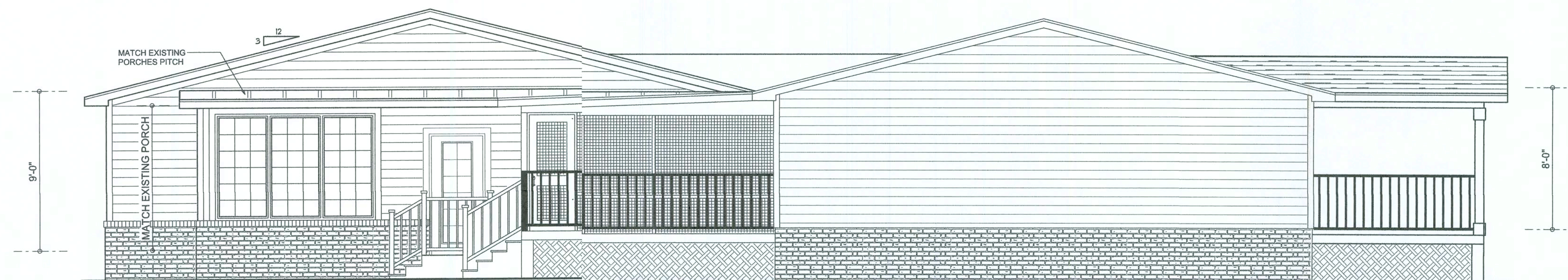


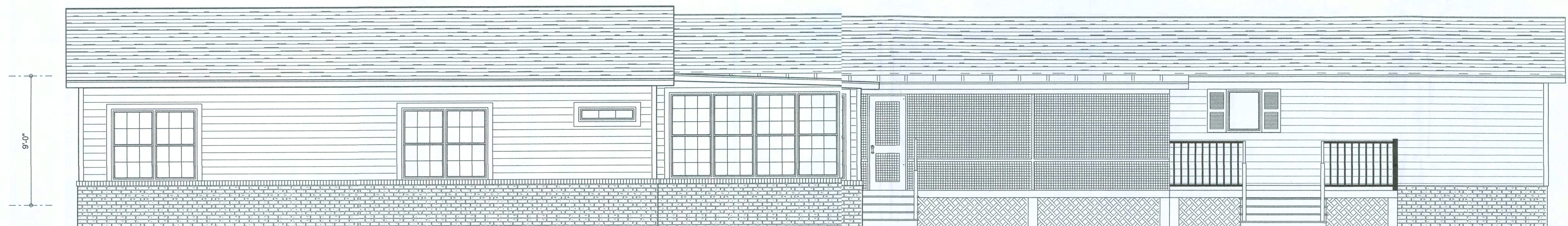
TYPICAL WALL SECTION
SCALE: 1" = 1'-0"



LEFT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/4" = 1'-0"



REAR ELEVATION
SCALE: 1/4" = 1'-0"



FRONT ELEVATION
SCALE: 1/4" = 1'-0"

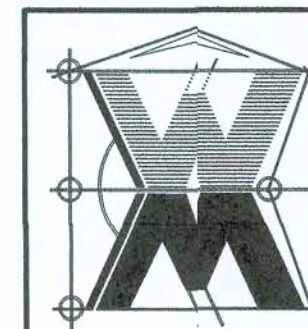
REVISIONS	DATE
1	September 04, 2008

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

EXTERIOR ELEVATIONS
SCALE: 1/4" = 1'-0"

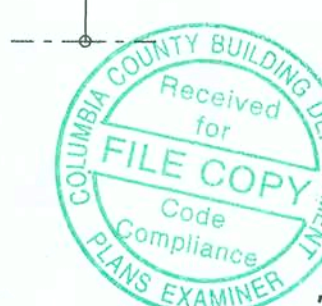
PROPOSED RENOVATIONS & ADDITIONS FOR:
SHARPE & BURKI
PROJECT ADDRESS: COLUMBIA COUNTY, FL

WILLIAMS DESIGN
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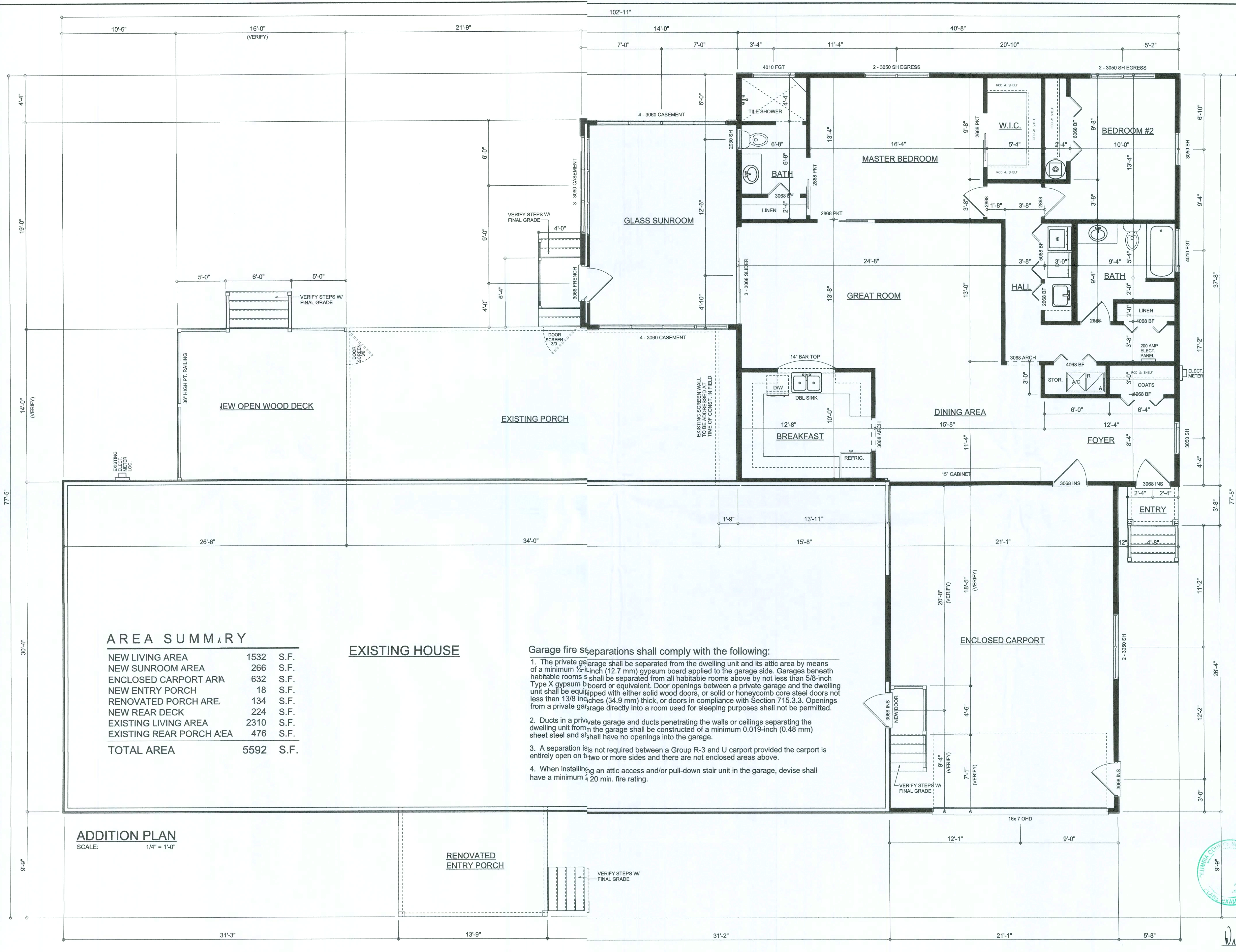


