1) DESIGN CRITERIA

A. STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE FOLLOWING CODES AND CRITERIA:
1. 2017 FLORIDA BUILDING CODE
2. ASCE 7-10, MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
3. AISC360-10, AMERICAN INSTITUTE OF STEEL CONSTRUCTION
4. NDS 2012, NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
5. AISI S100-12, NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS

B. BUILDING CLASSIFICATION
1. BUILDING OCCUPANCY CLASSIFICATION: R

C. DEAD LOADS
1. 5 PSF + STEEL SELF WEIGHT

D. LIVE LOADS
1. ROOF = 20.0 PSF (REDUCIBLE)

E. WIND LOADS DESIGN CRITERIA

DESIGN WIND SPEED, V_{ULT}	=	130 MPH
NOMINAL WIND SPEED, V_{ASD}	=	102 MPH
RISK CATEGORY	=	II
EXPOSURE CATEGORY	=	C
INTERNAL PRESSURE, GC_{pi}	=	+/- 0.18
VELOCITY PRESSURE, K_d	=	0.85

F. SEISMIC DESIGN CRITERIA

DESIGN CATEGORY	=	B
IMPORTANCE FACTOR, I_e	=	1.00

MAPPED SPECTRAL RESPONSE ACCELERATION:

S_S	=	0.099 g
S_1	=	0.056 g

SITE CLASS = D

SPECTRAL RESPONSE COEFFICIENTS:

S_{DS}	=	0.105 g
S_{D1}	=	0.090 g

SEISMIC FORCE RESISTING SYSTEM:
STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE

R	=	3.00
C_S	=	0.035

ANALYSIS PROCEDURE USED:
EQUIVALENT LATERAL FORCE

G. SNOW LOADS

GROUND SNOW LOAD, P_g	=	00.0 PSF
FLAT ROOF SNOW LOAD, P_f	=	0.0 PSF
SNOW EXPOSURE FACTOR, C_e	=	1.00
THERMAL FACTOR, C_t	=	1.20

2) FOUNDATIONS

A. FOUNDATION IS TO BE DESIGNED BY OTHERS BASED ON REACTIONS PROVIDED ON SHEET E1.1

B. MAXIMUM ANCHOR BOLT DIAMETER IS 5/8"; MINIMUM ANCHOR BOLT DIAMETER IS 1/2"

C. PROVIDE WASHERS ON ALL ANCHOR BOLTS

3) STRUCTURAL STEEL

A. MATERIAL
1. HOT ROLLED STRUCTURAL MEMBERS. ALL HOT ROLLED STEEL PLATES, SHAPES, SHEET PILING, AND BARS SHALL BE NEW STEEL CONFORMING TO ASTM SPECIFICATION A6.
2. ASTM SPECIFICATION AND GRADE UNLESS NOTED OTHERWISE ON THE DRAWINGS, STRUCTURAL STEEL SHALL BE AS FOLLOWS:

a) STRUCTURAL ANGLES:	A572, Gr. 50
b) STRUCTURAL ROD:	A572, Gr. 50
c) W-SHAPES:	A992
d) PLATE:	A36
e) S, C, M, AND MC SHAPES:	A36
f)HSS, RECTANGULAR:	A500, Gr. C - Fy = 50 ksi
g) HSS, ROUND:	A500, Gr. C - Fy = 46 ksi
h) PIPE:	A53, Gr. B

B. STRUCTURAL BOLTS AND THREADED FASTENERS
1. ASTM A325 or SAE J429 GRADE 5 BOLTS U.N.O.
2. ALL BOLTS TO BE BEARING TYPE BOLTS WITH THREADS ASSUMED TO BE IN BEARING SURFACE (TYPE N)
3. ALL BOLTS TO BE TIGHTENED TO THE "PRETENSIONED JOINTS" REQUIREMENTS PER RCSC SPECIFICATION SECTION 4.2 UNLESS NOTED OTHERWISE. ANY OF THE INSTALLATION METHODS SPECIFIED IN SECTION 8.2 OF THE RCSC ARE PERMITTED (TURN OF NUT, CALIBRATED WRENCH, ETC.)

C. WELDING
1. UNLESS NOTED OTHERWISE, ELECTRODES FOR WELDING SHALL CONFORM TO E70XX
2. ALL WELDING SHALL CONFORM TO AWS (AMERICAN WELDING SOCIETY) SPECIFICATION D1.1

4) COLD-FORMED STEEL METAL FRAMING

A. ALL ROOF AND WALL SHEET METAL SHALL BE 29 GA., 80-KSI MINIMUM, PANEL-LOC PLUS™ OR EQUIVALENT WITH THE FOLLOWING MINIMUM SECTION PROPERTIES:

$Ix(T)_{MIN} = 0.0110\ in^4/ft$
$Ix(B)_{MIN} = 0.0073\ in^4/ft$
$Sx(T)_{MIN} = 0.0181\ in^3/ft$
$Sx(B)_{MIN} = 0.0160\ in^3/ft$

B. ALL FRAMING MEMBERS SHALL BE FORMED FROM CORROSION-RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTM A446, WITH A MINIMUM YIELD STRENGTH OF 50 KSI AS INDICATED.

C. ALL MEMBERS SHOWN ARE STANDARD DESIGNATIONS OF THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA)

D. CONNECTIONS
1. FASTENING OF COMPONENTS SHALL BE BY SELF-DRILLING SCREWS OR BY WELDING AS DEFINED BELOW U.N.O. ON THE DRAWINGS
2. SELF DRILLING SCREW CONNECTIONS
   a) A MINIMUM OF THREE (3) EXPOSED THREADS SHALL PENETRATE THROUGH ALL JOINED MATERIALS

5) TIMBER DESIGN

A. LIMITATIONS
1. ALL WOOD CONSTRUCTION IS TO BE PROVIDED BY OTHERS. PROVIDE BRIDGING AS REQUIRED

B. MATERIAL
1. ALL DIMENSIONAL LUMBER FRAMING MEMBERS ARE TO BE #2 YELLOW SOUTHERN PINE. ALTERNATIVELY LUMBER FRAMING SHALL MEET THE FOLLOWING MINIMUM BASE DESIGN VALUES IN ACCORDANCE WITH THE REFERENCED ISSUE OF THE NDS:
   1.1.1. Fb = 1,500 psi
   1.1.2. E = 1,600,000 psi
   1.1.3. Fv = 175 psi
   1.1.4. Fc|| = 1,650 psi
2. ROOF PURLINS: 2x8, 2'-0" C/C MAX, AND AT ENDS
3. WALL GIRTS: 2x6, 2'-0" C/C MAX, AND AT ENDS
4. LUMBER WITH MOISTURE CONTENT NOT EXCEEDING 19%. LUMBER SHALL BE GRADE STAMPED WITH THE APPROPRIATE WWPA OR SPIB STAMP INDICATING COMPLIANCE WITH PS-20 LUMBER DEFECTS OCCURRING IN THE CONNECTOR PLATE

C. CONNECTIONS
1. PROVIDE A MINIMUM OF 1½" PENETRATION INTO CROSS SECTION OF EACH MEMBER BEING JOINED. SIZE SCREWS ACCORDINGLY

6) MISCELLANEOUS

A. WALK DOORS AND WINDOWS
1. WALK DOORS AND WINDOWS MAY BE FIELD LOCATED. NO WIND ROD BRACING OR COLUMN CHORD BRACING SHALL BE CUT TO ACCOMMODATE WINDOWS OR DOOR

B. INSPECTION REQUIREMENTS
1. AS BUILDING IS A UTILITY BUILDING THAT IS ACCESSORY TO A RESIDENCE, BUILDING IS EXEMPT FROM SPECIAL INSPECTIONS PER SECTION 1704.2 OF THE INTERNATIONAL BUILDING CODE

7) LEGEND

EL	= RELATIVE ELEVATION W/ RESPECT TO GROUND FLOOR
C/C	= CENTER TO CENTER
C/L	= CENTERLINE
CONT	= CONTINUOUS
PSF	= POUNDS PER SQUARE FOOT
TYP	= TYPICAL
UNO	= UNLESS NOTED OTHERWISE
CB	= CHORD BRACING
EXT.	= EXTENSION

