

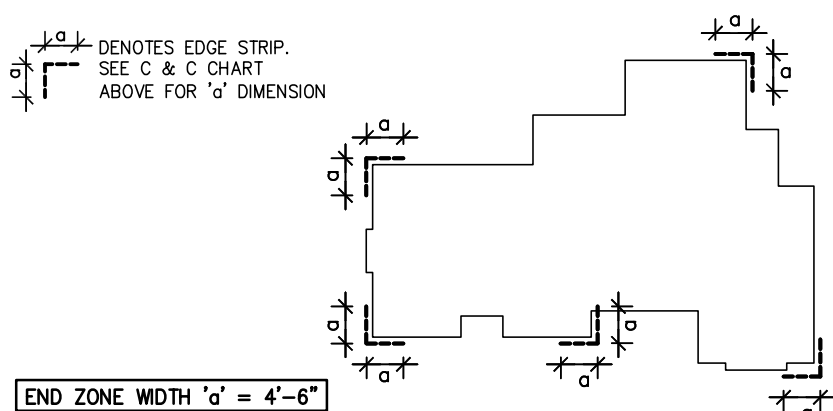
DESIGN SPECIFICATIONS		
<b>DESIGN CODE:</b> 2020 FLORIDA BUILDING CODE – RESIDENTIAL <b>DESIGN IS VOID ONE YEAR AFTER THE DATE OF THE ORIGINAL PLANS, UNLESS PLANS HAVE BEEN REVIEWED FOR CODE COMPLIANCE.</b> <b>DESIGN LOADS:</b> ACTUAL AND UNIFORM		
<b>ROOF LOADING:</b> TOP CHORD LIVE LOAD TOP CHORD DEAD LOAD TOP CHORD DEAD LOAD BOTTOM CHORD LIVE LOAD BOTTOM CHORD DEAD LOAD DEFLECTION CRITERIA: ROOF FRAMING: LIVE LOAD L/240 TOTAL LOAD L/180 FLOOR FRAMING: LIVE LOAD L/360 & TOTAL LOAD L/240 0.75" MAX ANY CASE	<b>ROOF:</b> (12x12) 25 20 psf (ARCH SHINGLES) 20 psf (TILE SHINGLES) 10 psf 5 psf	<b>FLOOR:</b> (12x12) 25 40 psf 10 psf 10 psf 5 psf
<b>WIND LOADING:</b> ASCE 7/16 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A MIN. DEAD LOAD CONDITION OF 0.25" TOP CHORD, AND 0.5" PSF BOTTOM CHORD. REACTIONS CALCULATED FOR THE BEARING POINTS OF ROOF TRUSSES SHALL BE REDUCED, SPECIFICALLY, ATTIC FLOOR LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED BY 0.75 WHEN COMBINED w/ DEAD LOAD.		
<b>BASIC WIND SPEED (ASCE 7-16)</b> ----- 130mph <b>IMPORTANCE FACTOR</b> ----- 1.00 <b>MEAN ROOF HEIGHT</b> ----- 20.0 FT <b>ROOF PITCH</b> ----- 6/12 <b>BUILDING CATEGORY</b> ----- II <b>EXPOSURE CATEGORY</b> ----- C <b>ENCLOSURE CLASSIFICATION</b> ----- ENCLOSURED <b>INTERNAL PRESSURE COEFFICIENT</b> ----- ± .18		

## MATERIAL SPECIFICATIONS

**HARDWARE AND ANCHORS:**  
**ANCHOR BOLTS & THREADED ROD:** SHALL BE IN ACCORDANCE WITH ASTM A 307 OR ASTM F 1554 GRADE 36  
**WASHERS:** SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B)  
**NUTS:** SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE A HEX  
**METAL CONNECTORS:** ALL METAL CONNECTORS WHICH ARE EXPOSED TO EXTERIOR SHALL BE GALVANIZED.  
**REINFORCING STEEL:** SHALL BE ASTM A615, GRADE 60.  
**STRUCTURAL STEEL:** SHALL BE ASTM A992, GRADE 50.  
**WELDED WIRE FABRIC (WFW):** SHALL BE ASTM A185.  
**MINIATED VENEER LUMBER (VLV):** ALL LAMINATED VENEER LUMBER SHALL MEET OR EXCEED THE FOLLOWING DESIGN PROPERTIES – ELASTIC MODULUS (E)1,900ksi, BENDING STRESS (Fb) 2600psi

COMPONENTS & CLADDING ALLOWABLE DESIGN PRESSURES			
TRIBUTARY AREA (sf)	INTERIOR ZONE (PSF)	EDGE STRIP (PSF): 'a' = 4'-6"	
10	+24.61 – 26.70	+24.61	1 CAR GARAGE DOOR (8'x7')
50	+23.42 – 25.51	+23.42 – 30.58	2 CAR GARAGE DOOR (16'x7')
100	+22.01 – 24.09	+20.91 – 27.74	

- THE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE ABOVE WIND PRESSURES HAVE BEEN REDUCE BY 0.60 AS PERMITTED BY THE ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION SHALL BE PERMITTED.
- COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.
- LINEAR INTERPOLATION IS PERMISSIBLE.
- PLUS = PRESSURE AND MINUS = SUCTION.
- DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE RESPONSIBILITY OF THE WINDOW/DOOR MANUF./SUPPLIER & SHALL MEET THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.



## SCOPE OF SERVICE

**MEANS AND METHODS:**  
THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

**LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES:**  
THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT SPECIFICALLY ADDRESSED, TRUSS-TO-TRUSS CONNECTION, AND ANY ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

## GENERAL NOTES & CONSTRUCTION SPECIFICATIONS

**FLOOR SHEATHING SPECIFICATIONS:**  
23/32" 1/8" OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD

**ROOF SHEATHING SPECIFICATIONS:**  
SHINGLE – MIN. 15/32", 32/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

**TILE** – MIN. 15/32" 32/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

**METAL** – MIN. 1/2", 24/16, APA RATED PLYWOOD SHEATHING, NAILED w/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" WITHIN 4'-0" OF ROOF EDGE).

**WALL SHEATHING SPECIFICATIONS:**  
FLEXIBLE FINISH – MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED w/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS INCLUDE: WOOD, CEMENT, OR VINYL SIDING, HARDI PANEL & BRICK. ALL OTHER WALL FINISHES ARE CONSIDERED BRITTLE FINISH.

**STUCCO FINISH** – MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED w/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 3/4" STRUCTURAL 1 GRADE SHEATHING OR 1/2" OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

**MASONRY SPECIFICATIONS:**  
MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI305.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2000 psi PER ASTM C1019. GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF 3/4" PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING.

**CONCRETE MASONRY UNITS (CMU):**  
CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

**MASONRY STEM WALLS:** ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE 'S' MORTAR. WALL COURSING SHALL BE RUNNING BONDS. STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI FEA ROOF CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER, WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REIN. SHALL BE INCREASED AS NOTED ON 1/5.0. UNLESS NOTED OTHERWISE, LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

**CONCRETE SPECIFICATIONS:**  
ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. CONTRACTOR AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

### GENERAL NOTES:

**FOOTING AND FOUNDATIONS:**  
FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR)

UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS

**CONCRETE SLABS ON GRADE:**  
SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. **SAWCUTS:** FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12"x12" GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

**WOOD FRAMING SPECIFICATIONS:**  
ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED. IF A DO OR NON-DOT DURABLE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF AZCA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

**PRE-ENGINEERED WOOD TRUSSES:**  
SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY" AND RECOMMENDATION FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91". AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

**ROOF COVERING SPECIFICATIONS:**  
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL" AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOF'S SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

**WATERPROOFING:**  
THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

WOOD FASTENING SPECIFICATIONS		
MEMBERS	CONNECTION TYPE	FASTENER
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.
TOP PLATE, LAPS/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAILS
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"
CEILING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS
CEILING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS
CEILING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS
ROOF RAFTER TO 2x... RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS
CONT. HEADER, TWO PIECES	FACE NAIL	16d @ 16" O.C. @ EDGE
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"

**NAIL SPECIFICATIONS**  
3"x0.131" = GUN NAILS  
2"x0.113" = 6d  
3"x0.148" = 10d  
1 1/2"x0.148" = 10d1 1/2"

BRICK NOTES / LINTEL SCHD		
LINTEL DIMENSION	MIN. BRG.	MAX. SPAN
L3 1/2"x3 1/2"x1/4"	4"	6'-0"
L4x3 1/2"x1/4"	6"	8'-0"
L5x3 1/2"x1/4"	6"	10'-0"
L6x3 1/2"x1/4"	6"	12'-0"
L7x3 1/2"x1/4"	6"	16'-0"

1. STEEL LINTELS TO BE MINIMAL 36" LINTEL MUST HAVE CORROSION RESISTANT COATING OF EPOXY BASED PAINT.

2. LINTEL MORE THAN 8'-0", SHOULD BE LATERALLY SUPPORTED NOT TO EXCEED 6 FT. O.C. w/ 2-1/4"x3" WD. SCREWS INTO CONCRETE MASONRY UNIT.

3. BRICK VENEER ATTACHMENT: HORIZONTAL TIES @ 24" O.C., VERT. TIES @ 12" O.C. (FOR 110mph WIND-ZONE VERT. TIES @ 16" O.C.). AT ALL OPENINGS SPACE TIES WITHIN 12" OF OPENINGS. PROVIDE 1/2" WEEP HOLES @ 33" O.C. IMMEDIATELY ABOVE FLASHING.

USP CONNECTORS				
CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
USP A35	450	450	(9)10d1 1/2"	
USP RT7	585	495	(5)8d EA. END	
USP RT8A	775	650	(5)10d1 1/2" EA. END	
USP MTW12	1195	860	(7)10d1 1/2" EA. END	
USP HTW20	1450	1245	(12)10d1 1/2" EA. END	
USP MSTA24	1640	1455	(9)10d EA. END	
USP MSTA36	2065	2065	(13)10d EA. END	
USP LTB20B	1105	1105	1/2" ROD TO FTG.	
USP JUS28	1305	1305	(6)10d TO HEADER	
USP HTT16	4290	4290	3/4" ROD TO FTG.	
USP HTT22	5370	5370	3/4" ROD TO FTG.	
USP PAU44	2535		3/4" ROD w/ (12)16d	
USP PAU66	2535		3/4" ROD w/ (12)16d	
USP MSTM24	1545	1455	(5)1/2"x2-1/4" TAPCONS	

SIMPSON CONNECTORS				
CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
A35	450	450	12--8d1 1/2"	10446.4
H2.5T	600	520	5--8d EA. END	11478.3
HTS16	1150	1085	16--10d EA. END	10456.6
MTS12	1000	860	7--10d1 1/2" EA. END	10456.3
HTS20	1450	1245	24--10d1 1/2" EA. END	13872.3
MSTA24	1765	1270	9--10d EA. END	13872.4
MSTA36	2050	1870	13--10d EA. END	13872.8
HTT4	3480	3080		11496.2
HTT5	5250	4670		11496.2
LHS28	930	780	6--10d TO TRUSS/BAM	10655.113
HLU410	905	785	14--16d TO HEADER	10531.36
ABU44	2200		1--3/4" ROD TO FTG.	
ABU66	2300		3/4" ROD EPOKIED 6" MIN	10849.6
SET	N/A	N/A	3/4" ROD EPOKIED 6" MIN	10849.6
SET	N/A	N/A	SIMPSON EPOXY-TIE	11506.4
LT720B	1675	1675	10--16d TO STUD/BEAM/POST	11496.3
LSTA12	805	695	10--10d	13872.5
CS16	1705	1705	13--8d	10852.1

1. DENOTES EDGE STRIP. SEE C & C CHART ABOVE FOR 'a' DIMENSION

2. END ZONE WIDTH 'a' = 4'-6"

3. MEANS AND METHODS: THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

4. LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES: THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT SPECIFICALLY ADDRESSED, TRUSS-TO-TRUSS CONNECTION, AND ANY ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

5. CONCRETE MASONRY UNITS (CMU): CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

6. MASONRY STEM WALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE 'S' MORTAR. WALL COURSING SHALL BE RUNNING BONDS. STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI FEA ROOF CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER, WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REIN. SHALL BE INCREASED AS NOTED ON 1/5.0. UNLESS NOTED OTHERWISE, LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

7. CONCRETE SPECIFICATIONS: ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. CONTRACTOR AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

8. FOOTINGS AND FOUNDATIONS: FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

9. FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR)

10. UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS

11. CONCRETE SLABS ON GRADE: SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. SAWCUTS: FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12"x12" GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

12. WOOD FRAMING SPECIFICATIONS: ALL WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED. IF A DO OR NON-DOT DURABLE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF AZCA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

13. PRE-ENGINEERED WOOD TRUSSES: SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BEARING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER "COMMENTARY" AND RECOMMENDATION FOR HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91". AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

14. ROOF COVERING SPECIFICATIONS: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL" AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOF'S SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

15. WATERPROOFING: THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

16. WIND LOADING: ASCE 7/16 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A MIN. DEAD LOAD CONDITION OF 0.25" TOP CHORD, AND 0.5" PSF BOTTOM CHORD. REACTIONS CALCULATED FOR THE BEARING POINTS OF ROOF TRUSSES SHALL BE REDUCED, SPECIFICALLY, ATTIC FLOOR LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED BY 0.75 WHEN COMBINED w/ DEAD LOAD.

