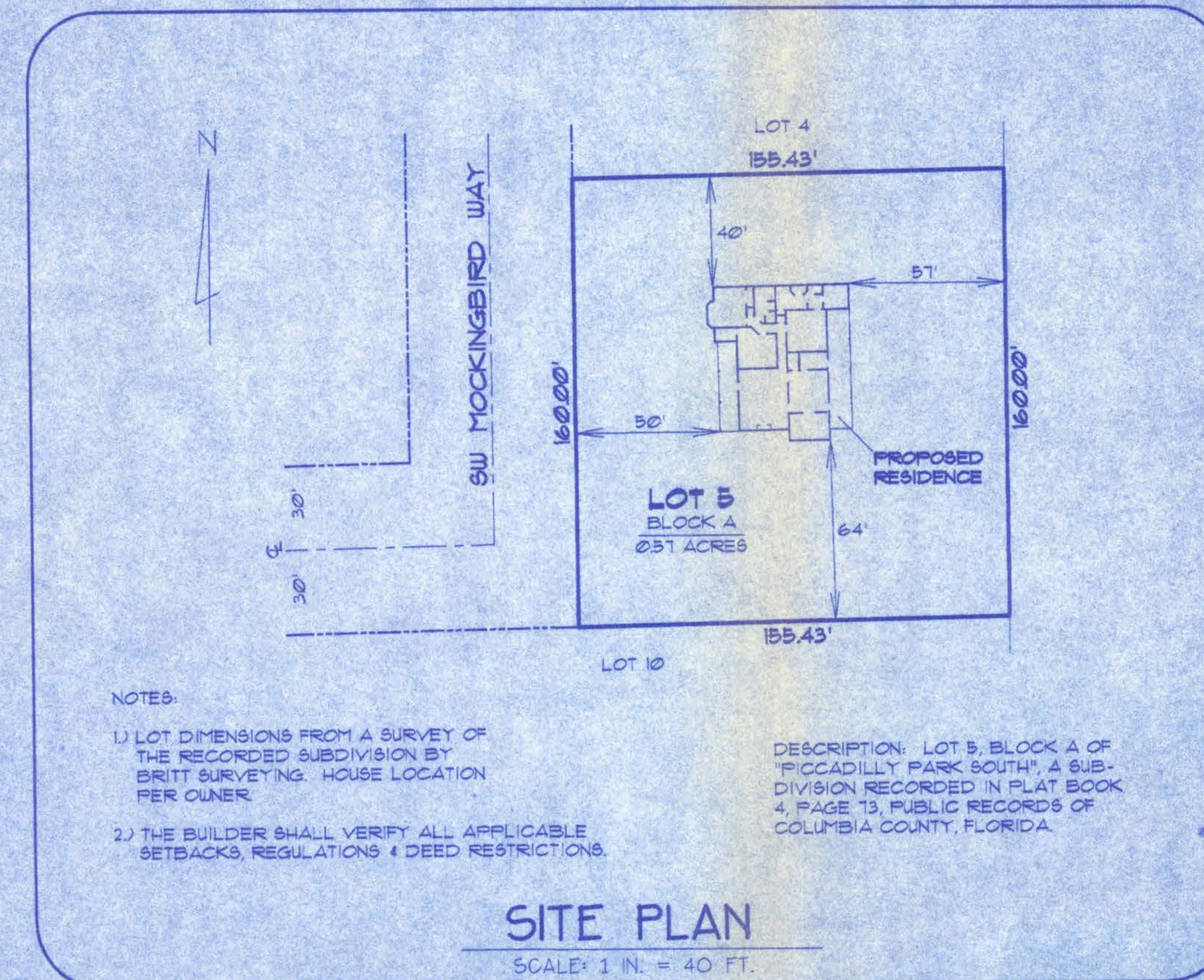
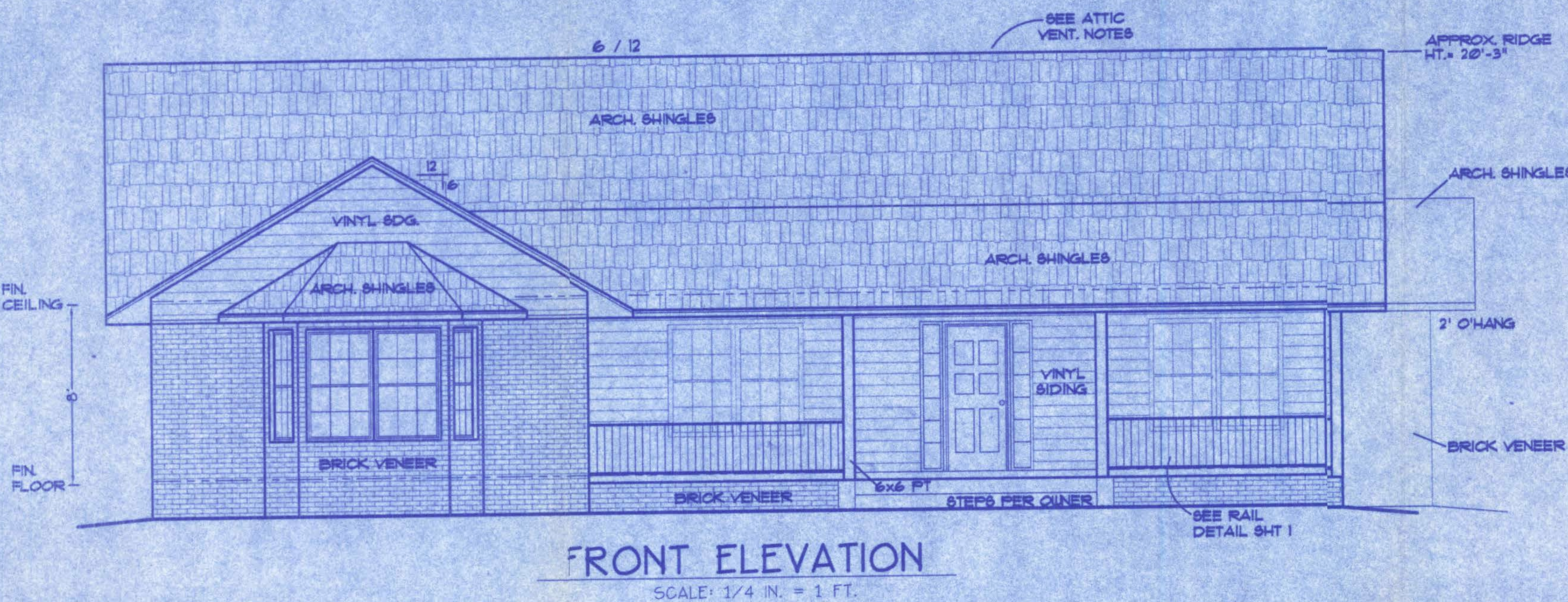
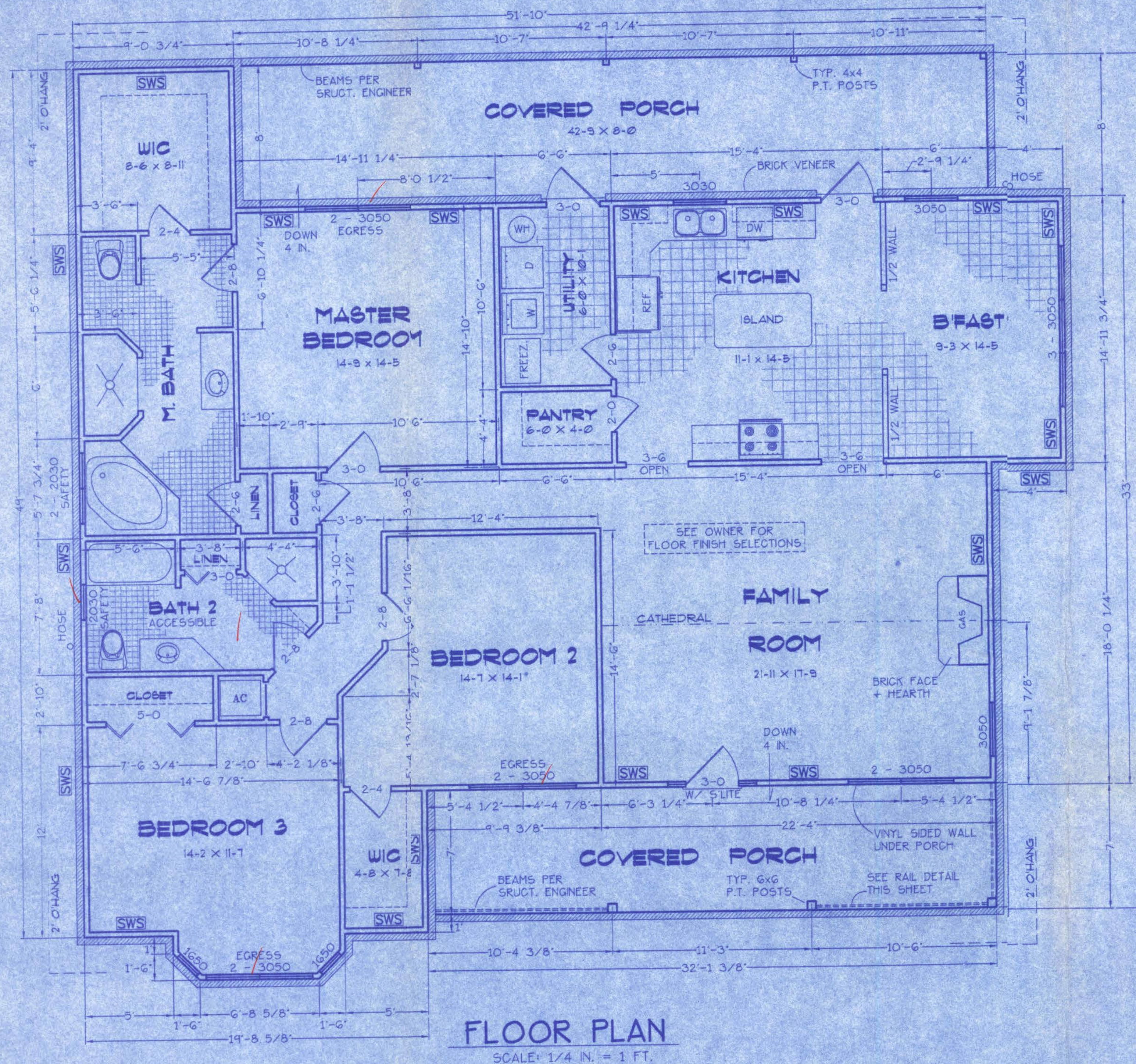


# Wrye Residence



## ATTIC VENTILATION

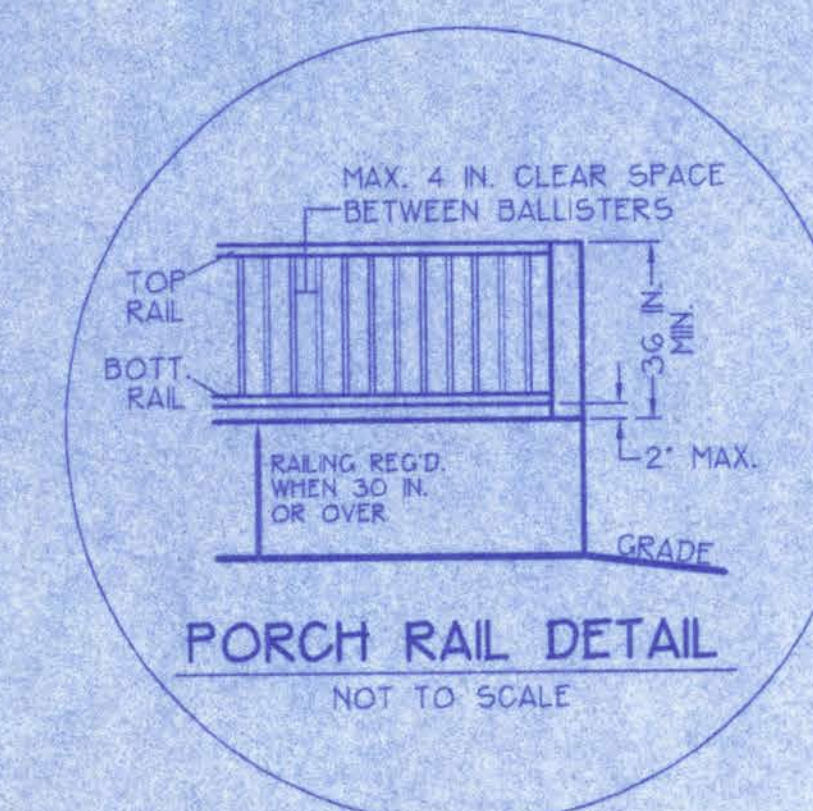
Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with a 1/8 inch (3.2 mm) minimum to 3/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

**SWS** - Indicates a shearwall segment location referring to the labeled section of wall lying between the adjacent window / door openings in either direction. The shearwall areas have a height/width aspect ratio of 3-1/2 : 1 or wider.

