ILLUSTRATED BELOW.

NOTE 2: STRONGBACK BRACING ALSO SATISFIES THE LATERAL BRACING REQUIREMENTS FOR THE BOTTOM CHORD OF THE TRUSS WHEN IT IS PLACED ON TOP OF THE BOTTOM CHORD, IS CONTINUOUS FROM END TO END, CONNECTED WITH A METHOD OTHER THAN METAL FRAMING ANCHOR, AND PROPERLY CONNECTED, BY OTHERS, AT THE ENDS.





THE STRONGBACKS SHALL BE SECURED AT THEIR ENDS TO ADEQUATE SUPPORT, DESIGNED BY OTHERS. IF SPLICING IS NECESSARY, USE A 4'-0" LONG SCAB CENTERED ON THE SPLICE AND JOINED WITH (12) - 10d NAILS (0.131" x 3") EQUALLY SPACED.

ALTERNATE METHOD OF SPLICING:
OVERLAP STRONGBACK MEMBERS A MINIMUM OF 4'-0" AND FASTEN WITH (12) - 10d NAILS (0.131" x 3") STAGGERED AND EQUALLY SPACED.
(TO BE USED ONLY WHEN STRONGBACK IS NOT ALIGNED WITH A VERTICAL)

1109 COASTAL BAY
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JANUARY 20, 2011

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY

MiTek Industries, Chesterfield, MO

MiTek Industries, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C ASCE 7-02, ASCE 7-05 DURATION OF LOAD INCREASE : 1.60
DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

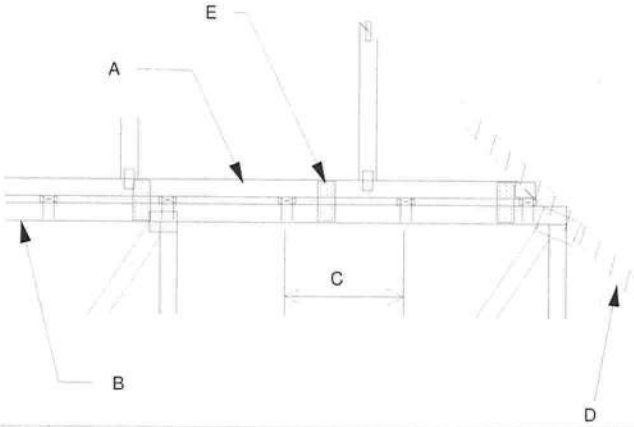
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) 0.131" X 3.5" TOE NAILED.

B - BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

C - PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) 0.131" X 3.5" NAILS EACH.

D - 2 X __ X 4'-0" SCAB, SIZE AND GRADE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF 0.131" X 3" NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
1. WIND SPEED OF 90 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR
2. WIND SPEED OF 91 MPH TO 140 MPH WITH A MAXIMUM PIGGYBACK SPAN OF 12 ft.

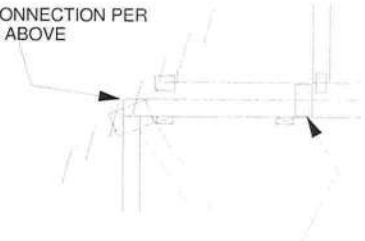
E - FOR WIND SPEEDS BETWEEN 101 AND 140 MPH, ATTACH MITEK 3X8 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)



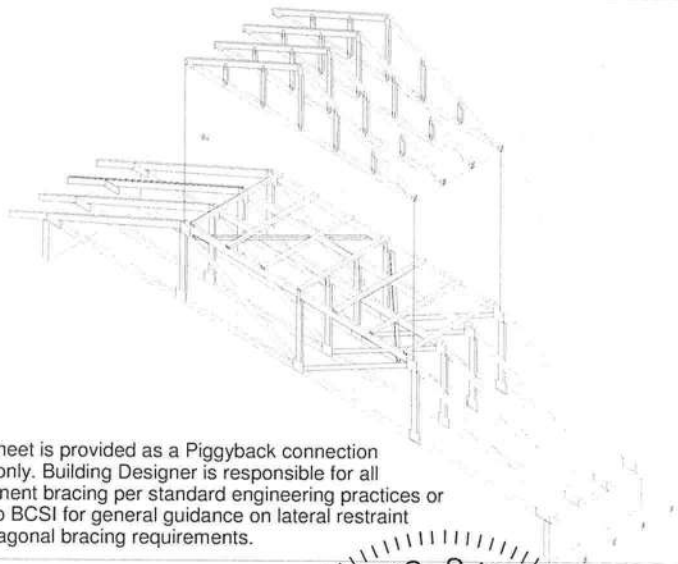
WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.

SCAB CONNECTION PER NOTE D ABOVE

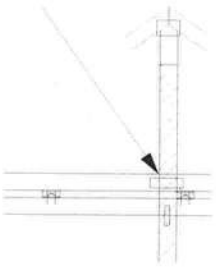


FOR ALL WIND SPEEDS, ATTACH MITEK 3X6 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) 0.131" X 1.5" PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" EDGE DISTANCE.



This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
2) ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
3) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.

JULIUS S.K. LEE
LICENSE
No 34869
6/22/11
STATE OF FLORIDA
PROFESSIONAL ENGINEER

1109 COASTAL BAY
BOYNTON BC, FL 33435

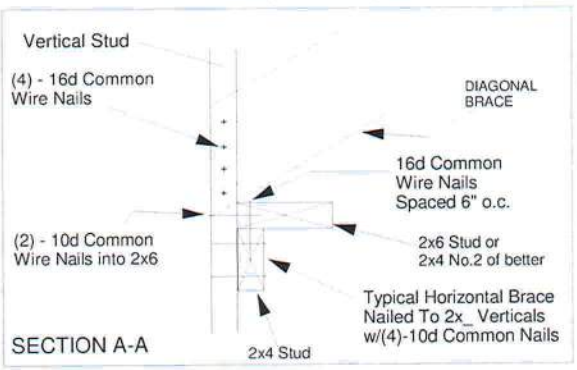
JANUARY 1, 2009

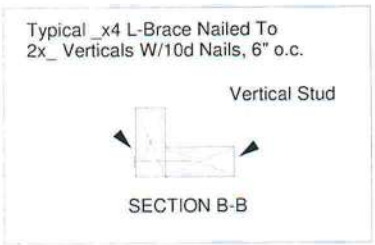
Standard Gable End Detail

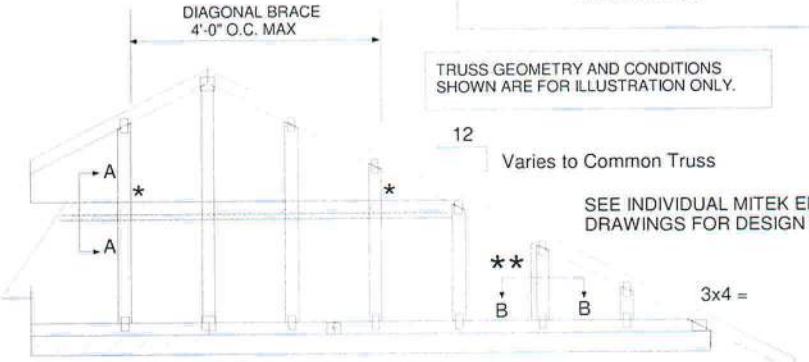
ST-GE120-001

MiTek Industries, Inc.