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CA 29951

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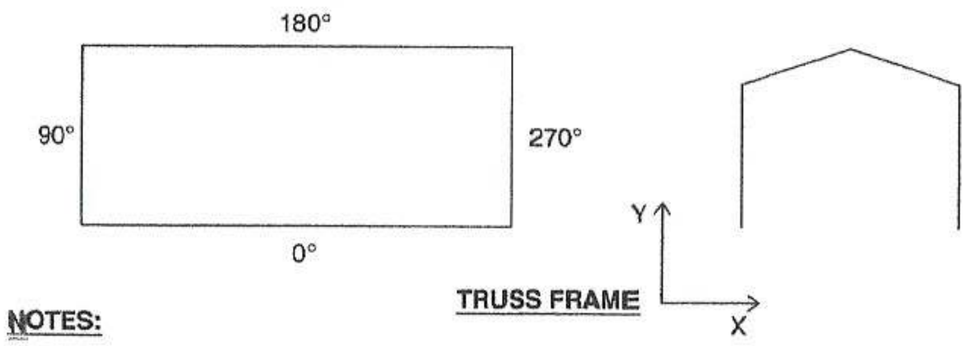
SHEET
E1.0

TRUSS FRAME COLUMN REACTIONS			END WALL COLUMNS
LOAD CASE	HORIZONTAL REACTION, X-DIR. (KIP)	VERTICAL REACTION, Y-DIR. (KIP)	HORIZONTAL REACTION, X-DIR. (KIP)
DEAD LOAD	0.69	3.13	0.18
ROOF LIVE	0.65	3.96	~
SNOW LOAD	~	~	~
WIND 0°	2.51	-6.67	4.98
WIND 90°	0.81	-9.28	4.98
WIND 180°	2.51	-6.67	5.30
WIND 270°	0.81	-9.28	5.30



TRUSS FRAME

NOTES:

- ALL REACTIONS ARE SERVICE LEVEL (UNFACTORED)
- END WALL COLUMNS ARE DESIGNED AS PIN-ROLLER, THEREFORE, ONLY HORIZONTAL REACTIONS ARE PROVIDED
- NEGATIVE VERTICAL REACTION (Y-AXIS) INDICATES UPLIFT

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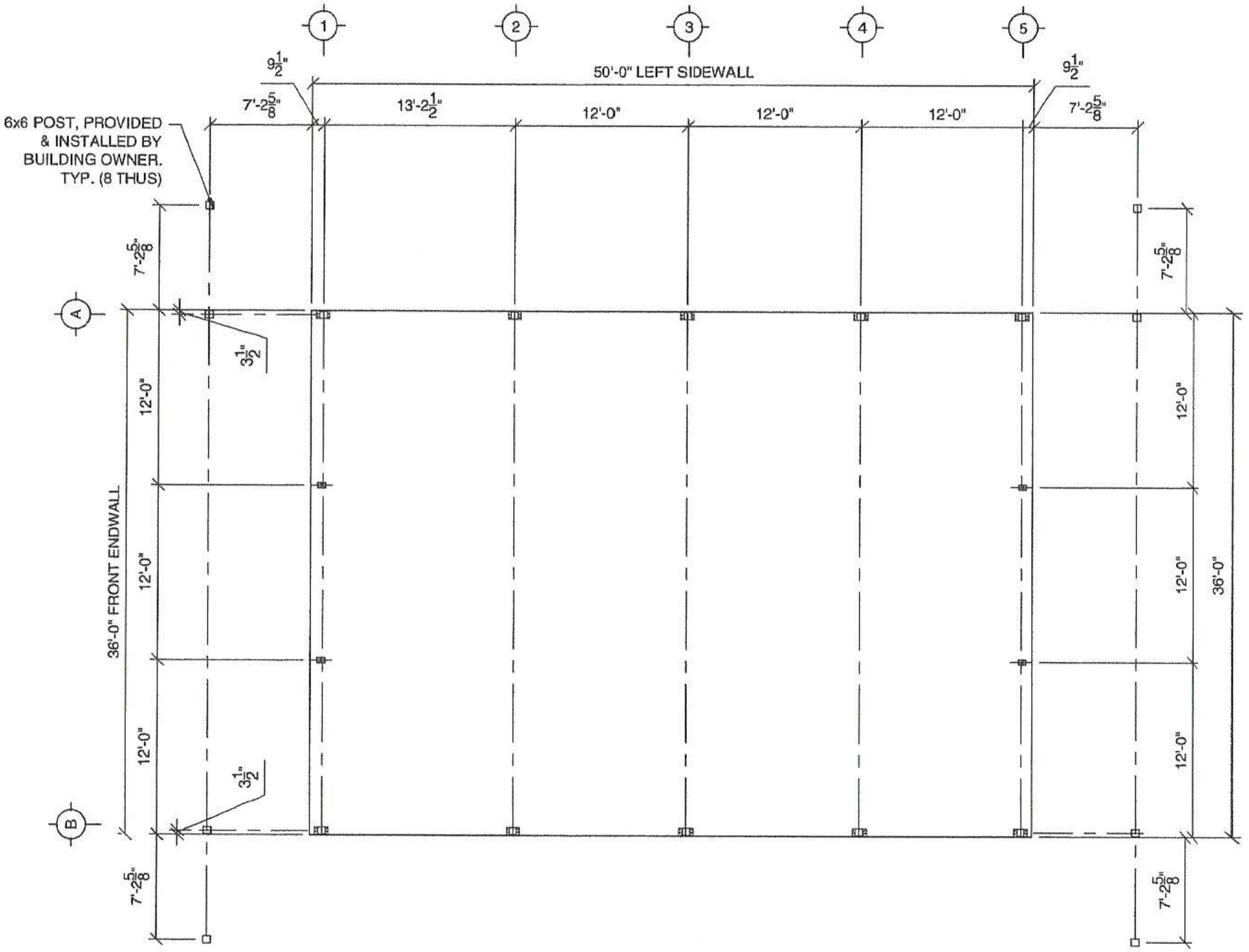

1 ANCHOR BOLT SETTING PLAN
SCALE: 1/8" = 1'-0"

NOTES, ANCHOR BOLT SETTING PLAN:

1. FOUNDATION TO BE DESIGNED BY REGISTERED ENGINEER IN THE STATE THE PROJECT IS LOCATED FOR THE FORCES INDICATED ON SHEET E1.1

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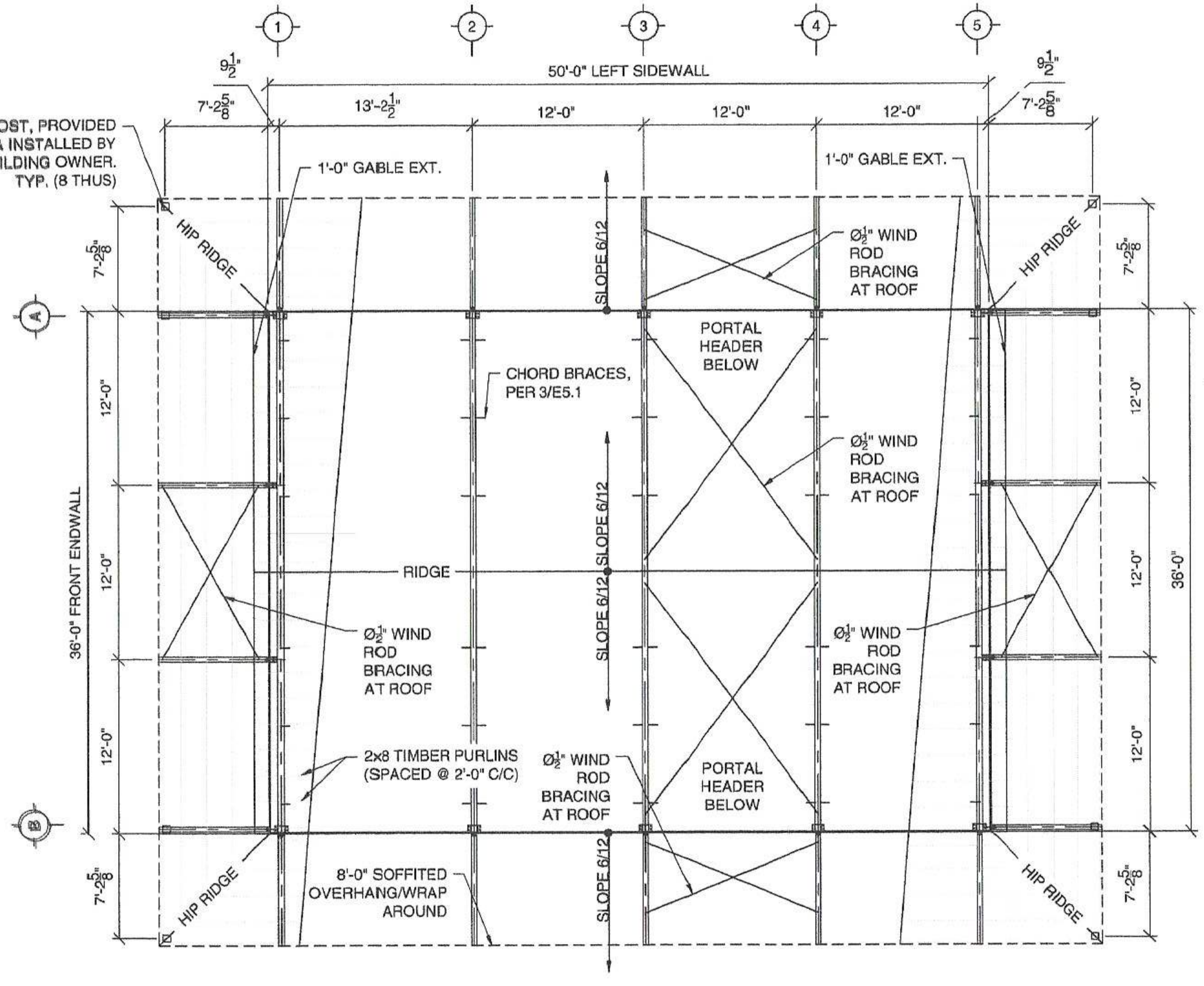