## AREA SUMMARY

CONDITIONED - - - - - 2030 SF  
PORCHES - - - - - 567 SF  
ROOF - - - - - 2597 SF



## Index to Sheets

SHEET A-1 - - - - - SITE PLAN  
+ FLOOR PLAN  
+ ELEVATION  
SHEET A-2 - - - - - ELEVATIONS  
+ GEN. NOTES  
SHEET A-3 - - - - - FOUNDATION  
+ SECTIONS  
SHEET A-4 - - - - - ELECTRICAL  
SHEET S-1 - - - - - WIND ENGINEERING

**A-1**

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

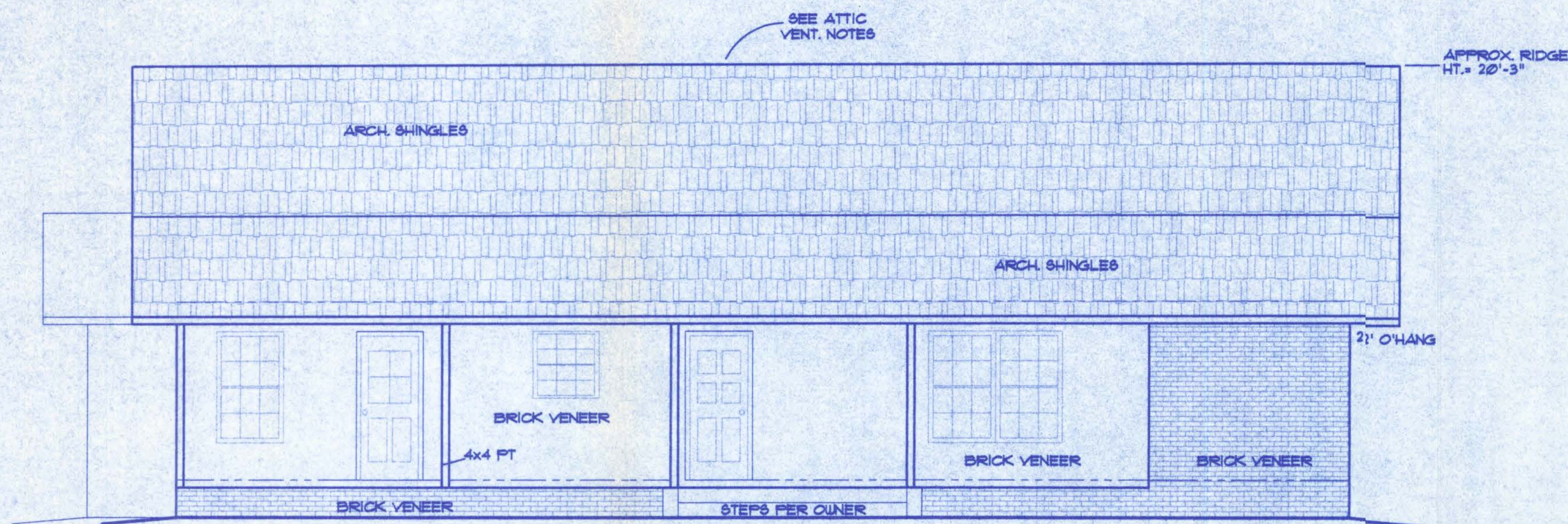
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: LOT 5, BLOCK A, PICCADILLY PARK SOUTH  
Job No.: 05-046

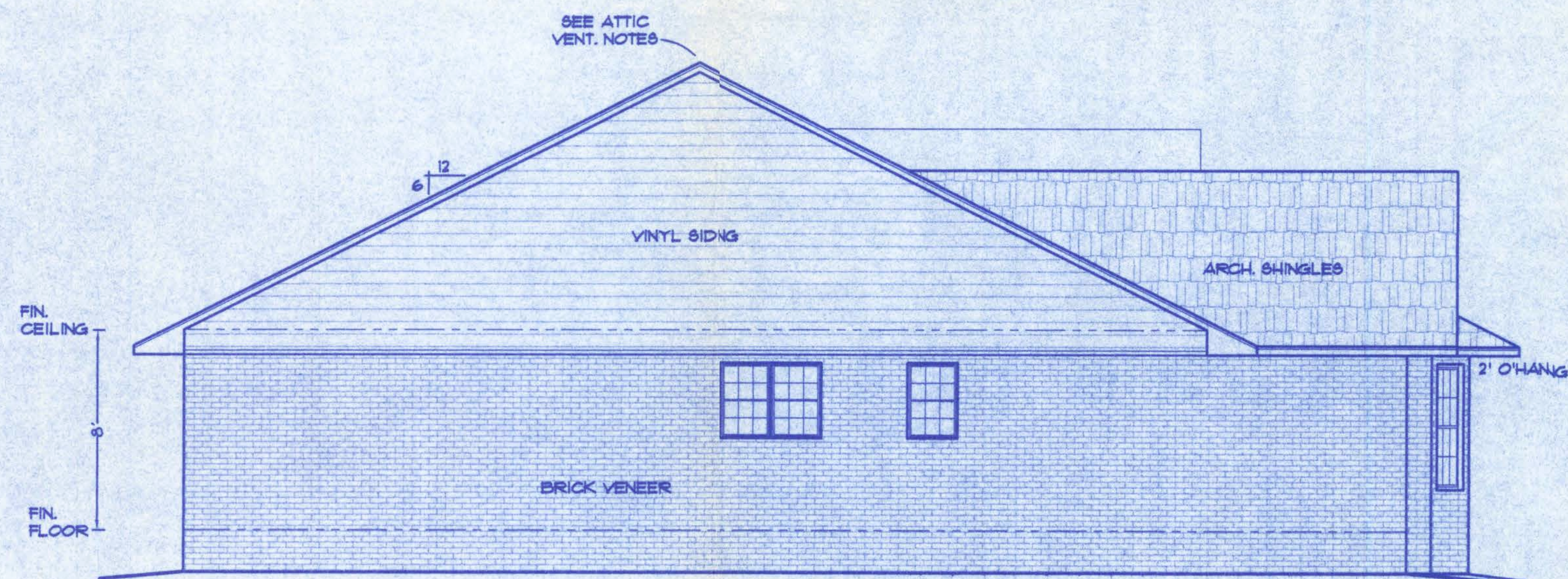
|                 |                                                                                                                                          |                    |
|-----------------|------------------------------------------------------------------------------------------------------------------------------------------|--------------------|
| FILE:<br>05-046 | <b>WRYE<br/>RESIDENCE</b>                                                                                                                | SHEET:<br>1 OF 4   |
| DATE:<br>2-4-06 |                                                                                                                                          | CAD-FILE:<br>05046 |
| DRAWN:<br>T A D | PREPARED BY:<br><b>TIM DELBENE</b><br>Drafting + Technical Services<br>192 SW Sagewood Cir. Lake City, FL 32024<br>Phone: (386) 755-5891 | REV:               |
| CHECK:<br>T A D |                                                                                                                                          | REV:               |

OFFICE COPY



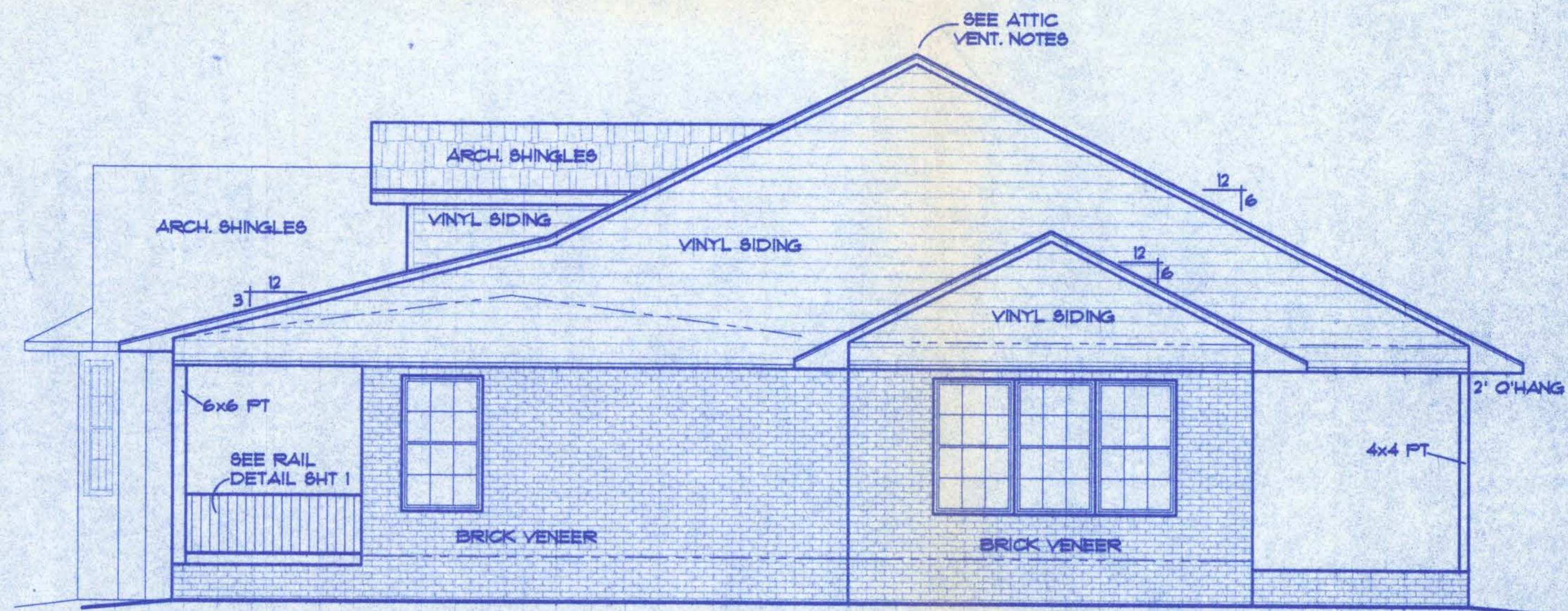
## REAR ELEVATION

SCALE: 1/4 IN. = 1 FT.



## LEFT ELEVATION

SCALE: 1/4 IN. = 1 FT.



## RIGHT ELEVATION

SCALE: 1/4 IN. = 1 FT.

