

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: IUM09114Z0114073137

73

Truss Fabricator: Anderson Truss Company
Job Identification: 12-085f--Doug Edgley JUSTICE RES Floor - 16"oc -- Lake City, FL
Truss Count: 10
Model Code: Florida Building Code 2004 and 2006 Supplement
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - N/A
Floor - 55.0 PSF @ 1.00 Duration
Wind - No Wind

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

05/14/2012

-Truss Design Engineer-
Walter P. Finn

Florida License Number: 22839
1950 Marley Drive
Haines City, FL 33844

Details: STRBRI BR-CNSY42PL-

#	Ref	Description	Drawing#	Date
1	65743--F1	25'2" Gable	12135001	05/14/12
2	65744--F2	25'2" Floor T	12135002	05/14/12
3	65745--F3	28'2" Floor T	12135009	05/14/12
4	65746--F4	28'2" Floor T	12135010	05/14/12
5	65747--F5	3'4"8 Floor T	12135008	05/14/12
6	65748--F6	3'4"8 Floor T	12135007	05/14/12
7	65749--F7	12'6" Floor T	12135003	05/14/12
8	65750--F8	3'10"8 Floor	12135004	05/14/12
9	65751--F9	28'2" Floor T	12135005	05/14/12
10	65752--F10	28'2" Floor	12135006	05/14/12



(12-085F--Doug Edgley JUSTICE RES Floor - 16'oc -- Lake City, FL - F1 25'2" Gable)

Top chord 4x2 SP #1
Bot chord 4x2 SP #1
Webs 4x2 SP #3

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

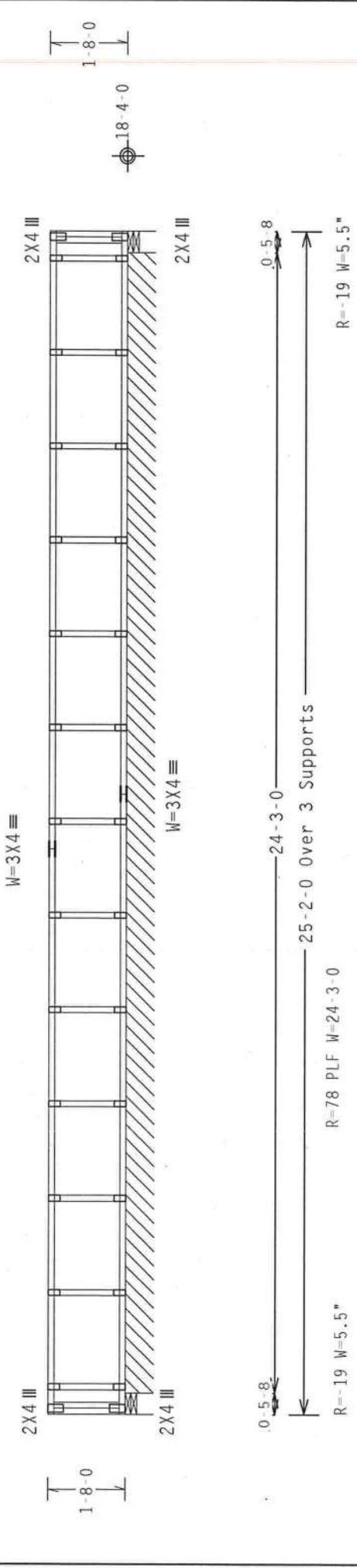
Fasten rated sheathing to one face of this frame.

See detail STRBRIBR0211 for bracing and bridging recommendations.

Trusses to be spaced at 16.0" OC maximum.

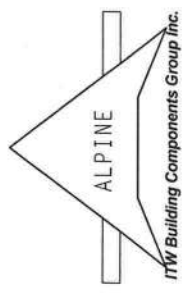
Truss must be installed as shown with top chord up.

Sheathing is required for any longitudinal(drag) forces. All connections to be designed by the building designer.



Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)



ALPINE

ITW Building Components Group Inc.

12-085F-- Doug Edgley

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET:
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MICA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ABSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ABSI/TPI 1 Sec.2. For more information see: This Job's

****IMPORTANT****

PLT TYP. Wave

QTY: 2

Scale = .3125" / Ft.

TC LL	40.0 PSF	REF	R9114- 65743
TC DL	10.0 PSF	DATE	05/14/12
BC DL	5.0 PSF	DRW	HCUSR9114 12135001
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	94993
DUR.FAC.	1.00		



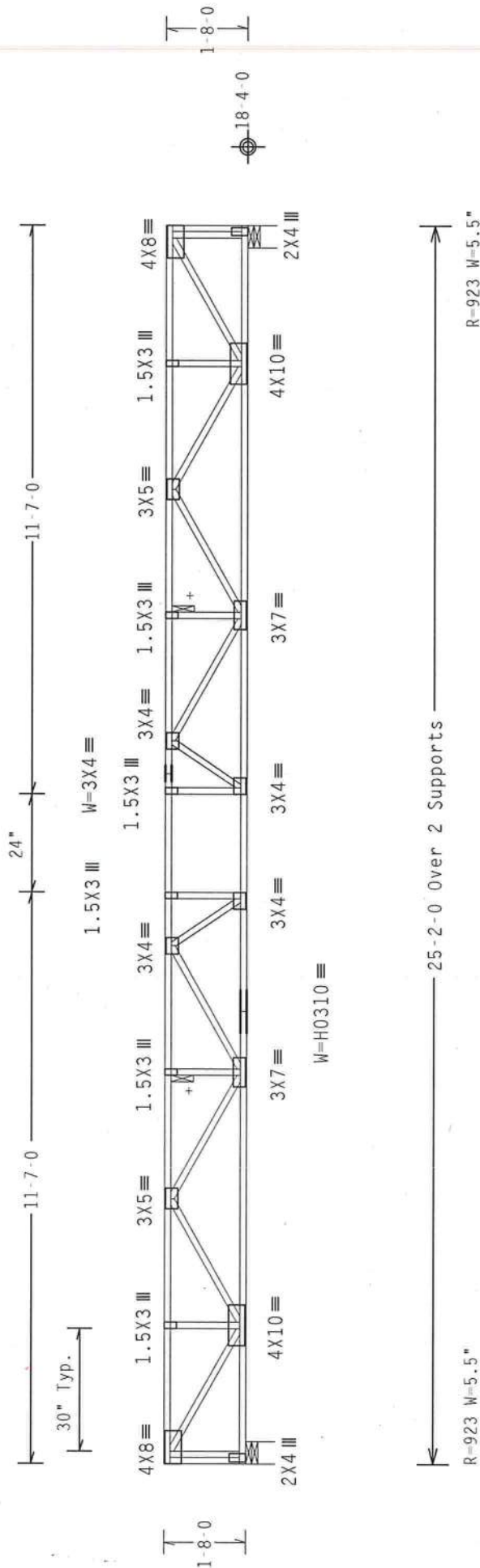
Top	chord	4x2	SP	#1
Bot	chord	4x2	SP	#1
	webs	4x2	SP	#3

+ 2x6 continuous strongback. See detail STRBRIBR0211 for bracing and bridging recommendations.

Max JT VERT DEFL: LL: 0.41" DL: 0.29" recommended camber 3/8"

Trusses to be spaced at 16.0" OC maximum.

Truss must be installed as shown with top chord up.



PLT TYP. 20 Gauge HS, Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)

QTY:14 FL/-/5/-/-/R/- Scale =.3125"/Ft.

TC LL	40.0	PSF	REF R9114-	65744
TC DL	10.0	PSF	DATE	05/14/12
BC DL	5.0	PSF	DRW	HCUSR9114 12135002
BC LL	0.0	PSF	HC-ENG	JB/WPF
TOT.LD.	55.0	PSF	SEQN-	94994
DUR.FAC.	1.00			

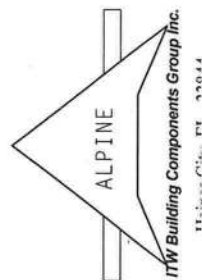


***WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
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***IMPORTANT**

Trusses, require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the following information for installing RCSS. Including Component information, by IPI and WPA for safety and proper installation. For more information, please contact the manufacturer. RCSS shall be installed in accordance with the manufacturer's instructions. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Ceilings shall be for permanent lateral restraint of webs. RCSS shall have bracing installed per RCSS sections B3, B7 or B9, as applicable.

The Building Components Group Inc. (TUBCOG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ABSI/TPI 1, or for handling, shipping, installation & unloading of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1600-2 for standard plate positions. A seal on this detail and/or cover plate flaking this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ABSI/TPI 1 Sec.2. For more information see: This job's



+ 2x6 continuous strongback. See detail STRBRIBR0211 for bracing and bridging recommendations.

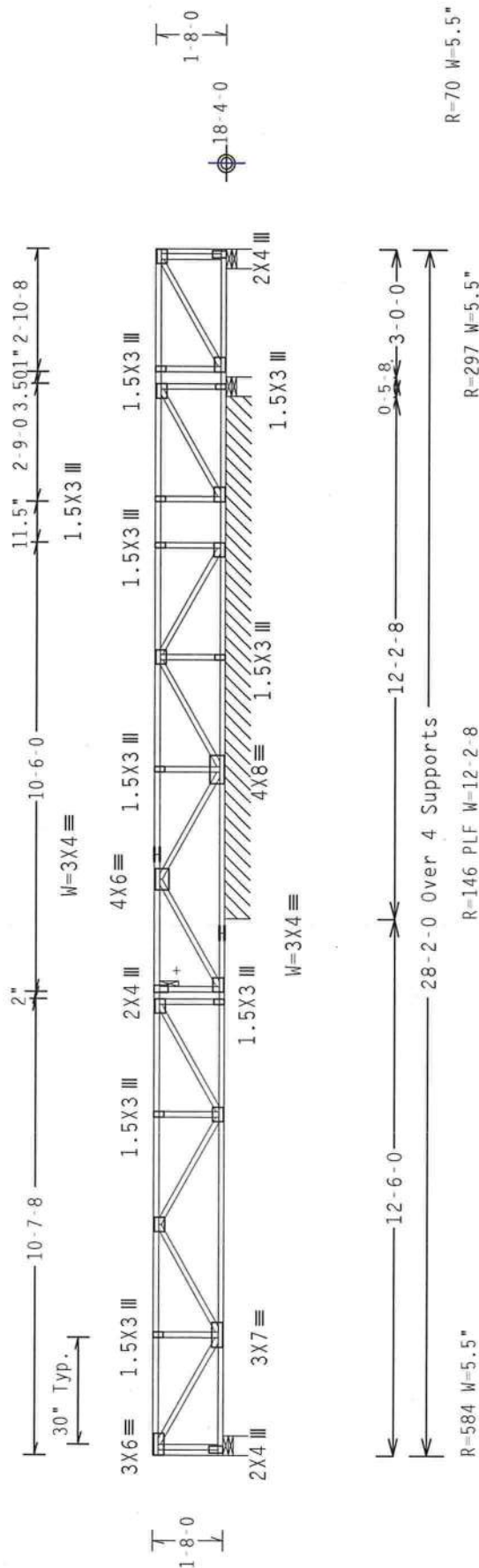
Special loads
----- (Lumber

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

Species	Number of trees	Dur.Fac.=1.00 /	Plate Dur.Fac.=1.00)
TC- From	67	plf at 0.00 to	67 plf at 14.00
TC- From	67	plf at 14.00 to	67 plf at 28.17
BC- From	7	plf at 0.00 to	7 plf at 12.17
BC- From	7	plf at 12.17 to	7 plf at 28.17
TC- 473.71 lb	Conc. Load at	12.65	
TC- 189.54 lb	Conc. Load at	24.15	



Note: All Plates Are 3X4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007 (STD)

PLT TYP. Wave

10.03.11 02:00:00 01/15/11 01/15/11

Scale = .25"/Ft.

TC LL	40.0	PSF	REF R9114-	65745
TC DL	10.0	PSF	DATE	05/14/12
BC DL	5.0	PSF	DRW HCUR9114	12135009
BC LL	0.0	PSF	HC-ENG JB/WPF	
TOT.LD.	55.0	PSF	SEQN-	3784
DUR.FAC.	1.00			



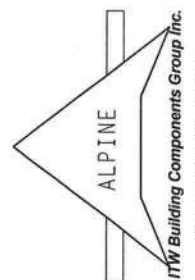
****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT****

Trusses require extreme care in fabrication, handling, shipping, installing and bracing. Refer to and follow the instructions on the attached drawings for proper erection and bracing. The design is based on the following assumptions:

- 1. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 2. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 3. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 4. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 5. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 6. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 7. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 8. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 9. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.
- 10. All trusses shall be fabricated in accordance with the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 1989 Edition.

The building components, Group Inc. (LH0000) shall not be responsible for any deviation from this design, including the failure to build the truss in accordance with AISC 1989, 1.1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design form structure is the responsibility of the user.



Top chord	4x2	SP	M-30	T2	4x2	SP	#1:
Bot chord	4x2	SP	#1				
webs	4x2	SP	#3				

2 COMPLETE TRUSSES REQUIRED

See DWG CNSY42PL0109 for connection details of 2 ply trusses.

+ 2x6 continuous strongback. See detail STRBRIBR0211 for bracing and bridging recommendations.

Max JT VERT DEFL: LL: 0.54" DL: 0.30" recommended camber 1 1/2"

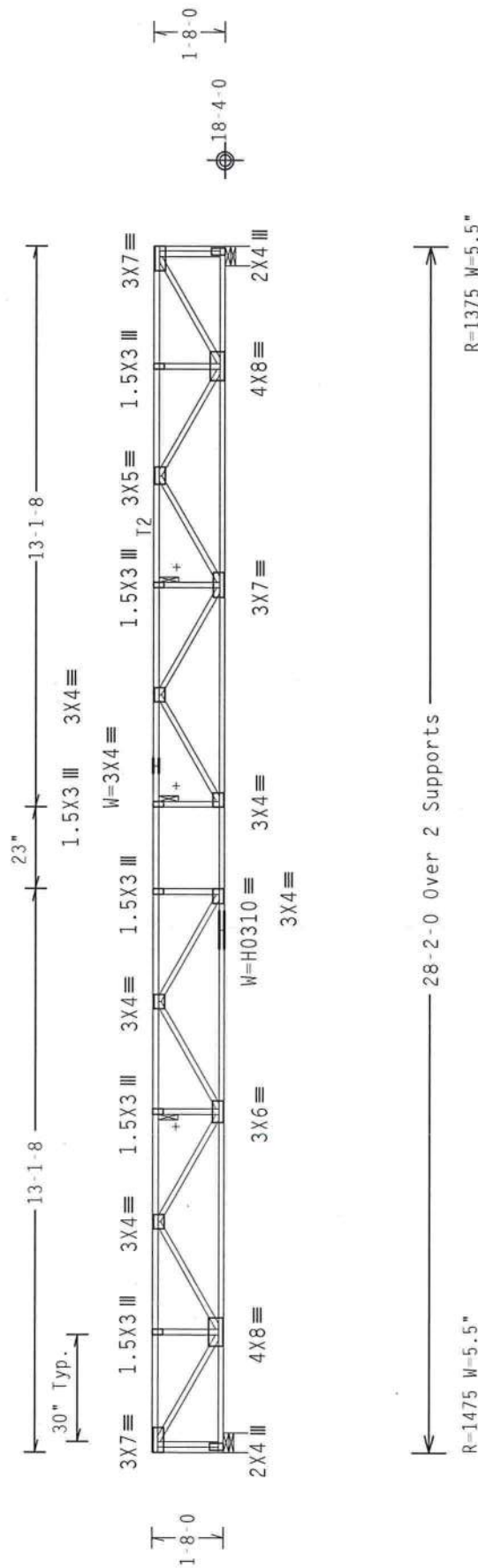
Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Special loads

	(Lumber	Dur.Fac.=1.00 /	Plate Dur.=1.00)
TC- From	67 plf at 0.00	to 67 plf at 28.17	
BC- From	7 plf at 0.00	to 7 plf at 28.17	
TC- 218.37 lb Conc.	Load at 4.02		
TC- 566.71 lb Conc.	Load at 15.52		

Truss must be installed as shown with top chord up.



PLT TYP. 20 Gauge HS.Wave
Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=12%(0%) /10(0)

QTY:1 FL/-/5/-/-/R/-
Scale = .25"/Ft.

10.03.11

PLT TYP. 20 Gauge HS.Wave

TC LL	40.0	PSF	REF R9114 - 65746
TC DL	10.0	PSF	DATE 05/14/12
BC DL	5.0	PSF	DRW HCUSR9114 12135010
BC LL	0.0	PSF	HC-ENG JB/WPF
TOT.LD.	55.0	PSF	SEQN- 3783
DUR.FAC.	1.00		

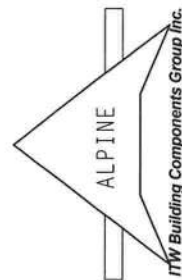


****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT****

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BEST (Building Component Safety Information, by TPI and MCA) for safety information. Trusses are designed to be installed in a vertical position. Trusses are designed to be installed with the top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BEST sections B3, B7 or B10, as applicable.

TPI Building Components Group Inc. (TIBCOG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI for handling, shipping, installation and bracing of trusses. Apply plates to each side of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings TIBO-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is



Top	chord	4x2	SP	#1
Bot	chord	4x2	SP	#1
	webs	4x2	SP	#3

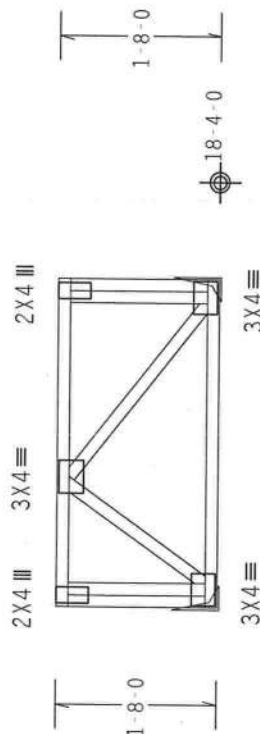
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

These support conditions used at bearings indicated

(H1) = Simpson THA422 w/ (1)4x2 SP #1 supporting member.
(2) 0.162"x3.5" nails into supporting member,
(4) in flange and
(6) 0.148"x3" nails into supported member.

(H2) = Simpson THA422 w/ (2)4x2 SP #1 supporting member.
(2) 0.162"x3.5" nails into supporting member,
(4) in flange and
(6) 0.148"x3" nails into supported member

21.461" Typ.



3-4-8 Over 2 Supports >

R=474 R=567

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLER**

****IMPORTANT****

Trusses require extreme care in fabricating, handling, shipping, installing, and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by TPI and WCCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per B1 sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBC) shall not be responsible for any deviation from this design, any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This lab's

Special loads

---(Lumber Dur.Fac.=1.00 / Plate Dur.Fac.=1.00)
 TC- From 33 plf at 0.00 to 33 plf at 3.38
 BC- From 33 plf at 0.00 to 3 plf at 3.38
 IC- 458.33 lb Conc. Load at 1.15, 2.52

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.

PLT TYP. Wave

10.03.11.020 QTY:1 FL/-/5/-/-/R/- Scale = 5"/Ft.

REF R9114- 65747

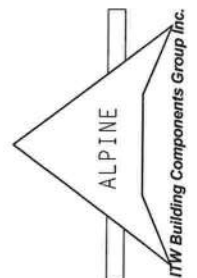
DATE 05/14/12

DRW HCUSR9114 12135008

HC-ENG JB/WPF

3782 SEQN-

DUR.FAC. 1.00

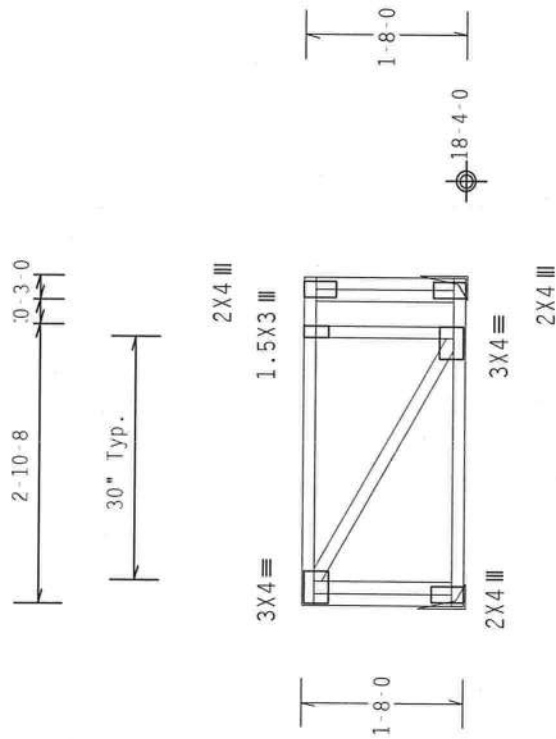


Top	chord	4x2	SP	#1
Bot	chord	4x2	SP	#1
	Webs	4x2	SP	#3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(H1) = (J) Hanger not calculated (1)4x2 SP #1 supporting member.
(H2) = Simpson THA422 w/ (2)4x2 SP #1 supporting member.
(2) 0.162"x3.5" nails into supporting member.
(4) in flange and
(6) 0.148"x3" nails into supported member

Truss must be installed as shown with top chord up.



3-4-8 Over 2 Supports >
R-190 R-218

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)

10.03.11

QTY:1 FL/-/5/-/-/R/-

$$\text{Scale} = 5''/\text{Ft}$$

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

****IMPORTANT****

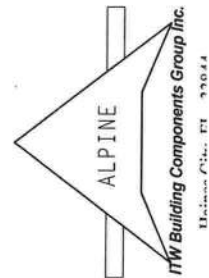
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP Building Code, Part I and II, and AIA/CES for safety practices prior to performing these functions. Installers shall provide, at their own expense, any additional instructions noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Ceilings shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (IBCO) shall not be responsible for any deviation from this design, including, but not limited to, any failure to build the truss in accordance with ABSI/PI 1, or for handling, shipping, installation & attaching of trusses. Apply plates to each face of truss and position as shown above and on the joint. If the drawings, unless noted otherwise, refer to drawings 1600-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ABSI/PI 1 Sec.2. For more information see: This job's



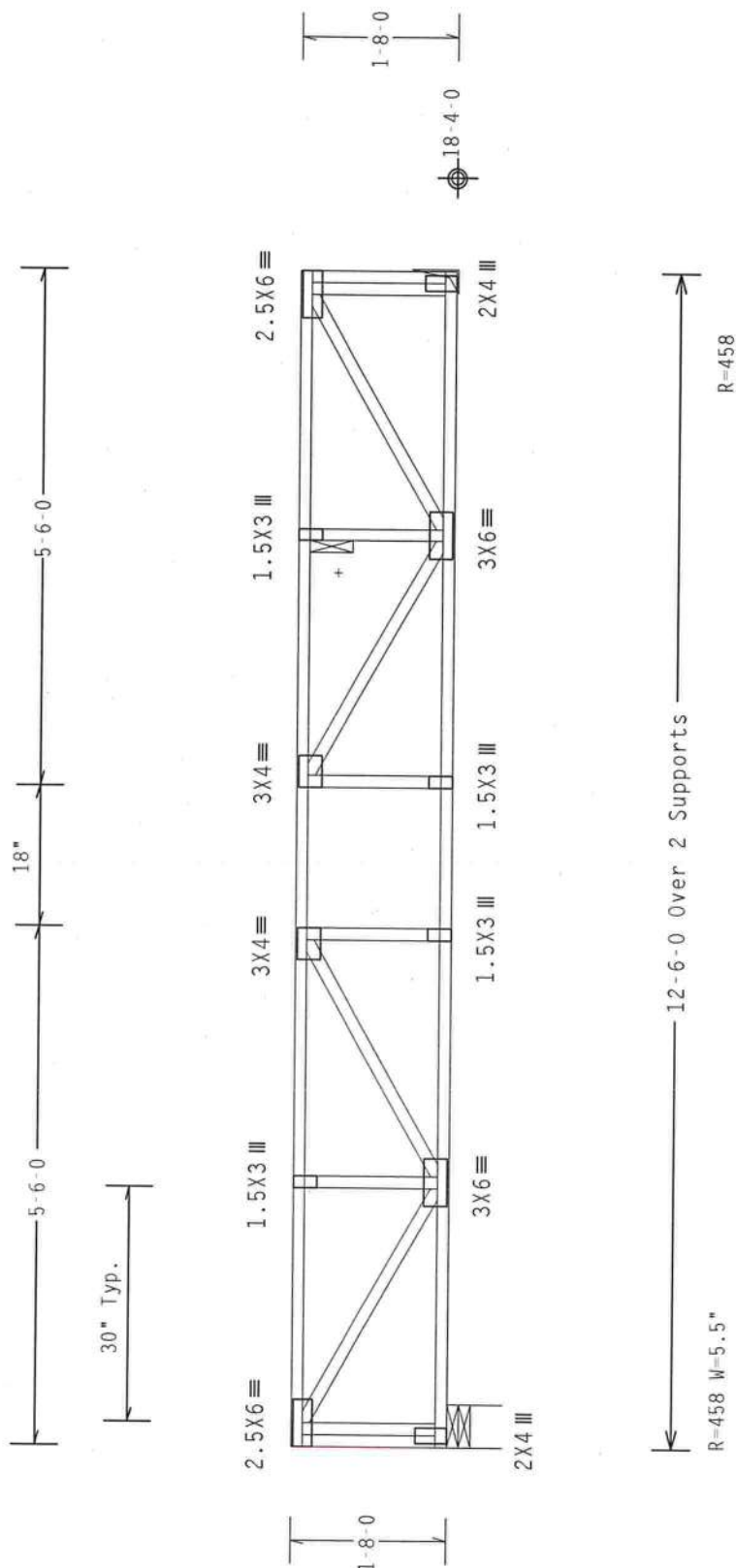
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TC DL	10.0	PSF	DATE 05/14/12
BC DL	5.0	PSF	DRW HCUSR9114 12135007
BC LL	0.0	PSF	HC-ENG JB/WPF
TOT.LD.	55.0	PSF	SEQN- 3772
DUR.FAC.	1.00		



+ 2X6 continuous strongback. See detail STRBRIBR0211 for bracing and bridging recommendations.