JOB NUMBER
080901

SHEET NUMBER
A1
OF 3 SHEETS



Wm C. Myers



AREA SUMMARY

NEW LIVING AREA	1532	S.F.
NEW SUNROOM AREA	266	S.F.
ENCLOSED CARPORT AREA	632	S.F.
NEW ENTRY PORCH	18	S.F.
RENOVATED PORCH AREA	134	S.F.
NEW REAR DECK	224	S.F.
EXISTING LIVING AREA	2310	S.F.
EXISTING REAR PORCH AREA	476	S.F.
TOTAL AREA	5592	S.F.

EXISTING HOUSE

Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 1 3/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
4. When installing an attic access and/or pull-down stair unit in the garage, device shall have a minimum 420 min. fire rating.

ADDITION PLAN

SCALE: 1/4" = 1'-0"

REVISIONS
September 23, 2008

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

ADDITION PLAN
SCALE: 1/4" = 1'-0"

PROPOSED RENOVATIONS & ADDITIONS FOR:
SHARPE & BURKI
PROJECT ADDRESS: COLUMBIA COUNTY, FL

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JOB NUMBER
080901

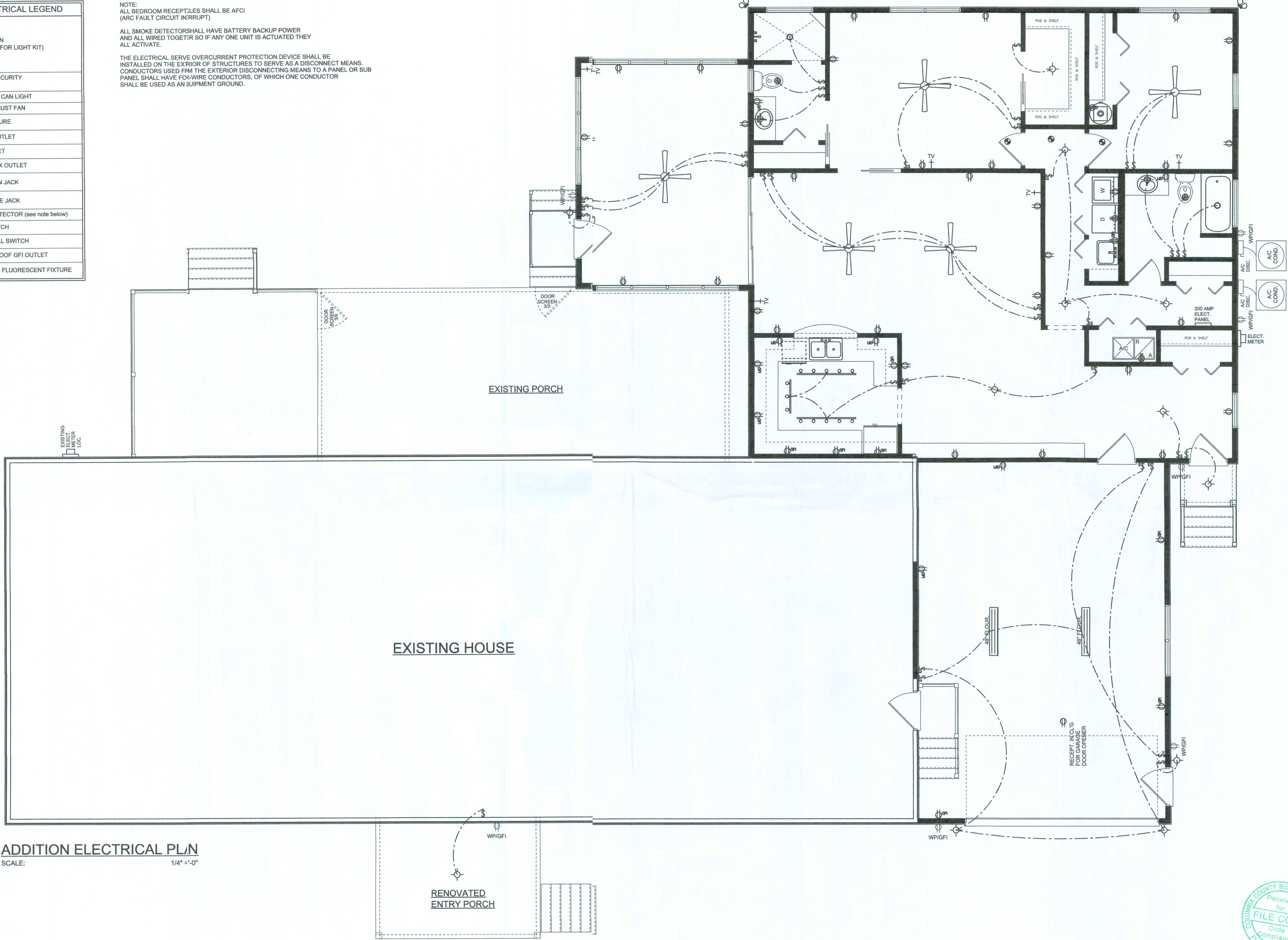
SHEET NUMBER
A.2
OF 3 SHEETS

ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET
	220v OUTLET
	GFI DUPLEX OUTLET
	TELEVISION JACK
	TELEPHONE JACK
	SMOKE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	48" FLUOR. 2 OR 4 TUB FLUORESCENT FIXTURE

NOTE:
ALL BEDROOM RECEPTLES SHALL BE AFCI
(ARC FAULT CIRCUIT INTERRUPT)

ALL SMOKE DETECTOR SHALL HAVE BATTERY BACKUP POWER
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY
ALL ACTIVATE.

THE ELECTRICAL SERVE OVERCURRENT PROTECTION DEVICE SHALL BE
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB
PANEL SHALL HAVE FOR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR
SHALL BE USED AS AN EQUIPMENT GROUND.