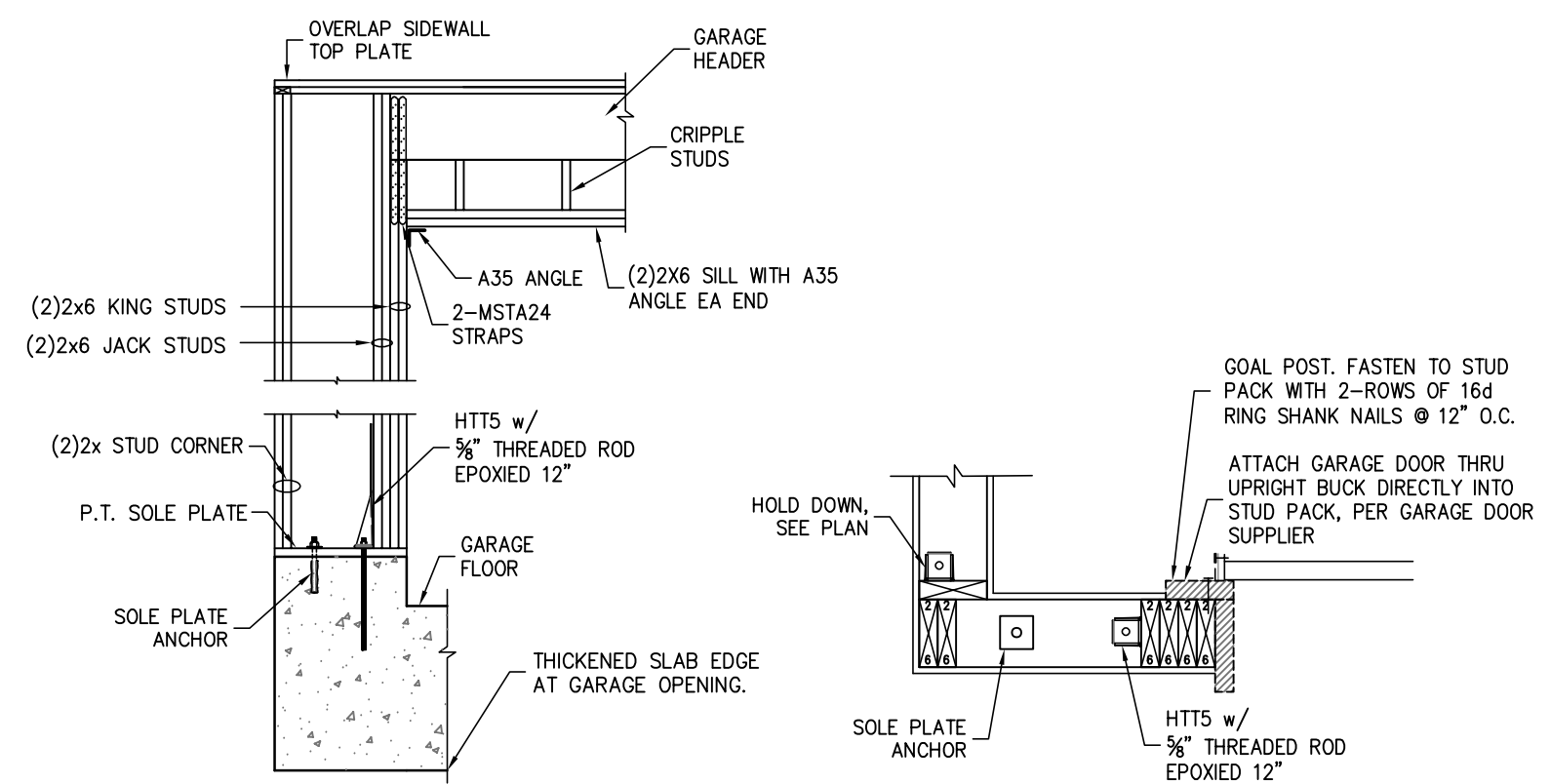
17. BASIC WIND SPEED (ASCE 7-16) ----- 130mph  
IMPORTANCE FACTOR ----- 1.00  
MEAN ROOF HEIGHT ----- 20.0 FT  
ROOF PITCH ----- 6/12  
BUILDING CATEGORY ----- II  
EXPOSURE CATEGORY ----- C  
ENCLOSURE CLASSIFICATION ----- ENCLOSURED  
INTERNAL PRESSURE COEFFICIENT ----- ± .18

18. HARDWARE AND ANCHORS: ANCHOR BOLTS & THREADED ROD: SHALL BE IN ACCORDANCE WITH ASTM A 307 OR ASTM F 1554 GRADE 36  
WASHERS: SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B)  
NUTS: SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE A HEX  
METAL CONNECTORS: ALL METAL CONNECTORS WHICH ARE EXPOSED TO EXTERIOR SHALL BE GALVANIZED.  
REINFORCING STEEL: SHALL BE ASTM A615, GRADE 60.  
STRUCTURAL STEEL: SHALL BE ASTM A992, GRADE 50.  
WELDED WIRE FABRIC (WFW): SHALL BE ASTM A185.  
MINIATED VENEER LUMBER (VLV): ALL LAMINATED VENEER LUMBER SHALL MEET OR EXCEED THE FOLLOWING DESIGN PROPERTIES – ELASTIC MODULUS (E)1,900ksi, BENDING STRESS (Fb) 2600psi

19. MEANS AND METHODS: THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

20. LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES: THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED PRE



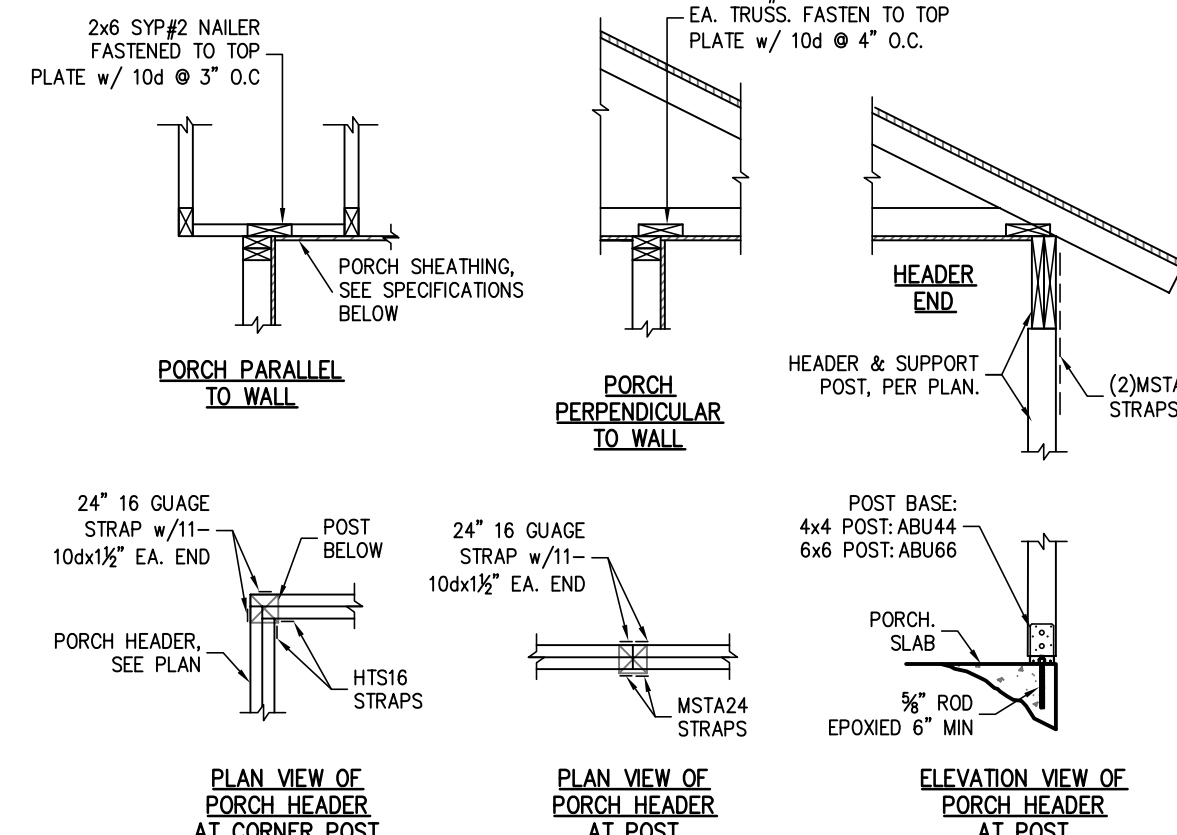


GARAGE WING WALL ELEVATION

GARAGE WING WALL SECTION

1 WHEN NOTED SO.1 GARAGE HEADER FRAMING

SCALE: N.T.S.



PORCH PARALLEL TO WALL

PORCH PERPENDICULAR TO WALL

PLAN VIEW OF PORCH HEADER AT CORNER POST

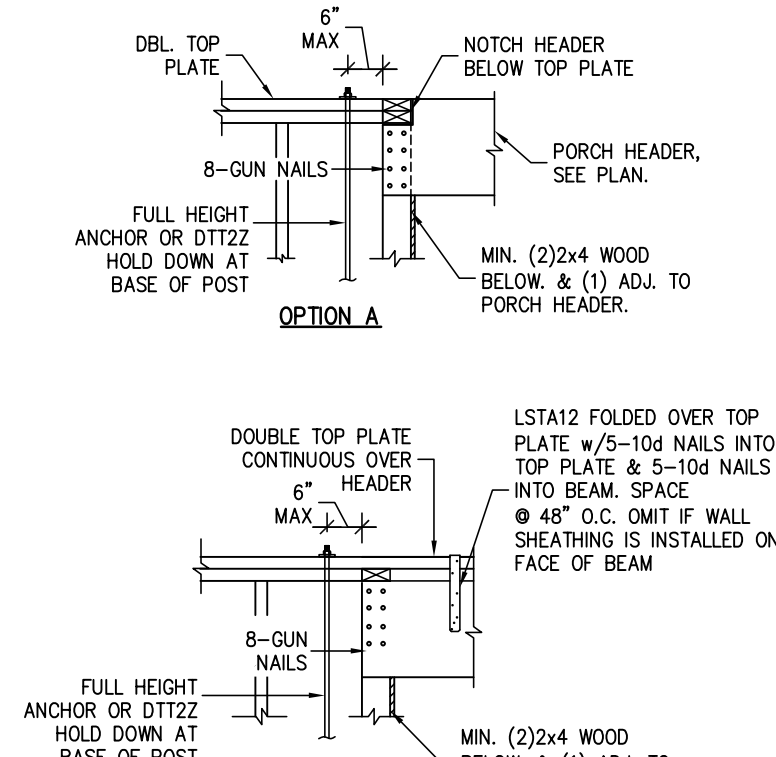
PLAN VIEW OF PORCH HEADER AT POST

ELEVATION VIEW OF PORCH HEADER AT POST

PORCH SHEATHING SPECIFICATIONS:  
1/2" GYPSUM (WATER PROOF) GREEN BOARD, w/ 5d COOLER NAILS @ 6" O.C. EDGE & FIELD  
OR  
3/4" (OR GREATER) OSB OR PLYWOOD, w/ 8d COMMON NAILS @ 6" O.C. EDGE AND FIELD

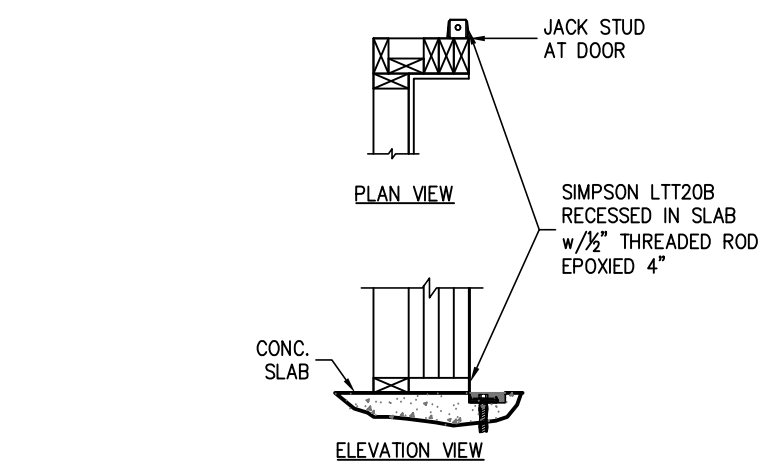
2 TYPICAL PORCH FRAMING DETAILS

SCALE: N.T.S.



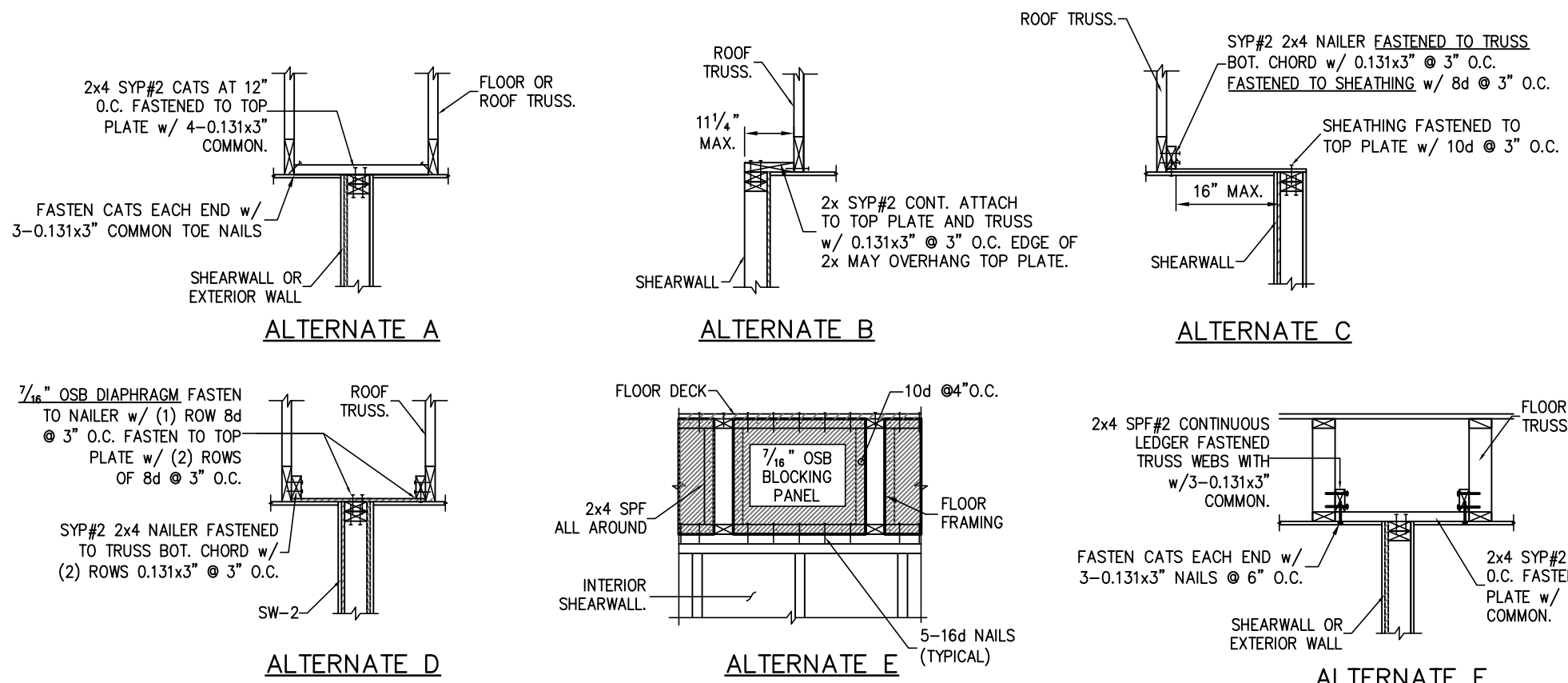
3 TYPICAL PORCH BEAM CONNECTION

SCALE: N.T.S.