1 ROOF FRAMING PLAN
SCALE: 1/8" = 1'-0"

NOTES, ROOF FRAMING PLAN:

1. BOTTOM CHORD BRACING (MARKED WITH '-' ON PLANS) IS VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL NOT BE REMOVED, RELOCATED, OR MODIFIED IN ANY WAY. ERECTOR SHALL CONFIRM THAT BRACING IS INSTALLED PER THESE DRAWINGS BEFORE TURNING THE BUILDING OVER TO THE OWNER. BRACING TO BE PRESENT ON EVERY TRUSS
2. CONNECTIONS ARE VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL BE INSTALLED PER TYPICAL CONNECTION DETAILS ON E5.0. BUILDING IS NOT STABLE UNTIL ALL BOLTS ARE TIGHTENED TO REQUIRED TIGHTNESS PER GENERAL NOTES ON E1.0
3. ROOF DECKING AND PURLINS ARE VITAL THE STABILITY OF THE STEEL BUILDING. BUILDING IS NOT STABLE UNTIL ALL DECKING IS INSTALLED. SEE ATTACHMENT PATTERN ON E5.0

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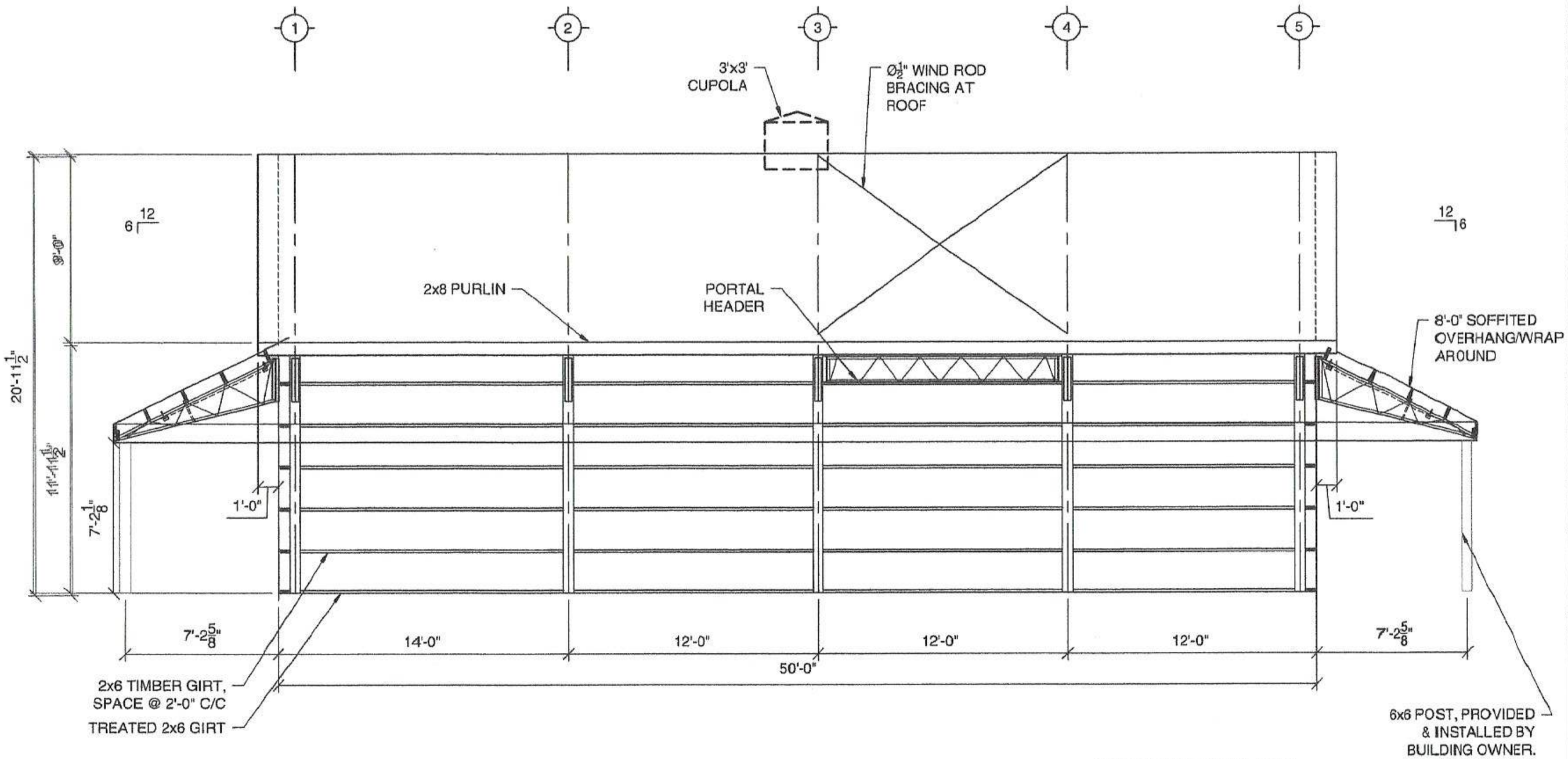

1 ELEVATION AT RIGHT SIDEWALL
SCALE: 3/16" = 1'-0"

NOTES, WALL ELEVATIONS:

1. COLUMN CHORD BRACING IS VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL NOT BE REMOVED, RELOCATED, OR MODIFIED IN ANY WAY. ERECTOR SHALL CONFIRM THAT BRACING IS INSTALLED PER THESE DRAWINGS BEFORE TURNING THE BUILDING OVER TO THE OWNER. BRACING TO BE PRESENT ON EVERY TRUSS
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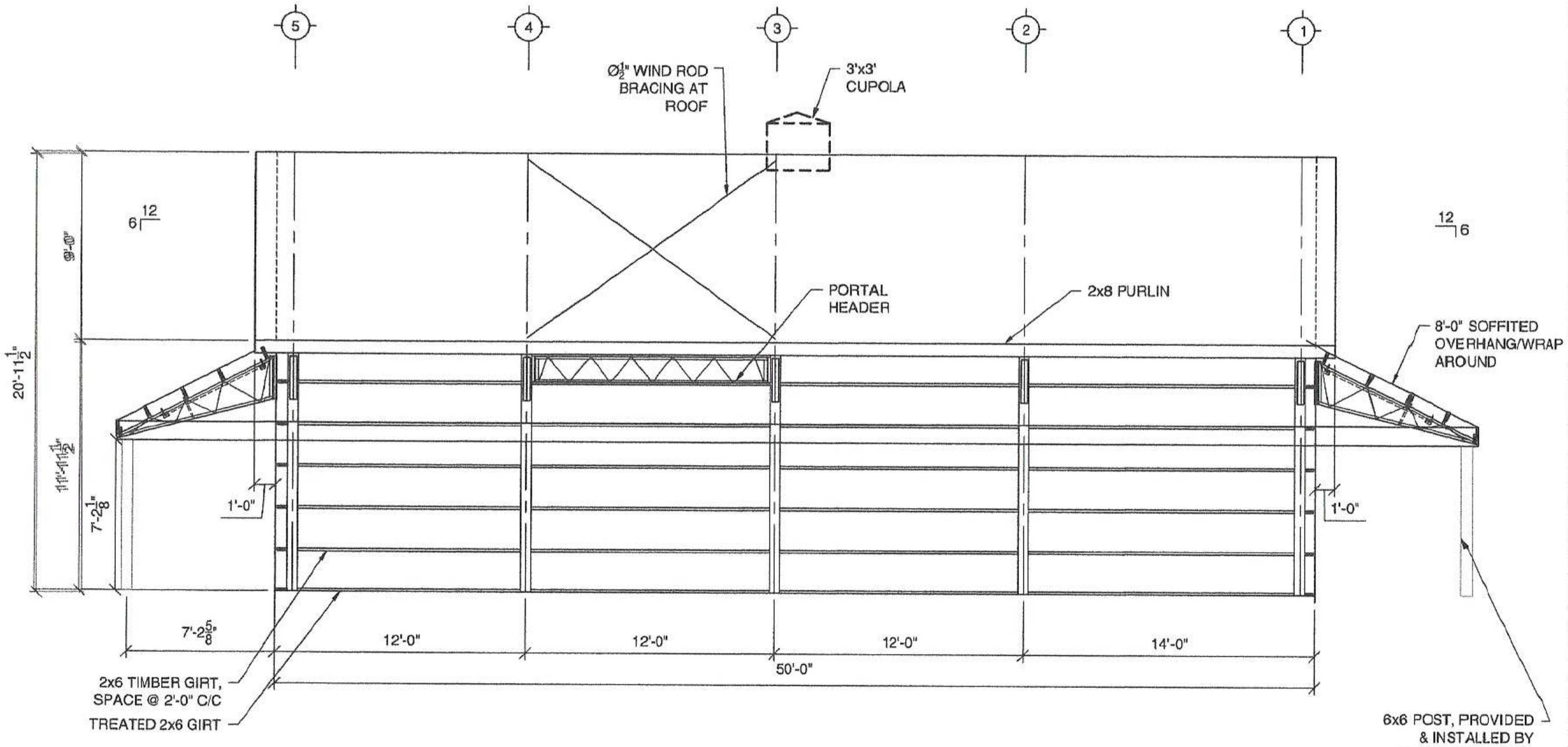

1 ELEVATION AT LEFT SIDEWALL
SCALE: 3/16" = 1'-0"

NOTES, WALL ELEVATIONS:

1. COLUMN CHORD BRACING IS VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL NOT BE REMOVED, RELOCATED, OR MODIFIED IN ANY WAY. ERECTOR SHALL CONFIRM THAT BRACING IS INSTALLED PER THESE DRAWINGS BEFORE TURNING THE BUILDING OVER TO THE OWNER. BRACING TO BE PRESENT ON EVERY TRUSS
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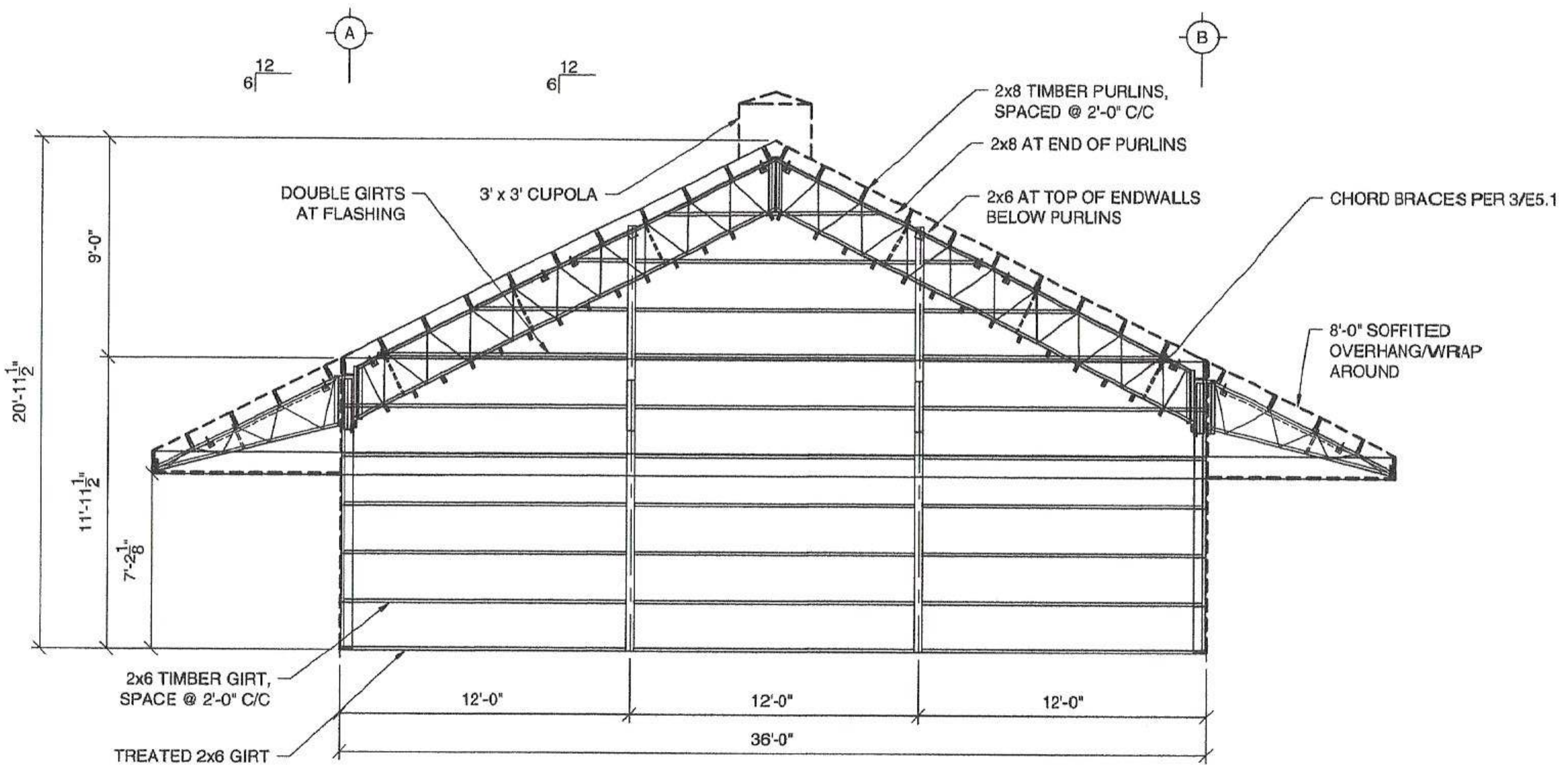

1 ELEVATION AT FRONT ENDWALL
SCALE: 3/16" = 1'-0"

NOTES, WALL ELEVATIONS:

1. COLUMN CHORD BRACING IS VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL NOT BE REMOVED, RELOCATED, OR MODIFIED IN ANY WAY. ERECTOR SHALL CONFIRM THAT BRACING IS INSTALLED PER THESE DRAWINGS BEFORE TURNING THE BUILDING OVER TO THE OWNER. BRACING TO BE PRESENT ON EVERY TRUSS
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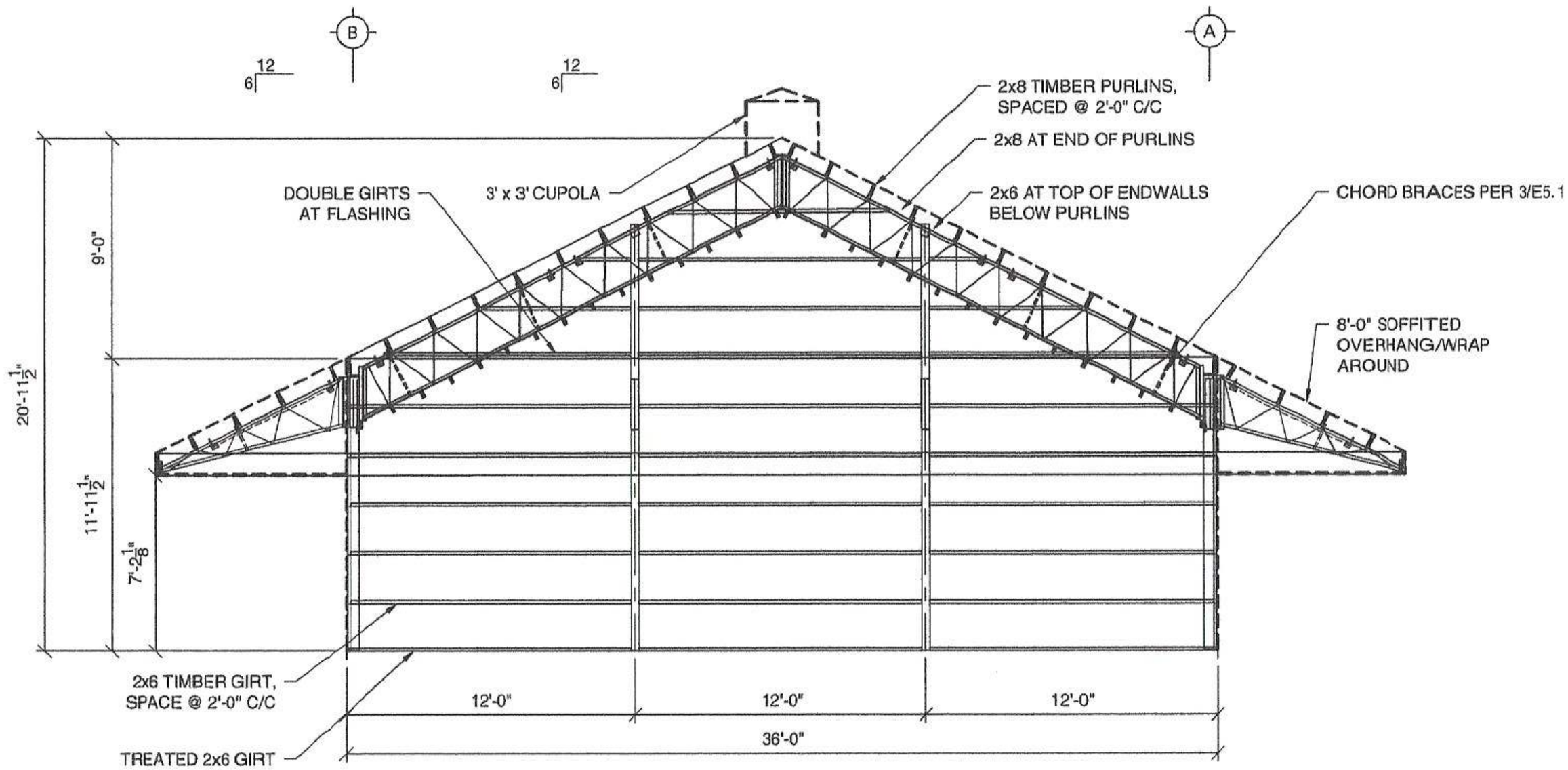

1 ELEVATION AT REAR ENDWALL
SCALE: 3/16" = 1'-0"

NOTES, WALL ELEVATIONS:

1. COLUMN CHORD BRACING IS VITAL TO THE STABILITY OF THE STEEL FRAMES AND SHALL NOT BE REMOVED, RELOCATED, OR MODIFIED IN ANY WAY. ERECTOR SHALL CONFIRM THAT BRACING IS INSTALLED PER THESE DRAWINGS BEFORE TURNING THE BUILDING OVER TO THE OWNER. BRACING TO BE PRESENT ON EVERY TRUSS
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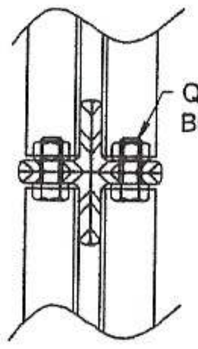
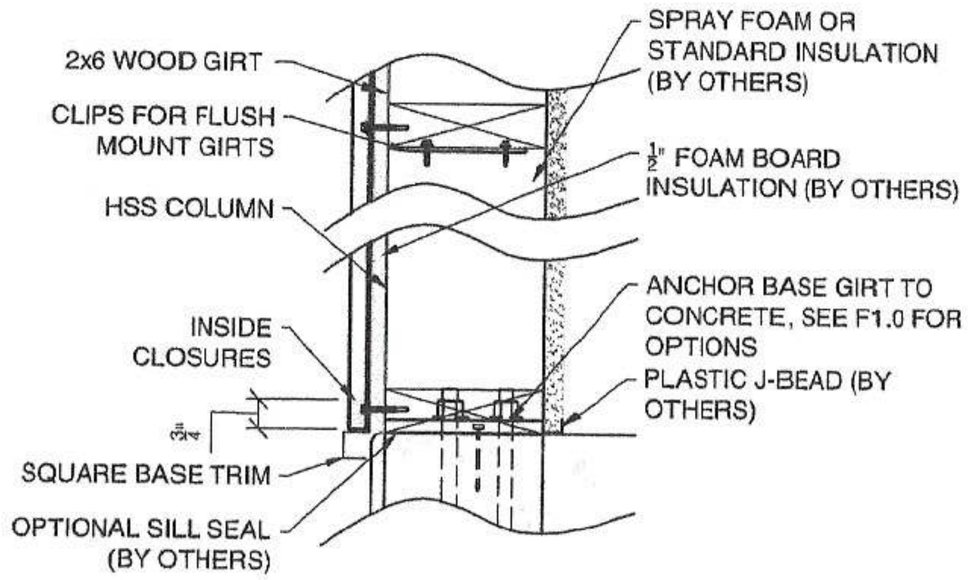
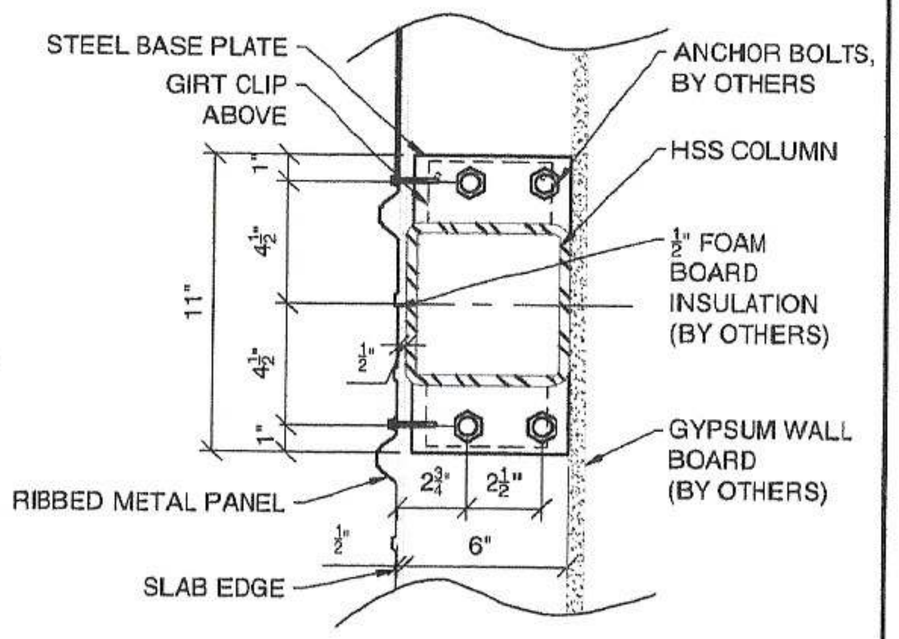