(H1) = (J) Special hanger required (1)4x2 SP #1 supporting member.

Truss must be installed as shown with top chord up.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)

QTY:2 FL/-/5/-/-/R/- Scale =.5"/Ft.

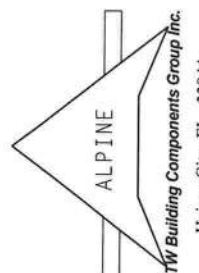
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TC DL	10.0 PSF	DATE 05/14/12
BC DL	5.0 PSF	DRW HCURSR9114 12135003
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT.LD.	55.0 PSF	SEQN - 94995
DUR.FAC.	1.00	



****IMPORTANT****
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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by TPI and KTCO) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Unless otherwise noted, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (IUBCG) shall not be responsible for any deviation from this design. Any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this product is not required. IUBCG is not responsible for the design of the building. The manufacturer's responsibility shall be limited to the design of the building shown. The suitability and use of this design for any structure is the responsibility of the building owner.

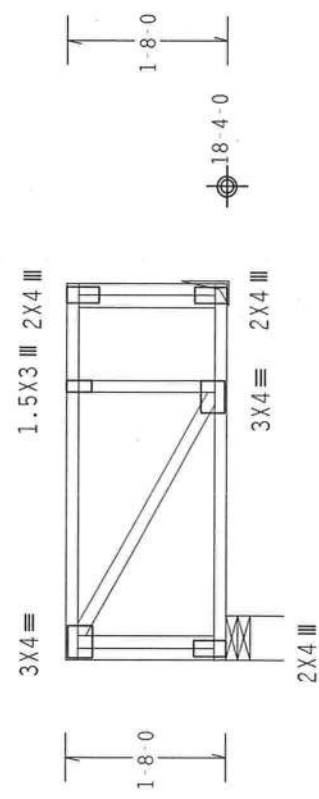
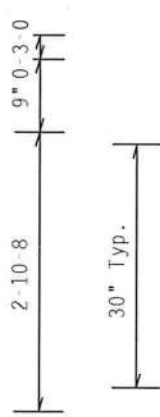


Top chord 4x2 SP #1
Bot chord 4x2 SP #1
Webs 4x2 SP #3

(H1) = (J) Special hanger required (1)4x2 SP #1 supporting member.

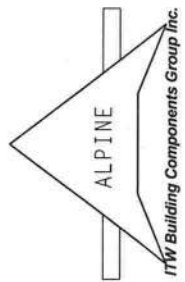
Trusses to be spaced at 16.0" OC maximum.
Truss must be installed as shown with top chord up.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



3'-10-8 Over 2 Supports
R=143 W=5.5" R=142

PLT TYP. Wave



ALPINE
r/w Building Components Group Inc.
Union City, CT 07014

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)

10.03.11

QTY:2

FL/-5/-/-/R/-

Scale = .5"/Ft.

TC LL	40.0 PSF	REF	R9114- 65750
TC DL	10.0 PSF	DATE	05/14/12
BC DL	5.0 PSF	DRW	HCUSR9114 12135004
BC LL	0.0 PSF	HC-ENG	JB/WPF
TOT.LD.	55.0 PSF	SEQN-	94996
DUR.FAC.	1.00		

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THE Building Components Group Inc. (THBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: THIS JOB'S

(12-085f--Doug Edgley JUSTICE RES Floor - 16"oc -- Lake City, FL - F9 28'2" Floor Truss)

Top chord 4x2 SP #1
Bot chord 4x2 SP #1
Webs 4x2 SP #3

+ 2x6 continuous strongback. See detail STRBR1BR0211 for bracing and bridging recommendations.

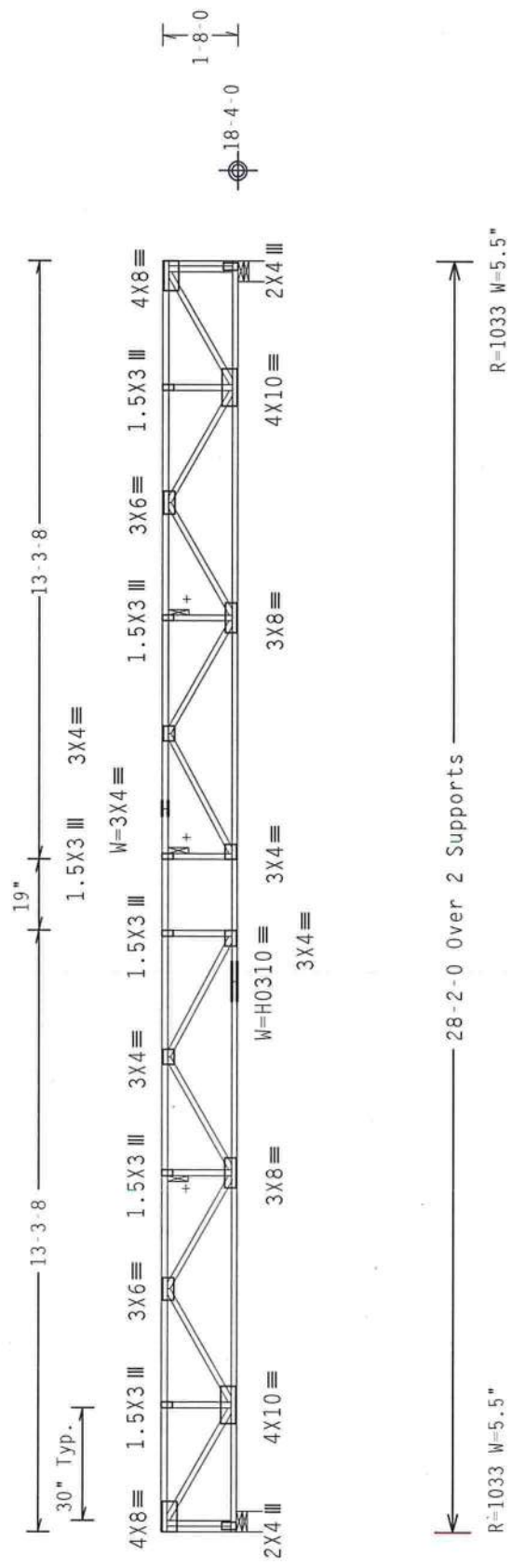
Max JT VERT DEFL: LL: 0.62" DL: 0.42" recommended camber 5/8"

Maximum panel length exceeds 30". TPI allows non-bearing partition walls to be supported at any point when panels are 30" or less.

Trusses to be spaced at 16.0" OC maximum.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.



PLT TYP. 20 Gauge HS.Wave

Design Crit: FBC2010Res/TPI-2007(STD)

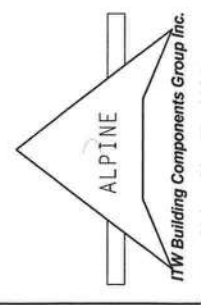
10.03.11

QTY:5

Scale = .25"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and MGA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
TPI Building Components Group Inc. (TPI/BCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's

TC LL	40.0 PSF	FL/-5/-/-/R/-	REF R9114- 65751
TC DL	10.0 PSF		DATE 05/14/12
BC DL	5.0 PSF		DRW HCUSR9114 12135005
BC LL	0.0 PSF		HC-ENG JB/WPF
TOT.LD.	55.0 PSF		SEQN- 3786
DUR.FAC.	1.00		



(12-085f--Doug Edgley JUSTICE RES Floor - 16"oc -- Lake City, FL - F10 28'2" Floor Truss)

Top chord 4x2 SP #1
Bot chord 4x2 SP #1
Webs 4x2 SP #3

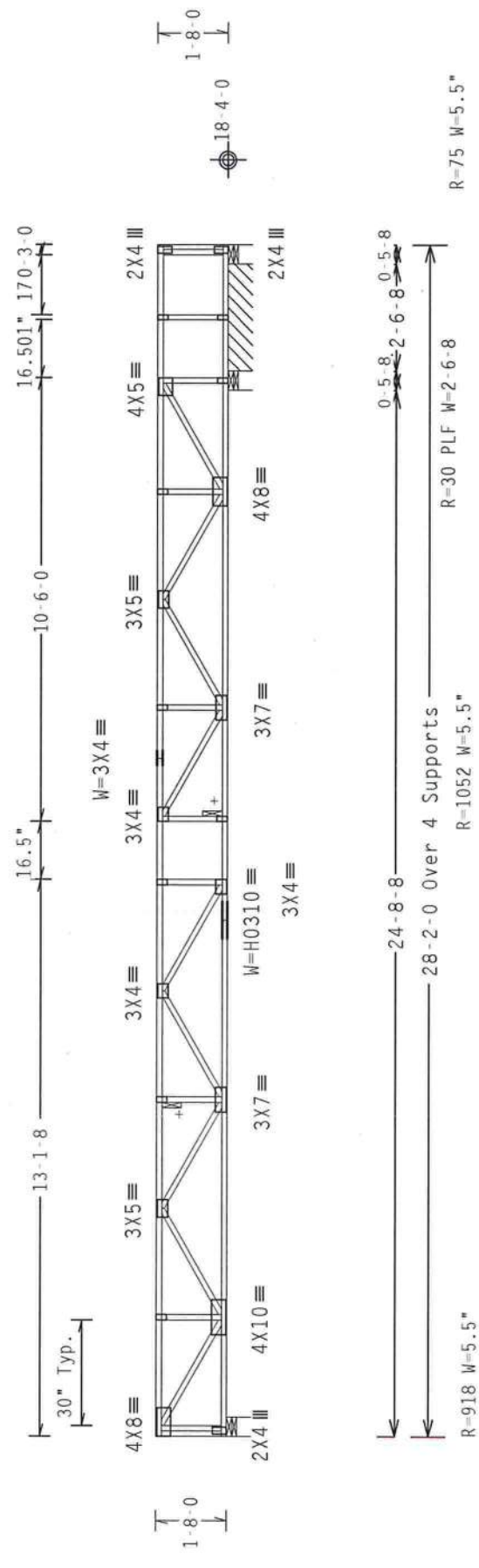
+ 2x6 continuous strongback. See detail STRBRI0211 for bracing and bridging recommendations.

Max JT VERT DEFL: LL: 0.44" DL: 0.32" recommended camber 3/8"

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Trusses to be spaced at 16.0" OC maximum.

Truss must be installed as shown with top chord up.



Note: All Plates Are 1.5X3 Except As Shown.

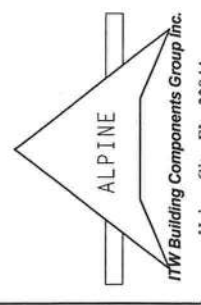
Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=12%(0%)/10(0)

PLT TYP. 20 Gauge HS, Wave

QTY: 2	FL / - / 5 / - / - / R / -	Scale = .25" / Ft.
TC LL	40.0 PSF	REF R9114- 65752
TC DL	10.0 PSF	DATE 05/14/12
BC DL	5.0 PSF	DRW HCUSR9114 12135006
BC LL	0.0 PSF	HC-ENG JB/WPF
TOT.LD.	55.0 PSF	SEQN- 94998
DUR.FAC.	1.00	



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
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The Building Components Group Inc. (BCHG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



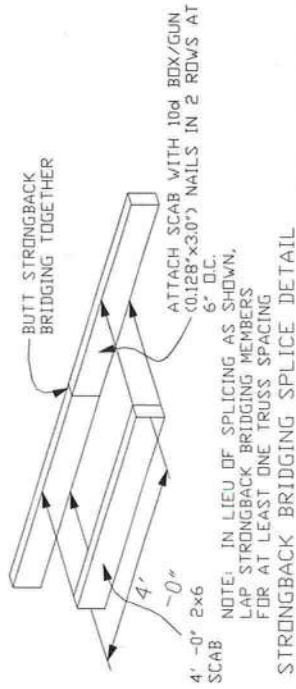
STRONGBACK BRIDGING RECOMMENDATIONS

- ▶ All scab-on blocks shall be a minimum 2x4 "stress graded lumber."
- ▶ All strongback bridging and bracing shall be a minimum 2x6 "stress graded lumber."
- ▶ The purpose of strongback bridging is to develop load sharing between individual trusses, resulting in an overall increase in the stiffness of the floor system. 2x6 strongback bridging, positioned as shown in details, is recommended at 10' - 0" o.c. (max.)

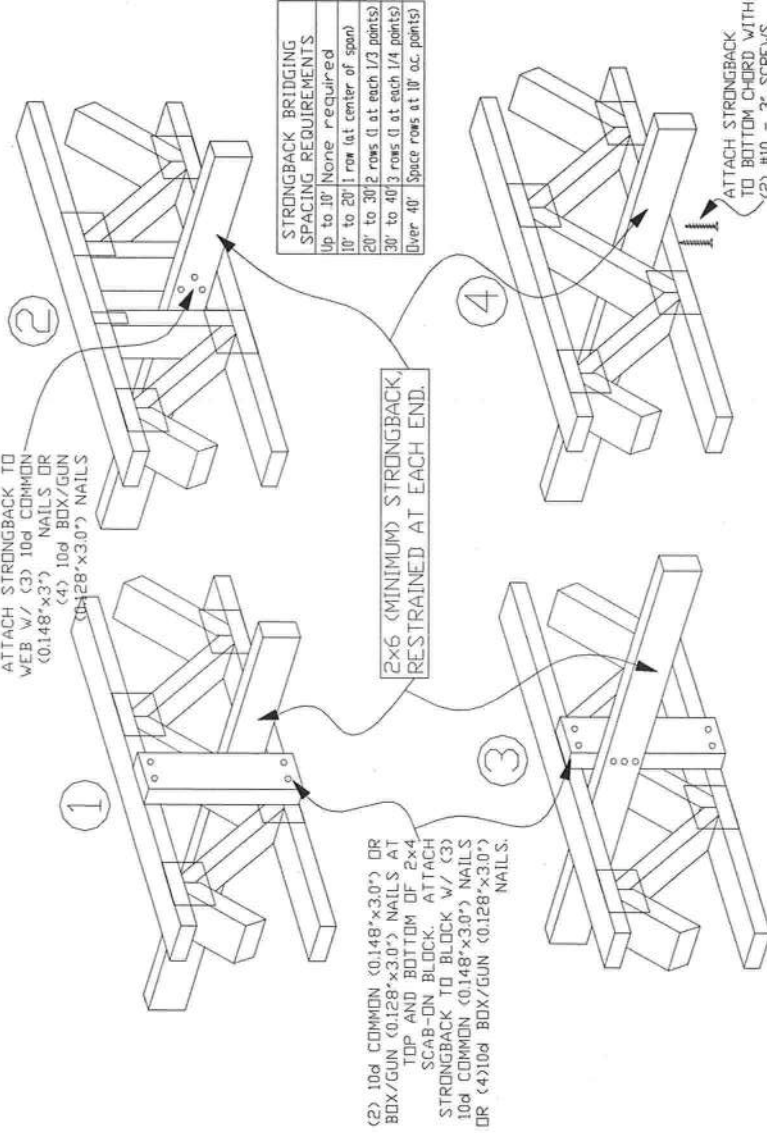
The terms "bridging" and "bracing" are sometimes mistakenly used interchangeably. "Bracing" is an important structural requirement of any floor or roof system. Refer to the Truss Design Drawing (TDD) for the bracing requirements for each individual truss component. "Bridging," particularly "strongback bridging" is a recommendation for a truss system to help control vibration. In addition to aiding in the distribution of point loads between adjacent truss, strongback bridging serves to reduce "bounce" or residual vibration resulting from moving point loads, such as footsteps.

The performance of all floor systems are enhanced by the installation of strongback bridging and therefore is strongly recommended by ITW Building Components Group Inc.

For additional information regarding strongback bridging, refer to BCSI (Building Component Safety Information).



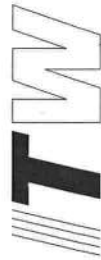
NOTE: Details 1 and 2 are the preferred attachment methods



STRONGBACK BRIDGING ATTACHMENT ALTERNATIVES

WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET. Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural panels and bottom chord shall have a properly attached lateral bracing. Lateral bracing shall be installed in accordance with the bracing installed per BCSI sections 88 & 87. See this job's general notes page for more information.

IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing, bracing, or erecting trusses. ITWBCG connector plates are made of 2018/1604 (W/H/S/K) ASTM A653 galvanized steel. Apply plates to each face of truss, positioned as shown above and on Joint Details. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2. ITW-BCG www.itwbcg.com, TPI www.tpi.com, WCA www.wcaindustry.com, ICC www.iccsafe.org



Building Components Group Inc.

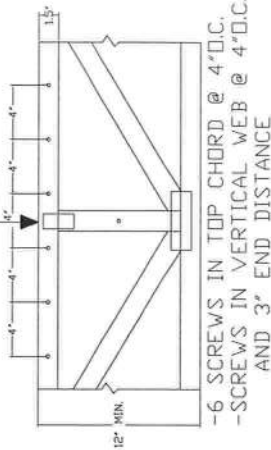
Earth City, MO 63045

TC LL	PSF	REF	STRONGBACK
TC DL	PSF	DATE	2/28/11
BC DL	PSF	DRWG	STRBIBR0211
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			



SYSTEM 42 PLY TO PLY CONNECTION DETAIL

MAX. CONCENTRATED LOAD
AS PER CHART BELOW

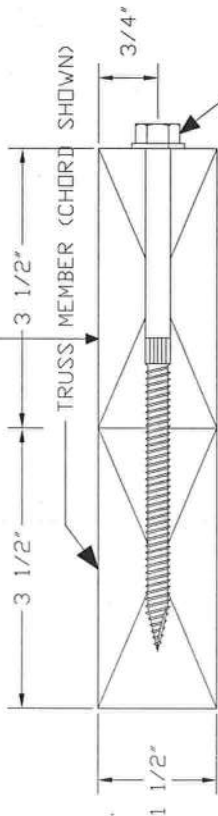


-6 SCREWS IN TOP CHORD @ 4\"/>

USING SIMPSON 1/4\"/>

REFER TO ITWBCG SEALED DRAWING
FOR INDIVIDUAL TRUSS DESIGN.

LOAD APPLIED TO THIS PLY

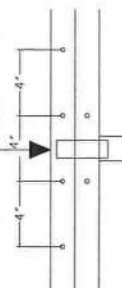


SCREW POSITION IN THE TRUSS

FOR DOUBLE TOP CHORDS

USE (4) SCREWS IN UPPER TOP CHORD
USE (2) SCREWS IN LOWER TOP CHORD

MAX. CONCENTRATED LOAD
AS PER CHART BELOW



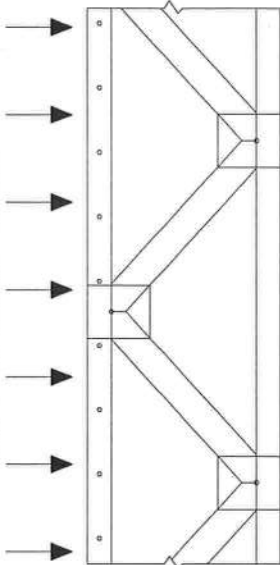
SCREWS CAN BE EQUALLY DIVIDED
(ROUND UP TO THE NEAREST WHOLE
NUMBER) INTO MULTIPLE VERTICALS.
USE A MINIMUM OF (1) SCREW PER
VERTICAL.

TRUSS DEPTH (INCH)	# OF SCREWS IN VERTICAL WEB	MAXIMUM CONCENTRATED LOAD (LBS)
12	1	3360
14	2	3840
16	2	3840
18	4	4320
20	4	4320
22	5	4800
24	5	4800

GENERAL NOTES:

1. SCREWS CENTERED ALONG THE 1.5\"/>
2. MINIMUM END DISTANCE OF 3\"/>
3. SCREWS INSTALLED WITH HEAD IN LOADED MEMBER.
4. GAP BETWEEN PLYS NOT TO EXCEED 1/8\"/>
5. HOLES NOT ALLOWED THRU A METAL CONNECTOR PLATE
6. SCREW LOCATION MAY BE ADJUSTED UP TO 1\"/>
7. DO NOT INSTALL SCREWS IN AREAS WHERE LUMBER WANE
EXCEEDS 1/4\"/>
8. CONCENTRATED LOADS TO BE APPLIED AT TRUSS PANEL POINT WITH
VERTICAL WEB.
9. FLOOR SHEATHING IS ASSUMED TO BE SECURELY FASTENED TO EACH
TRUSS TOP CHORD.
10. FOR 3x2 MEMBERS USE SIMPSON'S 1/4\"/>
11. CONTACT ITWBCG FOR SPECIAL CONNECTIONS NOT COVERED BY THIS
DETAIL.

MAX. UNIFORM LOAD
AS PER CHART BELOW



TOP CHORD SCREW SPACING PER CHART BELOW
(FOR SINGLE TOP CHORD ONLY)

UNIFORM LOADS

FOR DOUBLE TOP CHORD THE TOP
CHORD SCREW O.C. SPACING MAY BE
DOUBLED, (BUT MAY NOT EXCEED
24\"/>

TOP CHORD SCREW O.C. SPACING (INCH)	MAXIMUM UNIFORM LOAD (PLF) ALONG TOP CHORD (1.00 DF)
4	1440
6	960
8	720
10	576
12	480
14	411
16	360
18	320
20	288
22	261
24	240

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with TPI, or fabricating, handling, shipping, installing, bracing of trusses. ITWBCG connector plates are made of 2018/16GA (2018/16GA) ASTM A653 steel. All steel shall be galvanized (G90) per ASTM A1065. A seal on this drawing or cover page indicates acceptance and professional engineering responsibility solely for the truss component design shown. The suitability and use of this component for any building is the responsibility of the Building Designer per ANSI/TPI 1 Sec. 2.