4 LTT20B RECESSED IN SLAB

SEE CONSTRUCTION SPECIFICATIONS FOR ROOF AND WALL SHEATHING AND STUD FRAMING.



ALTERNATE A

ALTERNATE B

ALTERNATE C

ALTERNATE D

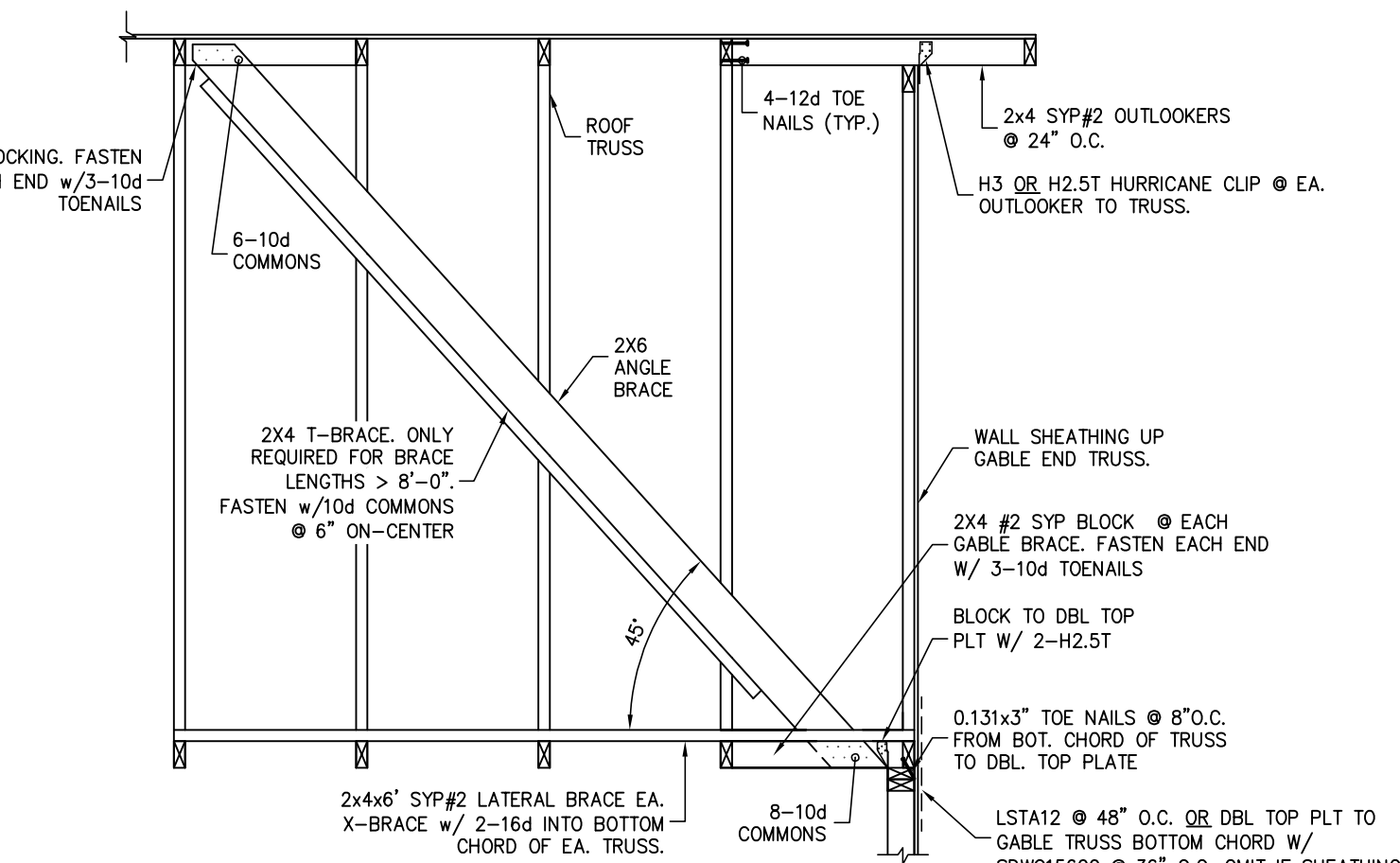
ALTERNATE E

ALTERNATE F

SHEARWALL NOTES:  
1. AT ROOF TRUSS, CONTRACTOR MAY CHOOSE A,B,C,D, OR E.  
2. USE ALTERNATE A. AT EXTERIOR WALLS ENDING BETWEEN ROOF TRUSSES.  
3. USE ALTERNATE E & F FOR FLOOR TRUSS ATTACHMENT

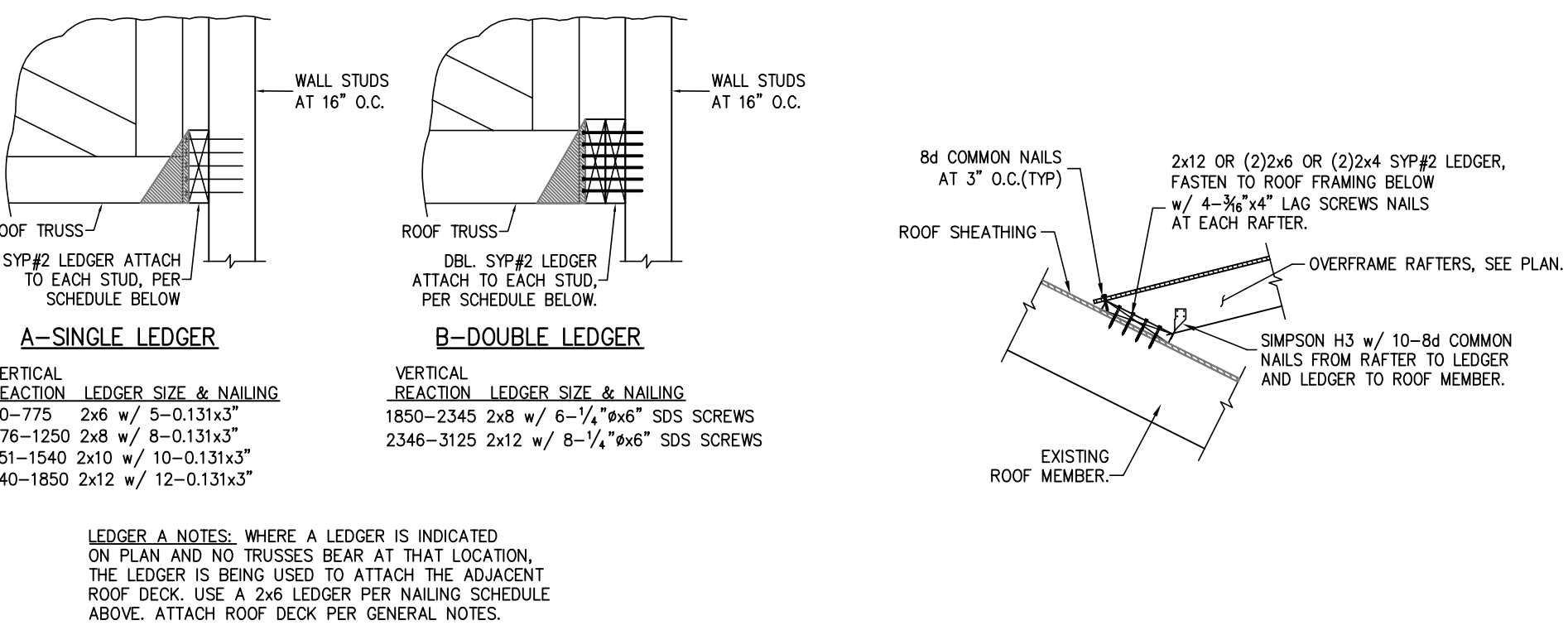
5 SHEARWALL ATTACHMENT AT ROOF & FLOOR

SCALE: N.T.S.



6 TYPICAL SHEARWALL ELEVATION

PROVIDE SOLID BLOCKING WITHIN FLOOR SYSTEM AT SW END POSTS.



A-SINGLE LEDGER

B-DOUBLE LEDGER

LEDGER A NOTES: WHERE A LEDGER IS INDICATED ON PLAN AND NO TRUSSES BEAR AT THAT LOCATION, THE LEDGER IS BEING USED TO ATTACH THE ADJACENT ROOF DECK. USE A 2x6 LEDGER PER NAILING SCHEDULE ABOVE. ATTACH ROOF DECK PER GENERAL NOTES.

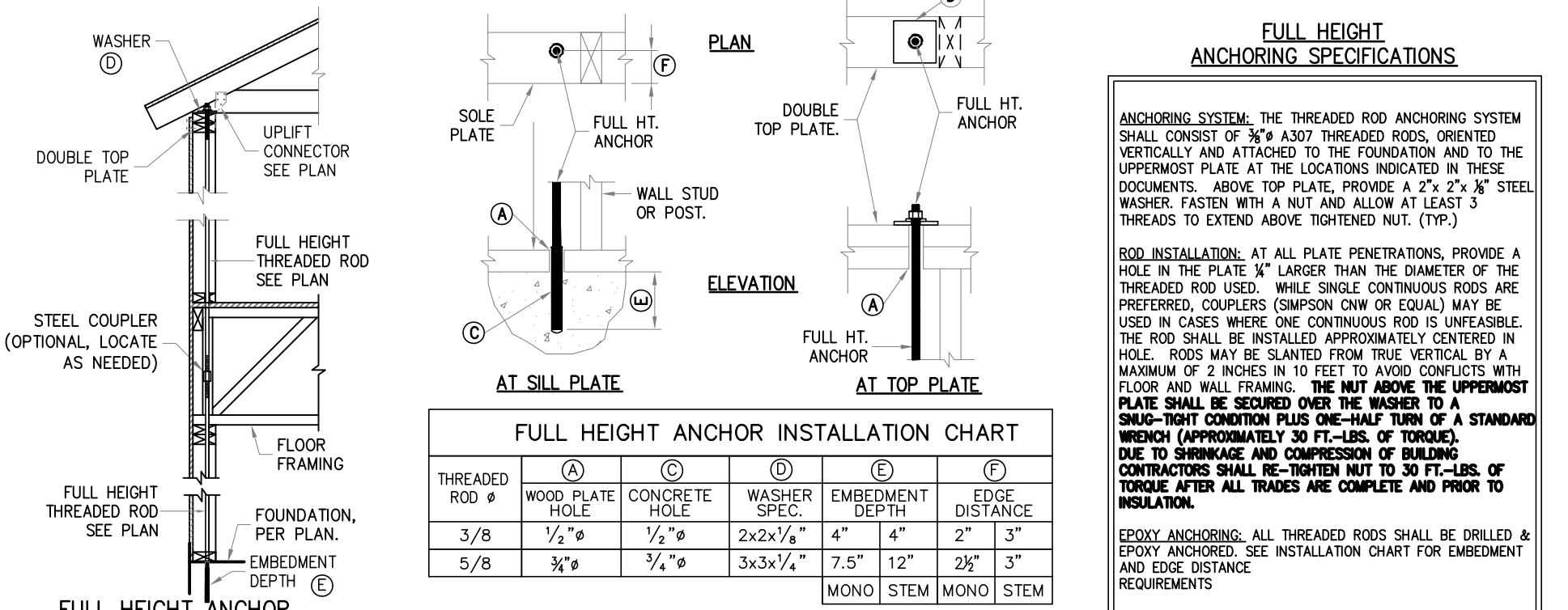
LEDGER B NOTES: WHERE A LEDGER IS INDICATED ON PLAN AND NO TRUSSES BEAR AT THAT LOCATION, THE LEDGER IS BEING USED TO ATTACH THE ADJACENT ROOF DECK. USE A 2x6 LEDGER PER NAILING SCHEDULE ABOVE. ATTACH ROOF DECK PER GENERAL NOTES.

8 LEDGER CONNECTION

TRUSS TO LEDGER CONNECTION BY TRUSS ENGINEER, NOT SHOWN FOR CLARITY

9 DECK LEDGER AT OVERFRAME RAFTERS

USE THIS DETAIL TO FASTEN OVERFRAMED ROOFS, VALLEYS, ETC.



FULL HEIGHT ANCHORING SPECIFICATIONS

ANCHORING SYSTEM: THE THREADED ROD ANCHORING SYSTEM SHALL CONSIST OF 3/4" A307 THREADED RODS, ORIENTED VERTICALLY AND ATTACHED TO THE FOUNDATION AND TO THE UPPERMOST PLATE AT THE LOCATIONS INDICATED IN THESE DOCUMENTS. ABOVE TOP PLATE, PROVIDE A 2"x2"x1/2" STEEL WASHER. FASTEN WITH A NUT AND ALLOW AT LEAST 3 THREADS TO EXTEND ABOVE TIGHTENED NUT. (TYP.)