ADDITION ELECTRICAL PLAN
SCALE: 1/4" = 1'-0"



Wm C. My

REVISIONS
September 04, 2008

SOFTPLAN
ARCHITECTURAL DRAFTING SOFTWARE

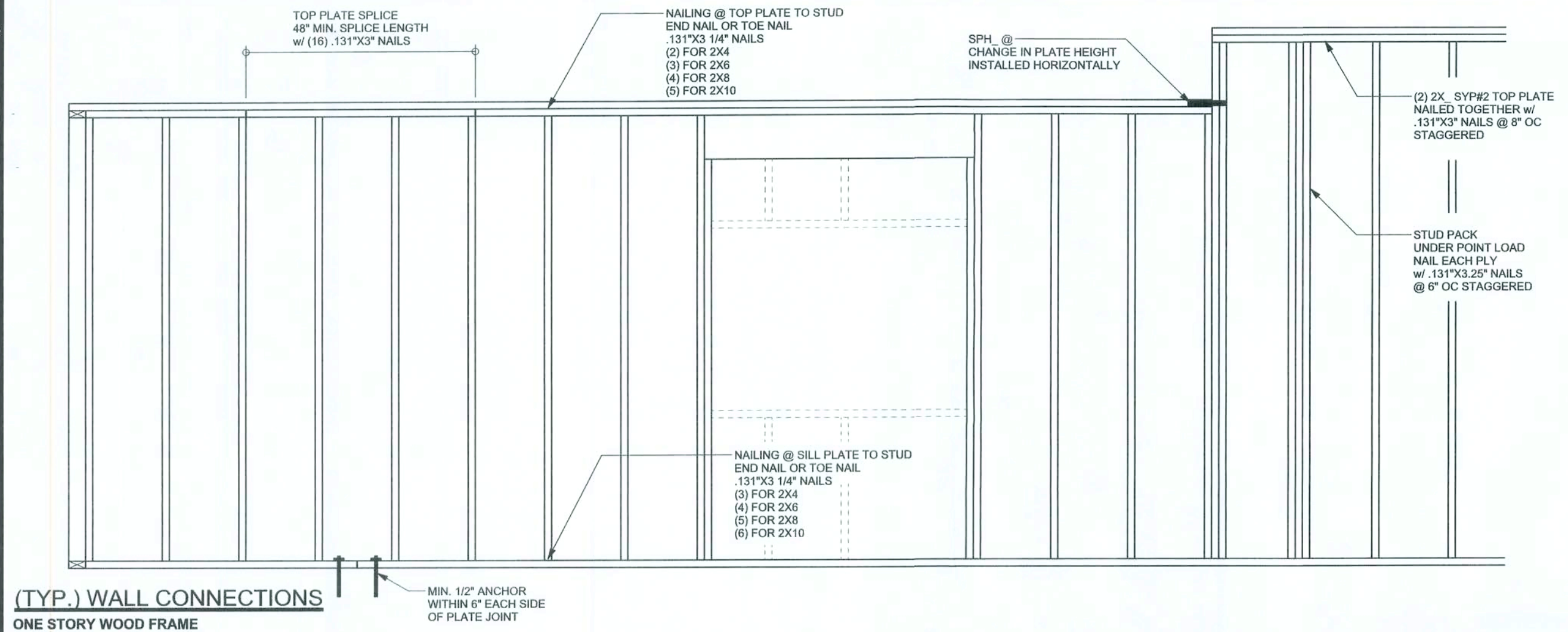
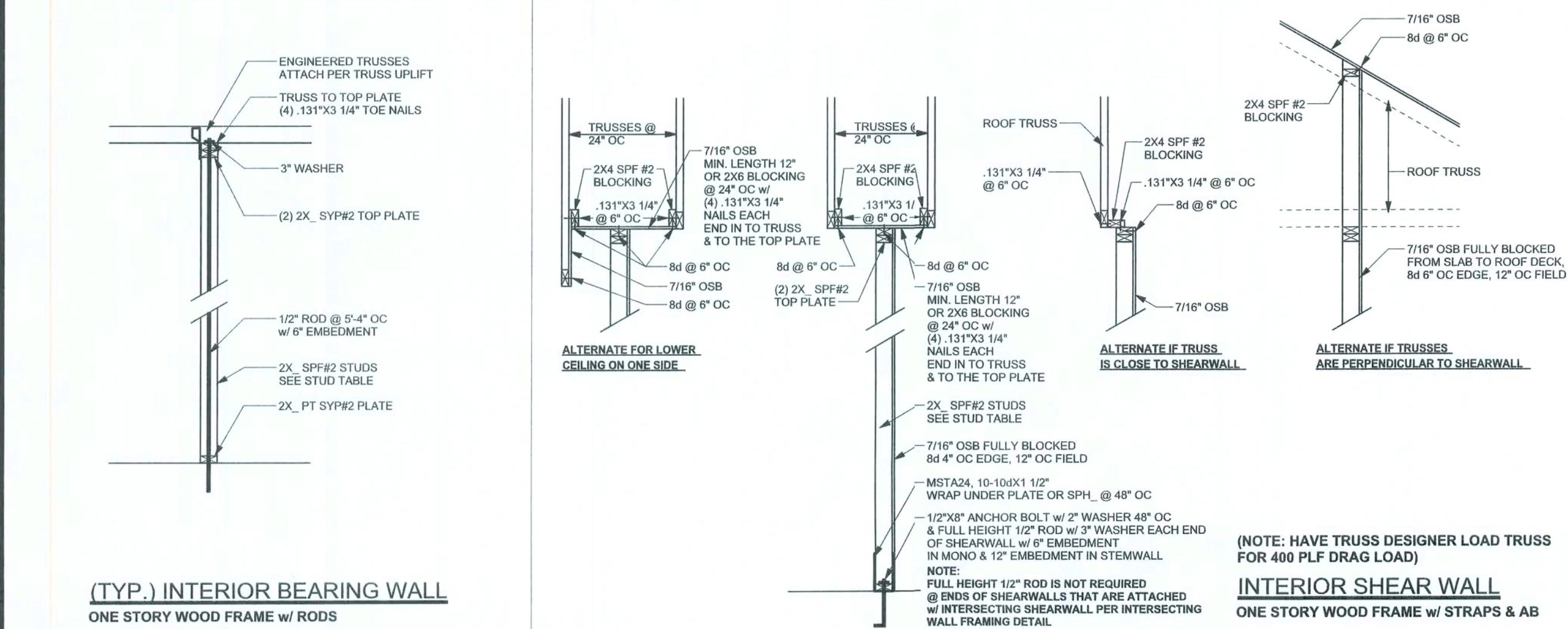
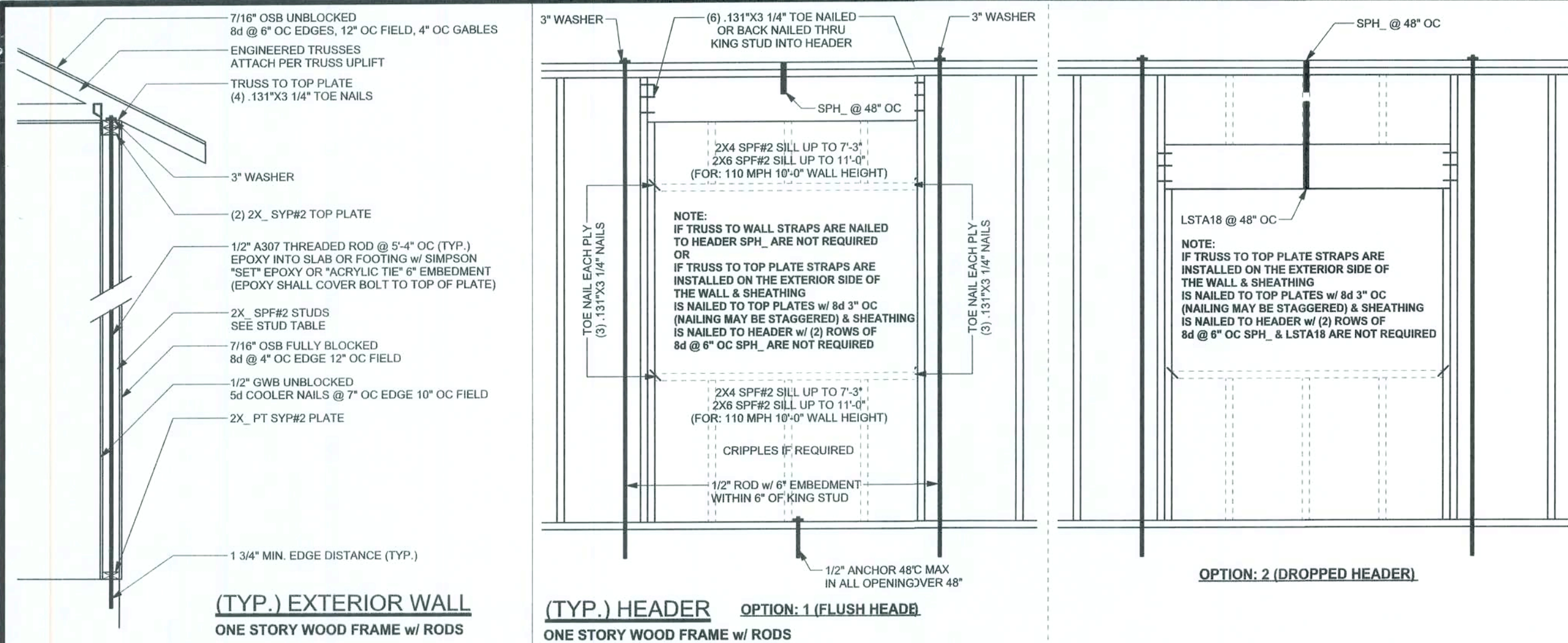
ADDITION ELECTRICAL PLAN
1/4" = 1'-0"