ITW-BCG www.itwbcg.com TPI www.tpi.com WCA www.wcaindustry.com ICC www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045



TC LL	PSF	REF	SY42 CONNECTION
TC DL	PSF	DATE	1/1/09
BC DL	PSF	DRWG	CNSY42PL0109
BC LL	PSF		
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID: IULR9114Z0102152810

Truss Fabricator: Anderson Truss Company
Job Identification: 12-085--Doug Edgley JUSTICE RES -- Lake City, FL
Truss Count: 32
Model Code: Florida Building Code
Truss Criteria: FBC2010Res/TPI-2007(STD)
Engineering Software: Alpine Software, Version 10.03.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: 12015EC1-GBLLETIN-GABRST10-12030EC1-

#	Ref	Description	Drawing#	Date
1	12750--EJ2		12123001	05/02/12
2	12751--EJ3		12123002	05/02/12
3	12752--CJ1		12123003	05/02/12
4	12753--CJ2		12123004	05/02/12
5	12754--CJ3		12123005	05/02/12
6	12755--EJ1		12123006	05/02/12
7	12756--HJ1		12123007	05/02/12
8	12757--G1		12123008	05/02/12
9	12758--B3		12123009	05/02/12
10	12759--B2		12123010	05/02/12
11	12760--B1		12123011	05/02/12
12	12761--A4		12123012	05/02/12
13	12762--A3		12123013	05/02/12
14	12763--A2		12123014	05/02/12
15	12764--A1		12123015	05/02/12
16	12765--EJ4		12123016	05/02/12
17	12766--D1		12123017	05/02/12
18	12767--D4		12123018	05/02/12
19	12768--D5		12123019	05/02/12
20	12769--D2		12123020	05/02/12
21	12770--D3		12123021	05/02/12
22	12771--C4		12123022	05/02/12
23	12772--C3		12123023	05/02/12
24	12773--C2		12123024	05/02/12
25	12774--C1		12123025	05/02/12
26	12775--E1		12123026	05/02/12
27	12776--E2		12123027	05/02/12
28	12777--E3		12123028	05/02/12
29	12778--E4		12123029	05/02/12
30	12779--F1		12123030	05/02/12
31	12780--F3		12123031	05/02/12
32	12781--F2		12123032	05/02/12



05/02/2012

-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

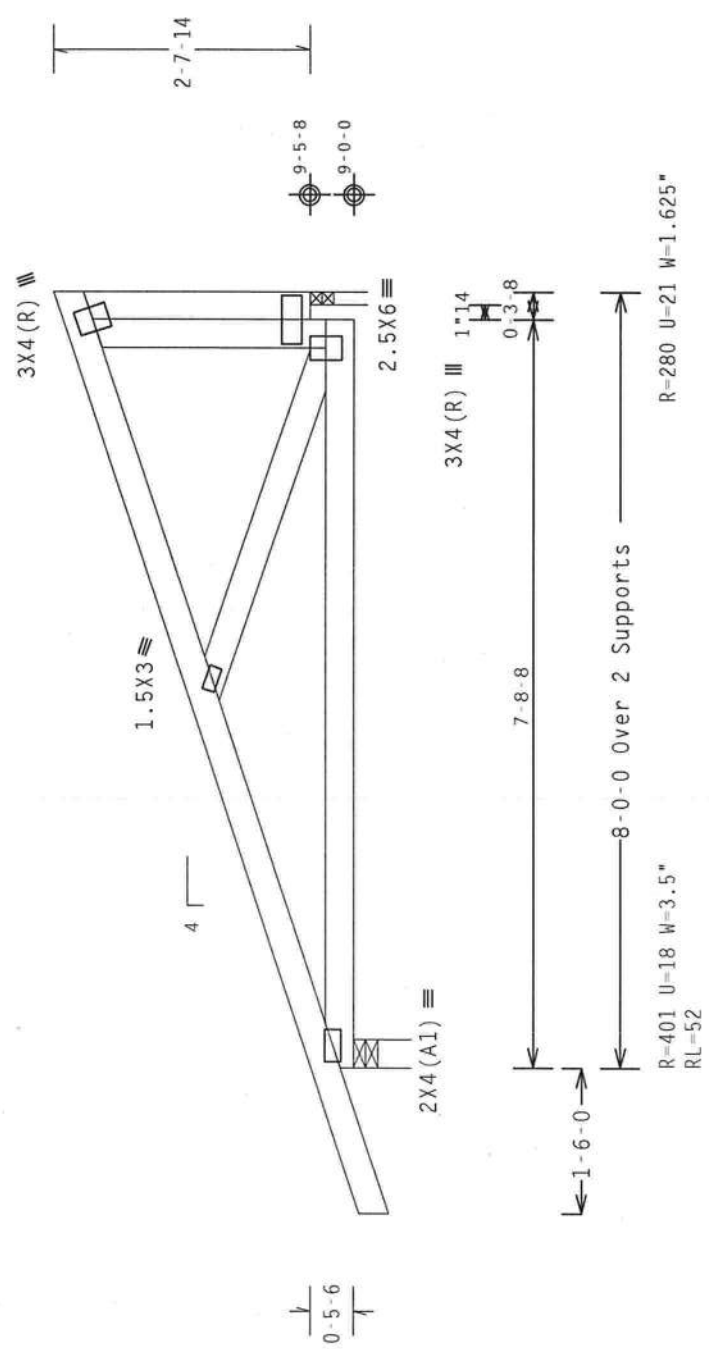


(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - EJ2)

Top chord 2x4 SP #3
Bot chord 2x4 SP #1
Webs 2x4 SP #3
:Rt Bearing Leg 2x4 SP #3:

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18
Wind loads and reactions based on MWFRS with additional C&C member design.
Bottom chord checked for 10.00 psf non-concurrent live load.
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.

Roof overhang supports 2.00 psf soffit load.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

Scale = 5" / Ft.

QTY: 12 FL / - / 5 / - / - / R / -

PLT TYP. Wave

10.03.11

10.03.11

10.03.11

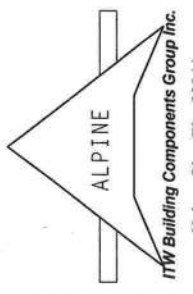
10.03.11

10.03.11

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ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



TC LL	20.0 PSF	REF	R9114- 12750
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123001
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2869
DUR.FAC.	1.25		



120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.

:Rt Bearing Leg 2x4 SP #3:

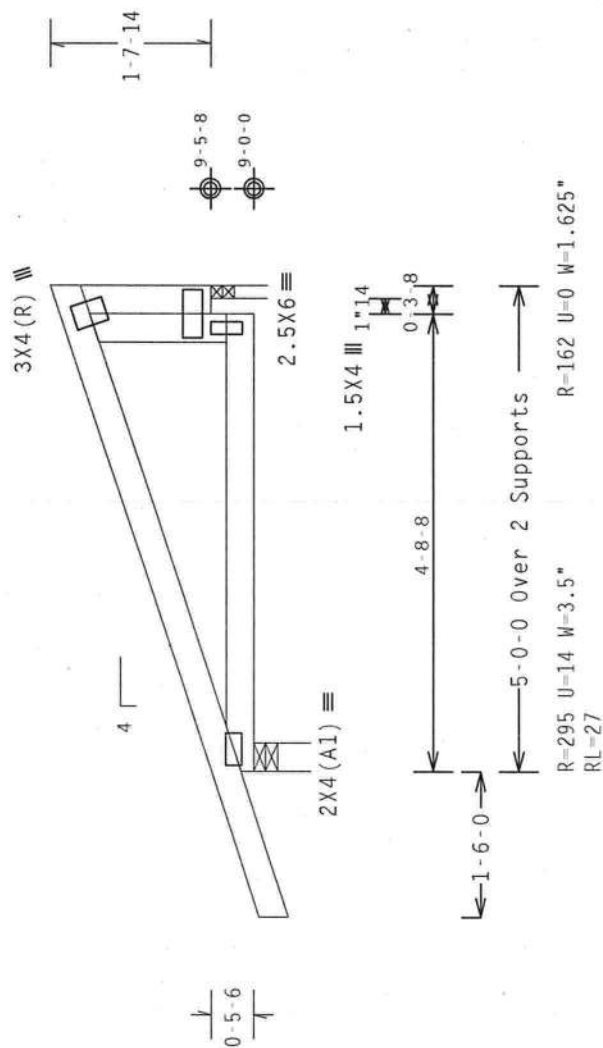
Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

MFERS loads based on trusses located at least 15.00 ft. from roof edge.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

10.03.11

QTY:7 FL/-/5/-/-/R/-

Scale = .5"/Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by IP1 and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Business noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have properly installed per BCSP sections B3, B7 or B10, as applicable.

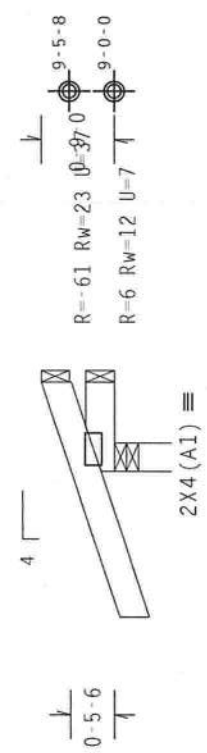
ITW Building Components Group Inc. (IBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & unloading of the truss. Apply plates to each face of truss and position as shown above and on the Joint Detail Drawing. The drawings are intended to provide information for the fabricator's reference only. The responsibility for the design of the truss, including the selection of materials, unless noted otherwise, rests solely on the Designer. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's Specification.



TC LL	20.0 PSF	REF R9114- 12751
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCUSR9114 12123002
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2870
DUR.FAC.	1.25	

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - CJ1)
Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Roof overhang supports 2.00 psf soffit load.
Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18
Wind loads and reactions based on MWFRS with additional C&C member design.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

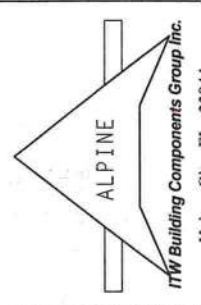


←1-6-0→
0-10-15 Over 3 Supports
R-233 U=50 W=3.5"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave	10.03.11	QTY: 4	FL/-/5/-/-/R/-	Scale = .5"/Ft.
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	TC DL	7.0 PSF	DATE	05/02/12
	BC DL	10.0 PSF	DRW	HCUSR9114 12123003
	BC LL	0.0 PSF	HC-ENG	DF/DF
	TOT.LD.	37.0 PSF	SEQN-	2871
	DUR.FAC.	1.25		

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(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - C32)

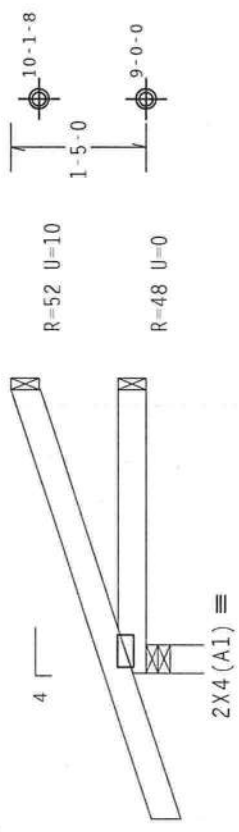
Top chord 2x4 SP #3
Bot chord 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt., ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



1-6-0 →
2-10-15 Over 3 Supports
R-232 U=24 W=3.5"
RL=24

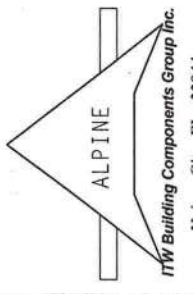
Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

QTY: 4 FL/-/5/-/-/R/- Scale = .5"/Ft.



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and VITA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Details, unless noted otherwise. Refer to drawings 160A.2 for standard plate positions. A seal on this drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's

PLT TYP. Wave



TC LL	20.0 PSF	REF	R9114- 12753
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123004
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2872
DUR.FAC.	1.25		

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - CJ3)

Top chord 2x4 SP #3
Bot chord 2x4 SP #3

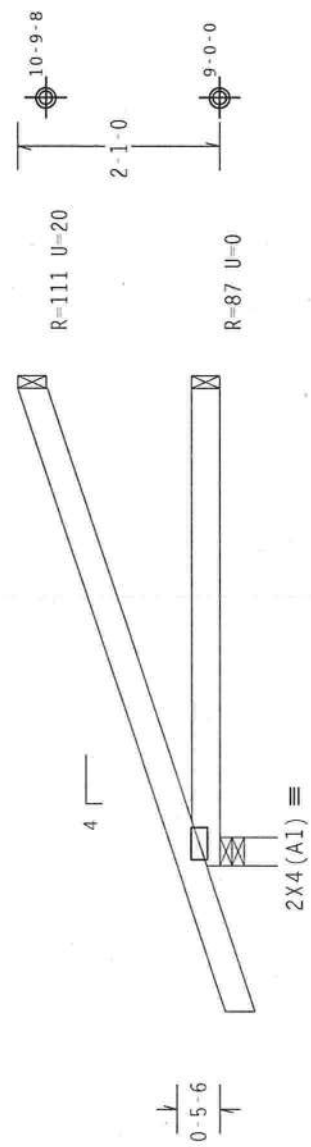
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18

Wind loads and reactions based on MWERS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



←1-6-0→

←4-10-15 Over 3 Supports →

R-294 U=18 W=3.5"

RL=35

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

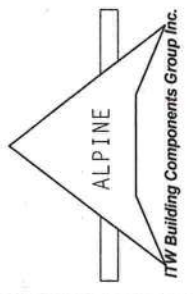
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TC LL	20.0 PSF	REF R9114- 12754
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCUSR9114 12123005
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2873
DUR.FAC.	1.25	



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

TIM Building Components Group Inc. (TIBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1606-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - EJ1)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1

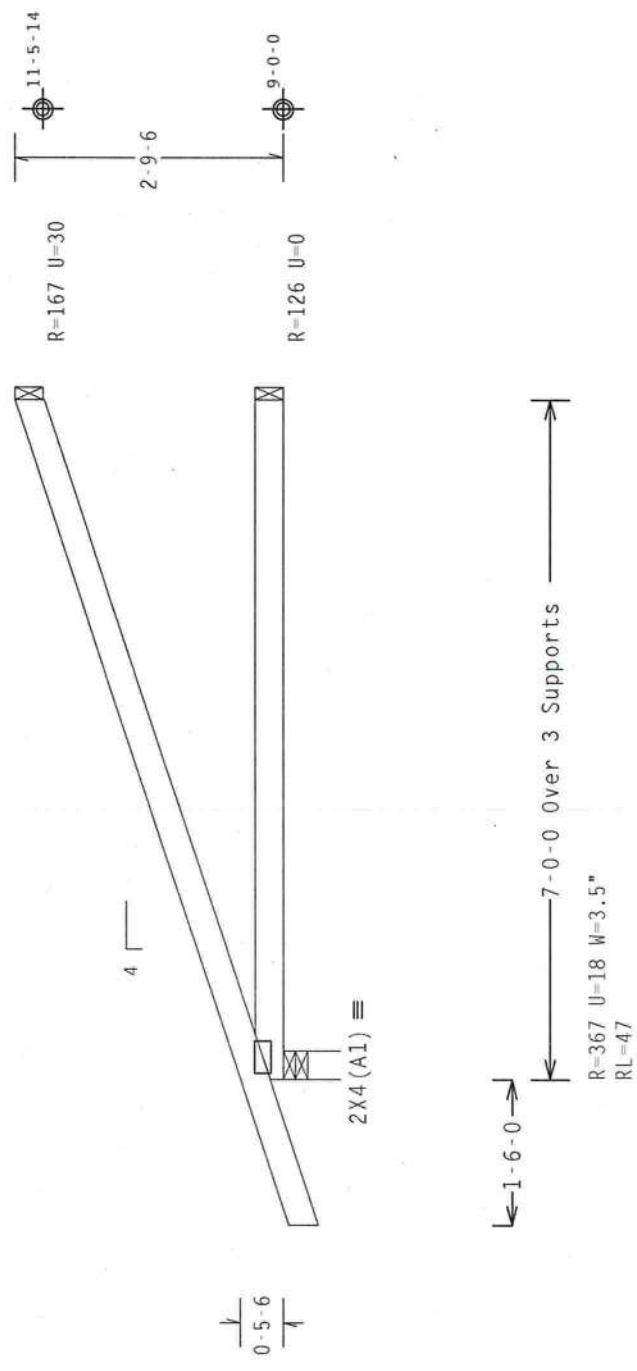
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP 8, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18

Wind loads and reactions based on MWERS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



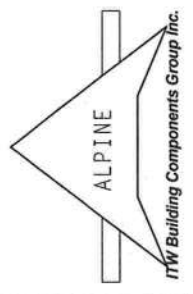
Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20%(0%)/10(0)

QTY: 12 FL/-/5/-/-R/- Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R9114- 12755
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123006
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2874
DUR.FAC.	1.25		



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ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1606-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec 9 For more information see: This job's



PLT TYP. Wave

Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Webs 2x4 SP #3

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads

----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)

TC- From 0 plf at -2.12 to 54 plf at 0.00

TC- From 2 plf at 0.00 to 2 plf at 9.84

BC- From 0 plf at -2.12 to 4 plf at 0.00

BC- From 2 plf at 0.00 to 2 plf at 9.84

TC- -38.23 lb Conc. Load at 1.33

TC- 103.98 lb Conc. Load at 4.16

BC- 221.81 lb Conc. Load at 6.99

BC- 12.06 lb Conc. Load at 1.33

BC- 96.17 lb Conc. Load at 4.16

BC- 173.14 lb Conc. Load at 6.99

Right end vertical not exposed to wind pressure.

2-1-7

9-10-1 Over 2 Supports

R=313 U-86 W=4.2

R=376 U-8

1.5X3 III

3X4

2.83

2.93

0.56

2X4 (A1)

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007 (STD)
FT/RT=20% (0%) / 10 (0)

10.03.11

QTY: 2

FL/-/5/-/-/R/-

Scale = .5" / Ft.

ALPINE

MTW Building Components Group Inc.

DOUGLAS E. EMMING
LICENSE
No. 66648
STATE OF FLORIDA
PROFESSIONAL ENGINEER

10.03.11

QTY: 2

FL/-/5/-/-/R/-

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R9114- 12756
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123007
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2897
DUR.FAC.	1.25		

Top chord 2x4 SP #1

Bot	chord	2x6	SP	#2
	Web	2x4	SP	#3

1# 1C 4V7 OM, C# 1C 4V7 2D2M

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18

pas opvol puzm

Roof overhang supports 2.00 psf soffit load.

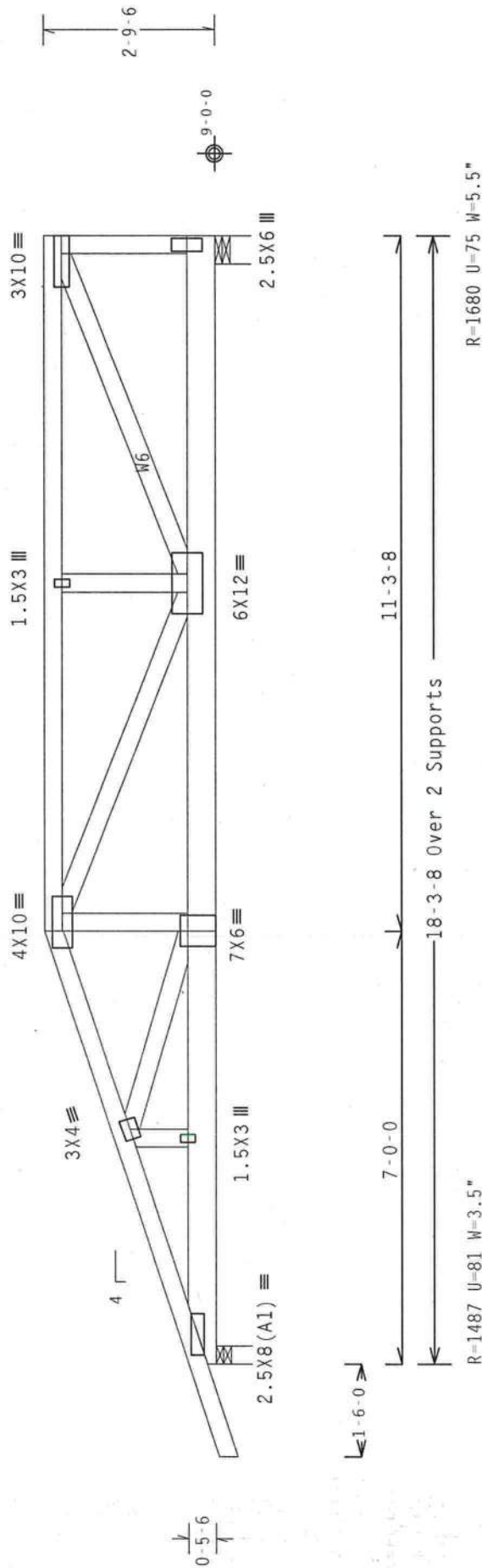
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Special loads

----- (Lumber Dur. Fac. = 1.25 / Plate Dur. Fac. = 1.25)	
TC- From	55 plf at 1.50 to 55 plf at 7.00
TC- From	27 plf at 7.00 to 27 plf at 18.29
8C- From	4 plf at 1.50 to 4 plf at 0.00
8C- From	20 plf at 0.00 to 20 plf at 7.00
8C- From	10 plf at 7.00 to 10 plf at 18.29
TC- 166.80	lb Conc. Load at 7.06, 9.06, 11.06, 13.06, 15.06, 17.06
8C- 501.94	lb Conc. Load at 7.03
8C- 126.12	lb Conc. Load at 9.06, 11.06, 13.06, 15.06, 17.06

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

10.03.11

QTY:2 FL/-/5/-/-/R/-/-
Scale =.375"/Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by IP1 and WCA) for safety practices noted or to performing these functions. Installers shall provide temporary bracing per BCSP. Unlessness mentioned otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B30, as applicable.

ITW Building Components Group Inc. (ITWBC) shall not be responsible for any deviation from this design or any failure to build the trusses in accordance with ANSI/APA 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/APA 1 Sec-2. For more information see: This job's



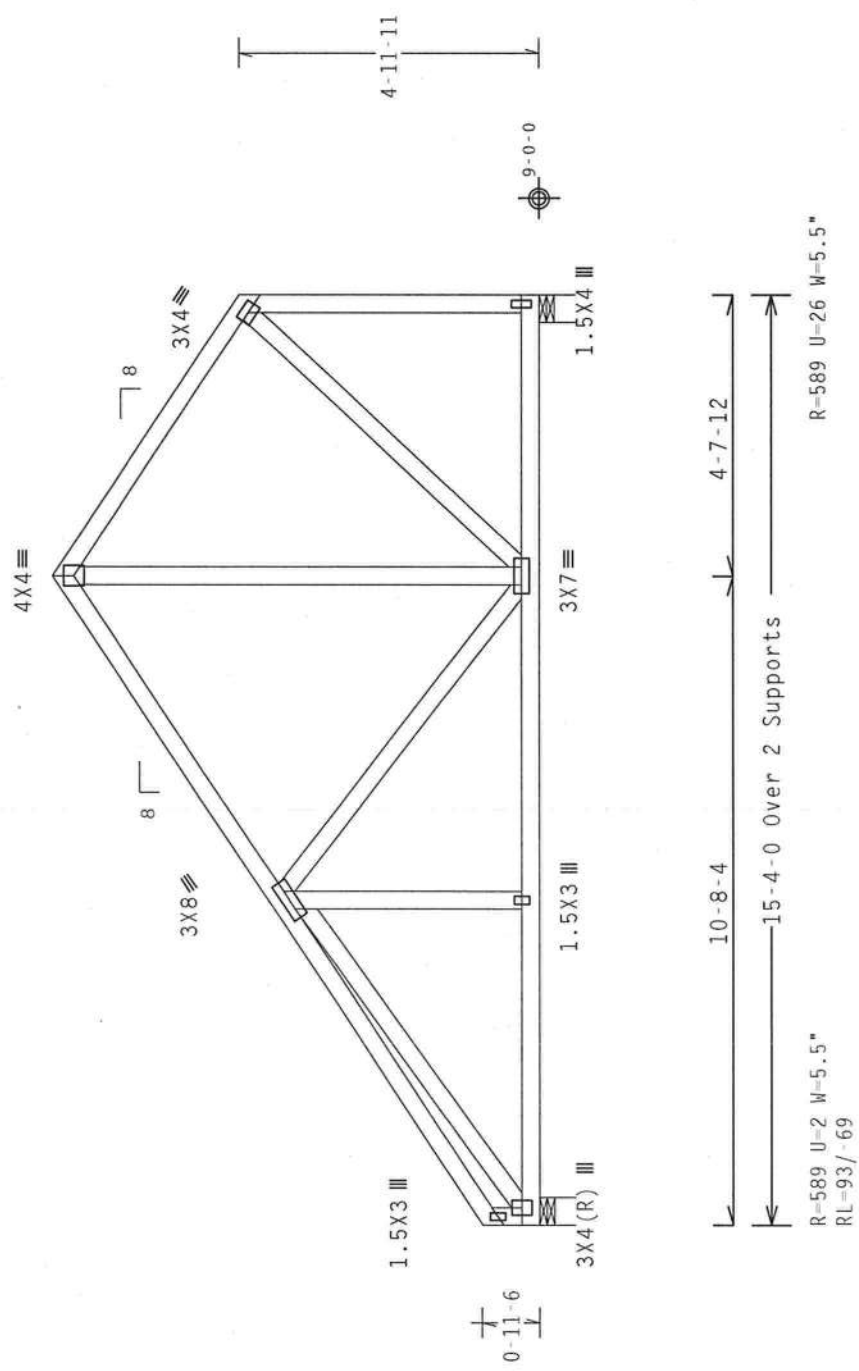
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TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123008
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2898
DUR.FAC.	1.25		

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - B3)

Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Webs 2x4 SP #3

Bottom chord checked for 10.00 psf non-concurrent live load.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18
Wind loads and reactions based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
MWFRS loads based on trusses located at least 7.50 ft. from roof edge.