ROD INSTALLATION: AT ALL PLATE PENETRATIONS, PROVIDE A HOLE IN THE PLATE 1/4" LARGER THAN THE DIAMETER OF THE THREADED ROD USED. WHILE SINGLE CONTINUOUS RODS ARE PREFERRED, COUPLERS (SIMPSON COW OR EQAL) MAY BE USED IN CASES WHERE ONE CONTINUOUS ROD IS UNDESIRABLE. THE ROD SHALL BE INSTALLED APPROXIMATELY CENTERED IN HOLE. RODS MAY BE SLANTED FROM TRUE VERTICAL BY A MAXIMUM OF 2 INCHES IN 10 FEET TO AVOID CONFLICTS WITH FLOOR AND WALL FRAMING. THE NUT ABOVE THE UPPERMOST PLATE SHALL BE SECURED OVER THE WASHER TO A SHIP-TIGHT CONDITION PLUS ONE-HALF TURN OF A STANDARD WRENCH (APPROXIMATELY 30 FT.-LBS. OF TORQUE). DUE TO SHRINKAGE AND COMPRESSION OF BUILDING CONTRACTORS SHALL RE-TIGHTEN NUT TO 30 FT.-LBS. OF TORQUE AFTER ALL TRADES ARE COMPLETE AND PRIOR TO INSULATION.

EPOXY ANCHORING: ALL THREADED RODS SHALL BE DRILLED & EPOXY ANCHORED. SEE INSTALLATION CHART FOR EMBEDMENT AND EXISTENCE REQUIREMENTS

THREADED ROD #	(A)	(B)	(C)	(D)	(E)	(F)
3/8"	WOOD PLATE HOLE	CONCRETE HOLE	WASHER SPEC.	EMBEDMENT DEPTH	EDGE DISTANCE	
1/2"	3/4"	1/2"	2x2x1/2"	4"	2"	3"
5/8"	3/4"	3/4"	3x3x1/2"	7.5"	28"	3"
				(MONO) STEM	(MONO) STEM	

FULL HEIGHT ANCHOR WALL SECTION

THREADED ROD INSTALLATION CHART

12 FULL HEIGHT WOOD FRAME WALL ANCHORING SYSTEM

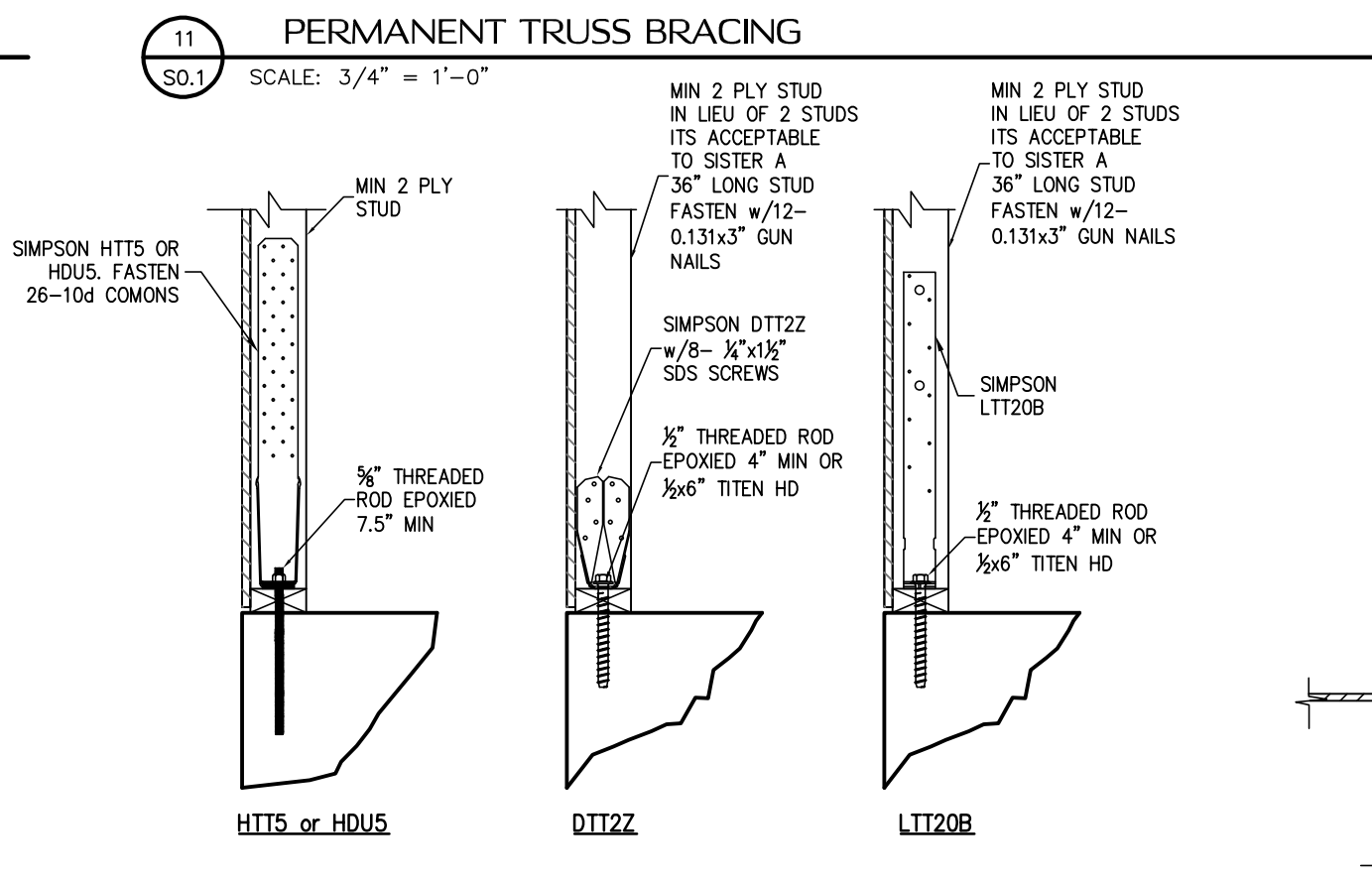
THIS DETAIL ONLY APPLIES WHEN NOTED ON PLAN

13 3/8" FULL HEIGHT ROD ALTERNATE ATTACHMENT

SCALE: N.T.S.

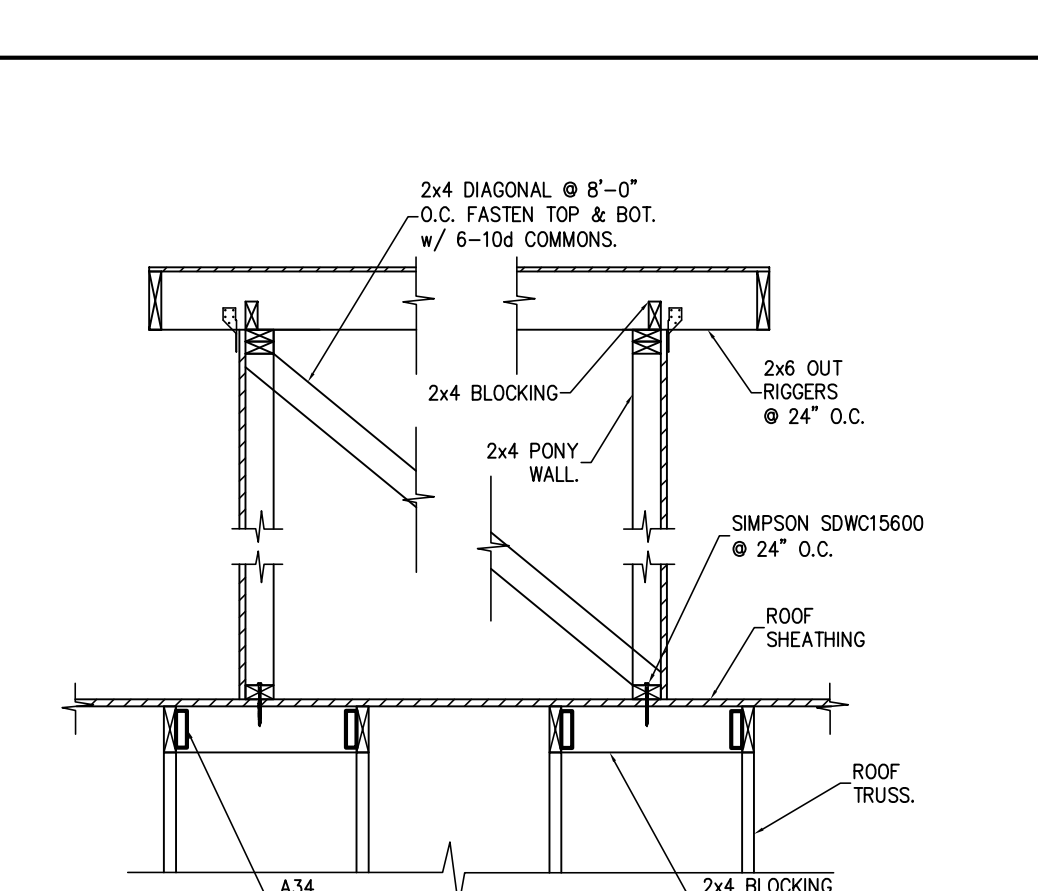
14 FULL HEIGHT THREADED ROD ALTERNATE

SCALE: N.T.S.



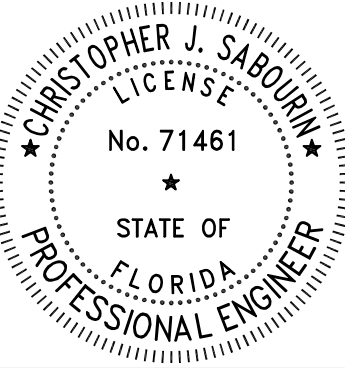
11 PERMANENT TRUSS BRACING

SCALE: 3/4" = 1'-0"



16 CUPOLA DETAIL

SCALE: N.T.S.



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PLAN NAME  
BZEC  
SSE No.  
22-0287

ISSUE DATE  
PERMIT 06.27.22  
REVISIONS DATE

STRUCTURAL ENGINEERING FOR  
THE JUDSON RESIDENCE

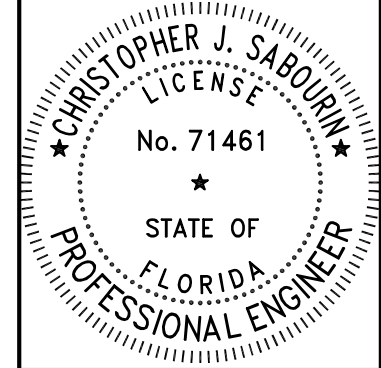
FIELD ALTERATION  
CONTRACTOR SHALL CONTACT SABO STRUCTURAL ENGINEERING PRIOR TO MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER SABOURIN MAY RESULT IN ADDITIONAL ENGINEERING OR INSPECTION FEES.

SCALING  
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TYPICAL FRAMING DETAILS