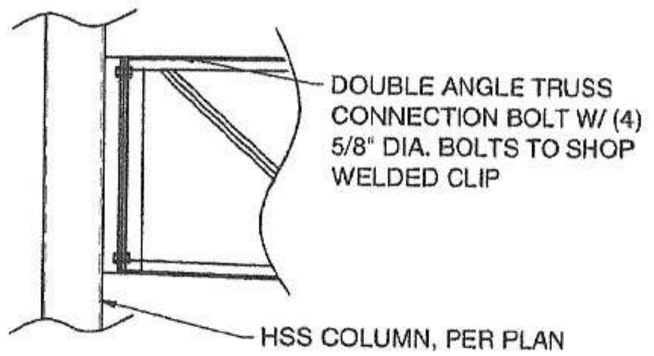
7 TRUSS COLUMN CONNECTION
SCALE: NTS



4 COLUMN BASEPLATE DETAIL
SCALE: NTS



1 SIDEWALL COLUMN BASEPLATE (STD)
SCALE: NTS



8 PORTAL HEADER CONNECTION
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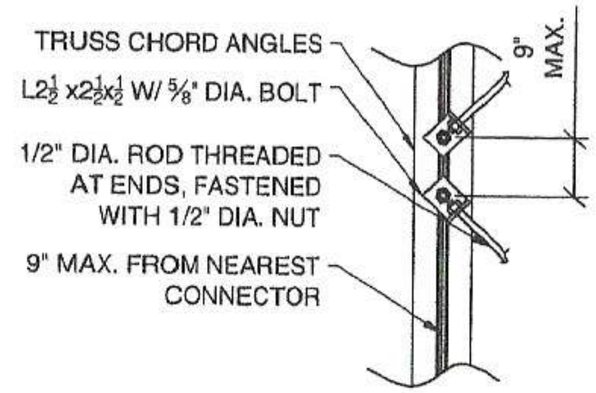

5 ROD BRACING CONNECTION
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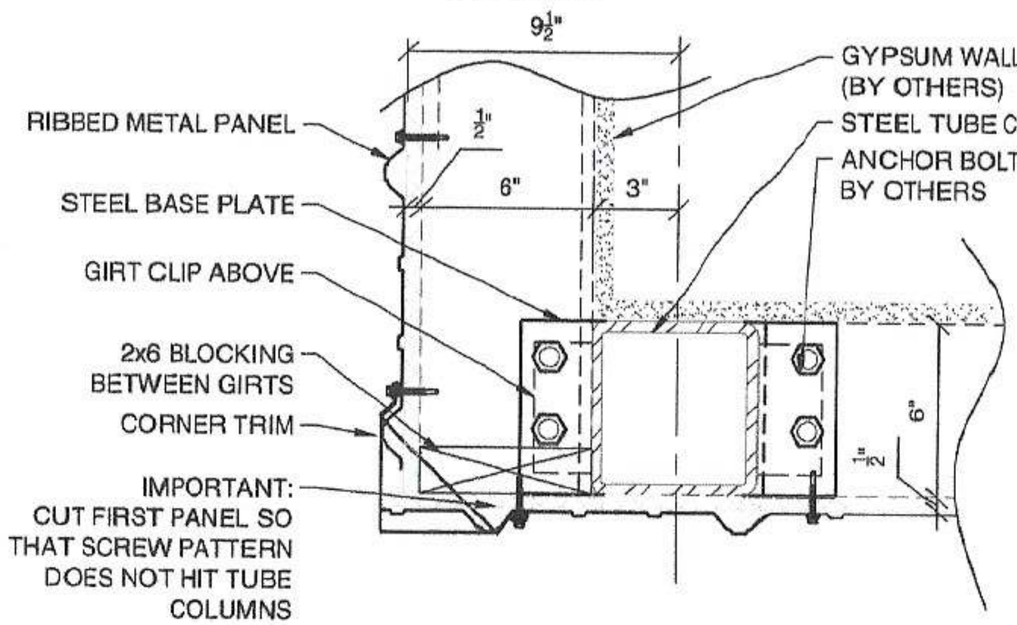

2 CORNER BASEPLATE DETAIL
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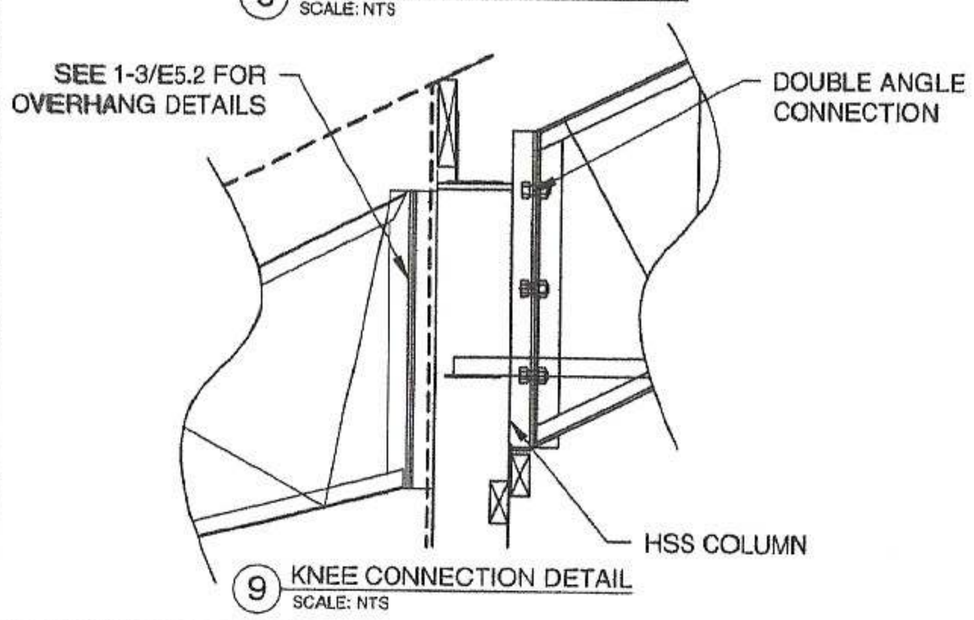

9 KNEE CONNECTION DETAIL
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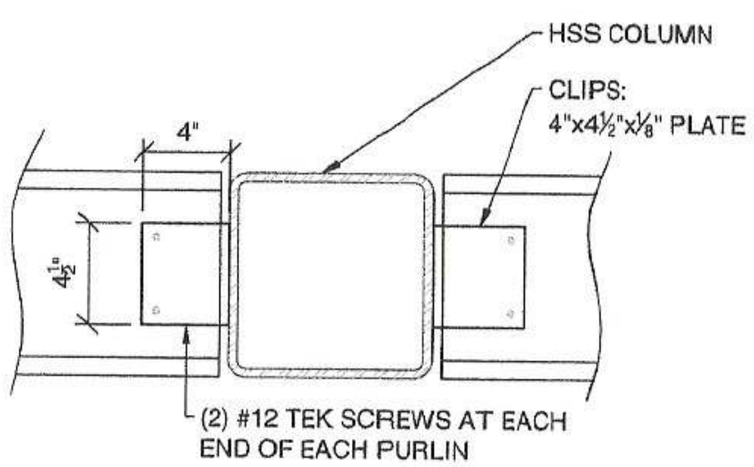
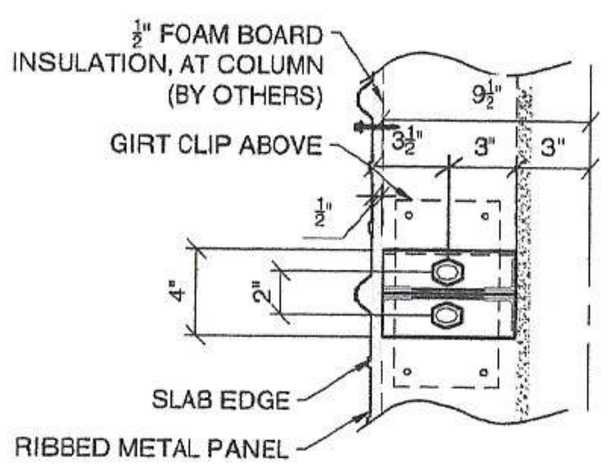