PLT TYP. Wave

ALPINE
nW Building Components Group Inc.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

10.03.11

QTY: 6

FL/-/5/-/-/R/-

Scale = .3125"/Ft.

REF R9114- 12758

DATE 05/02/12

DRW HCUSR9114 12123009

HC-ENG DF/DF

SEQN- 2875

TOT.LD. 37.0 PSF

DUR.FAC. 1.25

DOUBLE FLEMING LICENSE

No. 66648

FLORIDA PROFESSIONAL ENGINEER

02 '12

IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BECI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BECI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BECI sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's

Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#3
	webs	2x4	SP	#3

120 mph wind, 15.00 ft mean hgt., ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpf (+/-)=0.18

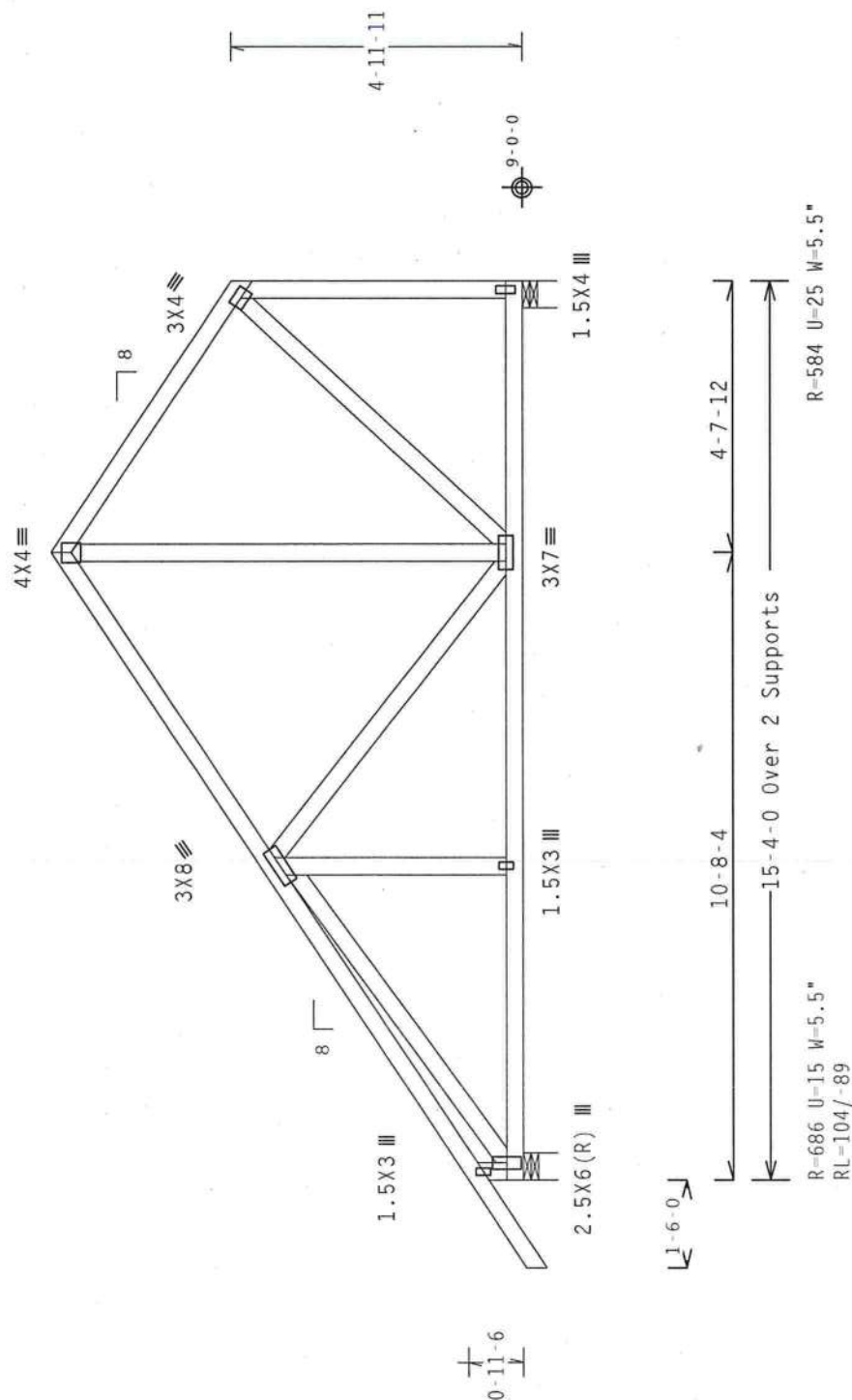
Roof overhang supports 2.00 psf soffit load.

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Right end vertical not exposed to wind pressure.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



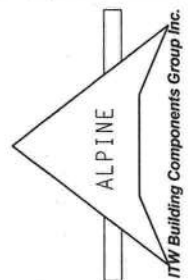
PLT TYP. Wave

Design Crit: $\text{FBC2010Res/TPI} - 2007(\text{STD})$
 $\text{FT/RT} = 20\%(0\%) / 10(0)$

10.03.11

QTY:2 FL/-/5/-/-/R/-

Scale = .3125"/Ft.



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Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety) information by TPI and WCA for safety practices noted to performing these functions. Installers shall provide temporary bracing per BCSP-1 regardless unless otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections 83, B7 or B10, as applicable.

The Building Components Group Inc. (BINGCO) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 360A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This job's

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - B1)

Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Gable end supports 8" max rake overhang.

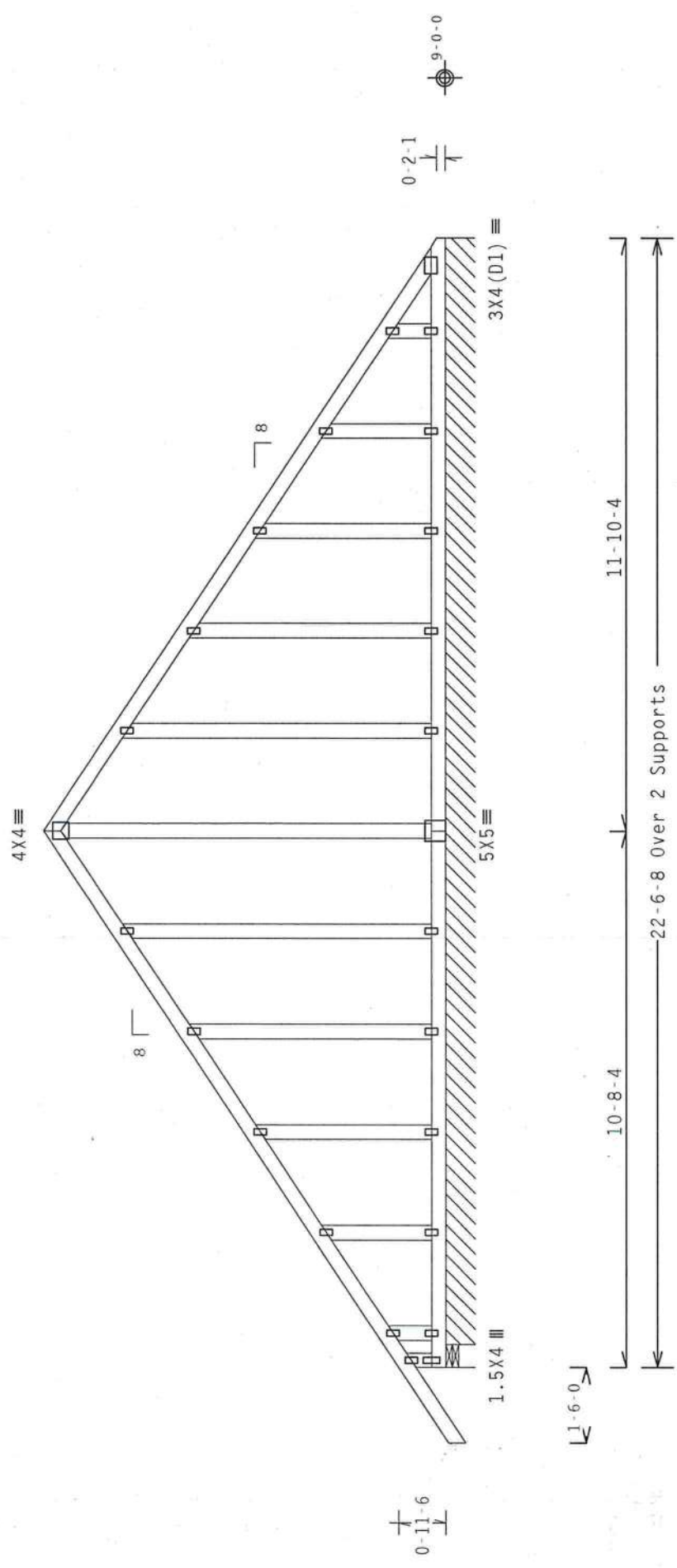
Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP 8, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-216 U-54 W-5.5"
R-73 RIE-128/ P140 W-22-1-0

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

FT/RT=20%(0%)/10(0)

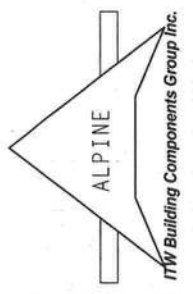
10.03.1

QTY: 1 FL/-/5/-/-/R/- Scale = .3125"/Ft.

TC LL	20.0 PSF	REF	R9114- 12760
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123011
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEON-	2877
DUR.FAC.	1.25		



****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
TPI Building Components Group Inc. (TIBCOG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ARS/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1604.2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ARS/TPI 1 Sec.2. For more information see: This Job's



Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#3
	webs	2x4	SP	#3

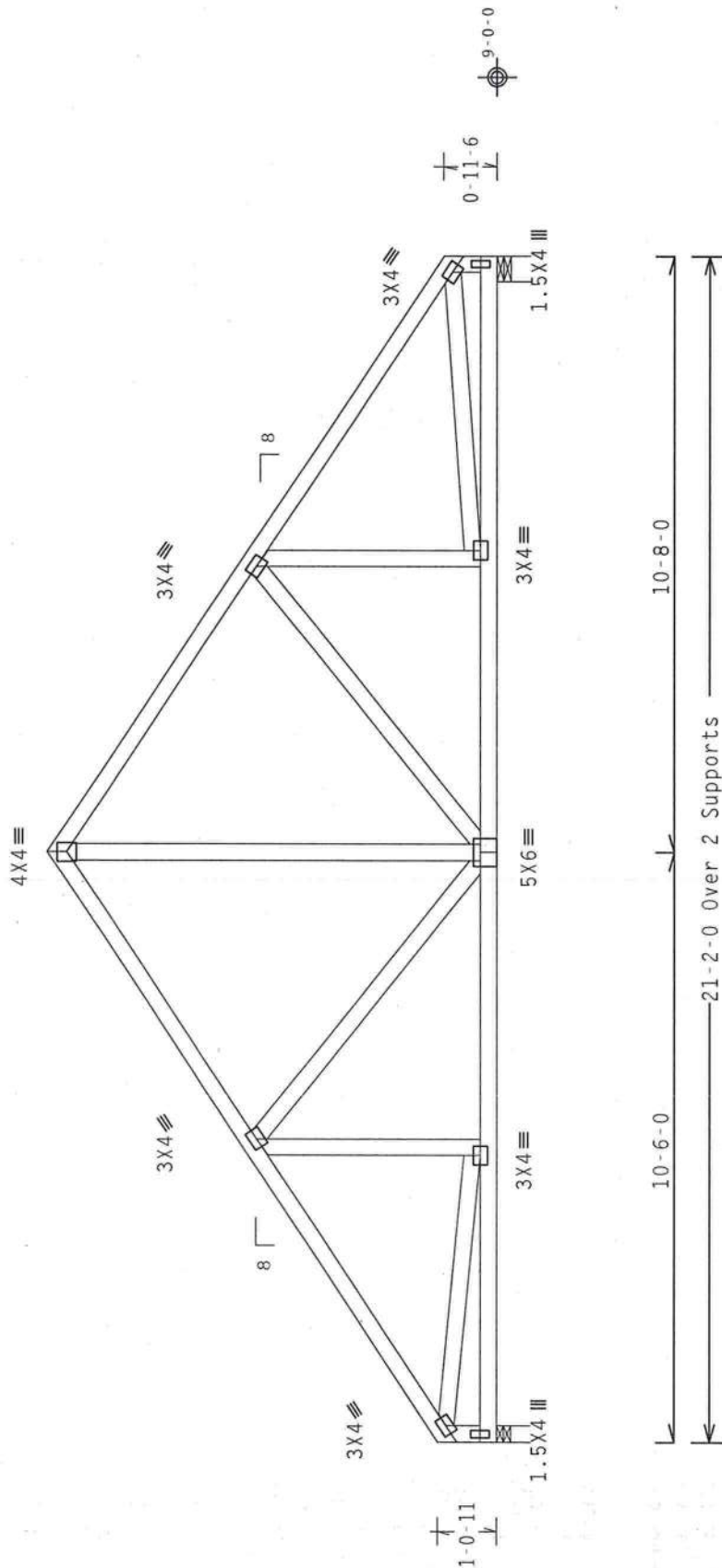
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpl (+/-)=0.18

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 15.00 ft. from roof edge.



R=813 U=0 W=3.5"
RL=112/-113

R=813 U=0 W=5.5"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

10.03.11

QTY: 4 FL/-/5/-/-/R/-/

Scale = .3125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS.

****IMPORTANT****

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and obtain the latest edition of BCSP Building Component Safety Information, by IPF and WCA for safety information pertaining to truss erection. Trusses are designed to meet specific loading conditions. The manufacturer's specifications shall govern all aspects of installation. Trusses must be installed and braced as shown for permanent lateral restraint of webs.

The Building Components Group Inc. (EUBCG) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with AHSI/TPI 1, or for handling, shipping, installation & unloading of trusses. Apply plates to each face of truss and position as shown above and on the joint. The contractor shall be responsible for all other details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per AHSI/TPI 1 Sec.2. For more information see: This job's specification.



TC LL	20.0 PSF	REF R9114- 12761
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCU8R9114 12123012
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2878
DUR.FAC.	1.25	

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - A3)

Top chord 2x4 SP #3
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5" min. nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

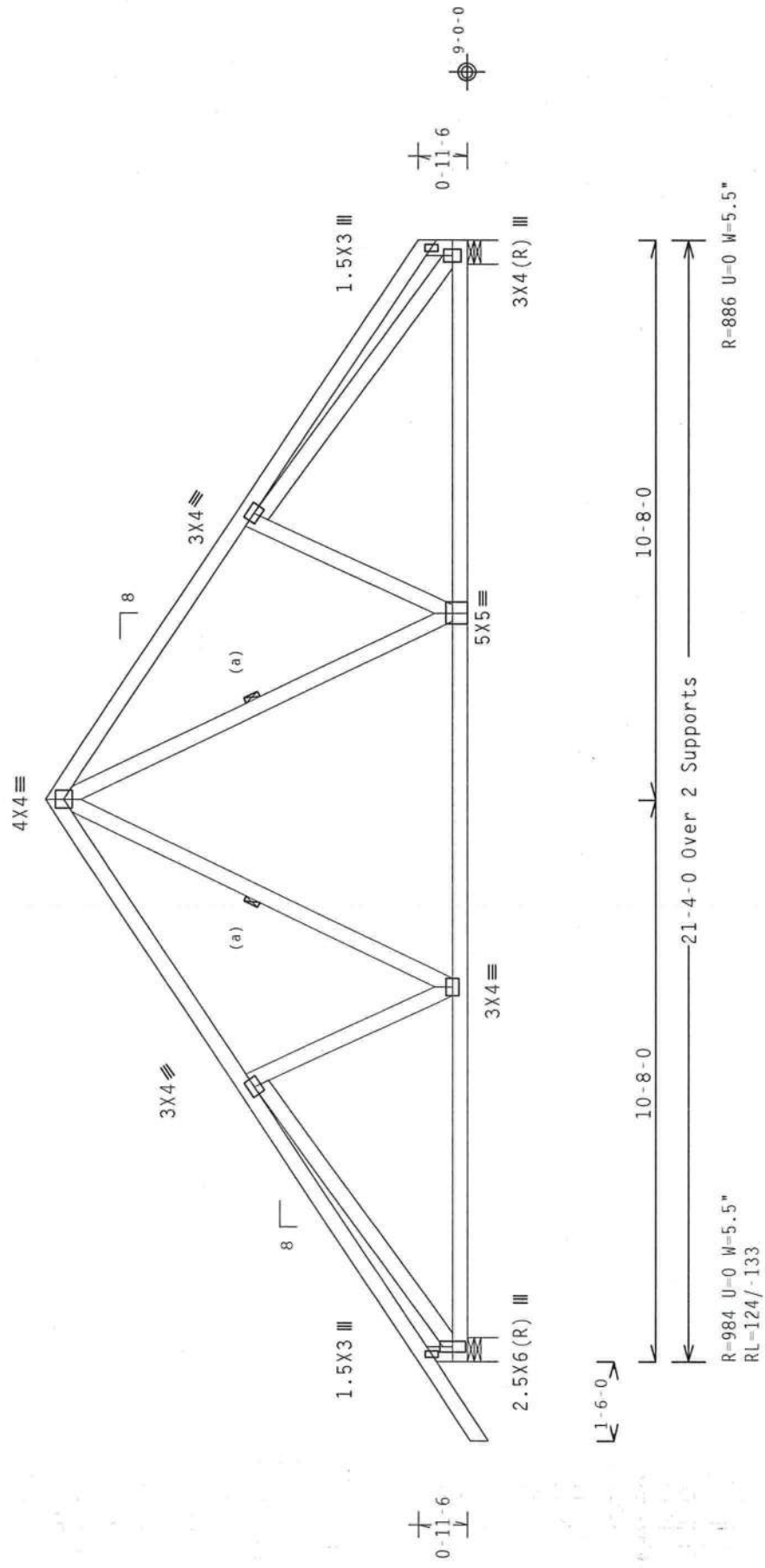
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

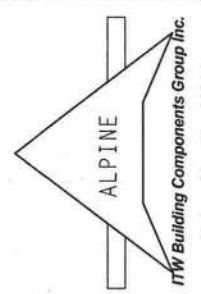
10.03.11

QTY:3

FL/-/5/-/R/-

Scale = .3125"/Ft.

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
****IMPORTANT**** Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Component Safety Information, by TPI and MCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



TC LL	20.0 PSF	REF	R9114- 12762
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123013
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN	2879
DUR.FAC.	1.25		

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - A2)

Top chord 2x4 SP #3
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SRB SPF-S or better "I" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5" min.) nails @ 6" OC.

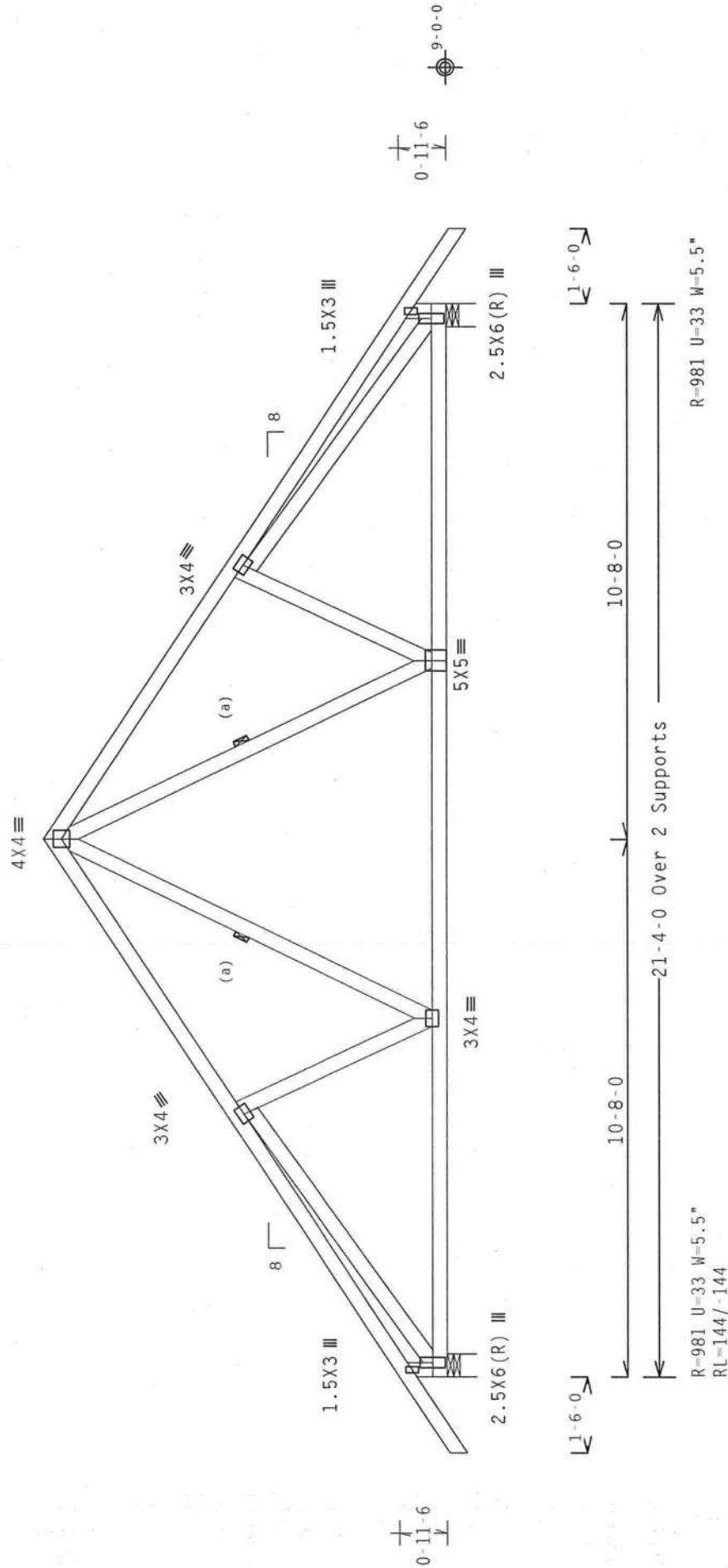
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg. Located anywhere in roof. RISK CAT II. EXP 8, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCp1 (+/-) = -0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

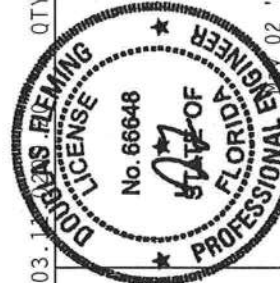
Bottom chord checked for 10.00 psf non-concurrent live load.



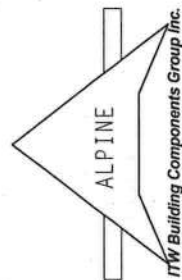
Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

QTY: 7 FL/-/5/-/-/R/- Scale = .3125" / Ft.