CONTRACTOR TO  
VERIFY DIMENSIONS

SYMBOLS LEGEND	
	DESIGNATES FOOTING LINE
	DESIGNATES SAWCUT LINE
	STEM WALL FOOTING
	DESIGNATES SLAB RECESS



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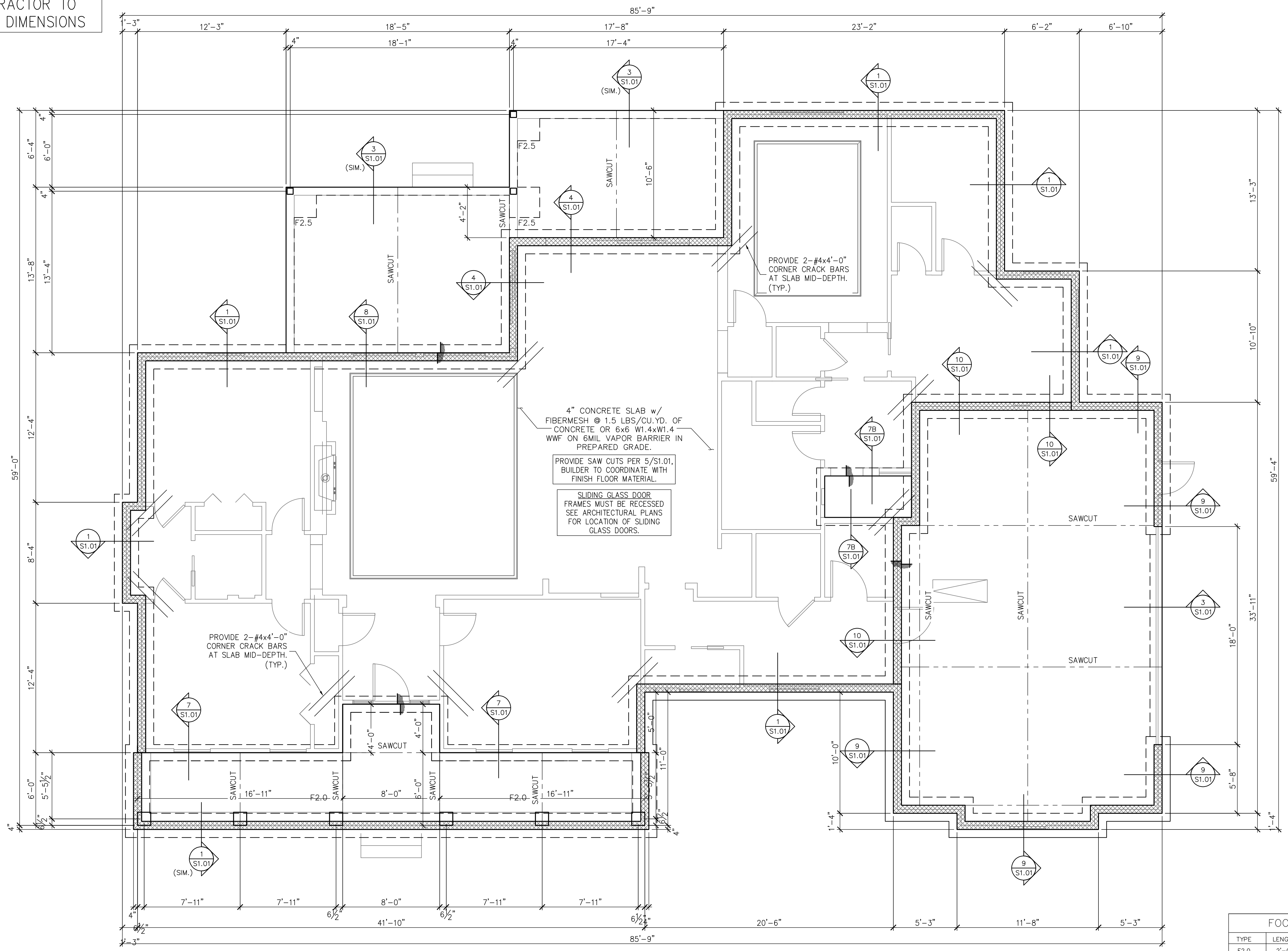
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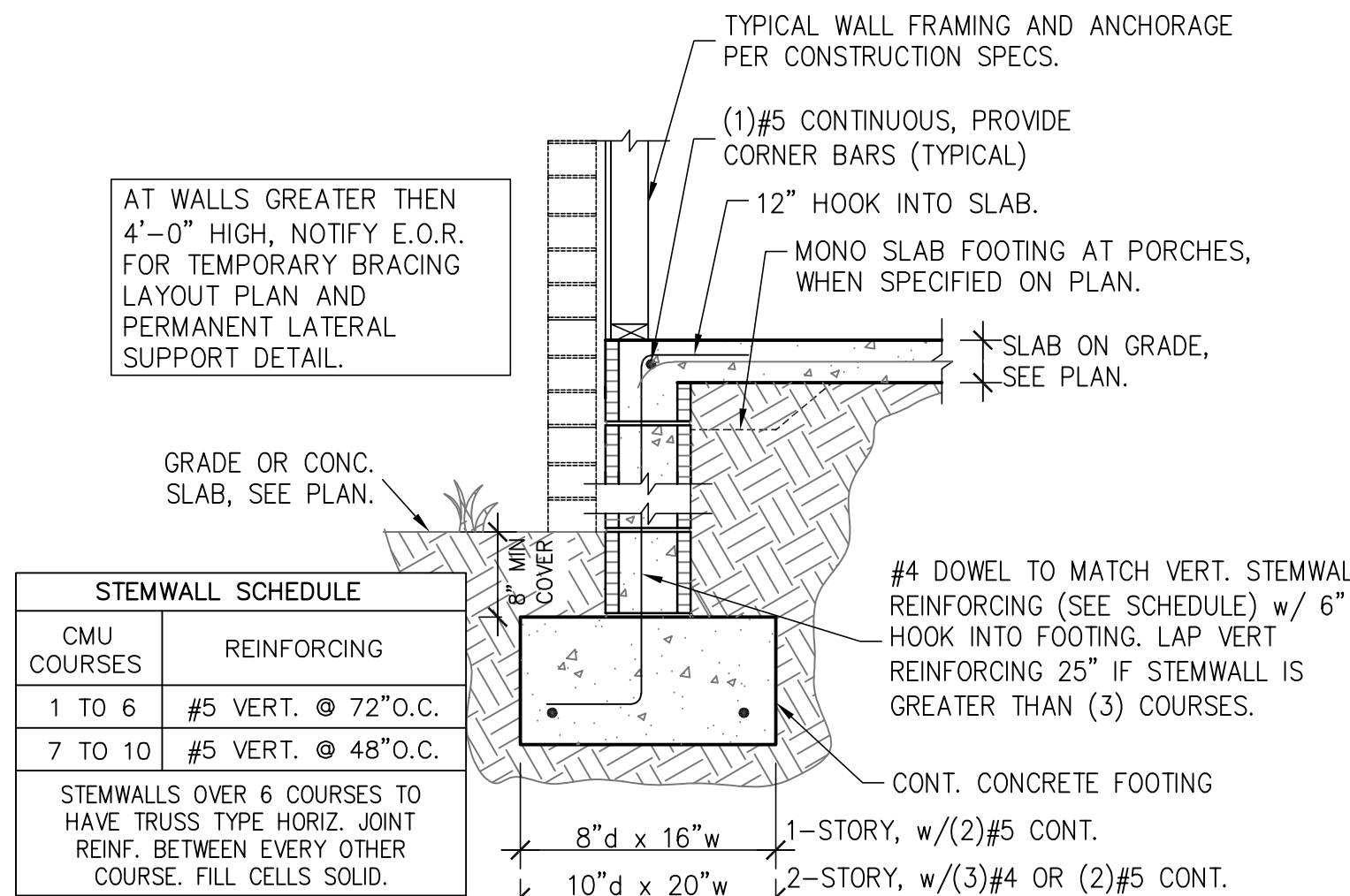
FOUNDATION  
PLAN

SHEET  
**S1.0**  
SHEET 3 OF 7

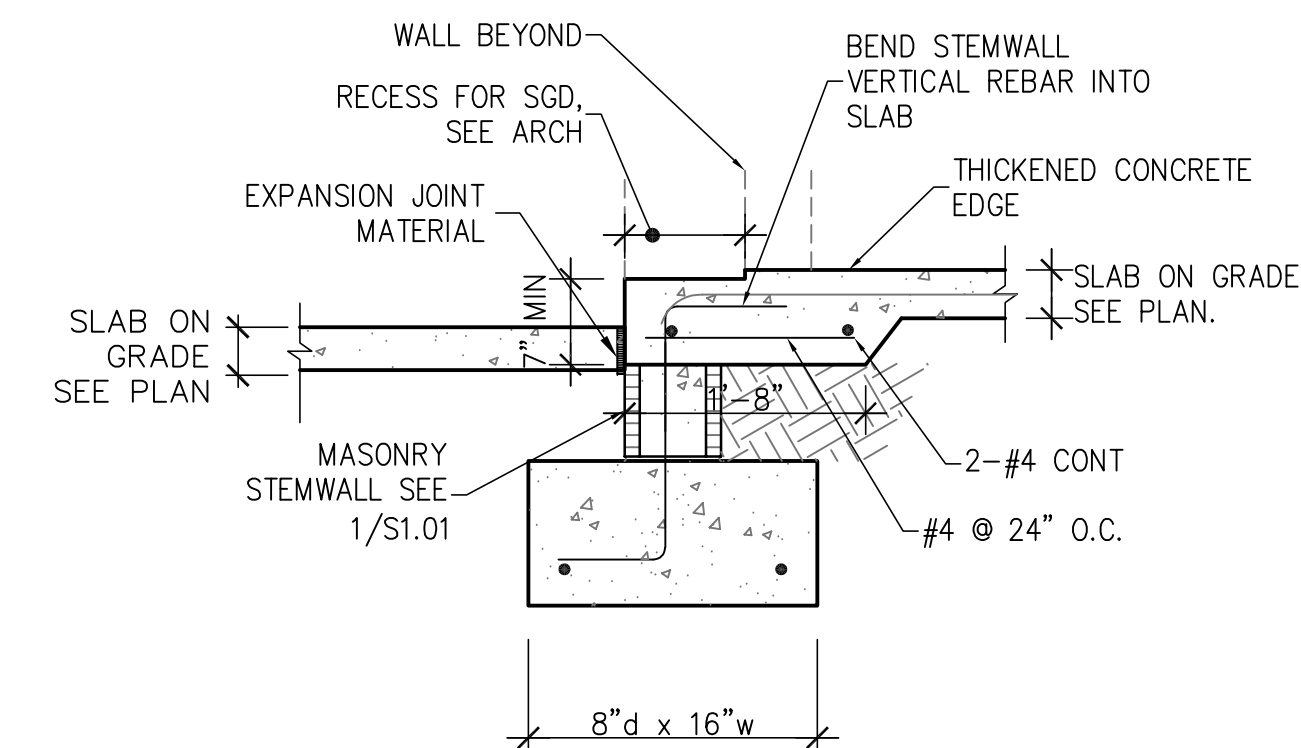


FOUNDATION PLAN  
SCALE: 1/4" = 1'-0"

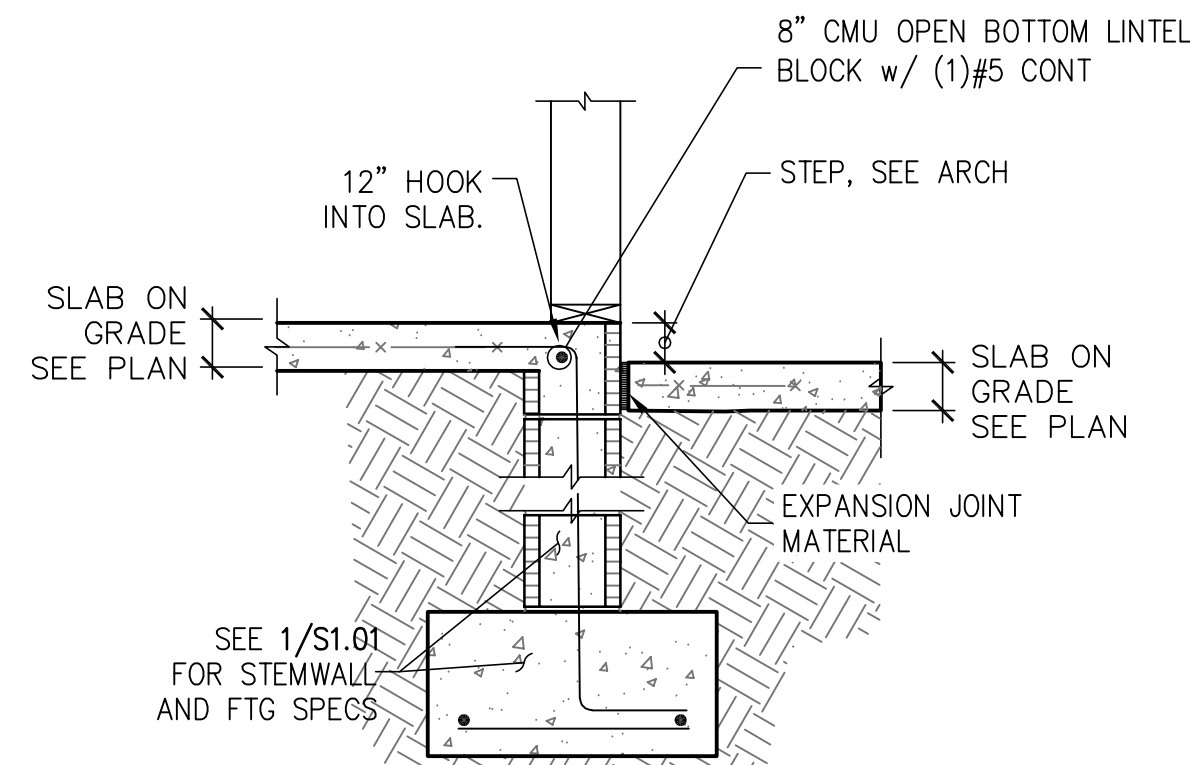
FOOTING SCHEDULE AND NOTES				
TYPE	LENGTH	WIDTH	DEPTH	BOTTOM BARS
F2.0	2'-0"	2'-0"	1'-0"	3-#5 EA. WAY BOT.
F2.5	2'-6"	2'-6"	1'-0"	3-#5 EA. WAY BOT.
F3.0	3'-0"	3'-0"	1'-0"	3-#5 EA. WAY BOT.
F3.5	3'-6"	3'-6"	1'-0"	4-#5 EA. WAY BOT.
F4.0	4'-0"	4'-0"	1'-0"	4-#5 EA. WAY BOT.
F4.5	4'-6"	4'-6"	1'-0"	4-#5 EA. WAY BOT.
1. THIS FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFO. RELATED TO THE FOUNDATION. FOR GENERAL FEATURES, DIMENSIONS, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ECT., SEE ARCH. PLAN. ARCHITECTURAL PLAN SHOWN HERE IN FOR REFERENCE ONLY.				
2. FTGS. & FND. SHALL BE IN ACCORDANCE w/ LOCAL BUILDING CODES.				
3. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.				



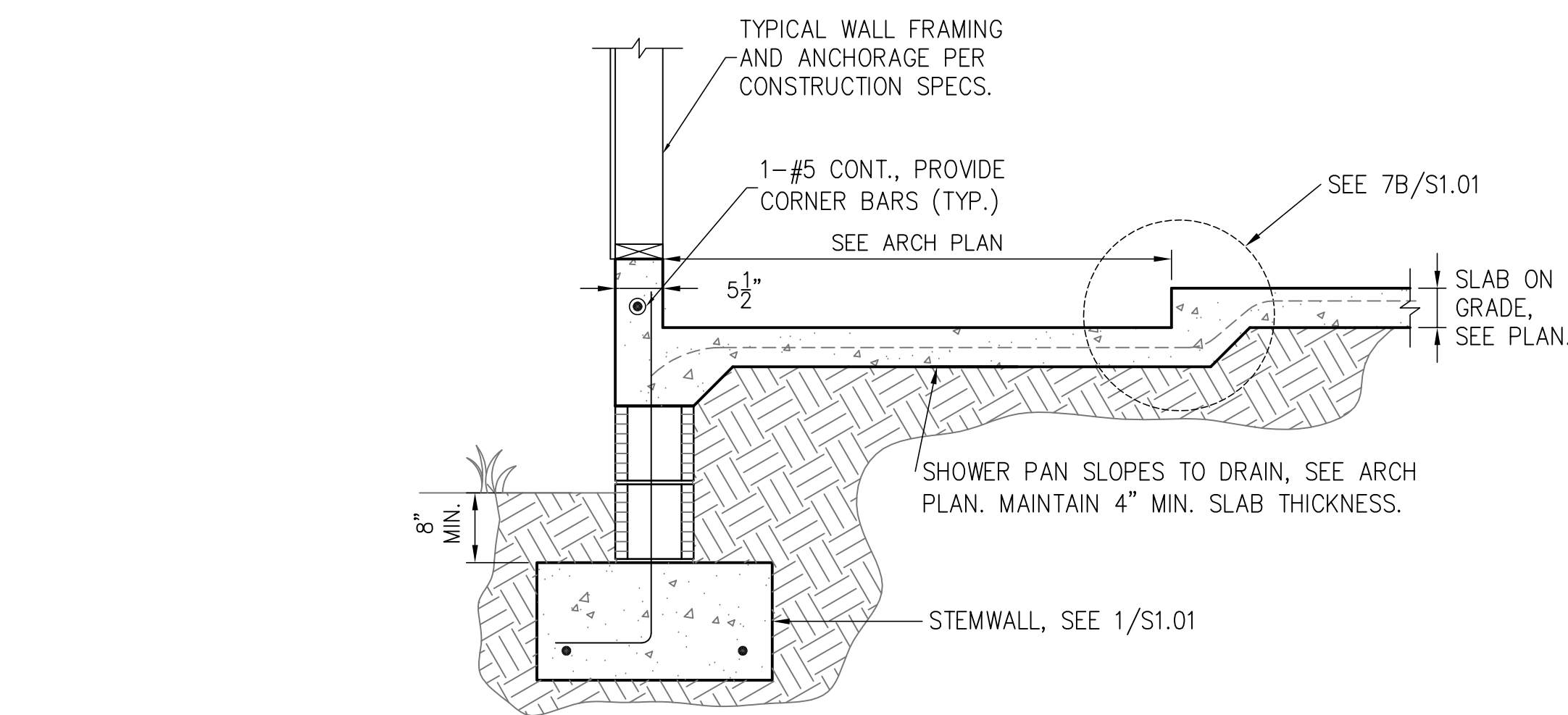
1 STEMWALL FOOTING  
S1.01 SCALE: 3/4" = 1'-0"