MiTek Industries, Chesterfield, MO







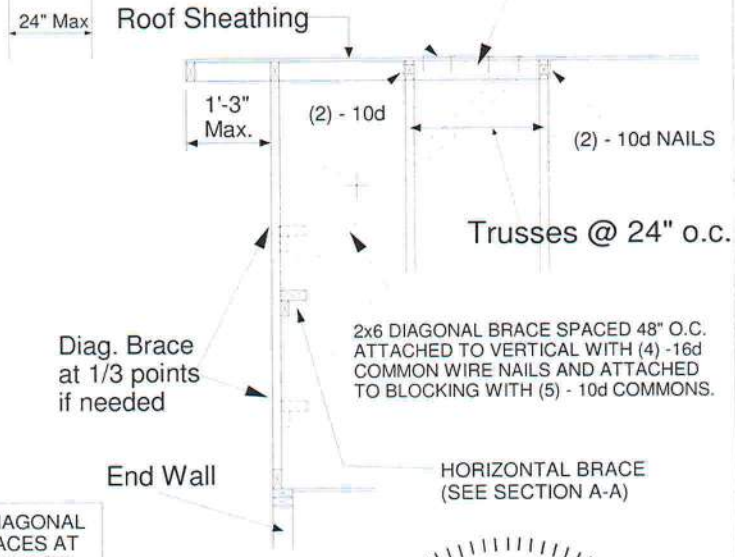
SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d COMMON WIRE NAILS.

(4) - 8d NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B



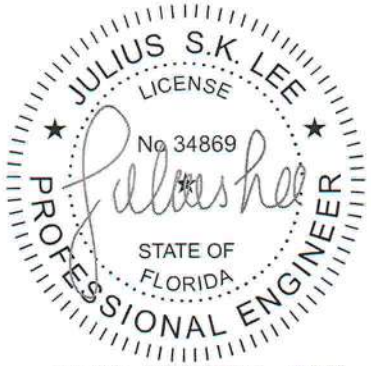
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. *GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	4-3-2	4-7-6	6-6-5	8-6-3	12-9-6
2x4 SPF Std/Stud	16" O.C.	3-10-7	4-0-0	5-7-13	7-8-14	11-7-5
2x4 SPF Std/Stud	24" O.C.	3-2-0	3-3-2	4-7-6	6-4-0	9-6-0

+ Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 120 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.



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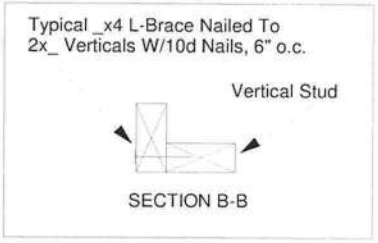
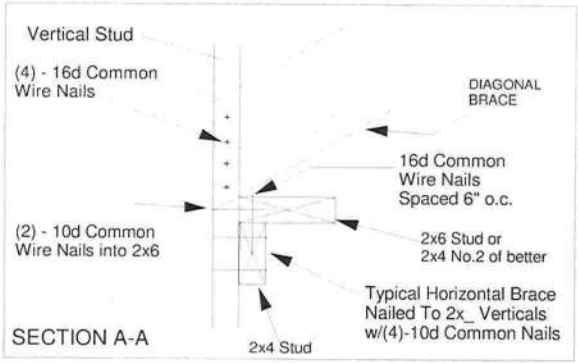
JANUARY 1, 2009	Standard Gable End Detail	ST-GE140-001

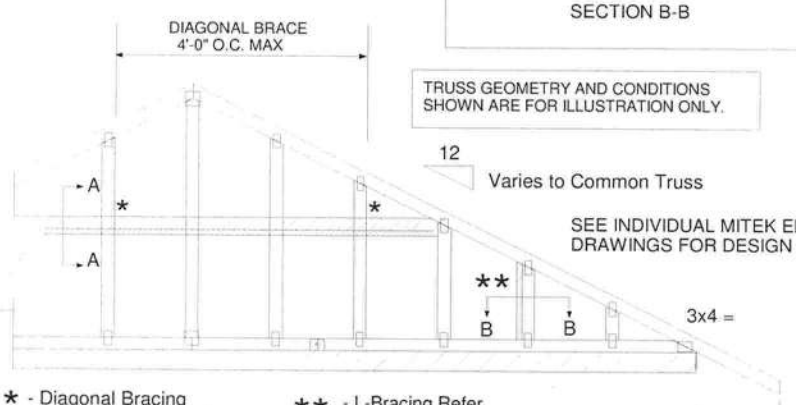
R

MiTek Industries, Inc.



MiTek Industries, Chesterfield, MO





TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

12

Varies to Common Truss

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d COMMON WIRE NAILS.

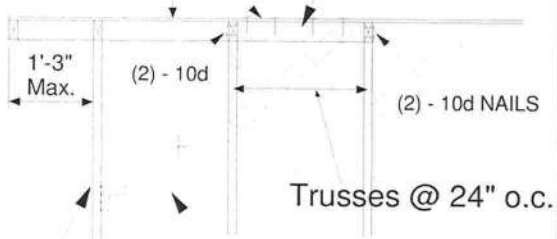
(4) - 8d NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

* - Diagonal Bracing Refer to Section A-A

** - L-Bracing Refer to Section B-B

24" Max

Roof Sheathing



Trusses @ 24" o.c.

Diag. Brace at 1/3 points if needed

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d COMMON WIRE NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d COMMONS.