6 GIRT CONNECTION
SCALE: NTS



3 ENDWALL COLUMN BASEPLATE (STD)
SCALE: NTS

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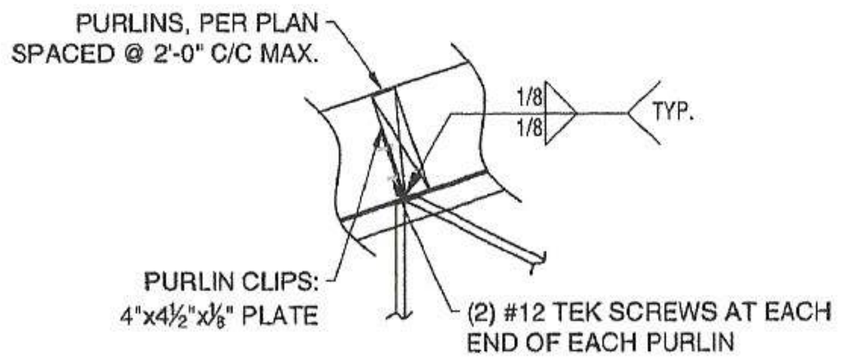

4 PURLIN CONNECTION
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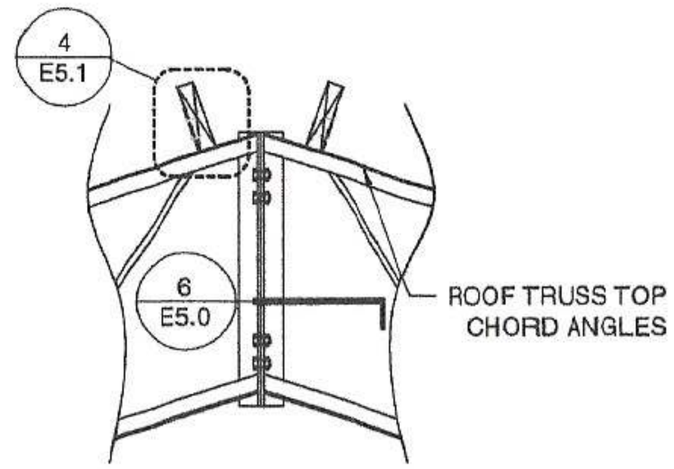
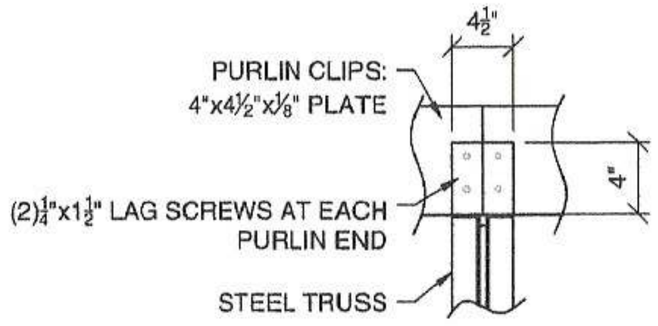
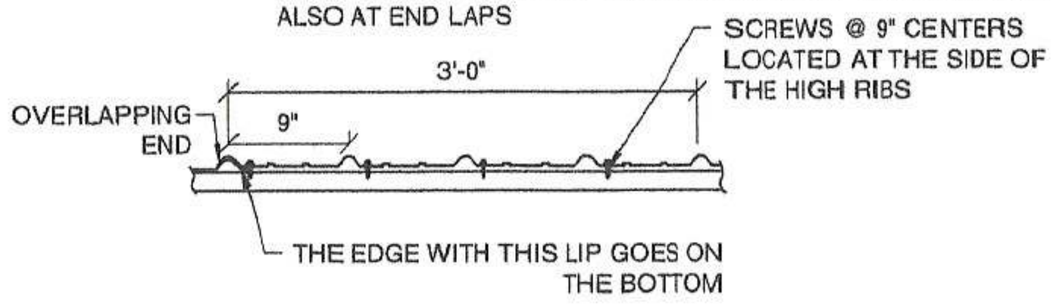


1 RIDGE CONNECTION DETAIL
SCALE: NTS



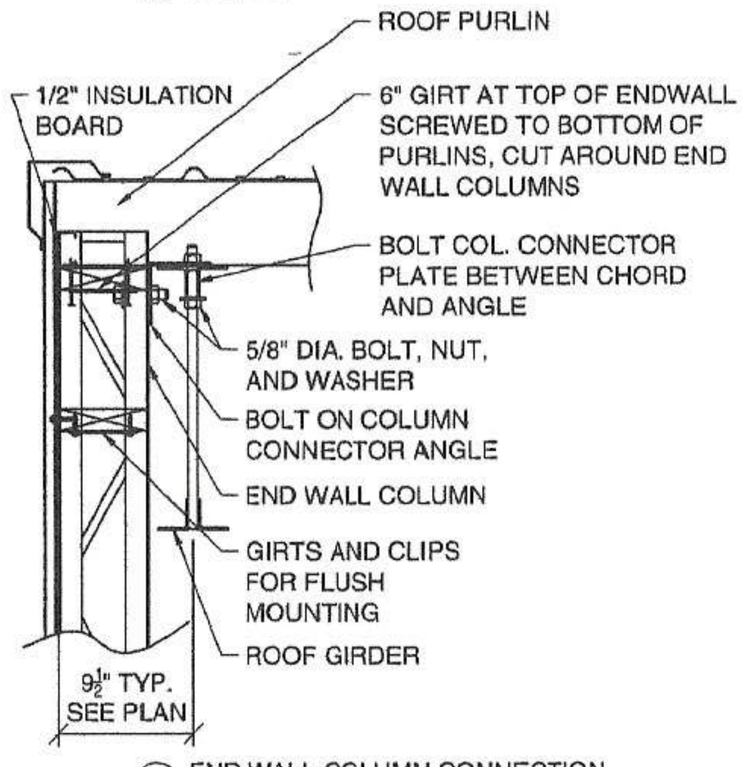
5 PURLIN CONNECTION
SCALE: NTS

EAVE SCREW PATTERN:
ADDITIONAL SCREWS ON THE OTHER SIDE OF HIGH RIBS. USE ALSO AT END LAPS



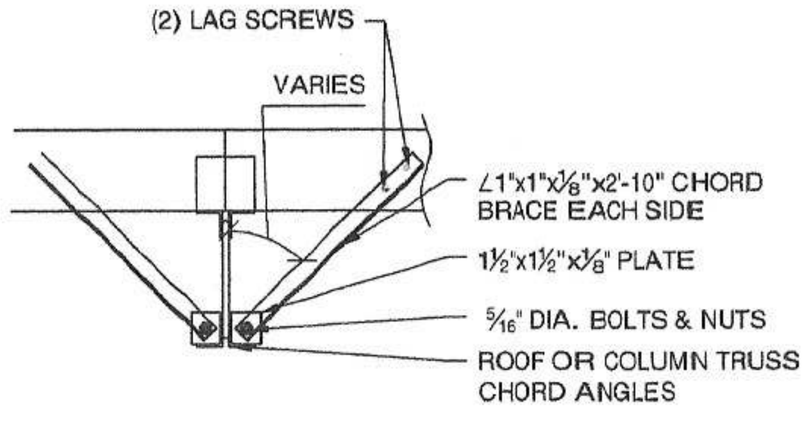
METAL SHEETING FOR ROOF AND WALLS
ATTACH WITH #10 x 1 1/2" WOOD SCREWS

2 STANDARD SHEET METAL CONNECTION
SCALE: NTS



6 END WALL COLUMN CONNECTION
SCALE: NTS

NOTES:
1. BRACE AT LOCATIONS SHOWN ON TRUSS SECTIONS
2. WHERE BRACES ARE CROWDED BY DOOR, FIELD OUT ANGLE TO PLACE THE END AS CLOSE TO THE DOOR JAMB AS POSSIBLE



3 CHORD BRACE DETAIL
SCALE: NTS

Worldwide Steel Buildings
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WWSB Project #63-70

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LICENSE
No 83440
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
9/23/2020

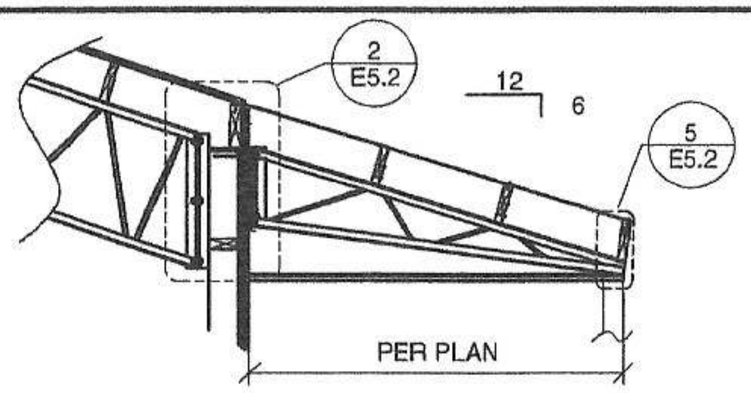
Complete Structural Consulting, Inc.
CA 29951