TC LL	20.0 PSF	REF	R9114- 12763
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123014
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT. LD.	37.0 PSF	SEQN	2880
DUR. FAC.	1.25		



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TPI Building Components Group Inc. (TIBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A.2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



PLT TYP. Wave

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpf (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

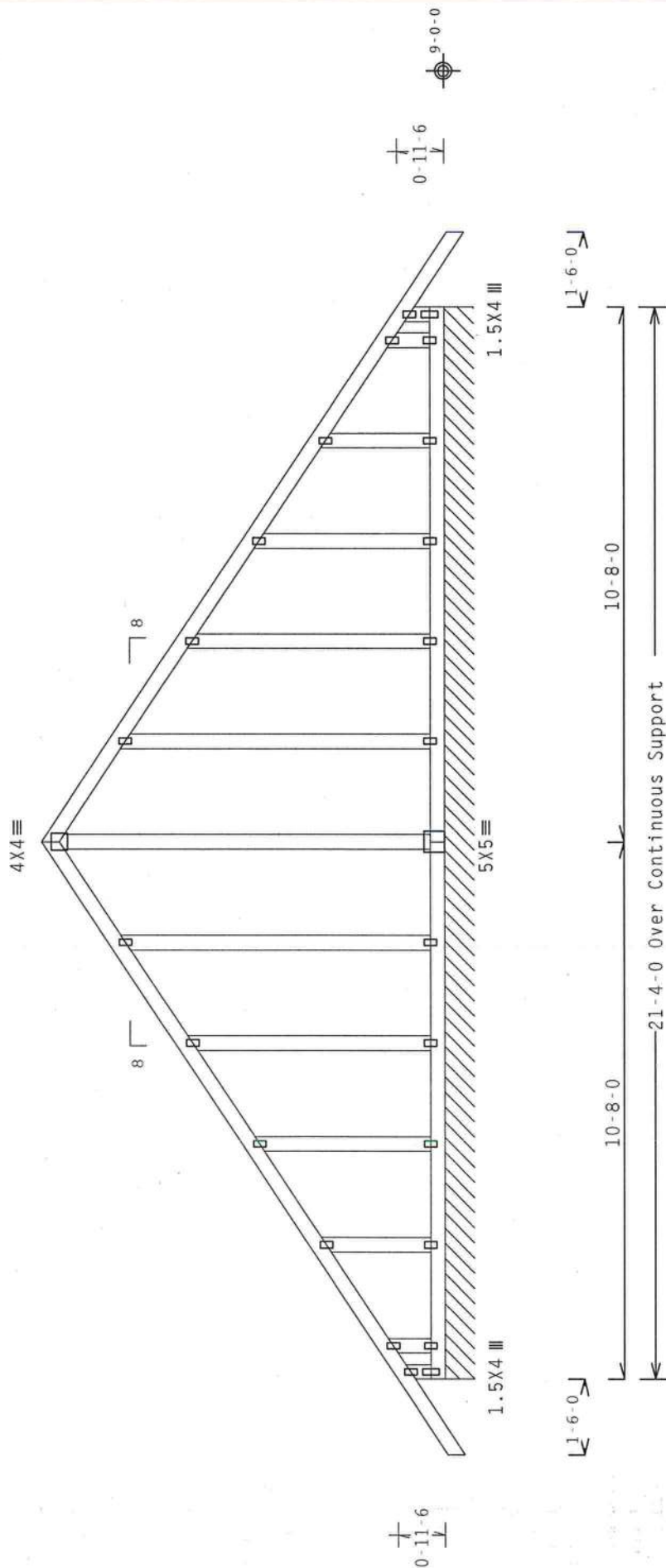
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#3
	webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Fasten rated sheathing to one face of this frame.



R=85 PLF U=3 PLF W=21-4-0
RL=7/-7 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: $\text{FBC2010Res/TPI-2007(STD)}$
 $\text{FT/RT}=20\%(0\%)/10(0)$

QTY:1 FL/-/5/-/-/R/- Scale =.3125"/Ft.

TC LL	20.0 PSF	REF R9114- 12/64
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCU9R114 12123015
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2881
DUR.FAC.	1.25	

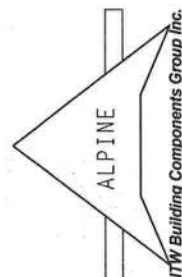


****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
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****IMPORTANT****

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety) Information by IPT and HICA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Truss installers noted otherwise are responsible for providing adequate support and ensuring no restraint of webs or chords during installation. All trusses shall comply with current code requirements and all applicable building codes. If installed over BCSP sections R3-R6, R7E, R10 - as applicable.

TM Building Components Group Inc. (TBG) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with ANSI/TPI 1, or for handling, shipping, installation & unloading of trusses. Apply plates to each face of truss and position as shown above and on the joint marking of trusses. Refer to drawings 160A-2 for standard plate positions. A seal on this metal, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: this job's drawing.



ITW Building Components Group Inc.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10. CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind SC DL=5.0 psf. Gcpl (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

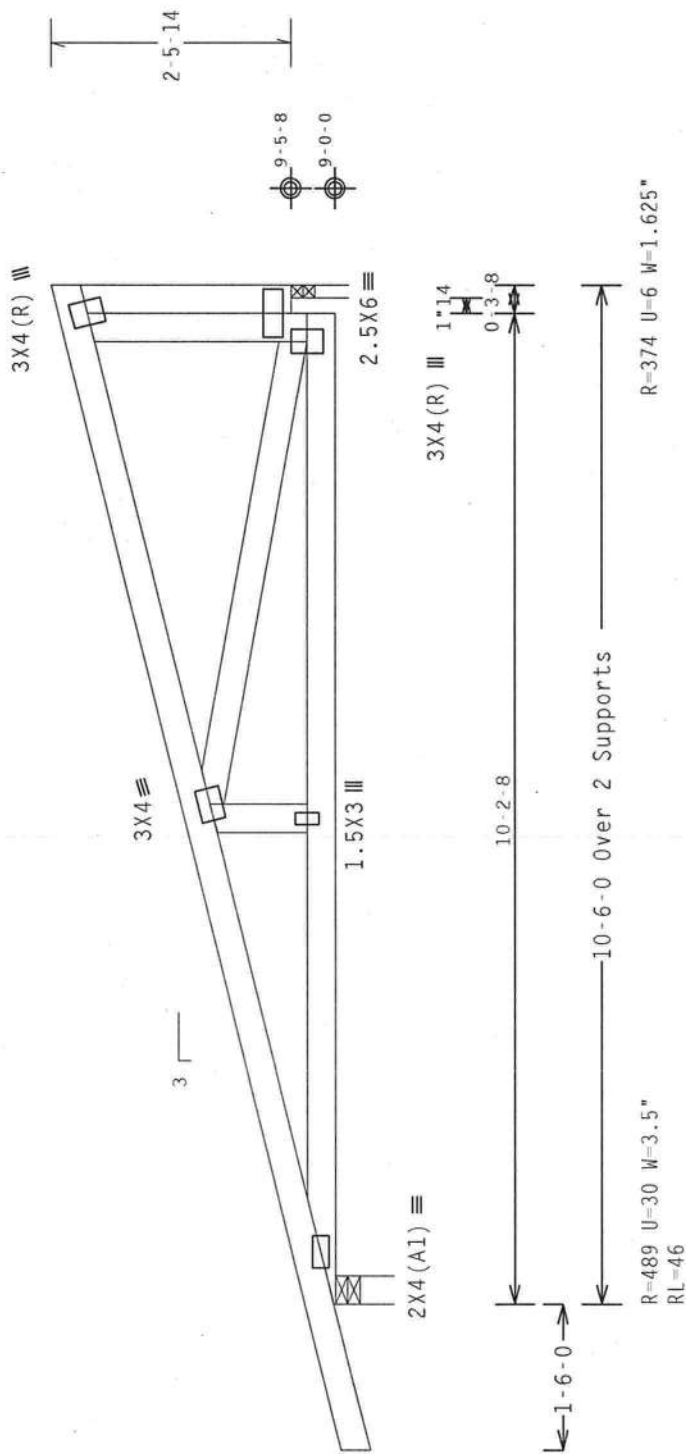
Bottom chord checked for 10.00 psf non-concurrent live load.

MWFRS loads based on trusses located at least 15.00 ft. from roof edge.

Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Webs 2x4 SP #3
:Rt Bearing Leg 2x4 SP #3:

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: $\text{FBC2010Res/TPI-2007(STD)}$
 $\text{FT/RT}=20\%(0\%)/10(0)$

QTY:14 FL/-/5/-/-/R/-
Scale =.5"/Ft.

TC LL	20.0	PSF	REF R9114- 12765
TC DL	7.0	PSF	DATE 05/02/12
BC DL	10.0	PSF	DRW HCUSR9114 12123016
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	37.0	PSF	SEQN- 2882
DUR.FAC.	1.25		



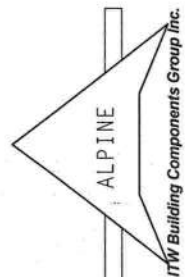
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET.
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trosses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow all instructions, drawings, specifications, safety information, BIPV and WCA, for safety practices noted otherwise. Installers shall provide temporary bracing per BCSP.

The manufacturer does not warrant or accept responsibility for permanent lateral restraint of webs. The chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (IBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1- or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 For standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the user.

****IMPORTANT****



Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

Gable end supports 8" max rake overhang.

Bottom chord checked for 10.00 psf non-concurrent live load.

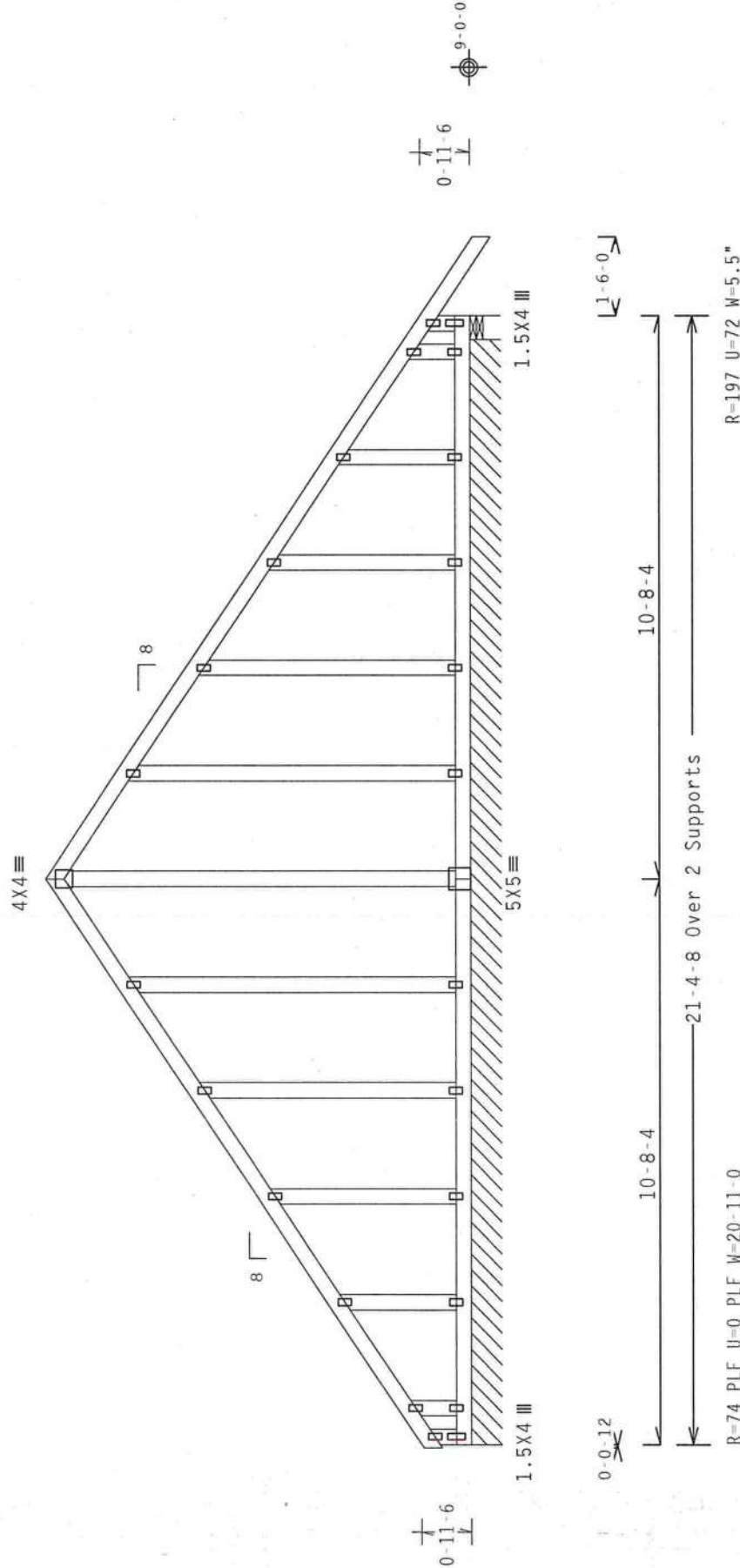
Fasten rated sheathing to one face of this frame.

MMFRS loads based on trusses located at least 30.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design. See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



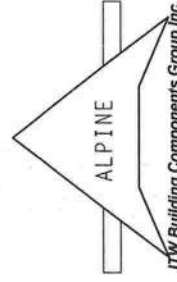
Note: All Plates Are 1.5X3 Except As Shown.
Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

QTY: 1	FL/-/5/-/-/R/-	Scale = .3125"/Ft.
TC LL	20.0 PSF	REF R9114- 12766
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCUSR9114 12123017
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2883
DUR.FAC.	1.25	



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ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint. Details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec 2. For more information see: This job's



Top	chord	2x4	SP	#1
Bot	chord	2x4	SP	#1
	Web's	2x4	SP	#3

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Calculated horizontal deflection is 0.22" due to live load and 0.31" due to dead load.

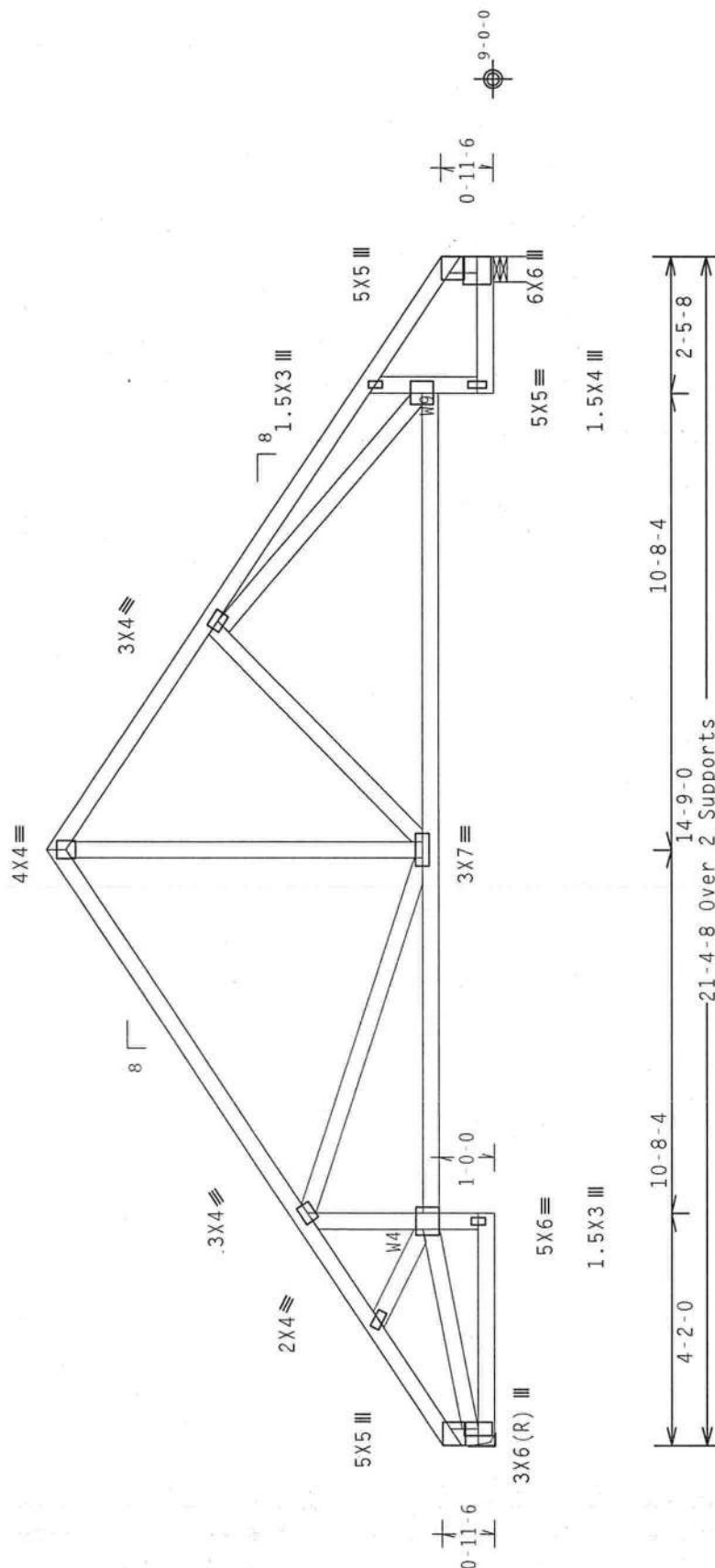
Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Wind loads and reactions based on MWFRS with additional C&C member design.

(H1) = (J) Hanger not calculated (2)2x6 SP #2 supporting member.

MMWFRS Loads based on trusses located at least 30.00 ft. from roof edge.



R=821 U=0
RL=114/-11

R-821 U=0 W=5.5"

Design Crit: $\text{FBC2010Res/TPI} - 2007(\text{STD})$
 $\text{FT/RT} = 20\%(0\%) / 10(0)$

Scale = .3125" / Ft.

10.03.11

QTY:3 FL/-/5/-/-/R/-/

Scale = .3125"/Ft.
REF R9114- 12767

Scale = .3125" / Ft

Scale = .3125"/Ft.

REF	R9114-	12767
DATE	05/02/12	
DRW	HCUSR9114	12123018

Scale = .3125
REF R9114 -
DATE 05/03
DRW HCUSR9114
HC-ENG DF/DF
SEON - 2884

FL / - / 5 / - / - / R /	
TC LL	20.0
TC DL	7.0
BC DL	10.0
BC LL	0.0
TOT.LD.	37.0
DUR.FAC.	1.25

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ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design and shall have tracing installed per UBCS sections 632, or 6302, as applicable.

ALPINE

ITW Building Components Group Inc.

Top chord 2x4 SP #3
Bot chord 2x4 SP #1
Webs 2x4 SP #3

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SRB SPF-S or better 1" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

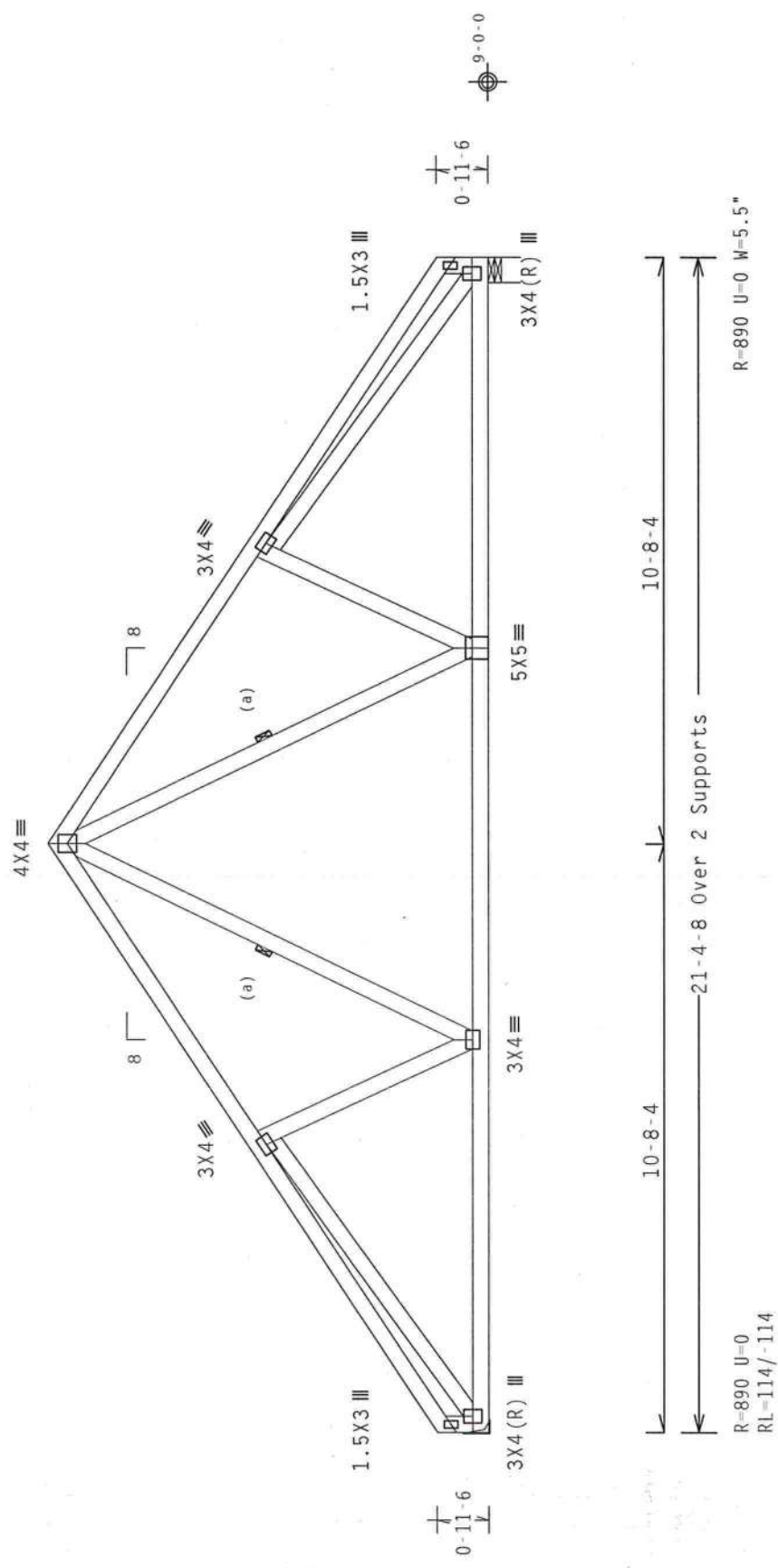
Bottom chord checked for 10.00 psf non-concurrent live load.

MFERS loads based on trusses located at least 30.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI (+/-)=0.18

Wind loads and reactions based on MFERS with additional C&C member design. Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=890 U=0 W=5.5"

R=890 U=0
RL=114/-114

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

Scale = .3125"/Ft.

REF	R9114-	12768
DATE	05/02/12	
DRW	HCUSR9114	12123019
HC-ENG	DF/DF	
SEQN	2885	

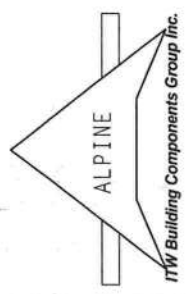


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TPI Building Components Group Inc. (TPI/BCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's

PLT TYP. Wave



(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - D2)

Top chord 2x4 SP #3
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

(H1) = (J) Hanger not calculated (2)2x6 SP #2 supporting member.

Bottom chord checked for 10.00 psf non-concurrent live load.