End Wall

HORIZONTAL BRACE (SEE SECTION A-A)

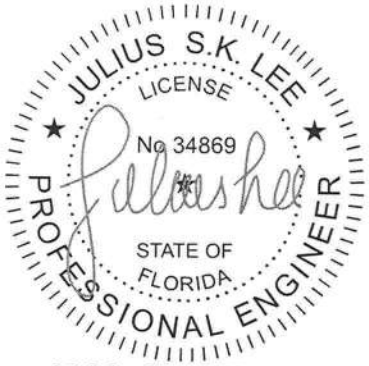
NOTE:
1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS.
2. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.
3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY. CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C.
6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
7. GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
8. THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
9. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SPF Std/Stud	12" O.C.	3-10-1	3-11-7	5-7-2	7-8-2	11-6-4
2x4 SPF Std/Stud	16" O.C.	3-3-14	3-5-1	4-10-2	6-7-13	9-11-11
2x4 SPF Std/Stud	24" O.C.	2-8-9	2-9-8	3-11-7	5-5-2	8-1-12

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of web with 10d common wire nails 8in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length.

MAXIMUM WIND SPEED = 140 MPH
MAX MEAN ROOF HEIGHT = 30 FEET
CATEGORY II BUILDING
EXPOSURE B or C
ASCE 7-98, ASCE 7-02, ASCE 7-05
DURATION OF LOAD INCREASE : 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MWFRS.



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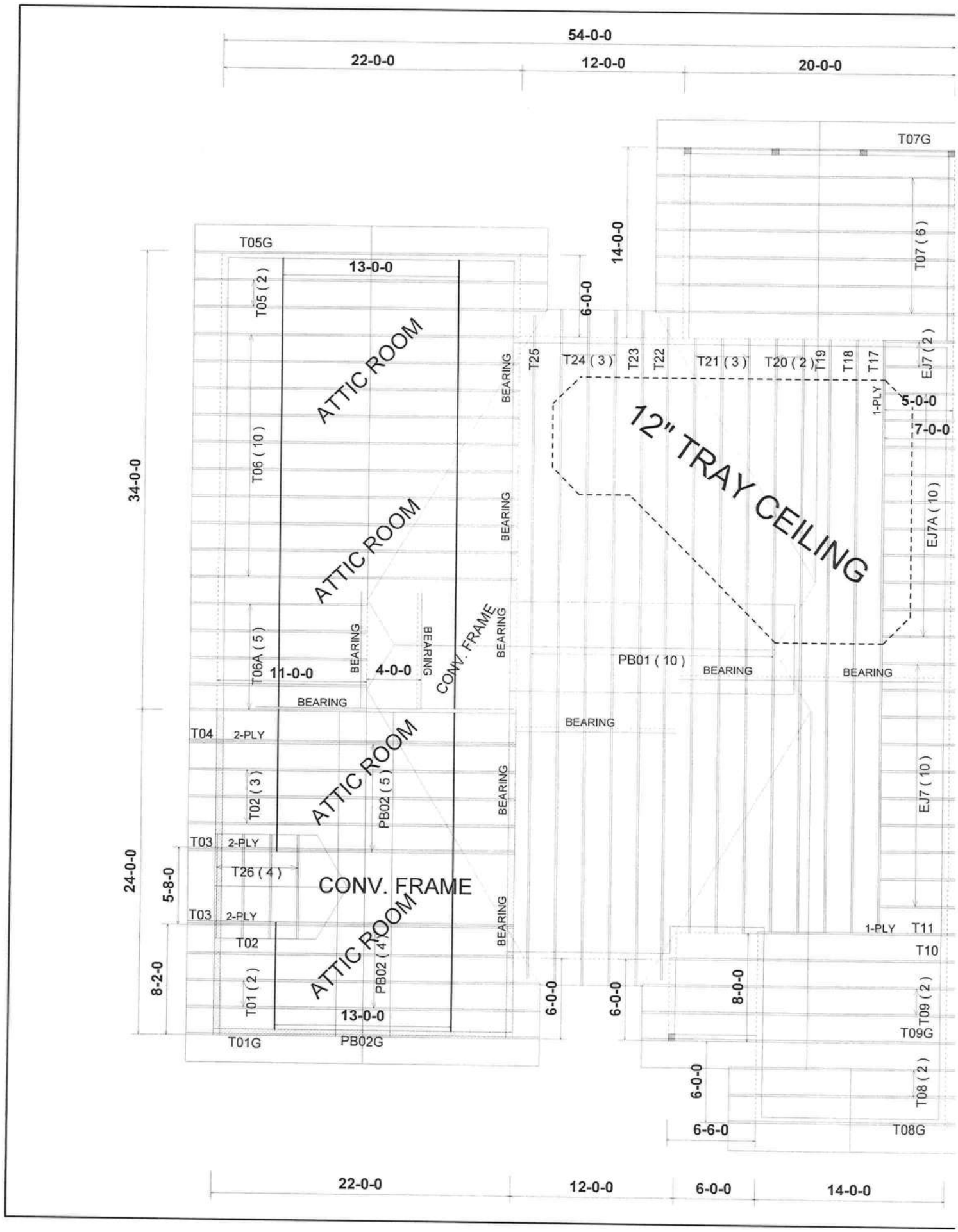
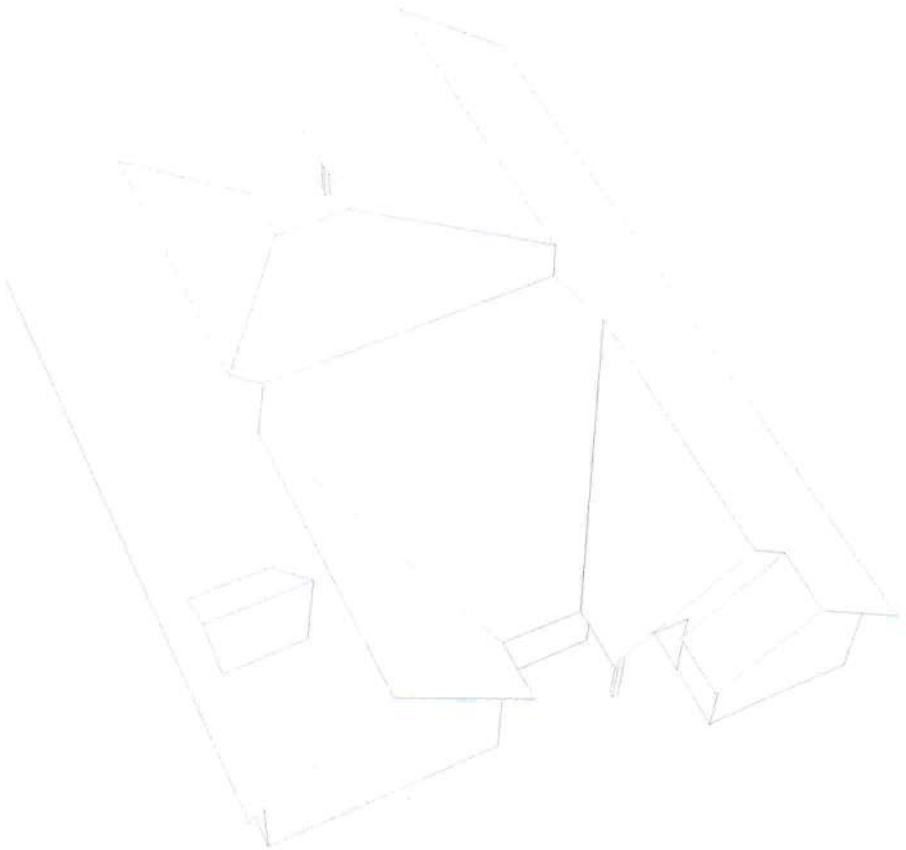

