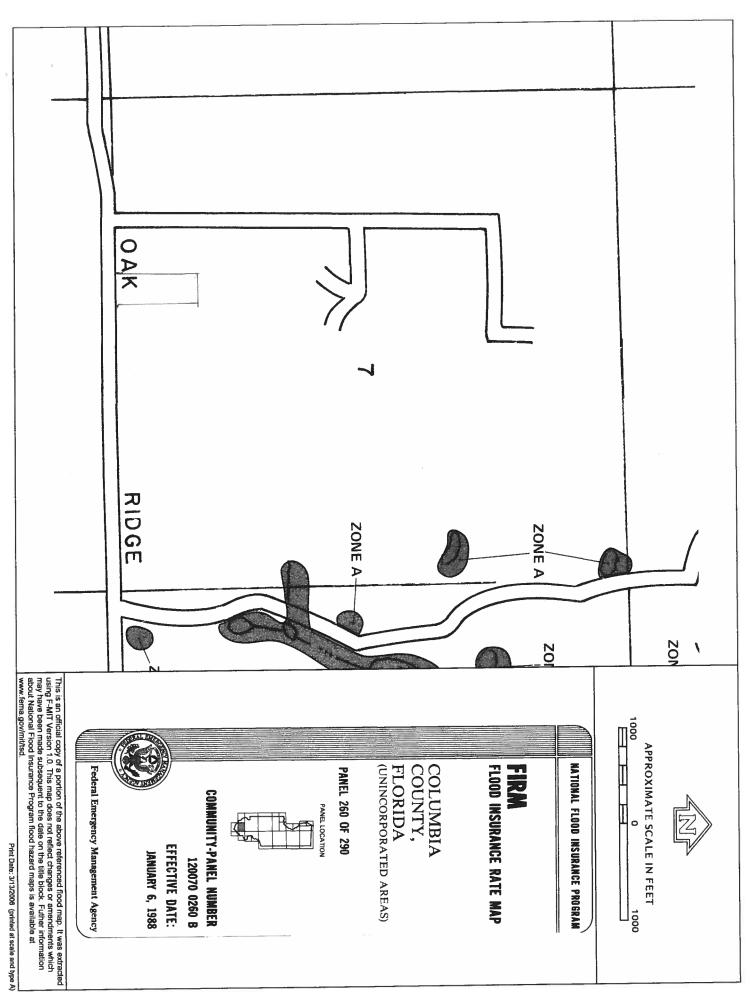
Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 003 29 Date Ro	acelved 3/9 By Jo Permit # 24290
Application Approved by - Zoning Official 624 Date	Plans Examiner OKJTH Date 3-15-06
Flood Zone Development Permit <u>V/4</u> Zoning	
Comments	
	305-876-6749
Applicants Name	Phone 305-525-1997
Address _ 19802 Nov 198th AVE, 7 NO	h spurs (4) 316/18
Owners Name RONNIE+ BRIGITIE K	(LEIN Phone 305 525 - 1997
911 Address 2535 SWCR 778 FT	WHITE FI 32038
Contractors Name Scott ROSENBOOM	Phone 352-538-3877
Address 19802 100 180 = AUE H	
Fee Simple Owner Name & Address	
Bonding Co. Name & Address NA	
Architect/Engineer Name & Address ONO JLET	ZELTER 3860SW 145TICA
Mother Mander Market MIRAM	
Circle the correct power company - FL Power & Light - Cla	
Property ID Number 01-13-12-09996-00	Estimated Cost of Construction
Subdivision Name	Lot Block Unit Phase
Driving Directions SOUTH ON 441 40	778 TURW RX APROX
5 mi ON RT	
// 0	Number of Existing Dwellings on Property vert Permit or Culvert Walver or Have an Existing Drive 5. Side 525 Side 800 Rear 800
Actual Distance of Structure from Property Lines - From	Heated Floor Area 2093 Roof Pitch 6
Total Building Height 219" Number of Stories	TOTAL 2938
Application is hereby made to obtain a permit to do work and i installation has commenced prior to the issuance of a permit a all laws regulating construction in this jurisdiction. OWNERS AFFIDAVIT: I hereby certify that all the foregoing information with all applicable laws and regulating construction warning to owner. Your FAILURE TO RECORD A NOTICE TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU IN LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE.	ormation is accurate and all work will be done in on and zoning. E OF COMMENCMENT MAY RESULT IN YOU PAYING ITEND TO OBTAIN FINANCING, CONSULT WITH YOUR
Mi. & Ih Bright Kon	Spatte K. K. Similar / 1/2
Owner Builder or Agent (Including Contractor)	Contractor Signature Contractors License Number CR - CO5 7796
STATE OF FLORIDA	Competency Card Number
COUNTY OF COLUMBIA	NOTARY STAMP/SEONNIE P. PRESNELL
Sworm to (or affirmed) and subscribed before me	My comm. exp. (Mar.) 1 (2000)
Personally known or Produced Identification	Notary Signature Comm. No. DD 277528
FL DL # K 450-971-50-5950	
386-758-2160 Feb. 15 2006 09:13AM P2	
APVISED SCOTI	



2000 010 /	Legal Description		28204 6524 18952 2800 56480	lumbia County Land 002 AG 001 Bldg 002 Xfea 003 TOTAL B*
3 LYING N OF. 5 7 9 11 13 15 17 19 21 23	CR-778 IN 18-75-17	Mnt 8/09/20		4 6 8 10 12 14 16 18 20 22 24 26 28



STATE OF FLORIDA DEPARTMENT OF HEALTH

Q10 ≥=46

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number <u>96-0172</u> PART II - SITE PLANicale: Each block represents 5 feet and 1 inch = 50 feet. DRAIN FIECD DIM. From House -> E/L lite Plan submitted by: Signature lan Approved . Not Approved Date County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

THIS DOCUMENT MUST BE RECORDED AT THE COUNTY CLERKS OFFICE BEFORE YOUR FIRST INSPECTION.

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 07-75-17-09940-00/

						4.4				
	2535	5. W.	C.K.	778	F4.4	Phite	, FL	SZC	32	
							<u></u>	(9		
. General d	escription of in	nprovement:	Sin	910	Fam	ily Ho	ome_	·		
Owner Na	me & Address	Romaia	and l	Rrigi	He K	'ec'n	P.O.1	Rax	3/57	
1-10	me & Address	ac F/3	7455.	3/57	ntarast i	Property	000	4	P4 305	C25
Namo	ddress of Fee	Simple Owner	r (if other	than own	or).	i i Toperty	Own	12/	79303.	<u> </u>
. Name of A	duless of ree	Simple Owne	ii (ii Oulei	man Own	ei)	• • •				
Contracto	or Name <u>Scot</u>	H Rosen	6000			Phone I	Mumbar	385	454 28	94
Addaga	19 202	V12 191	2 Aus	- 64.	6 Ca	FIIONE		325	<u> </u>	• 6
Address	or Name <u>3 (6)</u> 19 862 / olders Name	10	7776	1 (430	rings,	76	326	<u> </u>	
. Surety Ho	olders Name	n/a				Phone N	lumber _		3	
										
	of Bond									
. Lender Na	ame	a				Phone I	Number_			
Address_			·					5.		
	widhin the Ctete	of Elorida de						41 1		w bo
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erved as pro Name Address _ In addition	ovided by section of the himself/he	on 718.13 (1)	(a) 7; Florion	ates	es:	Phone N	umber _	ided in S	Section 713.	
erved as pro Name Address _ In addition (a) 7. Pho	ovided by section of the himself/he	erself the owi	ner design	ates	a / o	Phone N	as prov	ided in S	Section 713.	13 (1)
Name Address _ In addition (a) 7. Photo Expiration	on to himself/he	erself the ow	ner design	ates	a / o	Phone N	as prov	ided in S	Section 713.	13 (1)
erved as pro Name Address _ In addition (a) 7. Photos	ovided by section of the himself/he	erself the ow	ner design	ates	a / o	Phone N	as prov	ided in S	Section 713.	13 (1)
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Address _ In addition (a) 7. Photographic (Unless a	on to himself/he one Number of on date of the Na different date	erself the own the designer otice of Com is specified)	ner design to receive a	ates	the Lien	Phone N	as prov	ided in S	Section 713.	13 (1) ordin
Address _ In addition (a) 7. Photographic (Unless a	on to himself/he one Number of on date of the Na different date	erself the own the designer otice of Com is specified)	ner design to receive a	ates	the Lien	Phone N	as prov	ided in S	Section 713.	13 (1) ordin
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Address Address In addition (a) 7. Photographic temperature (Unless a	on to himself/he one Number of on date of the Na different date	erself the own the designer otice of Com is specified)	ner design to receive a	ates	the Lien	Phone Notice date is 1 (or	as prov	ided in S from the sign in I	date of receives steaments beckles 20 M F DD 43095 Aay 17, 2009	13 (1) ordin
Address Addres	on to himself/he one Number of on date of the Na different date	erself the own the designer otice of Com is specified) 713, Florida Stice of comm	ner design to receive a mencement Statutes: encement	ates	the Lien	Phone Notice date is 1 (or	as prov	ided in S from the sign in I	date of receives steaments beckles 20 M F DD 43095 Aay 17, 2009	13 (1) ordin

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

Project Name: Address: City, State: Owner:	Rosenboom - Kleii	n Res.	Builder: Same Permitting Office: Cou Permit Number: 2429 Jurisdiction Number: 27	0
Climate Zone:	North			
a. U-factor:	nulti-family f multi-family f multi-family oms e? area (ft²) ea: (Label reqd. by 13-104	ription Area	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits	Cap: 60.0 kBtu/hr SEER: 13.00 Cap: 60.0 kBtu/hr HSPF: 8.00 Cap: 50.0 gallons EF: 0.90
b. N/A c. N/A 11. Ducts a. Sup: Unc. Ret: Un b. N/A		p. R=6.0, 186.0 ft	(CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
Glass	s/Floor Area: 0.13	Total as-built p Total base p	oints: 29135 oints: 31089	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY:

DATE: 2-21-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: _____

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL:

DATE:

SUMMER CALCULATIONS

ADDRESS: ,,,	PERMIT #:

	BASE					AS-	BUI	LT				
GLASS TYPES .18 X Condition Floor Are		SPM = I	Points	Type/SC	Ove Ornt	erhang Len	Hgt	Area X	SPI	мх	SOF	= Points
.18 2093.0)	20.04	7549.9	Double, Clear	E	0.0	0.0	119.0	42.0	06	1.00	5005.6
				Double, Clear	S	0.0	0.0	32.0	35.8	87	1.00	1147.7
				Double, Clear	W	0.0	0.0	72.0	38.		1.00	2773.7
				Double, Clear	N	0.0	0.0	50.0	19.2	20	1.00	960.0
				As-Built Total:				273.0				9887.0
WALL TYPES	Area X	BSPM	= Points	Туре		R-	Value	e Area	X	SPI	/i =	Points
Adjacent	0.0	0.00	0.0	Concrete, Int Insul, Exterior			4.2	1970.0		1.12		2206.4
Exterior	1970.0	1.70	3349.0									
Base Total:	1970.0		3349.0	As-Built Total:				1970.0				2206.4
DOOR TYPES	Area X	BSPM	= Points	Туре				Area	X	SPN	<i>1</i> =	Points
Adjacent Exterior	0.0 37.0	0.00 6.10	0.0 225.7	Exterior Insulated				37.0		4.10		151.7
Base Total:	37.0		225.7	As-Built Total:				37.0				151.7
CEILING TYPES	Area X	BSPM	= Points	Туре		R-Valu	ie /	Area X S	SPM	X S	CM =	Points
Under Attic	2093.0	1.73	3620.9	Under Attic		;	30.0	2093.0	1.73)	X 1.00		3620.9
Base Total:	2093.0		3620.9	As-Built Total:				2093.0				3620.9
FLOOR TYPES	Area X	BSPM	= Points	Туре		R-	Value	e Area	X	SPN	/I =	Points
Slab 19	97.0(p)	-37.0	-7289.0	Slab-On-Grade Edge Insula	tion		0.0	197.0(p		41.20		-8116.4
Raised	0.0	0.00	0.0	_				**				
Base Total:			-7289.0	As-Built Total:				197.0				-8116.4
INFILTRATION	Area X	BSPM	= Points					Area	Х	SPN	1 =	Points
	2093.0	10.21	21369.5					2093.	0	10.2		21369.5

SUMMER CALCULATIONS

· · · · · · · · · · · · · · · · · · ·	
ADDRESS: , , ,	PERMIT #:
ADDRESS. , , ,	PERIVIT #.

BASE			AS-BUILT				
Summer Ba	se Points: 28	3826.0	Summer As-Built Points:	29119.1			
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Cooling Points			
28826.0	0.4266	12297.2	(sys 1: Central Unit 60000 btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0 29119 1.00 (1.09 x 1.147 x 0.91) 0.263 1.000 29119.1 1.00 1.138 0.263 1.000	(INS) 8697.7 8697.7			

WINTER CALCULATIONS

ADDRESS: , , ,	PERMIT #:	

		AS-BUILT
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Type/SC	Overhang Ornt Len Hgt Area X WPM X WOF = Point
.18 2093.0 12.74 4799.7	Double, Clear	E 0.0 0.0 119.0 18.79 1.00 2236.
	Double, Clear	S 0.0 0.0 32.0 13.30 1.00 425.
	Double, Clear	W 0.0 0.0 72.0 20.73 1.00 1492.
	Double, Clear	N 0.0 0.0 50.0 24.58 1.00 1228.
	As-Built Total:	273.0 5383.
WALL TYPES Area X BWPM = Points	Туре	R-Value Area X WPM = Points
Adjacent 0.0 0.00 0.0	Concrete, Int Insul, Exterior	4.2 1970.0 6.34 12489.
Exterior 1970.0 3.70 7289.0		
Base Total: 1970.0 7289.0	As-Built Total:	1970.0 12489.
DOOR TYPES Area X BWPM = Points	Туре	Area X WPM = Points
Adjacent 0.0 0.00 0.0	Exterior Insulated	37.0 8.40 310.
Exterior 37.0 12.30 455.1		
Base Total: 37.0 455.1	As-Built Total:	37.0 310.
CEILING TYPES Area X BWPM = Points	Туре	R-Value Area X WPM X WCM = Points
Under Attic 2093.0 2.05 4290.6	Under Attic	30.0 2093.0 2.05 X 1.00 4290.0
Base Total: 2093.0 4290.6	As-Built Total:	2093.0 4290.0
FLOOR TYPES Area X BWPM = Points	Туре	R-Value Area X WPM = Points
Slab 197.0(p) 8.9 1753.3	Slab-On-Grade Edge Insulation	on 0.0 197.0(p 18.80 3703.
Raised 0.0 0.00 0.0		
Base Total: 1753.3	As-Built Total:	197.0 3703.
INFILTRATION Area X BWPM = Points		Area X WPM = Points
2093.0 -0.59 -1234.9		2093.0 -0.59 -1234.9

WINTER CALCULATIONS

ADDRESS:,,,	PERMIT #:

BASE			AS-BUILT					
Winter Base	Points:	17352.8	Winter As-Built Points:	24943.2				
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit Component Ratio Multiplier Multiplier Multiplier (System - Points) (DM x DSM x AHU)	= Heating Points				
17352.8	0.6274	10887.2	(sys 1: Electric Heat Pump 60000 btuh ,EFF(8.0) Ducts:Unc(S),Unc(R),Inc(24943.2	nt(AH),R6.0 12356.4 12356.4				

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

	 			
ADDRESS: ,,,			PERMIT #:	

BASE				AS-BUILT								
WATER HEA Number of Bedrooms	ATING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	×	Tank X Ratio	Multiplier	X Credit = Multiplier	Total
3		2635.00		7905.0	50.0	0.90	3		1.00	2693.56	1.00	8080.7
					As-Built To	otal:						8080.7

	CODE COMPLIANCE STATUS									
BASE							AS	-BUILT		
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
12297	10887	7905	31089	8698		12356		8081		29135

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: , , ,	PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall;	
		foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility	
		penetrations; between wall panels & top/bottom plates; between walls and floor.	
		EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends	
		from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.	
		EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed	
		to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases,	
		soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate;	
		attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is	
		installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a	
		sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from	
		conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA,	
		have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir	
		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools	
		must have a pump timer. Gas spa & pool heaters must have a minimum thermal	
		efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically	
		attached, sealed, insulated, and installed in accordance with the criteria of Section 610.	
		Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides.	
		Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 84.4

The higher the score, the more efficient the home.

