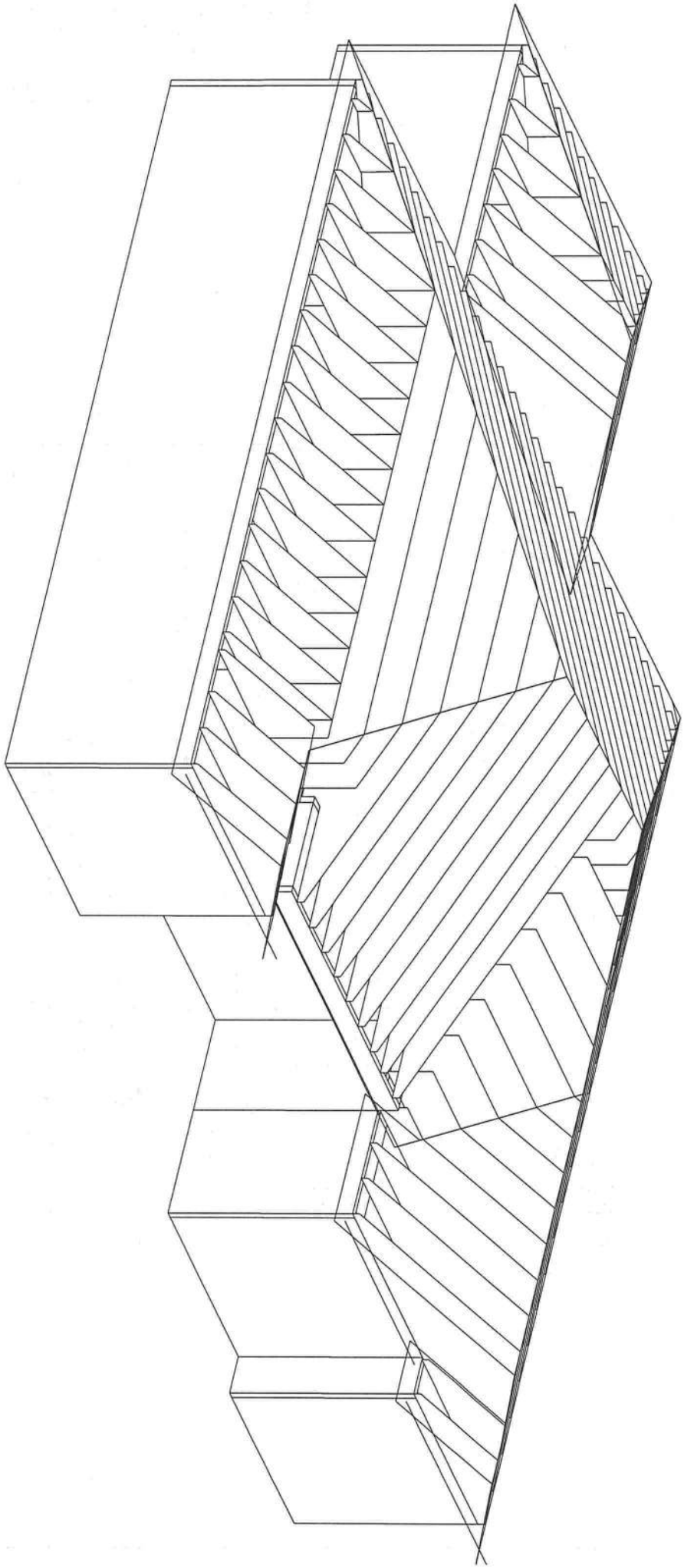
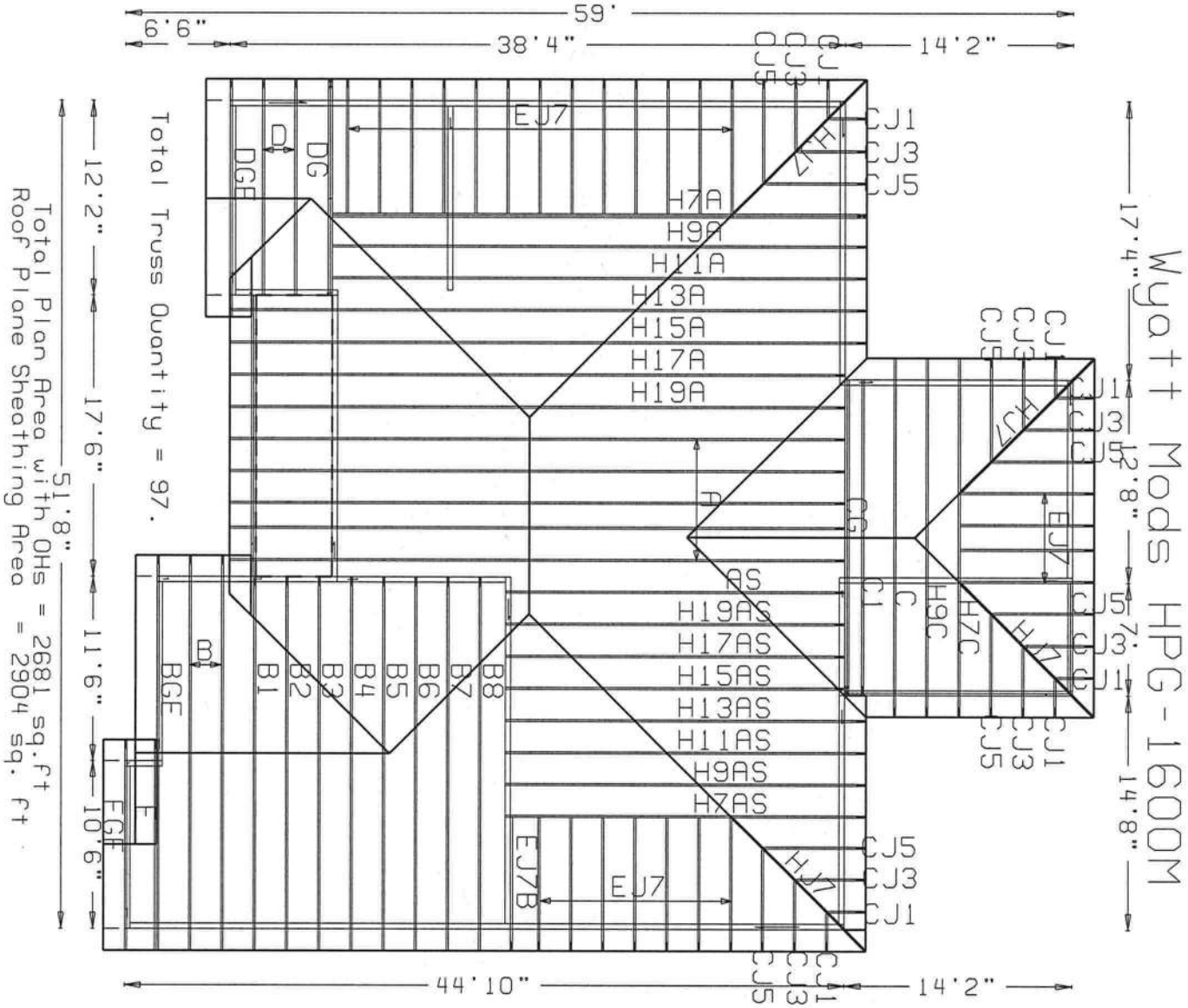




PAGE NO: 1 OF 1	JOB NO: 12-381	ADDRESS: JOB DESCRIPTION:: BRYAN ZECHER /: Wyatt HPG-1600M JOB #: 12-381 DESIGNER: Curt V Burlingame SALESMAN: Curt V Burlingame

: <Not Found>

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1URU9114Z0107091314

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

Truss Fabricator:	Anderson Truss Company
Job Identification:	12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL
Truss Count:	42
Model Code:	Florida Building Code 2010
Truss Criteria:	FBC2010Res/TPI-2007(STD)
Engineering Software:	Alpine Software,Version 10.03.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H. Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: BRCLBSUB-12015EC1-GBLLETIN-GABRST10-

#	Ref Description	Drawing#	Date
1	06520--A 37' Special	12342005	12/07/12
2	06521-AS 21'2" Special	12342006	12/07/12
3	06522--B 22' Common	12342007	12/07/12
4	06523--B1 22' Common	12342008	12/07/12
5	06524--B2 22' Common	12342009	12/07/12
6	06525--B3 22' Common	12342010	12/07/12
7	06526--B4 22' Common	12342011	12/07/12
8	06527--B5 22' Common	12342012	12/07/12
9	06528--B6 22' Common	12342013	12/07/12
10	06529--B7 22' Common	12342014	12/07/12
11	06530--B8 22' Common	12342015	12/07/12
12	06531--BGE 22' Gable	12342016	12/07/12
13	06532--C 19'8" Common	12342017	12/07/12
14	06533--C1 19'8" Common	12342018	12/07/12
15	06534-CG 19'8" Common	12342019	12/07/12
16	06535--CJ1 1' Jack	12342020	12/07/12
17	06536--CJ3 3' Jack	12342021	12/07/12
18	06537--CJ5 5' Jack	12342022	12/07/12
19	06538--D 12'2" Common	12342023	12/07/12
20	06539-DG 12'2" Common	12342024	12/07/12
21	06540--DGE 12'2" Gable	12342025	12/07/12
22	06541-E 10'6" Common G	12342026	12/07/12
23	06542-EGE 10'6" Gable	12342027	12/07/12
24	06543--EJ7 7' End Jack	12342028	12/07/12
25	06544-EJ7B 7' End Jack	12342029	12/07/12
26	06545-H11A 32' Stepdow	12342030	12/07/12
27	06546-H11AS 21'2" Mono	12342031	12/07/12
28	06547-H13A 37' Stepdow	12342032	12/07/12
29	06548-H13AS 21'2" Mono	12342033	12/07/12
30	06549-H15A 37' Stepdow	12342034	12/07/12
31	06550-H15AS 21'2" Step	12342035	12/07/12
32	06551-H17A 37' Stepdow	12342036	12/07/12
33	06552-H17AS 21'2" Step	12342037	12/07/12
34	06553-H19A 37' Special	12342038	12/07/12
35	06554-H19AS 21'2" Spec	12342039	12/07/12
36	06555-H7A 32' Special	12342040	12/07/12
37	06556-H7AS 21'2" Mono	12342041	12/07/12
38	06557-H7C 19'8" Stepdo	12342042	12/07/12
39	06558--H9A 32' Special	12342043	12/07/12
40	06559-H9AS 21'2" Mono	12342044	12/07/12
41	06560-H9C 19'8" Stepdo	12342045	12/07/12
42	06561-HJ7 9'10"13 Hip	12342046	12/07/12

COLUMBIA COUNTY BUILDING DEPARTMENT Received for FILE COPY Code Compliance PLANS EXAMINER

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

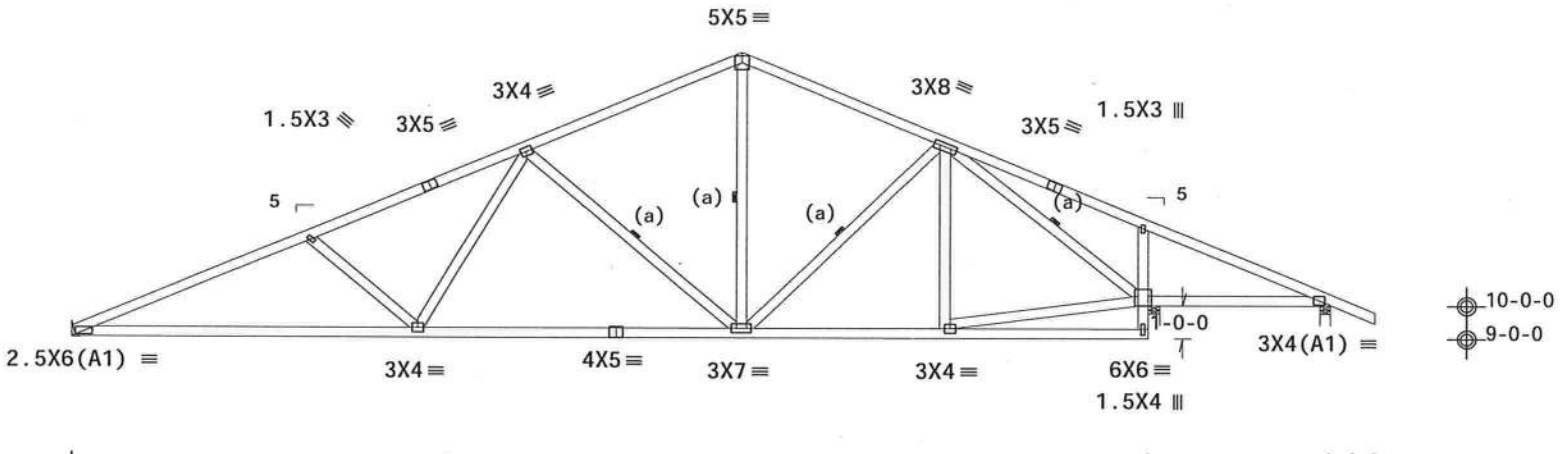
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:5 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6520
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342005
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36147
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - AS 21'2" Special)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

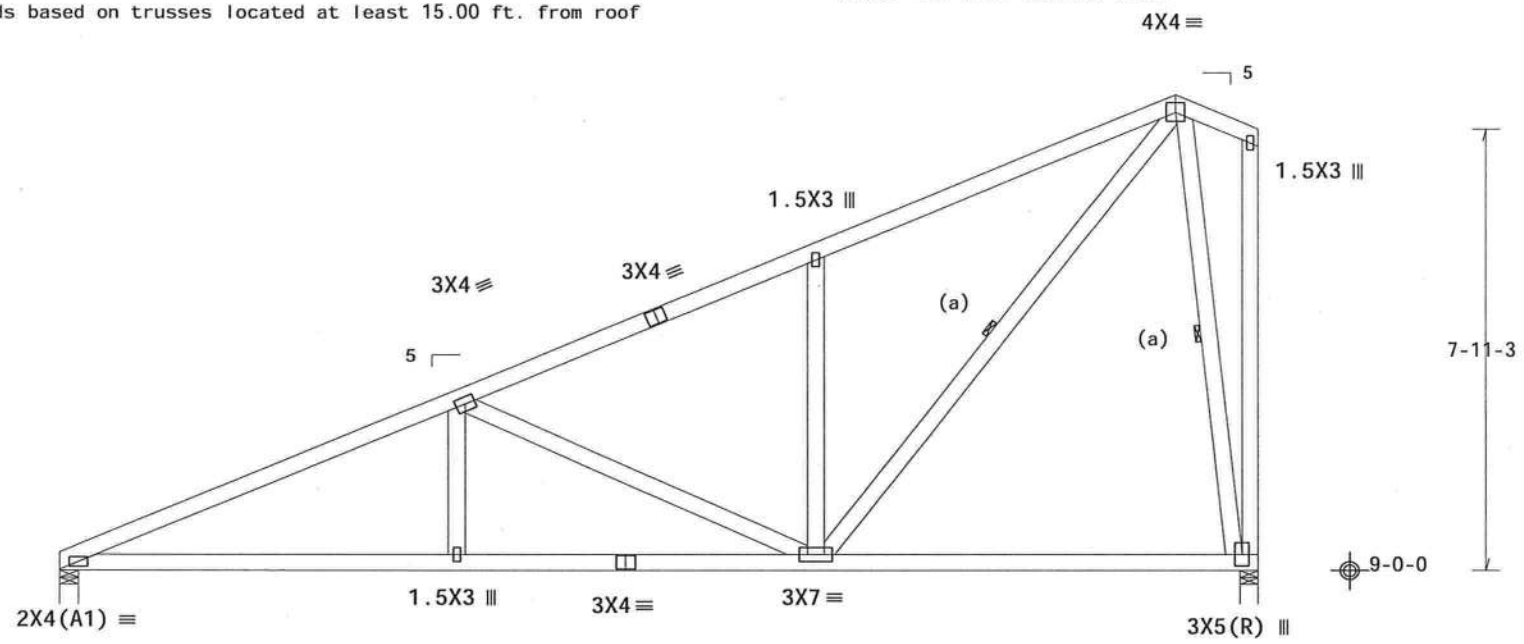
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY: 1 | FL/-/4/-/-/R/- | Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6521
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342006
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36161
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B 22' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

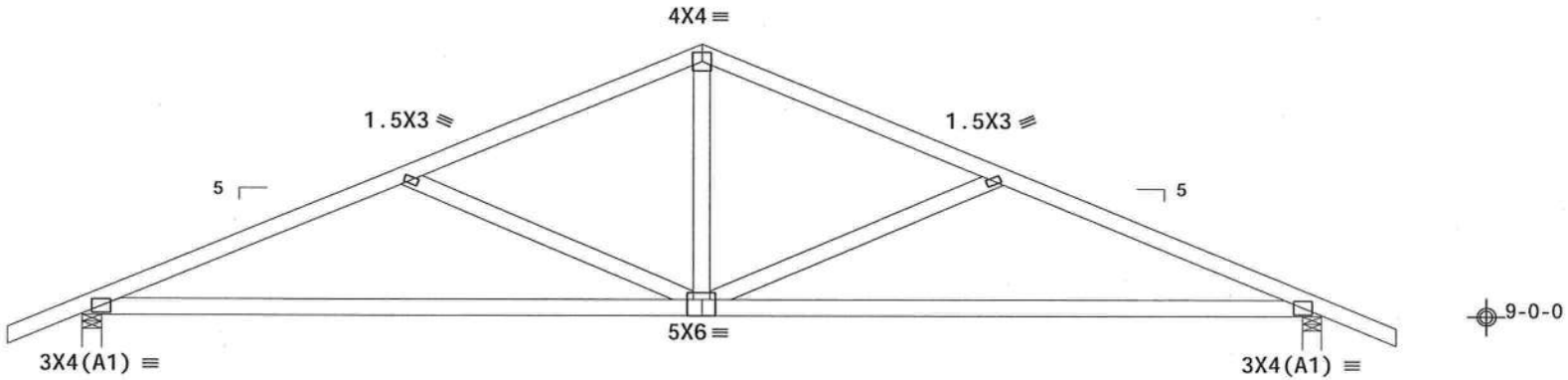
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD) 10.03.11.0209.19 QTY:2 FL/-/4/-/-/R/-