PROPOSED RENOVATIONS & ADDITIONS FOR:
SHARPE & BURKI
PROJECT ADDRESS: COLUMBIA COUNTY, FL

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JOB NUMBER
080901

SHEET NUMBER
A.3
OF 3 SHEETS



ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

TRUSS CONNECTOR	UPLIFT SYP	UPLIFT SP#	F1 SYP	F2 SYP	F1 SP#	F2 SP#	TO RAFTER/TRUSS	TO PLATES
H5	455	265	115	200	100	170	4-8d x 1 1/2"	4-8d x 1 1/2"
H3	415	290	125	180	105	140	4-8d x 1 1/2"	4-8d x 1 1/2"
H2.5	415	365	150	150	130	130	5-8d x 1 1/2"	5-8d x 1 1/2"
H2.5A	480	480	110	110	110	110	5-8d x 1 1/2"	5-8d x 1 1/2"
H6	950	820					8-8d	8-8d
H8	745	565					5-10d x 1 1/2"	5-10d x 1 1/2"
H14-1	1465	1050	515	265	480	245	12-8d x 1 1/2"	13-8d
H14-2	1465	1050	515	265	480	245	12-8d x 1 1/2"	15-8d
H10	990	850	585	525	505	450	8-8d x 1 1/2"	8-8d x 1 1/2"
H10-2	760	655	455	395	390	340	6-10d	6-10d
H16	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
H16-2	1470	1265					2-10d x 1 1/2"	10-10d x 1 1/2"
LTS12 - LTS20	1000	620					6-10d x 1 1/2"	6-10d x 1 1/2"
MTS12 - MTS30	1000	860					7-10d x 1 1/2"	7-10d x 1 1/2"
HTS16 - HTS30	1450	1245					12-10d x 1 1/2"	12-10d x 1 1/2"
HEAVY GIRDER TIEDOWNS								TO FOUNDATION
LG12	2050	1785	700	170	700	170	14-16d	14-16d
LG13-SDS2.5	3685	2655	795	410	795	410	12-SDS 1/4" x 3"	26-16dS
LG14-SDS3	4060	3860	2000	675	2000	675	12-SDS 1/4" x 3"	36-16dS
MO1	3965	3330					22-10d	5/8" ANCHOR
HGT-2	10980	6485					16-10d	2-5/8" ANCHOR
HGT-3	10530	9035					16-10d	2-5/8" ANCHOR
HGT-4	9250	9250					16-10d	2-5/8" ANCHOR
STUD STRAP CONNECTOR								TO STUDS
SSP DOUBLE TOP PLATE	435	435					3-10d	4-10d
SSP SINGLE SILL PLATE	455	420					1-10d	4-10d
DSP DOUBLE TOP PLATE	825	825					6-10d	8-10d
DSP SINGLE SILL PLATE	825	600					2-10d	8-10d
SP1	585	535					4-10d	6-10d
SP2	1065	605					4-10d	6-10d
SP4	885	760					6-10d x 1 1/2"	6-10d x 1 1/2"
SPH4	1240	1065					6-10d x 1 1/2"	6-10d x 1 1/2"
SPH6	885	760					6-10d x 1 1/2"	6-10d x 1 1/2"
LSTA18	1235	1110					14-10d	14-10d
LSTA21	1235	1235					16-10d	16-10d
CS20	1030	1030					14-10d	14-10d
CS16	1705	1705					22-10d	22-10d
STUD ANCHORS								TO FOUNDATION
LTT19	1350	1305					8-16d	1/2" ANCHOR
LTT131	2310	2310					18-10d x 1 1/2"	5/8" ANCHOR
HDA2	2775	2570					2-5/8" BOLTS	5/8" ANCHOR
HTT16	4175	3695					18-16d	5/8" ANCHOR
HTT22	5260	5250					32-16d	5/8" ANCHOR
ABU44	2200	2200					12-16d	5/8" ANCHOR
ABU66	2300	2300					12-16d	5/8" ANCHOR
ABU88	2320	2320					18-16d	2-5/8" ANCHOR

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(2) FOR SYP GIRDER & SPF STUDS

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EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-9" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 20'-0" STUD HEIGHT

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B. EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS 110 MPH EXPOSURE B. STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE TRUSS DESIGNER. THE TRUSS DESIGNER SHALL SATISFY ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS STRUCTURE. STRAP STUDS SHALL BE 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $f'_c = 3000$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 89KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIO OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, $f_y = 60$ KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-98, U.N.O.