, , , ,

1.	New construction or existing	New	_	Cooling systems		
2.	Single family or multi-family	Single family	a	. Central Unit	Cap: 60.0 kBtu/hr	_
3.	Number of units, if multi-family	1	_		SEER: 13.00	_
4.	Number of Bedrooms	3	_ b	. N/A		******
5.	Is this a worst case?	Yes	-			
6.	Conditioned floor area (ft²)	2093 ft ²	_ c	. N/A		-
7.	Glass type 1 and area: (Label reqd. b	by 13-104.4.5 if not default)				_
a.	U-factor:	Description Area		Heating systems		
	(or Single or Double DEFAULT)	7a.(Dble Default) 273.0 ft ²	_ a	Electric Heat Pump	Cap: 60.0 kBtu/hr	_
b.	SHGC:				HSPF: 8.00	_
	,	7b. (Clear) 273.0 ft ²	_ b	. N/A		_
8.	Floor types					_
a.	Slab-On-Grade Edge Insulation	R=0.0, 197.0(p) ft	_ c	. N/A		_
_	N/A		_			
c.	N/A		14.	Hot water systems		
9.	Wall types		a	Electric Resistance	Cap: 50.0 gallons	_
a.	Concrete, Int Insul, Exterior	R=4.2, 1970.0 ft ²			EF: 0.90	_
b.	N/A		b	. N/A		
c.	N/A		_			
d.	N/A		c.	Conservation credits		
e.	N/A		_	(HR-Heat recovery, Solar		
10.	Ceiling types			DHP-Dedicated heat pump)		
a.	Under Attic	R=30.0, 2093.0 ft ²	15.	HVAC credits		
b.	N/A		_	(CF-Ceiling fan, CV-Cross ventilation	,	
c.	N/A		_	HF-Whole house fan,		
11.	Ducts			PT-Programmable Thermostat,		
a.	Sup: Unc. Ret: Unc. AH: Interior	Sup. R=6.0, 186.0 ft	_	MZ-C-Multizone cooling,		
b.	N/A			MZ-H-Multizone heating)		
Con	rtify that this home has complie struction through the above ene	rgy saving features whic	h will be ir	istalled (or exceeded)	OF THE STATE	, da
	ais home before final inspection and on installed Code compliant in		Display Ca	rd will be completed		ROR I
Buil	der Signature:		Date:		13	DA
Add	ress of New Home:		City/FL Z	ip:	GOD WE TRUST	A STATE OF THE STA

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.0) PCEASE ALLOW MANUEL BERNARDO TO PIEK UP THE PERMITT FOR THE ICLEIW JOB

Scott Rosenboom South Rosenboom

BONNIE P. PRESNELL
Notary Public, State of Florida
My comm. exp. Mar. 1, 2008
Comm. No. DD 277528

Bounie P. Presneed

Date:

March 14,2006

To:

Mr. Joe Halfilwanger, Plans Examiner Fax: 386-754-7088

From:

Ron Klein

Ref:

REF 0603-29

In response to you questions on my plan submittals please see the following comments.

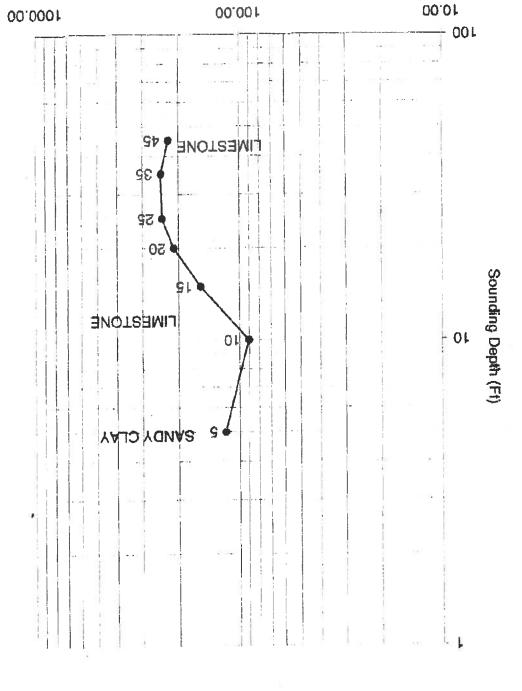
- 1. No response required.
- 2. W-7 and W-12 exceed the code for safety tempered glass. They are rated for large and small impact by Miami-Dade County product approval for wind speeds of 150 mph. The glass is laminated with an interlayer of Polyvinylbutyral making it almost shatterproof, like the front windshield on a car.
- 3. The Geotech report is attached. The engineer gave me 3,000 psf for soil bearing, and I used 2,000 for design.
- 4. If you look on sheet 1. "New Home Site Plan" Just below the proposed home site I have a note and show the location of the ground mounted transformer. All exterior disconnects are mounted at this point. Four-wire conductors will be used from this disconnect point to the interior panel with one of the conductors being used as an equipment ground.

Please contact me at 305-525-1997 if you have any additional questions.

Total Pages of Fax: 6

Revised Transmittal

GEOHAZARDS INC.
Electrical Resistivity Survey
Investigation#:2005202A
Array Orientation: NA0E
Station Number: 6



(17-smrlO) ytivitaiseR

Date:

March 14,2006

To:

Mr. Joe Haltiwanger, Plans Examiner Fax: 386-754-7088

From:

Ref:

REF 0603-29

In response to you questions on my plan submittals please see the following comments.

1. No response required.

2. W-7 and W-12 exceed the code for safety tempered glass. They are rated for large and small impact by Miami-Dade County product approval for wind speeds of 150 niph. The glass is laminated with an interlayer of Polyvinylbutyral making it almost shatterproof, like the front windshield on a car.

3. The Geotech report is attached. The engineer gave me 3,000 psf for

soil bearing, and I used 2,000 for design.

4. If you look on sheet 1. "New Home Site Plan" Just below the proposed home site I have a note and show the location of the ground mounted transformer. All exterior disconnects are mounted at this point. Four-wire conductors will be used from this disconnect point to the interior panel with one of the conductors being used as an equipment ground.

Please contact me at 305-525-1997 if you have any additional questions.

Total Pages of Fax: 6

Revised Transmittal

The purposes of our investigation were to determine the general substitute conditions at the site and to provide recommendations for foundation design and construction.

Site Investigation

The subsurface conditions were investigated by performing four (4) Standard Penetration Test borings advanced to depths of 10 feet. Borings were performed at the approximate locations indicated on the attached Boring Location Plan. These locations were selected by Cal-Tech Testing, Inc., and the building limits were identified on site by painted lines at the building corners. Two borings were offset because of trees.

The Standard Penetration Test (ASTM D-1586) is performed by driving a standard split-barrel sampler into the soil by blows of a 140-pound hammer falling 30

"Excellence in Engineering & Geoscience"



Cal-Tech Testing, Inc.

Engineering

Geotechnical Environmental P.O. Box 1625 • Lake City, FL 32056-1625 5919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-0633 • Fax (386) 752-5456 Tel. (904) 262-4045 • Fax (904) 262-4047

September 6, 2005

Ron Kline P. O. Box 660026 Miami, Florida 33266-0026

Reference:

Proposed Kline Residence

2535 C. R. 778 Fort White, Florida

Cal-Tech Project No. 05-372

Dear Mr. Kline,

Cal-Tech Testing, Inc. has completed the subsurface investigation and engineering evaluation of the site for the proposed Kline residence to be constructed at the referenced location. Our work was authorized by you.

Introduction

We understand you will construct a single-story residence with a plan area of about 2,800 square feet. Support for the residence is to be provided by a monolithic foundation or by conventional, shallow spread footings. Detailed foundation loads have not been provided; however, we assume column and wall loads will not exceed 25 kips and 2 kips per foot, respectively.

The proposed building site is open and grassy, and the ground surface appears to slope very gently in a westerly direction.

The purposes of our investigation were to determine the general subsurface conditions at the site and to provide recommendations for foundation design and construction.

Site Investigation

The subsurface conditions were investigated by performing four (4) Standard Penetration Test borings advanced to depths of 10 feet. Borings were performed at the approximate locations indicated on the attached Boring Location Plan. These locations were selected by Cal-Tech Testing, Inc., and the building limits were identified on site by painted lines at the building corners. Two borings were offset because of trees.

The Standard Penetration Test (ASTM D-1586) is performed by driving a standard split-barrel sampler into the soil by blows of a 140-pound hammer falling 30

inches. The number of blows required to drive the sampler 1 foot, after seating 6 inches, is designated the penetration resistance, or N-value; this value is an index to soil density or consistency.

Findings.

The soil borings generally encountered three soil strata. The first layer consists of 1 to 2.5 feet of very loose to loose, dark gray or tannish gray sand (SP) or sand with silt (SP/SM). The N-values of this layer are on the order of 3 to 4 blows per foot.

The second layer consists of 4.5 to 7.5 or more feet of generally loose to medium dense, tannish gray, brownish gray or gray and orange, clayey sand (SC). The Nvalues of this layer range from 8 to 35 blows per foot.

The third layer consists of an undetermined thickness of medium stiff to very stiff, generally tannish gray, sandy clay (CL). The N-values of this layer range from 4 to 19 blows per foot.

Groundwater was not encountered at any boring location at the time of our investigation, and we estimate the seasonal high ground water table will occur at a depth of more than 6 feet below the existing surface grade. Note however that infiltrated storm water will perch on the clayey soils following storm events and may affect normal compaction procdures.

For a more detailed description of the subsurface conditions encountered, please refer to the attached Boring Logs. Note specifically the transition between soil layers may be gradual and not abrupt as indicated by the logs: therefore, the thickness of soil layers should be considered approximate.

Discussion and Recommendations

Clayey sands were encountered at depths of 1 to 2.5 feet; however, we believe these clayey sands are not particularly active and do not need to be replaced. If fact, we recommend these clayey sands be left in place since they will help shield the potentially active clays encountered at depths of 5.5 or more feet below the ground surface. Laboratory testing was performed for representative samples of the clayey sands, and test results are attached.

Based upon our findings, it is our opinion the proposed residence can be supported by a monolithic foundation or by conventional, shallow spread footings sized to exert a maximum soil bearing pressure of 2,500 pounds per square foot. The foundations should have minimum widths of 16 and 24 inches for strip and isolated footings, respectively, even though this allowable bearing pressure may not be developed. Foundations should be embedded at least 14 inches below the lowest adjacent grade (finished surface grade, for example).

It is also our opinion the existing site soils, when compacted, are suitable to provide support for the structure. Replacement soils should not be required.

For preparation the site should be stripped of grass, roots and other deleterious Excavation should then be performed as required to establish the appropriate foundation and floor grades. Clean, sandy soils should be stockpiled for later use as fill. The subgrade should then be thoroughly proof-rolled with heavy rubber-tired equipment (a large, loaded, front-end loader, for example). Proof rolling helps to compact the bearing soils and to locate zones of especially loose or soft soil that may be present. Such zones should be undercut and back-filled or otherwise treated as directed by the geotechnical engineer.

The subgrade should then be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density to a depth of 2 feet in foundation areas and to a depth of 1 foot in floor slab areas. Moderate weight, vibratory equipment should provide adequate compaction of the subgrade soils.

Fill to raise the site can be placed as required following preparation of the subgrade. Fill should consist of relatively clean, fine sand containing less than 10% passing the No. 200 sieve. Fill should be placed in maximum 12-inch, loose lifts, and each lift should be proof-compacted to a minimum of 95% of the Modified Proctor maximum dry density. Foundation cuts may be placed in the compacted fill; however, disturbed fill materials should be recompacted prior to placement of foundations or floor

Field density testing should be performed in the compacted subgrade, in each lift of fill, and in foundation excavations to verify the recommended compaction has been

Our recommendations are based upon our findings as described in this report, however, subsurface conditions may exist that were not encountered in the soil test borings. Cal-Tech Testing, Inc. should be notified immediately if different soil conditions are encountered during construction. It may be necessary to reevaluate this site and revise our recommendations.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted. Cal-Tech Testing, Inc.

" Geamer Linda Creamer President / C.E.O.

John C. Dorman, Jr., Ph.D., P.E.