54-0-0
22-0-0
12-0-0
20-0-0
T07G
T07 (6)
14-0-0
T05G
13-0-0
T05 (2)
6-0-0
ATTIC ROOM
T06 (10)
34-0-0
T25
T24 (3)
T23
T22
T21 (3)
T20 (2)
T19
T18
T17
EJ7 (2)
1-PLY
5-0-0
7-0-0
12" TRAY CEILING
EJ7A (10)
BEARING
T06A (5)
11-0-0
4-0-0
CONV. FRAME
PB01 (10)
T04
2-PLY
T02 (3)
PB02 (5)
EJ7 (10)
T03
T26 (4)
24-0-0
5-8-0
CONV. FRAME
T02
T01 (2)
PB02 (4)
8-2-0
T11
T10
8-0-0
T09 (2)
T09G
T08 (2)
6-6-0
T08G
T01G
PB02G
22-0-0
12-0-0
6-0-0
14-0-0

6/12 - 10/12 PITCH
24" CANTILEVER

14-0-0

58-0-0



BEARING HEIGHT SCHEDULE	
	10'-1 1/8"
	9'-1 1/8"

NOTES:

SHOP DRAWING APPROVAL

Builders FirstSource
Bunnell

Jacksonville

Lake City

Sanford

O'NEIL CONST.

NELSON RES.

CUSTOM		
6-23-11	K.L.H.	361993

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Load Short Form
Entire House
LARRY RESMONDO AIR CONDITIONING

Job: NELSON RESIDENCE
Date: Jun 14, 2011
By:

HIGH SPRINGS, FL

Project Information

For: O'NEIL CONSTRUCTION
HIGH SPRINGS, FL

Design Information

	Htg	Clg
Outside db (°F)	33	92
Inside db (°F)	70	75
Design TD (°F)	37	17
Daily range	-	M
Inside humidity (%)	30	50
Moisture difference (gr/lb)	11	52

Infiltration

Method	Simplified
Construction quality	Semi-loose
Fireplaces	1 (Semi-tight)

HEATING EQUIPMENT

Make	Ruud
Trade	RUUD 13PJL SERIES
Model	13PJL48
ARI ref no.	3605269

Efficiency	8.5 HSPF	
Heating input		
Heating output	46000	Btuh @ 47°F
Temperature rise	27	°F
Actual air flow	1550	cfm
Air flow factor	0.041	cfm/Btuh
Static pressure	0.10	in H2O
Space thermostat		

COOLING EQUIPMENT

Make	Ruud
Trade	RUUD 13PJL SERIES
Cond	13PJL48
Coil	RHSL-HM4821++RCSL-H*4821
ARI ref no.	3605269

Efficiency	10.9 EER, 13 SEER	
Sensible cooling	32550	Btuh
Latent cooling	13950	Btuh
Total cooling	46500	Btuh
Actual air flow	1550	cfm
Air flow factor	0.056	cfm/Btuh
Static pressure	0.10	in H2O
Load sensible heat ratio	0.81	

ROOM NAME	Area (ft²)	Htg load (Btuh)	Clg load (Btuh)	Htg AVF (cfm)	Clg AVF (cfm)
LAUNDRY	71	313	245	13	14
HALL/STAIRWELL	128	322	346	13	19
MASTER BATH	182	2921	1634	119	91
M/BEDROOM	255	5881	3892	240	217
W.I.CLOSET	105	1825	663	75	37
KITCHEN	250	941	4512	38	252
BREAKFAST AREA	144	2537	1762	104	98
DINING	144	3127	1957	128	109
FOYER	120	1785	1410	73	79
BEDROOM 2	203	6193	3193	253	178
HALL	32	53	76	2	4
BATH 2	57	856	326	35	18
BEDROOM 3	176	2406	1851	98	103
GREAT ROOM	500	8750	5894	358	329

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Entire House	2366	37911	27760	1550	1550
Other equip loads		0	0		
Equip. @ 0.97 RSM			26927		
Latent cooling			6719		
TOTALS	2366	37911	33646	1550	1550

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

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Building Analysis
Entire House
LARRY RESMONDO AIR CONDITIONING

Job: NELSON RESIDENCE
Date: Jun 14, 2011
By:

HIGH SPRINGS, FL

Project Information

For: O'NEIL CONSTRUCTION
HIGH SPRINGS, FL

Design Conditions

Location:
Gainesville, FL, US
Elevation: 151 ft
Latitude: 30°N

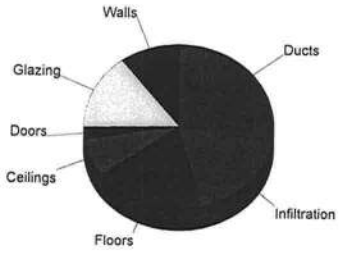
Outdoor:	**Heating**	**Cooling**
Dry bulb (°F)	33	92
Daily range (°F)	-	19 (M)
Wet bulb (°F)	-	77
Wind speed (mph)	15.0	7.5

Indoor:	**Heating**	**Cooling**
Indoor temperature (°F)	70	75
Design TD (°F)	37	17
Relative humidity (%)	30	50
Moisture difference (gr/lb)	10.6	52.0

Infiltration:	
Method	Simplified
Construction quality	Semi-loose
Fireplaces	1 (Semi-tight)

