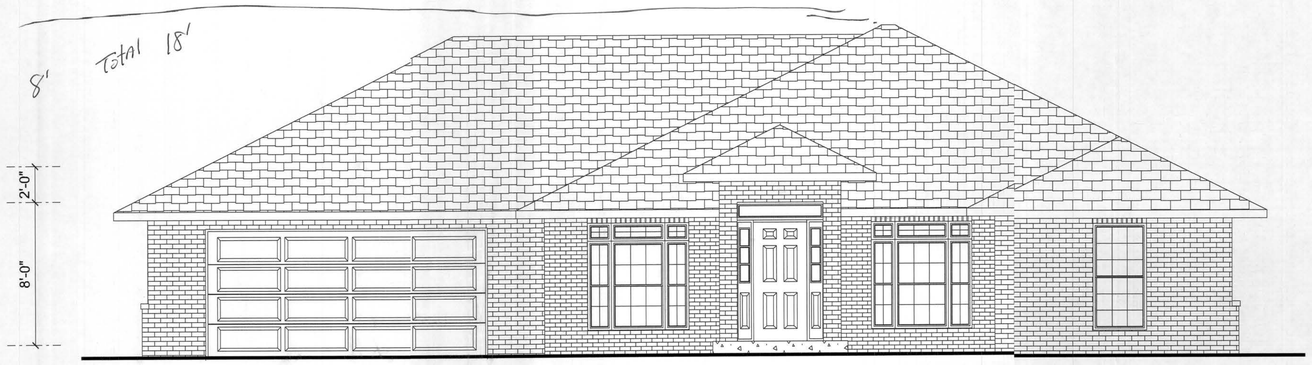
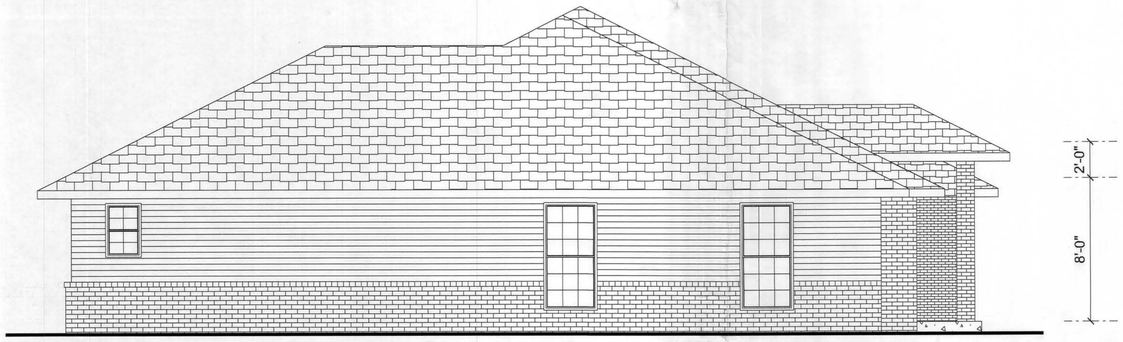
1952

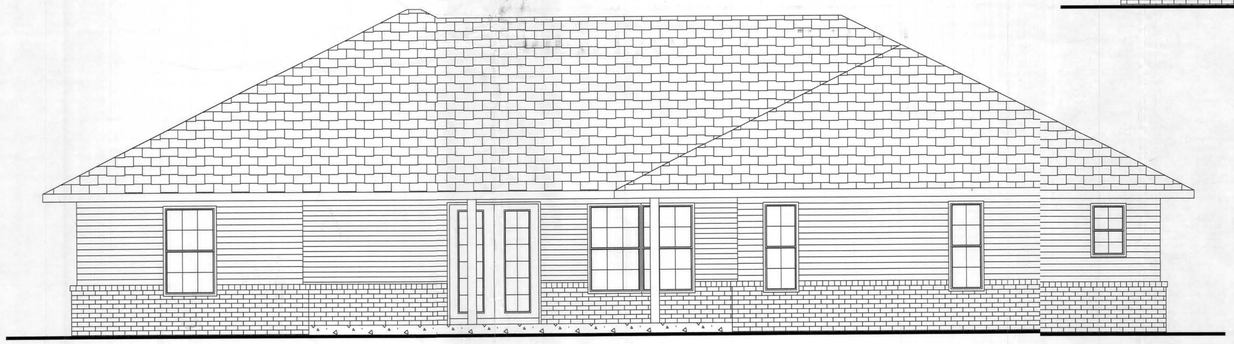
8' Total 18'



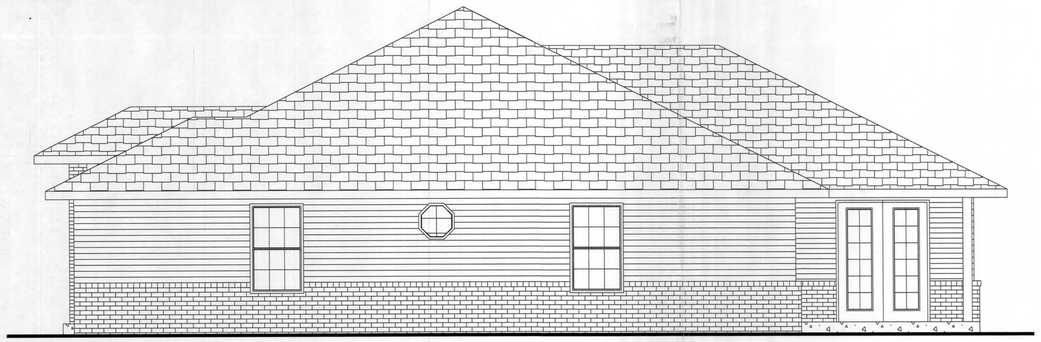
FRONT ELEVATION



LEFT ELEVATION



REAR ELEVATION



RIGHT ELEVATION

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4853 WEST SR. 238
LAKE BUTLER, FL. 32054
(386) 623-1741