PLT TYP. Wave FT/RT=10%(0%)/0(0) Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design any failure to build the trusses in conformance with ANSI/TPI 1, or for handling, shipping, installation bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6522
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342007
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36145
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B1 22' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

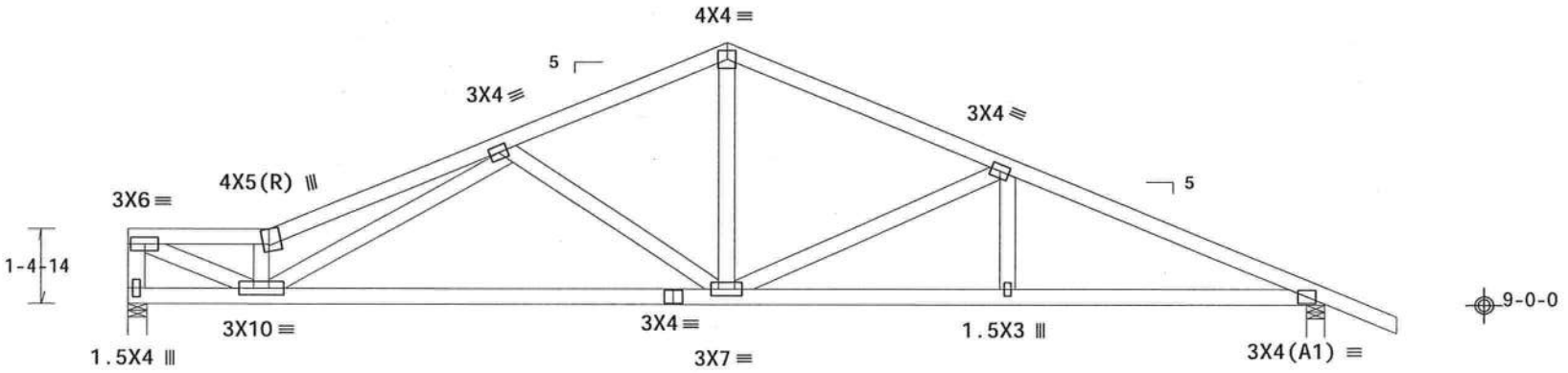
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

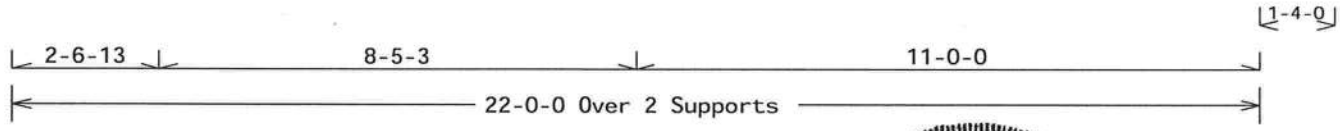
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R=818 U=32 W=4"
RL=57/-63

R=915 U=42 W=4"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY 1 | FL/-/4/-/-/R/- | Scale =.3125"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 6523
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342008
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36144
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B2 22' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

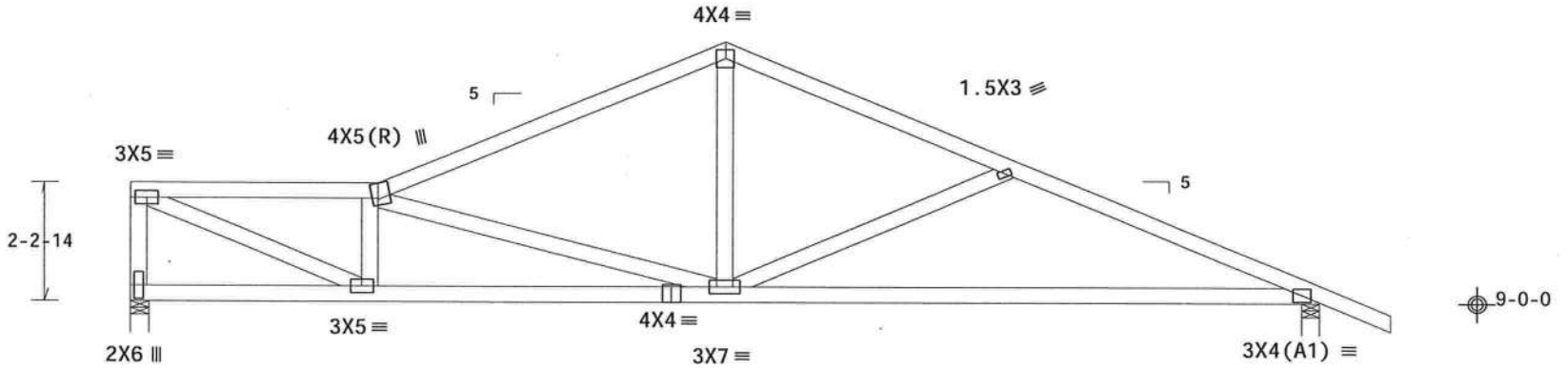
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

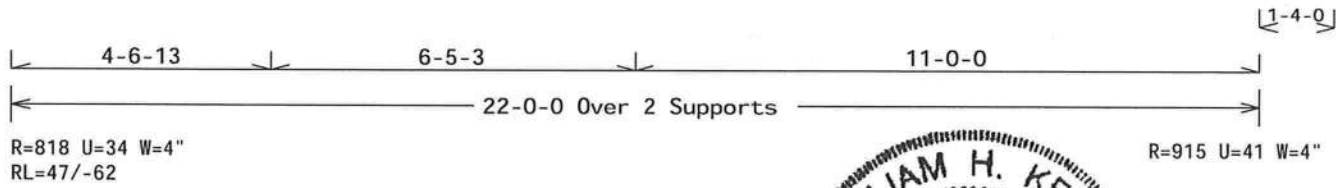
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6524
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342009
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36143
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B3 22' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

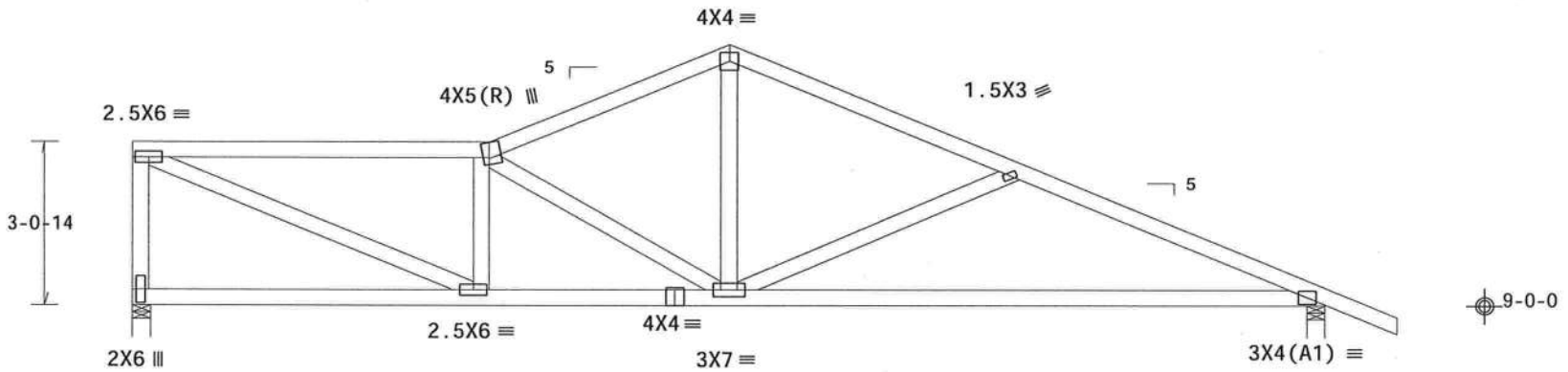
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

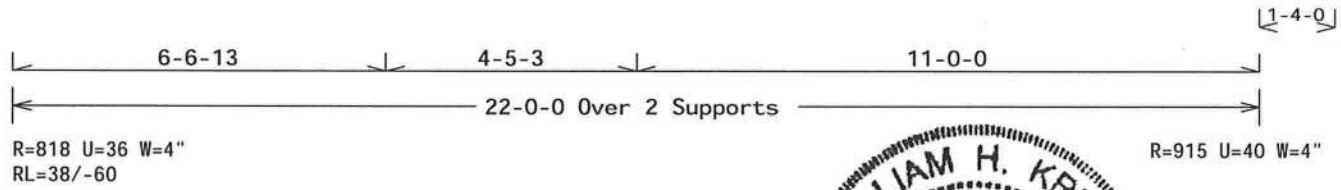
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY 1 | FL/-/4/-/-/R/- | Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6525
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342010
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36142
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B4 22' Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

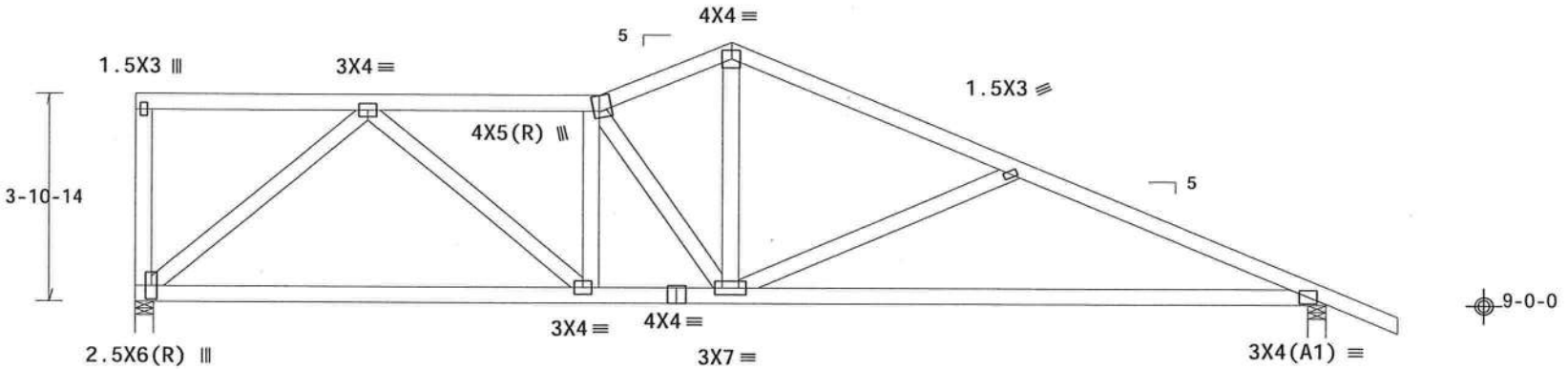
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

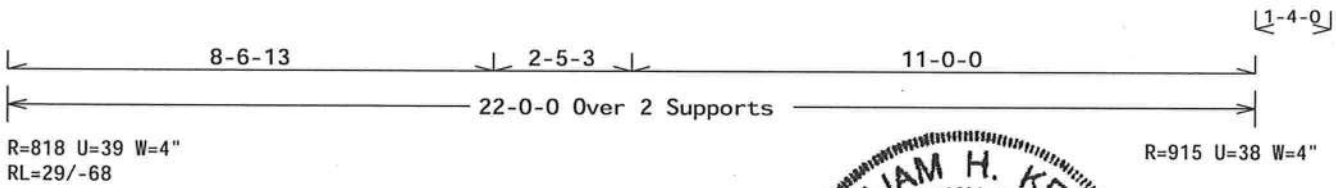
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 TQT:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6526
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342011
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36158
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B5 22' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

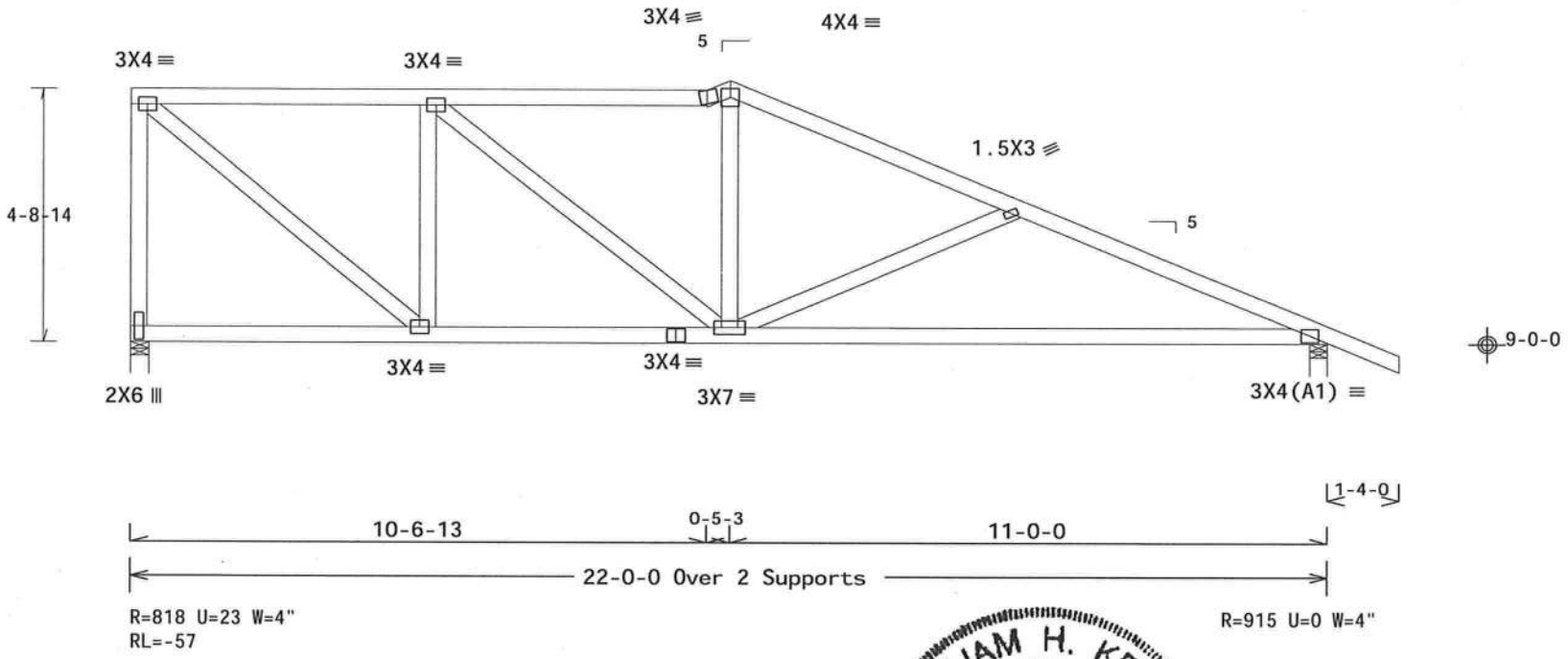
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD) 10.03.11.0209.19 TOT:1 FL/-/4/-/-/R/-

PLT TYP. Wave FT/RT=10%(0%)/0(0) Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6527
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342012
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36157
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B6 22' Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

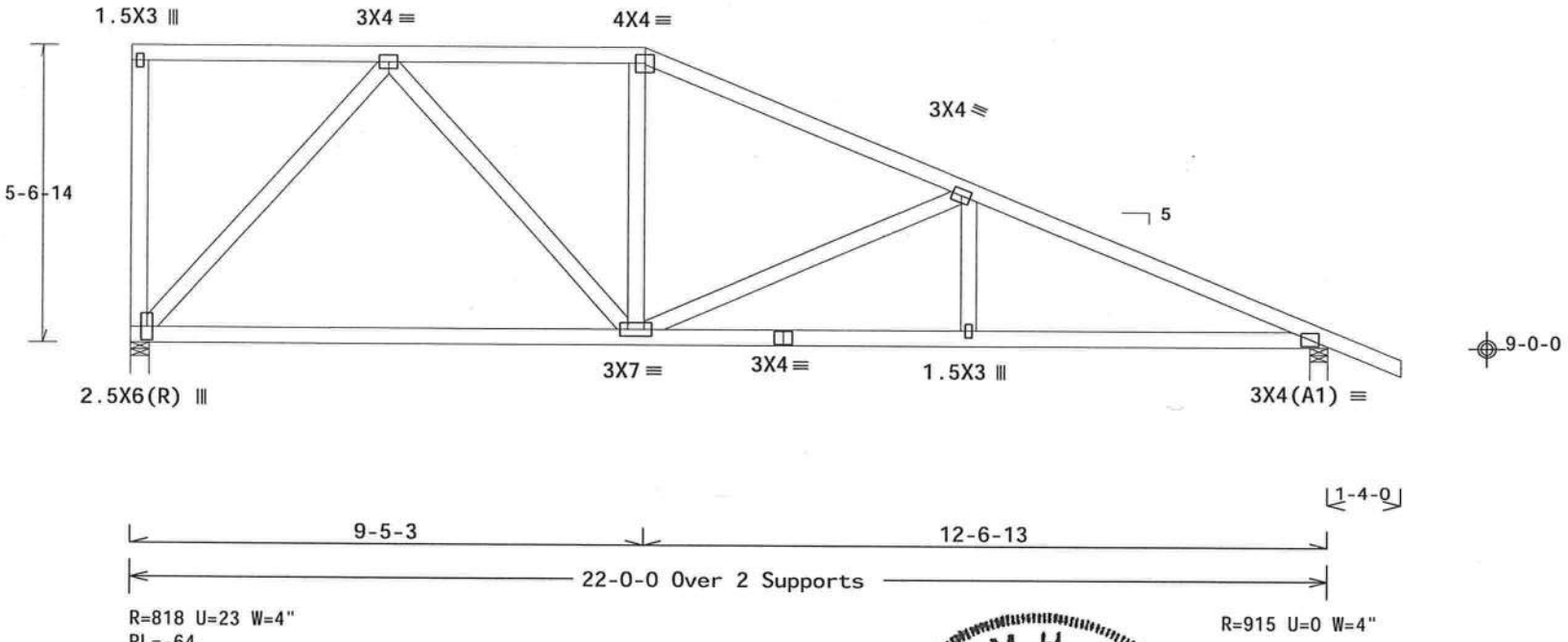
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