DRAWN	RZB
CHECKED	DML
DATE	9/23/2020
REVISIONS	

JOB NUMBER 20.553

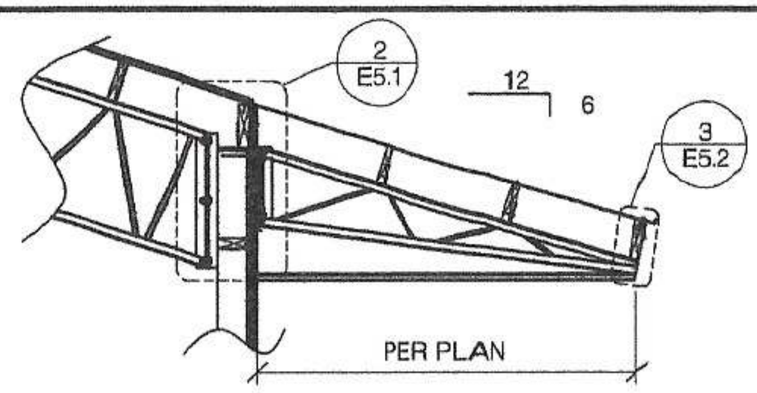
SHEET
E5.1

File location: H:\20553 CHARLES BUTTON 63-70 (WWSB)\04 DRAWINGS\CSC DRAWINGS\CHAD BUTTON - DRAWINGS.DWG File last saved: 9/23/2020 11:35 AM File saved by: ROSS.BOLTON Plotted: 9/23/2020 11:38 AM Plotted by: ROSS BOLTON



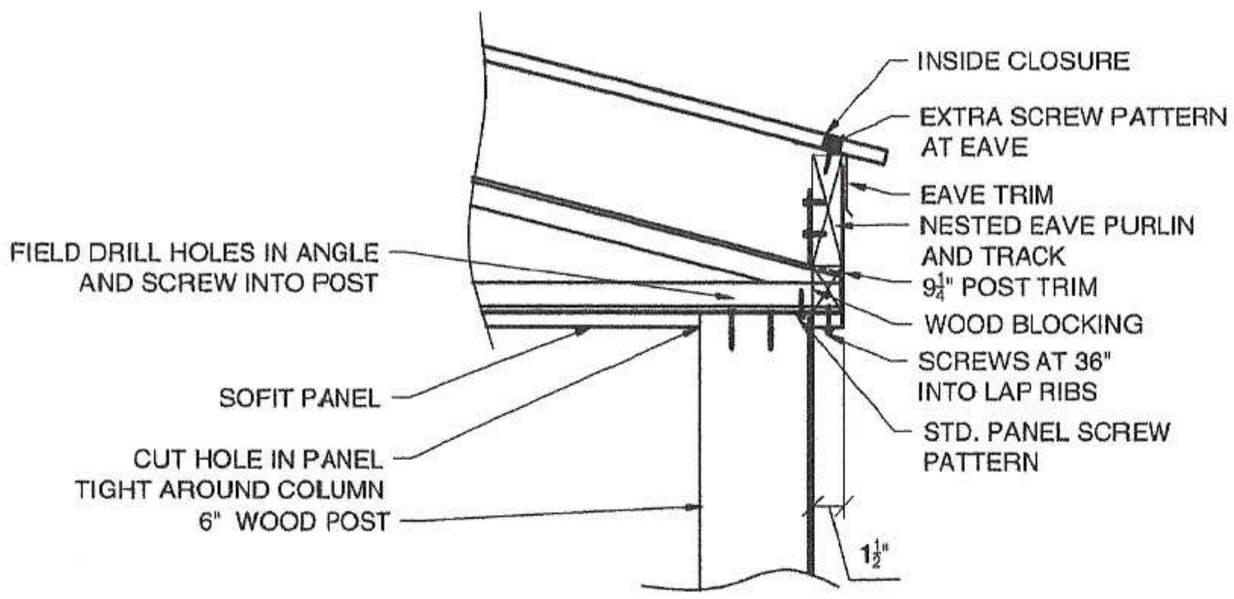
GENERAL NOTE:
INSTALL WALL PANELS
BEFORE OVERHANG TRUSSES

4 OVERHANG DETAIL - WITH WOOD POST
SCALE: N.T.S.

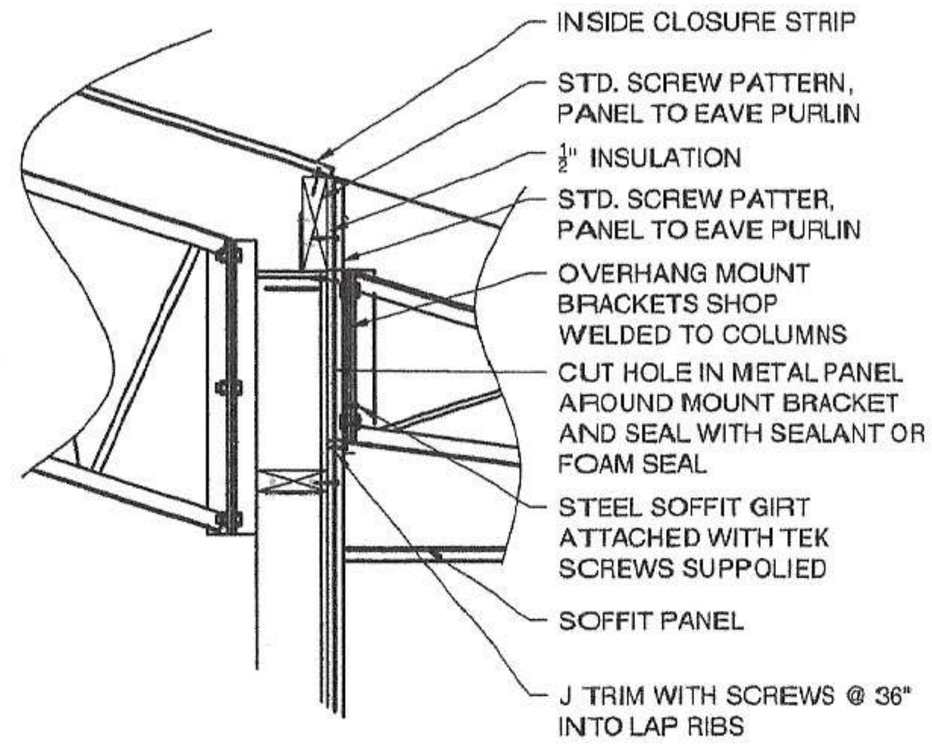


GENERAL NOTE:
INSTALL WALL PANELS
BEFORE OVERHANG TRUSSES

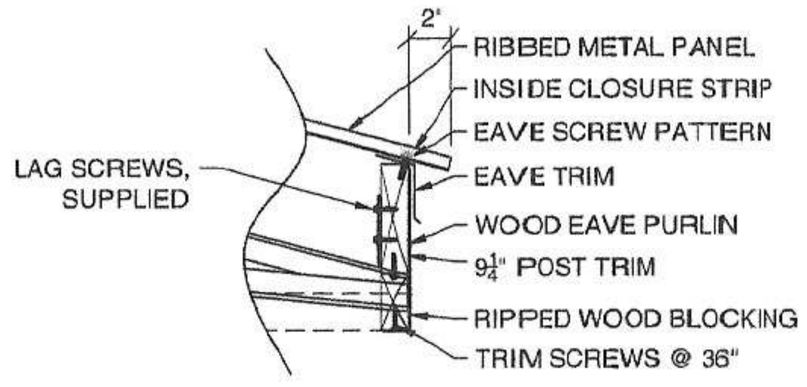
1 OVERHANG DETAIL
SCALE: N.T.S.



5 TRUSS CONNECTION DETAIL
SCALE: N.T.S.



2 OVERHANG DETAIL
SCALE: N.T.S.



3 OVERHANG EAVE DETAIL
SCALE: N.T.S.

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SHEET
E5.2