Geotechnical Engineer



Cal-Tech Testing, Inc. • Engineering

· Gcotechnical

P.O. Box 1625 - Lake City, FL 32056-1625

6919 Distribution Avenue S., Unit #6 - Jacksonvillo, FL 32257

Tel (386) 755-3433 • Fax (386) 752-5456 Tot (904) 262-4046 • Fax (904) 262-4047

Environmental

SUMMARY OF LABORATORY TEST RESULTS

PRO.	JECT:	Proposed Kline Residence	3							JOB	NO ·		05-3	79	
		High Springs, FL							-		DRT N	٥.	4	12	
CLIE	NT:	Ron Kline							-			U.;	<u> </u>		
	,								-	DATE	::		09/08	3/05	
NO.	PTH (ft.)		TYPE.	MOISTURE	1	RBERG	3 ℃		SIEVE	ANALY:	51S (% ,)ú9sing	1)	SOIL	- Q
BORING NO	SAMPLEDE	SOIL DESCRIPTION	SAMPLET	MATURAL MC	LIQUID LIMIT	PLASTICITY INDEX (%)	COEFFICIENT PERMEABILI (feel/day)	No.4	No.10	No. 40	No. 60	No.150	No. 200	AASHTO SC CLASSIFICAT	UNIFIED SOIL
B-1	3	Tannish Gray, Clayey Sand	SS										37.5		sc
B-3	2	Brownish Gray, Clayey Sand	\$S										28.3		SC
B-4	5	Gray and Orange, Clayey Sand	SS		36	12							49.8		SC

*SS- Split Spoon ST- Shelby Tube

A- Auger

Reviewed By

Date:

Florida Registration No.:

52612

Water Table: N/A Water Table: N/A Soil Soil Depth (ft) Depth (ft) N-value Description N-value Description Dark Tonnish Grey Sand with Silt, Trace Clay (SP/SM) Dark Tannish Grey Sand with Sitt (SP/SM) Very Loose, Dark Tannish Grey 4 Loose, Tannish Grey Sand (SP) 3 Sand with Silt (SP/SM) Loose to Medium Dense, Tannish Grey, Clayey Sand, Trace Limestone (SC) Medium Dense, Light Grey and Orange, Clayey Sand (SC) Medium Dense, Tannish Grey, Clayey Sand, Thin Lenses White Sandy Clay (CL) Medium Dense, Tannish Grey Sand with Clay, Trace Silt (SP/SC) Medium Dense, Light Grey and Orange, Clayey Sand (SC) Stiff, Tannish Grey to Light Grey Sandy Clay, Trace Limestone (CL) Stiff, Light Greyish Tan, Light Grey Medium Dense, Light Tannish Grey and Orange, Slightly Clayey Sand (SC) 12 and Orange, Sandy Clay, Trace Limestone (CL) Water Table: N/A Water Table: N/A Soil Soil N-value Description Depth (ft) Description Depth (ft) N-value Dark Grey Sand with Silt, Trace Dark Grey Sand with Silf, Trace Organics (SP/SM) Orgnaics (SP/SM) Loose, Tannish Grey to Dark Grey, Slightly Clayey Sand (SC) Loose to Medium Dense, Brownish Medium Dense, Tannish Grey and Orange, Clayey Sand (SC) Grey, Clayey Sand, Trace Limestone (SC) Medium Dense, Grey and Orange, 20 Clayey Sand (SC) Dense, Light Grey, Tannish Grey 35 and Orange, Very Clayey Sand, Trace Weathered Limestone (SC) Medium Stiff to Very Stiff, Light Tannish Grey, Sandy Clay, Trace Limestone (CL) Very Stiff, Greyish Tan and Orange, Sandy Clay, Trace Limestone (CL) Medium Stiff, Light Tannish Grey to Dark Grey, Sandy Clay, Trace Limestone (CL) Medium Dense, Blueish Grey and Orange to Dark Orange, Very Clayey Sand (SC) 40'

Boring Logs and Location Plan: Proposed Kline Residense County Road 778 Fort White, Florida

B-3



SINKHOLES . EXPANSIVE CLAYS . LAND SUBSIDENCE

P.O. Box 14956 Gainesville, Florida 32604 Professional Geological, Geophysical and Geotechnical Services

GEOPHYSICAL INVESTIGATION OF THE GEOLOGICAL SUBSURFACE, 2535 CR 778, FORT WHITE, FLORIDA

Report #2005202A

PREPARED FOR:

Mr. Ronnie S. Klein

P.O. Box 660026

Miami, Florida 33266-0026

June 2005

Telephone: (352) 371-7243 (800) 770-9990

Fax: (352) 371-4410

Web Page: http://www.sinkholes.com

E-mail: geohazards@bellsouth.net

Date:

March 14,2006

To:

Mr. Joe Halkilwanger, Plans Examiner Fax: 386-754-7088

From:

Ron Klein

Ref:

REF 0603-29

In response to you questions on my plan submittals please see the following comments.

1. No response required.

- 2. W-7 and W-12 exceed the code for safety tempered glass. They are rated for large and small impact by Miami-Dade County product approval for wind speeds of 150 mph. The glass is laminated with an interlayer of Polyvinylbutyral making it almost shatterproof, like the front windshield on a car.
- 3. The Geotech report is attached. The engineer gave me 3,000 psf for soil bearing, and I used 2,000 for design.
- 4. If you look on sheet 1. "New Home Site Plan" Just below the proposed home site I have a note and show the location of the ground mounted transformer. All exterior disconnects are mounted at this point. Four-wire conductors will be used from this disconnect point to the interior panel with one of the conductors being used as an equipment ground.

Please contact me at 305-525-1997 if you have any additional questions.



SINKHOLES . EXPANSIVE CLAYS . LAND SUBSIDENCE

P.O. Box 14956 Gainesville, Florida 32604

Professional Geological, Geophysical and Geotechnical Engineering Services

Anthony F. Randazzo, Ph. D. Geologist Florida PG# 0003 Georgia PG#1135 David Bioomquist, Ph. D. Geotechnical Engineer Florida PE# 37235 Douglas L. Smith, Ph.D. Geophysicis: Florida PG# 0018 Georgia PG# 1140

June 15, 2005

Geohazards, Inc., Investigation No. 2005202A

GEOPHYSICAL INVESTIGATION OF THE GEOLOGICAL SUBSURFACE AT THE KLEIN LOT, 2535 CR 778, FORT WHITE, FLORIDA

INTRODUCTION

Purpose

Geohazards, Inc. was tasked by Mr. Ronnie Klein to conduct a reconnaissance geophysical investigation at the above referenced locality.

This investigation was conducted to provide a geophysical characterization of the geological subsurface at specific locations. In particular, efforts were designed to determine the presence of subsurface cavities and subsurface zones of disruption that might contribute to subsidence, or near surface clay. Any of these conditions could be responsible for existing or potential subsidence at the site.

Telephone: (352) 371-7243 (800) 770-9990

Web page: www.sinkholes.com

Fax: (352) 371-4410

Email: geohazards@bellsouth.net

REGIONAL GEOLOGY

Based on map consultations and personal inspection, the surficial geologic material at the study site is the Hawthorn Group of geological formations overlain by a cover of very young unconsolidated sands and sandy clays. These consist of fine to medium grained, unconsolidated quartz sand, silt, and clay in varying proportions and thickness. Shrink/swell clays of significant size, continuity and nearness to the surface are a particularly troublesome characteristic of the Hawthorn where they occur in significant thickness and lateral continuity. Concrete slabs and foundations can be severely damaged where such a geologic condition occurs.

The Ocala Limestone underlies the Hawthorn. This limestone has experienced significant dissolution and the creation of an intricate cavernous system. Problems in the development of sinkholes are related to the size and nearness to the surface of the Ocala limestone and these underground cavities. The upper surface of this limestone is highly irregular.

FIELD TEST METHODS

Hand Auger Borings

Three hand auger borings (HA-1 through HA-3) were conducted at sites shown on the location map. The borings were performed in general accordance with ASTM standards D1452-80(1995) entitled "Standard Practice for Soil Investigation and Sampling by Auger Borings." The borings were conducted by manually rotating the auger into the ground to termination depths of approximately 5 feet (or refusal) to provide a continuous profile of the near-surface materials. Increments of approximately 0.5 feet are extracted for description and, if necessary, retention for later analyses. Results of the hand auger borings are shown in the Hand Auger Investigation Profiles.

From: The Columbia County Building Department

Plans Review

135 NE Hernando Av.

P. O Box 1529

Lake City Florida, 32056-1529

Reference to a building permit application Number: 0603-29

Scott Rosenboom Owner Ronnie & Brigitte Klein 2535 SW CR 778

On the date of March 13, 2006 application 0603-29 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0603-29 when making reference to this application.

- 1. Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.
- 2. Please confirm that the W7 & W12 windows in the showers will be safety tempered glass if required by section R308.4 of the FRC-2004.
- 3. Please submit the geotechnical report to confirm the soil bearing pressure of 2,000 psf.
- 4. Show the location of the overcurrent protection device, shall be installed on the exterior of structures to serve as a disconnecting means. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground.

Thank you,

Joe Haltiwanger Plan Examiner Columbia County Building Department

Project Name: Klein Residence

Location: 2535 sw county road 778 Fort White FI

By: Otto J. Letzelter, P.E.

Start Date: 2/5/2006

Comments: 150 mph wind speed

Otto J. Letzelter, P.E.

PE 54716

3860 SW 145th Ave

Miramar, FI 33027

954-650-3371

Local Information

Wind Dir.	Exposure
1	С
2	С
3	С
4	С

Basic Wind Speed: 150 mph

Topography: None

Optional Factors

This project uses load combinations from ASCE 7.

Section - Main Section

Enclosure Classification: Enclosed

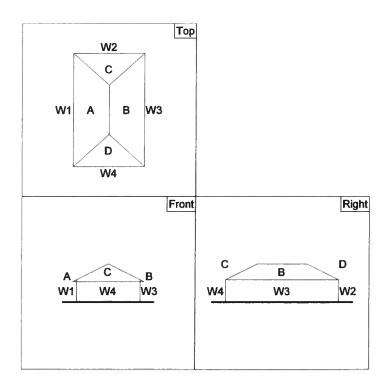
Building Category: II

Wall	Length(ft)	Overhang(ft)
1	60.5	2.0
2	34.0	0.0
3	60.5	2.0
4	34.0	0.0

Wall Height: 12 ft Parapet Height: 0 ft

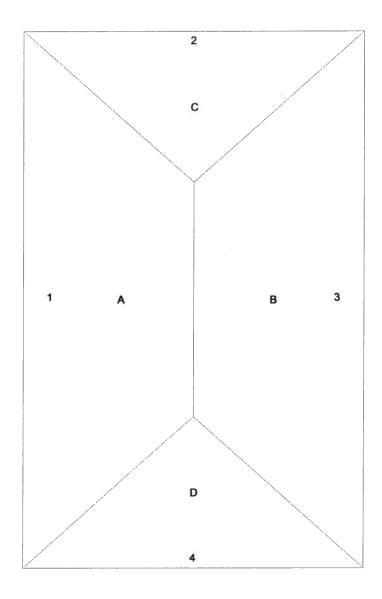
Roof Shape: Hipped

Roof	Slope(:12)
A&B	6.0
C&D	6.0



Composite Drawing











Components and Cladding Input

Component Description	Wall/Roof	Surface Label	Zone	Span(ft)	Effective Width(ft)	Area(sqft)
sheathing	Wall	1	(All)	4.0	8.0	32.0
wall	Wall	2	(All)	12.0	1.0	48.0
windows	Wall	3	(All)	5.0	2.0	10.0
doors	Wall	1	(All)	8.0	2.5	21.3

Components and Cladding Output

Component Description	Surface	Zone	z(ft)	q(psf)	GCp	GCpi	ExtPres(psf)	Net w/ +GCpi (psf)	Net w/ -GCpi (psf)
sheathing	1	4	16.3	42.3	0.91	0.18	38.5	30.9	46.1
			16.3	42.3	-1.01		-42.7	-50.3	-35.1
		5	16.3	42.3	0.91		38.5	30.9	46.1
			16.3	42.3	-1.22		-51.6	-59.2	-44 .0
wail	2	4	16.3	42.3	0.88	0.18	37.2	29.6	44.8
			16.3	42.3	-0.98		-41.5	-49.1	-33.8
		5	16.3	42.3	88.0		37.2	29.6	44.8
			16.3	42.3	-1.16		-4 9.1	-56.7	-41.5
windows	3	4	16.3	42.3	1.00	0.18	42.3	34.7	49.9
			16.3	42.3	-1.10		-46.5	-54.1	-38.9
10		5	16.3	42.3	1.00		42.3	34.7	49.9
			16.3	42.3	-1.40		-59.2	-66.8	-51.6
doors	1	4	16.3	42.3	0.94	0.18	39.8	32.1	47.4
			16.3	42.3	-1.04		-44.0	-51.6	-36.4
		5	16.3	42.3	0.94		39.8	32.1	47.4
			16.3	42.3	-1.28		-54.1	-61.8	-46.5

MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 1

#	Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi	(psf)
		<u> </u>	41.6	<u> </u>	0.80		29.3	21.7	36.9	(,, = .)
1	Windward Wall			Ų.00				21.7	30.5	
	Overhang Top	16.3	42.3		0.21		7.8			
1		16.3	42.3		-0.26		-9.7			
:	Overhang Bot	12.0	41.6		0.80		29.3			
2	Side Wall	16.3	42.3	0.88	-0.70	0.18	-26.1	-33.7	-18.4	
3	Leeward Wall	16.3	42.3	0.88	-0.50	0.18	-18.6	-26.2	-11.0	
4	Side Wall	16.3	42.3	0.88	-0.70	0.18	-26.1	-33.7	-18.4	
Α	Windward Roof	16.3	42.3	0.88	0.21	0.18	7.8	0.2	15.4	
		16.3	42.3		-0.26		-9.7	-17.3	-2.1	
В	Leeward Roof	16.3	42.3	0.88	-0.60	0.18	-22.3	-29.9	-14.7	
C&D	Roof	0 to 8.1	42.3	0.88	-0.90	0.18	-33.5	-41.1	-25.9	
		8.1 to 16.3	42.3				-33.5	-41.1	-25.9	
1		16.3 to 32.5	42.3		-0.50		-18.6	-26.2	-11.0	
		32.5 to 34.0	42.3		-0.30		-11.2	-18.8	-3.6	,

MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 2

#	Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi (psf)
1	Side Wall	16.3	42.3	0.89	-0.70	0.18	-26.4	-34.0	-18.7	
2	Windward Wall	15.0	41.6		0.80		29.6	22.0	37.2	
		16.3	42.3				30.1	22.5	37.7	
		20.5	44.4				31.6	24.0	39.2	
3	Side Wall	16.3	42.3	0.89	-0.70	0.18	-26.4	-34.0	-18.7	
4	Leeward Wall	16.3	42.3	0.89	-0.34	0.18	-12.8	-20.4	-5.2	
D	Windward Roof	16.3	42.3	0.89	0.29	0.18	10.9	3.3	18.5	
		16.3	42.3		-0.21		-7.9	-15.5	-0.3	ļ
С	Leeward Roof	16.3	42.3	0.89	-0.60	0.18	-22.6	-30.2	-15.0	
A&B	Roof	0 to 8.1	42.3	0.89	-0.90	0.18	-33.9	-41.5	-26.3	
		8.1 to 16.3	42.3				-33.9	-41.5	-26.3	
		16.3 to 32.5	42.3		-0.50		-18.8	-26.4	-11.2	
		32.5 to 60.5	42.3		-0.30		-11.3	-18.9	-3.7	

MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 3

#	Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi (psf)
1	Leeward Wall	16.3	42.3	0.88	-0.50	0.18	~18.6	-26.2	-11.0
2	Side Wall	16.3	42.3		-0.70		-26.1	-33.7	-18.4
3	Windward Wall	12.0	41.6	0.88	0.80	0.18	29.3	21.7	36.9
	Overhang Top	16.3	42.3		0.21	0	7.8		
		16.3	42.3		-0.26		-9.7		ļ
·	Overhang Bot	12.0	41.6	10	0.80		29.3		
4	Side Wall	16.3	42.3	0.88	-0.70	0.18	-26.1	-33.7	-18.4
В	Windward Roof	16.3	42.3	0.88	0.21	0.18	7.8	0.2	15.4
		16.3	42.3		-0.26		-9.7	-17.3	-2.1
A	Leeward Roof	16.3	42.3	0.88	-0.60	0.18	-22.3	-29.9	-14.7
C&D	Roof	0 to 8.1	42.3	0.88	-0.90	0.18	-33.5	-41.1	-25.9
		8.1 to 16.3	42.3				-33.5	-41.1	-25.9
		16.3 to 32.5	42.3		-0.50		-18.6	-26.2	-11.0
		32.5 to 34.0	42.3		-0.30		-11.2	-18.8	-3.6

MWFRS Net Pressures

This data was calculated using the building of all heights method.