V0.03.11.0209.19 TOEV:1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6528
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342013
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36156
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B7 22' Common)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

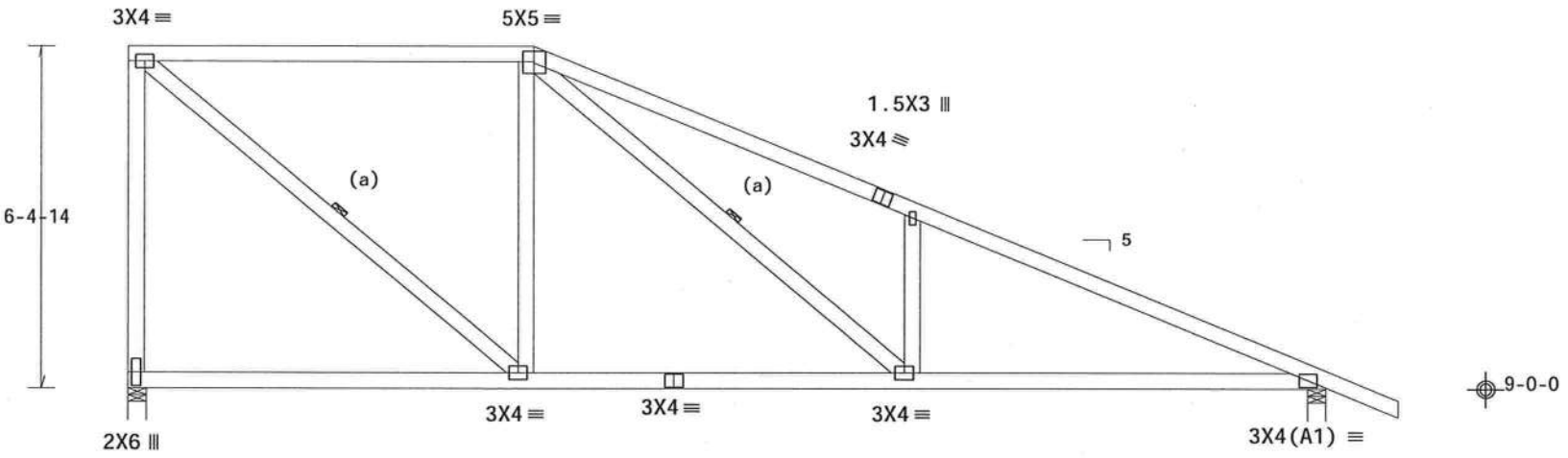
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

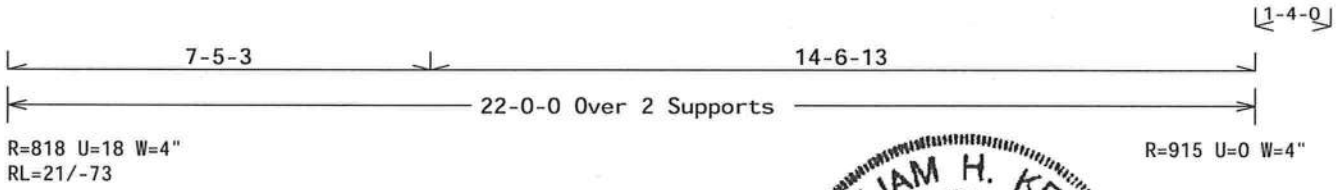
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6529
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342014
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36155
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - B8 22' Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Left end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

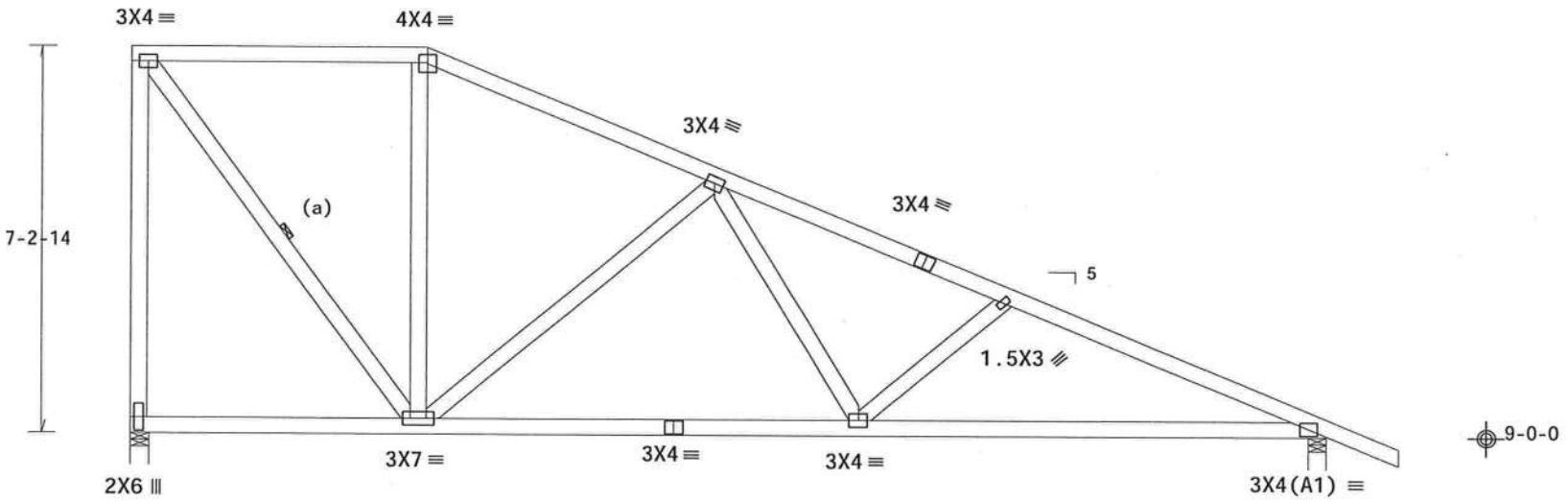
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

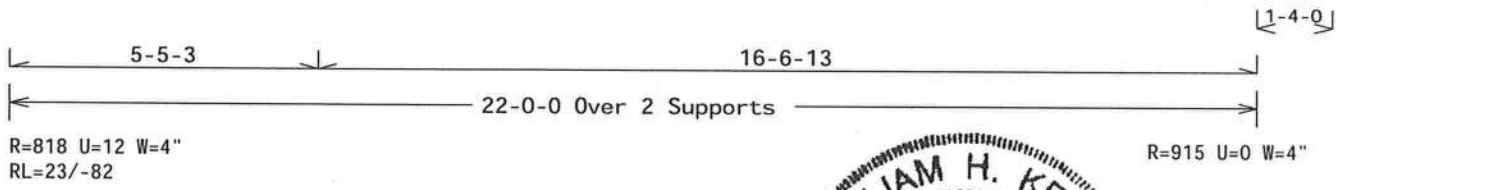
Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY:1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF R9114- 6530
TC DL	7.0 PSF	DATE 12/07/12
BC DL	10.0 PSF	DRW HCUSR9114 12342015
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 36154
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - BGE 22' Gable)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Gable end supports 8" max rake overhang.

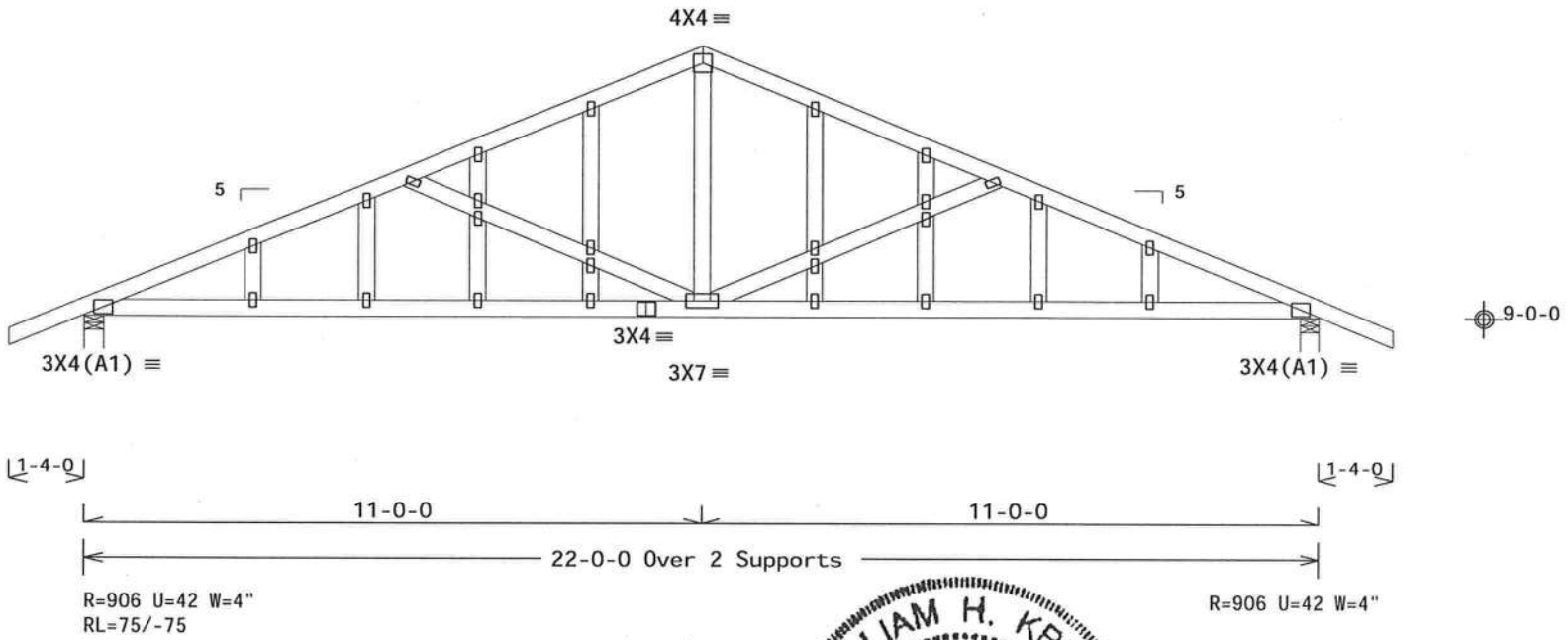
Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY: 1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF R9114- 6531
TC DL	7.0 PSF	DATE 12/07/12
BC DL	10.0 PSF	DRW HCUSR9114 12342016
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 36168
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - C 19'8" Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

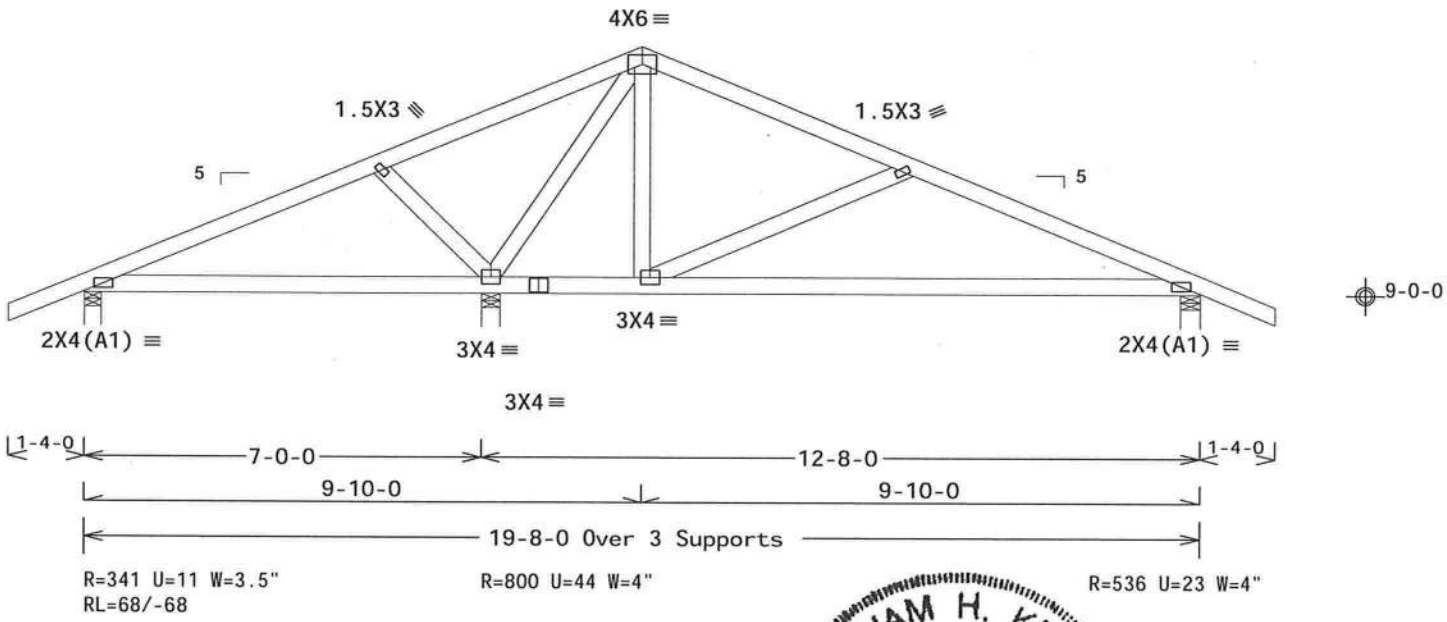
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD); 10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/-

PLT TYP. Wave

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6532
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342017
BC LL	0.0 PSF		HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36140
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - C1 19'8" Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

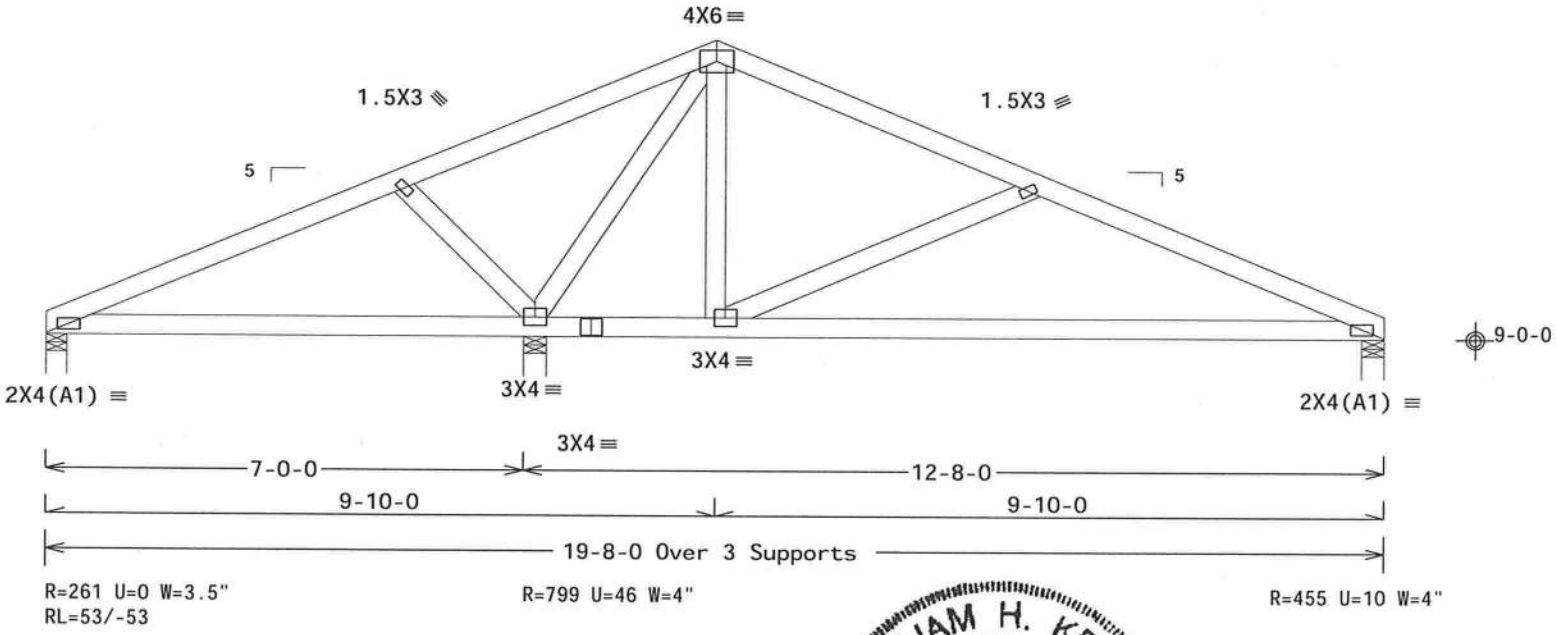
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD) 10.03.11.0209.19 TOT:1 FL/-/4/-/-/R/-

PLT TYP. Wave FT/RT=10%(0%)/0(0) Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6533
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342018
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36139
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - CG 19'8" Common Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A :W4 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 55 plf at 0.00 to 55 plf at 9.83
TC- From 55 plf at 9.83 to 55 plf at 19.67
BC- From 20 plf at 0.00 to 20 plf at 8.40
BC- From 10 plf at 8.40 to 10 plf at 12.00
BC- From 10 plf at 12.00 to 10 plf at 19.67
BC- 1231.42 lb Conc. Load at 8.40,10.40,11.94,13.94 15.94
BC- 1171.05 lb Conc. Load at 17.94

Bottom chord checked for 10.00 psf non-concurrent live load.