COLUMBIA COUNTY

KEEN JOB LOT 59
ROLLING MEADOWS

DATE: 04-28-07

ELEVATIONS

SCALE: 1/4" = 1 FOOT

PAGE
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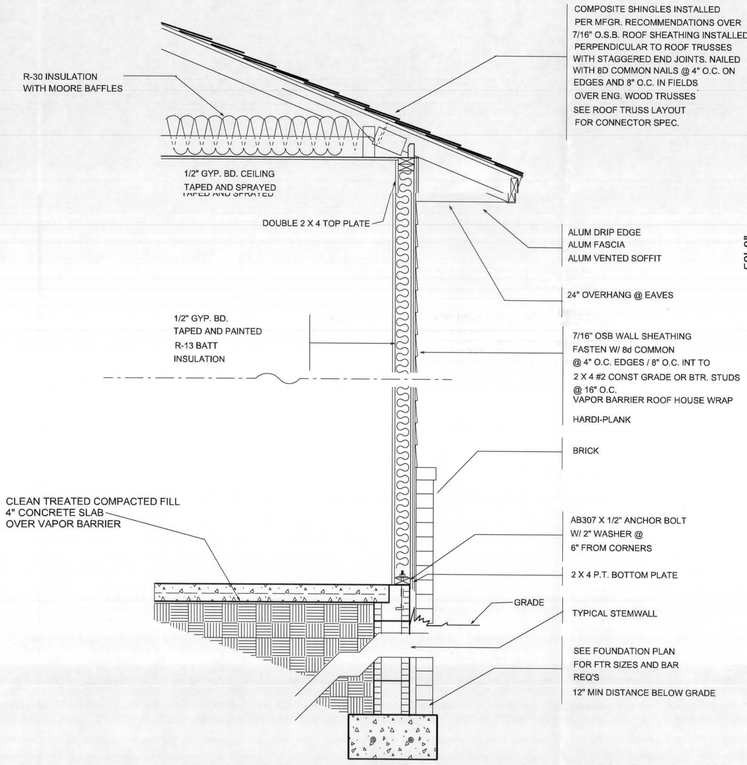
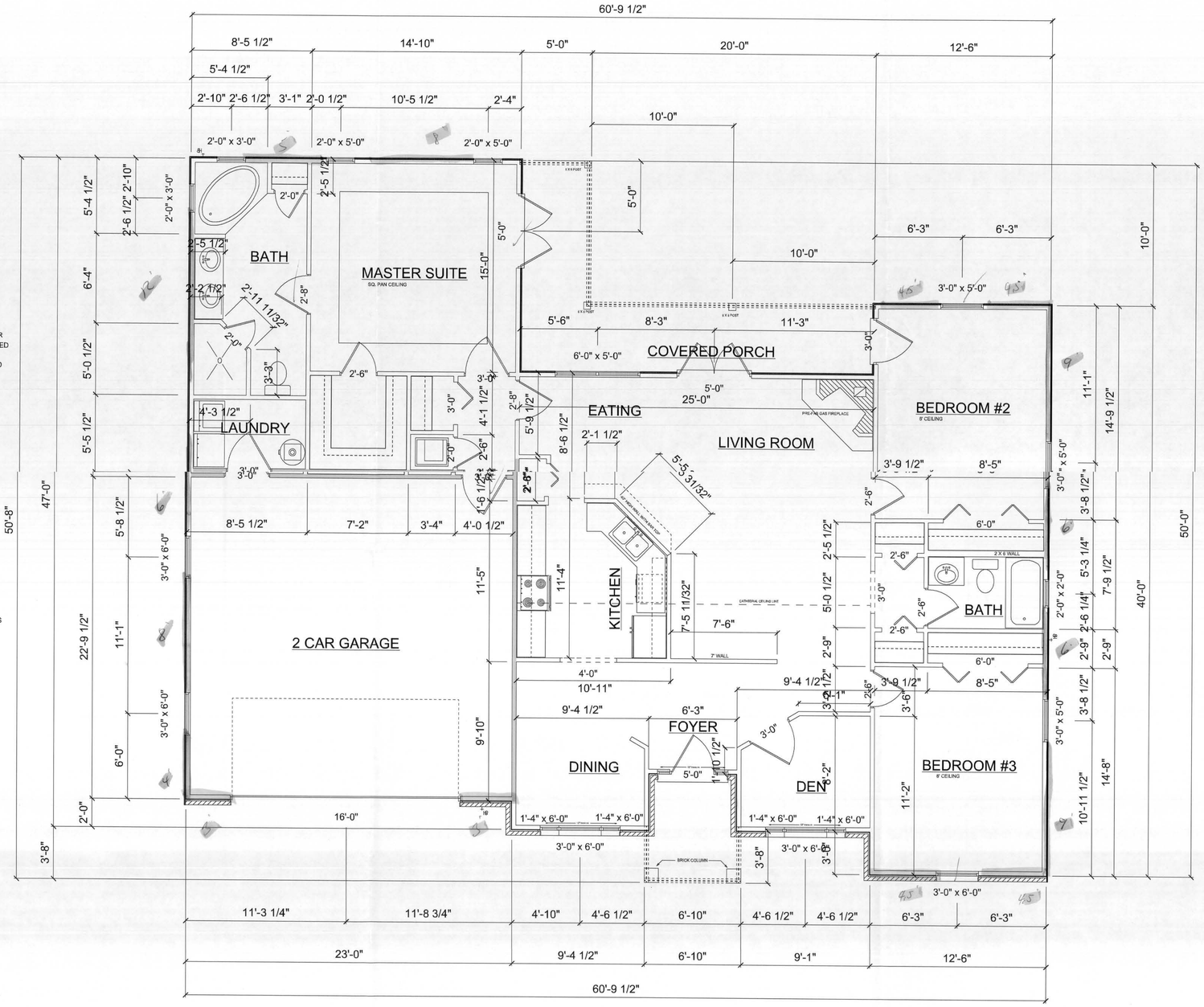
It is important that the Client and Contractor examine the drawings and documentation in detail. It shall be the final responsibility of the Contractor to review and double check the plans for accuracy and compliance with regulatory agencies. It is customary and ordinary not to include details well within the knowledge of licensed Contractor. If necessary, further clarification of these plans should be achieved before signing the construction contract and obtaining a building permit, otherwise the Contractor assumes responsibility for the construction in question. Methods of construction shall be determined by the Contractor.



TYPICAL WALL SECTION



FLOOR PLAN

NAME	AREA
Heated Space	1800 sq ft.
Garage	538 sq ft.
Area 7	226 sq ft.
Total	2564 sq ft.

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KEEN JOB LOT 59
ROLLING MEADOWS

COLUMBIA COUNTY

DATE: 04-28-07

FLOOR PLAN

SCALE: 1/4' = 1 FOOT

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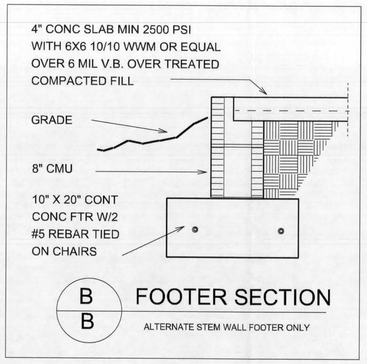
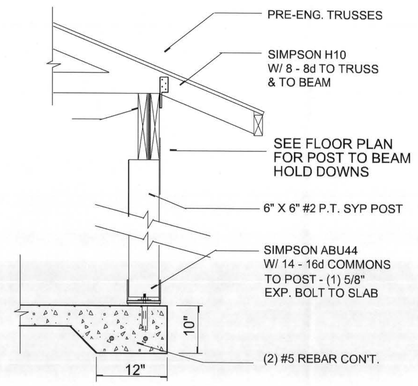