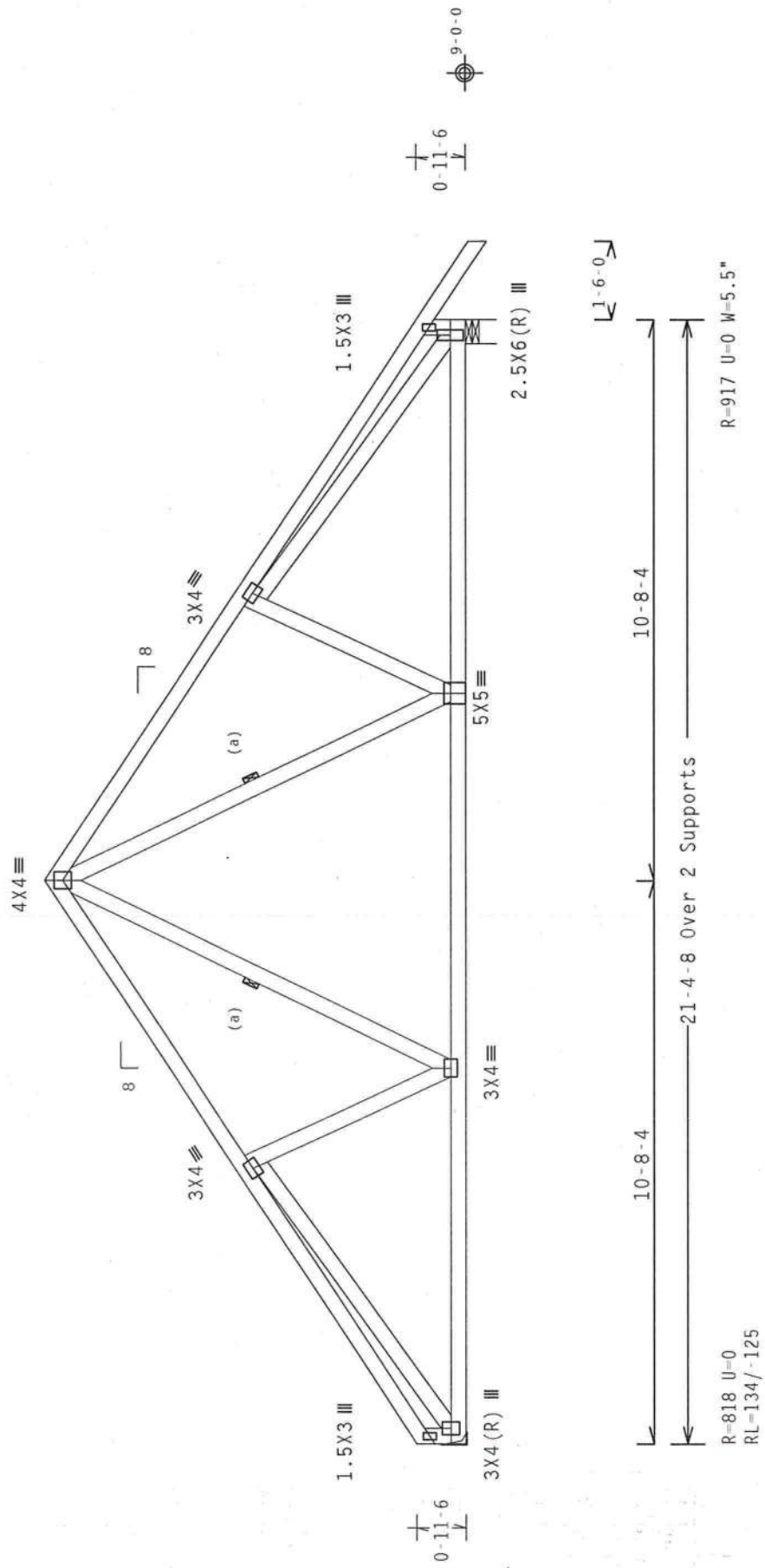
MWFRS loads based on trusses located at least 30.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP 8, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SRB SPF-S or better "I" brace, 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave	QTY: 1		FL / - / 5 / - / - / R / -	Scale = .3125" / Ft.	
	TC LL	20.0 PSF	REF	R9114-	12769
	TC DL	7.0 PSF	DATE	05/02/12	
	BC DL	10.0 PSF	DRW	HCUSR9114	12123020
	BC LL	0.0 PSF	HC-ENG	DF/DF	
TOT.LD.			37.0 PSF	SEQN-	2886
DUR.FAC.			1.25		

DOUGLAS FLEMING
LICENSE
No. 66648
FLORIDA
PROFESSIONAL ENGINEER

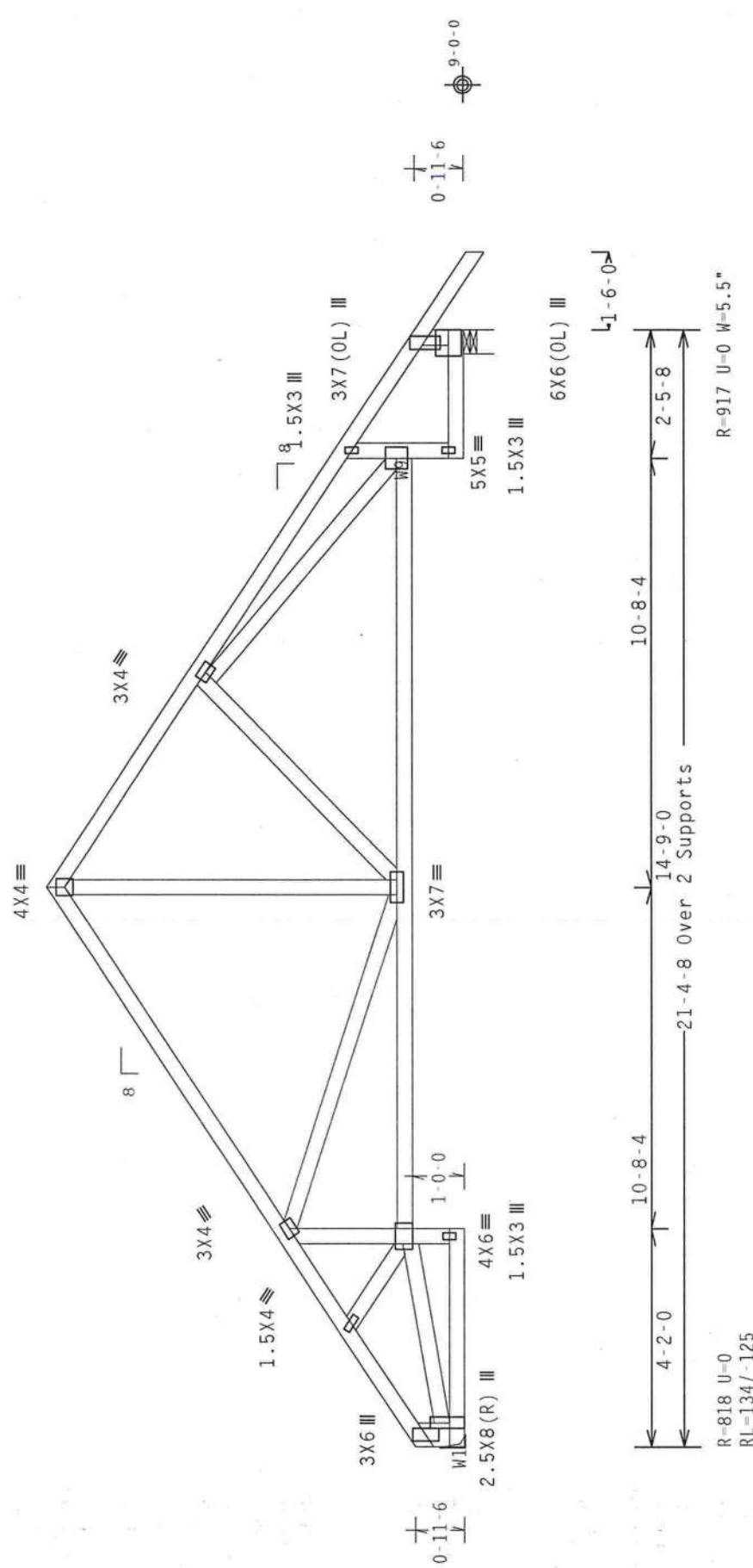
ALPINE
Building Components Group Inc.

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TPI Building Components Group Inc. (TIBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this design, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - D3)

Top chord 2x4 SP #1
Bot chord 2x4 SP #1
Webs 2x4 SP #3 :W1 2x6 SP #2: :W9 2x4 SP #1:
Roof overhang supports 2.00 psf soffit load.
Calculated horizontal deflection is 0.19" due to live load and 0.26" due to dead load.
Bottom chord checked for 10.00 psf non-concurrent live load.
MHFRS loads based on trusses located at least 30.00 ft. from roof edge.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge. RISK CAT II. EXP B. wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpl(+/-)=0.18
Wind loads and reactions based on MHFRS with additional C&C member design.
(H1) = (J) Hanger not calculated (2)2x6 SP #2 supporting member.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave	QTY:1	FL/-/5/-/-/R/-	Scale = .3125"/Ft.
ALPINE	TC LL	20.0 PSF	REF R9114- 12770
	TC DL	7.0 PSF	DATE 05/02/12
	BC DL	10.0 PSF	DRW HCUSR9114 12123021
	BC LL	0.0 PSF	HC-ENG DF/DF
	TOT.LD.	37.0 PSF	SEON- 2887
	DUR.FAC.	1.25	

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

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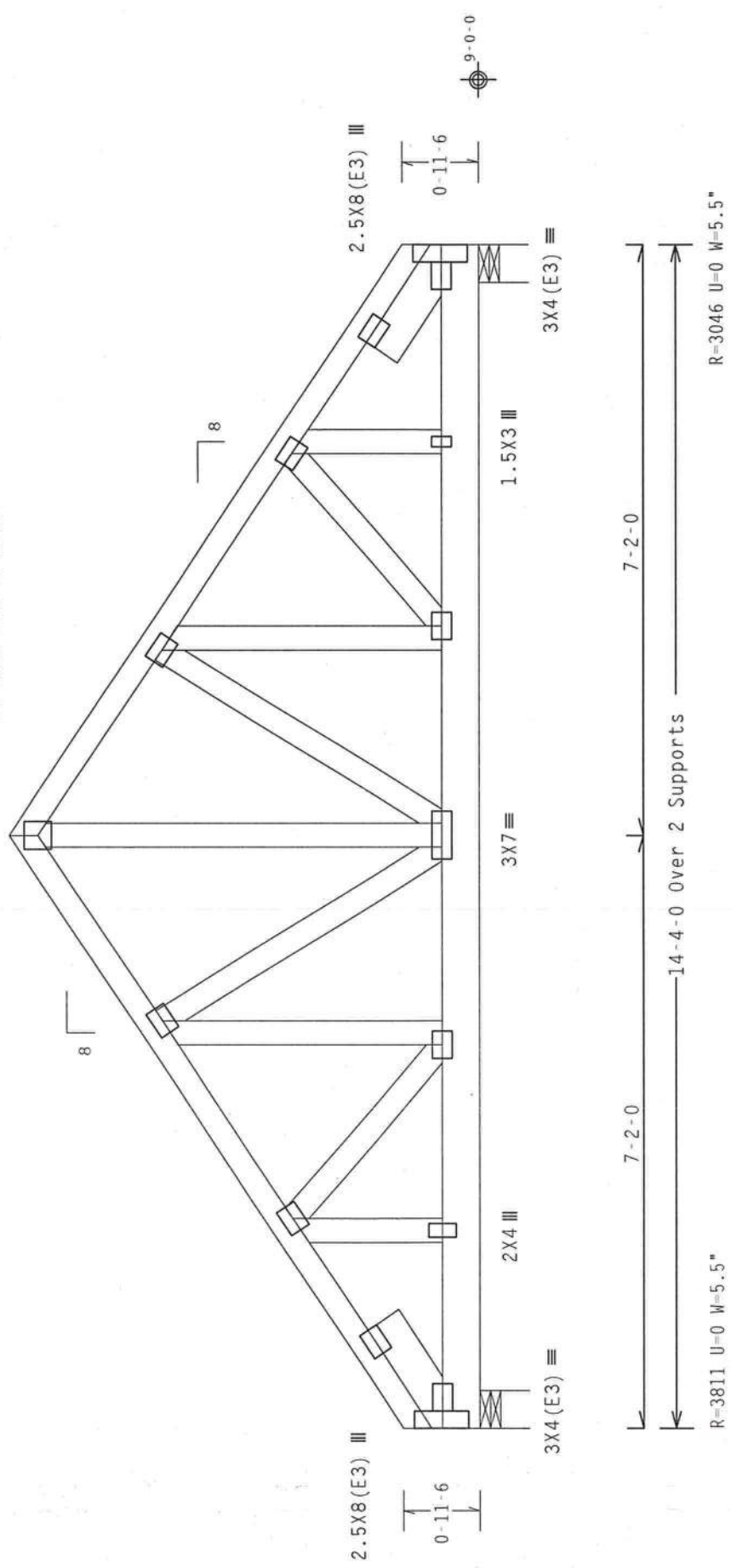
THE Building Components Group Inc. (TIBGG) shall not be responsible for any deviation from this design any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this design. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's

Top chord 2x4 SP #3
Bot chord 2x6 SP #2
Webs 2x4 SP #3
:Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

Special loads
----- (Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 57 pif at 0.00 to 57 pif at 7.17
TC- From 57 pif at 7.17 to 57 pif at 14.33
BC- From 10 pif at 0.00 to 10 pif at 12.27
BC- From 20 pif at 12.27 to 20 pif at 14.33
BC- 889.66 lb Conc. Load at 0.69, 2.27
BC- 821.16 lb Conc. Load at 4.27, 6.27, 8.27
BC- 817.92 lb Conc. Load at 10.27, 12.27

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 3.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18
Wind loads and reactions based on MWERS with additional C&C member design.
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 3x4 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

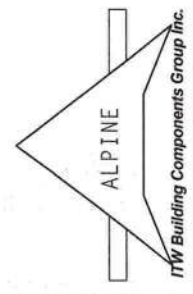
PLT TYP. Wave

QTY: 1 FL/- /5/- /- /R/-

Scale = 5"/Ft.

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
TTP Building Components Group Inc. (TTPBG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 100A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's

TC LL	20.0 PSF	REF	R9114- 12771
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123022
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2899
DUR.FAC.	1.25		



Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#3
	webs	2x4	SP	#3

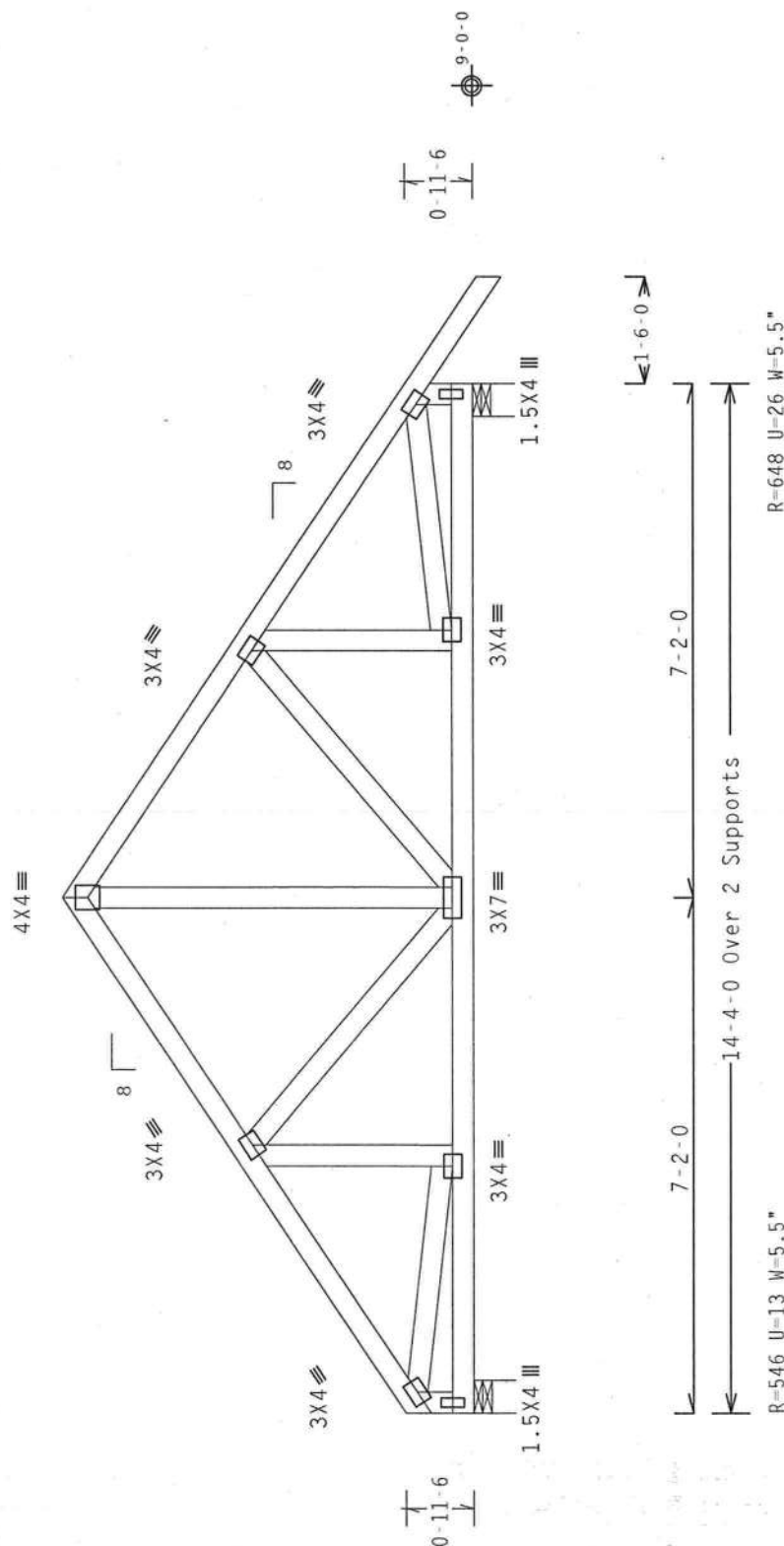
Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180' total load. Creep increase factor for dead load is 1.50.



R=546 U=13 W=5.5"
RL=96/-87

R=648 U=26 W=5.5"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

10.03.11

QTY:3 FL/-/5/-/-/R/-

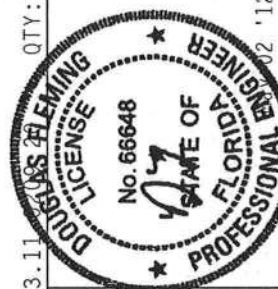
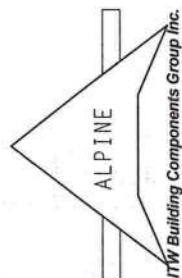
$$\text{Scale} = .375" / \text{Ft.}$$

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
IMPORTANT****

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BECS (Building Component Safety Information, by IPI and WGA) for safety specifications and recommendations pertaining to trusses.

Trusses shall be properly braced to maintain their structural integrity. All structural sheathing and bottom chord members shall have a properly attached rigid ceiling. Locations for permanent lateral restraint of webs shall have bracing installed per BECS, B7 or B10, as applicable.

ITT Building Components Group Inc. (ITBGC) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ANSI/APA 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings I60A-7 for standard plate positions. A seal on this drawing or cover page listing this drafting, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/APA 1 Sec.2. For more information see: This Job's



TC LL	20.0	PSF	REF R9114- 12772
TC DL	7.0	PSF	DATE 05/02/12
BC DL	10.0	PSF	DRW HCUSR9114 12123023
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	37.0	PSF	SEQN- 2888
DUR.FAC.	1.25		

Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

120 mph wind, 15.00 ft mean hgt., ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.

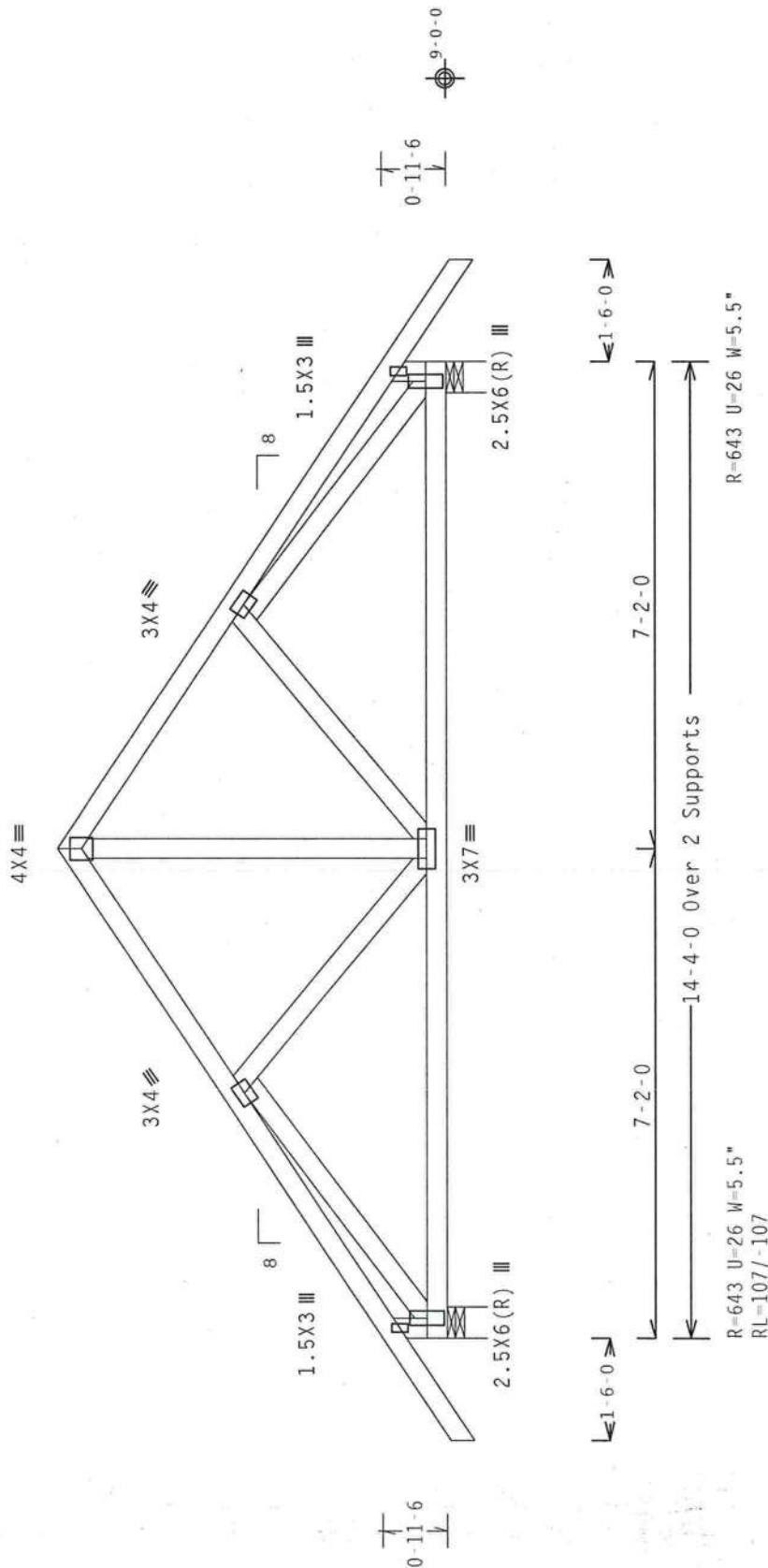
GCp1 (+/-) = 0.18

Roof overhang supports 2.00 psf soffit load.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)

10.03.11

QTY:2 FL/-/5/-/-/R/-/

Scale = .375"/Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**
 FURNISHED THIS DESIGN FOR A CONTRACTOR. NOT FOR THE INSTALLER.

Trusses, require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by TPI and WCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. Practices noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Ceilings shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

TM Building Components Group Inc. (TMBCG) shall not be responsible for any deviation from this design or any failure to build the truss in accordance with AISC/TPI 1, or for handling, shipping, installation & any prancing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings TM0-7 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer. See AISC/TPI 1, Sec. 2, for more information. See: This job's

ITW Building Components Group Inc.



DUR. FAC. 1.25

10

1

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - C1)

Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

See DWGS A12015ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Fasten rated sheathing to one face of this frame.