2 COMPLETE TRUSSES REQUIRED

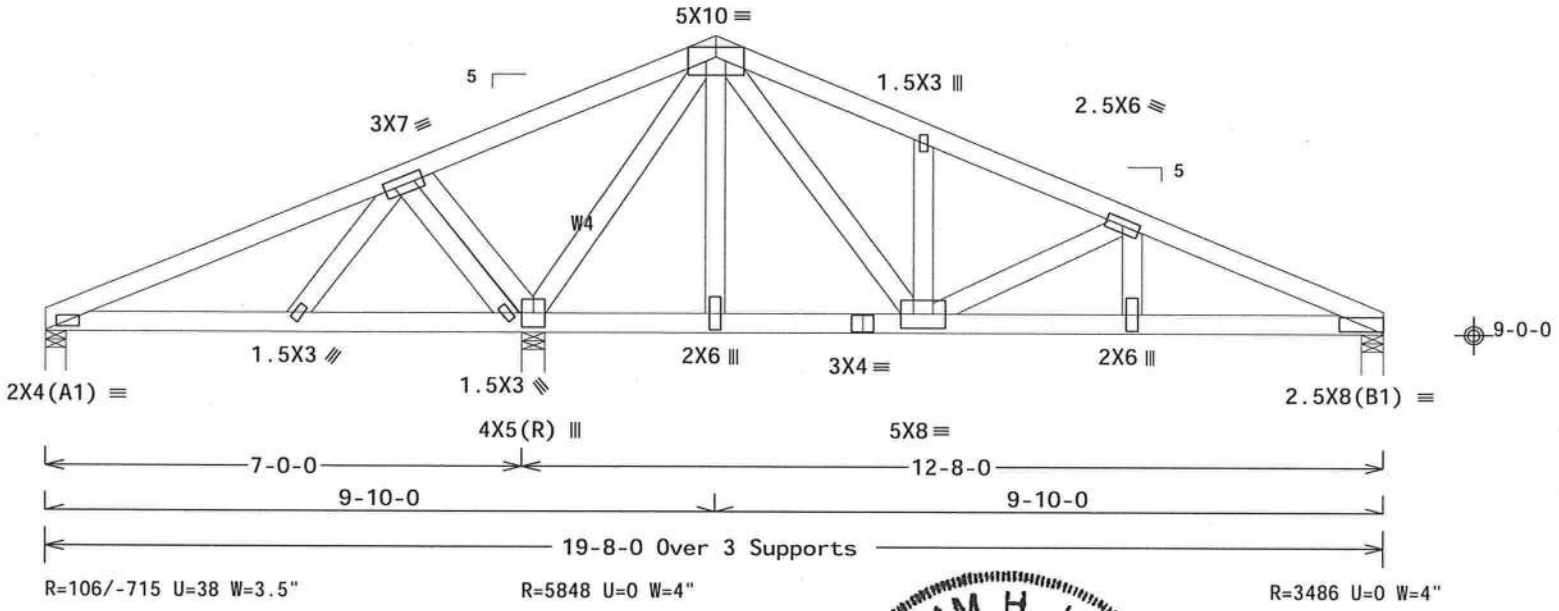
Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 4.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Negative reaction(s) of -715# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6534
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342019
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36172
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

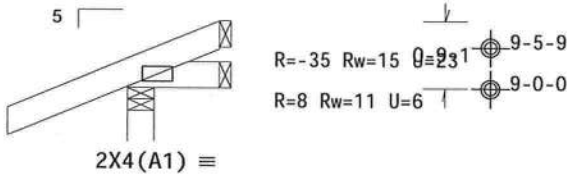
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

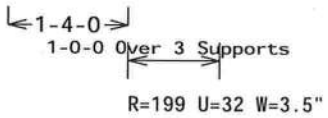
Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:8 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6535
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342020
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36138
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

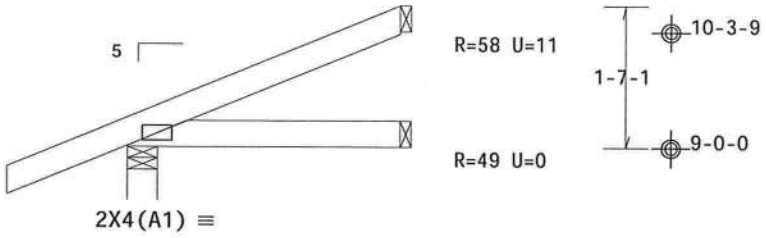
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD; 130.03.11.0209.19 ; 10Y:8 FL/-/4/-/-/R/-

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICH LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF R9114- 6536
TC DL	7.0 PSF	DATE 12/07/12
BC DL	10.0 PSF	DRW HCUSR9114 12342021
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 36136
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1URU9114Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

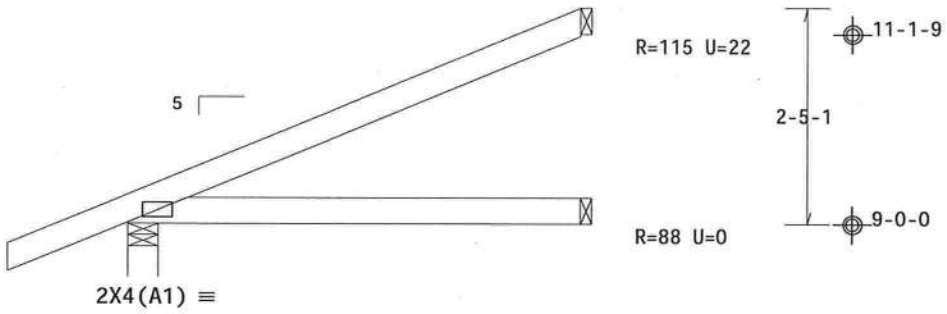
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD) WIND:03.11.0209.19 QTY:8 FL/-/4/-/-/R/-

PLT TYP. Wave FT/RT=10%(0%)/0(0) Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6537
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342022
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36137
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - D 12'2" Common)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

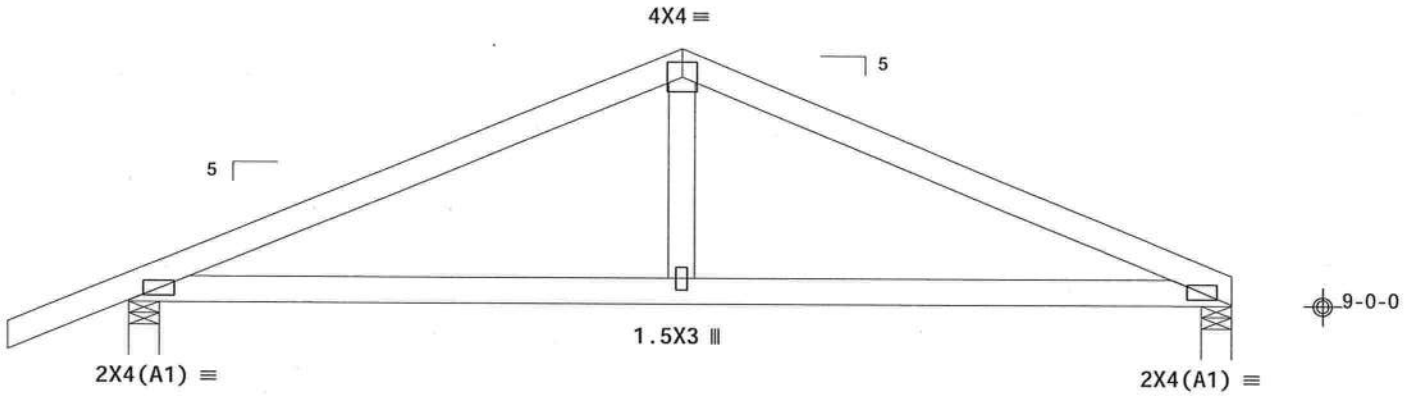
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

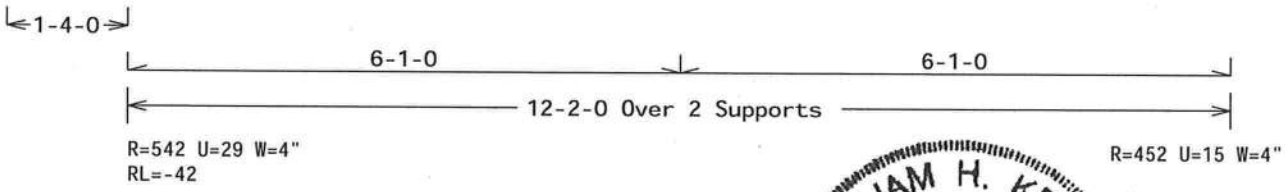
Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:2 FL/-/4/-/-/R/-

Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF R9114- 6538
TC DL	7.0 PSF	DATE 12/07/12
BC DL	10.0 PSF	DRW HCUSR9114 12342023
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 36152
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - DG 12'2" Common Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x6 SP_#2_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 55 plf at -1.33 to 55 plf at 6.08
TC- From 55 plf at 6.08 to 55 plf at 12.17
BC- From 4 plf at -1.33 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 7.06
BC- From 10 plf at 7.06 to 10 plf at 12.17
BC- -114.27 lb Conc. Load at 7.06
BC- 78.24 lb Conc. Load at 9.06
BC- 88.64 lb Conc. Load at 11.06

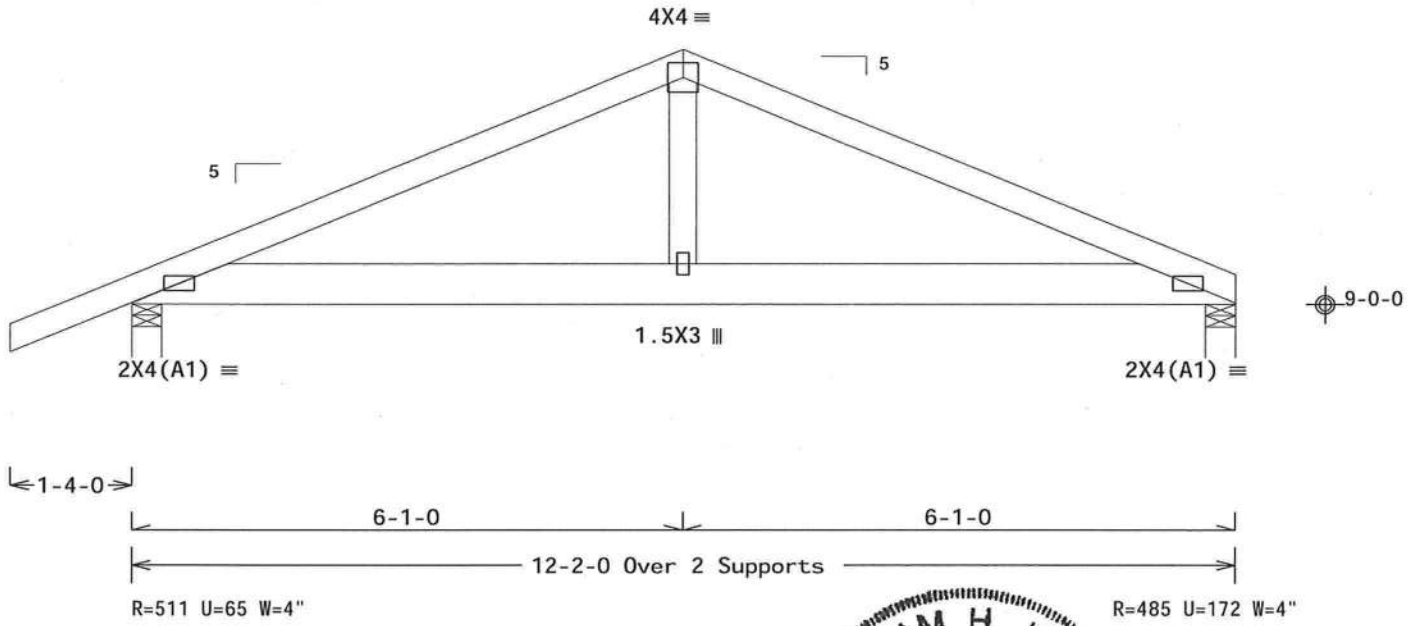
2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY:1 | FL/-/4/-/-/R/- | Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF R9114- 6539
TC DL	7.0 PSF	DATE 12/07/12
BC DL	10.0 PSF	DRW HCUSR9114 12342024
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 36177
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - DGE 12'2" Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A
:Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

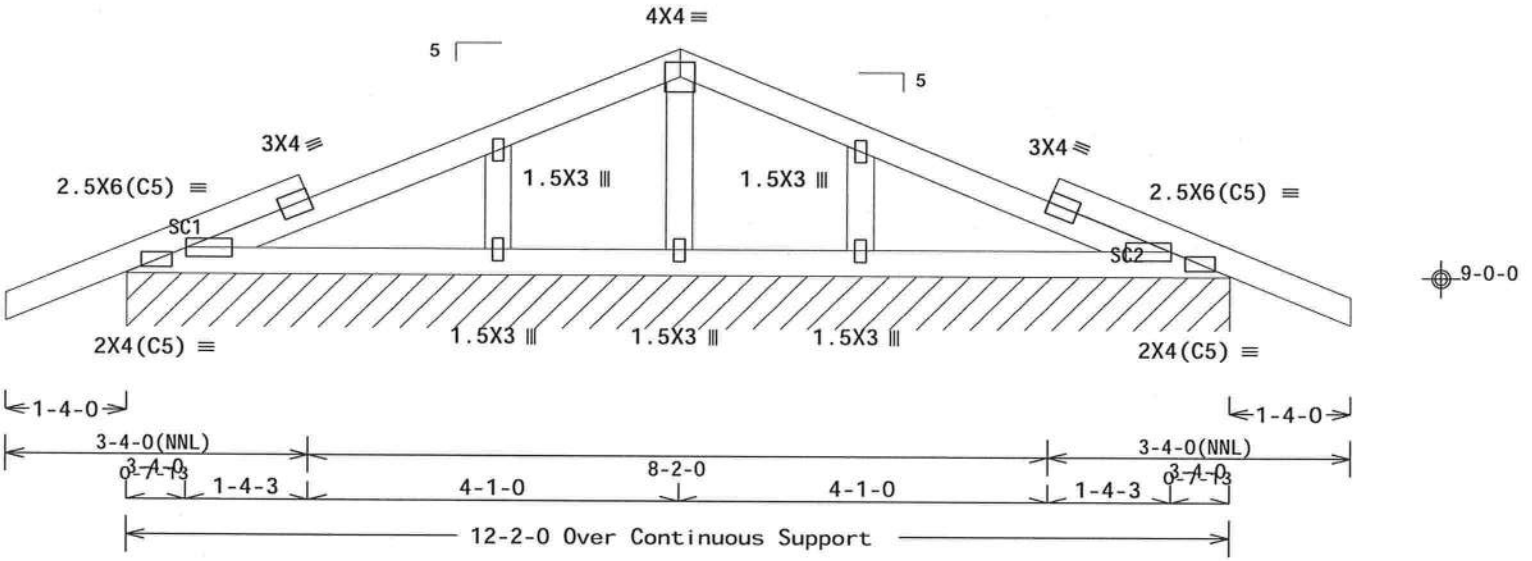
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=88 PLB U=4 PLF W=12-2-0
RL=4/-4 PLF

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI 9. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's general notes page; ITW-BCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6540
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342025
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36153
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - E 10'6" Common Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

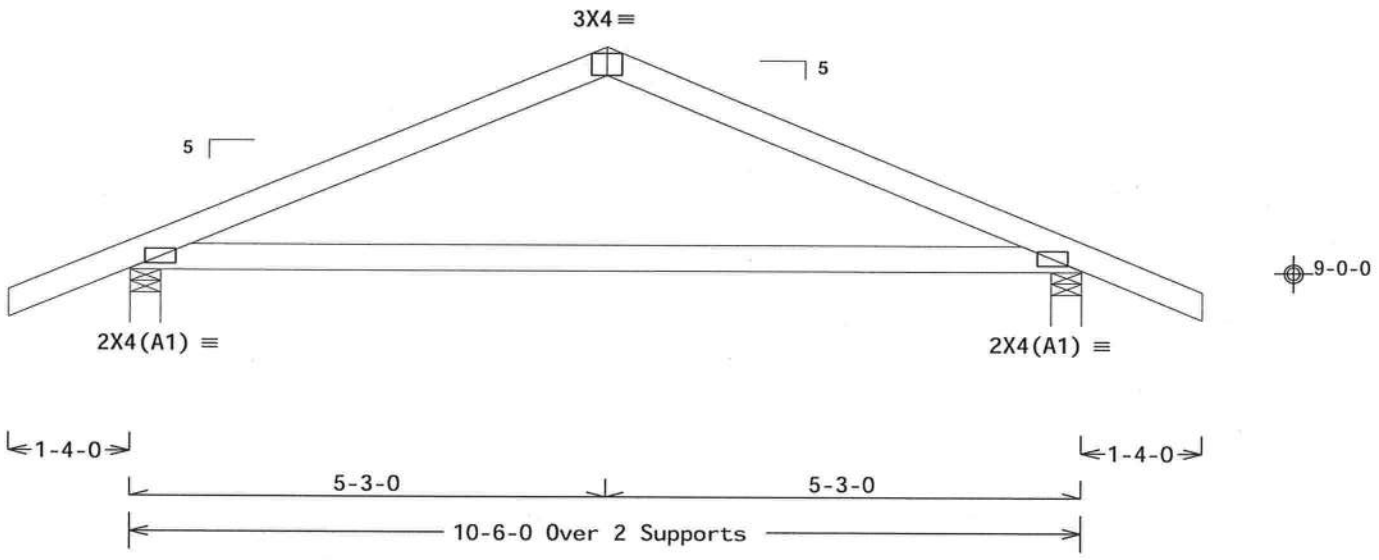
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

