

WIND ANALYSIS -- 120MPH Wind Velocity or as interpolated
2023 8th edition Florida Building Code
Calculations as per Section 1609ASCE 7-22

Prepared By
James Zaleski PE 51544

Prepared by (print legibly): James Zaleski
Design Professional FL Lic. #: 51544

Importance factor: 1.0 *Building Category:* ENCLOSED
Wind Exposure (s): B *Risk Category* II
Internal Pressure Coefficient +/- .18

Mean Roof Height 29.0 **End Zone Length** 8.0 feet

 MAX OVERHANG 1.5 FT MAX

MANUFACTURED TRUSSES TO BE USED

Roof Slope =-12/12

FOR: **REGINALD & WENDY**
DAVIS
242 SW GOLDBOLD AVE.
LAKE CITY, FL 32024



TRUSS SPAN/LOCATION HURRICANE CLIPS
HC MODEL-1 Simpson H-10A IN ALL AREAS 2 SDWC15600 SCREWS EQUAL 1-H10A

ROOF SHEATHING MATERIAL -7/16" OSB or PLYWOOD
NAILING - 8D RING SHANK

NAILING PATTERN
EDGES-
6" O.C **FIELD - 6" O.C**

4" O.C FIRST ROW AT ALL EAVES SEE ATTACHED

DESIGN CRITERIA:

2023 FLORIDA BUILDING CODE - BUILDING, 8TH EDITION

2018 WFCM (WOOD FRAME CONSTRUCTION MANUAL)

ASCE 7-22: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.

BUILDING OCCUPANCY CATEGORY= II

Plan May Be Mirrored at Contractors Option

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Wall Exterior Panel – Sheath with 5/8” OSB OR PLYWOOD

– 2 X 4 STUDS AT 16” O.C. UP TO 110 FEET

– 2 X 6 STUDS AT 16” O.C. UP TO 16 FEET

ALL WALLS OVER 10 FEET TO HAVE 2 ROWS OF BLOCKING

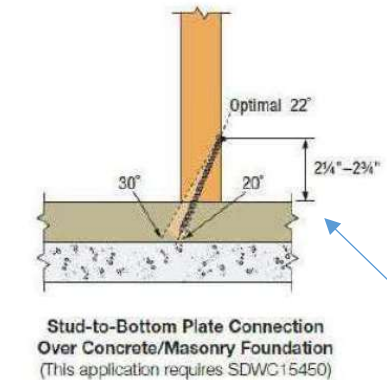
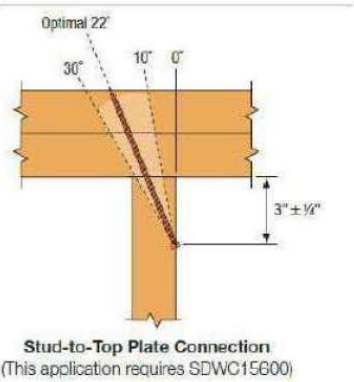
SIMPSON SP1 AT BASE OF STUDS AND SIMPSON SP2 AT TOP OF STUDS AT 32” O.C (SPH STRAPS MAY ALSO BE USED)

SEE ATTACHED FOR POST CONNECTIONS

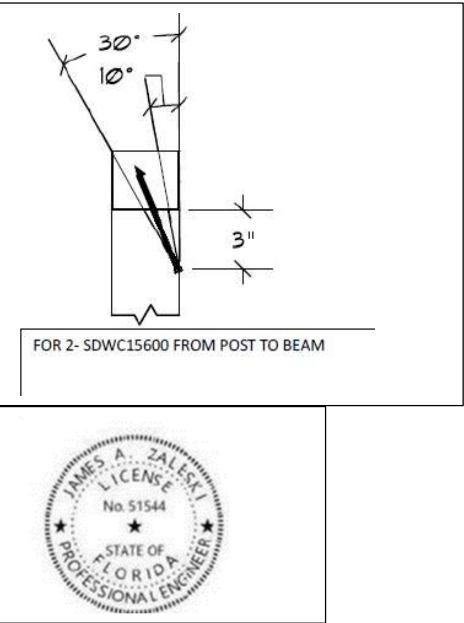
MIN NAIL PENETRATION – 1-1/2”
Nail Type 10D
Edge Nail Spacing 4” o.c
Intermediate Nail Spacing 6” o.c

BEAM TO WALL/CORNER CONNECTION – POCKET AND NAIL INTO WALL W/ (10) 16 PENNY NAILS, STRAP W/ SIMPSON H7Z.