## GENERAL NOTES

- 1.) See Wind Load Detail Sheet S-1 and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufacturer shall prepare Shop Drawings indicating Truss placement, Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis or preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

## ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion-resistant wire mesh, with 1/8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

# A-2

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

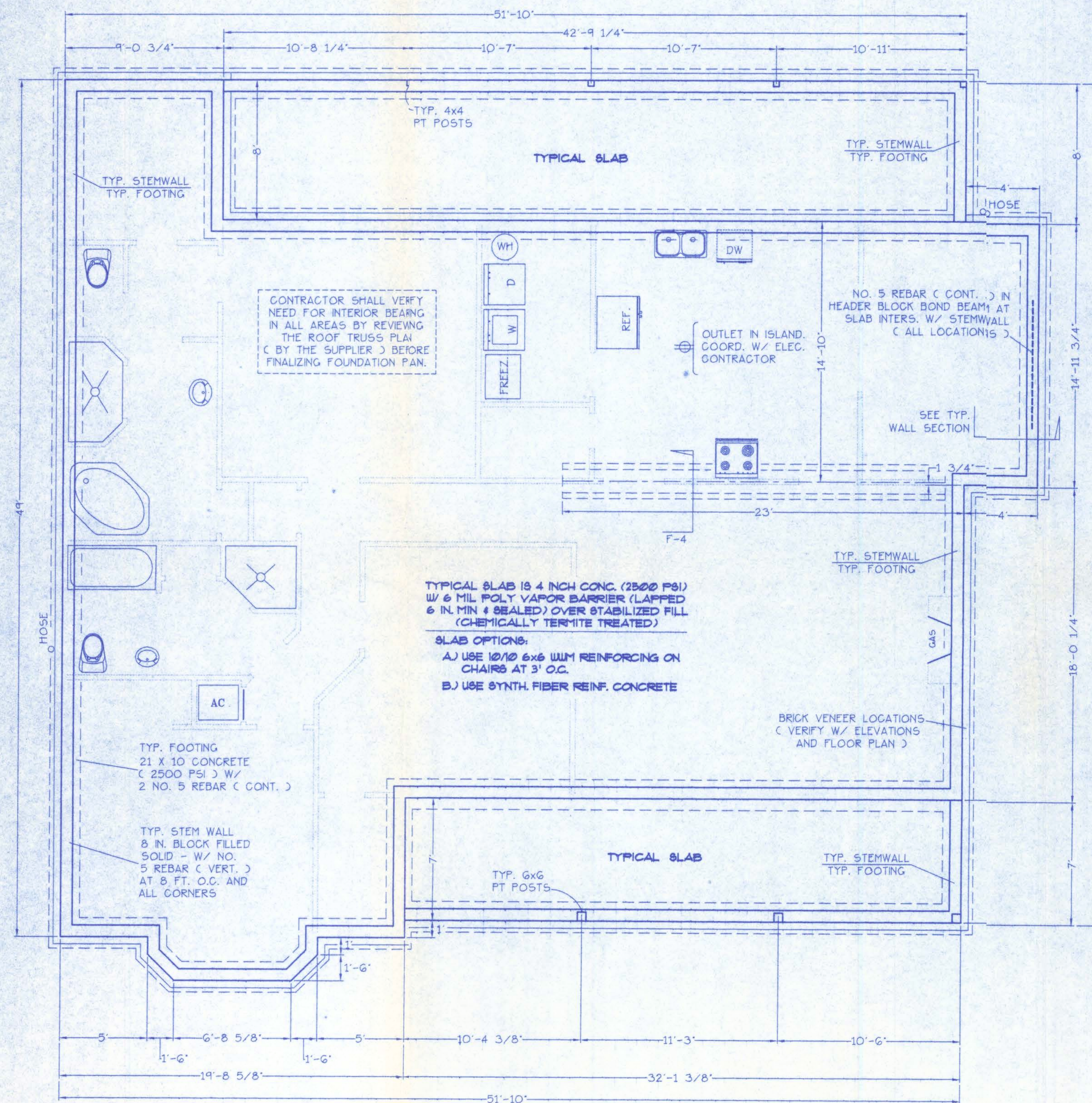
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: ~~LOT 5, BLK A~~  
LOT 5, BLK A  
PICCADILLY PARK SOUTH

Job No.:

|                 |                   |                    |
|-----------------|-------------------|--------------------|
| FILE:<br>05-04G | WRYE<br>RESIDENCE | SHEET:<br>2 OF 4   |
| DATE:<br>2-4-06 |                   | CAD FILE:<br>0504G |
| DRAWN:<br>T A D |                   | REV:               |
| CHECK:<br>T A D |                   | REV:               |

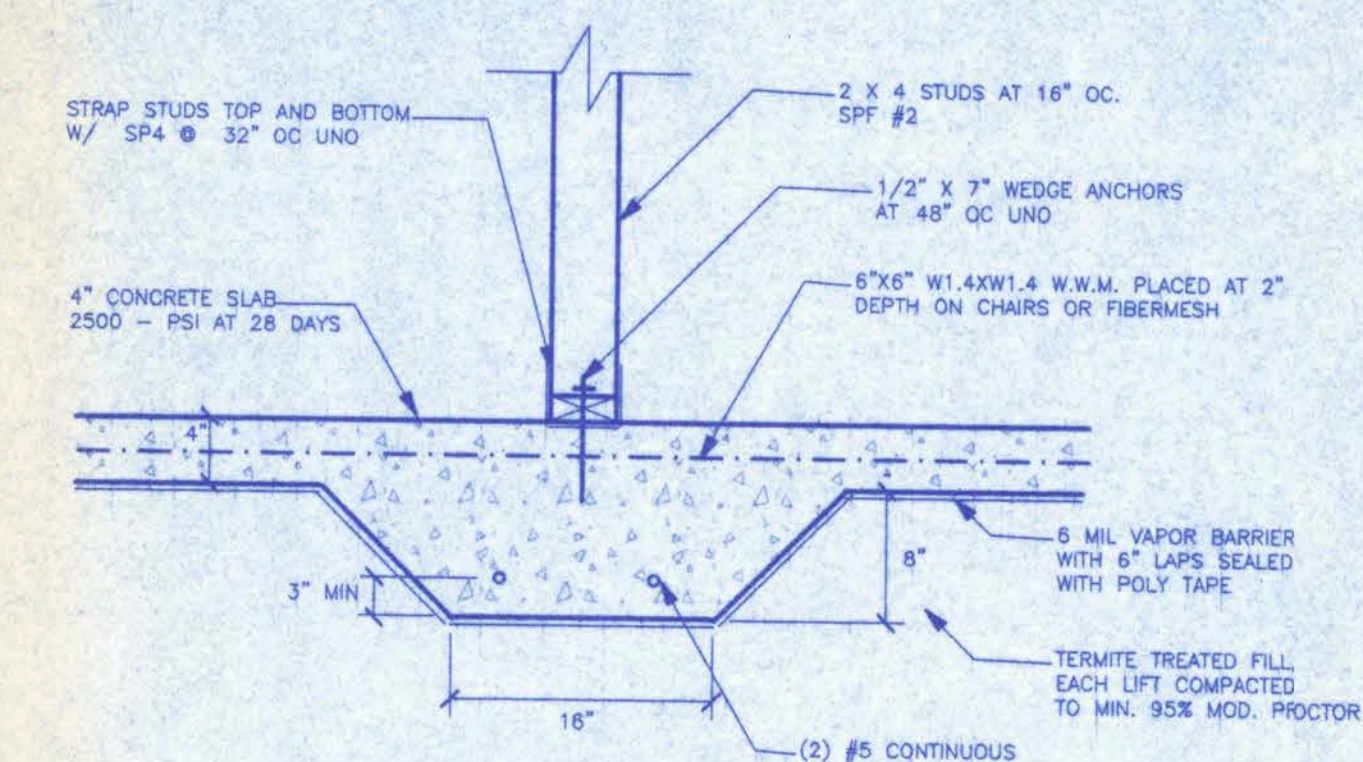
PREPARED BY:  
TIM DELBENE  
Drafting + Technical Services  
192 SW Sagewood Dr. Lake City, FL 32024  
Phone (386) 755-5891



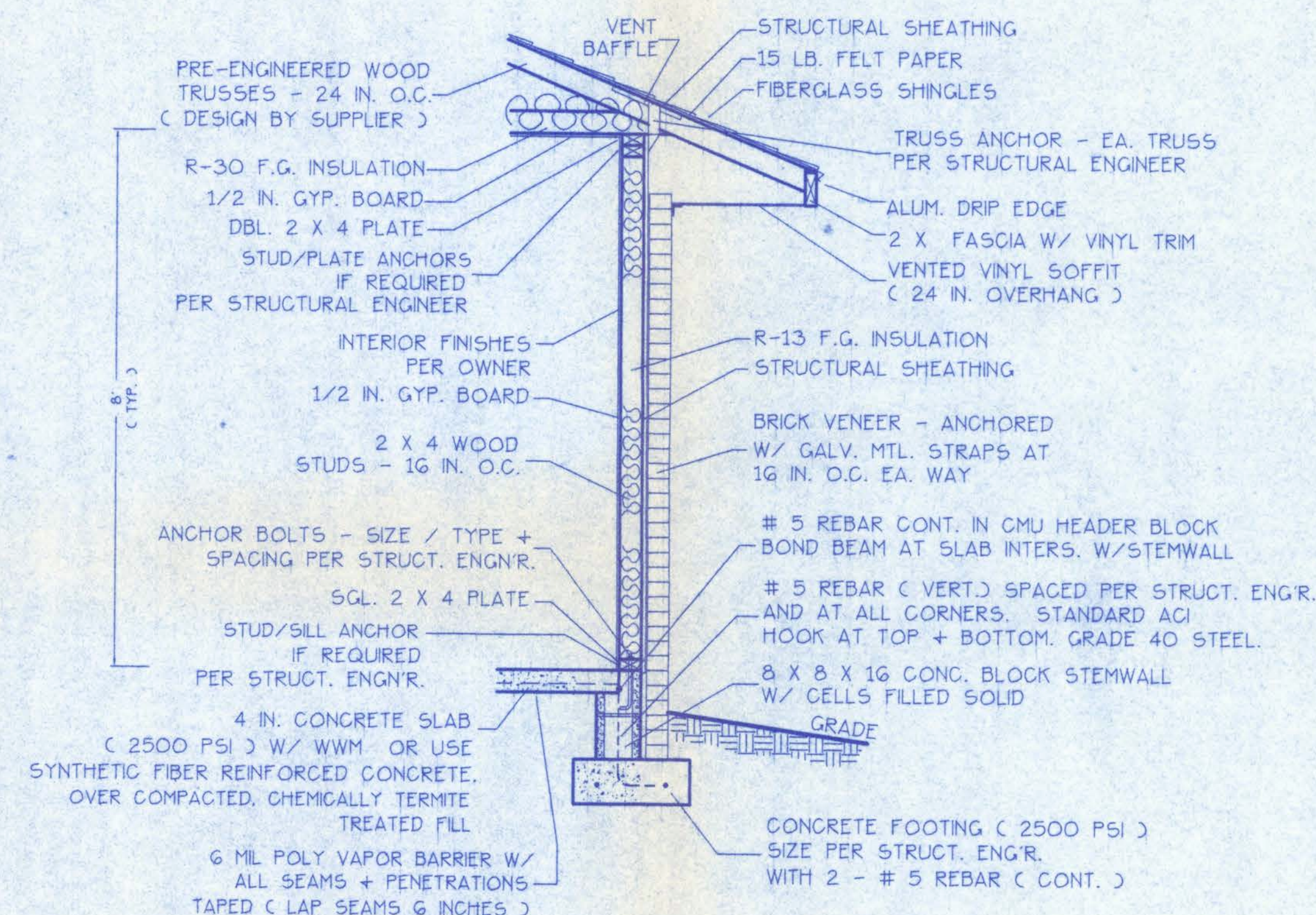
**FOUNDATION PLAN**  
SCALE: 1/4 IN. = 1 FT.