Wind Direction 4

#	Surface	z (ft)	q (psf)	G	Ср	GCpi	Ext Pres (psf)	Net w/ +GCpi	(psf) Net w/ -GCpi (psf)
1	Side Wall	16.3	42.3	0.89	-0.70	0.18	-26.4	-34.0	-18.7	
2	Leeward Wall	16.3	42.3		-0.34		-12.8	-20.4	-5.2	
3	Side Wall	16.3	42.3	0.89	-0.70	0.18	-26.4	-34.0	-18.7	
4	Windward Wall	15.0	41.6	0.89	0.80	0.18	29.6	22.0	37.2	
		16.3	42.3				30.1	22.5	37.7	
		20.5	44.4				31.6	24.0	39.2	
c	Windward Roof	16.3	42.3	0.89	0.29	0.18	10.9	3.3	18.5	
		16.3	42.3		-0.21		-7.9	-15.5	-0.3	
D	Leeward Roof	16.3	42.3	0.89	-0.60	0.18	-22.6	-30.2	-15.0	
A&B	Roof	0 to 8.1	42.3	0.89	-0.90	0.18	-33.9	-41.5	-26.3	
		8.1 to 16.3	42.3				-33.9	-41.5	-26.3	
		16.3 to 32.5	42.3		-0.50		-18.8	-26.4	-11.2	- 1
it:		32.5 to 60.5	42.3		-0.30		-11.3	-18.9	-3.7	

$$psf := \frac{lbf}{ft^2} \qquad plf := \frac{lbf}{ft} \qquad psi := \frac{lbf}{in^2} \qquad ksi := 1000 \cdot psi \qquad K := 1000 \cdot lbf$$

Steel Properties

$$E_s := 29 \cdot 10^6 \cdot psi$$

Modulus of Elasticity of steel, 5.5.2.1

$$f_v := 60 \cdot ksi$$

Grade of steel

$$f_S := 0.4 \cdot f_y$$

$$f_S = 24 \text{ ksi}$$

Allowable stress in the steel, 7.2.1.1.b

$$d_{bar} := 5$$

Size of bar

Center to center spacing of filled cells

$$K := \frac{s}{12 \cdot in} \qquad K = 2$$

Multiplier for width of wall * wind pressure

The wind load to the wall section is

$$w := 49.7 \cdot plf$$

Load, plf, into wall section: wall weight, gravity load due to roof trusses

$$P_{wall} := 2500 \cdot lbf$$

Masonry Properties

$$E_m := 1.6 \cdot 10^6 \cdot psi$$

Modulus of Elasticity of concrete masonry units, Type S mortar, Table 5.5.2.3

$$f_m := 1500 \cdot psi$$

Allowable compressive strength of concrete masonry units

$$F_b := \frac{f_m}{3}$$

Allowable compressive stress in masonry due to bending and axial loading, 7.3.2.2

$$F_b = 500 psi$$

$$h_{masonry} := 7.625 \cdot in$$

Overall depth of section for nominal 8" masonry.

$$d_{masonry} := \frac{h_{masonry}}{2}$$

$$d_{masonry} = 3.81 \text{ in}$$

Bar is assumed to be placed at 3.81"

$$t_{faceshell} := 1.25 \cdot in$$

Nominal thickness of faceshell

$$A_e := (t_{faceshell} \cdot 12 \cdot in) \cdot 2$$

$$A_e = 30 \, \text{in}^2$$

Net area considered for loading, face shells mortared, per foot basis

The width of the compressive block is the lessor of

$$b_1 := s$$

$$b_1 = 24 in$$

Compare these values and choose the least value for b s = 24 in

$$b_2 := 6 \cdot h_{masonry}$$
 $b_2 = 45.75 \text{ in}$ 7.3.3.1.a and b

$$b_2 = 45.75 in$$

$$b := if(b_1 < b_2, b_1, b_2)$$
 $b = 24 in$

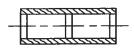
b = 24 in



Masonry Wall Design

Calculate the moment of Inertia and the radius of gyration for a typical wall section.

Consider the face shell only for a one foot section



$$h_{wall} = 12 ft$$

$$d_{\text{masonry}} = 3.813 \text{ in}$$

$$I1 := \frac{b1 \cdot t_{faceshell}}{12} \qquad I1 = 2 \text{ in}^4 \qquad \qquad A1 := b1 \cdot t_{faceshell} \qquad A1 = 15 \text{ in}^2 \qquad d1 := d_{masonry} - \frac{t_{faceshell}}{2} \qquad d1 = 3.19 \text{ in}$$

$$A1 := b1 \cdot t_{faceshell}$$
 A

$$d1 := d_{\text{masonry}} - \frac{t_{\text{faceshell}}}{2} \quad d1 = 1$$

$$d1 = 3.19 in$$

$$I := (I1 + A1 \cdot d1^{2}) \cdot 2$$
 $I = 308.71 \text{ in}^{4}$ $A := A1 \cdot 2$ $A = 30 \text{ in}^{2}$

$$A := A1 \cdot 2$$

$$A = 30 \, \text{in}^2$$

$$r := \sqrt{\frac{I}{A}}$$

$$r = 3.208 in$$

$$r = 3.208 \text{ in}$$
 $\frac{h_{\text{wall}}}{r} = 44.89$

Check for the allowable stress which is dependent upon the h/r ratio < 99

$$F_{a1} := \frac{1}{4} \cdot f_m \cdot \left[1 - \left(\frac{h_{wall}}{140 \cdot r} \right)^2 \right]$$

$$F_{a1} = 336.4 \, \text{psi}$$

$$F_{a2} \coloneqq \frac{1}{4} \cdot f_m \cdot \left(\frac{70 \cdot r}{h_{wall}} \right)^2$$

$$F_{a2} = 911.9 \, \text{psi}$$

$$F_{a2} = 911.9 \, psi$$
 6.3.2a yields F_{a2}

$$F_a := if \left(\frac{h_{wall}}{r} < 99, F_{a1}, F_{a2} \right)$$

$$F_a = 336.4 \, \text{psi}$$

Masonry Wall Design

Check for horizontal joint reinforcement

$$w = 49.7 plf$$

$$W := w \cdot \frac{8}{12}$$
 $W = 33.1 \text{ plf}$

Normalized the load for an 8" tall by s = 24 in wide section

$$1 = 24 in$$

$$M2 := \frac{W \cdot l^2}{12}$$

$$M2 = 132.5 \, lbf \cdot in$$

The moment of inertia for the masonry block is calculated

$$b1 := t_{faceshell}$$

$$d := h_{masonry}$$

$$b1 = 1.25 in$$

$$d = 7.625 in$$

$$d2 := \frac{d}{2} - \frac{b1}{2}$$

$$d2 = 3.188 in$$

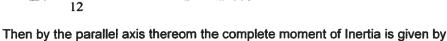
$$c := \frac{d}{2}$$

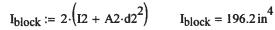
$$c = 3.813 in$$

$$A2 = 9.531 \text{ in}^2$$

$$I2 := \frac{d \cdot b1^3}{12}$$

$$I2 = 1.241 \, \text{in}^4$$





$$I_{block} = 196.2 \, \text{in}^4$$

$$f_{tension_actual} \coloneqq \frac{M2 \cdot c}{I_{block}}$$

$$f_t := 30 \cdot psi$$

Allowable flexure tension for concrete masonry units laid in Type S mortar, Table 6.3.1.1

 $f_{tension_actual} < f_t$

O.K. Horizontal joint reinforcement is not required

Masonry Wall Design

Ratio of steel stress to masonry stress

$$n := \frac{E_S}{E_m}$$

$$n = 18.125$$

$$A_{bar} := \frac{\pi}{4} \cdot \left(\frac{d_{bar}}{8}\right)^2 \cdot in^2 \qquad A_{bar} = 0.307 in^2$$

$$A_{bar} = 0.307 \, in^2$$

The moment at mid-span of the wall section is calculated

$$M := \frac{K \cdot w \cdot h_{\text{wall}}^2}{e}$$

$$M = 21470.4 lbf \cdot in$$

 $M := \frac{K \cdot w \cdot h_{wall}^{\ \ 2}}{8} \qquad M = 21470.4 \ lbf \cdot in \qquad \text{The moment at mid-span}$ of the wall section

$$\rho := \frac{A_{bar}}{b \cdot d_{masonry}}$$

uncracked section

$$k := \left[\sqrt{2 \cdot \rho \cdot n + (\rho \cdot n)^2} - \rho \cdot n \right]$$

$$k = 0.293$$

 $k \cdot d_{masonrv} = 1.118 in$

Location of neutral axis from outside face of block. Note that the neutral axis should be within the faceshell of the block, 1.25 inches

$$j := 1 - \frac{k}{3}$$

 $j = 0.90229$

$$j \cdot d_{\text{masonry}} = 3.44 \text{ in}$$

Length of lever arm between centers of Compression and Tension

$$f_b := \frac{M}{b \cdot d_{masonry}} \cdot \frac{2}{j \cdot k} \qquad \qquad f_b = 465.4 \, psi \qquad \qquad \text{Compressive stress in } \\ \text{masonry due to bending}$$

$$f_b = 465.4 \, \text{psi}$$

$$f_{S1} := \frac{M}{A_{bar} \cdot j \cdot d_{masonry}}$$
 $f_{S1} = 20.34 \text{ksi}$ Te

$$f_{S1} = 20.34 \, \text{ksi}$$

раг

$$A_{bar} = 0.307 \, \text{in}^2$$

Area of
$$d_{bar} = 5$$

Actual axial stresses due to wall weight, roof and floor load

 $f_a := \frac{P_{\text{wall}}}{A_a}$

$$f_a = 83.3 \text{ psi}$$

Check allowable stresses in masonry and steel and combined stresses

ACTUAL STRESS

ALLOWABLE STRESSES

$$f_{s1} = 20.3 \, \text{ks}$$

$$f_{S1} = 20.3 \text{ ksi}$$
 < $f_{S} \cdot \frac{4}{3} = 32 \text{ ksi}$ O.K.

$$f_a + f_b = 548.8 \, ps$$

$$f_a + f_b = 548.8 \,\text{psi}$$
 $\left(\frac{1}{3} \cdot f_m\right) \cdot \frac{4}{3} = 667 \,\text{psi}$ O.K. 7.3.2.2

$$f_a = 83.3 \text{ ps}$$

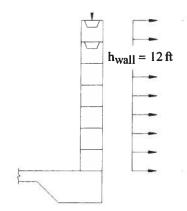
$$f_a = 83.3 \text{ psi}$$
 <= $(F_{a1}) \cdot \frac{4}{3} = 448.6 \text{ psi}$ O.K. 6.3.1.a

$$\frac{f_a}{F_a} + \frac{f_b}{F_b} = 1.179$$
 < 1.33 O.K.

Therefore the typical wall section can be reinforced with $d_{bar} = 5$ par @ s = 24 in i.c.. Further investigation should be given for openings, taller wall sections, snear wall reinforcing, and bond beam restraint for uplif

 $P_{wall} = 2500 lbf$

w = 49.7 plf



Masonry Pier Design openings WINDOW B END JAMBS

Assume that the load is transfered into the entire pier

This pier section shall carry the entire moment

$$h_{yyo} := 12.0 \cdot ft$$

$$h_{\text{wall}} := 12.0 \cdot \text{ft}$$
 $A_{\text{bar}} := .31 \cdot \text{in}^2$

$$W_{pier} := 1.33 \cdot ft$$

 $W_{pier} := 1.33 \cdot ft$ width of masonry pier

$$K := \frac{W_{pier}}{1 \cdot ft}$$

 $W_{opening} := 1.1 \cdot ft$ width of tributary opening(s)

$$W_{overall} := W_{pier} + W_{opening}$$

$$w = 49.7 plf$$
 zone 5 wind pressures

The wind force into the pier and door(s), due $p_{wind} := w$ to components cladding pressures

$$A_{bar} \coloneqq .31 {\cdot} in^2$$

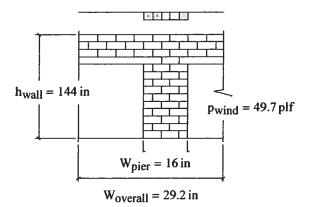
#5 bar PLACED AT 5.25"

$$n_{bar} := 2$$

number of bars in the masonry pier

The moment at mid-span of the wall section is calculated

$$M := \frac{p_{wind} \cdot \frac{\left(W_{pier} + W_{opening}\right)}{12 \cdot in} \cdot h_{wall}^{2}}{8}$$



 $M = 2173.9 lbf \cdot ft$

The moment at mid-span of the pier section

$$\rho := \frac{n_{bar} \cdot A_{bar}}{W_{pier} \cdot d_{masonry}}$$

 $\rho := \frac{n_{bar} \cdot A_{bar}}{W_{pier} \cdot d_{masonry}} \qquad \rho = 0.01019 \quad \text{Ratio of Area of steel to uncracked section}$

$$k := \left[\sqrt{2 \cdot \rho \cdot n + (\rho \cdot n)^2} - \rho \cdot n \right]$$

$$k = 0.451$$

$$k \cdot d_{masonry} = 1.718 in$$

Location of neutral axis from outside face of block

Check position of neutral axis, should be within faceshell of block, 1.25 in.

$$j := 1 - \frac{k}{3}$$
 $j = 0.84983$

Fill wall solid

$$j \cdot d_{masonry} = 3.24 in$$

Length of lever arm between centers of Compression and Tension

$$f_b := \frac{M}{W_{pier} \cdot d_{masonry}} \cdot \frac{2}{j \cdot k} \qquad f_b = 587.4 \, psi$$

$$f_b = 587.4 \, psi$$

Compressive stress in masonry due to bending

$$f_{s1} := \frac{M}{n_{bar} \cdot A_{bar} \cdot j \cdot d_{masonry}}$$
 $f_{s1} = 12.99 \text{ ksi}$

$$f_{s1} = 12.99 \, \text{ks}$$

Tensile stress in the steel

$$P := 1654 \cdot lbf$$

Axial loads due to wall weight, and roof system

$$f_a := \frac{P}{A_e \cdot K}$$

$$f_a = 41.5 \text{ psi}$$

Stress due to axial loads

$$F_a := F_b$$

$$F_a = 500 \text{ psi}$$

Allowable axial stress in masonry

Check allowable stresses in masonry pier and steel and combined stresses

ACTUAL STRESS ALLOWABLE STRESS

$$f_{s1} = 12.99 \text{ ksi}$$
 $f_{s} \cdot \frac{4}{3} = 32 \text{ ksi}$ O.K.

$$f_a + f_b = 628.9 \, ps$$

$$\left(\frac{1}{3} \cdot f_{\rm m}\right) \cdot \frac{4}{3} = 667 \,\mathrm{psi}$$

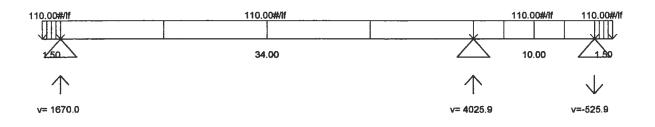
$$f_a = 41.45 \, ps$$

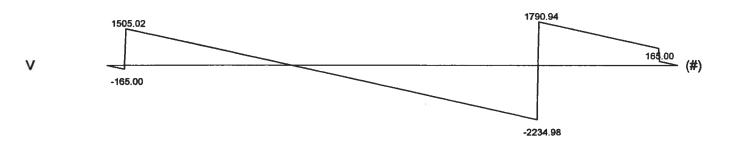
$$<<(F_{a1})\cdot\frac{4}{3}=448.59 \,\mathrm{psi}^{-6}$$

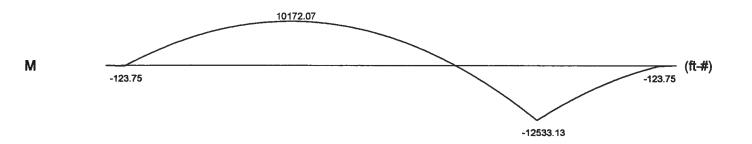
$$\frac{f_a}{F_{a1}} + \frac{f_b}{F_b} = 1.298$$