10" 'ANCHOR' BOLT MINIMUM 32" O.C. w/
3 GA. x 3" x 3" BEARING PLATE
And 6" from each corner



IN LIEU OF STRAPPING STUDS SDWC SCREWS MAY BE USED



USE (3) 2x HEADERS WITH 2x6 WALLS AND (2) 2x HEADERS WITH 2x4 WALLS.
VERIFY HEADER SIZE W/ OPENING FRAMING & HEADER SCHEDULE ON THIS SHEET.
ASSEMBLE ALL HEADERS WITH 16D NAILS STAGGERED 6” O.C. W/ 2” EDGE DISTANCE
ALL HEADERS SHALL BE 2x12 U.O.N.
ATTACH ALL LVL BEAMS W/ SIMPSON SDWS 0.220 SCREWS (OR EQUAL) STAGGERED
W/ 1-1/2” MIN. EDGE DISTANCE.

- (2) ROWS @ 24” O.C. < 10” LVL
- (3) ROWS @ 24” O.C. > 10” LVL
- 2 & 3 PLY – 3-1/2” LONG STAGGER INSTALLED FROM BOTH OUTER PLIES
- 4 PLY – 6” LONG STAGGER INSTALLED FROM BOTH OUTER PLIES

FOR WINDOWS PLACED WITHIN 3’-0” OF EXTERIOR CORNERS, WALL STUDS SHALL BE SPACED @ 8” O.C.



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COMPONENTS AND CLADDING PRESSURES: (WORST CASE LOADS MAY BE USED)

COMPONENTS AND
CLADDING

ZONE per

SEE ATTACHED

MAIN WIND FORCE RESISTING SYSTEMS (MWFRS) (WORST CASE LOADS MAY BE USED)

SEE ATTACHED

All Load Bearing and Shear Walls To be Framed as per FBC
Alternative Hurricane Clips are acceptable as long as they meet the requirements shown

See Attached header schedule

PROVIDE GABLE END BRACING DETAIL, all vaulted or high ceilings shall be balloon framed to the ceiling diaphragm.

NOTES: PLEASE READ & complete all blanks!!!!

- 1. See floor plan for wall bracing locations or circle 100% if structural sheathing is required on all exterior walls, with the nailing pattern indicated above.
- 2. There are , there are not X interior shear walls, locate interior shear walls on plan.
- 3. Gable ends required to be sheathed with same material as shear wall? Yes or (No) (circle one)
- 4. Wall sheathing used in lieu of vertical straps: Nailing @ N/A o.c. along top & bottom plates
- 5. Provide detail for 2 story bldgs showing continuous load path between 2nd floor stud & 1st floor studs.
- 6. Provide additional information for column base & column/beam connection if required for porches.
- 7. Provide calculations or documentation to substantiate method used as an attachment to this form(SEE PLANS)

Instructions:

- 1. The form should be completed & signed, sealed & dated by a Fla. licensed engineer or architect.
- 2. Since more than one methodology for determination of wind forces is permitted under Section 1609ASCE7-22, to comply with State Building Codes a space has been provided to indicate method used.
- 3. Wind Analysis Forms submitted & permitted to be used as Master Plans will be for identical plans only, minor deviations such as door swings. Any deviation from the exterior form, opening sizes or locations will not be permitted unless noted by the design professional.

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MecaWind v2502

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Calculations Prepared by:

JAMES ZALESKI P.E. 51544
2305 HAVERHILL RD
TALLAHASSEE, FL, 32312
Date: Sep 16, 2025

File Location: Current Project Not Saved

General:

Reference Abbreviations: T: Table, F: Figure, E: Equation, S: Section

Wind Load Standard	= ASCE 7-22	Basic Wind Speed	= 120.0 mph
Exposure Classification	= B	Risk Category	= II
Structure Type	= Building	Design Basis for Wind Pressures	= ASD
MWFRS Analysis Method	= Ch 27	C&C Analysis Method	= Ch 30 Pt 1
Dynamic Type of Structure	= Rigid	Show Advanced Options	= False

Building:

Roof	= Roof Type	= Hipped	Encl	= Enclosure Classification	= Enclosed
Help	= Help on Building Roof Type	= Help	Pitch	= Pitch of Roof	= 11.93 :12
θ	= Slope of Roof	= 44.837 °	R_{ht}	= Ridge Height	= 45.000 ft
E_{ht}	= Eave Height	= 16.000 ft	W	= Building Width	= 55.330 ft
L	= Building Length	= 74.000 ft	L_{hip}	= Ridge Hipped Length	= 20.000 ft
θ_{hip}	= Hipped End Slope of Roof	= 45.53 °	Pitch	= Hipped End Slope of Roof	= 12.226 :12
OH	= Overhang Configuration	= All Soffit	hip Par	= Parapet	= None
z_i	= Highest Opening Elevation	= 12.0000 ft	HT_{over}	= Override Mean Roof Height	= True
H_{tman}	= Mean Roof Height	= 29.000 ft	RA_{over}	= Override Roof Area	= False
GC_{pi_o}	= Override GC_{pi} value	= False	IsElev	= Building is Elevated	= False

Exposure Constants [T:26.11-1]:

α = 3-s Gust-speed exponent	= 7.500	Z_g = Nominal Ht of Boundary Layer	= 3280.000 ft
$\hat{\alpha}$ = Reciprocal of α	= 0.133	b = 3 sec gust speed factor	= 0.840
α_m = Mean hourly Wind-Speed Exponent	= 0.222	b_m = Mean hourly Windspeed Exponent	= 0.470
c = Turbulence Intensity Factor	= 0.300	ϵ = Integral Length Scale Exponent	= 0.3333

Overhang Inputs:

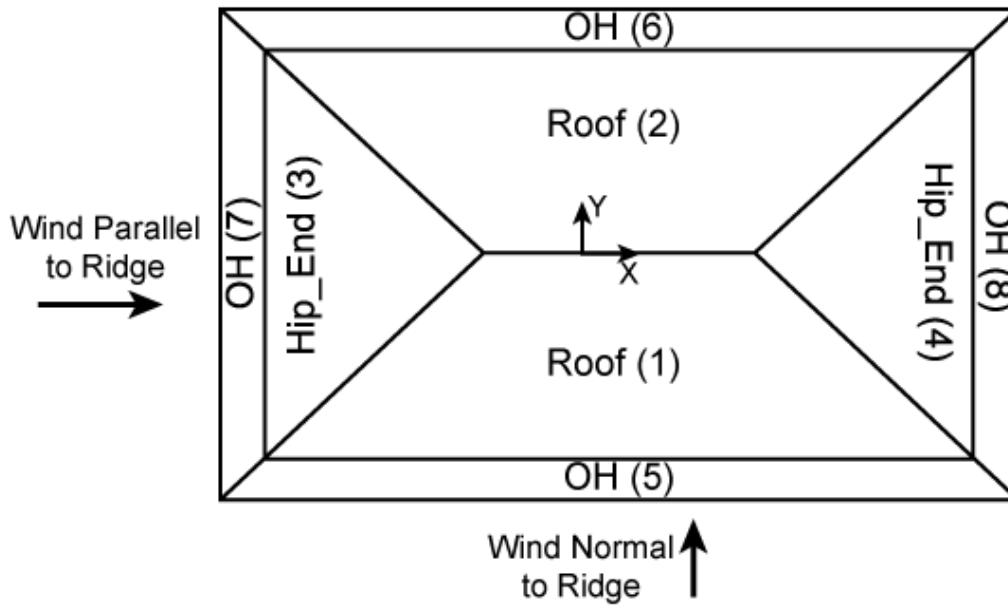
Std	= Overhangs on all sides are the same	= True
OHType	= Type of Roof Wall Intersections	= Soffit
OH	= Overhang of Roof Beyond Wall	= 1.500 ft

Main Wind Force Resisting System (MWFRS) Wind Calculations per Ch 27



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h = Mean structure height = 29.000 ft
K_{zt} = No Topographic Feature = 1.000
GC_{pi} = ± Internal Press Coef T:26.13-1 = ±0.18
K_e = Ground Elev Factor T:26.10-1 = 1.000
q_{in} = Negative Internal Pressure: q_h = 15.11 psf
A_{roof} = Roof Area = 6,359.83 ft²

K_h = 2.41 • (Z/Z_g)^{2/α} T:26.10-1 = 0.683
K_d = Directionality Factor T:26.6-1 = 0.85
LF = ASD Load Factor = 0.60
q_h = .00256 • K_h • K_{zt} • K_e • V² • LF E:26.10-1 = 15.11 psf
q_{ip} = For +GC_{pi} use q_h = 15.11 psf

MWFRS Wind Loads [Normal to Ridge]

h = Mean Roof Height of Building = 29.0000 ft
B = Building Width Normal To Wind = 74.0000 ft
L/B = Ratio: L/B = 0.748
θ = Slope of Main Roof = 44.84 °
G = Gust Factor: Min(G₁, G₂) = 0.850
C_{pLW} = Leeward Wall Coefficient = -0.500

R_{ht} = Ridge Height Of Roof = 45.0000 ft
L = Building Width Parallel To Wind = 55.3300 ft
h/L = Ratio: h/L = 0.524
θ_{hip} = Slope of Hipped Roof Ends = 45.53 °
C_{pWW} = Windward Wall Coefficient = 0.800
C_{pSW} = Side Wall Coefficient = -0.700

Wall Wind Pressures [Normal to Ridge] All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC _{pi}	q _i psf	K _z	K _{zt}	q _z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
16.000	+0.18	15.11	0.583	1.000	12.89	5.14	-7.77	-9.95	12.90	9.60
16.000	-0.18	15.11	0.583	1.000	12.89	9.76	-3.14	-5.33	12.90	9.60

K_z = 2.41 • (Z/Z_g)^{2/α}
GC_{pi} = +Internal Coef T:26.13-1
q_{ip} = For +GC_{pi} use q_h
Side = q_h • K_d • G • C_{pSW} - q_{ip} • K_d • (GC_{pi+}) E:27.3-1
Windward = q_z • K_d • G • C_{pWW} - q_{ip} • K_d • (GC_{pi+}) E:27.3-1
+Press = Pressure Acting Toward Surface
\$27.1.5 = MWFRS Min Wall Pressure = 9.60 psf