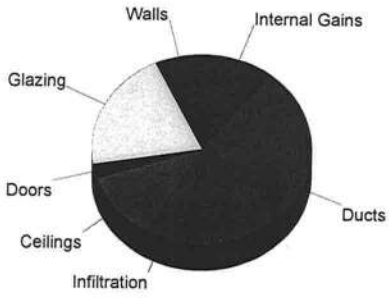
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	1.1	3970	10.5
Glazing	21.2	5650	14.9
Doors	14.4	909	2.4
Ceilings	1.2	2802	7.4
Floors	3.0	7154	18.9
Infiltration	4.3	6784	17.9
Ducts		10642	28.1
Piping		0	0
Humidification		0	0
Ventilation		0	0
Adjustments		0	
Total		**37911**	**100.0**



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	0.5	1979	7.1
Glazing	21.4	5711	20.6
Doors	11.4	716	2.6
Ceilings	1.4	3229	11.6
Floors	0	0	0
Infiltration	0.9	1467	5.3
Ducts		11738	42.3
Ventilation		0	0
Internal gains		2920	10.5
Blower		0	0
Adjustments		0	
Total		**27760**	**100.0**



Latent Cooling Load = 6719 Btuh
Overall U-value = 0.121 Btuh/ft²-°F

ERROR: negative wall area in GREAT ROOM - check windows.

wrightsoft

Project Summary
Entire House
LARRY RESMONDO AIR CONDITIONING

HIGH SPRINGS, FL

Job: NELSON RESIDENCE
Date: Jun 14, 2011
By:

Project Information

For: O'NEIL CONSTRUCTION
HIGH SPRINGS, FL

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	27269 Btuh
Ducts	10642 Btuh
Central vent (0 cfm)	0 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	37911 Btuh

Sensible Cooling Equipment Load Sizing

Structure	16023 Btuh
Ducts	11738 Btuh
Central vent (0 cfm)	0 Btuh
Blower	0 Btuh
Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	26927 Btuh

Infiltration

Method	Simplified
Construction quality	Semi-loose
Fireplaces	1 (Semi-tight)

	Heating	Cooling
Area (ft²)	2366	2366
Volume (ft³)	18930	18930
Air changes/hour	0.53	0.25
Equiv. AVF (cfm)	168	79

Latent Cooling Equipment Load Sizing

Structure	3573 Btuh
Ducts	3146 Btuh
Central vent (0 cfm)	0 Btuh
Equipment latent load	6719 Btuh
Equipment total load	33646 Btuh
Req. total capacity at 0.70 SHR	3.2 ton

Heating Equipment Summary

Make	Ruud
Trade	RUUD 13PJL SERIES
Model	13PJL48
ARI ref no.	3605269
Efficiency	8.5 HSPF
Heating input	
Heating output	46000 Btuh @ 47°F
Temperature rise	27 °F
Actual air flow	1550 cfm
Air flow factor	0.041 cfm/Btuh
Static pressure	0.10 in H2O
Space thermostat	

Cooling Equipment Summary

Make	Ruud
Trade	RUUD 13PJL SERIES
Cond	13PJL48
Coil	RHSL-HM4821++RCSL-H*4821
ARI ref no.	3605269
Efficiency	10.9 EER, 13 SEER
Sensible cooling	32550 Btuh
Latent cooling	13950 Btuh
Total cooling	46500 Btuh
Actual air flow	1550 cfm
Air flow factor	0.056 cfm/Btuh
Static pressure	0.10 in H2O
Load sensible heat ratio	0.81

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

wrightsoft

Duct System Summary
Entire House
LARRY RESMONDO AIR CONDITIONING

Job: NELSON RESIDENCE
Date: Jun 14, 2011
By:

HIGH SPRINGS, FL

Project Information

For: O'NEIL CONSTRUCTION
HIGH SPRINGS, FL

	Heating	Cooling
External static pressure	***0.10*** in H2O	***0.10*** in H2O
Pressure losses	0.25 in H2O	0.25 in H2O
Available static pressure	-0.2 in H2O	-0.2 in H2O
Supply / return available pressure	-0.11 / -0.04 in H2O	-0.11 / -0.04 in H2O
Lowest friction rate	***0.100*** in/100ft	***0.100*** in/100ft
Actual air flow	1550 cfm	1550 cfm
Total effective length (TEL)	320 ft	

Supply Branch Detail Table

Name		Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	H x W (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
LAUNDRY	c	245	13	14	0.100	4.0	0x0	VlFx	240.0	0	st1
HALL/STAIRWELL	c	346	13	19	0.100	4.0	0x0	VlFx	240.0	0	st1
MASTER BATH	h	2921	119	91	0.100	7.0	0x0	VlFx	240.0	0	st1
M/BEDROOM	h	5881	240	217	0.100	9.0	0x0	VlFx	240.0	0	st1
W.I.CLOSET	h	1825	75	37	0.100	5.0	0x0	VlFx	240.0	0	st1
KITCHEN-A	c	2256	19	126	0.100	7.0	0x0	VlFx	240.0	0	st1
KITCHEN	c	2256	19	126	0.100	7.0	0x0	VlFx	240.0	0	st1
BREAKFAST AREA	h	2537	104	98	0.100	6.0	0x0	VlFx	240.0	0	st1
DINING	h	3127	128	109	0.100	7.0	0x0	VlFx	240.0	0	st1
FOYER	c	1410	73	79	0.100	6.0	0x0	VlFx	240.0	0	st1
BEDROOM 2	h	6193	253	178	0.100	9.0	0x0	VlFx	240.0	0	st1A
HALL	c	76	2	4	0.100	4.0	0x0	VlFx	240.0	0	st1A
BATH 2	h	856	35	18	0.100	4.0	0x0	VlFx	240.0	0	st1A
BEDROOM 3	c	1851	98	103	0.100	6.0	0x0	VlFx	240.0	0	st1A
GREAT ROOM-A	h	4375	179	165	0.100	8.0	0x0	VlFx	240.0	0	st1B
GREAT ROOM	h	4375	179	165	0.100	8.0	0x0	VlFx	240.0	0	st1B

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Duct Material	Trunk
st1	Peak AVF	1550	1550	0.100	877	18.0	0 x 0	RectFbg	
st1A	Peak AVF	746	633	0.100	810	13.0	0 x 0	RectFbg	st1
st1B	Peak AVF	358	329	0.100	455	12.0	0 x 0	RectFbg	st1A

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	H x W (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb2	0x 0	240	217	80.0	0.100	544	9.0	0x 0		VlFx	
rb3	0x 0	253	178	80.0	0.100	573	9.0	0x 0		VlFx	
rb4	0x 0	98	103	80.0	0.100	526	6.0	0x 0		VlFx	
rb5	0x 0	358	329	80.0	0.100	656	10.0	0x 0		VlFx	