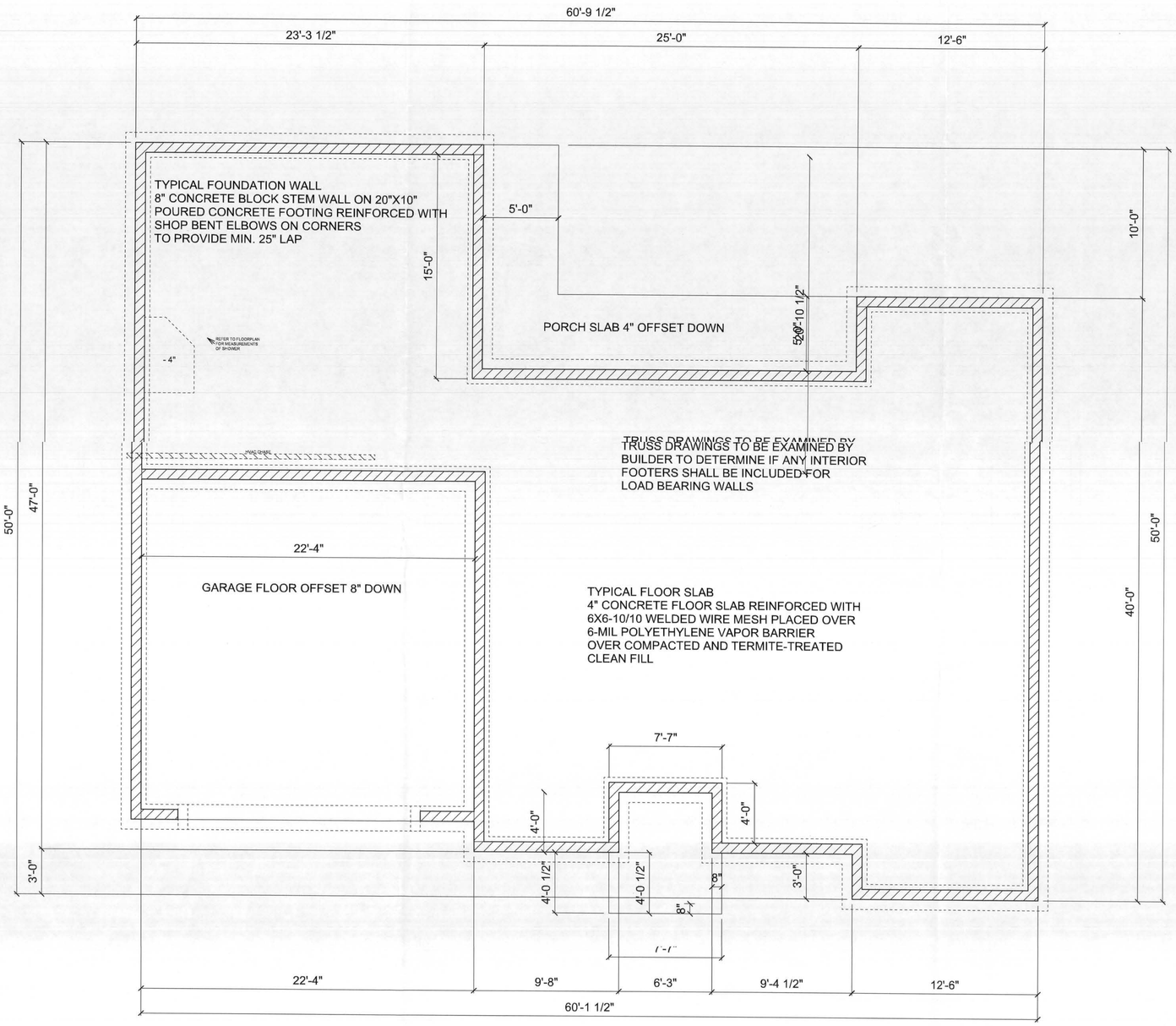
FOUNDATION NOTES

1. Footing trenches and the area within the foundation walls shall be free of all deleterious material.
2. All reinforcing rods shall be Grade 40 (40,000) p.s.i.) #5 deformed bars with a 25" min. lap and placed on support chairs @ 6' o.c.
3. All concrete for slabs, pads, and walkways shall be 2500 p.s.i. minimum at 28 days. Concrete for downpours and bond beams shall be 3000 p.s.i. (pea gravel mix).
4. Provide 6 mil vapor barrier under all concrete slabs with joints lapped 6" and taped.
5. Slab reinforcing shall be 6 X 6 #10/10 welded wire fabric (steel), edge lap 6" and tied.
6. All fill dirt shall be clean sand, free of deleterious material and laid in 12" lifts. Each lift to be wetted and compacted prior to next lift.
7. All fill dirt within the foundation walls shall be termite treated and according to the standards of the National Pest Control Association with a minimum one year warranty.
8. Slope all exterior slabs and garage floor slab for proper drainage.



N.T.S.

POST / BEAM DETAIL



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KEEN JOB LOT 59
ROLLING MEADOWS

(COLUMBIA COUNTY

DATE: 04-28-07

FOUNDATION PLAN

SCALE: 1/4" = 1 FOOT

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ELECTRICAL PLAN NOTES

WIRE ALL APPLIANCES HVAC UNIT AND OTHER EQUIPMENT PER MANUFACTURER SPECS

INSTALLATION SHALL BE PER NATIONAL ELECTRIC CODE

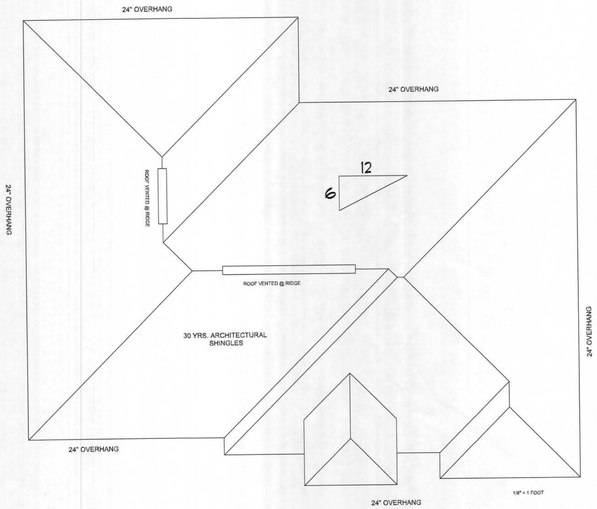
TELEPHONE, T.V. AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNERS DIRECTIONS AND IN ACCORDANCE WITH APPLICABLE SECTIONS OF NEC-LATEST EDITIONS

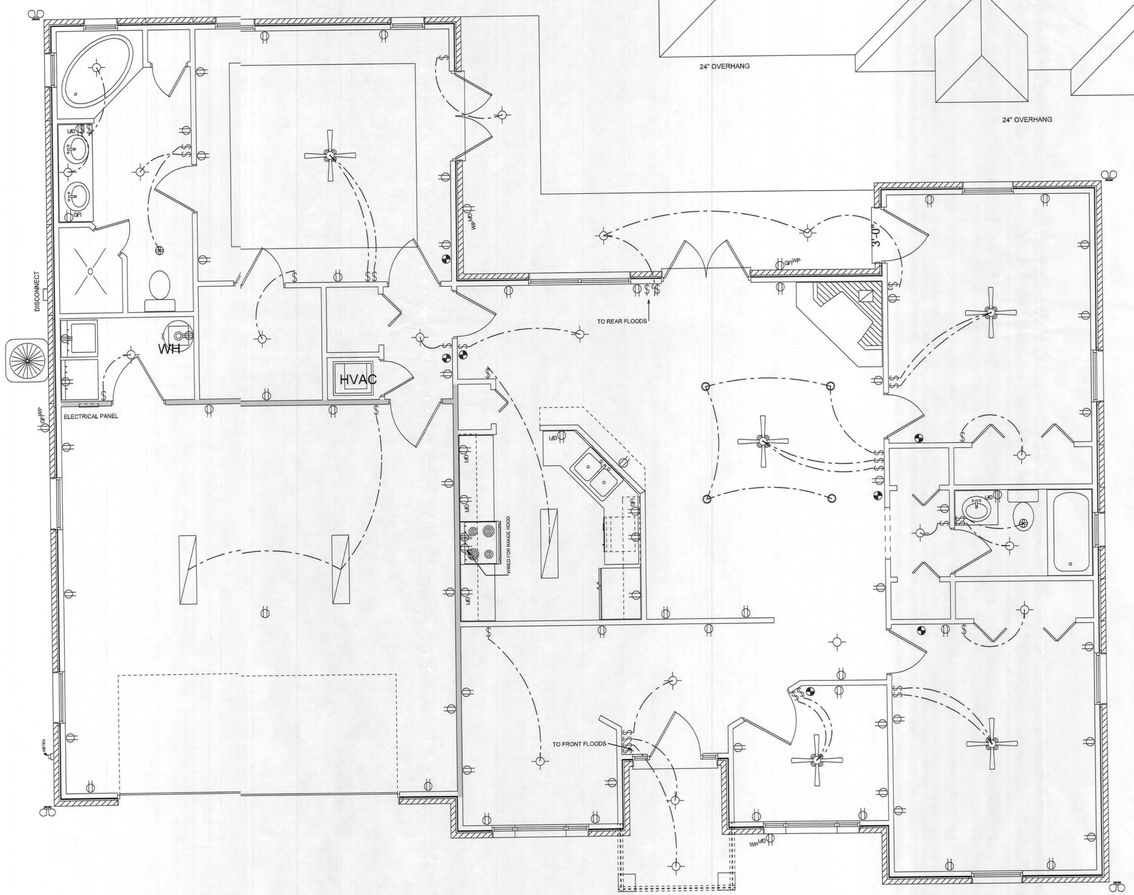
ALL SMOKE DETECTORS SHALL BE 120V WITH BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER

ALL MATERIAL SHALL HAVE A UL LABEL OR LISTING

ALL BEDROOM OUTLETS TO BE ARC-FAULT BREAKERS OR GFI OUTLETS

ELECTRICAL	SYMBOL
ceiling fan spotlights 1	
double spotlight	
fluorescent fixture	
pot light	
electrical panel	
fan	
light	
outlet	
outlet 220v	
outlet gfi	
smoke detector	
switch	
switch 3 way	
weather proof GFI	





ELECTRICAL PLAN

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KEEN JOB LOT 59
ROLLING MEADOWS

ELECTRICAL PLAN

DATE: 04-28-07

SCALE: 1/4" = 1 FOOT

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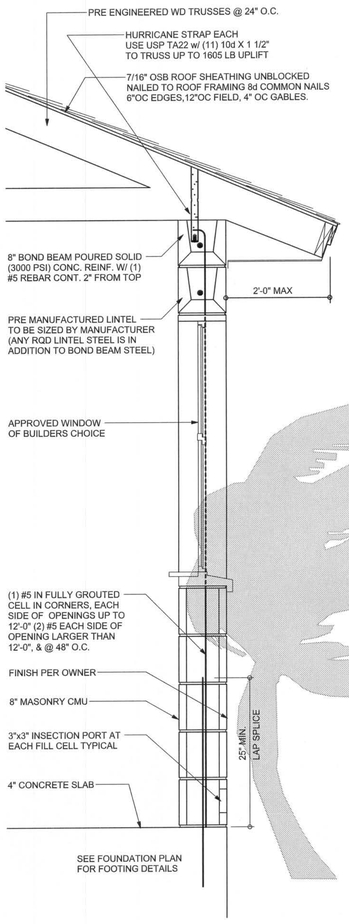
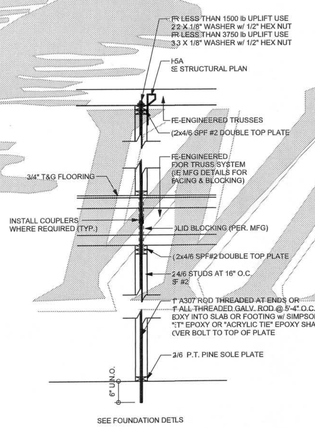