K_{zt} = No Topographic Feature
q_z = .00256 • K_z • K_{zt} • K_e • V² • LF E:26.10-1
q_{in} = Negative Internal Pressure: q_h
Leeward = q_h • K_d • G • C_{pLW} - q_{ip} • K_d • (GC_{pi+}) E:27.3-1
Total = Windward - Leeward
-Press = Pressure Acting Away from Surface

Roof Wind Pressures [Normal to Ridge] All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC _{pi}	C _{pMin}	C _{pMax}	P _{min} psf	P _{max} psf	P _{min} psf
Hip_End	Hipped End 0 to h/2	3,4	1.500	14.500	0.0	P	+0.18	-0.907	-0.18	-12.20	-4.28	4.80
Hip_End	Hipped End h/2 to h	3,4	14.500	29.000	0.0	P	+0.18	-0.89	-0.18	-12.02	-4.28	4.80
Hip_End	Hipped End h to 2•h	3,4	29.000	56.830	0.0	P	+0.18	-0.51	-0.18	-7.87	-4.28	4.80
OH_Top	Overhang Top	5	All	All	44.84	N	+0.18	0.394	-0.003	1.98	-2.35	4.80
OH_Top	Overhang Top 0 to h/2	7,8	0.000	14.500	0.0	P	+0.18	-0.907	-0.18	-12.20	-4.28	4.80
OH_Top	Overhang Top h/2 to h	7,8	14.500	29.000	0.0	P	+0.18	-0.89	-0.18	-12.02	-4.28	4.80
OH_Top	Overhang Top h to 2•h	7,8	29.000	58.000	0.0	P	+0.18	-0.51	-0.18	-7.87	-4.28	4.80
OH_Top	Overhang Top ≥ 2•h	7,8	58.000	58.330	0.0	P	+0.18	-0.319	-0.18	-5.79	-4.28	4.80



OH_Top	Overhang Leeward	7,8	All	All	44.84	N	+0.18	-0.6	-0.6	-8.86	-8.86	4.80
Roof_LW	Roof Leeward	2	All	All	44.84	N	+0.18	-0.6	-0.6	-8.86	-8.86	4.80
Roof_WW	Roof Windward	1	All	All	44.84	N	+0.18	0.394	-0.003	1.98	-2.35	4.80
Soffit	Soffit Bottom	5	All	All	0.0	N	+0.18	0.8	0.8	6.42	6.42	4.80
Hip_End	Hipped End 0 to h/2	3,4	1.500	14.500	0.0	P	-0.18	-0.907	-0.18	-7.58	0.35	4.80
Hip_End	Hipped End h/2 to h	3,4	14.500	29.000	0.0	P	-0.18	-0.89	-0.18	-7.40	0.35	4.80
Hip_End	Hipped End h to 2•h	3,4	29.000	56.830	0.0	P	-0.18	-0.51	-0.18	-3.25	0.35	4.80
OH_Top	Overhang Top	5	All	All	44.84	N	-0.18	0.394	-0.003	6.60	2.28	4.80
OH_Top	Overhang Top 0 to h/2	7,8	0.000	14.500	0.0	P	-0.18	-0.907	-0.18	-7.58	0.35	4.80
OH_Top	Overhang Top h/2 to h	7,8	14.500	29.000	0.0	P	-0.18	-0.89	-0.18	-7.40	0.35	4.80
OH_Top	Overhang Top h to 2•h	7,8	29.000	58.000	0.0	P	-0.18	-0.51	-0.18	-3.25	0.35	4.80
OH_Top	Overhang Top ≥ 2•h	7,8	58.000	58.330	0.0	P	-0.18	-0.319	-0.18	-1.17	0.35	4.80
OH_Top	Overhang Leeward	7,8	All	All	44.84	N	-0.18	-0.6	-0.6	-4.23	-4.23	4.80
Roof_LW	Roof Leeward	2	All	All	44.84	N	-0.18	-0.6	-0.6	-4.23	-4.23	4.80
Roof_WW	Roof Windward	1	All	All	44.84	N	-0.18	0.394	-0.003	6.60	2.28	4.80
Soffit	Soffit Bottom	5	All	All	0.0	N	-0.18	0.8	0.8	11.04	11.04	4.80

Roof Pressures based upon Ch 27:

Component = The building component for pressures

Start = Start Dist from Windward Edge

C_{pMin} = Smallest Coefficient Magnitude

P_{min} = q_h•K_d•G•C_{pMin}-q_{ip}•K_d•GC_{piE}; 27.3-1

GC_{pi} = +Internal Coef T_{1;26.13-1}

P_{min} = Min Press projected on vertical plane §27.1.5

§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf

+Press = Pressure Acting Toward Surface

• The smaller uplift pressures due to C_{pMin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