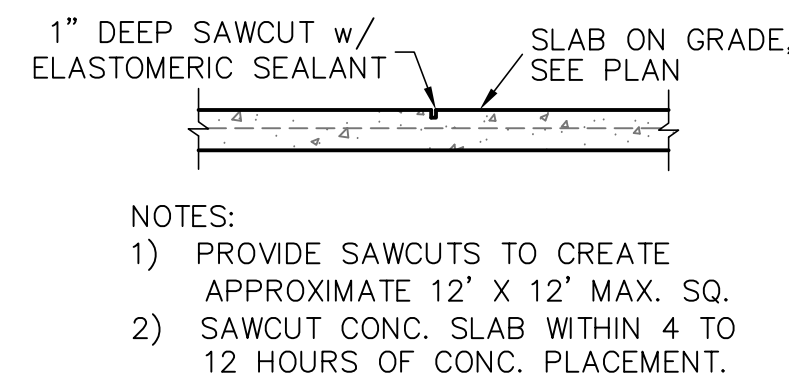
4 STEMWALL FOOTING AT SLIDER  
S1.01 SCALE: 3/4" = 1'-0"



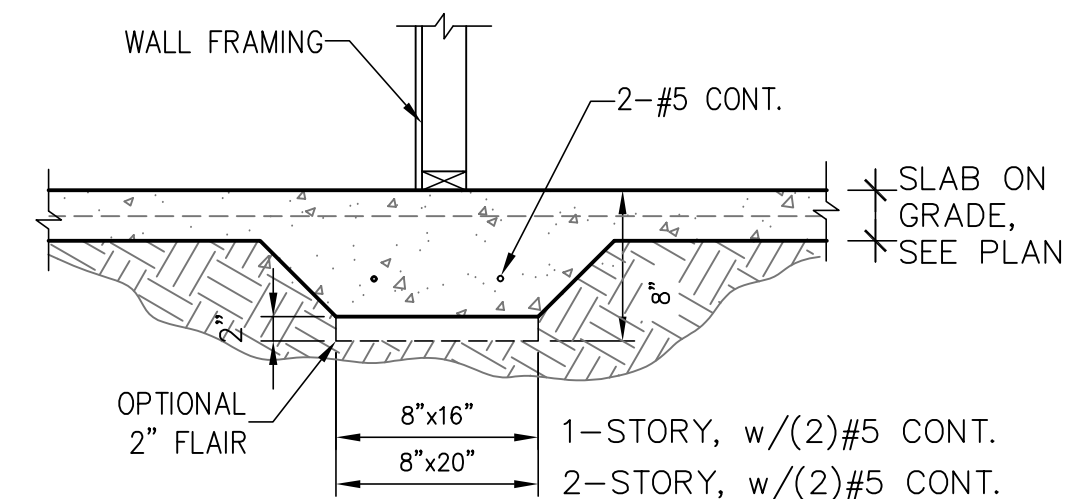
8 STEP AT STEMWALL  
S1.01 SCALE: 3/4" = 1'-0"



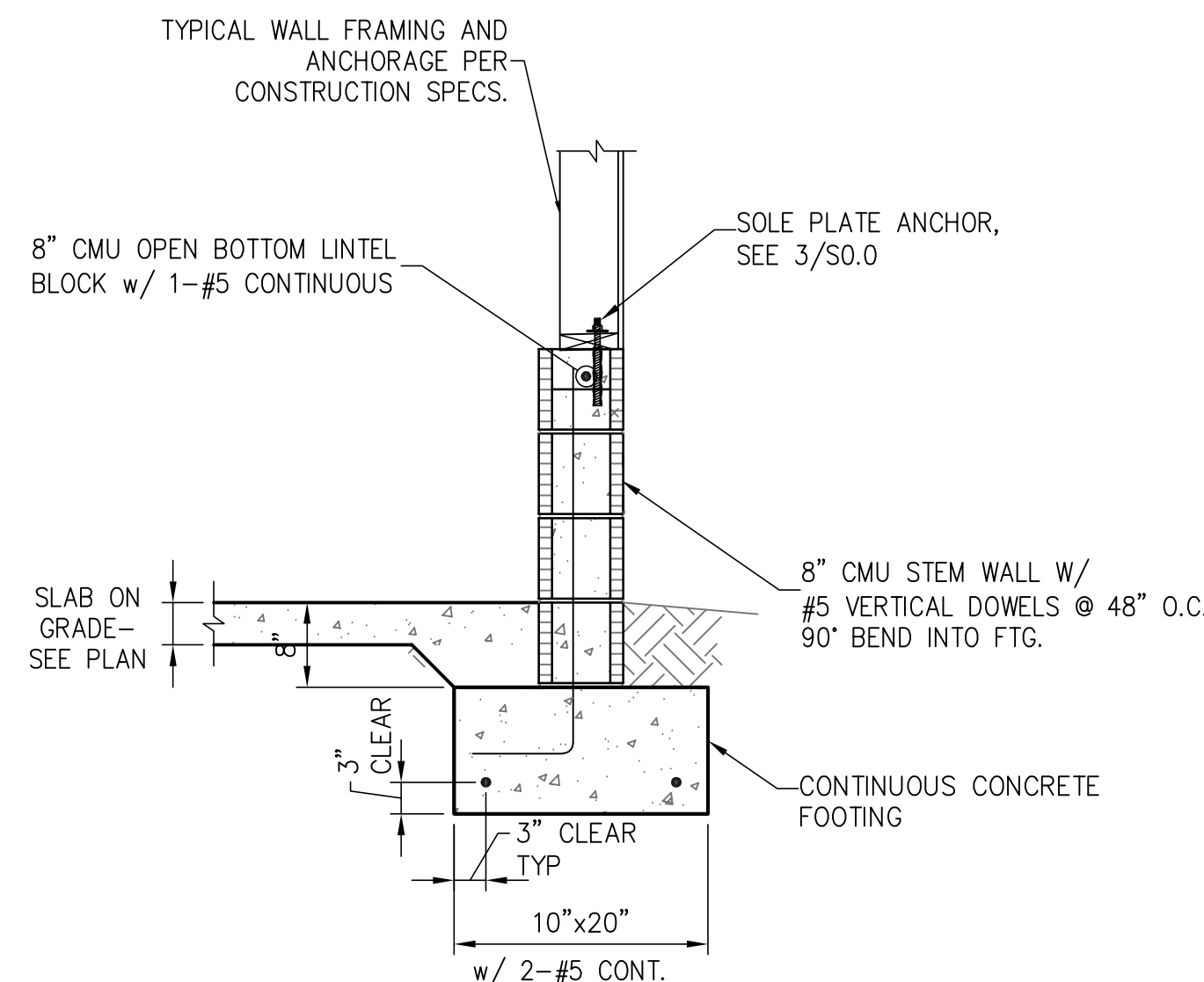
2 FOOTING W/ SHOWER RECESS  
S1.01 SCALE: 3/4" = 1'-0"



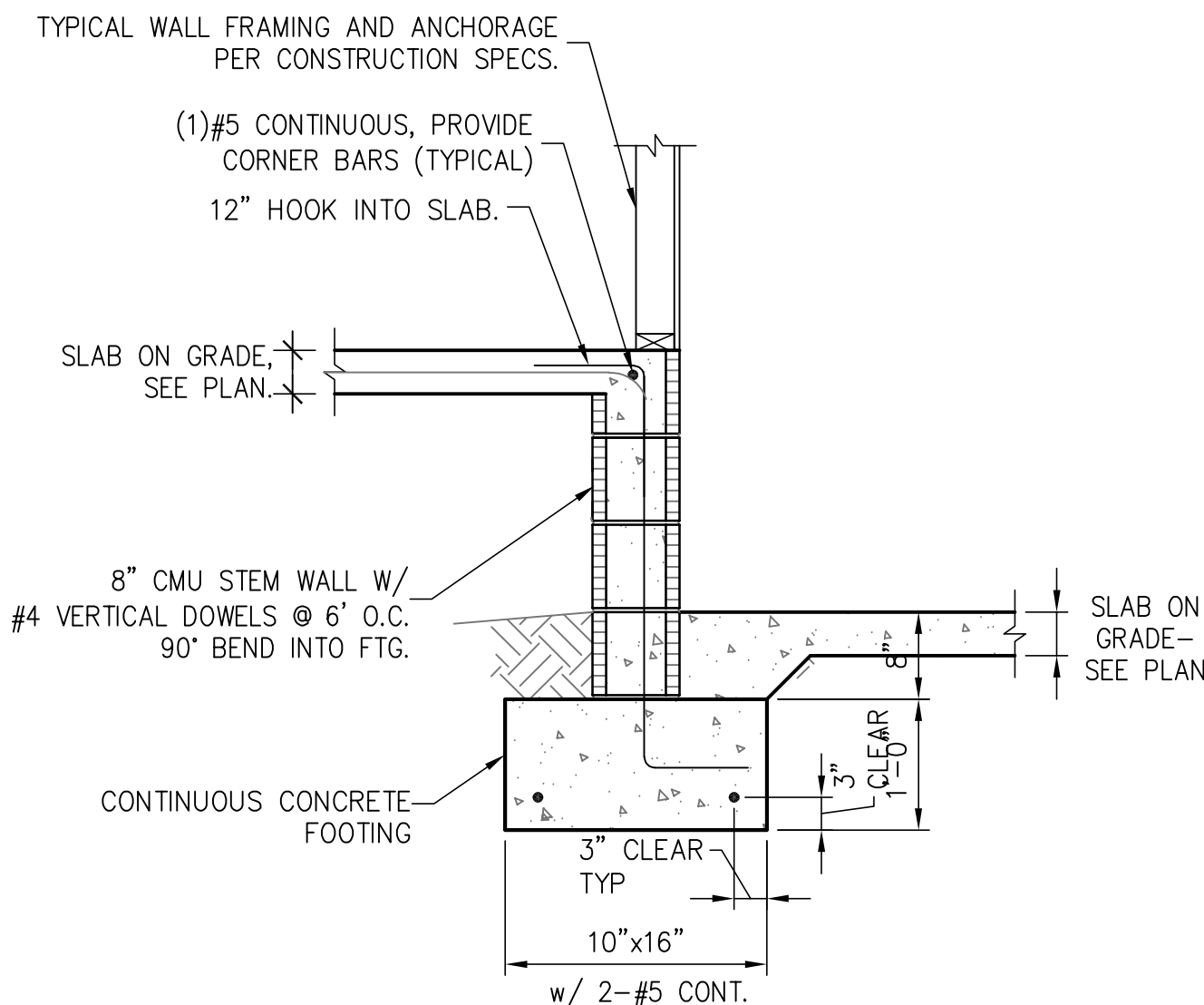
5 SAW CUT DETAIL  
S1.01 SCALE: 3/4" = 1'-0"



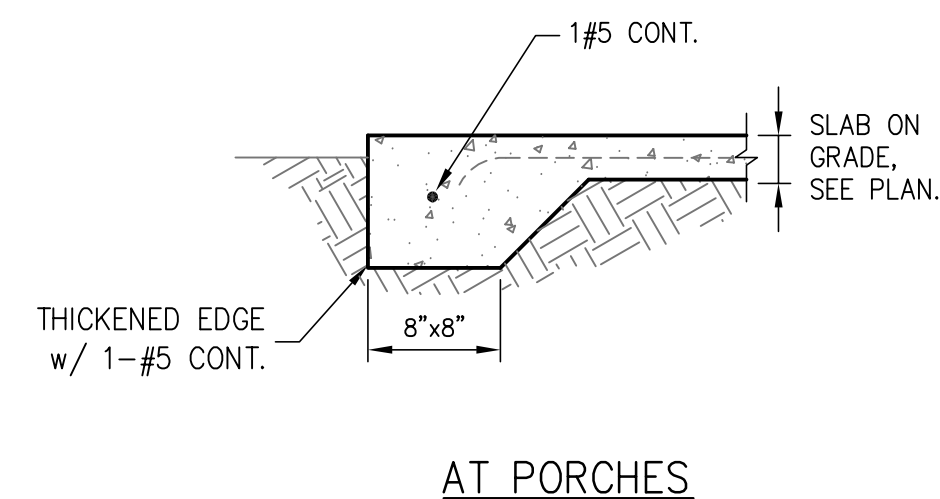
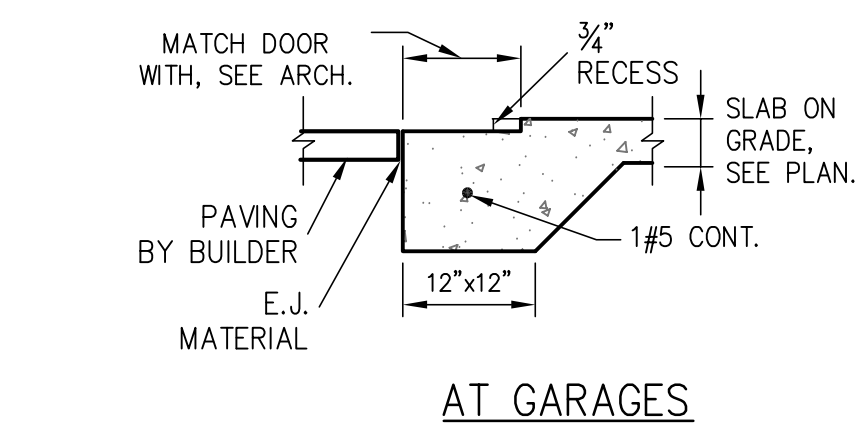
6 BEARING AT INTERIOR  
S1.01 SCALE: 3/4" = 1'-0"