FORM 1100A-08

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Performance Method A

Project Name:	O'NEIL - NELSON RESIDENCE	Builder Name:	O'NEIL CONSTRUCTION
Street:		Permit Office:	COLUMBIA COUNTY
City, State, Zip:	, FL ,	Permit Number:	
Owner:	NELSON	Jurisdiction:	
Design Location:	FL, Gainesville		

1. New construction or existing	New (From Plans)
2. Single family or multiple family	Single-family
3. Number of units, if multiple family	1
4. Number of Bedrooms	3
5. Is this a worst case?	No
6. Conditioned floor area (ft²)	2366

7. Windows(231.0 sqft.)

	Description	Area
a. U-Factor: SHGC:	Dbl, default Tinted, default	214.00 ft²
b. U-Factor: SHGC:	Gbl, default Clear, default	17.00 ft²
c. U-Factor: SHGC:	N/A	ft²
d. U-Factor: SHGC:	N/A	ft²
e. U-Factor: SHGC:	N/A	ft²

8. Floor Types (2366.2 sqft.)

	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=5.0	2366.20 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

9. Wall Types (4040.3 sqft.)

	Insulation	Area
a. Frame - Wood, Exterior	R=5.0	2132.30 ft²
b. Frame - Wood, Exterior	R=19.0	1908.00 ft²
c. N/A	R=	ft²
d. N/A	R=	ft²

10. Ceiling Types (2366.2 sqft.)

	Insulation	Area
a. Under Attic (Vented)	R=30.0	2366.20 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

11. Ducts

a. Sup: Attic Ret: Attic AH: Garage Sup. R= 6, 280 ft²

12. Cooling systems

a. Central Unit — Cap: 48.0 kBtu/hr SEER: 13

13. Heating systems

a. Electric Heat Pump — Cap: 48.0 kBtu/hr HSPF: 8.5

14. Hot water systems

a. Electric — Cap: 40 gallons EF: 0.93

b. Conservation features
None

15. Credits — None

Glass/Floor Area: 0.098

Total As-Built Modified Loads: 50.11

Total Baseline Loads: 58.85

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: Larry Resmondo a/c
DATE: June 28 2011

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: [signature]
DATE: 7-20-11

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL:
DATE:

GREAT SEAL OF THE STATE OF FLORIDA — IN GOD WE TRUST

COLUMBIA COUNTY BUILDING DEPARTMENT — Received for FILE COPY — Code Compliance — PLANS EXAMINER

PROJECT

Title:	O'NEIL - NELSON RESIDEN	Bedrooms:	3	Adress Type:	Street Address
Building Type:	FLAsBuilt	Conditioned Area:	2366	Lot #	
Owner:	NELSON	Total Stories:	1	Block/SubDivision:	
# of Units:	1	Worst Case:	No	PlatBook:	
Builder Name:	O'NEIL CONSTRUCTION	Rotate Angle:	0	Street:	
Permit Office:	COLUMBIA COUNTY	Cross Ventilation:	No	County:	COLUMBIA
Jurisdiction:		Whole House Fan:	No	City, State, Zip:	, FL ,
Family Type:	Single-family				
New/Existing:	New (From Plans)				
Comment:					

CLIMATE

√	Design Location	TMY Site	IECC Zone	Design Temp 97.5 %	Design Temp 2.5 %	Int Design Temp Winter	Int Design Temp Summer	Heating Degree Days	Design Moisture	Daily Temp Range
____	FL, Gainesville	FL_GAINESVILLE_REGI	2	32	92	75	70	1305.5	51	Medium

FLOORS

√	#	Floor Type	Perimeter	R-Value	Area	Tile	Wood	Carpet
____	1	Slab-On-Grade Edge Insulatio	195.5 ft	5	2366.2 ft²	0	0	1

ROOF

√	#	Type	Materials	Roof Area	Gable Area	Roof Color	Solar Absor.	Tested	Deck Insul.	Pitch
____	1	Gable or Shed	Composition shingles	2493 ft²	394 ft²	Medium	0.5	N	0	18.4 deg

ATTIC

√	#	Type	Ventilation	Vent Ratio (1 in)	Area	RBS	IRCC
____	1	Full attic	Vented	300	2366 ft²	N	N

CEILING

√	#	Ceiling Type	R-Value	Area	Framing Frac	Truss Type
____	1	Under Attic (Vented)	30	2366.2 ft²	0.1	Wood

WALLS

√	#	Ornt	Adjacent To	Wall Type	Cavity R-Value	Area	Sheathing R-Value	Framing Fraction	Solar Absor.
____	1	N	Exterior	Frame - Wood	19	1563.75 ft	0.6	0.25	0.8
____	2	-	Exterior	Frame - Wood	5	2132.25 ft	0	0.25	0.8
____	3	-	Exterior	Frame - Wood	19	344.25 ft²	0	0.25	0.8

DOORS

✓	#	Ornt	Door Type	Storms	U-Value	Area
____	1	N	Wood	None	0.39	21.11111
____	2	-	Wood	None	0.39	42 ft²

WINDOWS

Orientation shown is the entered, asBuilt orientation.

✓	#	Ornt	Frame	Panes	NFRC	U-Factor	SHGC	Storms	Area	Overhang Depth	Overhang Separation	Int Shade	Screening
____	1	N	Vinyl	Double (Tinted)	No	0.87	0.55	N	144 ft²	1 ft 6 in	1 ft 0 in	HERS 2006	None
____	2	N	Vinyl	Double (Tinted)	No	0.87	0.55	N	54 ft²	14 ft 0 in	1 ft 0 in	HERS 2006	None
____	3	N	Vinyl	Double (Tinted)	No	0.87	0.55	N	16 ft²	8 ft 0 in	1 ft 0 in	HERS 2006	None
____	4	N	Vinyl	Glazed Block	No	0.6	0.6	N	17 ft²	1 ft 6 in	1 ft 0 in	HERS 2006	None

INFILTRATION & VENTING

✓	Method	SLA	CFM 50	ACH 50	ELA	EqLA	---- Forced Ventilation ---- Supply CFM	Exhaust CFM	Run Time Fraction	Fan Watts
____	Default	0.00036	2234	7.08	122.7	230.7	0 cfm	0 cfm	0	0

GARAGE

✓	#	Floor Area	Ceiling Area	Exposed Wall Perimeter	Avg. Wall Height	Exposed Wall Insulation
____	1	382.8 ft²	382.8 ft²	64 ft	8 ft	1

COOLING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Air Flow	SHR	Ducts
____	1	Central Unit	None	SEER: 13	48 kBtu/hr	1440 cfm	0.8	sys#1