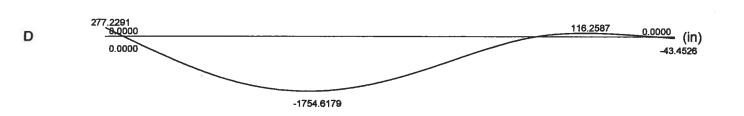
 $f_a + f_b = 628.9 \,\mathrm{psi}$ < $\left(\frac{1}{3} \cdot f_m\right) \cdot \frac{4}{3} = 667 \,\mathrm{psi}$ O.K. 7.3.2.2 $f_a = 41.45 \,\mathrm{psi}$ << $\left(F_{a1}\right) \cdot \frac{4}{3} = 448.59 \,\mathrm{psi}$ O.K. 6.3.1.a $\frac{f_a}{F_{a1}} + \frac{f_b}{F_b} = 1.298$ < 1.33 O.K. Load checks for 2-#5, pier detailed with 2-#5's MID

Therefore use 2-#5's MIN at EACH JAMB OF THE 16" CMU PIER, fm=1500 units





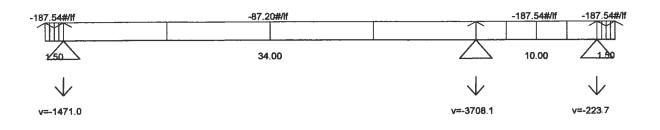


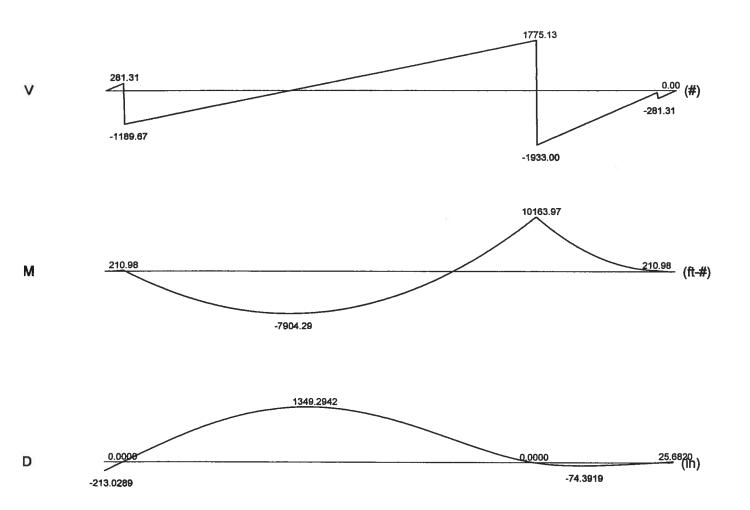


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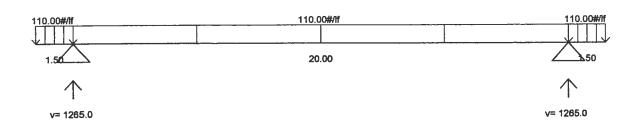


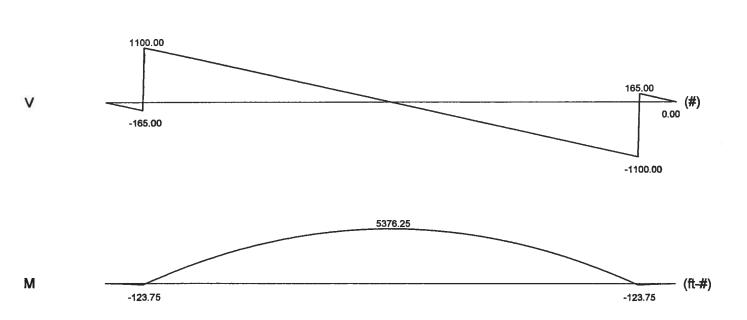


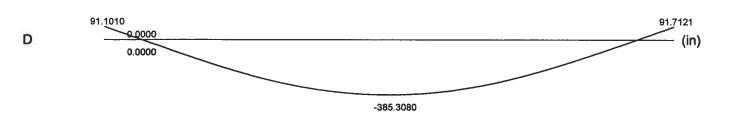
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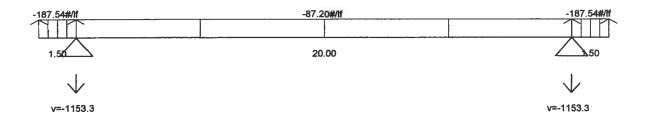


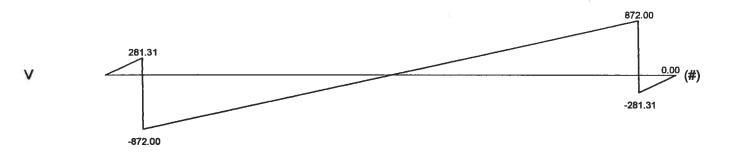


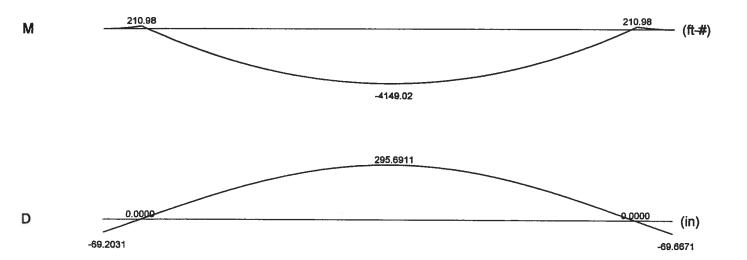


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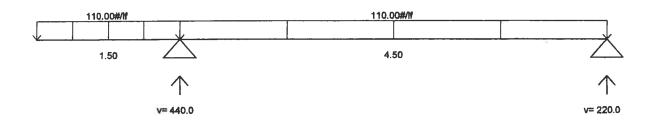


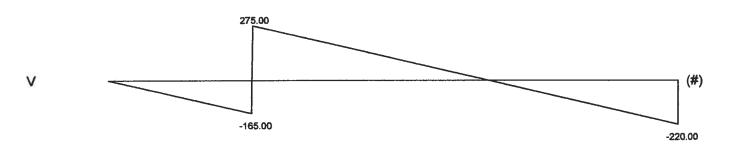


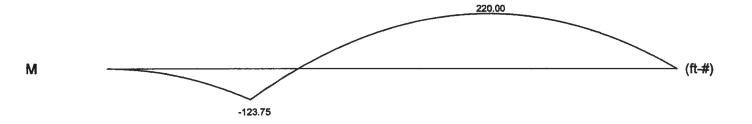
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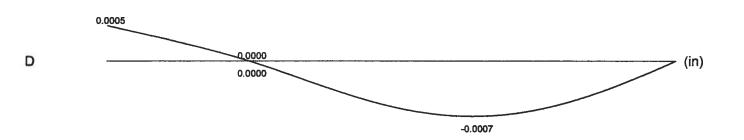
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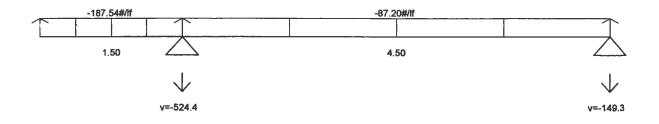


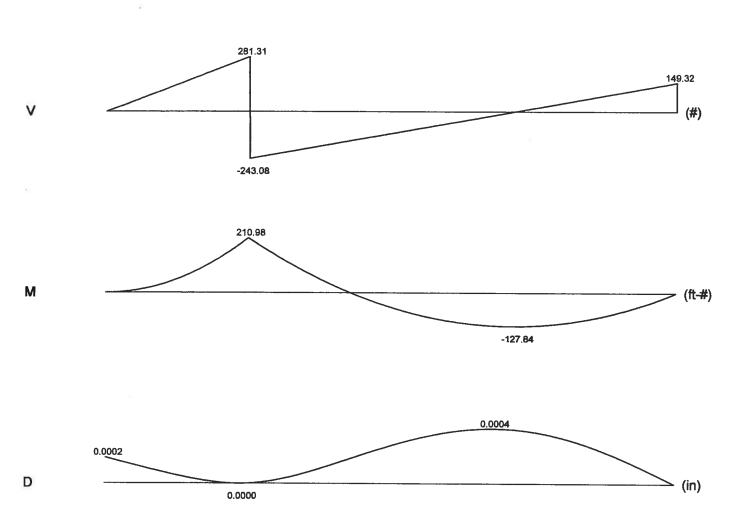




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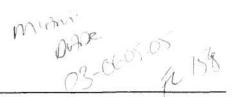




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8 INCH LINTEL ALLOWABLE SERVICE MOMENTS(FFLB)



CA:	ST- €	RETE				CRA	MY			
The same of the sa		TYPE	8U8	8F8-08	8F12-08	8F16-08	8F20-08	8F24-08	8F28-08	8F32-08
LENGTH*				878-18	8F12-18	SI-18	8F20-18	8F24-11	8F28-18	8F32-18
2'-10"	(34")	PRECAST	4161	4357	4705	6528	8353	10180	12010	13841
A 10	(017	I RECOUNT		8797 (14)	15309 (17)	21897 (14)	28494 (46)	23365 (40)	2/784 (19)	32211 (19)
3'-6"	(42")	PRECAST	4144	4329	4662	6472	8282	10095	11911	13727
	\			87/63 (16)	15266 (17)	21841 (10	28423 (44)	23200 (In)	27685 (17)	32097 (19
4'-0"	(48")	PRECAST	4129	4303	4624	6420	8218	10019	11821	13625
	1107			8737 nú	15228 (17)	21789 mm	28359 (44)	23294 (19)	27595 (14)	31995 (19
41-611	(54")	PRECAST	4111	4274	4580	6362	8144	9930	11718	13507
1	1317	THEOLOT		8700 (14)	151 84 m	21731 na	28285 (44)	23115 (19)	27492 (19)	31 077 (19
5'-4"	(64°)	PRECAST	4077	4217	4494	6247	8001	9759	11518	13278
-	1017	TREOLOI	1077	8651 (14)	15098 (17)	21616 (11)	28142 (44)	22944 (19)	27292 (19)	31648 (19
5'-10"	(70")	PRECAST	4053	4177	4435	6168	7903	9640	11380	13121
J - 10	(101	IRLODI	1030	8611 114	15039 (m	21537 (na	28044 (46)	22825 (19)	27154 (19)	31491 (14
6'-6"	(78")	PRECAST	6508	7403	12230	17097	14408	17619	20036	24057
0-0	(/0/	I KLOAJI	0300	11341 m	19800 (12)	28546 (13)	37228 (işi	45957 (14)	54716 (43)	41950 (14
7'-6"	(90")	PRECAST	6448	7303	12080	16897	14158	17319	20486	23657
/ 🗝	170 /	I WTOYN	טדרט	1124) no	19730 (12)	28346 (1th	34978 (13)	45657 (14)	54366 (cs)	41.550 (M
9'-4"	/110**	PRECAST	6315	7081	11747	16452	28926	36384	28534	33430
44	(112")	LYCON	0913	1101999	19397 (14)	27901 (13)	36422 (14)	44991 (14	53588 (40)	77326 (5
101 411	/19/"\	DDCCACT	6214	6913	11495	16117	28507	35880	27947	32759
10' - 6"	(126")	PRECAST	0214	10051 np	19145 (13)	27544 (12)	36003 (14)	44487 (14)	53001 (40	76655 (55
111 48	/19/5	DOLCACE	0.479	10405	17567	24826	39300	49316	5 9372	69456
11'-4"	(136")	PRECAST	8472	13736 m	24527 (n	35533 (m)	44566 (14)	57683 (III)	68849 rm	80049 m
TAL ALL	/1 AAM	DRECACT	0404	10291	17397	24599	39017	48976	5 89 75	69003
12'-0"	(144")	PRECAST	8404	13624 (7)	24357 (9)	35306 (le)	44283 (10)	57343 (IIII	69452 (m)	79596 (11
101 48	/1 / OB	DRECACT	201/	10045	17027	24106	38400	48236	58112	68016
13'-4"	(160")	PRECAST	8256	13378 (0	23987 (9)	34813 (m)	45666 (18)	56603 (m)	67589 (nn	78609 (11
PAL AN	/1/0W	DATCACT	017/	9911	16827	23839	38067	47836	57645	67483
14'-0"	(168")	PRECAST	8176	13244 m	23787 (11	34546 (1m)	45333 (10)	56203 (m)	67122 mi	7807£ m
	/17/8\	DO CETOCECEO	AID.	NR	NR	NR	NR	NR	NR	NR
14'-8"	(176")	PRESTRESSED	NR	13679 m	22196 (14)	41777 m	57838 (a)	74110 to	96452 m	106443 p
	/10/80	DD FCTD FCC FD	AID	NR	NR	NR	NR	NR	NR	NR
15'-4"	(184")	PRESTRESSED	NR	13524 (7)	21976 (10)	41494 (9)	57471 (a)	73670 m	89939 m	105 856 p
	/000m	DD FFT D FF C F D	110	NR	NR	NR	NR	NR	NR	NR
17'-4"	(208")	PRESTRESSED	NR	13044 (7)	21256 (11)	40524 (1)	56271 m	72230 (t)	88259 (9)	103936 a
				NR	NR	HR	NR	NR	NR	NR
19'-4"	(232")	PRESTRESSED	NR	12504 (7)	20446 (11).	39444 (1)	54921"(9)	70610 (n)	86369 (1)	10177619
	100000			NR	NR	NR	NR	NR	NR	HR
21'-4"	(256")	PRESTRESSED	NR	13101 (10)	20532 (12)	41977 (1)	68139 (14)	\$8670 (14)	109199(14)	129587(14
				NR	NR	NR	NR	NR	HR	NR
22'-0"	(264")	PRESTRESSED	NR	12887 (10)	20212 (13)	41550 m	67606 (14)	88030 (H)	108452(14)	128734na
				NR	NR	NR	NR	NR	NR	NR
24'-0"	(288")	PRESTRESSED	NR	12207 (10)	19192 (12)	40190 (1sp	65906 (14)	85990 (15)	106072(15)	126014 (15)