Location = Reference Graphic in Output for Values

End = End Dist from Windward Edge

C_{pMax} = Largest Coefficient Magnitude

P_{max} = q_h•K_d•G•C_{pMax}-q_{in}•K_d•GC_{piE}; 27.3-1

Basis = P=Parallel to Ridge: N=Normal to Ridge

θ = Roof Slope Relative to Wind

Reduction = Reduction Factor (.8) for h/L≥1 & 10≤θ≤15°

-Press = Pressure Acting Away from Surface

MWFRS Wind Loads [Parallel to Ridge]

h = Mean Roof Height of Building = 29.0000 ft

B = Building Width Normal To Wind = 55.3300 ft

L/B = Ratio: L/B = 1.337

θ = Slope of Main Roof = 44.84 °

G = Gust Factor: Min(G₁, G₂) = 0.850

C_{pLW} = Leeward Wall Coefficient = -0.433

R_{Ht} = Ridge Height Of Roof = 45.0000 ft

L = Building Width Parallel To Wind = 74.0000 ft

h/L = Ratio: h/L = 0.392

θ_{Hip} = Slope of Hipped Roof Ends = 45.53 °

C_{pWW} = Windward Wall Coefficient = 0.800

C_{pSW} = Side Wall Coefficient = -0.700

Wall Wind Pressures [Parallel to Ridge]

All wind pressures include a Load Factor (LF) of 0.6

Elev ft	GC _{pi}	q _i psf	K _z	K _{zt}	q _z psf	Windward Press psf	Leeward Press psf	Side Press psf	Total Press psf	Minimum Pressure* psf
45.000	+0.18	15.11	0.768	1.000	16.98	7.51	-7.03	-9.95	14.54	9.60
29.000	+0.18	15.11	0.683	1.000	15.11	6.42	-7.03	-9.95	13.45	9.60
16.000	+0.18	15.11	0.583	1.000	12.89	5.14	-7.03	-9.95	12.17	9.60
45.000	-0.18	15.11	0.768	1.000	16.98	12.13	-2.41	-5.33	14.54	9.60
29.000	-0.18	15.11	0.683	1.000	15.11	11.04	-2.41	-5.33	13.45	9.60
16.000	-0.18	15.11	0.583	1.000	12.89	9.76	-2.41	-5.33	12.17	9.60

K_z = 2.41•(Z/Z_g)^{2/a}

GC_{pi} = +Internal Coef T_{1;26.13-1}

q_{ip} = For +GC_{pi} use q_h

Side = q_h•K_d•G•C_{pSW}-q_{ip}•K_d•(GC_{pi+}) E:27.3-1

Windward = q_z•K_d•G•C_{pWW}-q_{ip}•K_d•(GC_{pi+}) E:27.3-1

+Press = Pressure Acting Toward Surface

§27.1.5 = MWFRS Min Wall Pressure = 9.60 psf

K_{zt} = No Topographic Feature

q_z = .00256•K_z•K_{zt}•K_d•V²•LF_E; 26.10-1

q_{in} = Negative Internal Pressure: q_h

Leeward = q_h•K_d•G•C_{pLW}-q_{ip}•K_d•(GC_{pi+}) E:27.3-1

Total = Windward - Leeward

-Press = Pressure Acting Away from Surface

Roof Wind Pressures [Parallel to Ridge]

All wind pressures include a Load Factor (LF) of 0.6

Component	Description	Location	Start ft	End ft	θ °	Basis	GC _{pi}	C _{pMin}	C _{pMax}	P _{min} psf	P _{max} psf	P _{min} psf
Hip_End	Hipped End 0 to h	3	1.500	28.500	45.53	P	+0.18	-0.9	-0.18	-12.13	-4.28	4.80
Hip_End	Hipped End h to 2•h	4	48.500	58.000	45.53	P	+0.18	-0.5	-0.18	-7.77	-4.28	4.80
Hip_End	Hipped End ≥ 2•h	4	58.000	75.500	45.53	P	+0.18	-0.3	-0.18	-5.59	-4.28	4.80