**FOUNDATION NOTES:**

- CONTRACTOR SHALL EXAMINE ROOF TRUSS PLAN (BY SUPPLIER) TO DETERMINE ANY ADDITIONAL BEARING REQUIREMENTS BEFORE FINALIZING THE FOUNDATION PLAN.
- ALL CONCRETE IS 2500 PSI STRENGTH (MIN.)
- VERIFY DIMENSIONS WITH FLOOR PLAN
- SITE ANALYSIS AND PREPARATION DATA IS NOT A PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE CONTRACTOR / OWNER.



**F4 - INTERIOR BEARING FOOTING**  
SCALE: 1" = 1'-0"



**WALL SECTION NOTES:**

- This Typical Wall Section is for Estimating purposes only.
- All data shown in this Wall Section shall be subject to review and final input by the Structural Engineer.

**DESIGN WALL SECTION**  
NON-STRUCTURAL DATA  
SCALE: 3/4 IN. = 1 FT.

**A-3**

WINDLOAD ENGINEER: Mark Disoway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

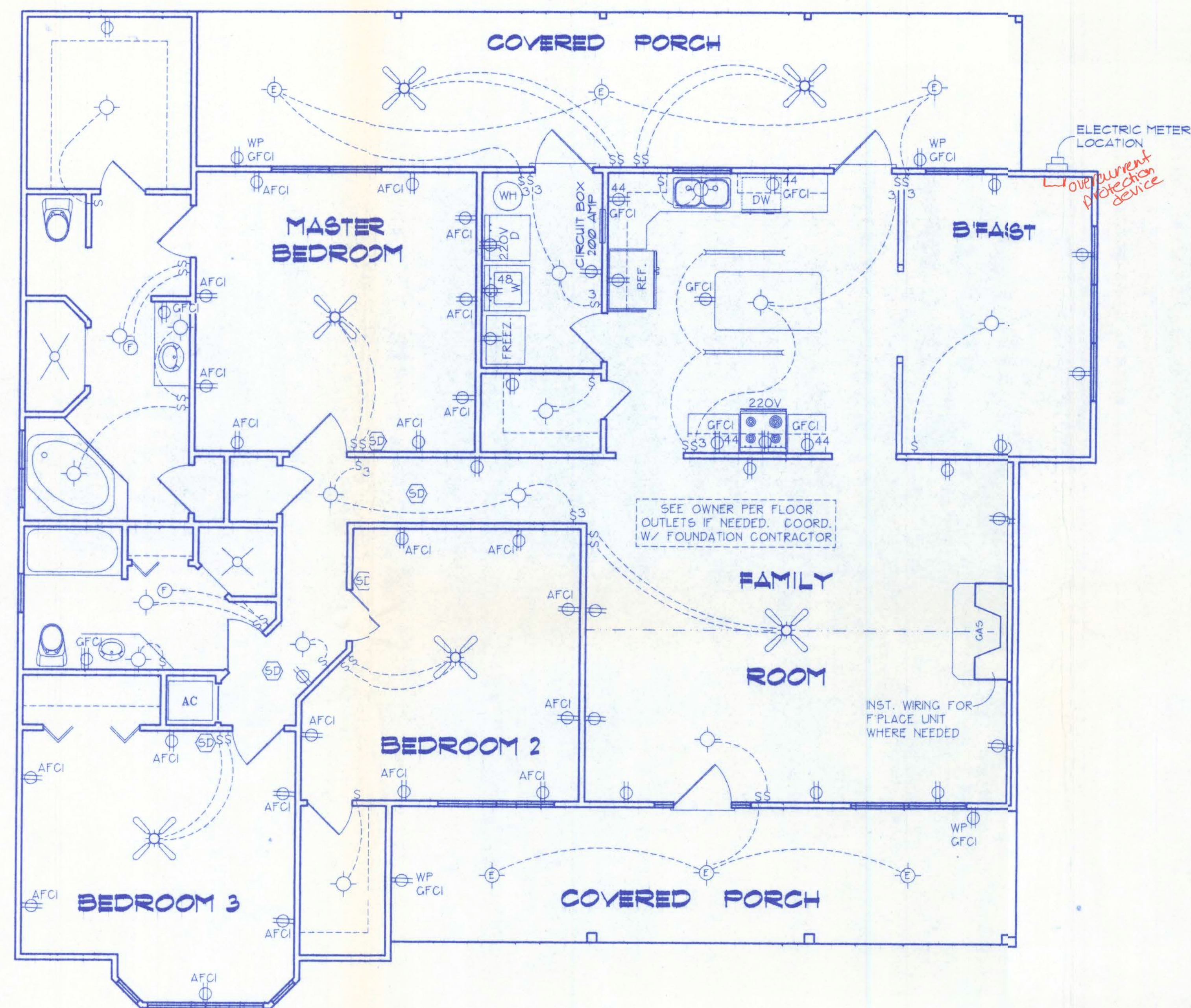
CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

Location: ~~LOT 5 BLK 4 PICCADILLY PARK SOUTH~~

Job No.: ~~1901006~~

|                 |                                                                                                                                         |                    |
|-----------------|-----------------------------------------------------------------------------------------------------------------------------------------|--------------------|
| FILE:<br>O5-O46 | <b>WRYE<br/>RESIDENCE</b>                                                                                                               | SHEET:<br>3 OF 4   |
| DATE:<br>2-4-06 |                                                                                                                                         | CAD FILE:<br>O5O46 |
| DRAWN:<br>T A D | PREPARED BY:<br><b>TIM DELBENE</b><br>Drafting + Technical Services<br>192 SW Sagewood Gln. Lake City, FL 32024<br>Phone (386) 755-5891 | REV:               |
| CHECK:<br>T A D |                                                                                                                                         | REV:               |



**ELECTRICAL PLAN**  
NOT TO SCALE

**ELECTRICAL SYMBOL LEGEND**

|  |                                     |
|--|-------------------------------------|
|  | FLUORESCENT LIGHTING FIXTURE.       |
|  | CEILING LIGHT FIXTURE               |
|  | EXTERIOR LIGHTING FIXTURE           |
|  | LIGHT SWITCH.                       |
|  | THREE-WAY SWITCH.                   |
|  | 110 V. DUPLEX OUTLET.               |
|  | SPECIAL HEIGHT 110 V. DUPLEX OUTLET |
|  | GROUND FAULT CIRC. OUTLET           |
|  | ARC FAULT CIRC. OUTLET              |
|  | 110 V. SINGLE RECEPTACLE OUTLET.    |
|  | 220 VOLT OUTLET ( 4 WIRE )          |
|  | FAN LOCATION ( CEILING )            |
|  | FAN LOCATION ( EXHAUST )            |
|  | SMOKE DETECTOR                      |

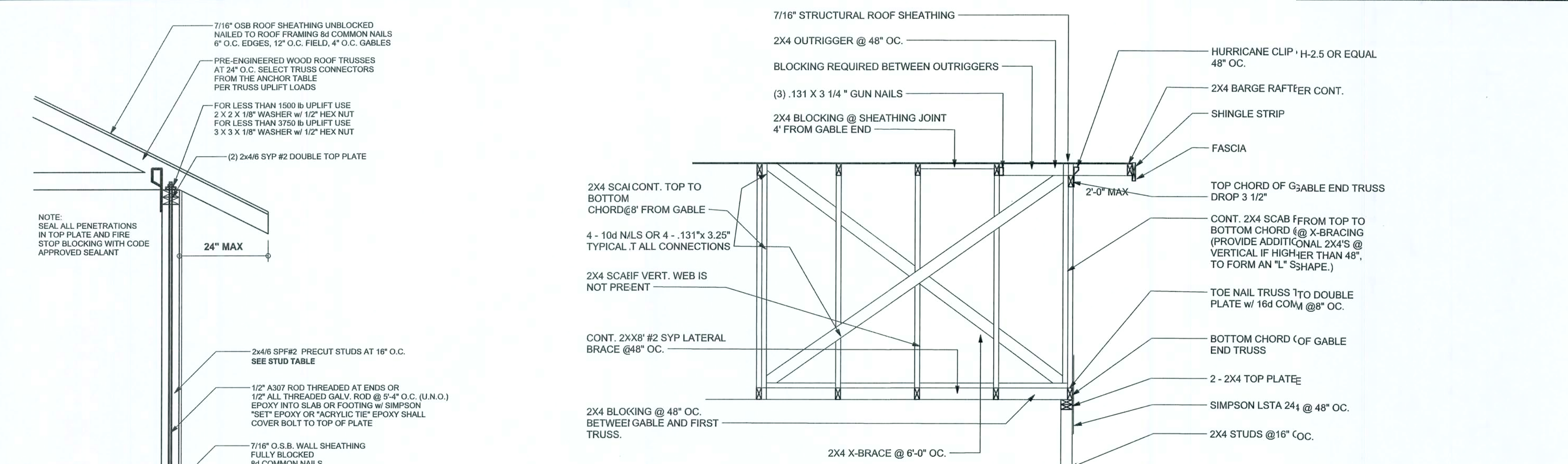
**ELECTRICAL PLAN NOTES**

- WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.
- CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.
- ALL INSTALLATIONS SHALL BE PER NAT'L. ELECTRIC CODE.
- ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.
- TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS, + IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.
- ELECTRICAL CONTR SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.
- ENTRY OF SERVICE ( UNDERGROUND OR OVERHEAD ) TO BE DETERMINED BY POWER COMPANY.