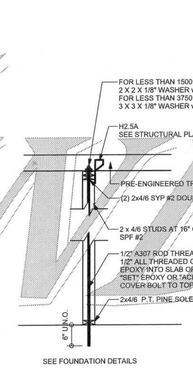
TYPICAL 1 STORY BLOCK WALL SECTION

SCALE: 1" = 1'-0"



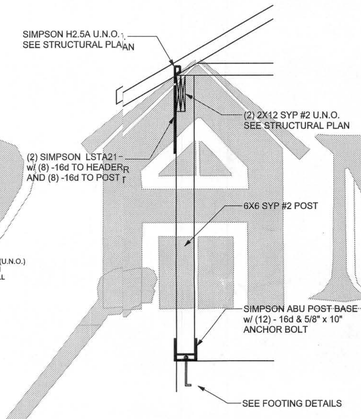
TWO STORY INTERIIR BEARING WALL

SCALE: 1/2" = 1'-0"



INTERIOR BEARING WALL

SCALE: 1/2" = 1'-0"



TYPICAL PORCH POST DETAIL

SCALE: 1/2" = 1'-0"

ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			**TO FOUNDATION**
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			**TO STUDS**
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825	< 825	DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885	< 760	SP6			6-10d, 1 1/2"
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	**TO STUDS**		**TO FOUNDATION**
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB
< 2775	< 2570	HD2A	2-5/8" BOLTS		5/8" AB
< 4175	< 3695	HTT16	18 - 16d		5/8" AB
< 1405	< 1400	PAHD42	16-16d		
< 3335	< 3335	HPAHD22	16-16d		
< 2200	< 2200	ABU44	12-16d		1/2" AB
< 2300	< 2300	ABU66	12-16d		1/2" AB
< 2320	< 2320	ABU88	18 - 16d		2-5/8" AB

MASONRY TRUSS ANCHOR TABLE

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

UPLIFT LBS.	TRUSS CONNECTOR MASONRY *	
< 1205	TA22	10-10d x 1 1/2"
< 1605	TA22	11-10d
< 860	MTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 7 - 10d IN TRUSS
< 1175	HTSM20	4 - 1/4"x2 1/4" TITEN IN BLOCK 10 - 10d IN TRUSS
< 1040	META20	7-10d, 1 1/2"
< 1490	META20	10-10d, 1 1/2"
< 1780	HETA20	7-16d
< 1780	LGT2	7 - 1/4"x2 1/4" TITEN IN BLOCK 14 - 16d SINKER IN GIRDER
< 2130	HHETA20	17-10d, 1 1/2"
< 2310	HHETA24	21-10d, 1 1/2"
< 3965	MGT	22-10d TO TRUSS 5/8 AB TO WALL 15" EMBEDMENT
< 10980	HGT-2	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT
< 10530	HGT-3	16-10d TO TRUSS (2) 3/4 AB TO WALL 15" EMBEDMENT

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131); 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

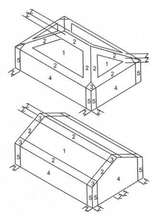
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1
(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)
BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE
BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION
1.) BASIC WIND SPEED = 110 MPH
2.) WIND EXPOSURE = B
3.) WIND IMPORTANCE FACTOR = 1.0
4.) BUILDING CATEGORY = II
5.) ROOF ANGLE = 10-45 DEGREES
6.) MEAN ROOF HEIGHT = <30 FT
7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)
8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

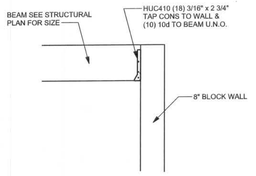


Zone	Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows Worst Case (Zone 5, 10 ft2)	21.8	-29.1		
8x7 Garage Door	19.5	-22.9		
16x7 Garage Door	18.5	-21.0		

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
SOIL BEARING CAPACITY 1000PSF	
NOT IN FLOOD ZONE (BUILDER TO VERIFY)	

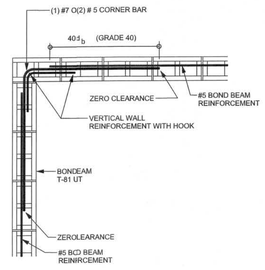
EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 10'-9" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 13'-0" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 18'-10' WALL HEIGHT
(1) 2x6 @ 12" OC	TO 20.0' WALL HEIGHT



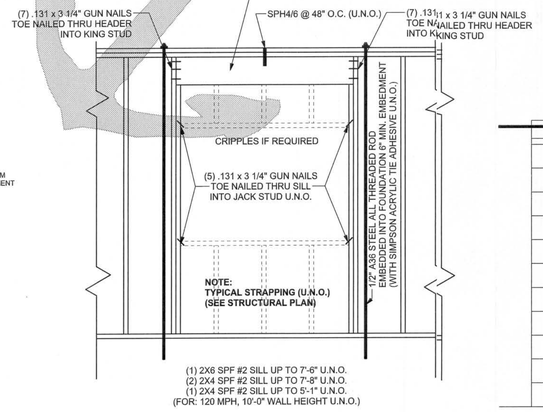
BEAM TO BLOCK CONNECTION DETAIL

SCALE: 1/2" = 1'-0"



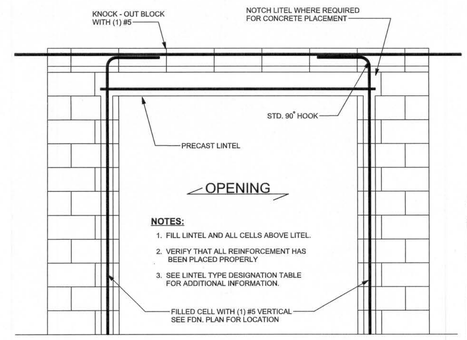
TYPICAL BONDEBEAM CORNER DETAIL

SCALE: 1/2" = 1'=0"



TYPICAL 1 STORY HEADER STRAPING DETAIL

SCALE: 1/2" = 1'-0"



TYPICAL FILLED LINTEL ASSEMBLY

SCALE: 1/2" = 1'=0"

GRADE & SPECIES TABLE

		Fb (psi)	E (10^6 psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2900	2.0
PSL	PARALAM	2900	2.0

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOADENGINEER: Mark Disosway, PE No.5391, POB 888, Lake City, FL 32056, 386-54-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined ths plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at swcified location.

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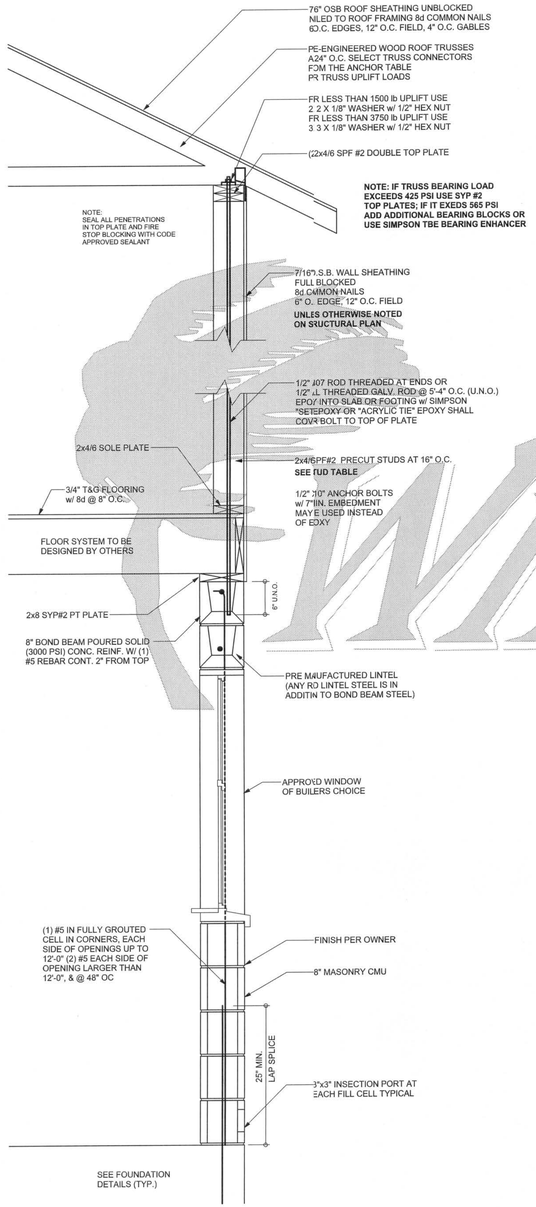
PRINTED DATE:
une 21, 2007