OH_Top	Overhang Top 0 to h	5,6,7	0.000	29.000	44.84	P	+0.18	-0.9	-0.18	-12.13	-4.28	4.80
OH_Top	Overhang Top h to 2•h	5,6	29.000	58.000	0.0	P	+0.18	-0.5	-0.18	-7.77	-4.28	4.80
OH_Top	Overhang Top ≥ 2•h	5,6,8	58.000	77.000	0.0	P	+0.18	-0.3	-0.18	-5.59	-4.28	4.80
Roof	Roof 0 to h	1,2	1.500	29.000	0.0	P	+0.18	-0.9	-0.18	-12.13	-4.28	4.80
Roof	Roof h to 2•h	1,2	29.000	58.000	0.0	P	+0.18	-0.5	-0.18	-7.77	-4.28	4.80
Roof	Roof ≥ 2•h	1,2	58.000	75.500	0.0	P	+0.18	-0.3	-0.18	-5.59	-4.28	4.80
Soffit	Soffit Bottom	7	All	All	0.0	N	+0.18	0.8	0.8	6.42	6.42	4.80
Hip_End	Hipped End 0 to h	3	1.500	28.500	45.53	P	-0.18	-0.9	-0.18	-7.51	0.35	4.80
Hip_End	Hipped End h to 2•h	4	48.500	58.000	45.53	P	-0.18	-0.5	-0.18	-3.15	0.35	4.80
Hip_End	Hipped End ≥ 2•h	4	58.000	75.500	45.53	P	-0.18	-0.3	-0.18	-0.96	0.35	4.80
OH_Top	Overhang Top 0 to h	5,6,7	0.000	29.000	44.84	P	-0.18	-0.9	-0.18	-7.51	0.35	4.80
OH_Top	Overhang Top h to 2•h	5,6	29.000	58.000	0.0	P	-0.18	-0.5	-0.18	-3.15	0.35	4.80
OH_Top	Overhang Top ≥ 2•h	5,6,8	58.000	77.000	0.0	P	-0.18	-0.3	-0.18	-0.96	0.35	4.80
Roof	Roof 0 to h	1,2	1.500	29.000	0.0	P	-0.18	-0.9	-0.18	-7.51	0.35	4.80
Roof	Roof h to 2•h	1,2	29.000	58.000	0.0	P	-0.18	-0.5	-0.18	-3.15	0.35	4.80
Roof	Roof ≥ 2•h	1,2	58.000	75.500	0.0	P	-0.18	-0.3	-0.18	-0.96	0.35	4.80
Soffit	Soffit Bottom	7	All	All	0.0	N	-0.18	0.8	0.8	11.04	11.04	4.80

Roof Pressures based upon Ch 27:

Component = The building component for pressures

Start = Start Dist from Windward Edge

C_{pMin} = Smallest Coefficient Magnitude

P_{min} = $q_h \cdot K_d \cdot G \cdot C_{pMin} - q_{ip} \cdot K_d \cdot GC_{piE:27.3-1}$

GC_{pi} = +Internal Coef $T:26.13-1$

P_{min} = Min Press projected on vertical plane $\S 27.1.5$

$\S 27.1.5$ = MWFRS Min Wall Pressure = 9.60 psf

-Press = Pressure Acting Away from Surface

Location = Reference Graphic in Output for Values

End = End Dist from Windward Edge

C_{pMax} = Largest Coefficient Magnitude

P_{max} = $q_h \cdot K_d \cdot G \cdot C_{pMax} - q_{in} \cdot K_d \cdot GC_{piE:27.3-1}$

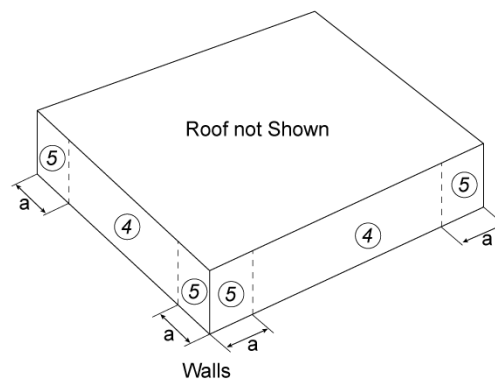
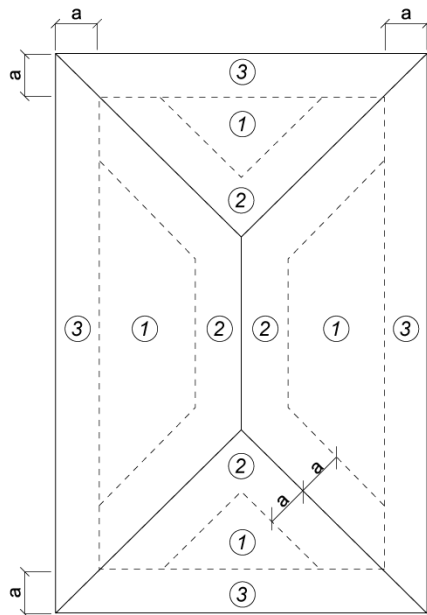
Basis = P=Parallel to Ridge: N=Normal to Ridge

θ = Roof Slope Relative to Wind

+Press = Pressure Acting Toward Surface

- The smaller uplift pressures due to C_{pMin} can become critical when wind is combined with roof live load or snow load; load combinations are given in ASCE 7

Components and Cladding (C&C) Wind Loads per Ch 30 Pt 1 Roof & Wall



h = Mean structure height = 29.000 ft
 K_{zt} = No Topographic Feature = 1.000
 GC_{pi} = ± Internal Press Coef $T:26.13-1$ = ±0.18
 K_e = Ground Elev Factor $T:26.10-1$ = 1.000
 θ = Slope of Roof = 44.84 °
 a = Max(a_1 , 0.04•B, 3 ft [0.9 m]) = 5.533 ft