8 INCH LINTEL ALLOWABLE SERVICE SHEARS(POUNDS)

CA	ST-	RETE				GRA	WITY			
ENGTH*		TYPE	8U8	8F8-08 8F8-18	8F12-08	8F16-08	8F20-08	8F24-08 8F24-18	8F28-08	8F32-08
				3037	4587	6129	7617	9118	10632	12162
2'-10"	(34")	PRECAST	2212	3037	4517	4129	7617	9118	10632	12162
	4.400			3017	4557	6089	7567	9058	10562	12082
3' - 6"	(42")	PRECAST	2200	3017	4557	4007	7567	9058	10542	12002
	4.400)	2256167		3002	4535	6059	7530	9013	10510	12022
4'-0"	(48")	PRECAST	2191	3002	4535	6059	7530	9013	10510	12022
41 4E	4 F 483	DDFC167	0100	2987	4512	6029	7492	8968	10457	11962
4'-6"	(54")	PRECAST	2182	2987	4512	6029	7492	8968	10457	11962
AT AT	44.483	DDFCACT	01/7	2962	4475	5979	7430	8893	10370	11862
5'-4"	(64")	PRECAST	2167	7962	4475	5979	7430	8893	10370	11862
el lan	(708)	DRECACT	01.50	2947	4452	5949	7392	8848	10317	11802
5'-10"	(70")	PRECAST	2158	2947	4452	5949	7392	1948	10317	11802
/1 /5	/70"\	DRECACT	014/	2927	4422	5909	7342	8788	10247	11722
61–611	(78")	PRECAST	2146	2927	4422	5909	7342	8788	102G	11722
71 / 11	/00%	DOLCTC	0100	2897	4377	5849	7267	8698	10142	11602
7' -6"	(90°)	PRECAST	2128	2097	4377	5849	7267	1478	10142	11602
NE AII	/110%	DOFCACT	2000	2842	4295	5739	7130	8533	9950	11382
9'-4"	(112")	PRECAST	209 5	2842	4295	5739	7130	8533	9950	11382
101 40	/10/5	DOCCACT	0074	2807	4242	5669	7042	8428	9827	11242
10' - 6"	(126")	PRECAST	2074	2007	4242	5669	7042	8428	9877	11242
111 48	/19/81	DRICACT	1701	2782	4205	5619	6980	8353	9740	11142
11'-4"	(136")	PRECAST	1731	2782	4205	5419	6980	8353	9749	11142
IAL AN	/3.4493	DDFCACT	1704	2762	4175	5579	6930	8293	9670	11062
12'-0"	(144")	PRECAST	1724	2762	4175	5579	6936	8293	1676	11062
101 48	/1/05	DATCACT	1711	2722	4115	5499	6830	8173	9530	10902
13'-4"	(160")	PRECAST	1711	7/22	4115	5499	6830	8173	9530	10902
HA IN	/1/0°\	DOLCACE	170/	2702	4085	5459	6780	8113	9460	10822
14'-0"	(168")	PRECAST	1706	2702	4085	5459	6780	8113	9460	10822
141 60	/17/P\	DOCTOCCED	ND	NR	NR	NR	NR	NR	NR	NR
14' - 8"	(176")	PRESTRESSED	NR	3010	4886	8092	10600	11763	12349	12402
1 51 411	/104")	DDCCTDCCCCD	NR	NR	NR	NR	NR	NR	NR	NR
15' -4 "	(184")	PRESTRESSED	nk	2845	4647	7799	10236	11469	12156	12329
17'-4"	/200°\	DOCCTDECCED	ND	NR	NR	NR	NR	NR	NR	NR
1/-4	(208")	PRESTRESSED	NR	2372	4150	6878	9158	10576	11537	12090
101 40	/000F\	DOLCLOCCED	ND	NR	NR	NR	NR	NR	NR	NR
19' -4 "	(232")	PRESTRESSED	NR	2093	3601	6100	8199	9708	10968	11851
911_AT	13544	DDECTDECCED	NR	NR	NR	NR	MR	NR	NR	NR
21'-4"	(256")	PRESTRESSED	пK	1783	3255	5525	7526	9550	11190	11606
DOI AH	104 431	DOCCTBECCEN	MD	NR	NR	HR	NR	NR	NR	NR
22'-0"	(264")	PRESTRESSED	NR	1690	3120	5265	7244	9325	11016	11527
AAI AII	/200°1	DRECTRECER	ND.	NR	NR	NR	NR	NR	NR	NR
24'-0"	(288")	PRESTRESSED	NR	1450	2727	4758	6697	8757	10454	11291

8 INCH LINTEL ALLOWABLE SERVICE MOMENTS(FFLB)

\$	-TZ/	CRETE				UPLIFI			
		TYPE	8F8-1T	8F12-1T	8F16-11	8F20-1T	8F24-1T	8F28-1T	8F32-1T
ENGTH*			8F8-2T	8F12-2F	#16-2T	#20-21	W24-27	8F28-2T	#32-2I
2'-10"	(34")	PRECAST	4737 (31)	6238 (3)	10353 (17)	13494 (17)	16655 (17)	19828 (17)	23014 (17
4-10	134 /	FRECASI	8470 (25)	7775	13834	21126	29512 tn	38387 (17)	44624 (17
3'-6"	(42")	PRECAST	4765 (31)	6281 (3)	10409 (14)	13565 (16)	16740 (16)	19927 (16)	23127 (10
3 -0	172 /	I KLOASI	8694 (2h)	711	13090	2117	29597 (9)	39494 (12)	44737 (11
4'-0"	(48")	PRECAST	4791 (31)	6319 (3)	10461 (16)	13629 (16)	16817 (16)	20017 (14)	23229 (10
7-0	(10/	TRECASI	8724 (20)	784	13942	27261	29474 tn	38574 (tr)	44839 (17
4'-6"	(54")	PRECAST	4820 (31)	6363 (3)	10519 (14)	13703 (16)	16905 (14)	20120 (16)	23347 (1
7-0	1347	INLONG	1753 (29)	7900	14000	71335	29762 (M	38679 (16)	44957 (N
5'-4"	(64")	PRECAST	4878 (30)	6449 (3)	10634 (14)	13846 (16)	17077 (16)	20320 (14)	23576 (1
	(177)	1 KEODI	8811 (29)	7986	14115	21471	29934 (9)	38079 (14)	451 14 (1
5'-10"	(70")	PRECAST	4917 (30)	6508 (3)	10713 (14)	13944 (16)	17195 (14)	20458 (16)	23734 (1
9-10	(707	I RECASI	8850 (29)	8045	14194	2)576	30052 (N	39017 ri4	45344 D
6'-6"	(78")	PRECAST	4975 (30)	6596 (3)	10829 (16)	14090 (16)	17370 (16)	20662 (16)	23967 (1
	1/0/	I KLOOJI	8908 (20)	8133	14310	21722	30227 (%	39221 (14)	45577 (1
7'-6"	(90")	PRECAST	5075 (29)	6746 (3)	11029 (16)	14340 (16)	17670 (14)	21012 (14)	24367 (1
, –	(70)	I KLOOI	9000 (30)	8203	14510	21,972	30527 (1)	39577 mg	45977 (t
9'-4"	(112")	PRECAST	8167 (16)	707 9 (3)	11474 (15)	14896 (15)	18337 (15)	21790 (15)	25256 (1
7 -4	(112)	i klosi	11564 (m	8616	14955	757	31194 (n	40349 (14)	46 86 n
10'-6"	(126")	PRECAST	8334 (16)	7331 (3)	11809 (14)	15315 (15)	18840 (15)	22377 (15)	25927 (1
10 -0	(120 /	I RECASI	11731 mg	8868	15298	22947	31697 m	40936 (10)	47537 b
11'-4"	(136")	PRECAST	8467 (15)	75 29 (3)	12074 (14)	15646 (14)	19237 (14)	22840 (14)	26456 (1
11 -4	1130 /	I KLOOI	11864 (10)	9066	15555	73778	32094 (a)	41399 (15)	48066 (i
12'-0"	(144")	PRECAST	8580 (15)	7699 (3)	12301 (14)	15929 (14)	19577 (14)	23237 (14)	26909 (1
14 7	(1111)	I KLODJI	11977 cm	9236	15742	23561	32434 (8)	417% ns	48519 p
13'-4"	(160")	PRECAST	8827 (15)	8069 (2)	12794 (13)	16546 (13)	20317 (14)	24100 (14)	2789 6 (1
13 -7	(1007	1 KLOOJI	12224 (14)	9606	16275	24178	33174 (4)	42659 (15)	49506 (1
14'-0"	(168")	PRECAST	8960 (15)	8269 (2)	13061 (13)	16879 (13)	20717 (13)	24547 (13)	28429 (1
14-0	(100 /	i klodi	12357 (18)	9806	16542	24511	33574	43126 ns	50039 (1
14'-8"	(176")	PRESTRESSED	7041 (12)	8479 (2)	13341 (13)	17229 (13)	21137 (13)	25057 (13)	28989 (1
14 -0	11707	1 KENIKENSEN	97,66 (15)	10014	16822	24861	33994 (a)	43616 (15)	50599 (1
15'-4"	(184")	PRESTRESSED	7188 (12)	8699 (2)	13634 (13)	17596 (13)	21577 (13)	25570 (13)	29576 (1
19 -4	(101)	IVININI	9313 ms	10234	17115	25224	34434 m	44129 (14)	511 84 (1
17'-4"	(208")	PRESTRESSED	7668 (11)	9419 (2)	14594 (12)	18796 (12)	23017 (12)	27250 (12)	31496 (1
17 -4	1200 /	INDINDIN	9793 ne	10956	18075	26428	35974 m	45809 (14).	53104 (t
19'-4"	(2327)	PRESTRESSED	8208 (11)	10229 (2)	15674 (11)	20146 (11)	24637 (11)	29140 (11)	33656 (1
17 -4	(Z3Z)	עזנטאוטאו	10333 pm	11764	19155	21771	37494 UT	47699 (13)	55 266 (1:
21'-4"	(256")	PRESTRESSED	8808 (10)	11129 (2)	16874 (10)	21646 (10)	26437 (10)	31240 (10)	36056 (11
a i 🔫	(170)	i MUINUJEV	13746 na	12666	20355	29278	39294 (r)	49799 (13)	57 664 (13
22'-0"	(264")	PRESTRESSED	9021 (10)	11449 (2)	17301 (10)	22179 (10) ~	27077 (10)	31987 (10)	36909 (10
44 - V	LUT	INDINDIN	13959 (TA)	12986	20702	29811	39934 m	50546 (rb)	58519 (iii
24'-0"	(288")	PRESTRESSED	9701 (9)	12469 (2)	18661 (9)	23879 (9)	29117 (9)	34367 (10)	39629 (10
AT T	(200	IVENIVEN	14639 (15)	14006	22142	31511	41974 (4)	52926 (12)	61237 (12

8 INCH LINTELS ALLOWABLE SERVICE SHEARS (POUNDS)

C	-T2	CRETE				UPLIFT			
PHOTHS		TYPE	8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-11
LENGTH*			8F8-2T	0F12-2T	8F16-2T	0F20-2T	8F24-2T	8728-2T	# BF32-7
2'-10"	(34")	PRECAST	2465	3967	5575	7184	8792	10402	12010
	(0.7	T REGIO	2465	3967	5575	7184	8792	10402	12010
31-611	(42")	PRECAST	2485	3997	5615	7234	8852	10472	12090
	1127	I REGIO	2425	3997	5615.	7234	2852	10472	12090
4'-0"	(48")	PRECAST	2500	4019	5645	7271	8897	10524	12150
T V	1107	(KLOO)	2500	4619	5645	727	1877	10524	12150
4'-6"	(54")	PRECAST	2515	4042	5675	7309	8942	10577	12210
7 - 0	(31)	INLODI	2515.	4042	5675	7309	8942	10577	12210
5'-4"	(64")	PRECAST	2540	4079	5725	7371	9017	10664	12310
7 7	(17)	I RECOI	2549	4079	5775	1 7371 A	9017	10664	12316
5'-10"	(70")	PRECAST	2555	4102	5755	7409	9062	10717	12370
<i>,</i> – 1 V	(101	! vrQQ1	2555	4102	5755	7409	9062	10717	12370
6'-6"	(78")	PRECAST	2575	4132	5795	7459	9122	10787	12450
UU	(/0/	I KLUASI	2575-	4132	5795	7459	9122	10710	12450
7'-6"	(90")	PRECAST	2605	4177	5855	7534	9212	10892	12570
/ -0	170]	rkecasi	2605	4177	5855	7534	9212	10992	12570
9'-4"	/119%	DDECACT	2660	4259	5965	7671	9377	11084	12790
y	(112")	PRECAST	2660	4259	5965	7671	9377	11084	12790
101 40	/196°	DDCCACT	2695	4312	6035	7759	9482	11207	12930
10'-6"	(126")	PRECAST	2695	4312	6035	715	9482	11207	12930
1 11 AD	/19/"	DDFCACT	2720	4349	6085	7821	9557	11294	13030
11'-4"	(136")	PRECAST	2/20	4349	6005	7120	9557	11294	13030
101 011	/1449	DDTCACT	2740	4379	6125	7871	9617	11364	13110
12'-0"	(144")	PRECAST	2740	4379	6125	7171	9417	11364	13110
141 411	/1/08\	DOLCACE	2780	4439	6205	7971	9737	11504	13270
13'-4"	(160°)	PRECAST	2780	4439	6205	7923	9737	11504	13270
S ALL AN	/1/08\	DRECACT	2800	4469	6245	8021	9797	11574	13350
14'-0"	(168")	PRECAST	2100	4469	6245	8021	9797	11574	13350
	/17/81	NACCTAPECED.	3008	4499	6285	8071	9857	11644	13430
14'-8"	(176")	PRESTRESSED	3008	4499	4285	8071	9857	W.11644.78	13430
	/10/81	DDCCTDCCCD	3028	4529	6325	8121	9917	11714	13510
15' -4"	(184")	PRESTRESSED	3028	4529	6325	8121	9917	11714	13510
	10000		3088	4619	6445	8271	10097	11924	13750
17'-4"	(208")	PRESTRESSED	3008	4619	6445	1271	10097	11924	13750
	1000	DATE PARTY OF	3148	4709	6565	8421	10277	12134	13990
19'–4"	(232")	PRESTRESSED	3148	4709	6565	8421	10277	12134	13990
	100.		3208	4799	6685	8571	10457	12344	14230
21' -4 "	(256")	PRESTRESSED	3206	4799	6685	8571	10457	12344	14230
			3228	4829	6725	8621	10517	12414	14310
22'-0"	(264")	PRESTRESSED	3228	4829	6725	8621	10517	12414	14310
			3288	4919	6845	8771	10697	12624	14550
24'-0"	(288")	PRESTRESSED	3200	4919	6845	8771	10697	12624	14550

8 INCH LINTEL

ALLOWABLE SERVICE MOMENTS(FFLB)

CAS	ST-C	RETE		LATERA	
LENGTH*		TYPE	8U8	8F8	REINF. CMU**
2'-10"	THRU	5'-10"	1258	1283	2294
6'-6"	THRU	7'-6"	3428	3428	2294
9'-4"	THRU	10'-6"	5180	5180	2294
11'-4"	THRU	14'-0"	6836	6836	2294
14'-8"	THRU	19'-4"	· M	8250	2294
21'-4"	THRU	24'-0"	NR	9938	2294

^{*} LENGTH = OVERALL LENGTH OF LINTEL

8 INCH LINTEL

ABLE SERVICE SHEAR(POUNDS)

CAS	T-\$	R€T€		MEN	
LENGTH*		TYPE	8U8	8F8	REINF. CMU**
2'-10"	THRU	14'-0"	1293	2621	2135
14'-8"	THRU	24'-0"	NR	3187	2135

^{*} LENGTH = OVERALL LENGTH OF LINTEL

^{**} SEE SAFE LOAD TABLES BROCHURE DATED JUNE 2003 FOR APPLICATION

^{**} SEE SAFE LOAD TABLES BROCHURE DATED JUNE 2003 FOR APPLICATION

February 3, 2006

To:

Scott Rosenboom

From: Ron Klein

Attached are the two signed and sealed record sets for the permit application. I think the only additional things you need are the manual J and D forms.

I included the standard electrical load calculations and one sealed and one copy of the survey.

You need to get me a contract. I am ready to start ASAP now that I have finally been able to finish the plans.

NOTE: we made one change since we talked. Brigitte just didn't like the way the dormer looked., so I removed it. You will need to let the truss people know so they can revise their drawings. If they want a new electronic file I can send it up to them.