DRAWN BY: David Disosway | CHECKED BY:

FINALS DATE:
28 / Mar / 07

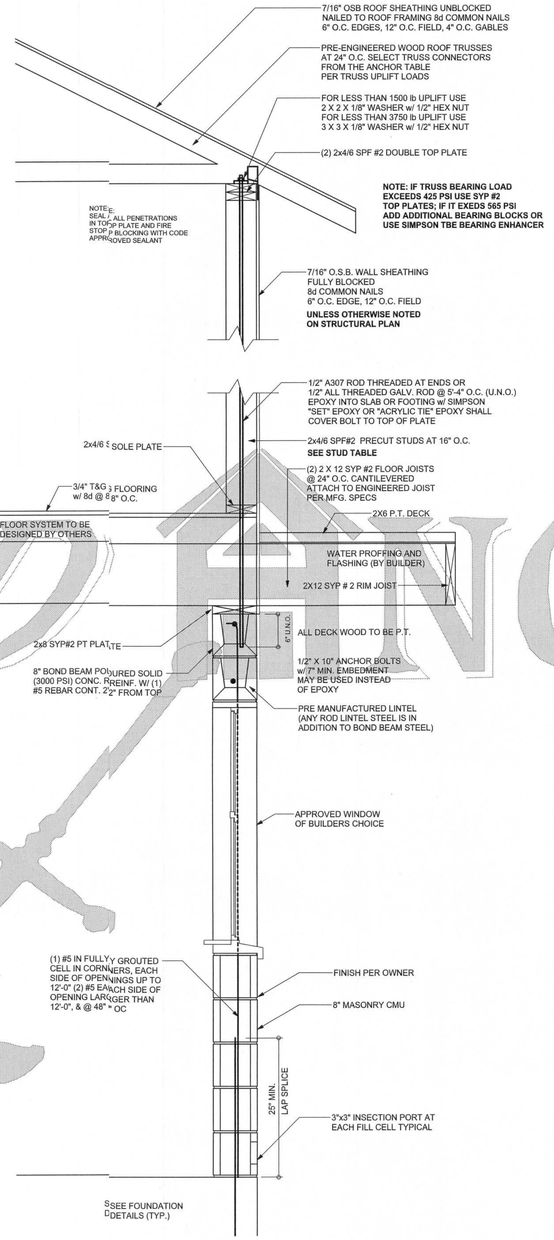
JOB NUMBER:
705154

DRAWING NUMBER
S-1
OF 5 SHEETS



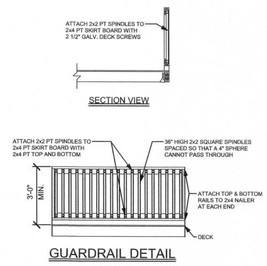
TYPICAL 2 STORY BLOCK / FRAME WALL SETION

SCALE: 1" = 1'-0"



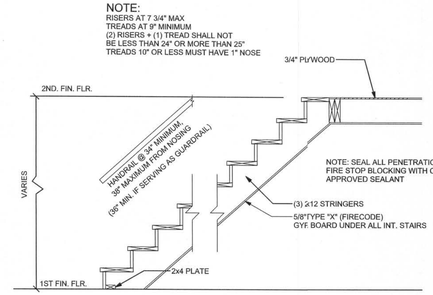
TYPICAL 2 STORY BLOCK / FRAME WALL SECTION @ CANTILEVERED BALCONIES

SCALE: 1" = 1'-0"



GUARDRAIL DETAIL

SCALE: NTS



1403 (3401/01) STAIR DETAIL

SCALE: NTS

REVISIONS	
6/21/2007	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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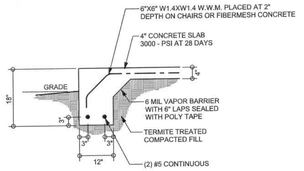
FINALS DATE: 28 / May / 07

JOB NUMBER: 705154

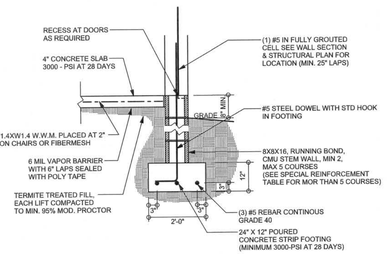
DRAWING NUMBER

S-1.1

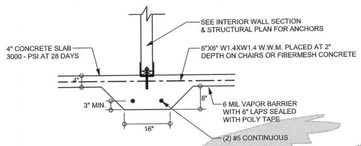
OF 5 SHEETS



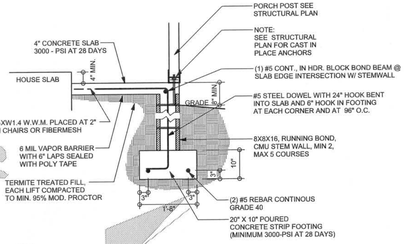
F1 S-2 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



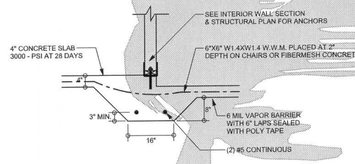
F17 S-2 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



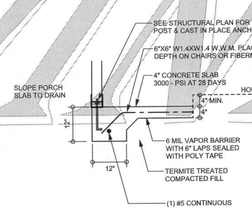
F2 S-2 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



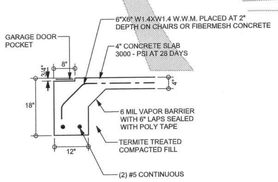
F12 S-2 ALT. STEM WALL PORCH FOOTING
SCALE: 1/2" = 1'-0"



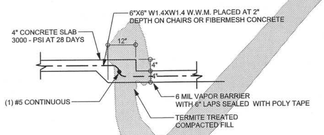
F3 S-2 INTERIOR BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"



F5 S-2 PORCH FOOTING
SCALE: 1/2" = 1'-0"



F4 S-2 GARAGE DOOR FOOTING
SCALE: 1/2" = 1'-0"



F6 S-2 TYPICAL NON - BEARING STEP FOOTING
SCALE: 1/2" = 1'-0"

MASONRY NOTES:

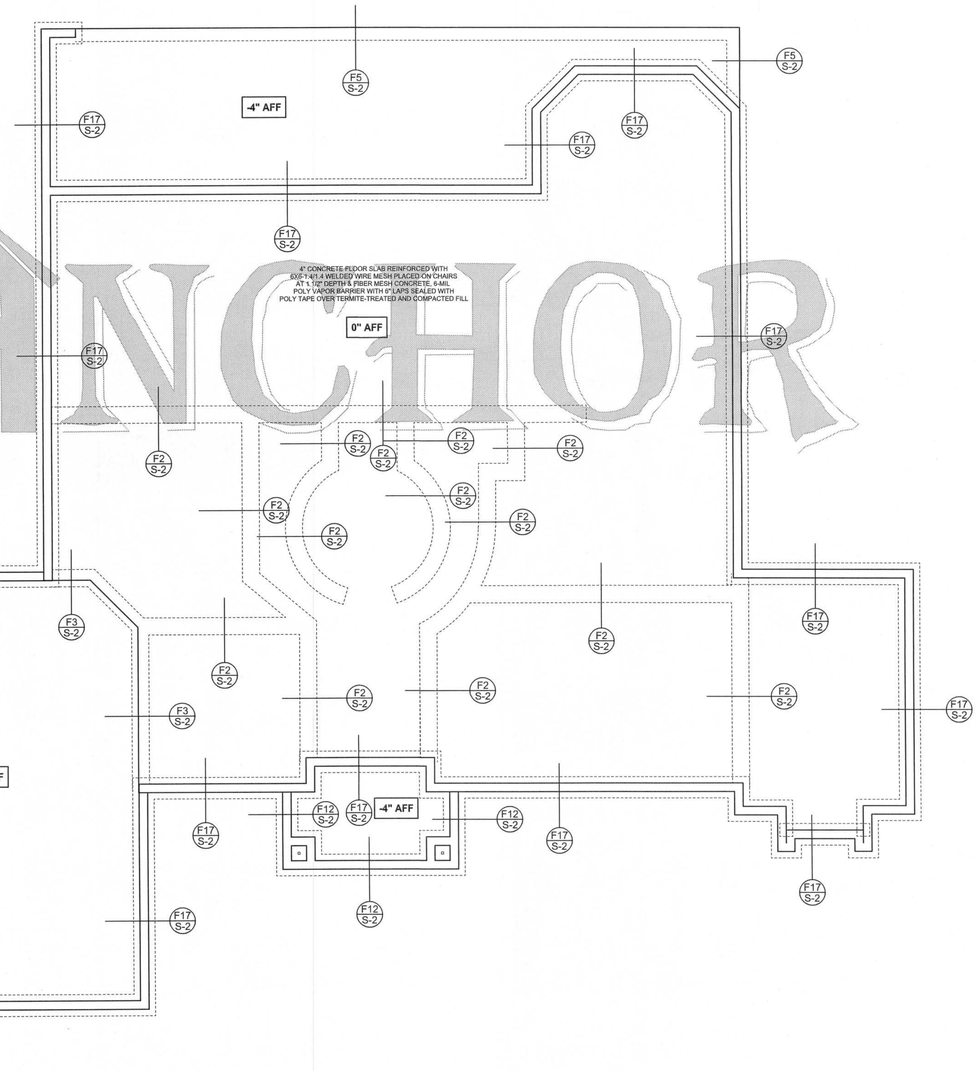
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 60, Fy = 60 ksi, Lap splices min 48 bar dia. (30" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