HEATING SYSTEM

✓	#	System Type	Subtype	Efficiency	Capacity	Ducts
____	1	Electric Heat Pump	None	HSPF: 8.5	48 kBtu/hr	sys#1

HOT WATER SYSTEM

✓	#	System Type	EF	Cap	Use	SetPnt	Conservation
____	1	Electric	0.93	40 gal	60 gal	120 deg	None

SOLAR HOT WATER SYSTEM

✓	FSEC Cert #	Company Name	System Model #	Collector Model #	Collector Area	Storage Volume	FEF
____	None	None			ft²		

DUCTS

√	#	---- Supply ---- Location	R-Value	Area	---- Return ---- Location	Area	Leakage Type	Air Handler	CFM 25	Percent Leakage	QN	RLF
____	1	Attic	6	280 ft²	Attic	120 ft²	Default Leakage	Garage	(Default)	(Default) %		

TEMPERATURES

Programable Thermostat: N

Ceiling Fans:

Cooling	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec
Heating	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec
Venting	[X] Jan	[X] Feb	[X] Mar	[X] Apr	[X] May	[X] Jun	[X] Jul	[X] Aug	[X] Sep	[X] Oct	[X] Nov	[X] Dec

Thermostat Schedule: HERS 2006 Reference

Schedule Type		Hours 1	2	3	4	5	6	7	8	9	10	11	12
Cooling (WD)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Cooling (WEH)	AM	78	78	78	78	78	78	78	78	78	78	78	78
	PM	78	78	78	78	78	78	78	78	78	78	78	78
Heating (WD)	AM	68	68	68	68	68	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	68	68
Heating (WEH)	AM	68	68	68	68	68	68	68	68	68	68	68	68
	PM	68	68	68	68	68	68	68	68	68	68	68	68

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS:
, FL,

PERMIT #:

INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	N1106.AB.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	N1106.AB.1.2	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	N1106.AB.1.2	Penetrations/openings > 1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	N1106.AB.1.2	Between walls & ceilings; penetrations of ceiling plane to top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	N1106.AB.1.2	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	N1106.AB.1.2	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	N1106.AB.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	N1112.AB.3	Comply with efficiency requirements in Table N1112.ABC.3 Switch or clearly marked circuit breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	N1112.AB.2.3	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%. Heat pump pool heaters shall have a minimum COP of 4.0.	
Shower heads	N1112.AB.2.4	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	N1110.AB	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated and installed in accordance with the criteria of Section N1110.AB. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	N1107.AB.2	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	N1104.AB.1 N1102.B.1.1	Ceilings-Min. R-19. Common walls-frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE INDEX* = 85

The lower the EnergyPerformance Index, the more efficient the home.

, , FL,

1. New construction or existing		New (From Plans)
2. Single family or multiple family		Single-family
3. Number of units, if multiple family		1
4. Number of Bedrooms		3
5. Is this a worst case?		No
6. Conditioned floor area (ft²)		2366
7. Windows**	Description	Area
a. U-Factor: SHGC:	Dbl, default Tinted, default	214.00 ft²
b. U-Factor: SHGC:	Gbl, default Clear, default	17.00 ft²
c. U-Factor: SHGC:	N/A	ft²
d. U-Factor: SHGC:	N/A	ft²
e. U-Factor: SHGC:	N/A	ft²

8. Floor Types	Insulation	Area
a. Slab-On-Grade Edge Insulation	R=5.0	2366.20 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

9. Wall Types	Insulation	Area
a. Frame - Wood, Exterior	R=5.0	2132.30 ft²
b. Frame - Wood, Exterior	R=19.0	1908.00 ft²
c. N/A	R=	ft²
d. N/A	R=	ft²

10. Ceiling Types	Insulation	Area
a. Under Attic (Vented)	R=30.0	2366.20 ft²
b. N/A	R=	ft²
c. N/A	R=	ft²

11. Ducts
a. Sup: Attic Ret: Attic AH: Garage Sup. R= 6, 280 ft²

12. Cooling systems
a. Central Unit — Cap: 48.0 kBtu/hr SEER: 13

13. Heating systems
a. Electric Heat Pump — Cap: 48.0 kBtu/hr HSPF: 8.5

14. Hot water systems
a. Electric — Cap: 40 gallons EF: 0.93

b. Conservation features
None

15. Credits — None

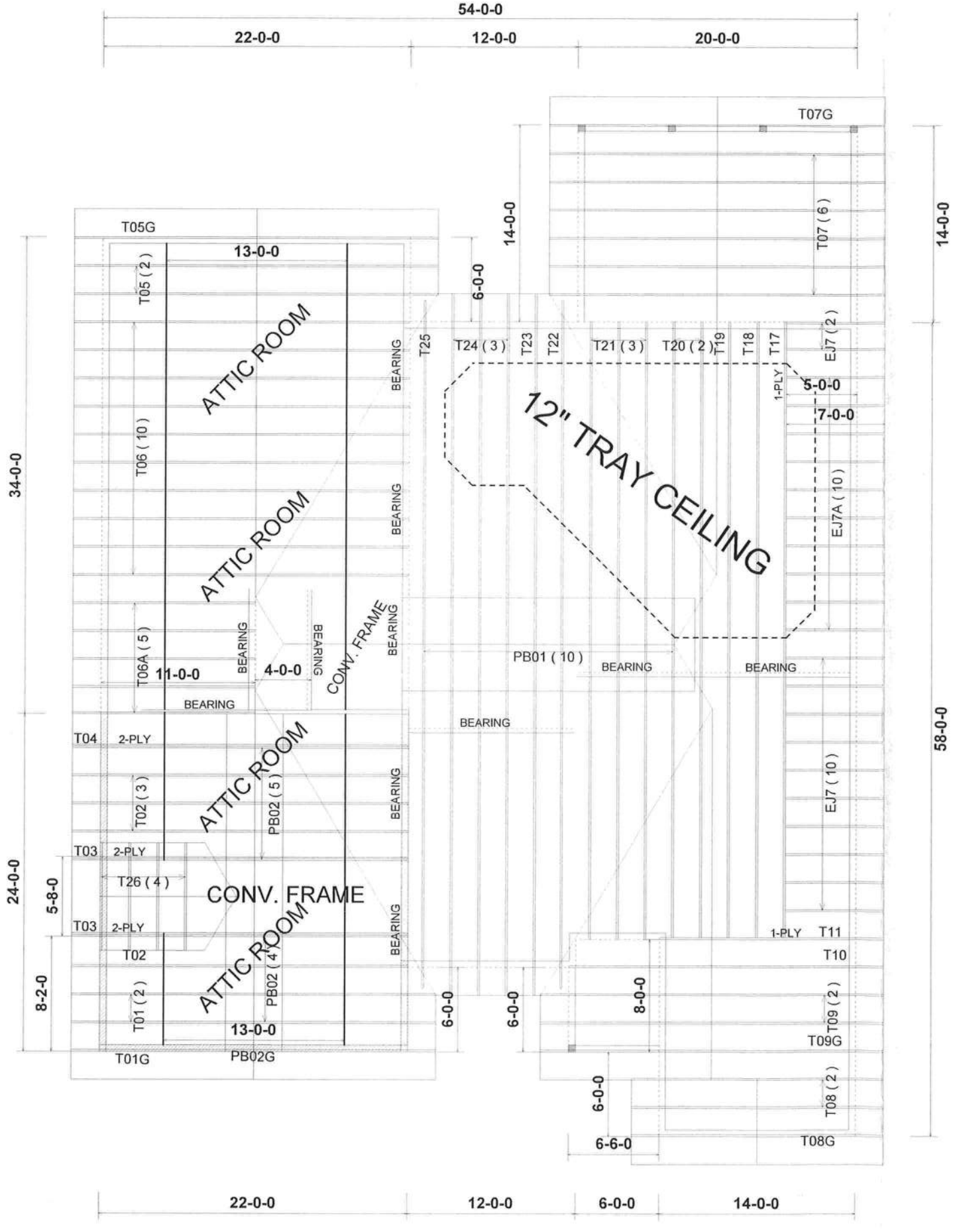
I certify that this home has complied with the Florida Energy Efficiency Code for Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: _____ Date: _____