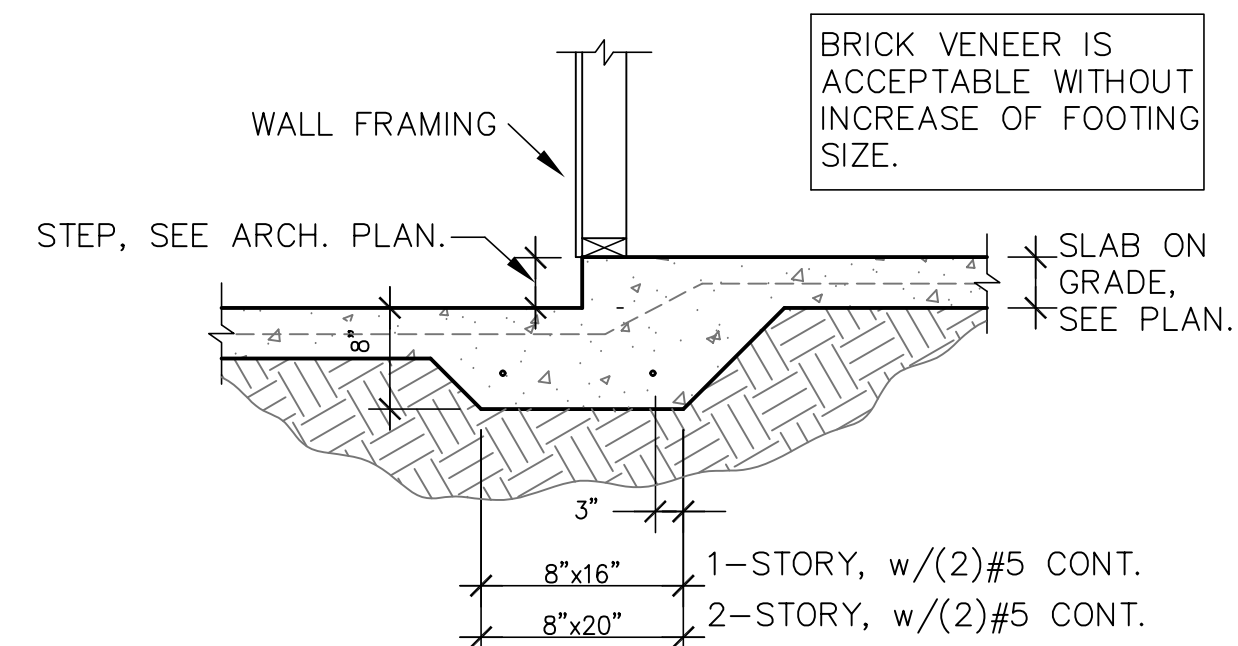
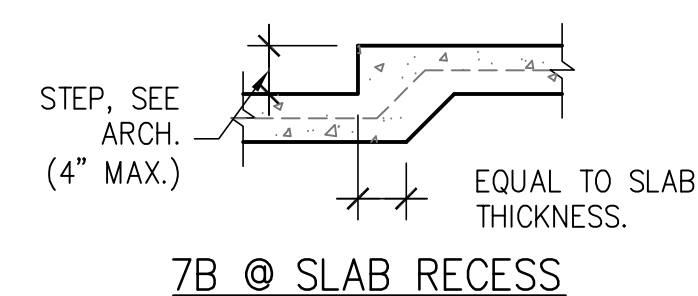
9 GARAGE STEM WALL  
S1.01 SCALE: 3/4" = 1'-0"



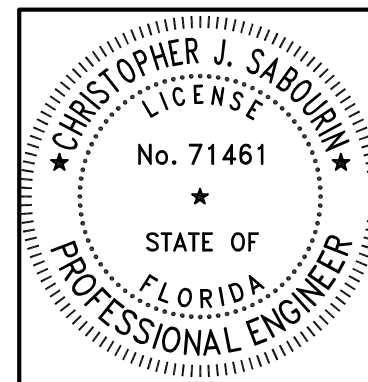
10 STEMWALL AT GARAGE  
S1.01 SCALE: 3/4" = 1'-0"



3 THICKENED SLAB  
S1.01 SCALE: 3/4" = 1'-0"



7 MONO. FOOTING AT STEP-DOWN  
S1.01 SCALE: 3/4" = 1'-0"



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STRUCTURAL ENGINEERING FOR  
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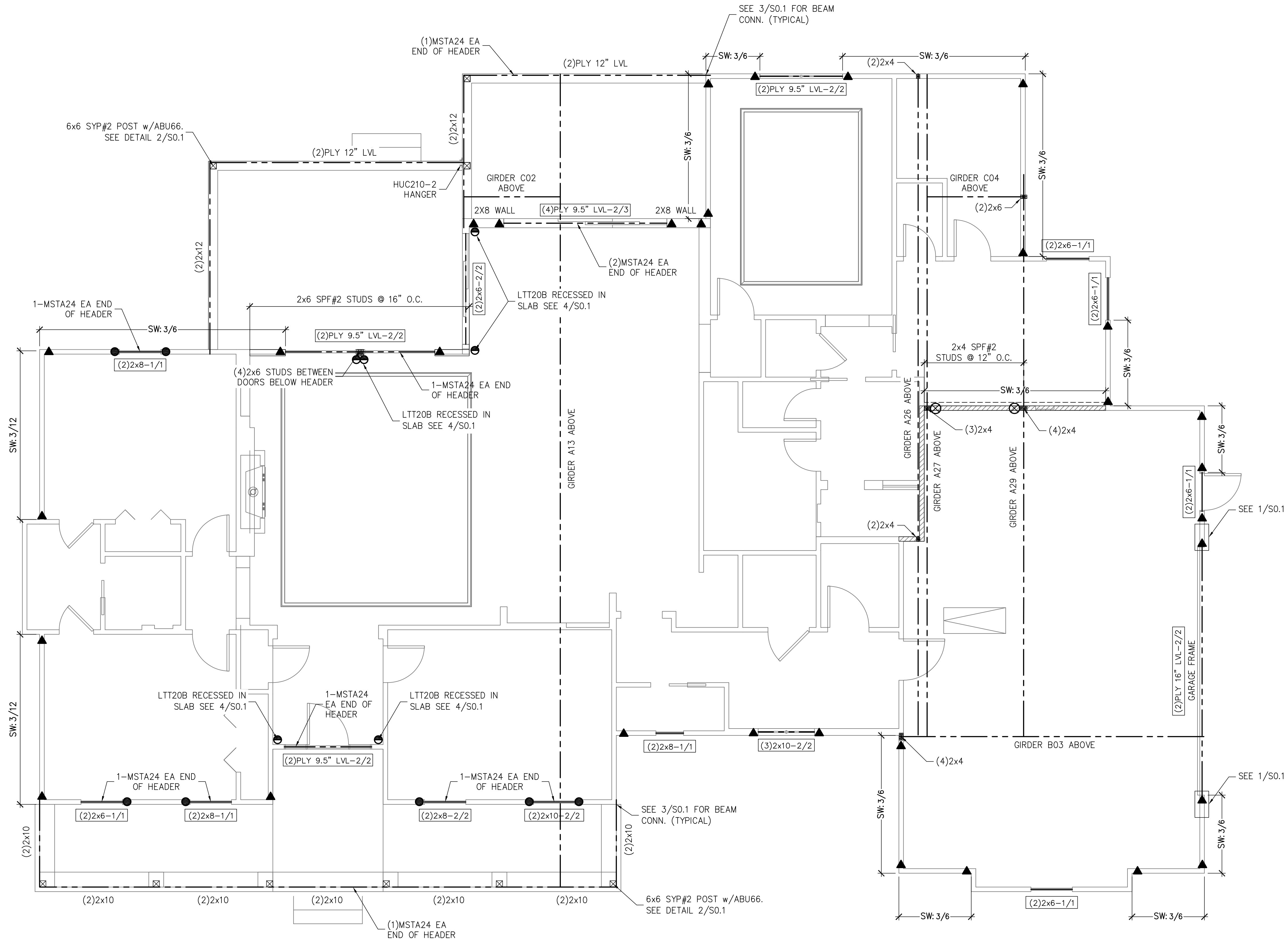
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FOUNDATION  
DETAILS

SHEET  
S1.01  
SHEET 4 OF 7





FIRST LEVEL WALL FRAMING PLAN  
SCALE: 1/4" = 1'-0"

SYMBOLS LEGEND

	DESIGNATES HIDDEN SHEARWALL. THE HIDDEN LINE DESIGNATES SIDE OF WALL. THE SHEARWALL SHEATHING TO BE APPLIED, 8d @ 6\"/>
	DESIGNATES THE HEADER SIZE, NUMBER OF PLYS & JACK/NOG STUDS NEEDED FOR SUPPORT HEADER.
	BEAM OR TRUSS, SEE PLAN

ANCHOR LEGEND

	3/4\"/>
	5/8\"/>
	3/4\"/>
	5/8\"/>
	SIMPSON HTTS SEE DETAIL 15/SO.1
	SIMPSON DTT22 SEE DETAIL 15/SO.1
	SIMPSON LTT20B SEE DETAIL 15/SO.1

WALL STUD SCHEDULE

LOCATION	PLATE HEIGHT	STUD SIZE & SPACING
EXTERIOR	9'-1\"/>	2x4 SPF#2 @ 16\"/>
EXTERIOR	10'-1\"/>	2x6 SPF#2 @ 16\"/>
EXTERIOR	10'-1\"/>	2x4 SPF#2 @ 12\"/>
EXTERIOR	10'-1\"/>	2x6 SPF#2 @ 16\"/>
INTERIOR	10'-0\"/>	2x4 SPF#2 @ 16\"/>
INTERIOR	12'-0\"/>	2x6 SPF#2 @ 16\"/>

- STUD NOTES:**
1. WALL STUDS SPECIFIED ON PLAN SUPERSEDE THIS TABLE.
  2. MINIMUM STUD SIZE AND SPACING ARE SHOWN. CONTRACTOR MAY INCREASE STUD SIZE TO MEET ARCHITECTURAL REQUIREMENTS.
  3. SPF DENOTES SPRUCE PINE FIR. SYP DENOTES SOUTHERN YELLOW PINE.
  4. USE SYP#2 FOR ALL TOP PLATES AND SOLE PLATES.
  5. FASTEN BOTTOM PLATE OF INTERIOR LOAD BEARING WALLS TO CONCRETE SLAB w/16d MASONRY CUT NAILS @ 16\"/>

COMBINED USE PANEL NOTES

1. EXTERIOR WALL SHEATHING SHALL BE CONTINUOUS FROM BOTTOM PLATE TO UPPER MOST TOP PLATE. SEE DETAIL 1/SO.1 FOR SHEATHING SPLICE LOCATIONS FOR MULTI STORY CONDITIONS.
2. SEE SHEET SO.0 FOR WALL SHEATHING SPECIFICATIONS.
3. UPPER MOST TOP PLATE SUPPORTING ROOF MEMBERS SHALL BE STRAPPED AS SHOWN IN DETAIL 1/SO.0.
4. INSTALL SOLE PLATE ANCHORS PER DETAIL 3/SO.0.

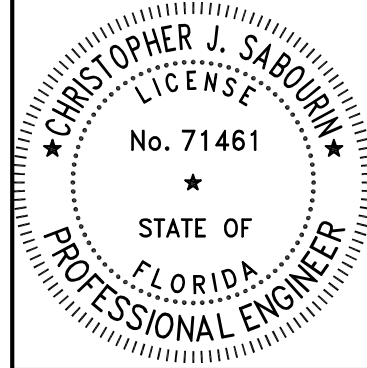
GENERAL NOTES

1. SEE DETAIL 2/SO.0 FOR WALL FRAMING DETAIL. SEE WALL STUD SCHEDULE THIS SHEET FOR STUD SIZES AND SPACING. AT GIRDERS AND BEAMS, PROVIDE STUDS BELOW TO MATCH BEAM/GIRDER PLYS.
2. SEE SHEET SO.0 FOR ROOF AND FLOOR SHEATHING SPECIFICATIONS.
3. WHERE FRAMING MEMBERS CONSIST OF MULTIPLE PLIES (BEAMS, HEADER, AND STUDS) FASTEN PLIES TOGETHER PER DETAIL 6/SO.0.
4. INSTALL SOLE PLATE ANCHORS PER DETAIL 3/SO.0.
5. AT SHEARWALLS, PROVIDE DIAPHRAGM ATTACHMENT PER DETAIL 5/SO.1.
6. FOR ATTACHMENT OF EXTERIOR WALLS THAT TERMINATE BETWEEN TRUSSES, SEE 5A/SO.1.
7. AT PORCHES, SEE DETAIL 2/SO.1 FOR FRAMING AND HOLD DOWNS.