TALL STEM WALL TABLE

The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"O/C vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



F6 FOUNDATION PLAN
SCALE: 3/16" = 1'-0"
DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, PCB B88, Lake City, FL 32056, 386-754-5419

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DRAWN BY: David Disosway
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FINALS DATE: 28 / May / 07

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DRAWING NUMBER
S-2
OF 5 SHEETS

MATERIALS

1. f'c 8" precast lintel = 3500 psi
2. f'c prestressed lintel = 6000 psi
3. Grout per ASTM C476 f'c = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

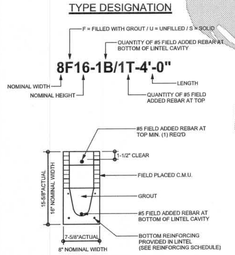
1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accomodate vertical cell reinforcing and grouting.
5. All lintels meet or exceed L/360 deflection, except lintels 17'-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.

9. Safe load rating based on rational design analysis per ACI 318 and ACI 530
10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

$$\frac{\text{Applied vertical load}}{\text{Safe vertical load}} + \frac{\text{Applied horizontal load}}{\text{Safe horizontal load}} \leq 1.0$$

13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right:

TYPE DESIGNATION



SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
   Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support.

7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintels lengths not shown, use safe load from next longest length shown
9. All safe loads in units of pounds per linear foot
10. All safe loads based on simply supported span.
11. The number in the the parenthesis indicates the percent reduction for grade 40 field added rebar.
    Example 7'-6" lintel type 8F32-1B safe gravity load = 6472\H0.0469;(15)\H0.0781; w/ 15% reduction 6472 ⇒ (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH \ TYPE			8U8	8F8-0B / 8F8-1B	8F12B / 8F12B	8F16-0B / 8F16-1B	8F20-0B / 8F20-1B	8F24-0B / 8F24-1B	8F28-0B / 8F28-1B	8F32-0B / 8F32-1B
2'-10"	(34")	PRECAST	2231	3069	46	6113	7547	8974	10394	11809
				3069	46	6113	7547	8974	10394	11809
3'-6"	(42")	PRECAST	2231	3069	37	5163	6607	8054	9502	10951
				3069	46	6113	7547	8974	10394	11809
4'-0"	(48")	PRECAST	1966	2561	27	3820	4890	5961	7034	8107
				2693	46	6113	7547	8974	10394	11809
4'-6"	(54")	PRECAST	1599	1969	21	2931	3753	4576	5400	6224
				2189	43	6113	7547 (7)	8672	10294	11809
5'-4"	(64")	PRECAST	1217	1349	14	1999	2560	3123	3686	4249
				1663	30	5365	7547(36)	7342 (19)	8733 (19)	10127(19)
5'-10"	(70")	PRECAST	1062	1105	11	1631	2090	2549	3009	3470
				1451	26	4360	7168 (45)	6036 (19)	7181 (19)	8328 (20)
6'-6"	(78")	PRECAST	908	1238	21	3480	3031	3707	4383	5061
				1238	21	3480	5381	8360	10394 (17)	8825 (14)
7'-6"	(90")	PRECAST	743	1011	17	2632	2205	2698	3191	3685
				1011	17	2661	3898	5681	8467 (44)	8472 (15)
9'-4"	(112")	PRECAST	554	699	11	1625	2564	3486	2818	3302
				752	12	1843	2564	3486	4705 (37)	6390 (47)
10'-6"	(126")	PRECAST	475	535	8(	1247	2093	2777	2163	2536
				643	10	1533	2093	2781	3643 (38)	4754 (45)
11'-4"	(136")	PRECAST	362	582	9(	1366	1846	2423	3127	4006
				582	9(	1366	1846	2423	3127	4006
12'-0"	(144")	PRECAST	337	540	8(	1254	1684	2193	2805	3552
				540	8(	1254	1684	2193	2805	3552
13'-4"	(160")	PRECAST	296	471	7(	1075	1428	1838	2316	2883
				471	7(	1075	1428	1838	2316	2883
14'-0"	(168")	PRECAST	279	424	7(	1002	1326	1697	2127	2630
				442	7(	1002	1326	1697	2127	2630
14'-8"	(176")	PRESTRESSED	N.R.	NR	N	NR	NR	NR	NR	NR
				458	7(	1370	1902	2245	2517	2712
15'-4"	(184")	PRESTRESSED	N.R.	NR	N	NR	NR	NR	NR	NR
				412	7'	1250	1733	2058	2320	2513
17'-4"	(208")	PRESTRESSED	N.R.	NR	N	NR	NR	NR	NR	NR
				300	5(	950	1326	1609	1849	2047
19'-4"	(232")	PRESTRESSED	N.R.	NR	N	NR	NR	NR	NR	NR
				235	4'	750	1037	1282	1515	1716
21'-4"	(256")	PRESTRESSED	N.R.	NR	N	NR	NR	NR	NR	NR
				180	3'	588	845	1114	1359	1468
22'-0"	(264")	PRESTRESSED	N.R.	NR	N	NR	NR	NR	NR	NR
				165	3	550	784	1047	1285	1399
24'-0"	(288")	PRESTRESSED	N.R.	NR	N	NR	NR	NR	NR	NR
				129	2(	450	654	884	1092	1222