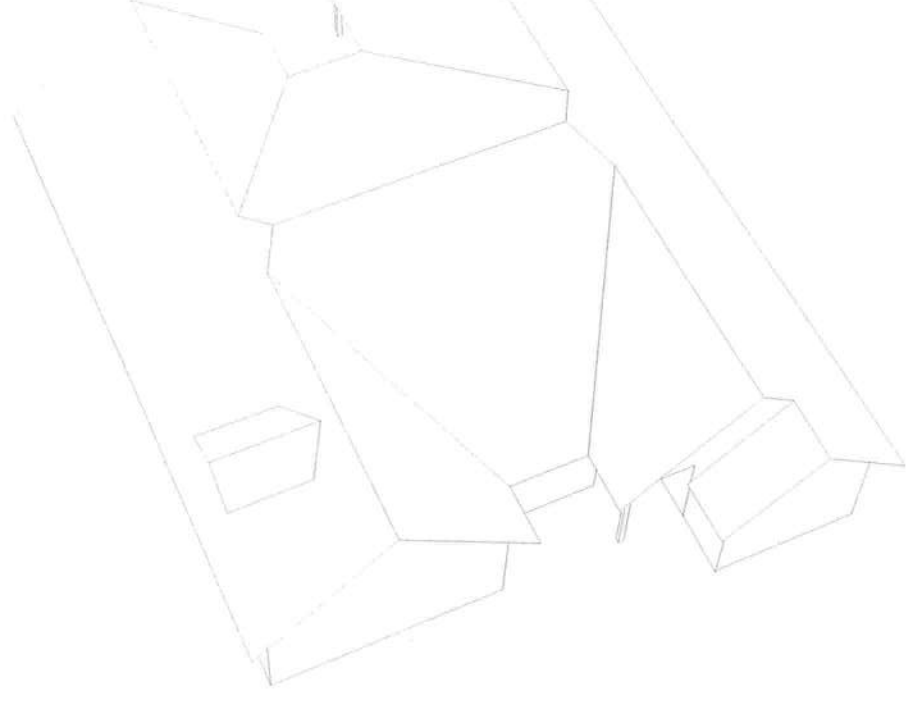
Address of New Home: _____ City/FL Zip: _____

*Note: The home's estimated Energy Performance Index is only available through the EnergyGauge USA - FlaRes2008 computer program. This is not a Building Energy Rating. If your Index is below 100, your home may qualify for incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at (321) 638-1492 or see the Energy Gauge web site at energygauge.com for information and a list of certified Raters. For information about Florida's Energy Efficiency Code for Building Construction, contact the Department of Community Affairs at (850) 487-1824.

**Label required by Section 13-104.4.5 of the Florida Building Code, Building, or Section B2.1.1 of Appendix G of the Florida Building Code, Residential, if not DEFAULT.



6/12 - 10/12 PITCH
24" CANTILEVER



BEARING HEIGHT SCHEDULE

	10'-1 1/8"
	9'-1 1/8"

NOTES:

1) REFER TO THE ENGINEERED FLOOR FRAMING INSTALLATION AND TRUSS DESIGN DRAWINGS FOR ADDITIONAL INFORMATION.

2) ALL TRUSSES SHOWN ON THIS LAYOUT ARE VALLEY TRUSSES AND MUST BE CAREFULLY DELIVERED AND SET TO THE FINAL LOCATION AS SHOWN ON THIS LAYOUT.

3) ALL VALLEY SETS TO BE CONSTRUCTED AS DETAILED ON PAGE 2.

4) ALL TRUSSES ARE TO BE SPACED 24" O.C. MAXIMUM UNLESS OTHERWISE NOTED.

5) ALL WALL SHEATHING AND FLOOR SHEATHING ARE ASSUMED TO BE LOAD BEARING UNLESS OTHERWISE NOTED.

6) ALL ROOF TRUSSES MUST BE BRACED WITH THE TOP CHORD.

7) ALL ROOF TRUSS HANGERS TO BE MINIMUM THD422 UNLESS OTHERWISE NOTED.

8) BEARING ASSEMBLIES NOT TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND MUST BE REVIEWED BY ALL TRADES, ARCHITECTURAL OR OTHER DRAWINGS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL DIMENSIONS TO MAKE SURE ANY CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Approved by: _____ Date: _____

Builders FirstSource

Bunnell
PHONE 904-437-3540 FAX 904-437-3984

Jacksonville
PHONE 904-772-0700 FAX 904-772-0975

Lake City
PHONE 386-755-0894 FAX 386-755-2475

Sanford
PHONE 407-322-0091 FAX 407-322-9955

BUYER: O'NEIL CONST.		
LOT/BLOCK/SUB: NELSON RES.		
PLAN: CUSTOM		ELEVATION: XRAY LITE
DATE: 6-23-11	DRAWN: K.L.H.	JOB #: 361993