PLT TYP. Wave

10.03.11.0209.19 01Y:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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WILLIAM H. KRICK LICENSED No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6541
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342026
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36169
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A
:Stack Chord SC1 2x4 SP_#1_12A::Stack Chord SC2 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

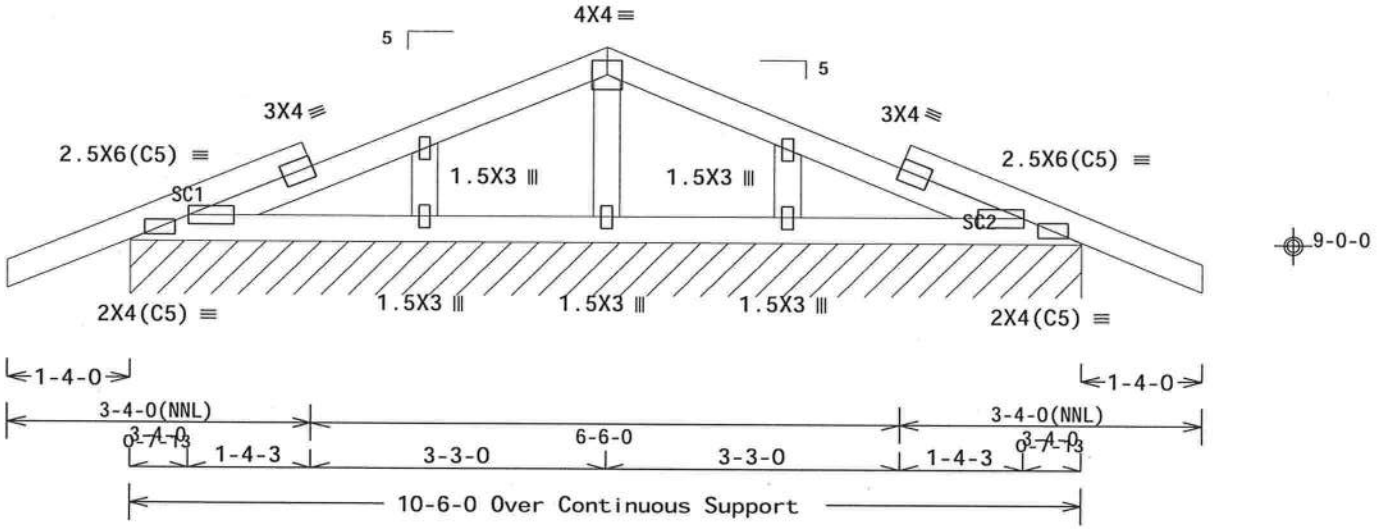
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=90 PLF U=5 PLF W=10-6-0
RL=4/-4 PLF

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 TOP:1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

WILLIAM H. KRICH LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/07/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 6542
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342027
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36176
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - EJ7 7' End Jack)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

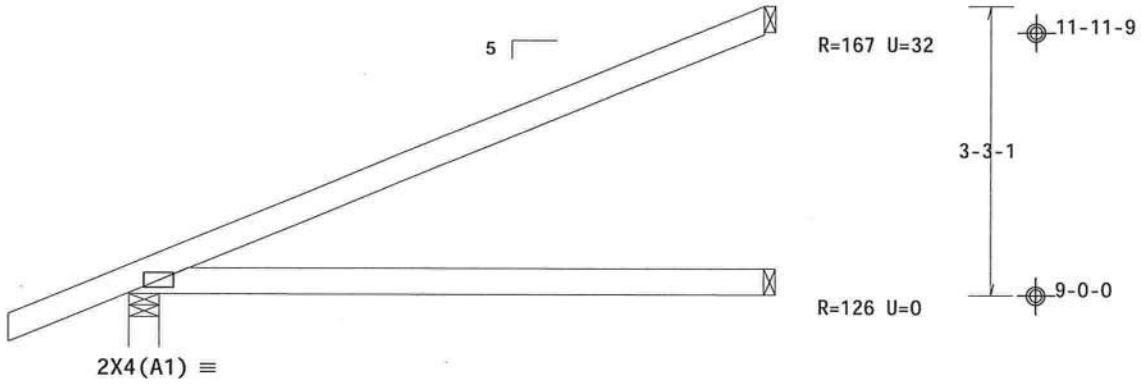
Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:24 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
IMPORTANT FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6543
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342028
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36146
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - EJ7B 7' End Jack Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

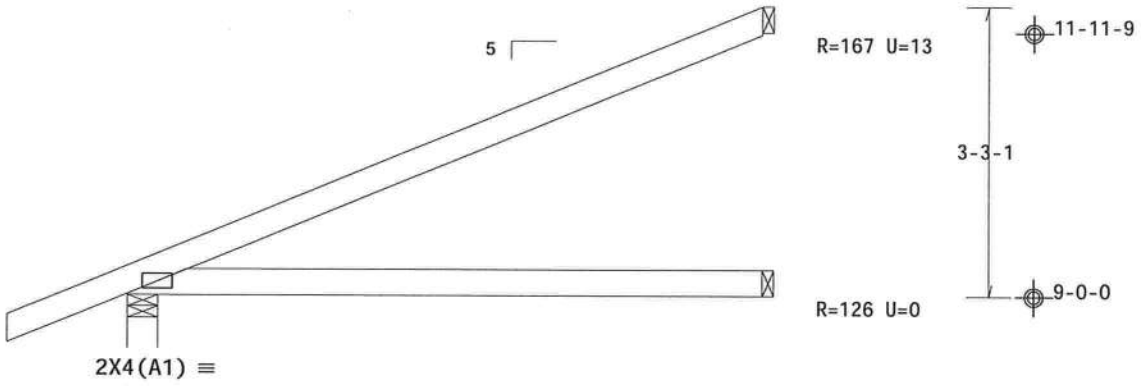
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6544
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342029
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36170
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H11A 32' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Negative reaction(s) of -287# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

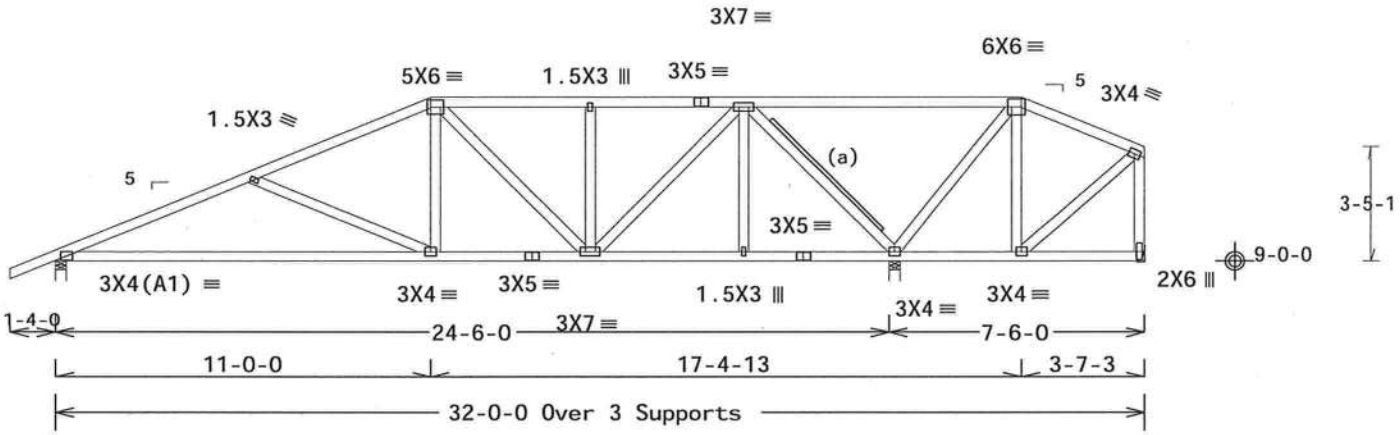
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=890 U=36 W=4"
RL=60/-36

R=740 U=307 W=4"

R=89/-288 U=81 H=Simpson LUS24
Supported Member Face: (2) 0.148"x3" nails
Supporting Member Face: (4) 0.148"x3" nails
Supporting Member : (2) 2x4 SP_#2_12A

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

V0.03.11.0209.19 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KEEN LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6545
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342030
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36150
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H11AS 21'2" Mono Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

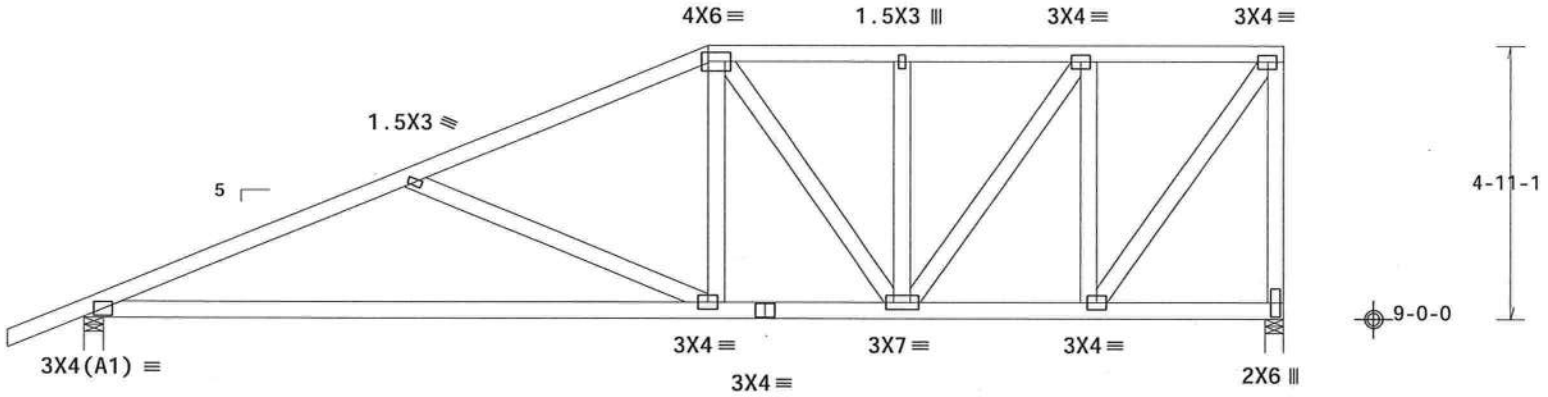
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6546
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342031
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36166
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H13A 37' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

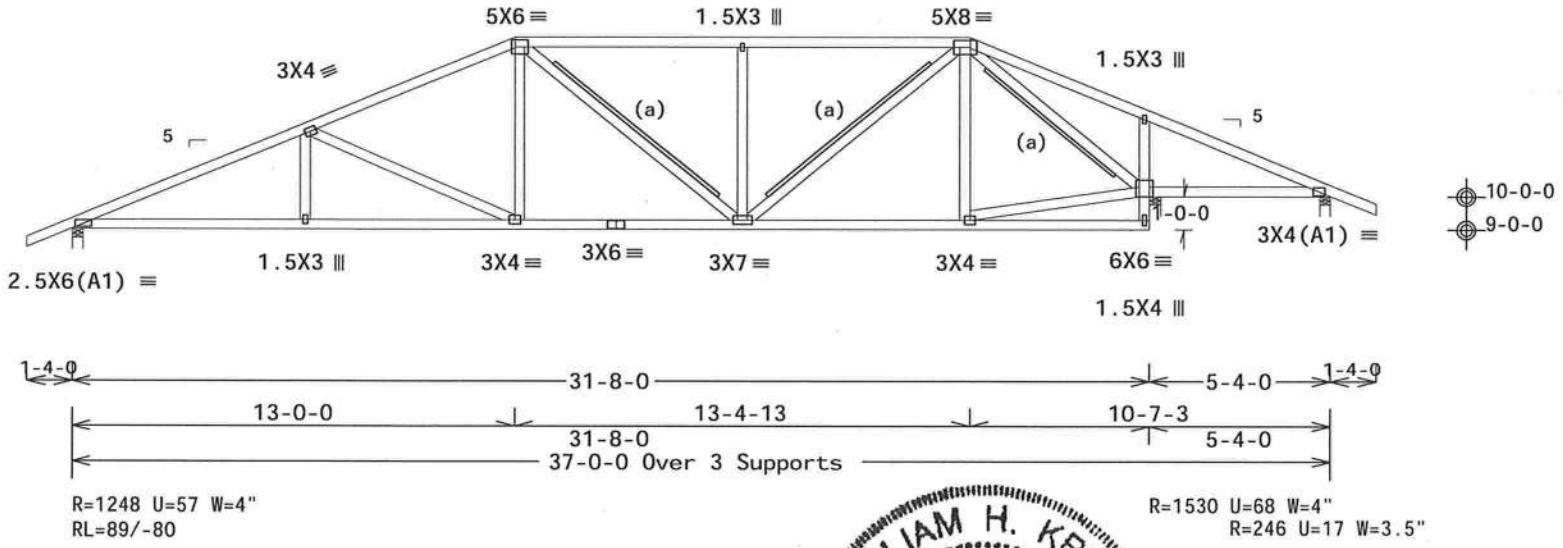
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No.70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6547
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342032
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36148
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H13AS 21'2" Mono Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

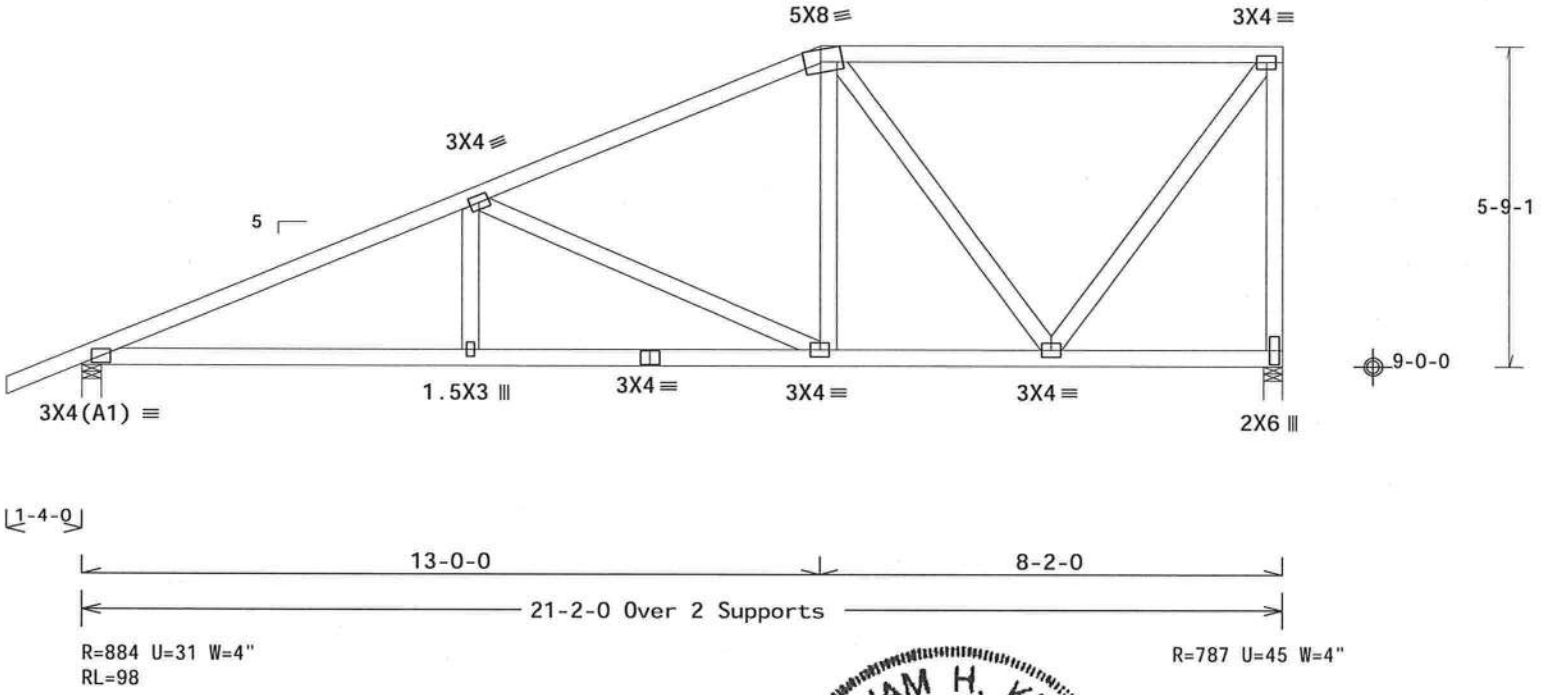
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD) 10.03.11.0209.19 QTY:1 FL/-/4/-/-/R/-

PLT TYP. Wave FT/RT=10%(0%)/0(0) Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSED No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6548
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342033
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36165
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H15A 37' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