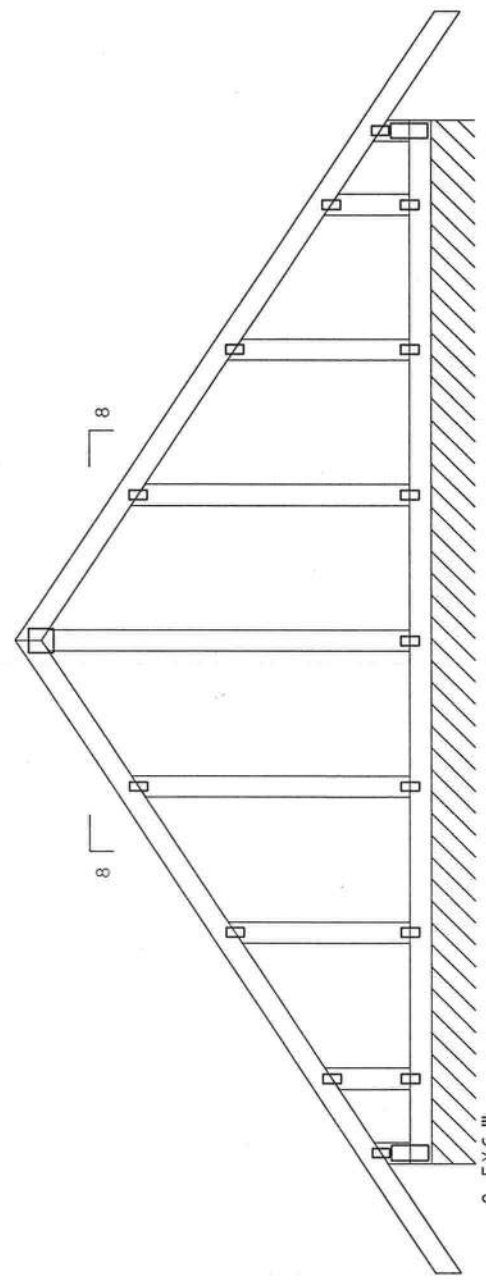
120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Wind loads and reactions based on MWERS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

4X4



0-11'-6"

0-11'-6"



1'-6-0"

7'-2-0"

7'-2-0"

1'-6-0"

14'-4-0" Over Continuous Support

R=90 PLF U=4 PLF W=14-4-0
RL=7/-7 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI-2007(STD)

FT/RT=20%(0%)/10(0)

10.03.11

QTY: 1 FL/-/5/-/-/R/-

Scale = .375"/Ft.

REF R9114- 12774

DATE 05/02/12

DRW HCUSR9114 12123025

HC-ENG DF/DF

SEQN- 2890

DUR.FAC. 1.25

TOT.LD. 37.0 PSF

BC LL 0.0 PSF

BC DL 10.0 PSF

TC DL 7.0 PSF

TC LL 20.0 PSF

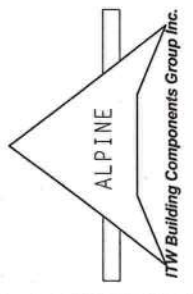
PLT TYP. Wave

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET

FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and UTCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.

ITG Building Components Group Inc. (ITGBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ABSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A.2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ABSI/TPI 1 Sec.2. For more information see: This job's



Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#3
	webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

Bottom chord checked for 10.00 psf non-concurrent live load.

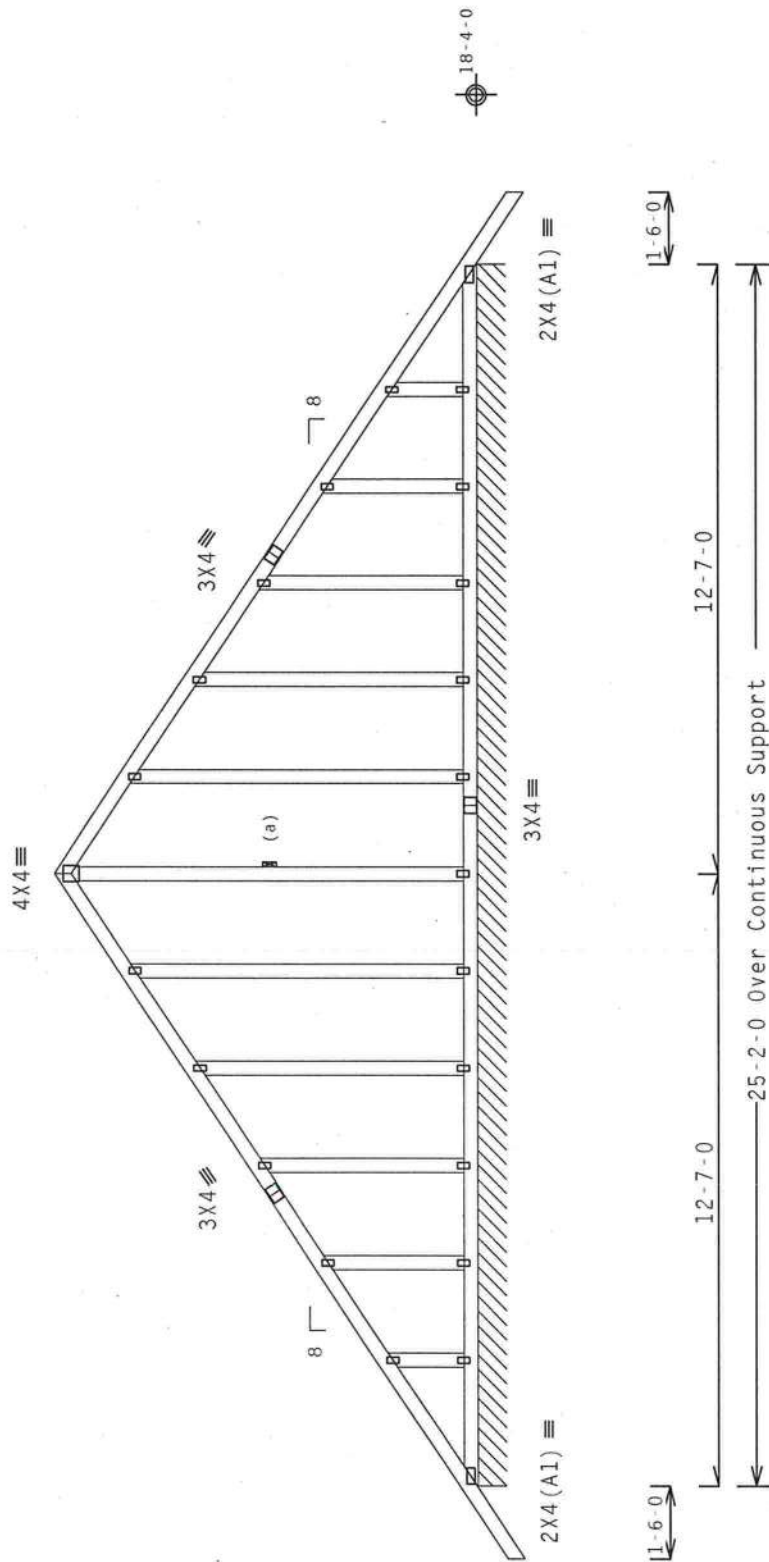
WWFRS loads based on trusses located at least 22.40 ft. from roof edge.

120 mph wind, 22.40 ft mean hgt., ASCE 7-10, CLOSED bldg., not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpl (+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-84 PLF U=0 PLF W=25-2-0
RL=7/-7 PLF

Note: All Plates Are 1.5X3 Except As Shown.

Design Crit: FBC2010Res/TPI -2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP. Wave

10.03.11

QTY:2 FL/-/5/-/-/R/-/

Scale = .25"/Ft.

IMPORTANT**
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FORNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by TPI and WTA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSP. If trusses are not otherwise noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B2 or B10, as applicable.

The Building Components Group Inc. (BCEG) shall not be responsible for any deviation from this design or specification which may result in any failure to build the truss in accordance with ANSI TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-7 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



TC LL	20.0 PSF	REF R9114- 12775
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCUR9114 12123026
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2891
DUR.FAC.	1.25	

Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SP8 SPF-S or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

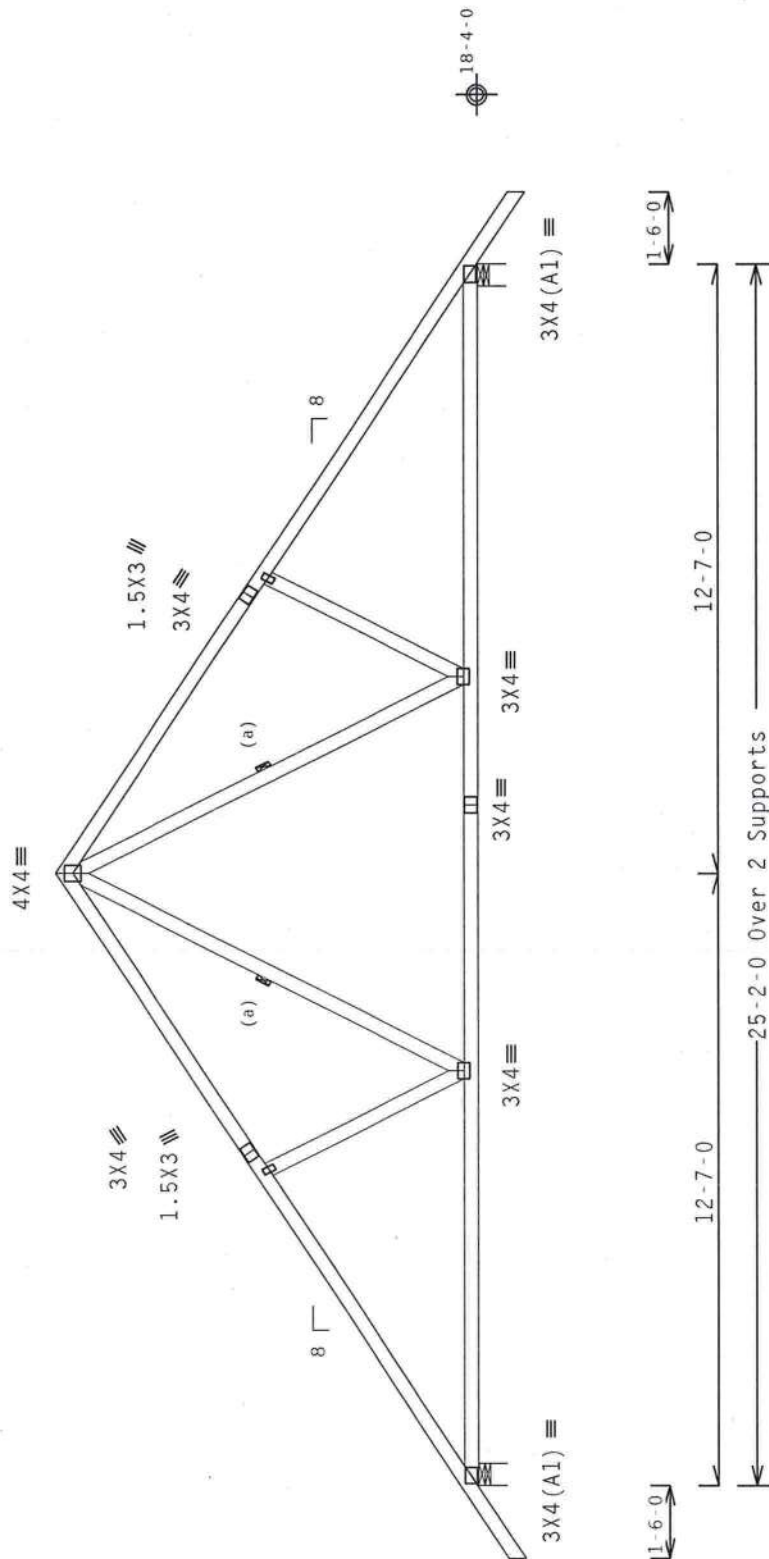
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

120 mph wind, 22.40 ft mean hgt., ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psi. GCPl (+/-) = 0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.



R=1145 U=127 W=5.5"
RL=178/-178

R=1146 U=127 W=5.5"

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

PLT TYP.: Wave

10.03.11:4209.20 QTY:8 FL/-/5/-/-/R/-

Scale = .25"/Ft.

TC LL	20.0	PSF	REF R9114- 12776
TC DL	7.0	PSF	DATE 05/02/12
BC DL	10.0	PSF	DRW HCUSR9114 12123027
BC LL	0.0	PSF	HC-ENG DF/DF
TOT.LD.	37.0	PSF	SEQN- 2892
DUR.FAC.	1.25		

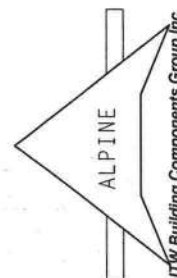


****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET.**
FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS.

****IMPORTANT****

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety) information, by IP1 and MICA for safety practices prior to performing any work. Trusses are designed to be installed by experienced installers. The bottom chord shall have a properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations for permanent lateral restraint of webs should have bracing installed per BCSP sections, B3, B7 or B10, as applicable.

The Building Components Group Inc. (BCEGCO) shall not be responsible for any deviation from this design and any failure to build the truss in conformance with ABCS/IP1 or, for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings, I600-2 For standard plate positions. A seal on this framing or cover plate fixing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is



Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SR8 SPF-S or better "T" brace. 80% length of web member. Attached with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

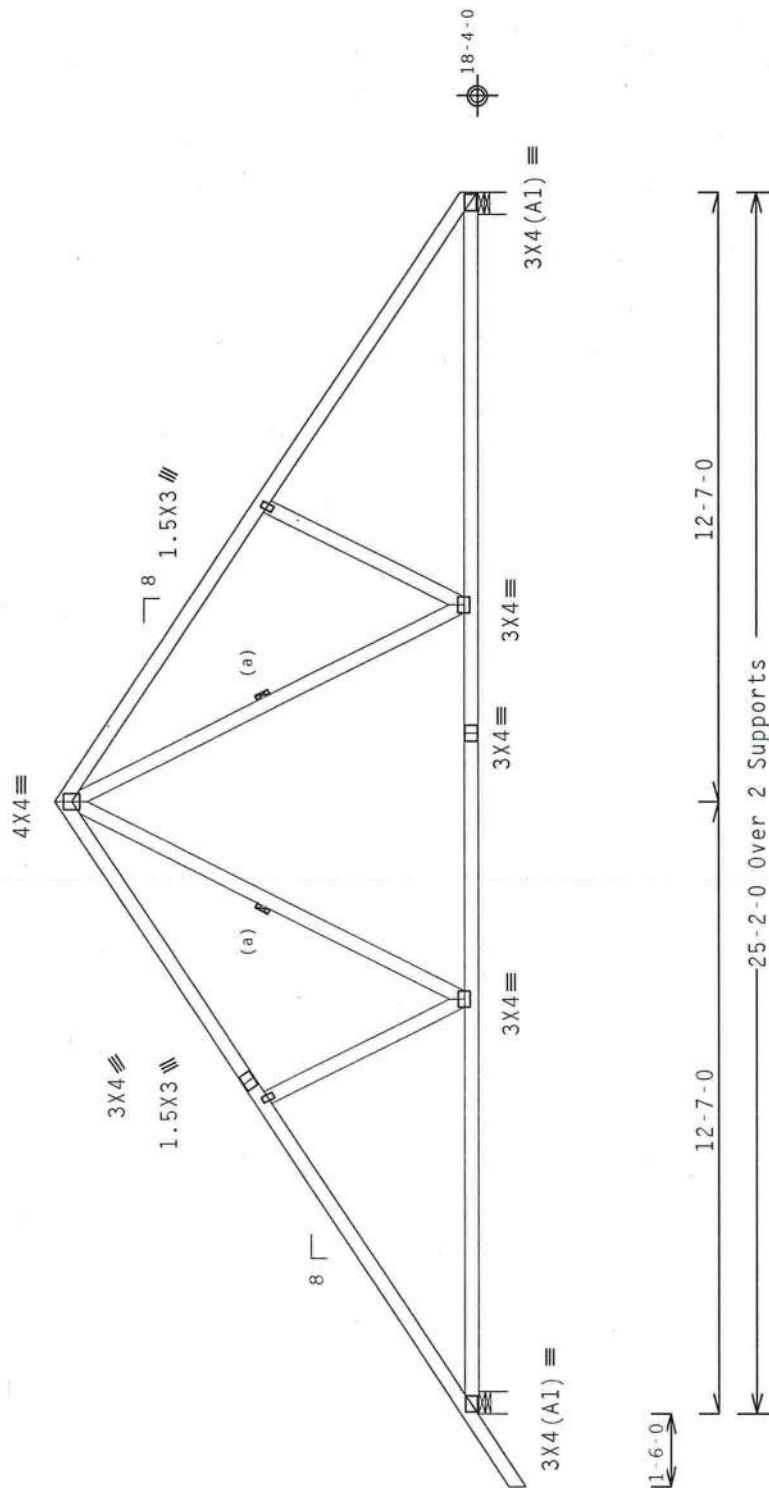
120 mph wind, 22.40 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

WWFRS loads based on trusses located at least 11.20 ft. from roof edge.



R=1148 U=42 W=5.5"
RL=156/-165

R-1049 U=28 W=5.5"

PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
 $FT/RT=20\%(0\%)/10(0)$

10.03.11

QTY:2 FL/-/5/-/-/R/-

Scale = .25"/Ft.

WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
 SUPPLIES: THIS PACKET INCLUDES ALL COMPONENTS REQUIRED TO COMPLETE THE PROJECT. IF YOU ARE MISSING ANY PARTS, PLEASE CONTACT US AT 1-800-828-8888.

Installers require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSP (Building Component Safety Information, by IPI and NITCA) for safety practices prior to performing these functions. Installers shall provide Temporary Bracing per BCSP. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Ceilings shown for permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITT Building Components Group Inc. (ITBCG) shall not be responsible for any deviation from this design, or any failure to build the product in accordance with ANSI/DP1-1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawing 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/DP1-1 Sec. 2. For more information see: This job's



TC LL	20.0 PSF	REF R9114- 12777
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCUSR9114 12123028
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2893
DUR.FAC.	1.25	

Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#1
	webs	2x4	SP	#3

Roof overhang supports 2.00 psf soffit load.

(H1) = (J) Hanger not calculated (2)2x4 SP #1 supporting member.

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

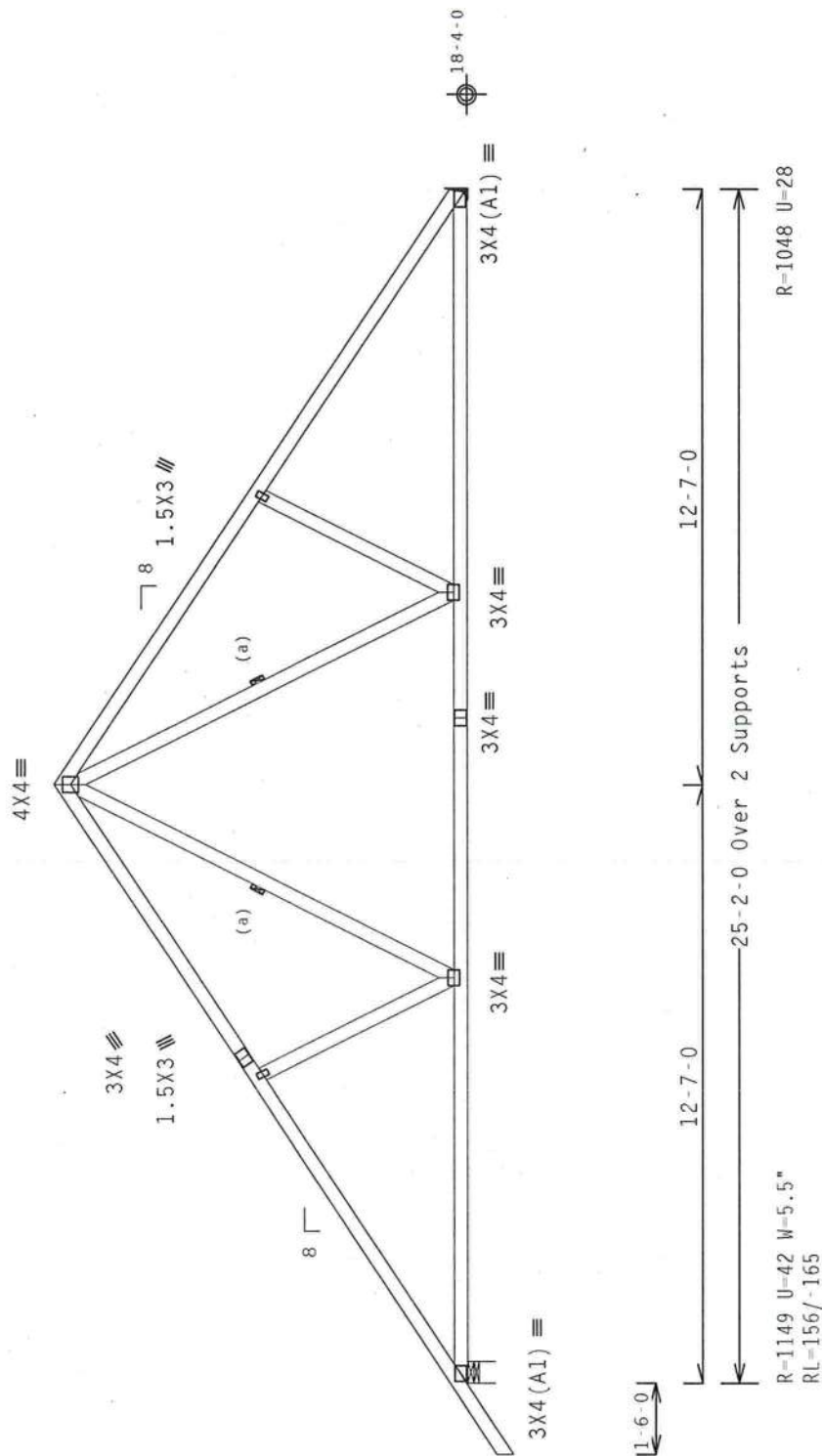
120 mph wind, 22.40 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf. GCpl(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

(a) Continuous lateral bracing equally spaced on member. Or 1x4 #3SRB SPF-S or better "T" brace. 80% length of web member. Attached with 8d Box nails @ 6" OC.

Bottom chord checked for 10.00 psf non-concurrent live load.

MMFRS loads based on trusses located at least 11.20 ft. from roof edge.



PLT TYP. Wave

Design Crit: $\text{FBC2010Res/TPI-2007(STD)}$
 $\text{FT/RT}=20\%(0\%)/10(0)$

10.03.11

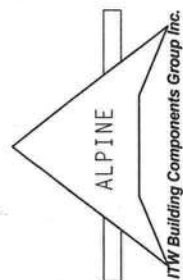
QTY:6 FL/-/5/-/-/R/-

Scale = .25"/Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and incorporate the latest edition of BCSP (Building Components Safety Information, by IP1 and IBCA) for safety practices prior to performing these functions. Installers shall provide competent bracing per BCSP (Building Components Safety Information, by IP1 and IBCA). Trusses shall be braced in accordance with the following: Trusses shall have a properly attached field ceiling. Trusses shall have permanent lateral restraint of webs shall have bracing installed per BCSP sections B3, B7 or B10, as applicable.