**A-4**

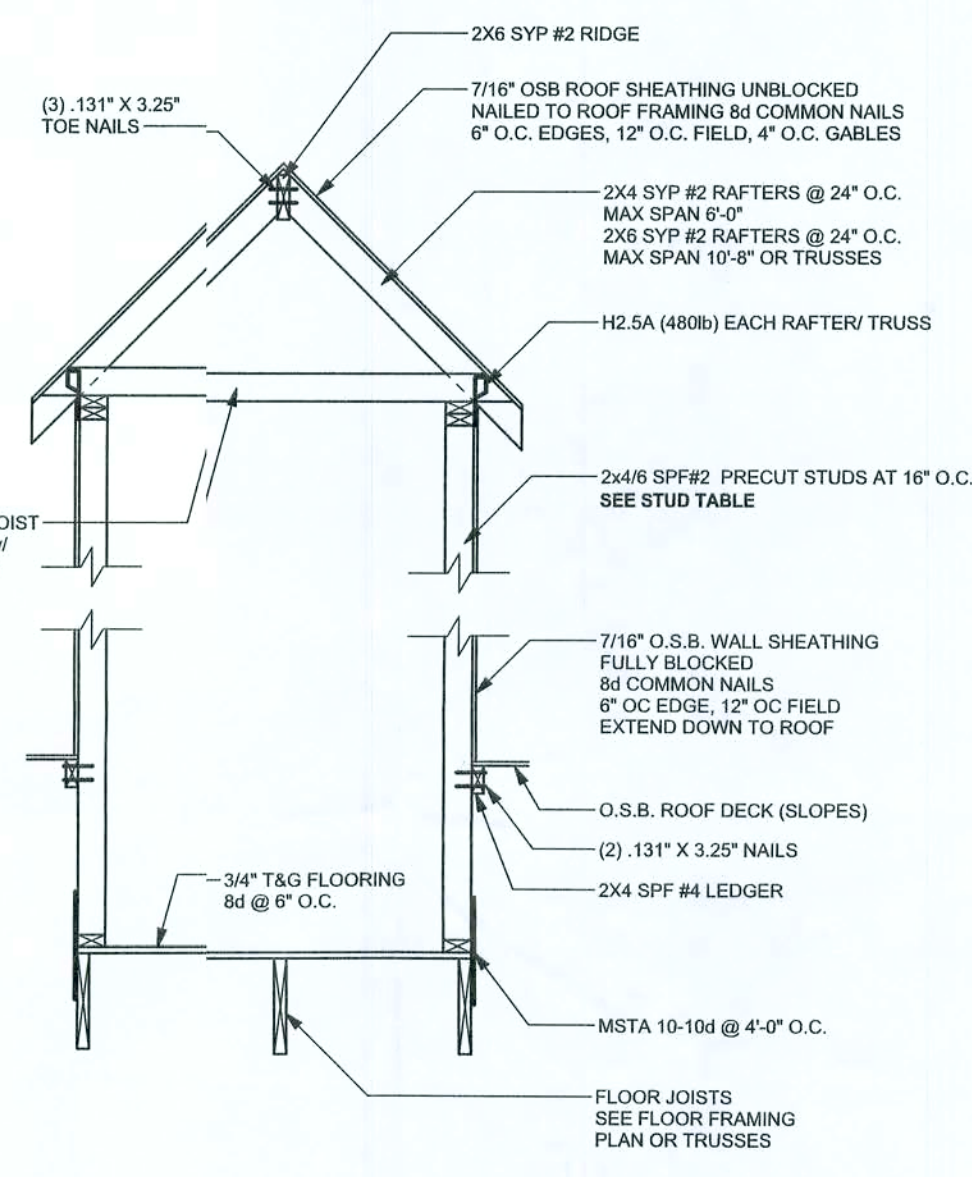
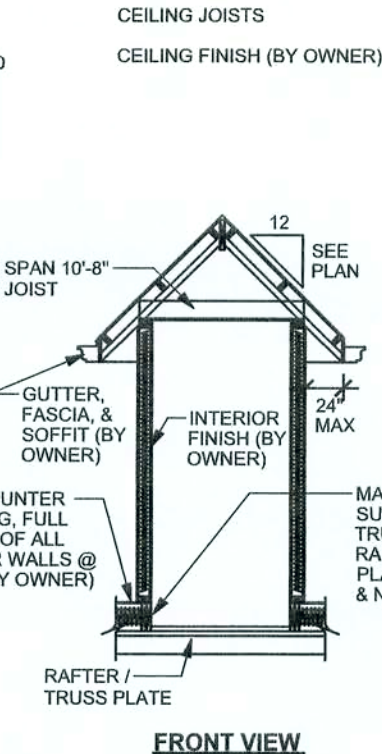
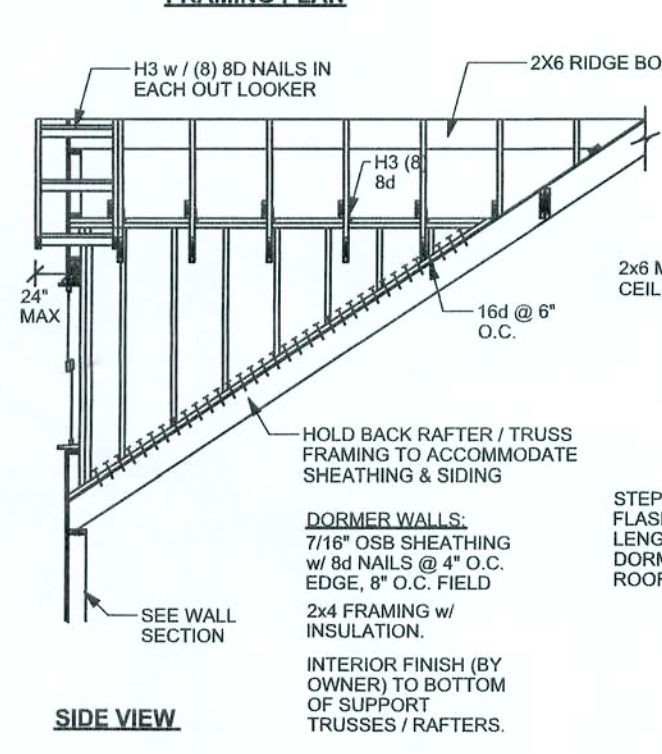
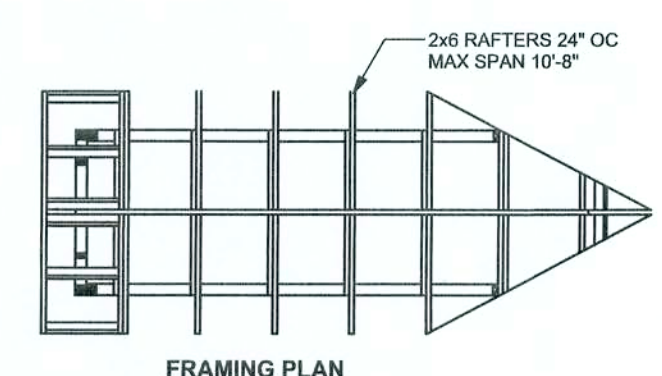
~~LOT 5, BLOCK A~~  
~~US 90 ROAD~~  
LOT 5, BLOCK A  
PICCADILLY PARK SOUTH

|                 |                                                                     |                    |
|-----------------|---------------------------------------------------------------------|--------------------|
| FILE:<br>05-046 | <b>WRYE<br/>RESIDENCE</b>                                           | SHEET:<br>4 OF 4   |
| DATE:<br>2-4-06 |                                                                     | CAD FILE:<br>05046 |
| DRAWN:<br>T A D | PREPARED BY:<br><b>TIM DELBENE</b><br>Drafting + Technical Services | REV:               |
| CHECK:<br>T A D | 192 SW Sagewood Cir., Lake City, FL 32024<br>Phone ( 386 ) 755-5891 | REV:               |



**TYPICAL GABLE END (X-BRACING)**

ALL MEMBERS SHALL BE SYP



**DORMER ANCHORING DETAIL (ON FLOOR)**  
SCALE: N.T.S.

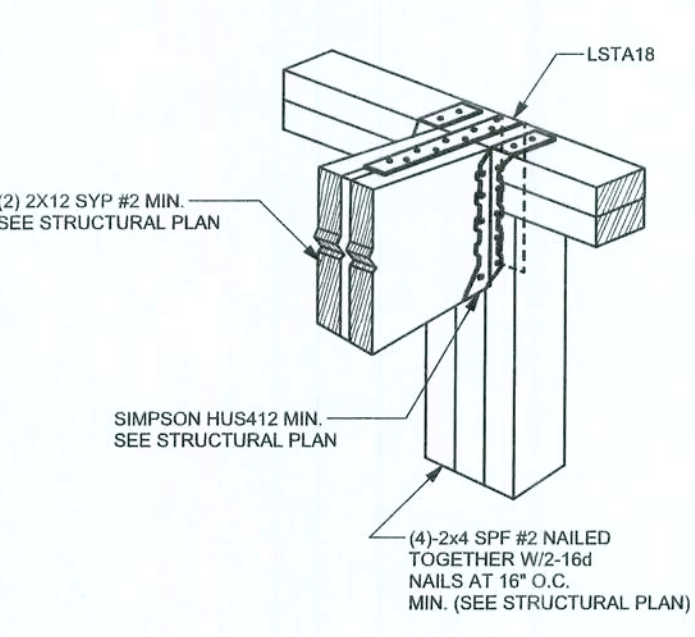
**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS**

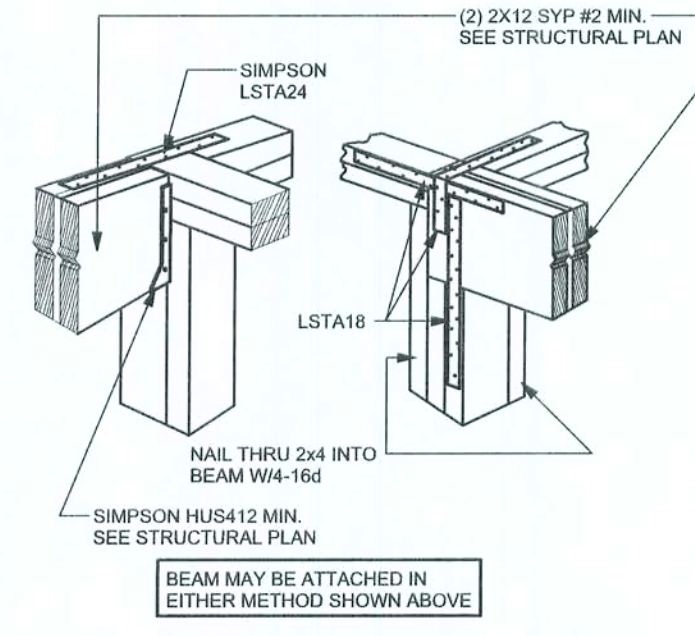
|                  |                       |
|------------------|-----------------------|
| (1) 2x4 @ 16" OC | TO 11'-9" STUD HEIGHT |
| (1) 2x4 @ 12" OC | TO 13'-0" STUD HEIGHT |
| (1) 2x6 @ 16" OC | TO 15'-0" STUD HEIGHT |
| (1) 2x6 @ 12" OC | TO 20'-0" STUD HEIGHT |

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WIND LOADS: 110 MPH EXPOSURE B, STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.

**CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL**  
SCALE: N.T.S.

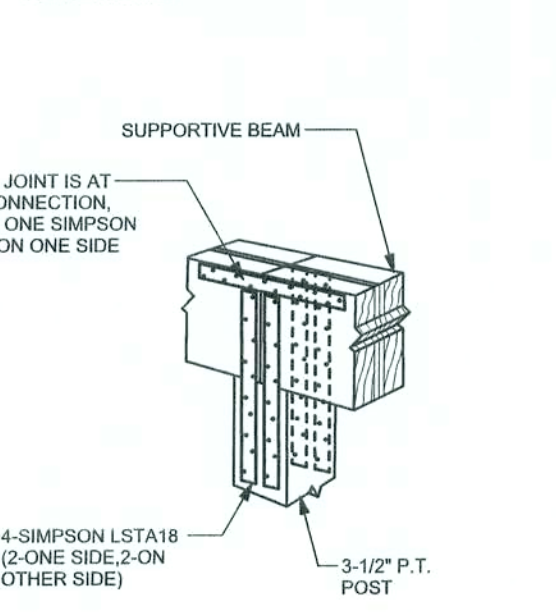


**BEAM MID-WALL CONNECTION DETAIL**  
SCALE: N.T.S.

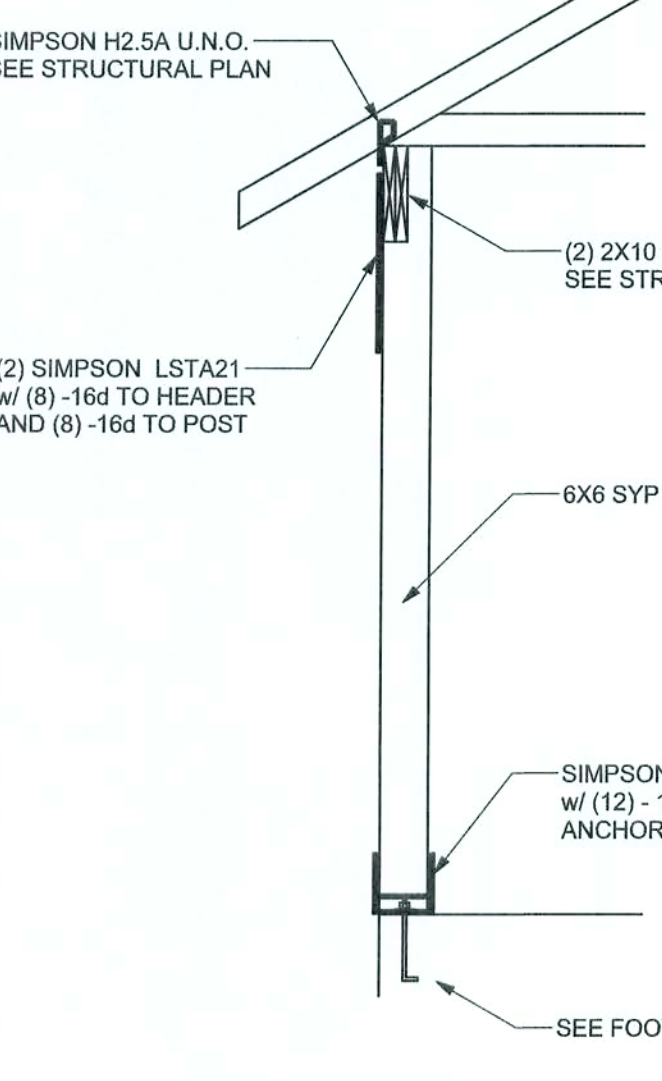


**BEAM CORNER CONNECTION DETAIL**  
SCALE: N.T.S.

**SUPPORTIVE POST TO BEAM DETAIL FOR SINGLE BEAM**  
SCALE: N.T.S.

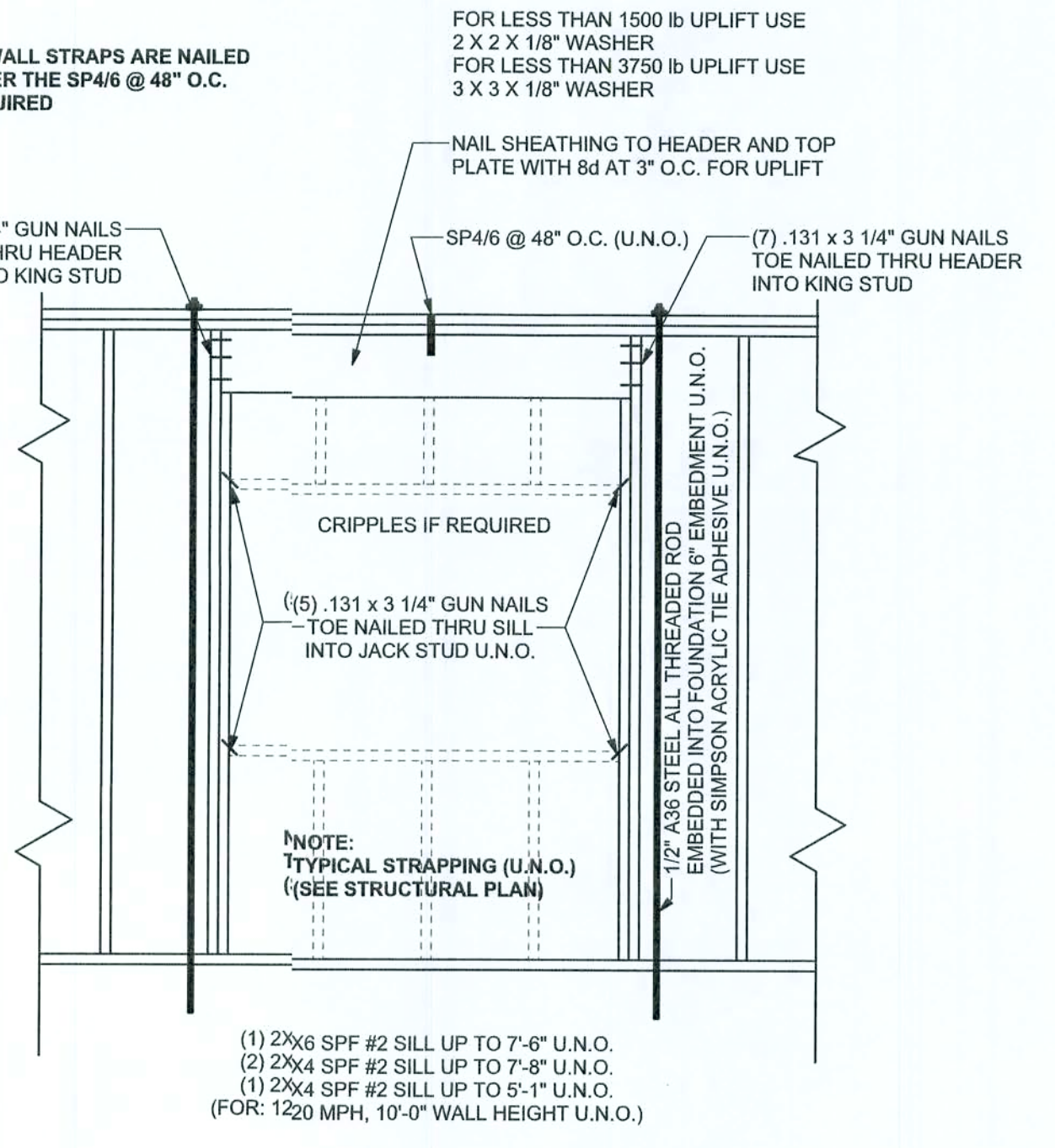


**SUPPORTIVE CENTER POST TO BEAM DETAIL**  
SCALE: N.T.S.



**TYPICAL PORCH POST DETAIL**  
SCALE: 1/2" = 1'-0"

NOTE: IF TRUSS TO WALL STRAPS ARE NAILED TO THE HEADER THE SP4/6 @ 48" O.C. ARE NOT REQUIRED



**TYPICAL 1 STORY HEADER STRAPPING DETAIL**  
SCALE: 1/2" = 1'-0"

**ANCHOR TABLE**

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS MANUFACTURER'S ENGINEERING

| UPLIFT LBS. SYP        | UPLIFT LBS. SPF | TRUSS CONNECTOR*      | TO PLATES      | TO RAFTER/TRUSS | TO STUDS                          |
|------------------------|-----------------|-----------------------|----------------|-----------------|-----------------------------------|
| < 420                  | < 245           | H5A                   | 3-8d           | 3-8d            |                                   |
| < 455                  | < 265           | H5                    | 4-8d           | 4-8d            |                                   |
| < 360                  | < 235           | H4                    | 4-8d           | 4-8d            |                                   |
| < 455                  | < 320           | H3                    | 4-8d           | 4-8d            |                                   |
| < 415                  | < 365           | H2.5                  | 5-8d           | 5-8d            |                                   |
| < 600                  | < 535           | H2.5A                 | 5-8d           | 5-8d            |                                   |
| < 950                  | < 820           | H8                    | 8-8d           | 8-8d            |                                   |
| < 745                  | < 565           | H8                    | 5-10d, 1 1/2"  | 5-10d, 1 1/2"   |                                   |
| < 1465                 | < 1050          | H14-1                 | 13-8d          | 12-8d, 1 1/2"   |                                   |
| < 1465                 | < 1050          | H14-2                 | 15-8d          | 12-8d, 1 1/2"   |                                   |
| < 990                  | < 850           | H10-1                 | 8-8d, 1 1/2"   | 8-8d, 1 1/2"    |                                   |
| < 760                  | < 655           | H10-2                 | 6-10d          | 6-10d           |                                   |
| < 1470                 | < 1265          | H16-1                 | 10-10d, 1 1/2" | 3-10d, 1 1/2"   |                                   |
| < 1470                 | < 1265          | H16-2                 | 10-10d, 1 1/2" | 2-10d, 1 1/2"   |                                   |
| < 1090                 | < 890           | MTS24C                | 7-10d 1 1/2"   | 7-10d 1 1/2"    |                                   |
| < 1450                 | < 1245          | MTS24                 | 12-10d 1 1/2"  | 12-10d 1 1/2"   |                                   |
| < 2900                 | < 2490          | 2 - HTS24             |                |                 |                                   |
| < 2050                 | < 1785          | LG72                  | 14 - 16d       | 14 - 16d        |                                   |
| HEAVY GIRDER TIEDOWNS* |                 |                       |                |                 | TO FOUNDATION                     |
| < 3965                 | < 3330          | MG7                   |                | 22 - 10d        | 1-5/8" THREADED ROD 12" EMBEDMENT |
| < 10980                | < 6485          | HGT-2                 |                | 16 - 10d        | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 10530                | < 9035          | HGT-3                 |                | 16 - 10d        | 2-5/8" THREADED ROD 12" EMBEDMENT |
| < 9250                 | < 9250          | HGT-4                 |                | 16 - 10d        | 2-5/8" THREADED ROD 12" EMBEDMENT |
| STUD STRAP CONNECTOR*  |                 |                       |                |                 | TO STUDS                          |
| < 435                  | < 435           | SSP DOUBLE TOP PLATE  | 3 - 10d        |                 | 4 - 10d                           |
| < 455                  | < 420           | SSP SINGLE SILL PLATE | 1 - 10d        |                 | 4 - 10d                           |
| < 825                  | < 825           | DSP DOUBLE TOP PLATE  | 6 - 10d        |                 | 8 - 10d                           |
| < 825                  | < 600           | DSP SINGLE SILL PLATE | 2 - 10d        |                 | 8 - 10d                           |
| < 885                  | < 760           | SP4                   |                |                 | 6-10d, 1 1/2"                     |
| < 1240                 | < 1065          | SPH4                  |                |                 | 10-10d, 1 1/2"                    |
| < 885                  | < 760           | SP6                   |                |                 | 6-10d, 1 1/2"                     |
| < 1240                 | < 1065          | SPH6                  |                |                 | 10-10d, 1 1/2"                    |
| < 1235                 | < 1165          | LSTA18                | 14-10d         |                 |                                   |
| < 1235                 | < 1235          | LSTA21                | 16-10d         |                 |                                   |
| < 1030                 | < 1030          | CS20                  | 18-8d          |                 |                                   |
| < 1705                 | < 1705          | CS16                  | 28-8d          |                 |                                   |
| STUD ANCHORS*          |                 |                       |                |                 | TO FOUNDATION                     |
| < 1350                 | < 1305          | LTT19                 | 8-16d          |                 | 1/2" AB                           |
| < 2310                 | < 2310          | LTT101                | 18-10d, 1 1/2" |                 | 1/2" AB                           |
| < 2775                 | < 2570          | HDS4                  | 2-5/8" BOLTS   |                 | 5/8" AB                           |
| < 4175                 | < 3695          | HTT16                 | 18 - 16d       |                 | 5/8" AB                           |
| < 1400                 | < 1400          | PAWH42                | 16-16d         |                 |                                   |
| < 3335                 | < 3335          | HPAH222               | 16-16d         |                 |                                   |
| < 2200                 | < 2200          | ABU44                 | 12-16d         |                 | 1/2" AB                           |
| < 2300                 | < 2300          | ABU66                 | 12-16d         |                 | 1/2" AB                           |
| < 2320                 | < 2320          | ABU88                 | 18 - 16d       |                 | 2-5/8" AB                         |

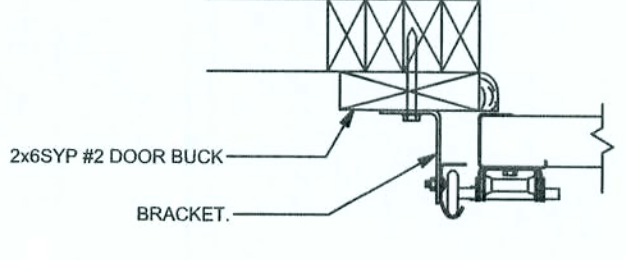
**GRADE & SPECIES TABLE**

|      |              | Fb (psi) | E (10 <sup>6</sup> psi) |
|------|--------------|----------|-------------------------|
| 2x8  | SYP #2       | 1200     | 1.6                     |
| 2x10 | SYP #2       | 1050     | 1.6                     |
| 2x12 | SYP #2       | 975      | 1.6                     |
| GLB  | 24F-V3 SP    | 2400     | 1.8                     |
| LSL  | TIMBERSTRAND | 1700     | 1.7                     |
| LVL  | MICROLAM     | 2900     | 2.0                     |
| PSL  | PARALAM      | 2900     | 2.0                     |