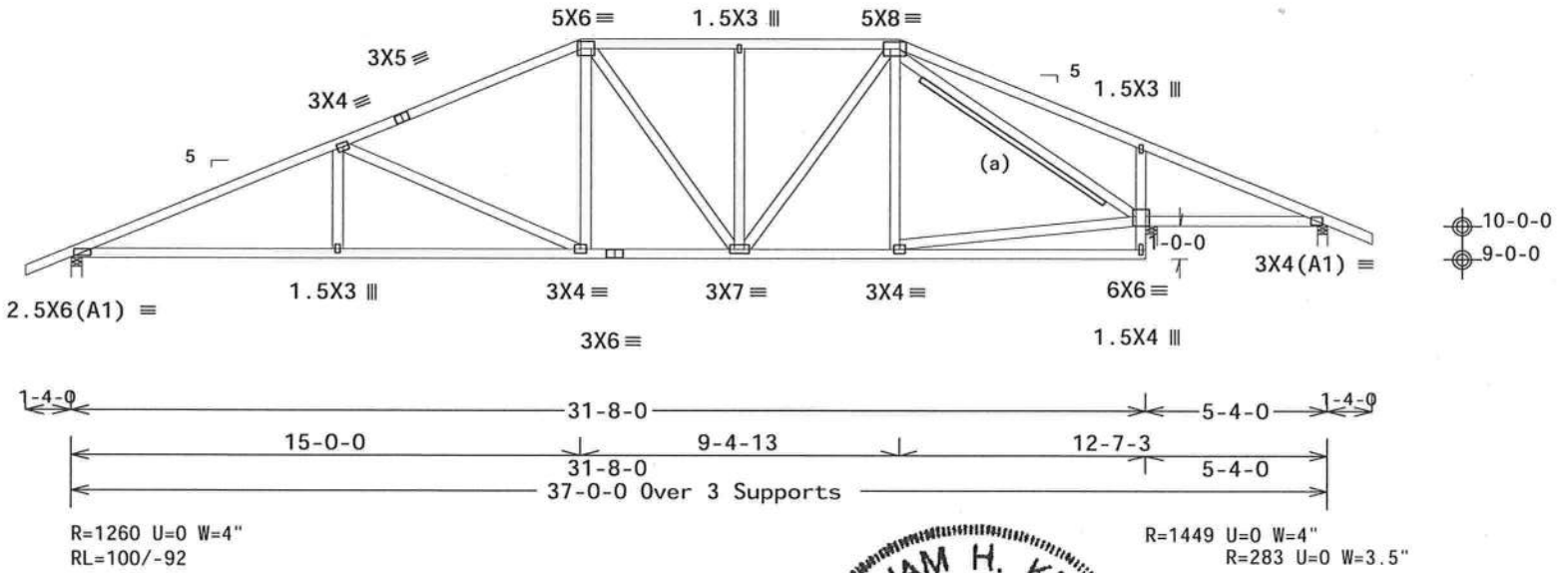
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD; 140.03.11.0209.19 QTY:1 FL/-/4/-/-/R/-

PLT TYP. Wave FT/RT=10%(0%)/0(0) Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF R9114- 6549
TC DL	7.0 PSF	DATE 12/07/12
BC DL	10.0 PSF	DRW HCUSR9114 12342034
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 36149
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H15AS 21'2" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

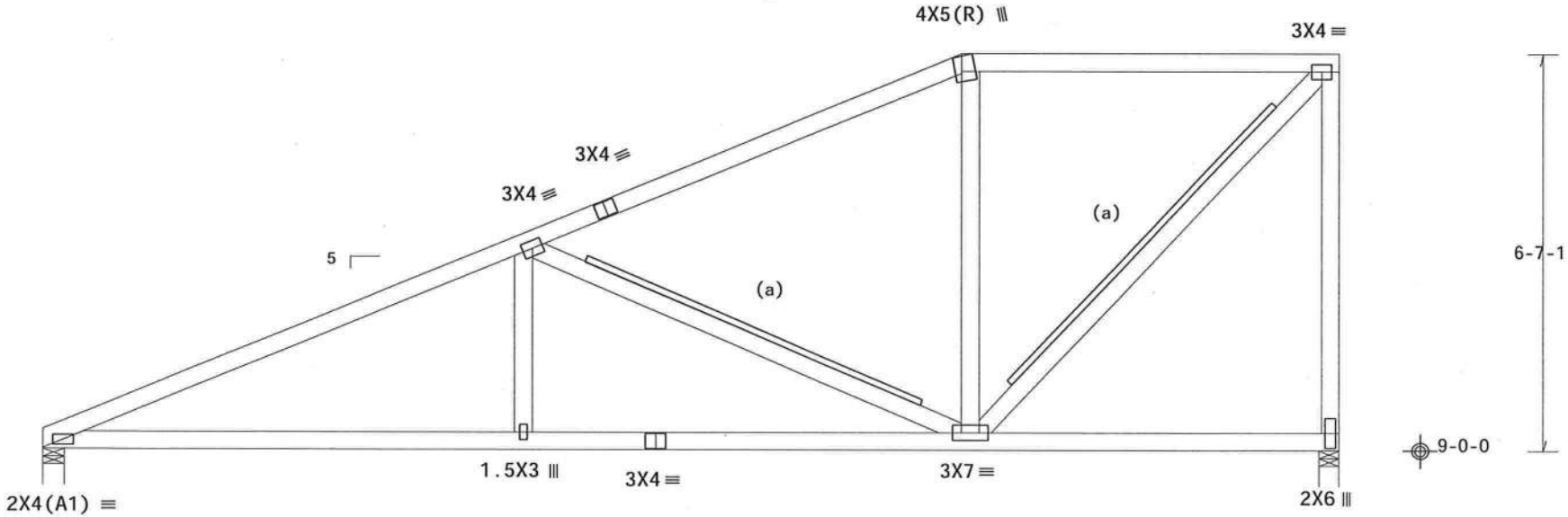
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD) 10.03.11.0209.19 TOT:1 FL/-/4/-/-/R/-

PLT TYP. Wave FT/RT=10%(0%)/0(0) Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6550
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342035
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36164
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H17A 37' Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

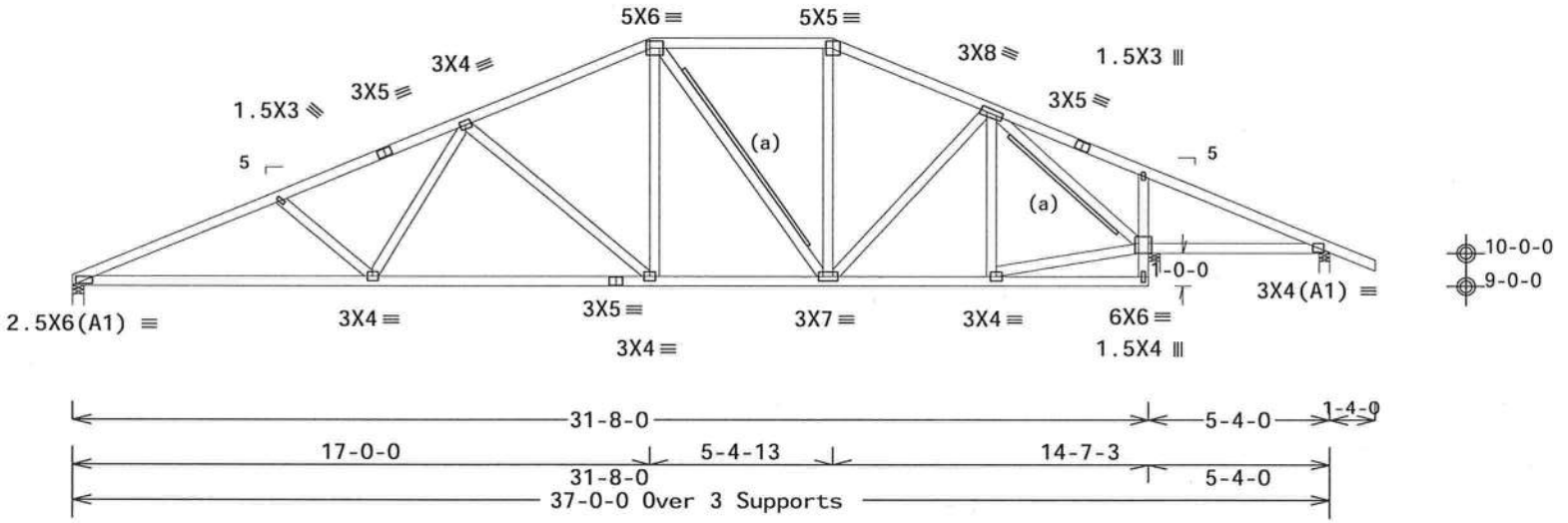
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1167 U=0 W=4"
RL=106/-94

R=1533 U=0 W=4"
R=246 U=21 W=3.5"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD) 10.03.11.0209.19 10Y:1 FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6551
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342036
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36159
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H17AS 21'2" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

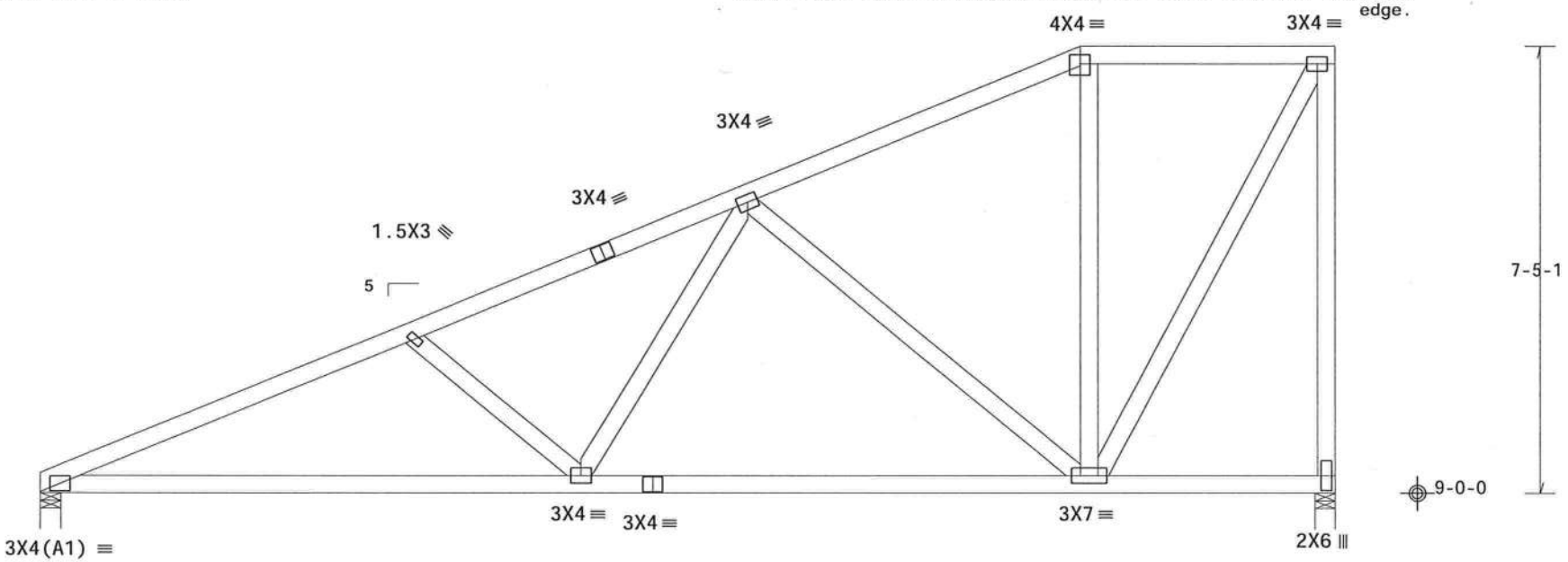
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19

QTY: 1

FL/-/4/-/-/R/-

Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/07/2012

TC LL	20.0 PSF	REF	R9114- 6552
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342037
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36163
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H19A 37' Special)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

Bottom chord checked for 10.00 psf non-concurrent live load.

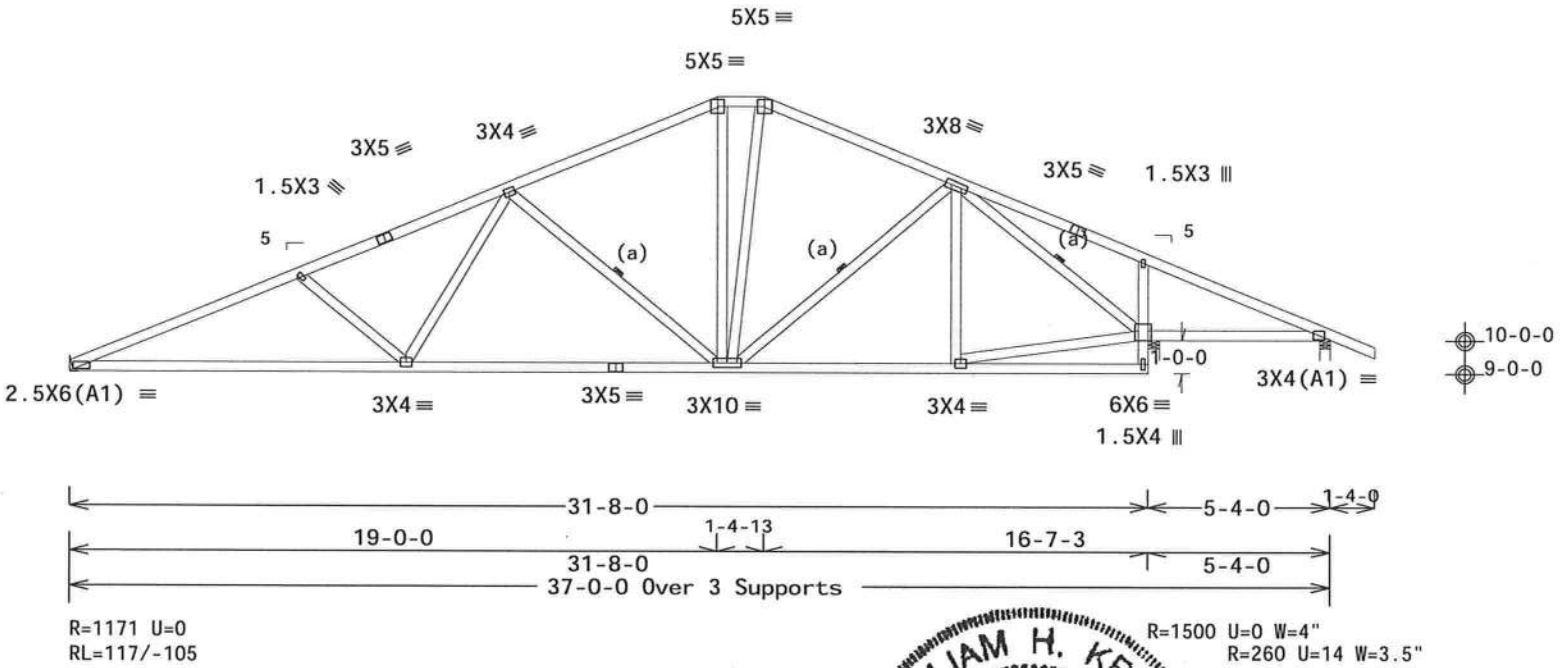
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=1171 U=0
RL=117/-105

R=1500 U=0 W=4"
R=260 U=14 W=3.5"

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY: 1 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/07/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 6553
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342038
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36160
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H19AS 21'2" Special)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

(a) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

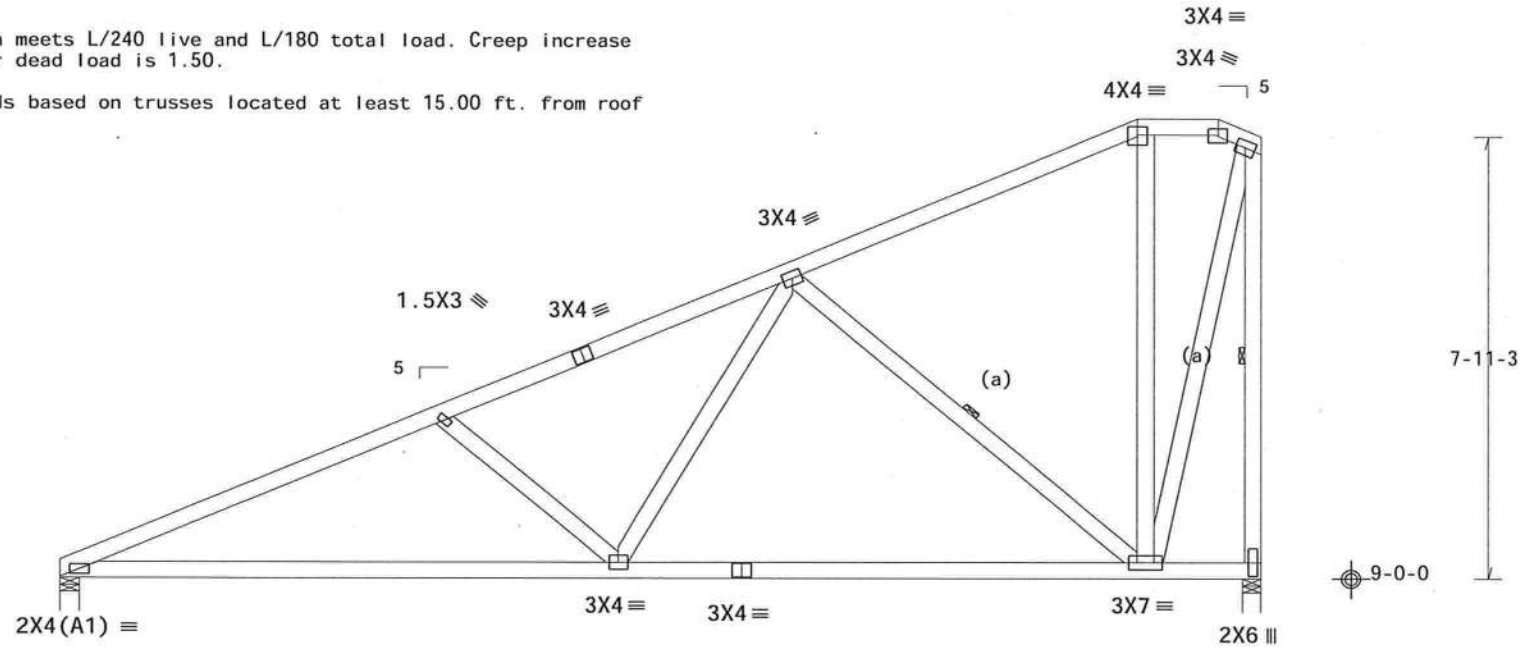
MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY:1 | FL/-/4/-/-/R/- | Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6554
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342039
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36162
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H7A 32' Special Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 55 plf at -1.33 to 55 plf at 7.00
TC- From 28 plf at 7.00 to 28 plf at 19.00
TC- From 28 plf at 19.00 to 28 plf at 32.00
BC- From 4 plf at -1.33 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 7.03
BC- From 10 plf at 7.03 to 10 plf at 16.00
BC- From 10 plf at 16.00 to 10 plf at 32.00
TC- 238.62 lb Conc. Load at 7.03
TC- 167.30 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,21.06,23.06,25.06,27.06,29.06,31.06
BC- 450.24 lb Conc. Load at 7.03
BC- 125.58 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,21.06,23.06,25.06,27.06,29.06,31.06