SOLE PLATE ANCHOR SPACING SCHD

ALL EXTERIOR WALL UNLESS OTHER NOTED	42\"/>
SHEARWALLS (SW 8d@3\"/>	24\"/>
SOLE PLT @ #	WHEN NOTED ON PLAN SEE NOTE 2

1. INSTALL SOLE PLATE ANCHORS PER DETAIL 3/SO.0.
2. ANCHOR SPACING SHALL BE AS NOTED, FOR EXAMPLE - SOLE PLT @ 36\"/>



06.27.22  
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**SABO**  
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PLAN NAME  
B2EC  
SSE No.  
22-0287

ISSUE	DATE
PERMIT	06.27.22
REVISIONS	DATE

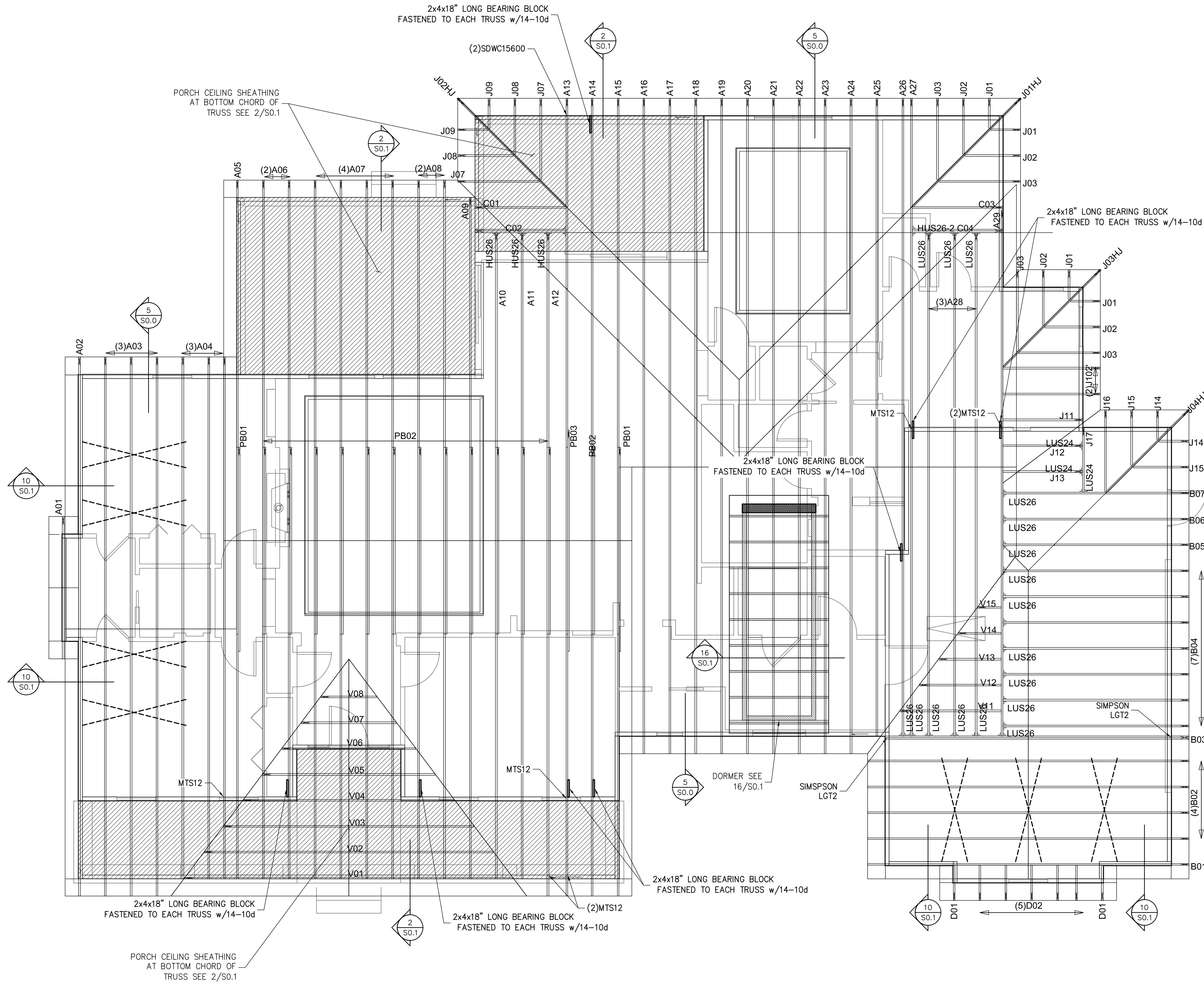
STRUCTURAL ENGINEERING FOR  
THE JUDSON RESIDENCE

**FIELD ALTERATION**  
CONTRACTOR SHALL CONTACT SABO STRUCTURAL ENGINEERING PRIOR TO MAKING ANY STRUCTURAL FIELD MODIFICATIONS WHICH MAY VARY FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY FIELD ALTERATIONS MADE PRIOR TO BEING APPROVED BY CHRISTOPHER SABOURIN MAY RESULT IN ADDITIONAL ENGINEERING OR INSPECTION FEES.

**SCALING**  
DO NOT SCALE DIMENSIONS FROM THESE DRAWINGS. IF A DIMENSION IS UNCLEAR REFER TO THE ARCHITECTURAL DRAWINGS OR CONTACT THE E.O.R.

FIRST LEVEL  
WALL  
FRAMING  
PLAN

SHEET  
S1.1  
SHEET 5 OF 7



TRUSS / ROOF RAFTER NOTES: STRAPPING NOTES

- STRAP ROOF TRUSSES AND RAFTERS TO BEARING WITH 2-12D TOENAILS & 1-SIMPSON SDWC15600 SCREW UNLESS OTHERWISE NOTED

ROOF TRUSS PLACEMENT PLAN  
SCALE: 1/4" = 1'-0"

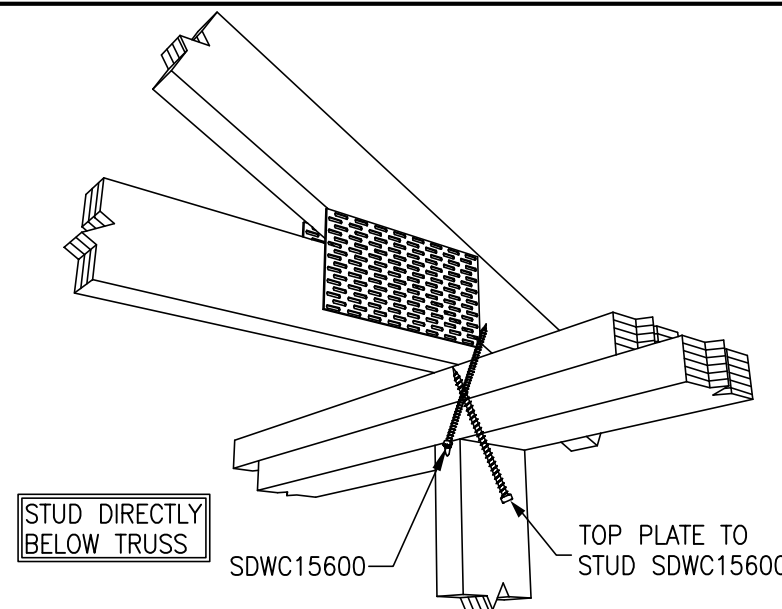
SYMBOLS LEGEND

HTS16 DESIGNATES UPLIFT CONNECTION.

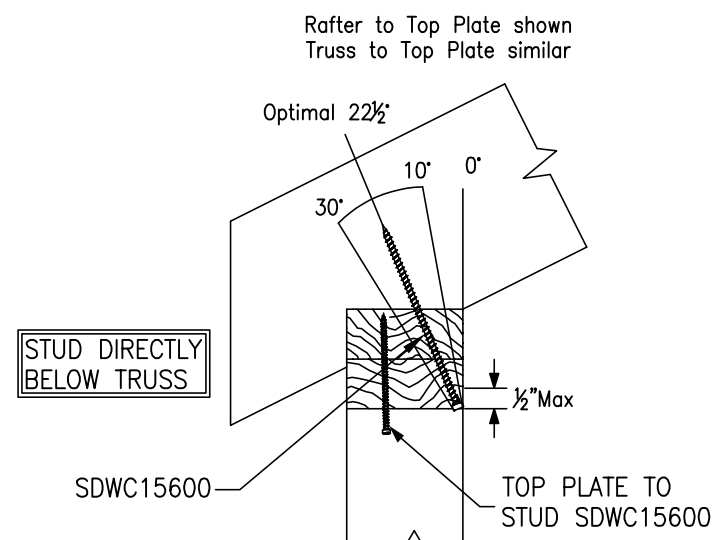
FRAMING PLAN NOTES:

- FOR TYPICAL ROOF SHEATHING AND FRAMING, SEE SHEET S0.0.
- FOR SPECIFIC UPLIFT CONNECTORS, SEE PLAN, MIN. (1)SDWC CONNECTOR.
- FOR GENERAL DESIGN SPECIFICATIONS SEE SHEET S0.0.
- WHEN USING (2)H2S1 CLIPS ON 1/2" WIDE LUMBER, PLACE CLIPS DIAGONALLY ACROSS DOUBLE TOP PLATE FROM EACH OTHER.

TRUSS FASTENING DETAILS

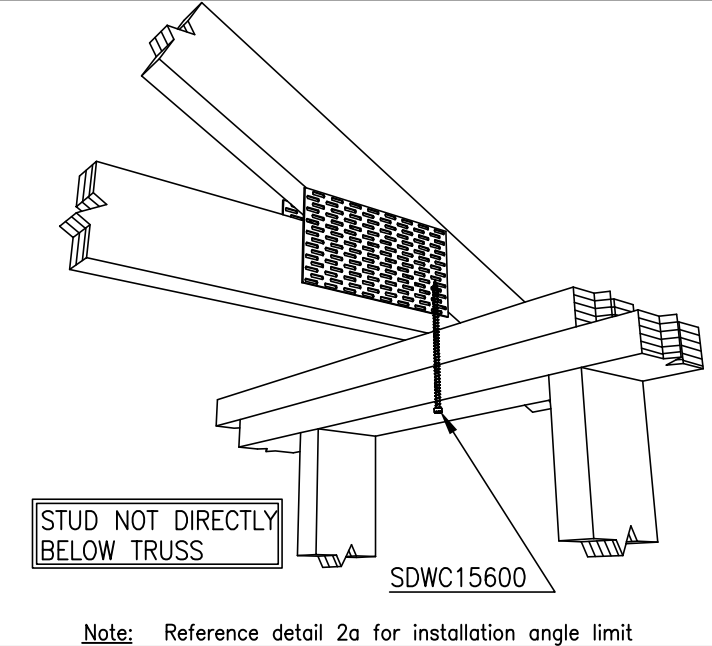


TRUSS TIE DOWN WITH SIMPSON SDWC

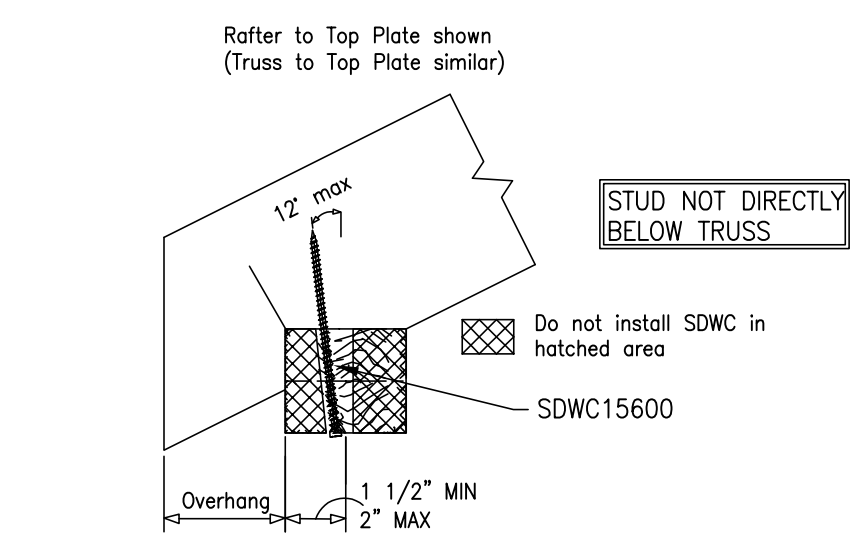


Note: 1. Sloped-roof rafters may be sloped up to and including a 12:12 pitch and must be "birds-mouth" cut.  
2. Reference detail 4 for installation instructions.

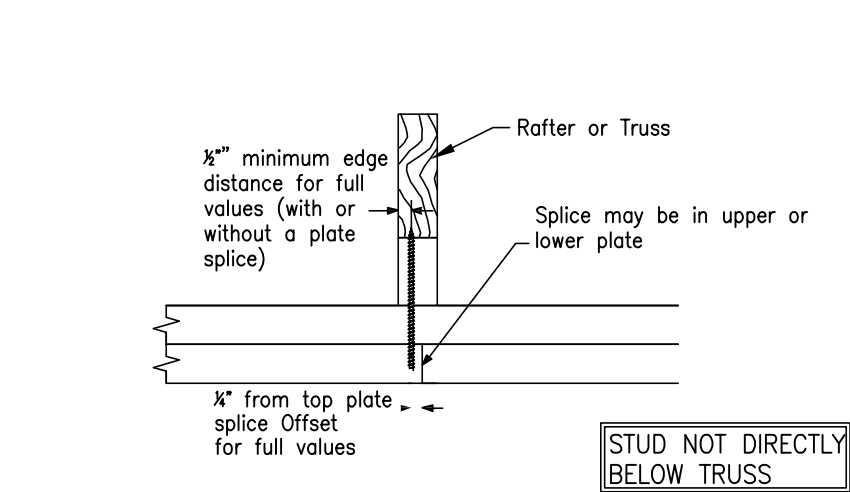
SIMPSON SDWC INSTALLATION RANGE



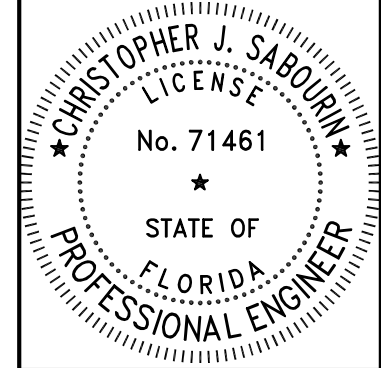
SDWC INSTALLATION



SDWC INSTALLATION RANGE



SDWC AT TOP PLATE SPLICE



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ROOF TRUSS  
PLACEMENT  
PLAN

SHEET  
**S1.2**  
SHEET 6 OF 7