**2x6 SYP #2 GARAGE DOOR BUCK ATTACHMENT**

ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8" x 4" LAG SCREWS w/ 1" WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF .131 x 3 1/4" ON PER TABLE BELOW.

| DOOR WIDTH | 3/8" x 4" LAG | 16d STAGGER | (2) ROWS OF .131 x 3 1/4" ON |
|------------|---------------|-------------|------------------------------|
| 8' - 10'   | 24" O.C.      | 8" O.C.     | 5" O.C.                      |
| 11' - 15'  | 18" O.C.      | 4" O.C.     | 4" O.C.                      |
| 16' - 18'  | 16" O.C.      | 3" O.C.     | 3" O.C.                      |



**GARAGE DOOR BUCK INSTALLATION DETAIL**  
SCALE: N.T.S.

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING WALLS. TRUSS ENGINEERING SHALL BE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F<sub>c</sub> = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, FB = 85KSI, WELDED WIRE REINFORCEMENT FIBRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, F<sub>y</sub> = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILLED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, F<sub>b</sub> = 2,400, E = 1800ksi, UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALC. ALL ROOFS ARE HORIZONTAL DIAPHRAGMS. 7/16" O.S.B. SHEATHING, UNBLOCKED. APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED. FASTENED WITH 6d COMMON NAILS (131). 6" OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4" OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 12" IN GROTTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 3/16", WITH 5/8" BOLTS TO BE 3" x 3" x 3/16", WITH 3/4" BOLTS TO BE 3" x 3" x 3/16", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

**BUILDER'S RESPONSIBILITY**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION IF YOU BELIEVE THE PLAN COMB'S A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2001 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**DESIGN DATA**

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1

(ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS; MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE

BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

| Zone                                     | Effective Wind Area (ft <sup>2</sup> ) |             |
|------------------------------------------|----------------------------------------|-------------|
|                                          | 10                                     | 100         |
| 1                                        | 19.9 - 21.8                            | 18.1 - 18.1 |
| 2                                        | 19.9 - 25.5                            | 18.1 - 21.8 |
| 2 Othg                                   | 40.6                                   | -40.6       |
| 3                                        | 19.9 - 25.5                            | 18.1 - 21.8 |
| 3 Othg                                   | 68.3                                   | -42.4       |
| 4                                        | 21.8 - 23.6                            | 18.5 - 20.4 |
| 5                                        | 21.8 - 29.1                            | 18.5 - 22.6 |
| Doors & Windows                          | 21.8                                   | -29.1       |
| Worst Case (Zone 5, 10 ft <sup>2</sup> ) |                                        |             |
| 8x7 Garage Door                          | 19.5                                   | -22.9       |
| 16x7 Garage Door                         | 18.5                                   | -21.0       |

**DESIGN LOADS**

|                                       |                                        |
|---------------------------------------|----------------------------------------|
| FLOOR                                 | 40 PSF (ALL OTHER DWELLING ROOMS)      |
|                                       | 30 PSF (SLEEPING ROOMS)                |
|                                       | 30 PSF (ATTICS WITH STORAGE)           |
|                                       | 10 PSF (ATTICS WITHOUT STORAGE, <3:12) |
| ROOF                                  | 20 PSF (FLAT OR <4:12)                 |
|                                       | 16 PSF (4:12 TO <12:12)                |
|                                       | 12 PSF (12:12 AND GREATER)             |
| STAIRS                                | 40 PSF (ONE & TWO FAMILY DWELLINGS)    |
| SOIL BEARING CAPACITY                 | 1000PSF                                |
| NOT IN FLOOD ZONE (BUILDER TO VERIFY) |                                        |

| REVISIONS |  |
|-----------|--|
|           |  |
|           |  |
|           |  |



WINDLOAD ENGINEER: Mark Disoway, PE No. 53915, FCB 88, Lake City, FL 32056, 386-754-5411

DIMENSIONS: Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section R301.2.1, Florida building code residential 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARC DISOWAY  
P.L. 53915  
*Mark Disoway*  
CFE 2006  
IEAL

**Norton Home Improvements**

Wrye Residence

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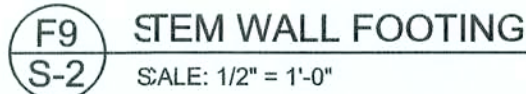
PRINTED DATE:  
April 03 2006  
DRAWN BY: STRUCTURAL BY:  
David Disoway

FINALS DATE:  
03 / Apr / 06

JOB NUMBER:  
603207  
DRAWING NUMBER

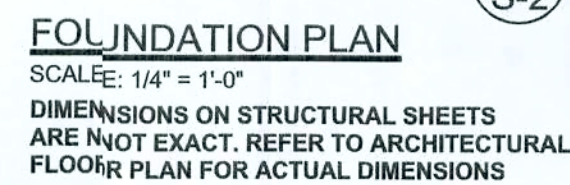
**S-1**  
OF 3 SHEETS

**SOFTPLAN**  
ARCHITECTURAL DESIGN SOFTWARE



The table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soffit/rein, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall dder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at 16" height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

| STEWALL<br>HEIGHT<br>(FEET) | UNBALANCE<br>BACKFILL<br>HEIGHT | VERTICAL REINFORCEMENT<br>FOR 8" CMU STEWWALL<br>(#5 O.C.) |    |    | VERTICAL REINFORCEMENT<br>FOR 12" CMU STEWWALL<br>(#7 O.C.) |    |    |
|-----------------------------|---------------------------------|------------------------------------------------------------|----|----|-------------------------------------------------------------|----|----|
|                             |                                 | #5                                                         | #7 | #8 | #5                                                          | #7 | #8 |
| 3.3                         | 3.0                             | 96                                                         | 96 | 96 | 96                                                          | 96 | 96 |
| 4.0                         | 3.7                             | 96                                                         | 96 | 96 | 96                                                          | 96 | 96 |
| 4.7                         | 4.3                             | 88                                                         | 96 | 96 | 96                                                          | 96 | 96 |
| 5.3                         | 5.0                             | 56                                                         | 96 | 96 | 96                                                          | 96 | 96 |
| 6.0                         | 5.7                             | 40                                                         | 80 | 96 | 80                                                          | 96 | 96 |
| 6.7                         | 6.3                             | 32                                                         | 56 | 80 | 56                                                          | 96 | 96 |
| 7.3                         | 7.0                             | 24                                                         | 40 | 56 | 40                                                          | 80 | 96 |
| 8.0                         | 7.7                             | 16                                                         | 32 | 48 | 32                                                          | 64 | 80 |
| 8.7                         | 8.3                             | 8                                                          | 24 | 32 | 24                                                          | 48 | 64 |
| 9.3                         | 9.0                             | 8                                                          | 16 | 24 | 16                                                          | 40 | 48 |



WINDLOAD ENGINEER: Mark Disosway,  
PE No.53915, POB368, Lake City, FL  
32056, 386-754-548

**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 3301.2.1, florida building code residential 2001, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specific location.

ARF DISOSWAY  
P.E. 53915

SEAL

## Norton Home Improvements

### Wrye Residence

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PRINTED DATE:  
May 01, 2006

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| DRAWN BY: | STRUCTURAL BY:<br>David Disosway |
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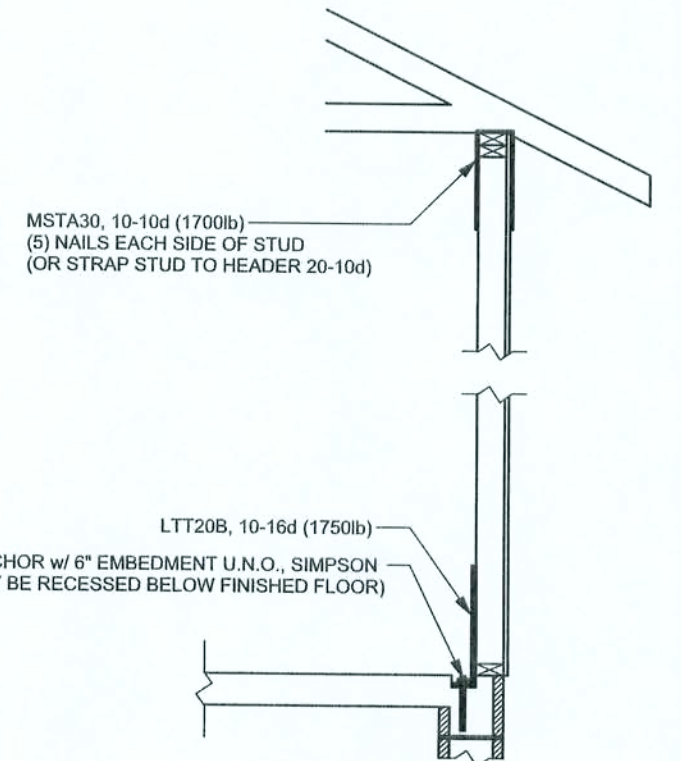
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| FINALS DATE: |  |
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JOB NUMBER:  
603207

DRAWING NUMBER

5-2

F 3SHEETS



STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP#2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-4 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS1-01, BCS1-02, & BCS1-03. BCS1-01, BCS1-02, & BCS1-03 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

ALTERNATE WALL TIE CONNECTION WHERE  
THREADED ROD CANNOT BE PLACED IN WALL.

SCALE: 1/2" = 1'-0"

### WALL LEGEND

|                                                                                                         |                                                              |
|---------------------------------------------------------------------------------------------------------|--------------------------------------------------------------|
| <p>SWS = 0.0'</p>  | 1ST FLOOR EXTERIOR WALL                                      |
| <p>SWS = 0.0'</p>  | 2ND FLOOR EXTERIOR                                           |
| <p>IBW</p>         | 1ST FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |
| <p>IBW</p>         | 2ND FLOOR INTERIOR BEARING WALLS<br>SEE DETAILS ON SHEET S-1 |

WINDLOAD ENGINEER: Mark Disosway,  
PE No.53915, POB368, Lake City, FL  
32056, 386-754-5493

**DIMENSIONS:**  
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permission and consent of Mark Disosway.

**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan relating to wind engineering comply with section R301.2.1, Florida building code residential 200, to the best of my knowledge.

**LIMITATION:** This design is valid for one building, at specific location.

HARRY DISOSWAY

PE. 53915



2/26/20

APR 2004

1997

AL

### THREADED ROD LEGEND

- INDICATES LOCATION OF:  
1ST FLOOR 1/2" A307 ALL THREADED ROD
- ⊗ INDICATES LOCATION OF:  
2ND FLOOR 1/2" A307 ALL THREADED ROD

### HEADER LEGEND

- 
- A diagram showing a rectangular box labeled (2) 2X12X0', 1J 1K. Six arrows point from text labels to the box:
  - HEADER/BEAM CALL-OUT (J.N.O.) points to the top of the box.
  - NUMBER OF KING STUDS (FULL LENGTH) points to the top edge of the box.
  - NUMBER OF JACK STUDS (UNDER HEADER) points to the top edge of the box.
  - SPAN OF HEADER points to the top edge of the box.
  - SIZE OF HEADER MATERIAL points to the top edge of the box.
  - NUMBER OF PLIES IN HEADER points to the top edge of the box.