SAFE UPLIFT LOADS FOR 8" PRECAST & RESTRESSED U-LINTELS

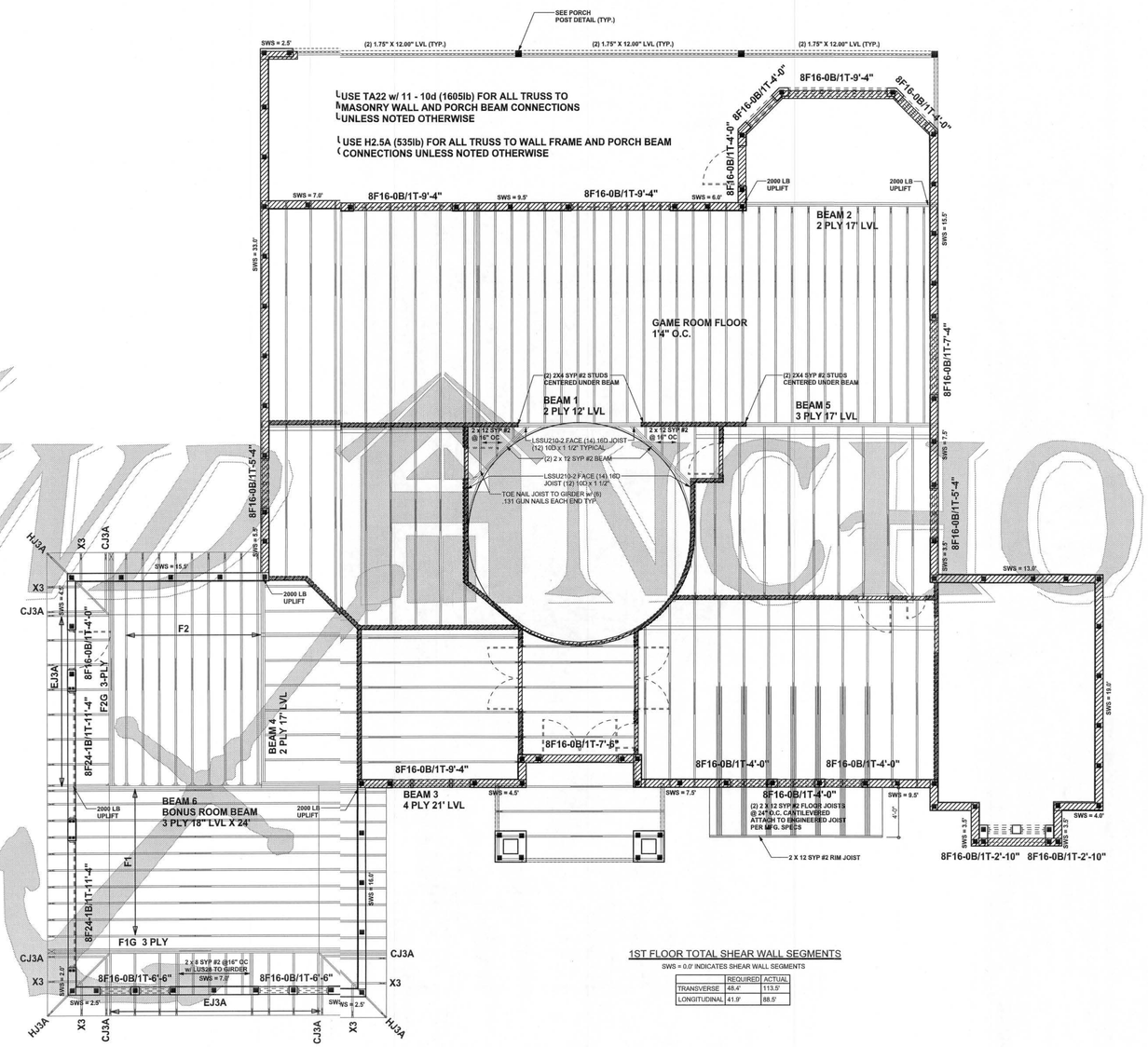
LENGTH \ TYPE			8F8-1T / 8F8-2T	8F12-1T / 8F12-2T	8F1(T / 8F1(T	8F20-1T / 8F20-2T	8F24-1T / 8F24-2T	8F28-1T / 8F28-2T	8F32-1T / 8F32-2T
2'-10"	(34")	PRECAST	1972	3173	44	5747	7034	8321	9608
			1972	3173	44	5747	7034	8321	9608
3'-6"	(42")	PRECAST	1569	2524	35	4569	5591	6613	7636
			1569	2524	35	4569	5591	6613	7636
4'-0"	(48")	PRECAST	1363	2192	30	3966	4853	5740	6627
			1363	2192	30	3966	4853	5740	6627
4'-6"	(54")	PRECAST	1207	1940	27	3508	4292	5077	5861
			1207	1940	27	3508	4292	5077	5861
5'-4"	(64")	PRECAST	1016	1632	22	2949	3607	4265	4924
			1016	1632	22	2949	3607	4265	4924
5'-10"	(70")	PRECAST	903	1492	20	2694	3295	3897	4498
			923	1492	20	2694	3295	3897	4498
6'-6"	(78")	PRECAST	835 (12)	1340	18	2419	2959	3498	4038
			835	1340	18	2419	2959	3498	4038
7'-6"	(90")	PRECAST	727 (22)	1021	16 (12)	2102 (22)	2571 (13)	3039 (13)	3508 (13)
			727	1169	16	2102	2571	3039	3508
9'-4"	(112")	PRECAST	591	680	11 (35)	1471 (30)	1811 (30)	2152 (29)	2494 (29)
			591	851	13	1705	2084	2463	2842
10'-6"	(126")	PRECAST	530	552	9	1185 (19)	1458 (19)	1732 (20)	2007 (29)
			530	686	11	1526	1865	2204	2544
11'-4"	(136")	PRECAST	474	485	7((35)	1034 (32)	1272 (32)	1510 (32)	1749 (31)
			494	599	10	1422	1738	2053	2369
12'-0"	(144")	PRECAST	470 (5)	441	7((36)	936 (34)	1151 (35)	1366 (35)	1582 (33)
			470	543	9(	1349	1648	1948	2247
13'-4"	(160")	PRECAST	418 (19)	373	6((36)	783 (34)	962 (34)	1141 (34)	1321 (34)
			428	455	7(	1145	1444	1718	1993
14'-0"	(168")	PRECAST	384 (18)	346	5((44)	723 (34)	887 (34)	1052 (34)	1218 (34)
			410	420	7(	1050	1434 (8)	1694 (8)	1954 (7)
14'-8"	(176")	PRESTRESSED	239	323	5' (41)	671 (41)	823 (13)	976 (14)	1129 (14)
			246	390	6(	968	1324 (8)	1625 (11)	1874 (11)
15'-4"	(184")	PRESTRESSED	224	302	4((35)	626 (33)	767 (13)	909 (13)	1052 (13)
			230	364	6(	897	1224 (8)	1562 (14)	1801 (14)
17'-4"	(208")	PRESTRESSED	187	255	4((12)	520 (12)	637 (12)	754 (12)	872 (12)
			192	303	5(	732	993 (8)	1268 (14)	1470 (16)
19'-4"	(232")	PRESTRESSED	162	222	3' (11)	446 (11)	546 (12)	646 (12)	746 (12)
			166	261	4(	616	831 (8)	1057 (16)	1225 (16)
21'-4"	(256")	PRESTRESSED	142	198	3((11)	393 (11)	480 (12)	567 (11)	654 (11)
			142	230	3(	531	713 (7)	903 (13)	1046 (13)
22'-0"	(264")	PRESTRESSED	137	192	2((10)	378 (10)	461 (10)	545 (10)	629 (11)
			137	221	3(	508	681 (7)	861 (13)	997 (13)
24'-0"	(288")	PRESTRESSED	124	175	2((10)	341 (10)	416 (10)	491 (10)	566 (10)
			124	200	3	450	600 (7)	756 (12)	875 (11)

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

LENGTH \ TYPE			8RU6	8RF6-0B / 8RF6-1B	8RF10B / 8RF1B	8RF14-0B / 8RF14-1B	8RF18-0B / 8RF18-1B	8RF22-0B / 8RF22-1B	8RF26-0B / 8RF26-1B	8RF30-0B / 8RF30-1B
4'-4"	(52")	PRECAST	1635	1749	33	3280	4349	5421	6493	7567
				1891	38	5206	6639	8060	9479	10893
4'-6"	(54")	PRECAST	1494	1596	30	2992	3968	4946	5924	6904
				1756	38	5206	6639	8060	9479	10893
5'-8"	(68")	PRECAST	866	920	17	1716	2277	2839	3402	3966
				1167	24	4567	6389	8060 (24)	7917 (18)	9311 (18)
5'-10"	(70")	PRECAST	810	859	16	1600	2124	2649	3174	3700
				1113	23	4242	6639 (18)	8060 (24)	7402 (18)	8706 (18)
6'-8"	(80")	PRECAST	797	901	18	3120	5048	7747	9448	7360
				901	18	3120	5048	7915	9479	10893 (20)
7'-6"	(90")	PRECAST	669	755	14	2459	3776	5743	7239	5623
				755	14	2459	3776	5743	8998 (18)	10893 (40)
9'-8"	(116")	PRECAST	411	466	9(	1568	2253	3129	4091	3146
				526	9(	1568	2253	3129	4150	5891 (47)

SAFE UPLIFT LOADS FOR 8" PRECAST w/" RECESS DOOR U-LINTELS

LENGTH \ TYPE			8RF6-1T / 8RF6-2T	8RF10-1T / 8RF10-2T	8RF1(T / 8RF 0T	8RF18-1T / 8RF18-2T	8RF22-1T / 8RF22-2T	8RF26-1T / 8RF26-2T	8RF30-1T / 8RF30-2T
4'-4"	(52")	PRECAST	905	1748	26	3522	4409	5296	6183
			905	1748	26	3522	4409	5296	6183
4'-6"	(54")	PRECAST	867	1675	25	3374	4224	5074	5924
			867	1675	25	3374	4224	5074	5924
5'-8"	(68")	PRECAST	675	1301	19	2618	3277	3935	4594
			675	1301	19	2618	3277	3935	4594
5'-10"	(70")	PRECAST	655	1262	19	2538	3176	3815	4453
			655	1262	19	2538	3176	3815	4453
6'-8"	(80")	PRECAST	570	1012	16	2204	2758	3312	3865
			570	1097	16	2204	2758	3312	3865
7'-6"	(90")	PRECAST	506	797	14 (4)	1952 (2)	2442 (6)	2931 (4)	3257
			506	967	14	1952	2442	2931	3421
9'-8"	(116")	PRECAST	395	491	9 (12)	1301 (13)	1640 (18)	1980 (20)	2322 (29)
			395	589	11	1514	1893	2272	2652



STRUCTURAL FLOOR PLAN
SCALE: 3/16" = 1'-0"

1ST FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	48.4'	113.5'
LONGITUDINAL	41.9'	88.5'

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

DOOR & WINDOW BUCK ATTACHMENT

TAPCON IN FACE OF CMU
2 1/2" MIN. EDGE DISTANCE
1 1/4" MIN. EMBEDMENT
3" MIN. SPACING

WINDOWS & DOORS UP TO 6'X8'

3/16" TAPCONS @ 2' O.C.
1/4" TAPCONS @ 3' O.C.