GULAM BEAMS: GLB, 24F-V3SP, $F_b = 2.4$ ksi, $E = 1800$ ksi. UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY, 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE. NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAD OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS: FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 50FT IN EXP. C AND 10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

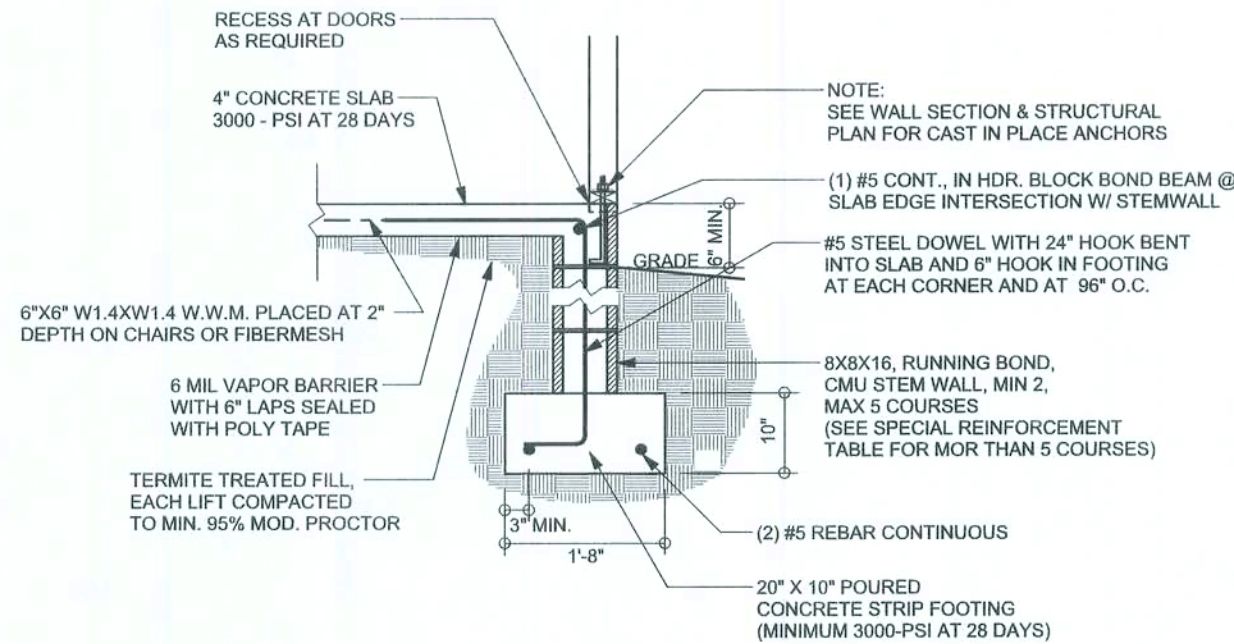
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

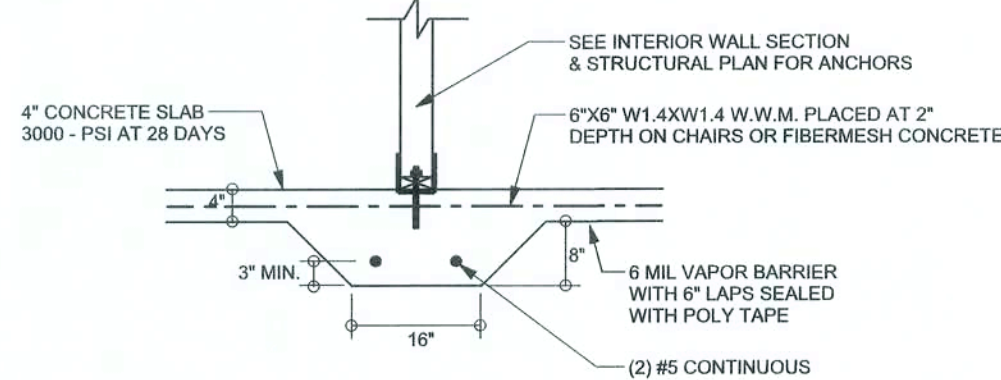
- BASIC WIND SPEED = 110 MPH
- WIND EXPOSURE = B
- WIND IMPORTANCE FACTOR = 1.0
- BUILDING CATEGORY = II
- ROOF ANGLE = 10-45 DEGREES
- MEAN ROOF HEIGHT = <30 FT
- INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
- COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Effective Wind Area (ft ²)			
	10		100	
1	19.9	21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2.0th		-40.6		-40.6
3	19.9	25.5	18.1	-21.8
3.0th		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6