TOTAL SHEAR WALL SEGMENTS

SWS = 0.0' INDICATES SHEAR WALL SEGMENTS

|              | REQUIRED | ACTUAL |
|--------------|----------|--------|
| TRANSVERSE   | 32.5'    | 78.0'  |
| LONGITUDINAL | 29.5'    | 58.5'  |

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #L151378

## Norton Home Improvements

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April 01, 2006

|              |                                  |
|--------------|----------------------------------|
| DESIGNED BY: | STRUCTURAL BY:<br>David Disosway |
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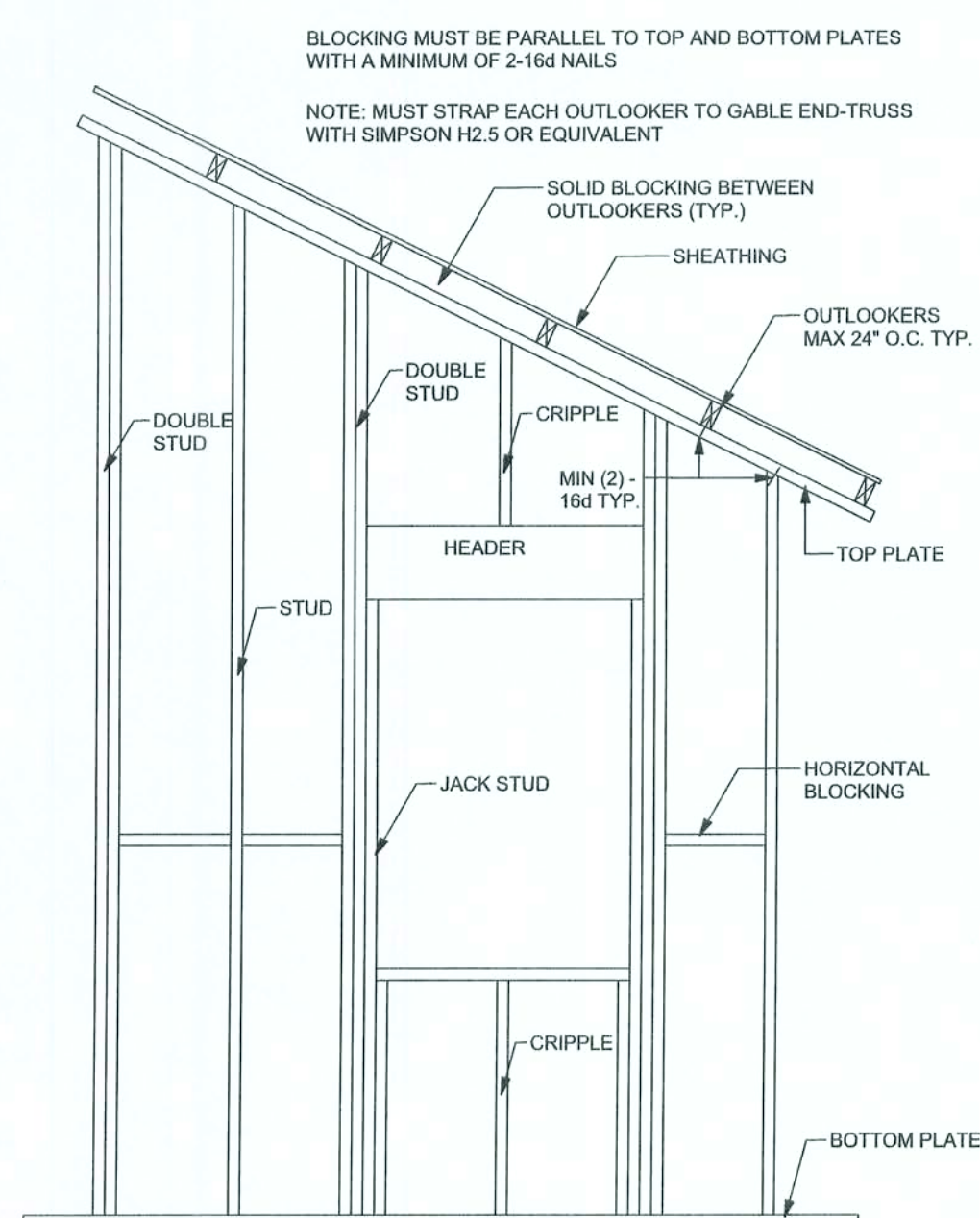
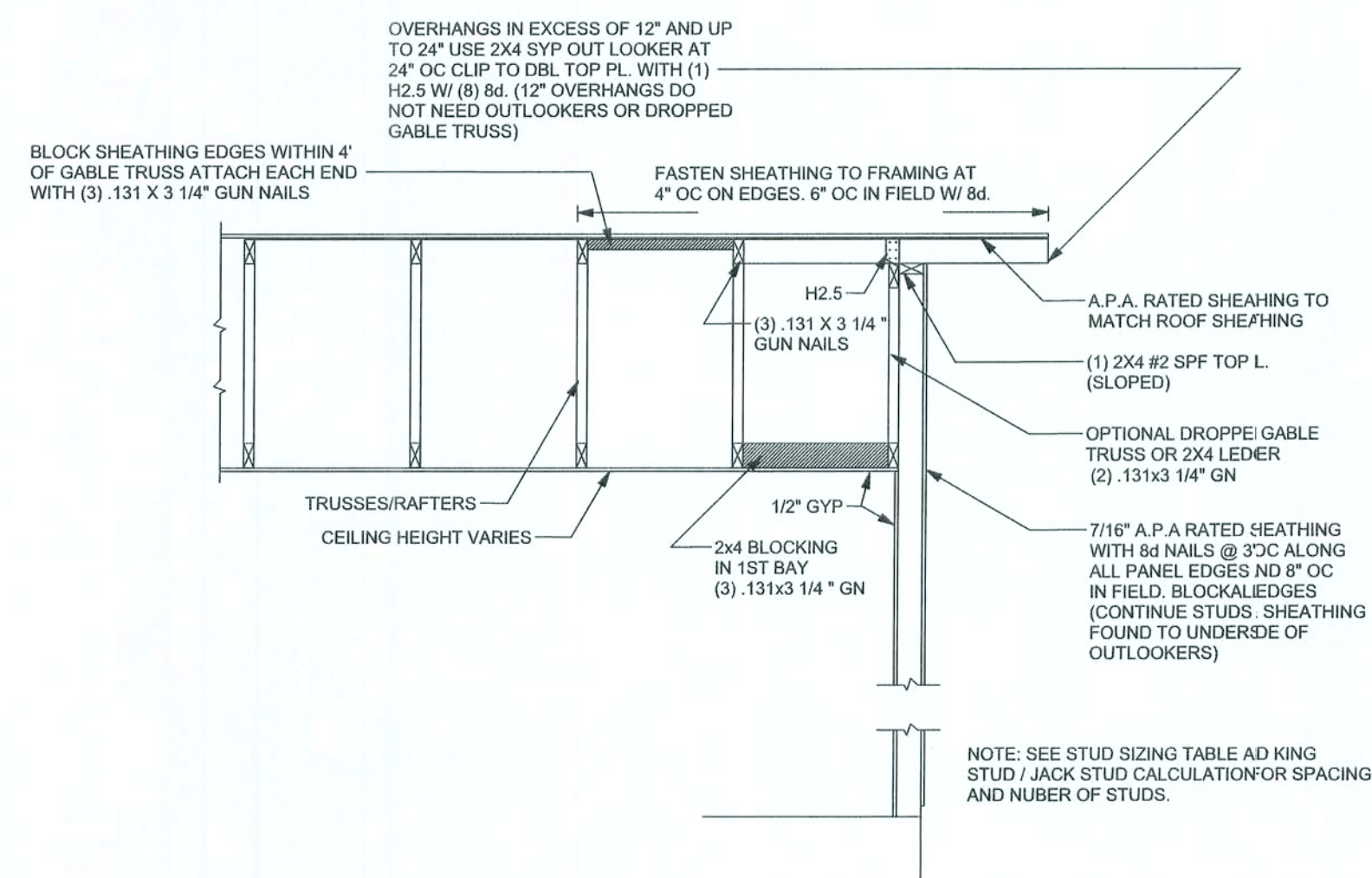
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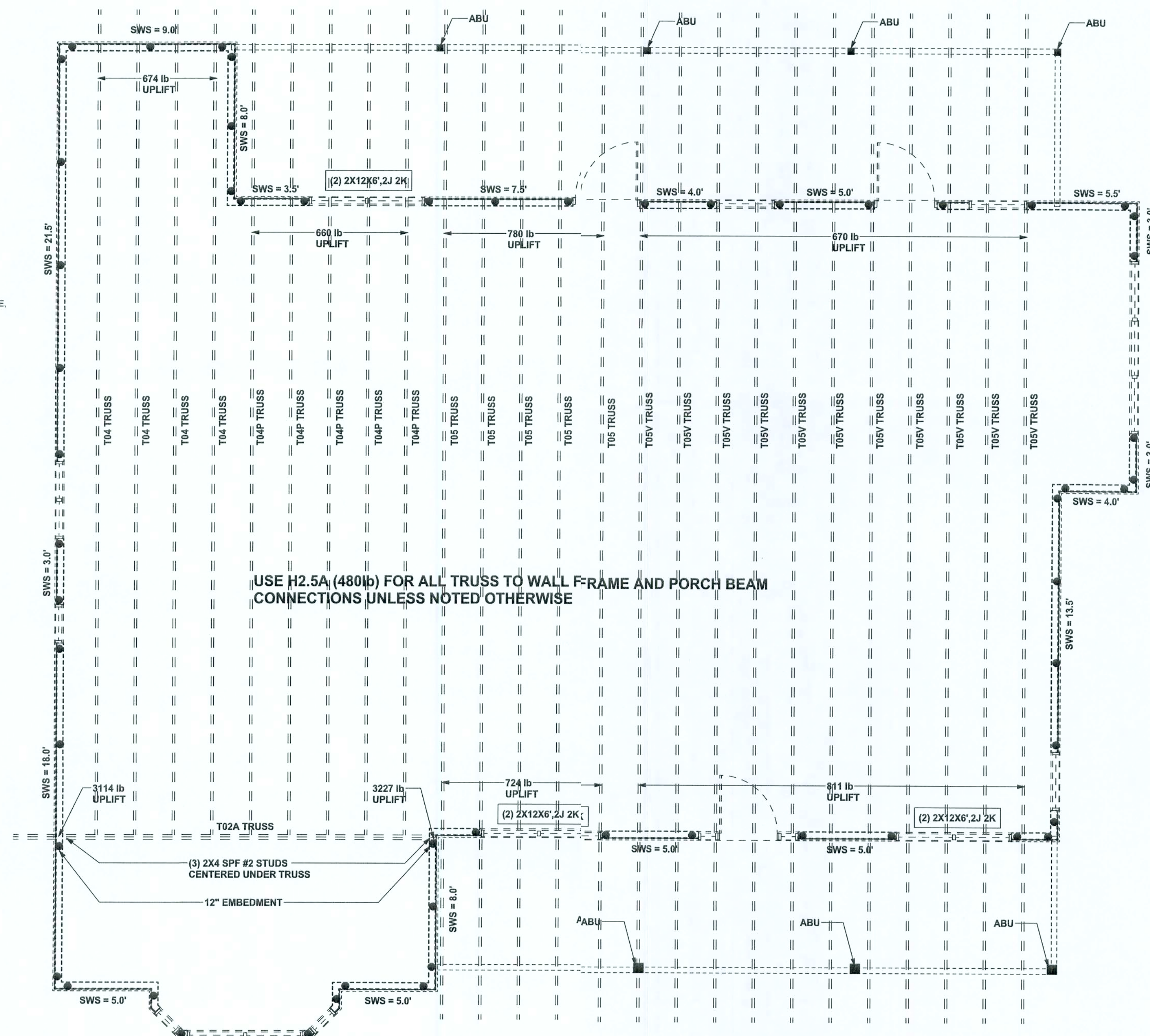
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S-3

OF 3 SHEETS



GABLE END WALL BALLOON FRAMING DETAIL  
SCALE: 1/2" = 1'-0"



**STRUCTURAL PLAN**  
SCALE: 1/4" = 1'-0"

USE H2.5A (480lb) FOR ALL TRUSS TO WALL F-RAME AND PORCH BEAM CONNECTIONS UNLESS NOTED OTHERWISE