K_h = $2.41 \cdot (Z/Z_g)^{2/\alpha}$ = 0.683
 K_d = Directionality Factor $T:26.6-1$ = 0.85
LF = ASD Load Factor = 0.60
 q_h = $.00256 \cdot K_h \cdot K_{zt} \cdot K_e \cdot V^2 \cdot LF_{E:26.10-1}$ = 15.11 psf
 a_1 = Min(0.1•B, 0.4•h) = 5.533 ft



C&C Wind Roof & Wall Detailed per Ch 30 Pt 1
All wind pressures include a Load Factor (LF) of 0.6

Description	Zone	Width ft	Span ft	Area ft ²	1/3 Rule	Reference	GC _{pi}	GC _{pd}	GC _{pu}	P _{down} psf	P _{uplift} psf
Zone 1	1	1.0000	1.0000	1.00	No	F:30.3-2G	±0.18	0.70	-1.50	11.30	-21.56
Zone 2	2	1.0000	1.0000	1.00	No	F:30.3-2G	±0.18	0.70	-1.80	11.30	-25.45
Zone 3	3	1.0000	1.0000	1.00	No	F:30.3-2G	±0.18	0.70	-2.40	11.30	-33.08
Zone 4	4	1.0000	1.0000	1.00	No	F:30.3-1	±0.18	1.00	-1.10	15.15	-16.44
Zone 5	5	1.0000	1.0000	1.00	No	F:30.3-1	±0.18	1.00	-1.40	15.15	-20.29

GC_{pd} = Down (+) External Coefficient
P_{down} = $q_h \cdot K_d \cdot [GC_{pd} - GC_{pi}]$ [E:30.3-1]
+Press = Pressure Acting Toward Surface
§30.2.2 = C&C Min Pressure = 9.60 psf
Width = Width of Component
Area = Span • Width
GC_{pi} = Internal Coef T:26.13-1

GC_{pu} = Uplift (-) External Coefficient
P_{uplift} = $q_h \cdot K_d \cdot [GC_{pu} - GC_{pi}]$ [E:30.3-1]
-Press = Pressure Acting Away from Surface
Zone = Applicable Zone per Figure
Span = Span of Component
1/3 Rule = Width limited to Span/3
Reference = Applicable Reference from Standard

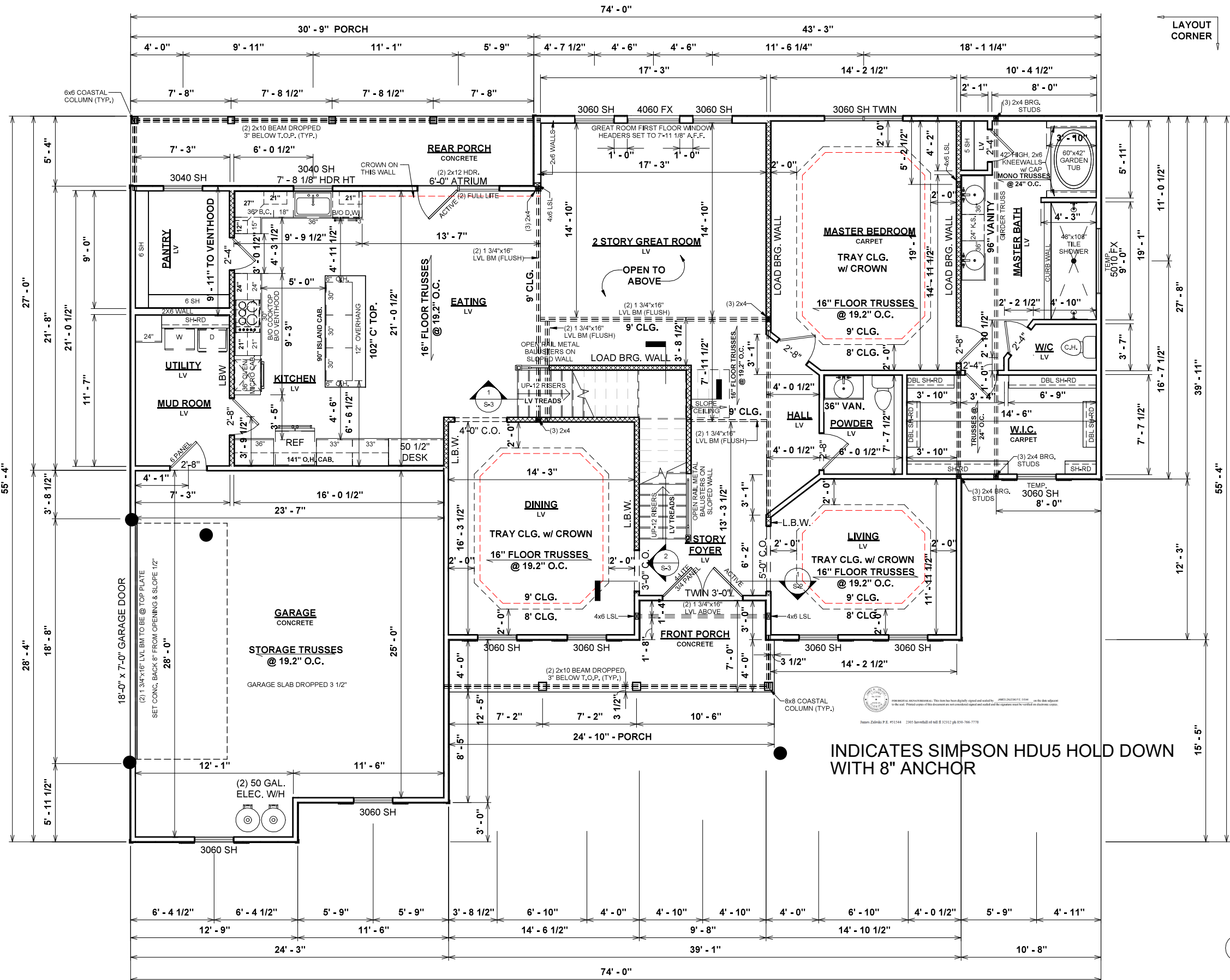
C&C Wind Roof & Wall Summary per Ch 30 Pt 1