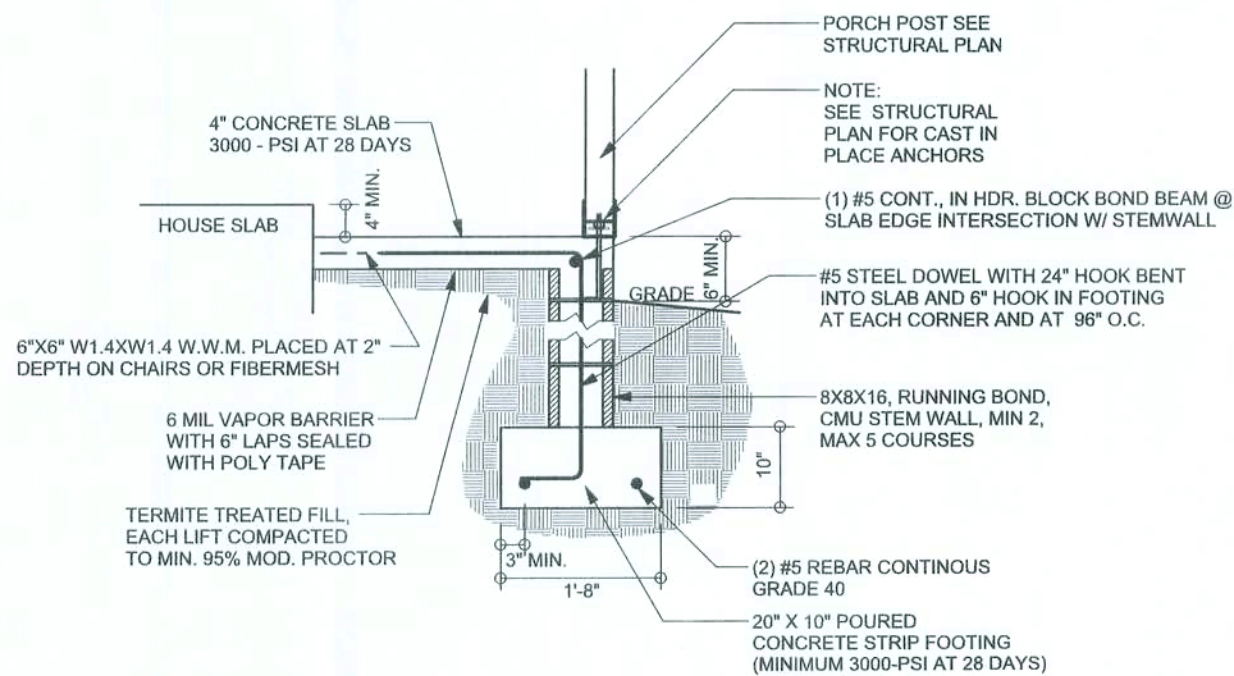
Dorms & Windows	21.8	-29.1
Worst Case (Zone 5, 10 ft RZ)		
6x7 Garage Door	19.5	-22.6
16x7 Garage Door	18.5	-21.0



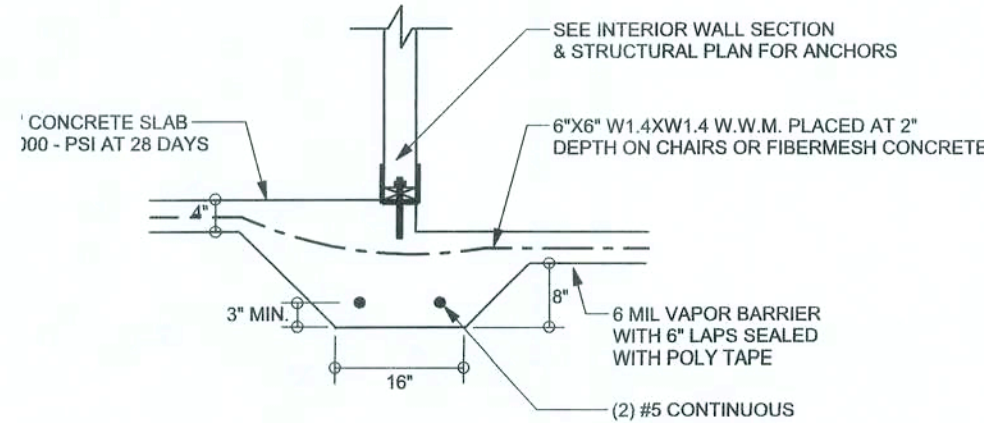
F9 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



F12 S-2 STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"