STANDARD CALCULATI	ON: ONE-FA	MILY DWELLING	
1. GENERAL LIGHTING: Table 220-3(a) 2/00 sq ft × 3 VA = Small appliances: 220-16(a) /5(2) VA × 3 circuits = Laundry: 220-16(b) /500 VA × 1 =	4500 VA 4500 VA 1500 VA	it snau not be rep	d for instructional use only
Applying Demand Factors: Table 220-11 First 3000 VA × 100% = Next 3000 VA × 35% = Remaining VA × 25% = Total	3000 VA 3255 VA 0 VA 6255 VA	PHASES	NEUTRAL VA
2. FIXED APPLIANCES: 220-17 Dishwasher =		9750 VA	(120 V Lands × 75%)
3. DRYER: 220-18; Table 220-18 SOO VA × 100 % = 4. COOKING EQUIPMENT: Table 220-19: Notes	3000 VA	<u> </u>	VA
Col A 4000 VA × 100 % = Col B 4000 VA × 100 % = Col C VA × 100 % = Total	VAVAVA	<u>\$ 000</u> VA × 70% =	
5. HEATING or A/C: 220-21 Heating unit = 2500 VA × 100% = A/C unit = VA × 100% = Heat pump = VA × 100% = Largest Load	/ 2 500 VA VA VA VA VA	/2 5 5 0 VA	
N VA × 25% =	VA	625 VA	VA
1¢ service: PHASES $I = \frac{42720 \text{ VA}}{\text{V}} = \frac{175}{\text{V}}$ NEUTRAL $I = \frac{\text{VA}}{\text{V}} = \frac{1}{\text{V}} = $		VA	VA
220-22; First 200 A × 100% = Remaining A × 70% = Total	200 A A		·

2/3/06



5602 N.W. 13th STREET GAINESVILLE, FLORIDA 32653-2198

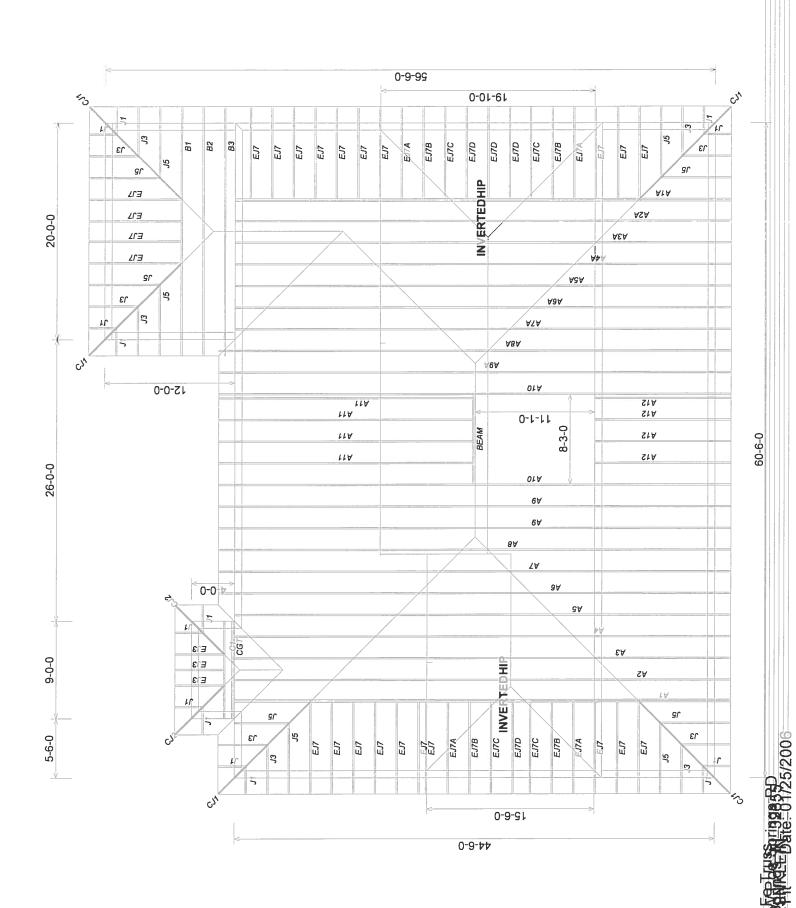
PO. BOX 5875 GAINESVILLE, FLORIDA 32627-5875

PHONE (352) 373-3642 FAX (352) 373-9037

CERTIFICATE OF PROTECTIVE TREATMENT

Builder:	Kosentinom		
Date:	5-30-06 Time:	PI AM	PΜ
Site Location:	2535 SUCP 77K		
Area Treated:	Living Futter, Porch	<i></i>	
Product Used:	B. F. L. T. Chemical Used: B.	Feathrie	
% Concentration:	Of M # Gallons Used:	500	1 1
Applicator:	Tran		

ATT GROW KLEIN



RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ----- 100 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

APPLICANT -- PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL REQUIREMENTS: Two (2) complete sets of plans containing the following:

Plans Examiner Site Plan including: a) Dimensions of lot

All drawings must be clear, concise and drawn to scale ("Optional" details that are not used shall be marked void or crossed off). Square footage of different areas shall be shown on plans.

Designers name and signature on document (FBC 106.1). If licensed architect or engineer, official seal shall be affixed.

- b) Dimensions of building set backs
- Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.
- Provide a full legal description of property.

Wind-load Engineering Summary, calculations and any details required Plans or specifications must state compliance with FBC Section 1609.

The following information must be shown as per section 1603.1.4 FBC

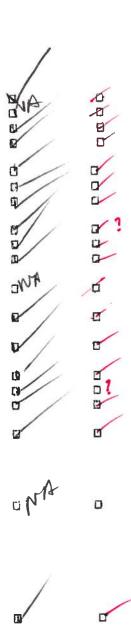
- a. Basic wind speed (3-second gust), miles per hour (km/hr)
- b. Wind importance factor, Iw, and building classification from Table 1604.3 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.
- c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.
- d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.
- e. Components and Cladding. The design wind pressures in terms of psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.

Elevations Including: a) All sides

b) Roof pitch

c) Overhang dimensions and detail with attic ventilation

1



- d) Location, size and height above roof of chimneys.
- e) Location and size of skylights
- f) Building height
- e) Number of stories

Floor Plan includios:

- a) Rooms labeled and dimensioned.
- b) Shear walls identified.
- c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attack forms).
- d) Show safety glazing of glass, where required by code.
- e) Identify egress windows in bedrooms, and size.
- t) Fireplace (gas vented), (gas non-vented) or wood burning with hearth, (Please circle applicable type).
- g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
- h) Must show and identify accessibility requirements (accessible bathroom) Foundation Plan including:
- a) Location of all load-bearing wall with required footings indicated as standard or monolithic and dimensions and reinforcing.
- b) All posts and/or column footing including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel.

Roof System:

- a) Truss package including:
 - 1. Truss layout and truss details signed and scaled by Fl. Pro. Eng.
 - 2. Roof assembly (FBC 106.1.1.2)Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
 - 1. Rafter size, species and specing

 - Attachment to wall and uplift
 Ridge beam sized and valley framing and support details
 - 4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections including:

- a) Masonry wall
 - 1. All materials making up wail
 - 2. Block size and mortar type with size and spacing of reinforcement
 - Lintel, tie-beam sizes and reinforcement
 - Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 - 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
 - Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 - Fire resistant construction (if required)
 - 8. Pireproofing requirements
 - Shoe type of termite treatment (termiticide or alternative method)
 - 10. Slab on grade
 - Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or Welded fire fabric reinforcement and supports
 - 11. Indicate where pressure treated wood will be placed
 - 12. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity

2

c. Crawl space (if applicable)

D	u	b) Wood frame wall
		1. All materials making up wall
		2. Size and species of studs
		3. Sheathing size, type and nailing schedule
		4. Henders sized
		5. Gable end showing balloon framing detail or gable truss and wall
		hinge bracing detail 6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor boits and washers)
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Roofing system, materials, manufacturer, fastening
		requirements and product evaluation with wind resistance rating)
		8. Fire resistant construction (if applicable)
		9. Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)
		11. Slab on grade
		a, Vapor retarder (6Mil. Polyethylene with joints iapped 6
		inches and sealed
		 Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports
		12. Indicate where pressure treated wood will be placed
		13. Provide insulation R value for the following:
		a. Attic space
		b. Exterior wall cavity
		c. Crawl space (if applicable)
	0	c) Metal frame wall and roof (designed, signed and sealed by Florida Prof.
_		Engineer or Architect)
		Floor Framing System:
□Ńγ		a) Floor truss package including layout and details, signed and scaled by Florida
N		Registered Professional Engineer
מ		b) Floor joist size and spacing
		c) Girder size and spacing
LJ	ĊΪ	d) Attachment of joist to girder
	0	e) Wind load requirements where applicable
	ฉ	Plumbing Finture layout
- 'V		Electrical layout including:
	G	a) Switches, outlets/receptacles, fighting and all required GFCI outlets identified
b /		b) Ceiling fans
ฮ์ /	13	c) Smoke detectors
		d) Service panel and sub-panel size and location(s)
13/	13	e) Meter location with type of service entrance (overhead or underground)
19	D.	f) Appliances and HVAC equipment
3 /		g) Arc Fault Circuits (AFCI) in bedrooms
ď		h) Exhaust fans in bathroom
		HYAC information a) Energy Calculations (dimensions shall match plans)
		b) Manual J sizing equipment or equivalent computation
đ		c) Clas System Type (LP or Natural) Location and BTU demand of equipment
0	a l	CA185 DYSTRIL 19PE (LF OI 1788 LLF) LOCALION COLUMN C. S.
		Disclosure Statement for Owner Builders *** Notice Of Commencement Required Before Any Inspections Will Be Done
		Private Potable Water
3		a) Size of pump motor
		b) Size of pressure tank
		c) Cycle stop valve if used
		a) along and there a mean
		3
		(Seith L. I

E€P° 12 5000 00:120W 60

LEOW : COLUMEIA CO BUILDING + ZONING PAX NO, :386-758-2168

PGT WinGuard

PRODUCT

Single Hung Window (Impact)	SH-701	
Horizontal Roller Window (Impact)	HR-710	
Casement Window (Impact)	CA-740	-
Casement Projected Window (Impact)	-CA-740	-
Fixed Window (Impact, Picture Window & Shapes)	PW-701 - New	
Sliding Glass Door (Impact, 2-track)	-SGD-70R	!
French Door (Impact)	SWD-101	. 1
French Door w/Sidelites (Impact)	SWD-101	: .1
French Door w/Sidelites (Impact)	FD-750	1

1" clipped Mullions



MIAMI-DADE COUNTY, FLORIDA METRO-DADE FLAGLER BUILDING 140 WEST FLAGLER STREET, SUITE 1603 MIAMI, FLORIDA 33130-1563 (305) 375-2901 FAX (305) 375-2908

NOTICE OF ACCEPTANCE (NOA)

PGT Industries 1070 Technology Drive Nokomis, FL 34274

SCOPE:

This NOA is being issued under the applicable rules and regulations governing the use of construction materials. The documentation submitted has been reviewed by Miami-Dade County Product Control Division and accepted by the Board of Rules and Appeals (BORA) to be used in Miami Dade County and other areas where allowed by the Authority Having Jurisdiction (AHJ).

This NOA shall not be valid after the expiration date stated below. The Miami-Dade County Product Control Division (In Miami Dade County) and/or the AHJ (in areas other than Miami Dade County) reserve the right to have this product or material tested for quality assurance purposes. If this product or material fails to perform in the accepted manner, the manufacturer will incur the expense of such testing and the AHJ may immediately revoke, modify, or suspend the use of such product or material within their jurisdiction. BORA reserves the right to revoke this acceptance, if it is determined by Miami-Dade County Product Control Division that this product or material fails to meet the requirements of the applicable building code.

This product is approved as described herein, and has been designed to comply with the Florida Building Code, including the High Velocity Hurricane Zone.

DESCRIPTION: Series "HS 710" Aluminum Horizontal Sliding Window

APPROVAL DOCUMENT: Drawing No.4112, titled "Aluminum Horizontal Sliding Window", sheets 1 through 6 of 6, prepared by PGT Industries, dated 2/16/98, with revisions 12/29/03 signed sealed by Robert L. Clark, P.E., bearing the Miami-Dade County Product Control Revision stamp with the Notice of Acceptance number and expiration date by the Miami-Dade County Product Control Division.

MISSILE IMPACT RATING: Large and Small Missile Impact

LABELING: Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved", unless otherwise noted herein.

RENEWAL of this NOA shall be considered after a renewal application has been filed and there has been no change in the applicable building code negatively affecting the performance of this product.

TERMINATION of this NOA will occur after the expiration date or if there has been a revision or change in the materials, use, and/or manufacture of the product or process. Misuse of this NOA as an endorsement of any product, for sales, advertising or any other purposes shall automatically terminate this NOA. Failure to comply with any section of this NOA shall be cause for termination and removal of NOA.

ADVERTISEMENT: The NOA number preceded by the words Miami-Dade County, Florida, and followed by the expiration date may be displayed in advertising literature. If any portion of the NOA is displayed, then it shall be done in its entirety.

INSPECTION: A copy of this entire NOA shall be provided to the user by the manufacturer or its distributors and shall be available for inspection at the job site at the request of the Building Official.

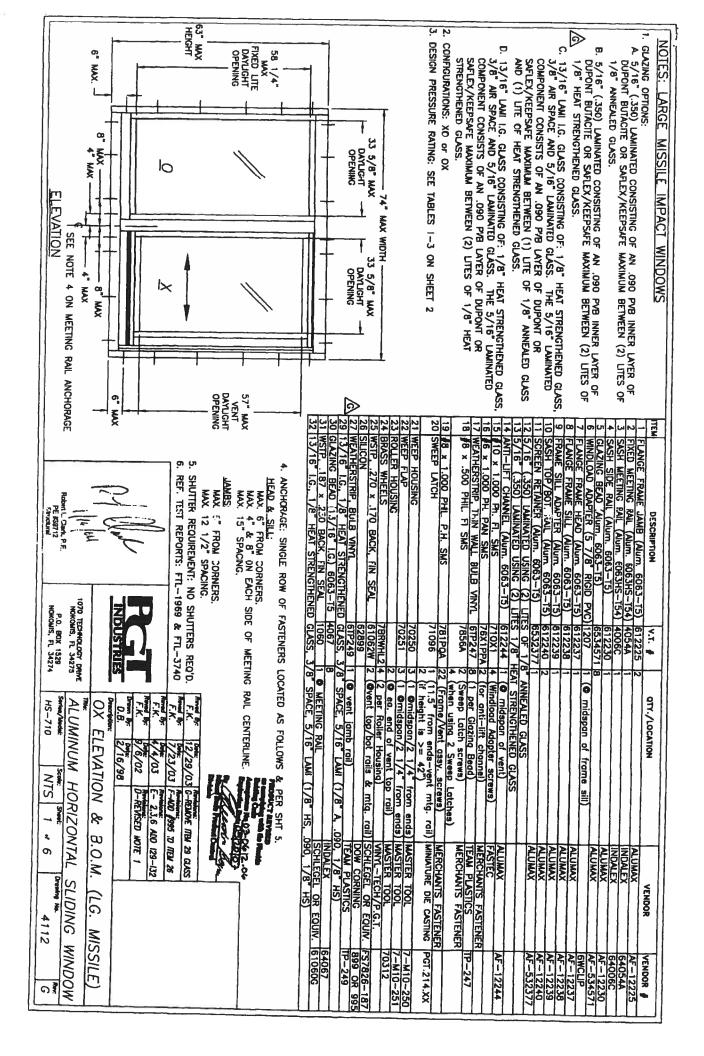
This NOA revises NOA # 02-0305.02 and, consists of this page 1 and evidence page E-1 and E-2, as well as approval document mentioned above.