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bottom chord checked for 10.00 psf non-concurrent live load.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule:0.131"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Negative reaction(s) of -628# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

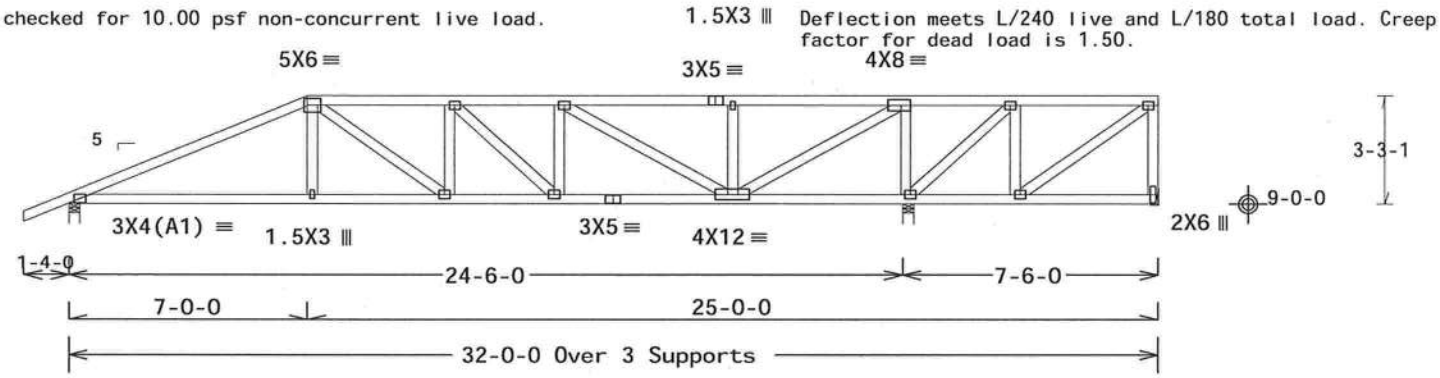
Wind loads and reactions based on MWFRS.

Right end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=5068 U=235 W=4"

R=-629 Rw=22 U=0 H=Simpson LUS26-2
Supported Member Face: (3) 0.162"x3.5" nails
Supporting Member Face: (4) 0.162"x3.5" nails
Supporting Member : (2) 2x6 SP_#2_12A

R=1956 U=99 W=4"

Note: All Plates Are 3X4 Except As Shown.

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF R9114- 6555
TC DL	7.0 PSF	DATE 12/07/12
BC DL	10.0 PSF	DRW HCUSR9114 12342040
BC LL	0.0 PSF	HC-ENG WHK/WHK
TOT.LD.	37.0 PSF	SEQN- 36174
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H7AS 21'2" Mono Hip Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A :W6 2x4 SP_#1_12A:

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

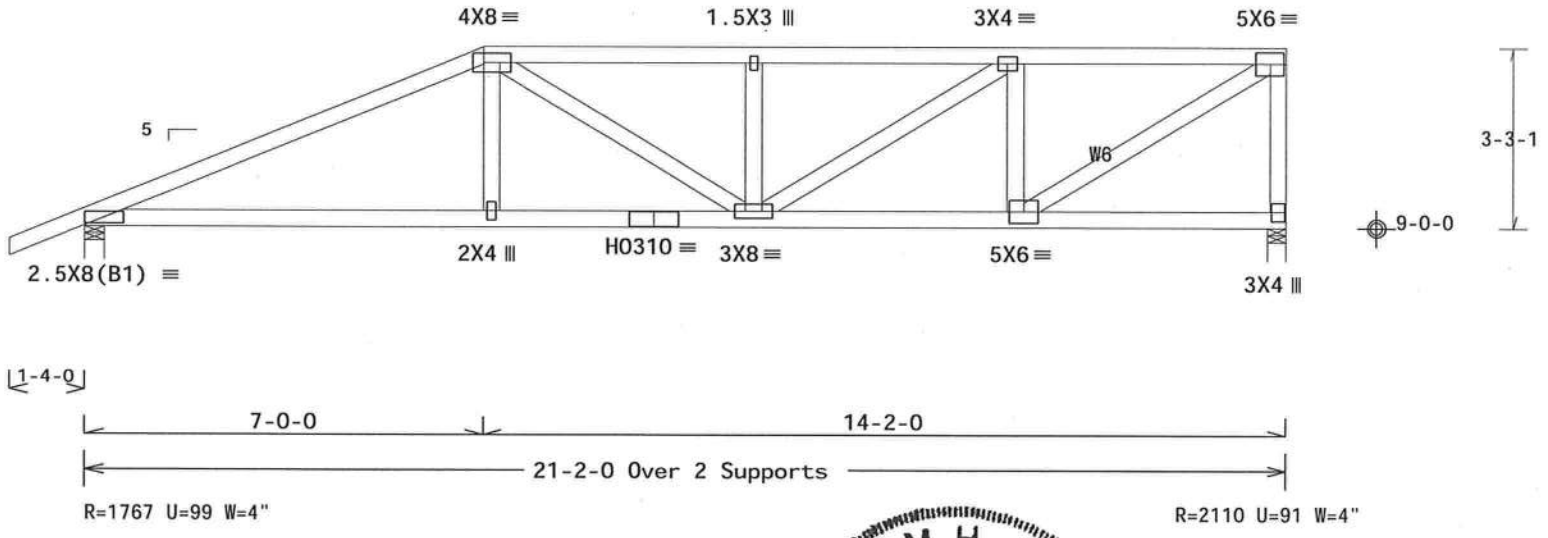
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 55 plf at -1.33 to 55 plf at 7.00
TC- From 28 plf at 7.00 to 28 plf at 21.17
BC- From 4 plf at -1.33 to 4 plf at 0.00
BC- From 20 plf at 0.00 to 20 plf at 7.03
BC- From 10 plf at 7.03 to 10 plf at 10.00
BC- From 10 plf at 10.00 to 10 plf at 21.17
TC- 238.62 lb Conc. Load at 7.03
TC- 167.30 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,20.77
BC- 450.24 lb Conc. Load at 7.03
BC- 125.58 lb Conc. Load at 9.06,11.06,13.06,15.06 17.06,19.06,20.77

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. 20 Gauge HS,Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

10.03.11.0209.19 -01Y:1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6556
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342041
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36175
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H7C 19'8" Stepdown Hip Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS.

(a) 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Special loads

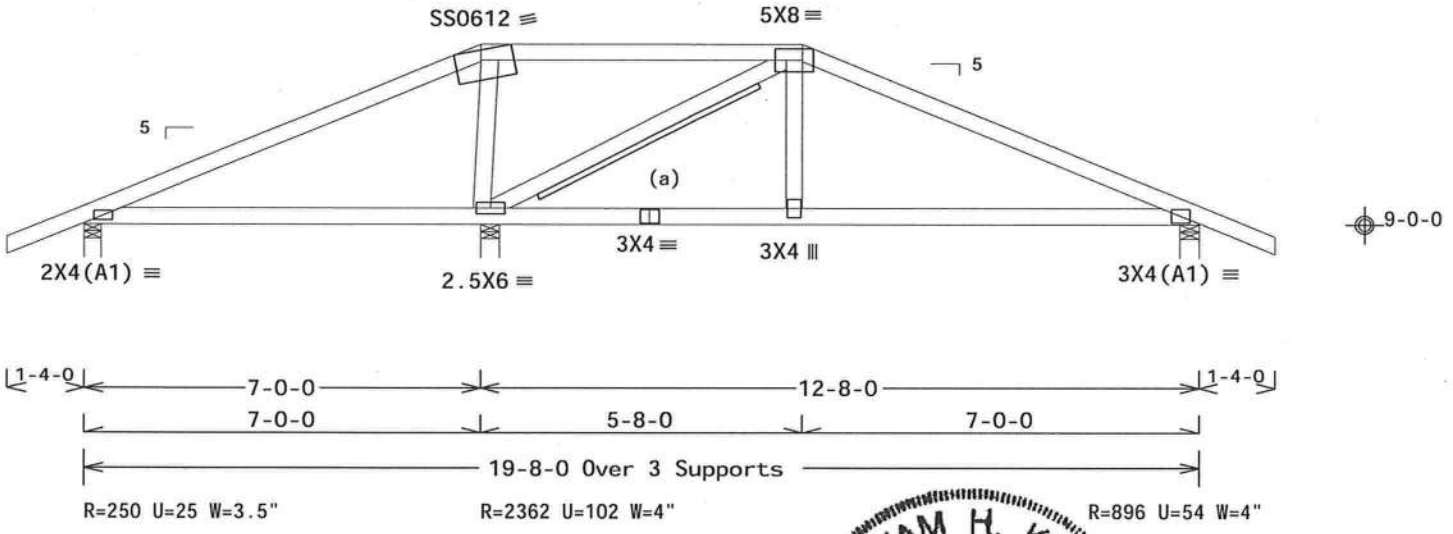
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From	55 plf at	-1.33 to	55 plf at	7.00
TC- From	28 plf at	7.00 to	28 plf at	12.67
TC- From	55 plf at	12.67 to	55 plf at	21.00
BC- From	4 plf at	-1.33 to	4 plf at	0.00
BC- From	20 plf at	0.00 to	20 plf at	7.03
BC- From	10 plf at	7.03 to	10 plf at	10.00
BC- From	10 plf at	10.00 to	10 plf at	12.64
BC- From	20 plf at	12.64 to	20 plf at	19.67
BC- From	4 plf at	19.67 to	4 plf at	21.00

TC- 238.62 lb Conc. Load at 7.03,12.64
TC- 167.30 lb Conc. Load at 9.06,10.60
BC- 450.24 lb Conc. Load at 7.03,12.64
BC- 125.58 lb Conc. Load at 9.06,10.60

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. 18 Gauge HS,Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY: 1 | FL/-/4/-/-/R/- | Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6557
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342042
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36173
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H9A 32' Special)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Negative reaction(s) of -283# MAX. (See below) from a non-wind load case requires uplift connection.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

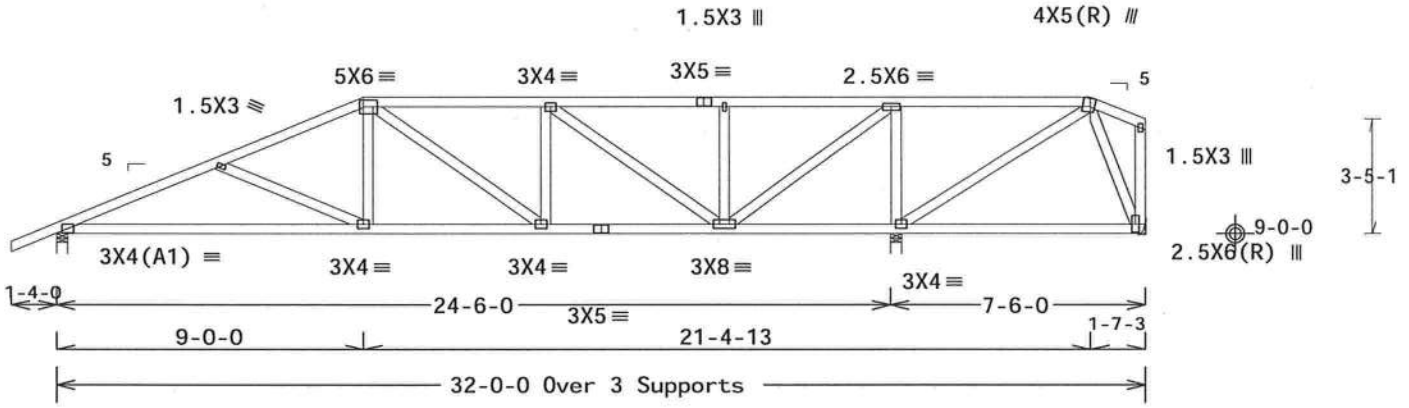
Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



R=901 U=39 W=4"
RL=60/-25

R=727 U=88 W=4"

R=78/-283 U=79 H=Simpson LUS24
Supported Member Face: (2) 0.148"x3" nails
Supporting Member Face: (4) 0.148"x3" nails
Supporting Member : (2) 2x6 SP_#2_12A

Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave | FT/RT=10%(0%)/0(0) | 10.03.11.0209.19 | QTY:1 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRUCK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6558
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342043
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36151
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H9AS 21'2" Mono Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

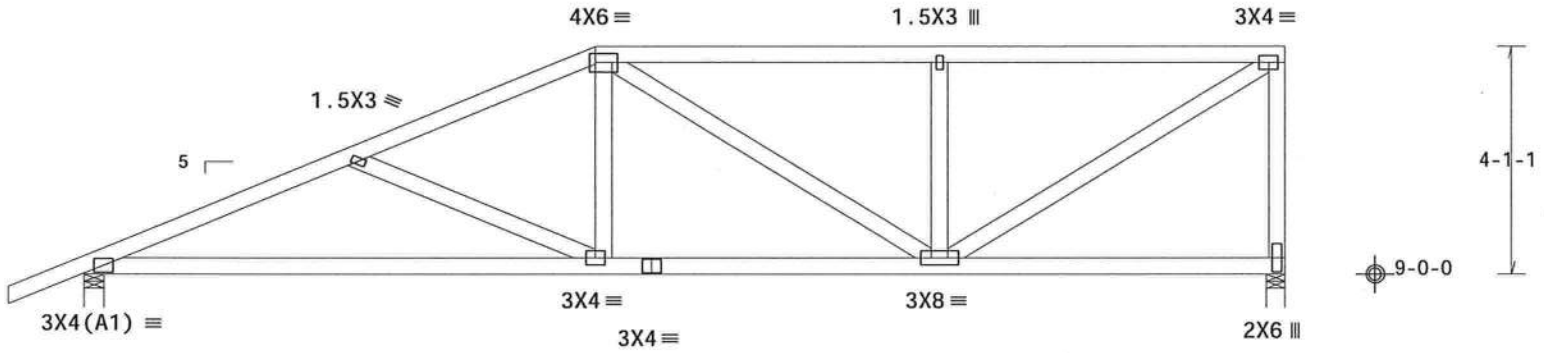
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6559
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342044
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36167
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - H9C 19'8" Stepdown Hip)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

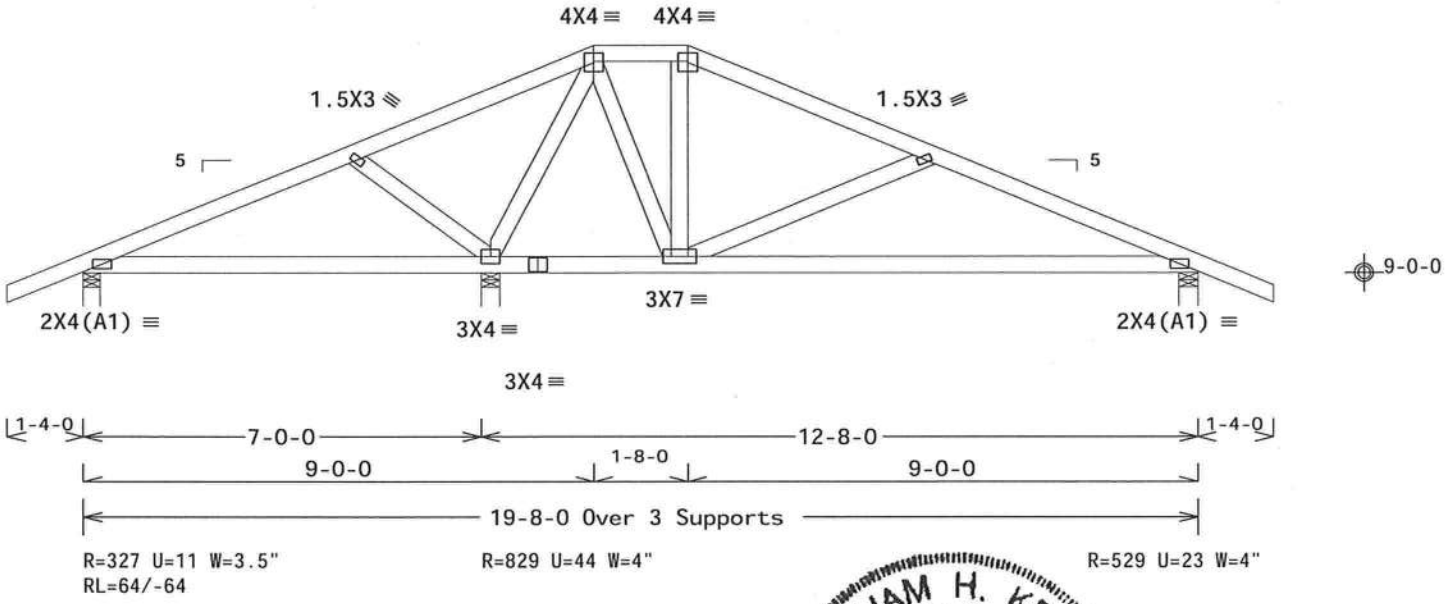
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave

FT/RT=10%(0%)/0(0)

10.03.11.0209.19 QTY:1

FL/-/4/-/-/R/-

Scale =.3125"/Ft.

WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

12/07/2012

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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TC LL	20.0 PSF	REF	R9114- 6560
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342045
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36141
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(12-381--Sparks Construction Wyatt HPG-1600M -- Lake City, FL - HJ7 9'10"13 Hip Jack Girder)

Top chord 2x4 SP_#1_12A
Bot chord 2x4 SP_#1_12A
Webs 2x4 SP_#3__12A

Lumber grades designated with "12A" use design values approved 1/5/2012 by ALSC.

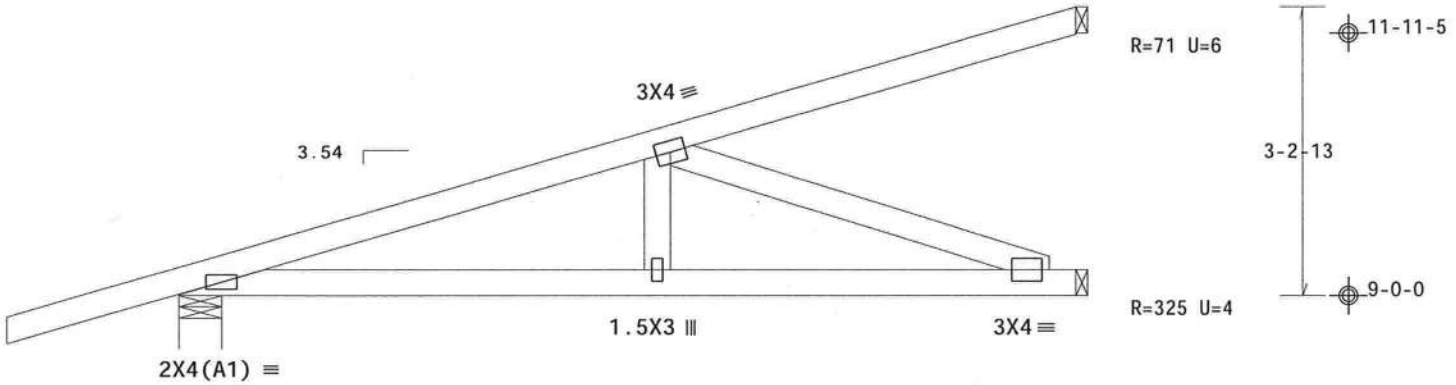
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpi(+/-)=0.18

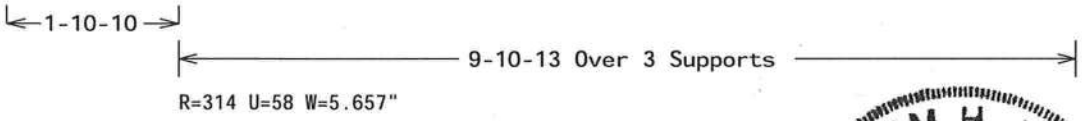
Wind loads and reactions based on MWFRS.

Bottom chord checked for 10.00 psf non-concurrent live load.

Special loads
------(Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 0 plf at -1.89 to 55 plf at 0.00
TC- From 2 plf at 0.00 to 2 plf at 9.90
BC- From 0 plf at -1.89 to 4 plf at 0.00
BC- From 2 plf at 0.00 to 2 plf at 9.90
TC- -19.48 lb Conc. Load at 1.48
TC- 116.04 lb Conc. Load at 4.31
TC- 229.07 lb Conc. Load at 7.13
BC- 15.78 lb Conc. Load at 1.48
BC- 98.48 lb Conc. Load at 4.31
BC- 175.55 lb Conc. Load at 7.13

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: FBC2010Res/TPI-2007(STD)

PLT TYP. Wave FT/RT=10%(0%)/0(0) 10.03.11.0209.19 QTY:4 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL COA #0 278

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
12/07/2012

TC LL	20.0 PSF	REF	R9114- 6561
TC DL	7.0 PSF	DATE	12/07/12
BC DL	10.0 PSF	DRW	HCUSR9114 12342046
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	36171
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1URU9114Z01

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON A TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

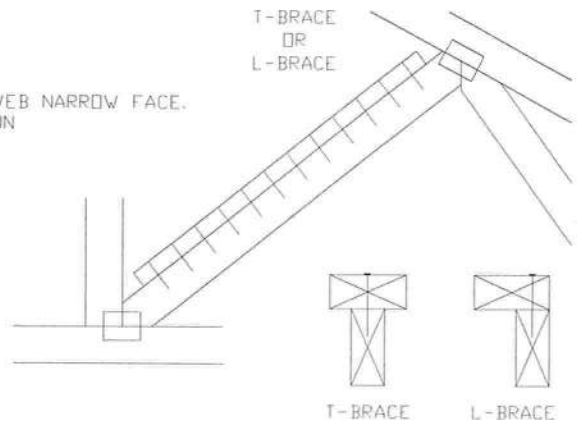
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

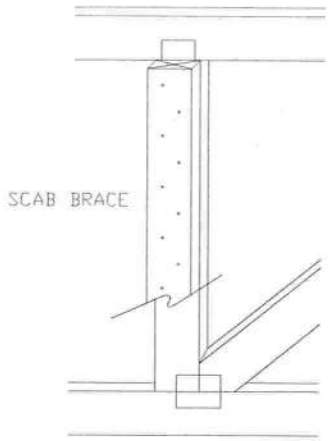
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3",MIN) NAILS.
AT 6" O.C.
BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER
Dec. 12
12/07/2012

Building Components Group Inc.

Earth City, MO 63045

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing & bracing of trusses. ITWBCG connector plates are made of 20/18/16GA (W,H/S/K) ASTM A653 grade 37/40/60 (K/W/H,S) galv. steel. Apply plates to each face of truss, positioned as shown in this and on Joint Details.
A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANST/TPI 1 Sec. 2.
ITW-BCG: www.itwbcg.com; TPI: www.tpinst.com; WTCA: www.sbcindustry.com; ICC: www.iccsafe.org

TC LL	PSF	REF CLB SUBST.
TC DL	PSF	DATE 1/1/09
BC DL	PSF	DRWG BRCLBSUB0109
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace *		(1) 2x4 "L" Brace *		(2) 2x4 "L" Brace **		(1) 2x6 "L" Brace *		(2) 2x6 "L" Brace **	
				Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" o.c.	SPF HF	#1 / #2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 11"	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 11"	7' 4"	9' 3"	9' 10"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 7"	6' 0"	6' 4"	8' 0"	8' 6"	10' 10"	11' 7"	12' 6"	13' 5"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 5"	9' 10"	11' 2"	11' 7"	13' 3"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 6"	9' 0"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 3"	7' 4"	7' 10"	9' 9"	10' 5"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	6' 2"	10' 5"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 9"	9' 9"	10' 5"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 9"	8' 6"	9' 0"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

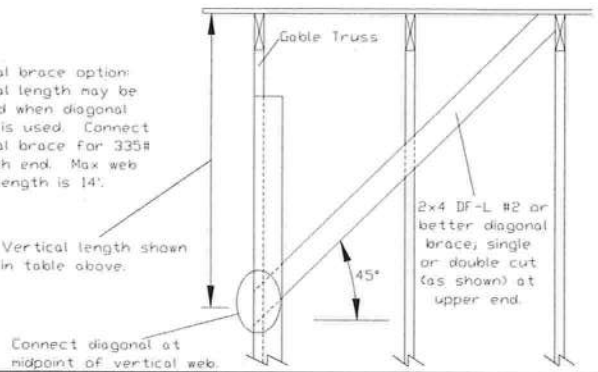
Douglas Fir-Larch
#1
#2

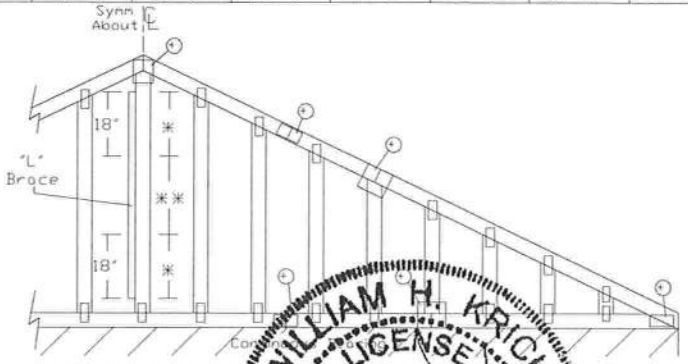
Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 335# at each end. Max web total length is 14'.





Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

So. Pine lumber design values based on the ALSC January, 2012 rulin

Attach "L" braces with 10d (0.128"x3.0" min) nails.

* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greather than 11' 6"	2.5X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

ITW Building Components Group Inc.

Earth City, MO 63045

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WILLIAM H. KRICK LICENSE No. 70861 STATE OF FLORIDA PROFESSIONAL ENGINEER

Dec. Qty: 12

12/07/2012

MAX. TOT. LD. 60 PSF

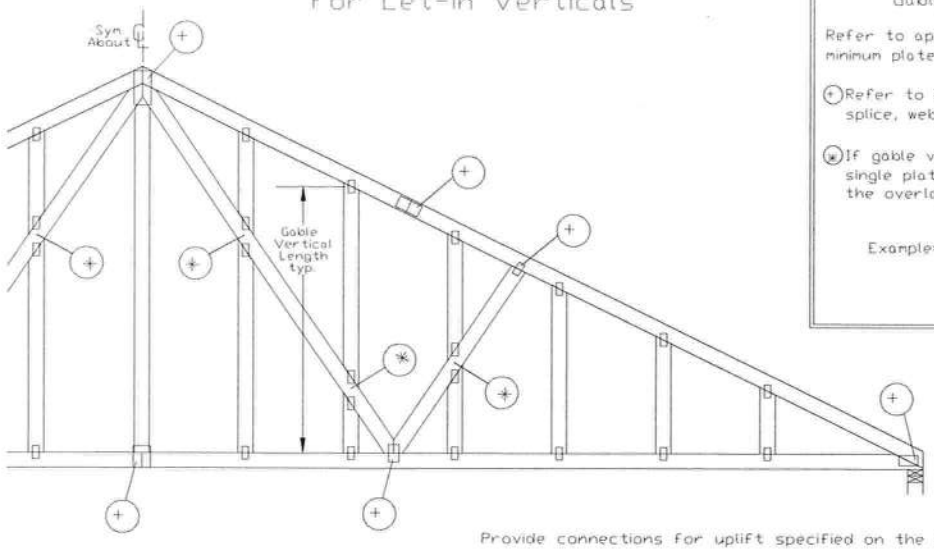
MAX. SPACING 24.0"

REF ASCE7-10-GAB12015

DATE 2/14/12

DRWG A12015ENC100212

Gable Detail
For Let-in Verticals



Gable Truss Plate Sizes

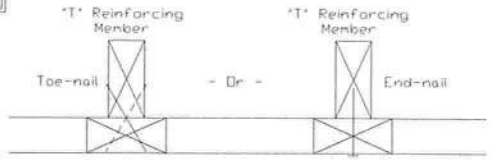
Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

(+) Refer to Engineered truss design for peak, splice, web, and heel plates.

(*) If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:

"T" Reinforcement Attachment Detail



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate ITW gable detail).

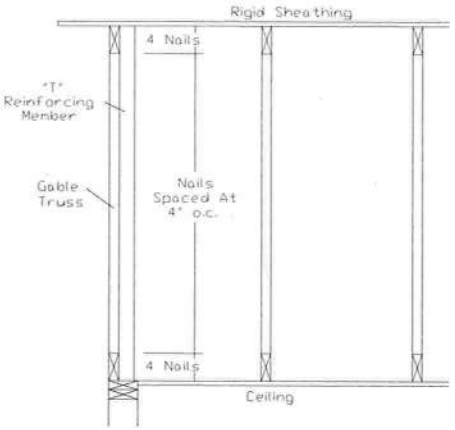
Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
- ASCE 7-10 Wind Speed = 120 mph
- Mean Roof Height = 30 ft, Kzt = 1.00
- Gable Vertical = 24"o.c. SP #3
- "T" Reinforcing Member Size = 2x4
- "T" Brace Increase (From Above) = 30% = 1.30
- (1) 2x4 "L" Brace Length = 8' 7"
- Maximum "T" Reinforced Gable Vertical Length
  1.30 x 8' 7" = 11' 2"



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
- 10d Common (0.148"x 3",min) Nails at 4" o.c. plus
- (4) nails in the top and bottom chords.

Toenailed Nails:
- 10d Common (0.148"x3",min) Toenails at 4" o.c. plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate ITW gable detail for ASCE wind load.

ASCE 7-98 Gable Detail Drawings
A13015980109, A12015980109, A11015980109, A10015980109,
A13030980109, A12030980109, A11030980109, A10030980109

ASCE 7-02 Gable Detail Drawings
A13015020109, A12015020109, A11015020109, A10015020109, A14015020109,
A13030020109, A12030020109, A11030020109, A10030020109, A14030020109

ASCE 7-05 Gable Detail Drawings
A13015050109, A12015050109, A11015050109, A10015050109, A14015050109,
A13030050109, A12030050109, A11030050109, A10030050109, A14030050109

ASCE 7-10 Gable Detail Drawings
A11515ENC100212, A12015ENC100212, A14015ENC100212, [illegible]212,
A18015ENC100212, A20015ENC100212, A20015END[illegible]D100[illegible]
A11530ENC100212, A12030ENC100212, A14030EN[illegible]0[illegible]C10[illegible]
A18030ENC100212, A20030ENC100212, A2003[illegible]D[illegible]0[illegible]

See appropriate ITW gable detail for maximum [illegible] for let-in gable vertical length.

12/07/2012

ITW Building Components Group Inc.

Earth City, MO 63045

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ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.or[illegible]

MAX. TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX. SPACING	24.0"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

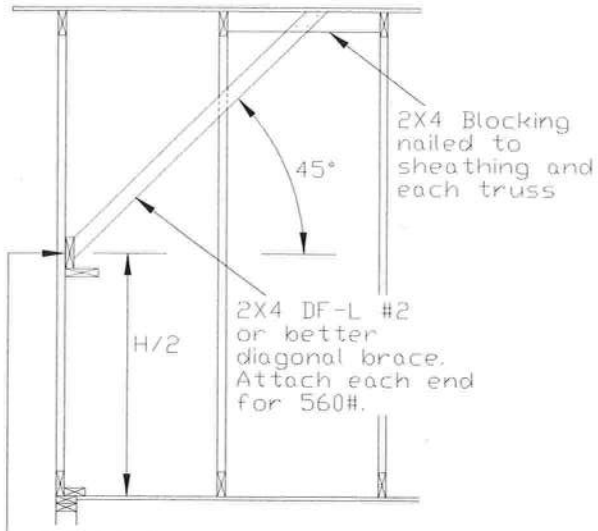
Nails: 10d box or gun (0.128"x3",min) nails.

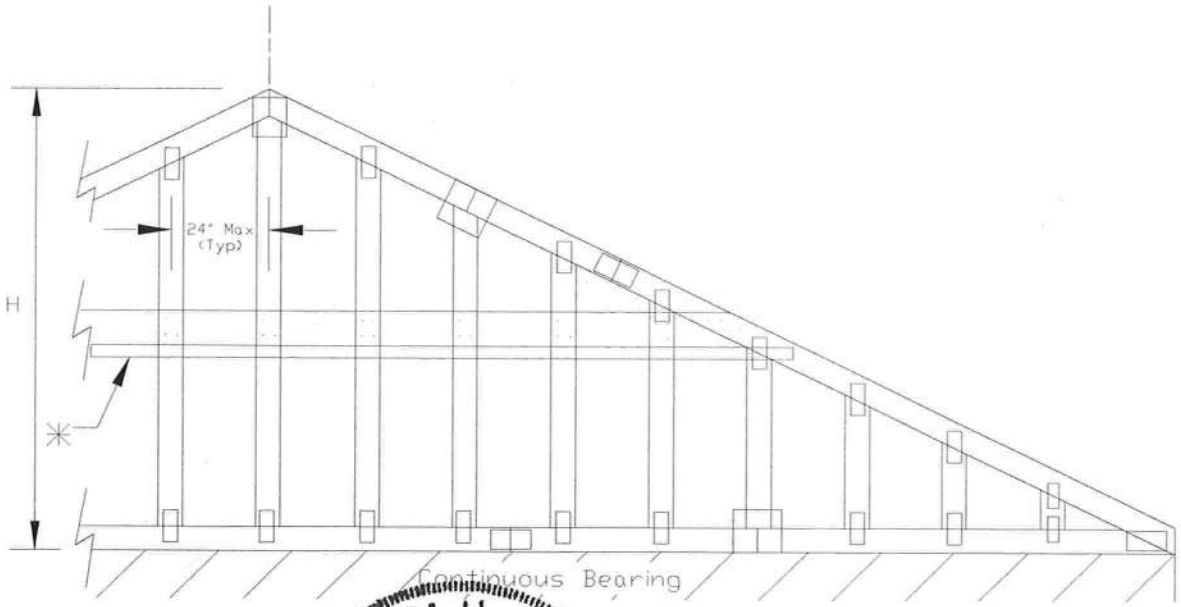
H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC10).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC10).

✳ Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.





WILLIAM H. KRICK
LICENSE
No. 70861
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Dec 07, 12
12/07/2012

ITW Building Components Group Inc.

Earth City, MO 63045

WARNING! READ AND FOLLOW ALL NOTES ON THIS DRAWING!
IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see this job's general notes page and these web sites:
ITWBCG: www.itwbcg.com; TPI: www.tpinst.org; WTCA: www.sbcindustry.org; ICC: www.iccsafe.or

MAX. TOT. LD. 60 PSF

MAX. SPACING

REF	GE WHALER
DATE	2/14/12
DRWG	GABRST100212