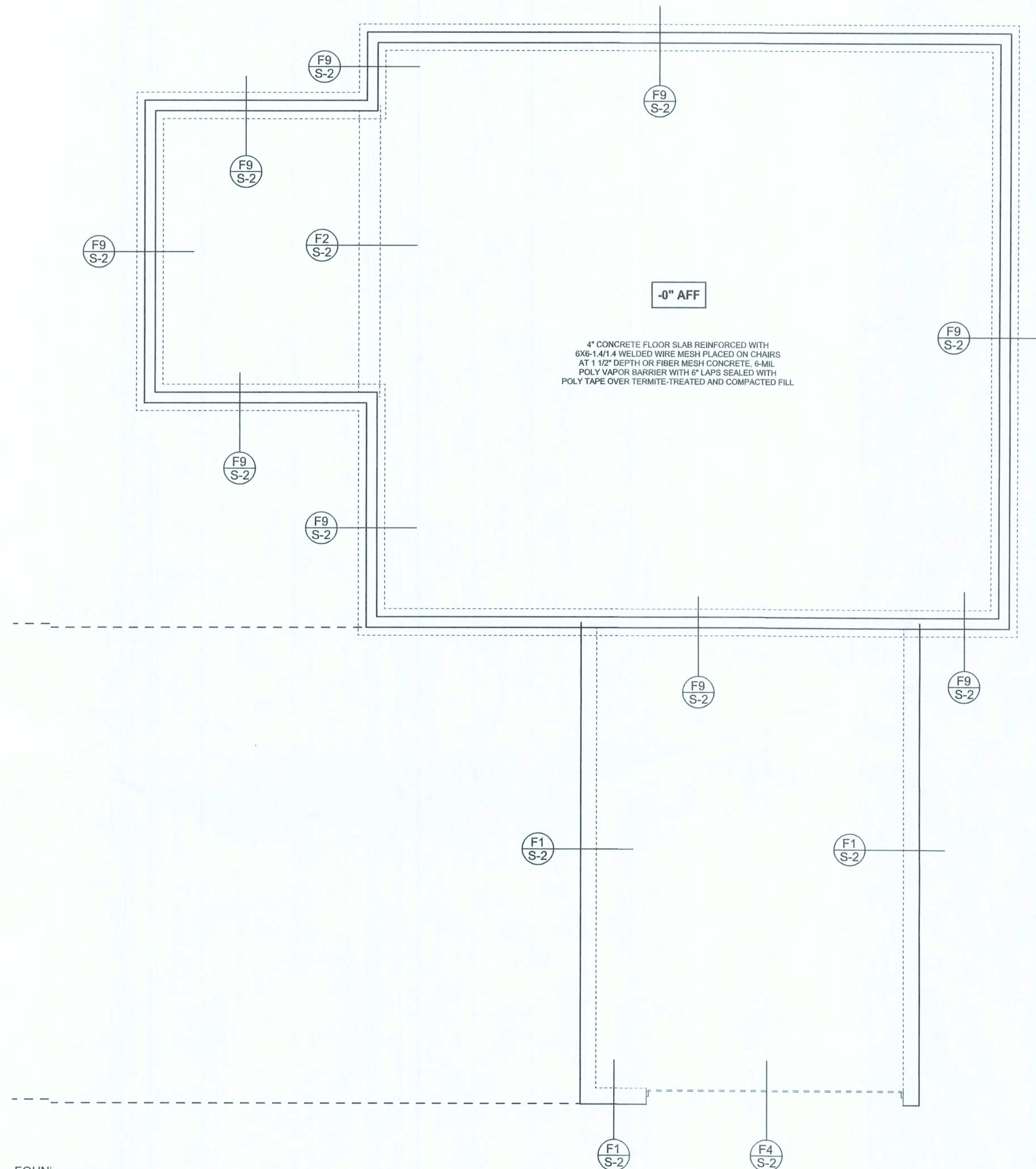


F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

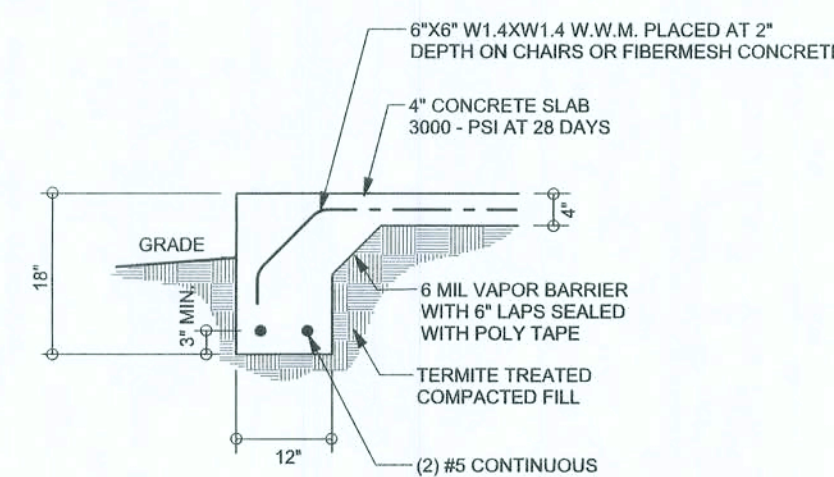
TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 185 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

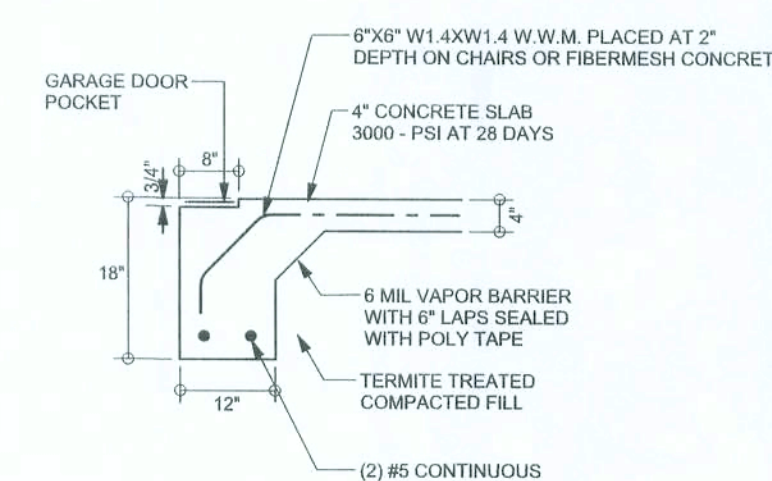
STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



FOUNDATION PLAN
SCALE: 3/32" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS



F1 S-2 (EXISTING VERIFY) MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 (EXISTING VERIFY) GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



REVISIONS	

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER:
Mark Dissway, P.E.
No. 53815, P.O. Box 868/Lake City, FL 32056,
386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Dissway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 9001.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISSWAY
P.E. 53815

Mark Dissway
P.E. 53815
SEAL

Isaac Construction

Sharpe / Birki Addition

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PRINTED DATE:
September 17, 2008

DRAWN BY: STRUCTURAL BY:
David Dissway

FINALS DATE:
17-Sep-08

JOB NUMBER:
809061

DRAWING NUMBER

S-2

OF 35 SHEETS

SOFTPLAN
ARCHITECTURAL DESIGN SOFTWARE



RETROFIT ROOF OVEFRAMING & BRACING DETAIL

VALLEY ROOF PLAN MEMBER LEGEND

TRUSS	
TRUSS UNDER VALLEY FRAMING	
MEMBER	VALLEY RAFTER OR RIDGE
	CRIPPLE
CRIPPLES 4" E.O.C. For 20 psf (TL) and 10 psf (ET) (TY: SHINGLE ROOF) MAX	