The submitted documentation was reviewed by Theodore Berman, P.E.

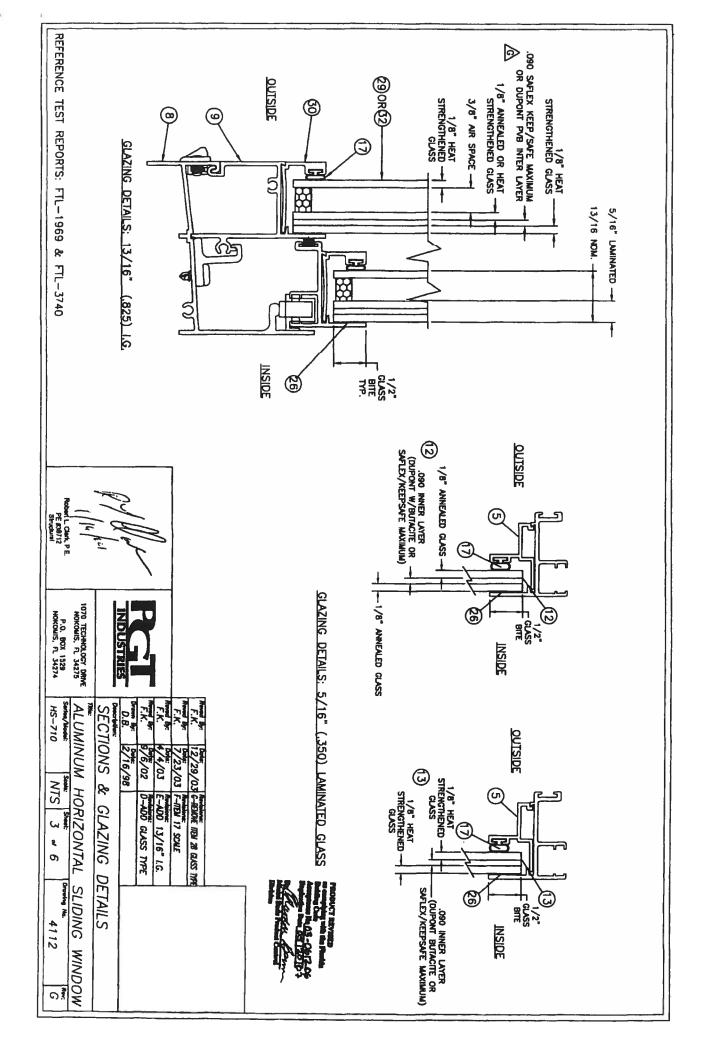
NOA No 03-0612.06 Expiration Date: May 20, 2007 Approval Date: April 08, 2004

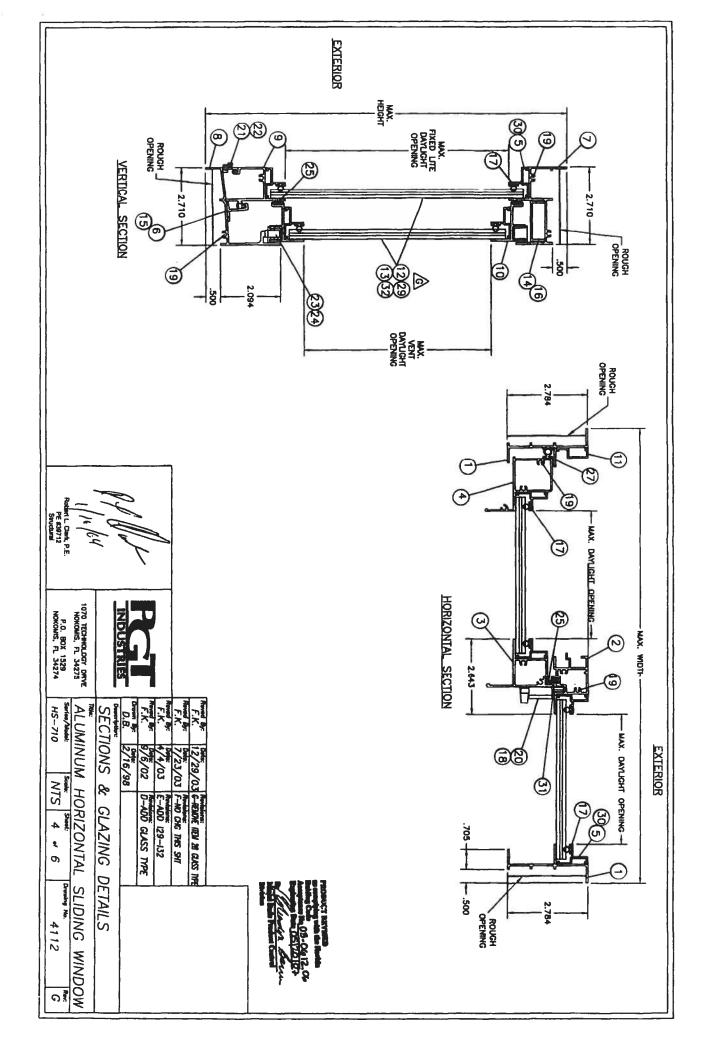
Page 1

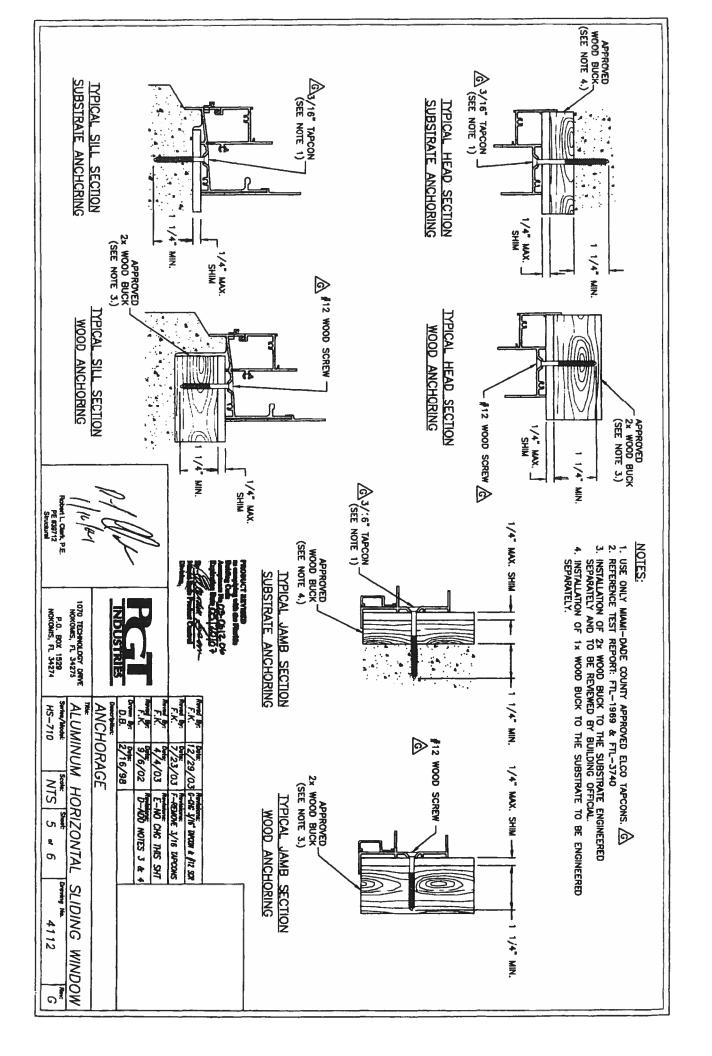


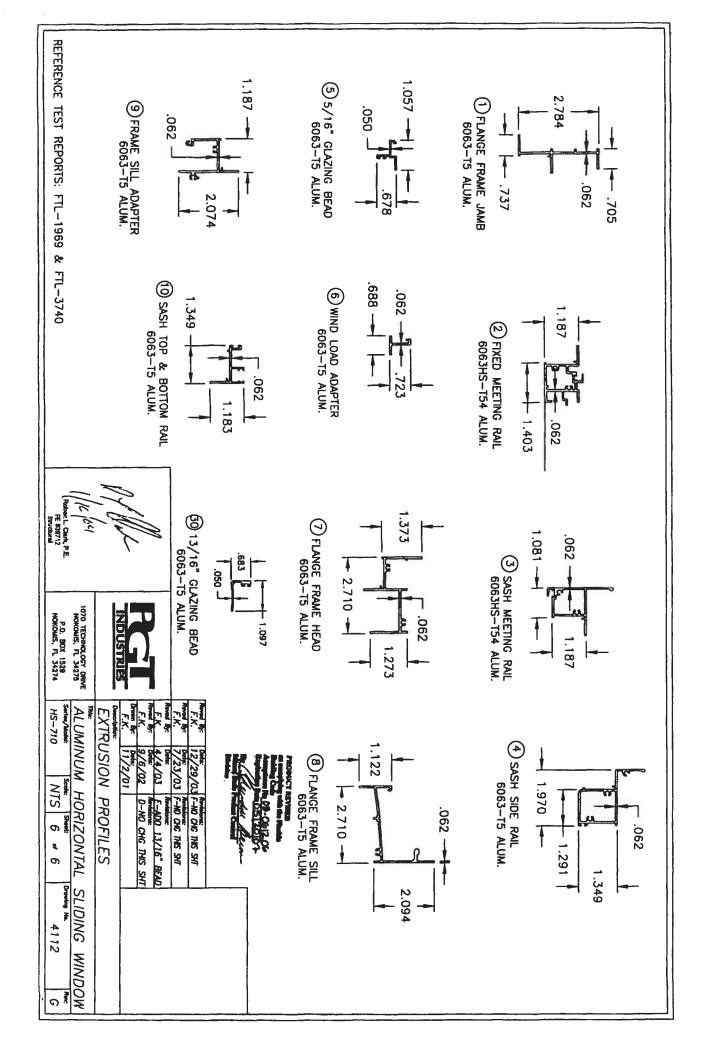


MPARATIVE ANALYSIS TABLE 1. SCHOOL 1/10° ANNICALED, D90 PVB, 1/10° ANNICALED). 1916 LAMINATED (1/10° ANNICALED). 280° SPACE. 5/16 LAMI (1/10° HEAT STRENGTHENED). 380° SPACE. 5/17. 43.0 48.0 48.0 48.0 48.0 48.0 48.0 48.0 48	SLIDING WINDOW Process AND	ONTAL or 6	HORIZ	NUM	ALUMI Serten/Audat HS-710	34275 34275 34274	P.O. BOX 1529 NOKOMIS, FL 34271 P.O. BOX 1529 NOKOMIS, FL 34274	z ~9	PE #39712 Shuchinal	Robert C	-	DE COON		2 2	2	1 2	1 2	3,0	1	I I	
D. DOD PUB. 108" ANNIEALED) WINDTHENED, 308" SPACE, 5110 LAMI (108"HEAT STRENGHTENED, .090 PVB, 108"ANNIEALED) WINDOW HEIGHTS ### ANNIEALED WINDOW HEIGHTS ### ANNIEALED WINDOW HEIGHTS ### ANNIEALED ### ANNIEALED ### ANNIEALED ### ANNIEALED WINDOW HEIGHTS ### ANNIEALED ### ANNI	1	MPARATIN	E CO	S TYP	GLAS			T	1/64	177	₹		DADE CO	E MIAMI-I	4 OUTSIE	1300-9	ASIM E	IRFS LIND	E 1300-	C DESIGN	
D. D90 PMB. 1/8" ANNIEALED) NGTHENIED, 3/8" SPACE, 5/10 LAMI (1/8" HEAT STRENIGHTENED, D90 PVB, 1/8" ANNIEALED) WINDOW HEIGHT'S 1.75.0 68.7 -75.0 68.7				2/16/98			NDUST		3		 	SS TABLE	AND GLA	RESSURE	TEST P	N WATER	BASED O	N LOADS		B. POSITIV	
CLASS TYPES A & C		SED TABLES	D-REV	9/6/02	7		7		3	_		TABLES	GLASS 1	E MIAMI-L	D PRESS	ON TESTE		-98 (AND	VE DESIG	A. NEGATIV	
D. DOGO PIB. 1/18" ANNEALED) NIGTHENED, 3/8" SPACE, 5/18 LAMI (1/18" HEAT STRENGHTENED, .090 PVB. 1/18" ANNEALED) WINDOW HEIGHTS ### 17.75.0 86.7 -75.		342 C 31810 L	7-COMM	7/23/0	1			-			\neg									NOTES:	
D.,090 PVB, 1/8" ANNEALED) NIGTHENED, 3/8" SPACE, 5/18 LAMI (1/8" HEAT STRENGHTENED, .090 PVB, 1/8" ANNEALED) WINDOW HEIGHTS 44.000 50.625 59.000 60.70 75.0 66.7		TE TABLES	J F-UPD	12/29/0	1 1	-															
D.,000 PVB, 1/8" ANNEALED) D.,000 PVB, 1/8" ANNEALED) NISTHENED, 3/8" SPACE, 5/16 LAMI (1/8" HEAT STRENGHTENED, .090 PVB, 1/8" ANNEALED) NINDOW HEIGHTS 15.0 66.7 -75.0 66.		-69.6	H	-70.9	66.7	-72.2	66.7	-73.5	H	-74.8	Н	-75.0	66.7	-75.0	Н	-75.0	-	-75.0	66.7	74.000	
CLASS TYPES A & C		-72.3	-	-73.5	66.7	-74.9	66.7	-75.0		-75.0	H	-75.0	66.7	-75.0	Н	-75.0	Н	-75.0	66.7	70.000	
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PREIS, Mike

From: Steve Clements [steve.clements@virginamerica.com]

Sent: Monday, February 20, 2006 11:17

To: PREIS, Mike; BUSSIERE, Alain; VAUDON, Jean-paul; GEBBIA, Giovanna; RUNDLETT, Miriam;

WALBY, Thomas; RIVENBARK, William

Cc: Joe Houghton; Bob Weatherly; michael.a.barnett@faa.gov

Good Morning All from San Francisco,

Although we are not in the office today I wanted to bring you up to date on our schedule of events. The MFTD installation is on schedule and we will have an update this week if any changes. The remainder of the schedule remains unchanged and we are awaiting the detailed schedule for the simulators. Our POI will adjust his schedule as necessary. I have copied him on this communiqué.

On other issues, Bob, Joe and I met last week prior to their departure and are requesting the following:

- Mike Preis and Jeff Prine have been instrumental in assisting with the development of our program. We
 would like to keep that relationship in place and on an unofficial basis want to designate these two
 individuals as our liasons.
- 2. We would like to have Mike oversee the simulator events in Miami and Jeff Prine oversee the MFTD/Ground Training events in SFO on an extended rotation basis if possible.
 - a. As this is our program, being instructed by airbus persons, we have complete flexibility on the ground curriculum and we believe Jeff is the perfect candidate to assist with the transition and instruction in the MFTD. Although we understand Jeff is not a Pilot, his professionalism, knowledge of the aircraft, and the manual system have earned him the respect of the Virgin America team and the FAA. These areas are essential to the success of our program. Virgin America will train Jeff in the areas necessary to comply with our program per our FAA approved program and FOTM.
- 3. We need the list of items from our teleconference last week so I can complete our bridge training curriculum.

I look forward to seeing you in early April.

Kindest Regards

Steve Clements Director of Training

virgin america 🌃

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