WINDOWS & DOORS UP TO 8'X12'

3/16" TAPCONS @ 16" O.C.
1/4" TAPCONS @ 24" O.C.

SLIDERS UP TO 8'HX20'W

3/16" TAPCONS @ 12" O.C.
1/4" TAPCONS @ 18" O.C.

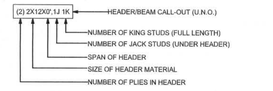
GARAGE DOOR UP TO 10'W

(2) 3/16" TAPCONS & 16" O.C.
(2) 1/4" TAPCONS & 24" O.C.

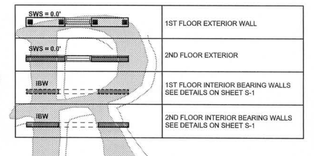
GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C.
(2) 1/4" TAPCONS & 12" O.C.

HEADER LEGEND



WALL LEGEND



INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

THREADED ROD LEGEND

INDICATES LOCATION OF: 1ST FLOOR 1/2" A307 ALL THREADED ROD

INDICATES LOCATION OF: 2ND FLOOR 1/2" A307 ALL THREADED ROD

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. (JOB #7-100)

REVISIONS	

SOFTPLAN ARCHITECTURAL DESIGN SOFTWARE

WINDLOAD ENGINEER: Mark Disosway, IE No.53915, PO8 868, Lake City, FL 2056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dmensions. Refer all questions to fark Disosway, P.E. for resolution. to not proceed without clarification.

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IERTIFICATION: I hereby certify that I have examined this plan, and that the applicable prtions of the plan, relating to wind engineering omply with section R301.2.1, florida building ode residential 2004, to the best of my lowledge.

LMITATION: This design is valid for one bilding, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Isaac Construction

Nick Patel Residence

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Lake City, Florida 32055

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Phone: (386) 754 - 5419
Fax: (386) 269 - 4871

PRINTED DATE: June 21, 2007

DRAWN BY: David Disosway

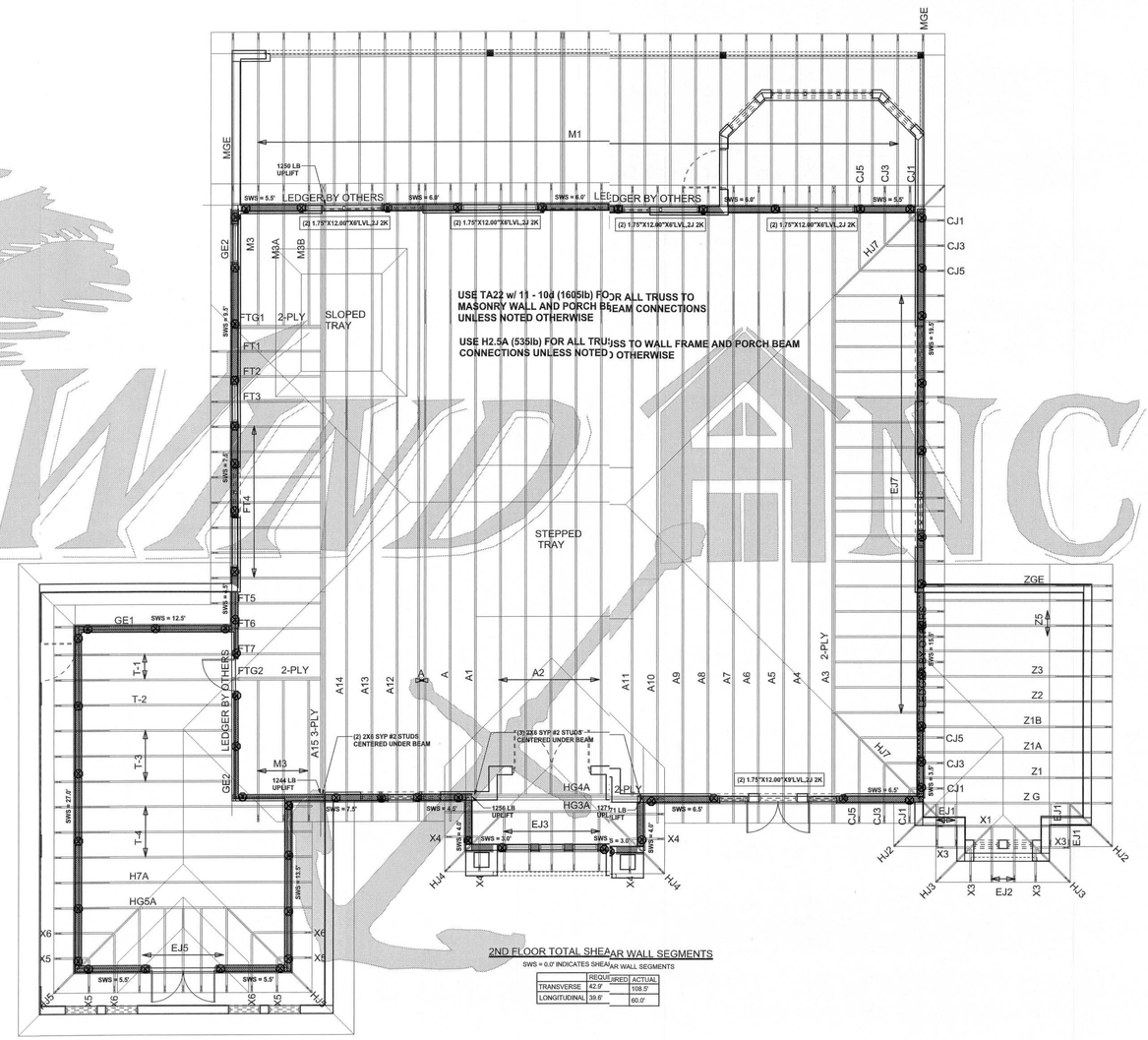
CHECKED BY:

FINALS DATE: 28 / May / 07

JOB NUMBER: 705154

DRAWING NUMBER **S-3**

OF 5 SHEETS



STRCTURAL ROOF PLAN
SCALE:1/8" = 1'-0"

2ND FLOOR TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	42.9'	108.5'
LONGITUDINAL	39.6'	60.0'

STRUCTURAL PLAN NOTES

SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYPI#2 (U.N.O.)

SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)

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2 1/2" MIN. EDGE DISTANCE
1 1/4" MIN. EMBEDMENT
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1/4" TAPCONS @ 3' O.C.

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SLIDERS UP TO 8'6X20'6W

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GARAGE DOOR UP TO 10'W

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(2) 1/4" TAPCONS & 24" O.C.

GARAGE DOOR UP TO 18'W

(2) 3/16" TAPCONS & 8" O.C.
(2) 1/4" TAPCONS & 12" O.C.

HEADER LEGEND

(2) 2X12X0',1J 1K — HEADER/BEAM CALL-OUT (U.N.O.)
- NUMBER OF KING STUDS (FULL LENGTH)
- NUMBER OF JACK STUDS (UNDER HEADER)
- SPAN OF HEADER
- SIZE OF HEADER MATERIAL
- NUMBER OF PLIES IN HEADER

WALL LEGEND

SWS = 0.0'	1ST FLOOR EXTERIOR WALL
SWS = 0.0'	2ND FLOOR EXTERIOR
IBW	1ST FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1
IBW	2ND FLOOR INTERIOR BEARING WALLS SEE DETAILS ON SHEET S-1

INDICATES LOCATION OF #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

INDICATES LOCATION OF (2) #5 VERTICAL DOWEL IN CMU CELL, FILLED SOLID W/ 3000 P.S.I. CONC.

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CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. ANDERSON TRUSS CO. (JOB #7-100)

REVISIONS	

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WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

DIMENSIONS:
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915
SEAL

Isaac Construction

Nick Patel Residence

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DRAWING NUMBER
S-4
OF 5 SHEETS