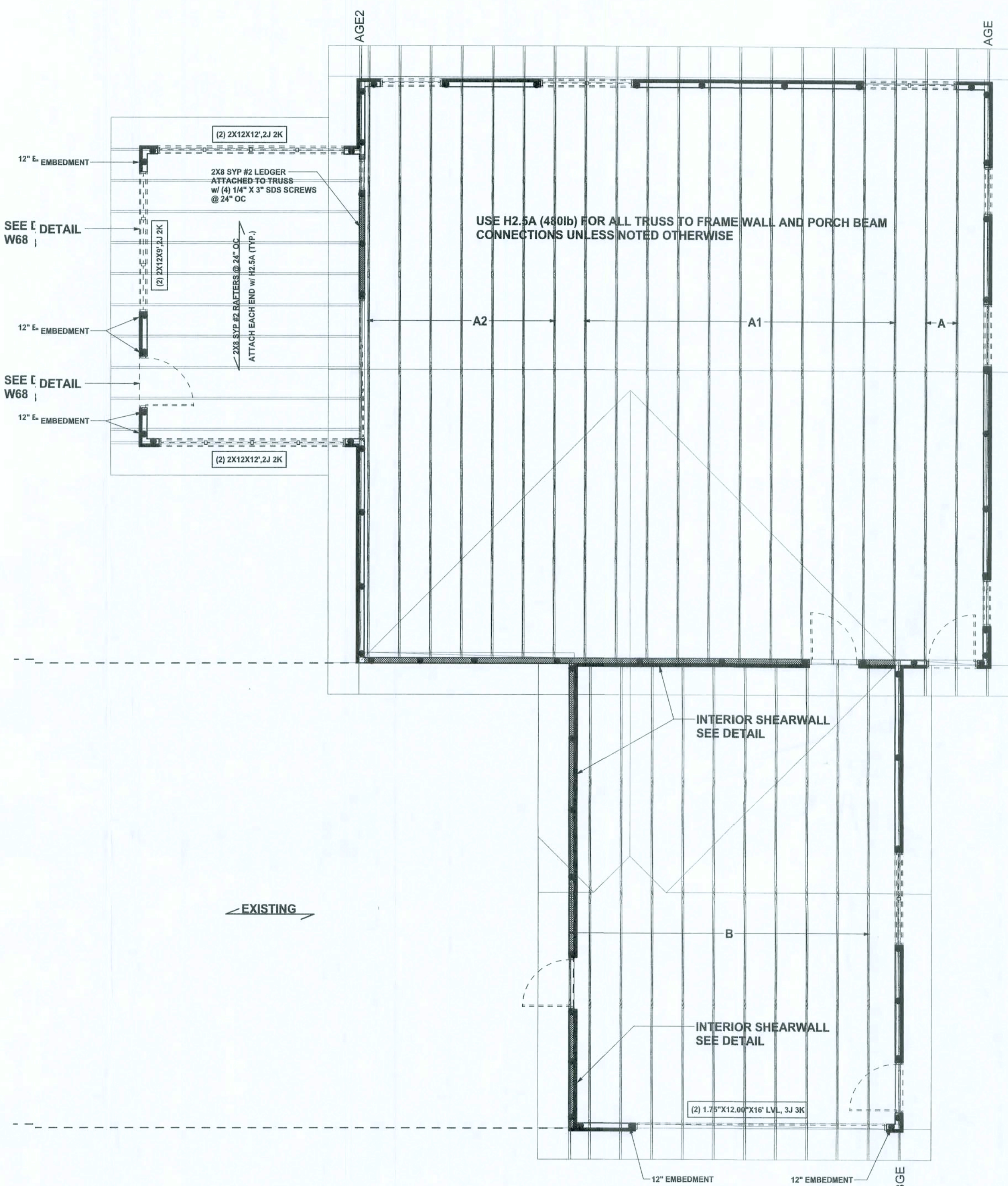
CONNECTION REQUIREMENT NOTES	
1) 2X4 RAFTERS TO RIDGE	3-16d GR-C • 131" x 2" TOE NAILS
2) RIDGE TO RIDGE	2-16d GR-C • 131" x 2" FACE NAILS
3) CRIPPLE TO RAFTER	3-16d GR-C • 131" x 2" FACE NAILS
4) RAFTER TO SLEEPER OR BLOOMING	4-16d GR-C • 121" x 2" TOE NAILS
5) SLEEPER TO TRUSS	4-16d GR-C • 131" x 2" FACE NAILS EACH TRUSS
6) RIDGE BOARD TO ROOF BLOCK	3-16d GR-C • 131" x 2" TOE NAILS
7) RIDGE BOARD TO TRUSS	2-16d GR-C • 131" x 2" FACE NAILS
8) PURLIN TO TRUSS (TYP)	3-16d GR-C • 131" x 2" FACE NAILS
9) PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	3-16d GR-C • 131" x 2" FACE NAILS
10) TRUSS TO BLOOMING	4-16d GR-C • 131" x 2" END NAILS
11) CRIPPLE TO TRUSS	3-16d GR-C • 131" x 2" FACE NAILS
12) CRIPPLE TO PURLIN	3-16d GR-C • 131" x 2" FACE NAILS

GENERAL NOTES

MAXIMUM RAFTER SPANS
MAXIMUM FOR 2X4, 2X6 OR 2X8 SPF #2 OR DYP #2
MAXIMUM ROOF AREA PER PURLIN
1602 IN ZONES 2 & 3, 2082 IN ZONE 1 (EXAMPLE: 40' O.C. X 4' SPAN
1602 IN ZONE 2 OR X 0' SPAN = 1602)
PURLIN BEARING 250' OF 12" LESTING SHEATHING IS REMOVED.
PURLIN SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.
IN CASES WHERE THE MINIMUM TRUSS SPACING IS SHEATHING A MINIMUM OF
6' AND, NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A
MINIMUM OF 1/4" 60 COMMON WIRE NAILS.
THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:
SPANS (DISTANCES BETWEEN HILLS) 40' OR LESS
MAXIMUM VALLEY HEIGHT: 10' OR LESS
MAXIMUM WIND SPEED: 120 MPH
MAXIMUM MEAN ROOF SLOPE: 30 FEET
MAXIMUM TOTAL LOADING: 30 PSF
MEETS TRC 2001/ASCE 7-08 WIND REQUIREMENTS
EXPOSURE CATEGORY "B", 1.5, Kz1 = 1.0
ENCLOSURED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

- 2X4 CONTINUED LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG
- 2X4 MAX 2' - 101 NAILS OR 2X4 T" OR SCAB BRACE NAIL TO FLAT EDGE OF CRIPPLE WITH 8 NAILS @ 8" O.C. "OR SCAB MUST BE 80% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLBS OR BOTH FACETS W/ 8" T" OR SCAB. USE STRESS GRAD LUMBER AND BOX OR 2X4 NAILS
- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD
- INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.
- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FLAT BETWEEN SLEEPERS
- LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED.
- NAIL ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 8.3.2. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



STRUCTURAL PLAN

SCALE: 3/16" = 1'-0"

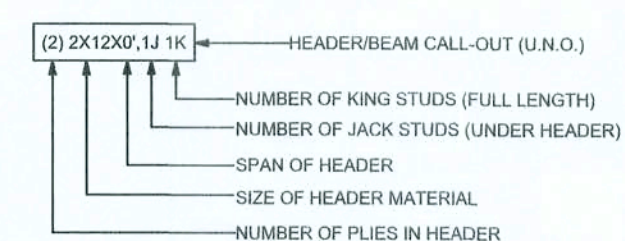
STRUCTURAL PLAN NOTES

- | | | |
|-------|---|---|
| SNN-1 | ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE MINIMUM OF (2) 2X12 SYPR# (U.N.O.) | ● |
| SNN-2 | ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.) | ⊗ |
| SNN-3 | DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT, REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS | |
| SNN-4 | PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING TO BE PROVIDED PER BC501-B, BC501-B1, BC501-B2, BC501-B3, BC501-B4, BC501-B5 & BC501-B6 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE | |





THREADED ROD LEGEND

- INDICATES LOCATION OF:
1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊗ INDICATES LOCATION OF:
2ND FLOOR 1/2" A307 ALL THREADED ROD

HEADER | LEGEND



WALL LEGEND

	EXTERIOR WALL
	INTERIOR NON-LOAD BEARING WALL
	INTERIOR LOAD BEARING WALL w/ NO UPLIFT
	INTERIOR LOAD BEARING WALL w/ UPLIFT

TOTAL SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	31.46'	39.5'
LONGITUDINAL	22.8'	56.5'



CONNECTIONS, WALL, & HEADER DESIGN IS BASED
ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING
FURNISHED BY BUILDER. ANDERSON TRUSS
JOB # 8-220

WINDLOAD ENGINEER:
Mark Disosway, PE
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DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refr all questions to Mark Disosway, ²E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the pin, relating to wind engineering, comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSW
P.F. 53915

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Sharpe /Burki Addition

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PRINTED DATE:
September 17, 2000

DRAWN BY	STRUCTURAL BY
	David Dierksen

FINALS DATE:
17Sep08

JOB NUMBER:
309061

DRAIVING NUMBER

S-3

CF 3 SHEETS