ITW Building Components Group Inc. (ITWBG) shall not be responsible for any deviation from this design or any failure due to the truss in conformance with AISI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the building designer per AISI/TPI 1, Sec.2. For more information see: This job's



TC LL	20.0 PSF	REF R9114- 12778
TC DL	7.0 PSF	DATE 05/02/12
BC DL	10.0 PSF	DRW HCUSR9114 12123029
BC LL	0.0 PSF	HC-ENG DF/DF
TOT.LD.	37.0 PSF	SEQN- 2894
DUR.FAC.	1.25	

(12-085--Doug Edgley JUSTICE RES -- Lake City, FL - FI)

Top chord 2x4 SP #3
Bot chord 2x4 SP #3
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

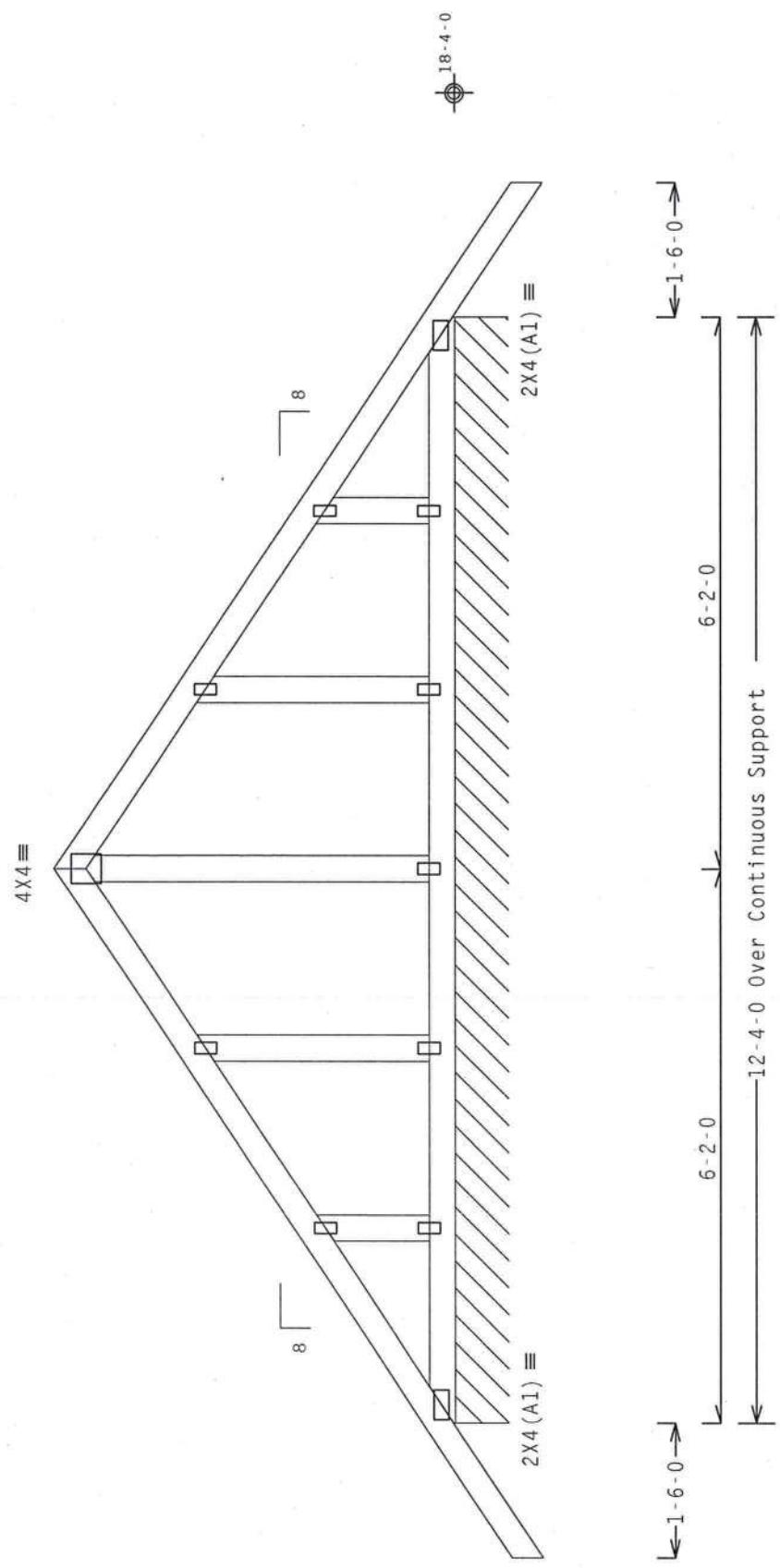
See DWGS A12030ENC100212, GBLLETIN0212, & GABRST100212 for more requirements.

120 mph wind, 20.26 ft mean hgt., ASCE 7-10, CLOSED bldg. Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCp1(+/-)=0.18

Wind loads and reactions based on MWFRS with additional C&C member design.

Bottom chord checked for 10.00 psf non-concurrent live load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R-92 PLF U-9 PLF W-12-4-0
RL-8/-8 PLF

Note: All Plates Are 1.5X3 Except As Shown.

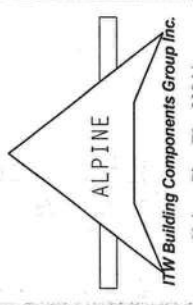
Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

QTY: 1 FL/-/5/-/R/- Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R9114- 12779
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123030
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2895
DUR.FAC.	1.25		



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and UIC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or any failure to build the truss in conformance with ABSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 160A-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ABSI/TPI 1 Sec.2. For more information see: This Job's



Top chord 2x4 SP #3
Bot chord 2x4 SP #1
Webs 2x4 SP #3

Special loads

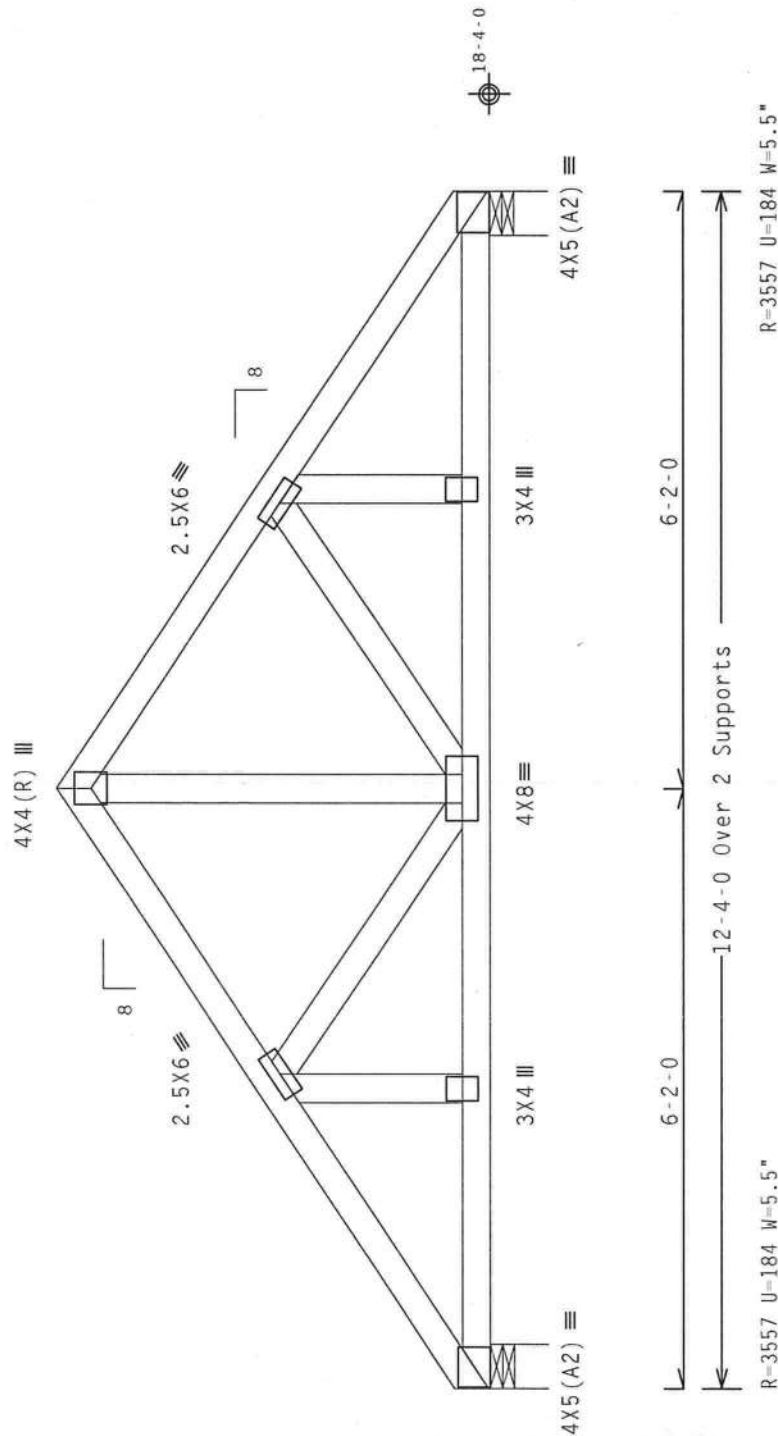
-----Lumber Dur.Fac.=1.25 / Plate Dur.Fac.=1.25)
TC- From 57 plf at 0.00 to 57 plf at 6.17
TC- From 57 plf at 6.17 to 57 plf at 12.33
BC- From 10 plf at 0.00 to 10 plf at 12.33
BC- 1048.46 lb Conc. Load at 1.40, 3.40, 5.40, 6.94
8.94, 10.94

Wind loads and reactions based on MWERS with additional C&C member design.

2 COMPLETE TRUSSES REQUIRED

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @3.00" o.c.
Webs: 1 Row @4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.
120 mph wind, 20.76 ft mean hgt, ASCE 7-10, CLOSED bldg. Located anywhere
in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf.
GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor
for dead load is 1.50.



PLT TYP. Wave

Design Crit: FBC2010Res/TPI-2007(STD)
FT/RT=20%(0%)/10(0)

QTY: 1 FL/-/5/-/-/R/-

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R9114- 12780
TC DL	7.0 PSF	DATE	05/02/12
BC DL	10.0 PSF	DRW	HCUSR9114 12123031
BC LL	0.0 PSF	HC-ENG	DF/DF
TOT.LD.	37.0 PSF	SEQN-	2900
DUR.FAC.	1.25		



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
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ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint details, unless noted otherwise. Refer to drawings 1600-2 for standard plate positions. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this design for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see: This Job's



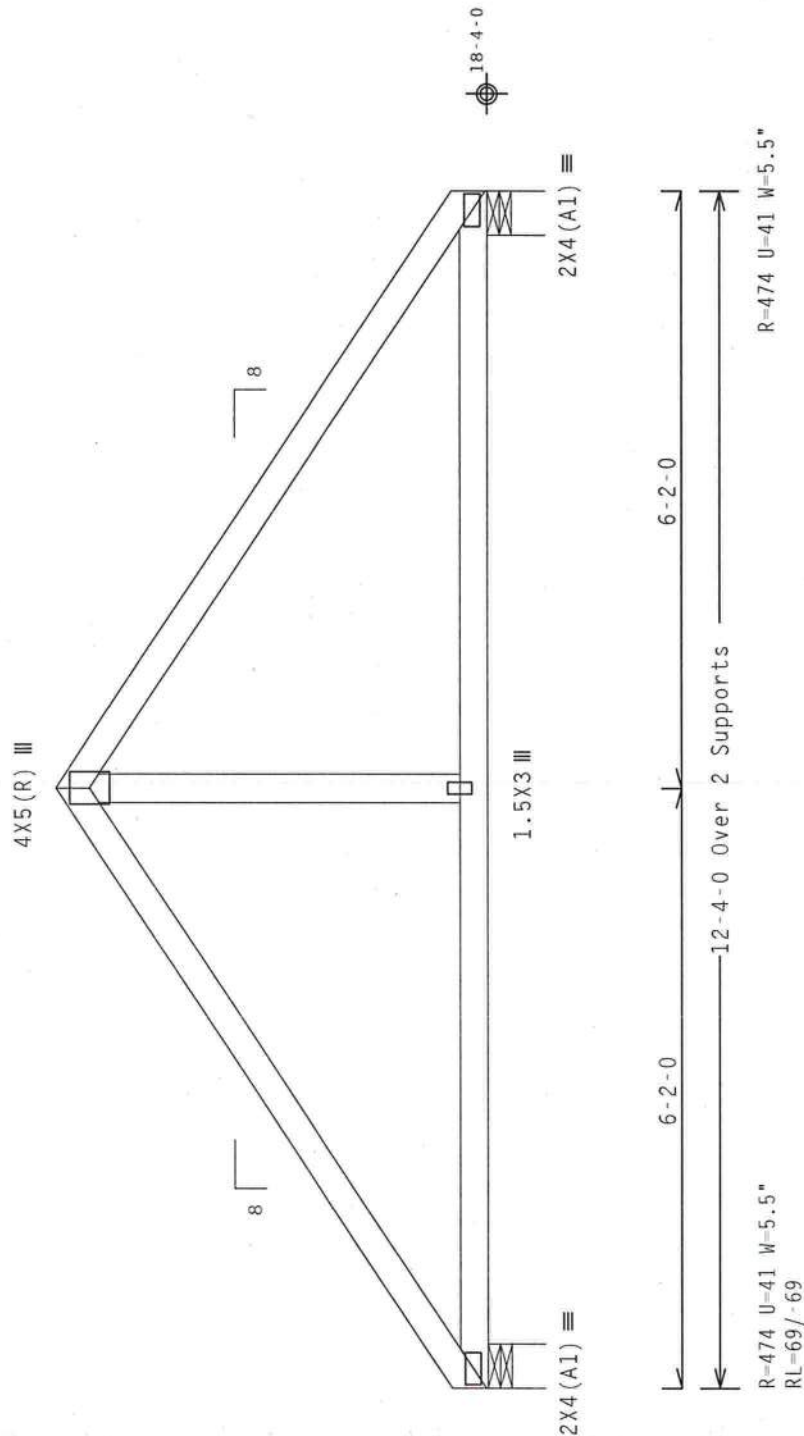
Top	chord	2x4	SP	#3
Bot	chord	2x4	SP	#3
	webs	2x4	SP	#3

120 mph wind, 20.76 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCpf(+/-)=0.18

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind loads and reactions based on MWFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: $\text{FBC2010Res/TPI-2007(STD)}$
 $\text{FT/RT}=20\%(0\%)/10(0)$

10.03.12

Scale = .5"/Ft.

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
IMPORTANT**

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BECU (Building Component Safety Information, by TPI and WCA) for safety information regarding truss components.

All members, including chords, shall have properly attached structural sheathing and bottom chord bracing. All chords and top chords shall have permanent lateral restraint of webs.

Top chords shall have a permanently attached rafter ceiling sections B33, B7 or B10, as applicable.

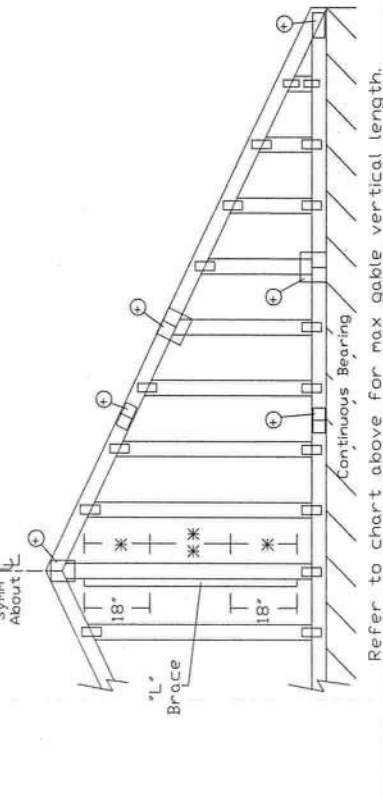
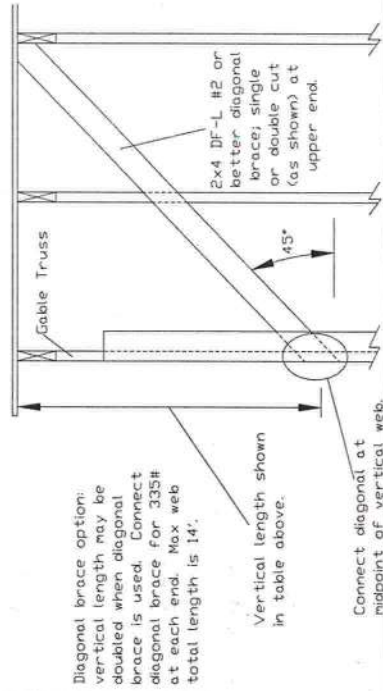
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TW Building Components Group Inc.

ASCE 7-10: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00 Gable Stud Reinforcement Detail

Gable Vertical Spacing		2x4 Braces		(1) 1x4 "L" Brace		(2) 2x4 "L" Brace		(2) 2x6 "L" Brace		(2) 2x6 "L" Brace		(2) 2x6 "L" Brace	
		Species	Grade	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	Standard	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Stud	Standard	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	Standard	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#2	Standard	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1 / #2	Standard	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Stud	Standard	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	Standard	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#2	Standard	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1 / #2	Standard	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	HF	Stud	Standard	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	Standard	8' 1"	8' 4"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
	DFL	#2	Standard	8' 3"	8' 7"	9' 9"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"



Refer to chart above for max gable vertical length.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING. ***IMPORTANT*** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

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ITW Building Components Group, Inc. shall not be responsible for any deviation from this drawing, any failure, or any damage to property or persons resulting from the use of this drawing. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites:
 ITWBCG www.itwbcg.com, TPI www.tpinstr.org, WCA www.sbcindustry.org, ICC www.iccsafe.org



Building Components Group, Inc.

Earth City, MO 63045



MAX. TOT. LD. 60 PSF
 MAX. SPACING 24.0"

REF ASCE7-10-GABI2015
 DATE 2/14/12
 DRWG A12015ENC100212

Bracing Group Species and Grades:			
Group A:		Group B:	
Spruce-Pine-Fir		Douglas Fir-Larch	
#1 / #2	Standard	#1	Standard
#3	Stud	#2	Standard
Hem-Fir		Southern Pine***	
#2	Standard	#1	Standard
#3	Stud	#2	Standard

1x4 Braces shall be SRB (Stress-Rated Board).
 ***For 1x4 So. Pine use only Industrial S5 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
 Wind Load deflection criterion is L/240.
 Provide uplift connections for 35 pif over continuous bearing (3 psf TC Dead Load).
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.
 So. Pine lumber design values based on the ALSC January, 2012 rule

Attach "L" braces with 10d (0.128"x3.0" min) nails.
 * For (1) "L" brace: space nails at 2' o.c.
 In 18" end zones and 4' o.c. between zones.
 ** For (2) "L" braces: space nails at 3' o.c.
 In 18" end zones and 6' o.c. between zones.
 "L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	2.5X4

* Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

ASCE 7-10: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-10, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail
for lumber, plates, and other information not shown
on this detail.

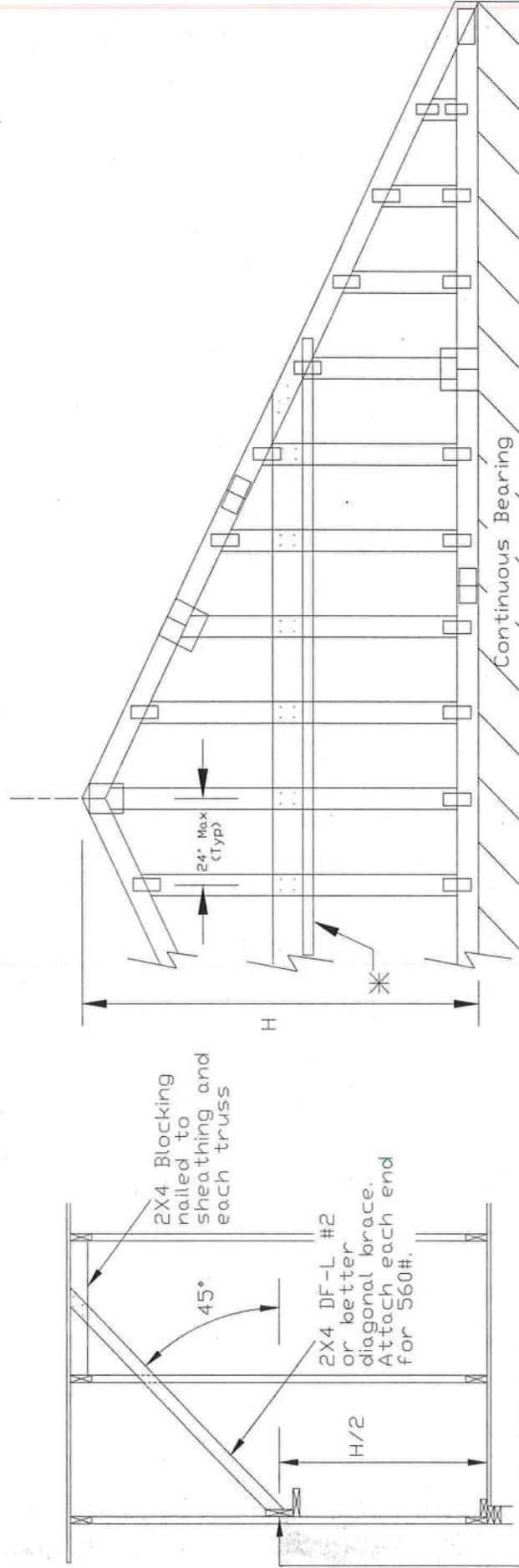
Nails: 10d box or gun (0.128"x3", min.) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC10).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 4'0" (see detail below or
refer to DRWG A12030ENC10).

* Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128" x 3", min.) nails @ 6" o.c.



2x6 #2 Stiffback
attached to each
Stud w/ (4) 10d box or gun (0.128" x 3", min.) nails.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING
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ITWBCG www.itwbcg.com, TPI www.tpinstr.org, WICA www.bcsindustry.org, ICC www.iccsafe.org



Building Components Group Inc.

Earth City, MO 63045

REF GE WHALER

DATE 2/14/12

DRWG GABRST100212

MAX. TOT. LD. 60 PSF

MAX. SPACING



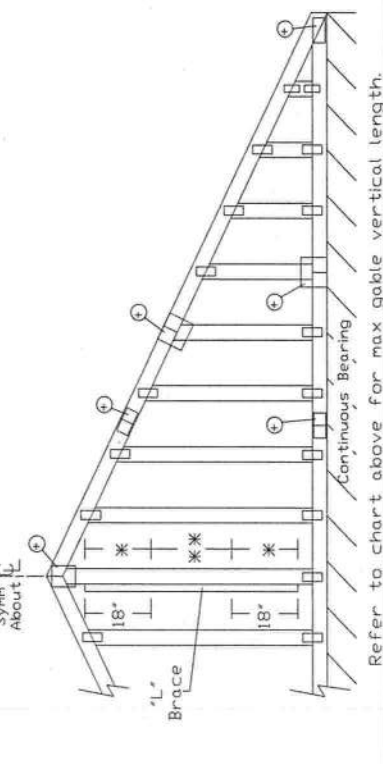
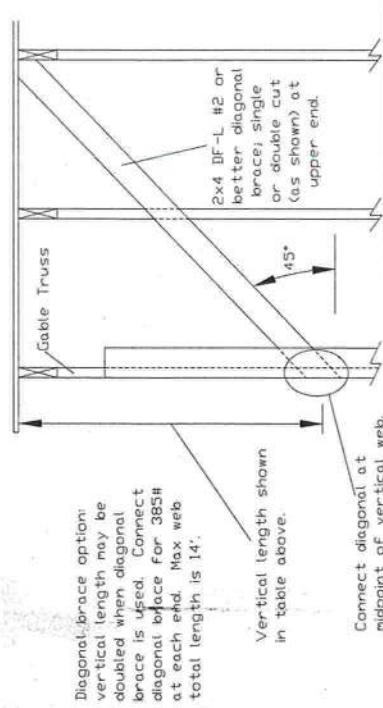
Gable Stud Reinforcement Detail

ASCE 7-10: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Dr: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Dr: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

2x4 Gable Vertical Spacing		Brace		No Braces		(1) 1x4 "L" Brace		(2) 2x4 "L" Brace		(1) 2x6 "L" Brace		(2) 2x6 "L" Brace		(1) 2x6 "L" Brace		(2) 2x6 "L" Brace		(1) 2x6 "L" Brace		(2) 2x6 "L" Brace	
		Species	Grade	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
Max Gable Vertical Length	12" o.c.	SPF	#1 / #2	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	Stud	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		SP	Standard	7' 8"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#1	7' 6"	8' 2"	9' 3"	9' 8"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	24" o.c.	SP	#2	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	Stud	7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		SP	Standard	7' 8"	8' 0"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#1	7' 6"	8' 2"	9' 3"	9' 8"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	16" o.c.	SPF	#1 / #2	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	Stud	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		SP	Standard	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#1	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	12" o.c.	SPF	#1 / #2	9' 0"	9' 4"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	Stud	9' 0"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		SP	Standard	9' 0"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#1	9' 0"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"



Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 385# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.

Refer to the Building Designer for conditions not addressed by this detail.

REF ASCE 7-10-GAB12030
DATE 2/14/12
DRWG A12030ENC100212

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

DOUGLAS FLEMING
LICENSE
No. 86648
STATE OF FLORIDA
PROFESSIONAL ENGINEER
02 '12

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Earth City, MO 63045