Zone	Reference	P _{max} A ≤ 10 ft ² psf	P _{min} A ≤ 10 ft ² psf	P _{max} A = 20 ft ² psf	P _{min} A = 20 ft ² psf	P _{max} A = 50 ft ² psf	P _{min} A = 50 ft ² psf
1	F:30.3-2G	11.30	-21.56	9.75	-18.48	9.60	-14.40
2	F:30.3-2G	11.30	-25.45	9.75	-21.58	9.60	-16.47
3	F:30.3-2G	11.30	-33.08	9.75	-27.69	9.60	-20.55
4	F:30.3-1	15.15	-16.44	14.47	-15.75	13.57	-14.85
5	F:30.3-1	15.15	-20.29	14.47	-18.92	13.57	-17.12
1_OHS	F:30.3-2G/F:30.3-1	9.60	-34.40	9.60	-30.63	9.60	-25.65
2_OHS	F:30.3-2G/F:30.3-1	9.60	-38.29	9.60	-33.74	9.60	-27.73
3_OHS	F:30.3-2G/F:30.3-1	9.60	-45.92	9.60	-39.84	9.60	-31.81

Zone	Reference	P _{max} A = 100 ft ² psf	P _{min} A = 100 ft ² psf	P _{max} A = 200 ft ² psf	P _{min} A = 200 ft ² psf	P _{max} A > 500 ft ² psf	P _{min} A > 500 ft ² psf
1	F:30.3-2G	9.60	-11.31	9.60	-11.31	9.60	-11.31
2	F:30.3-2G	9.60	-12.61	9.60	-12.61	9.60	-12.61
3	F:30.3-2G	9.60	-15.15	9.60	-15.15	9.60	-15.15
4	F:30.3-1	12.88	-14.17	12.20	-13.49	11.30	-12.58
5	F:30.3-1	12.88	-15.75	12.20	-14.39	11.30	-12.58
1_OHS	F:30.3-2G/F:30.3-1	9.60	-21.89	9.60	-21.20	9.60	-20.30
2_OHS	F:30.3-2G/F:30.3-1	9.60	-23.18	9.60	-22.50	9.60	-21.60
3_OHS	F:30.3-2G/F:30.3-1	9.60	-25.73	9.60	-25.04	9.60	-24.14

P_{max} = Maximum Pressure
Area = Span • Width
Span = Span of Component
§30.2.2 = C&C Min Pressure = 9.60 psf

P_{min} = Minimum Pressure
Width = Width of Component
Reference = Applicable Reference from Standard
Interpolate = Interpolate for Areas between columns



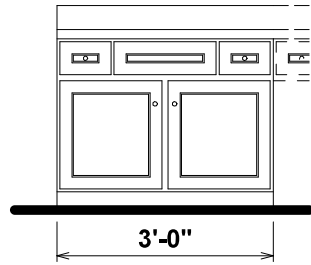
1 FIRST FLOOR PLAN
1/8" = 1'-0"

GENERAL NOTES:

- MINIMUM 9'-1 1/8" CEILING HEIGHT ON FIRST FLOOR U.N.O.
- ALL LOAD BEARING WALLS AND EXTERIOR OPENINGS TO HAVE (2) 2x10 HEADERS U.N.O.
- STANDARD FIRST FLOOR WINDOW HEADER HEIGHTS SHALL BE FRAMED DOWN 2'-2" FROM T.O.P., U.N.O.
- WINDOW HEADERS FOR WINDOWS LOCATED ABOVE KITCHEN SINK SHALL BE FRAMED DOWN 1'-5" FROM T.O.P., U.N.O.
- STANDARD SECOND FLOOR WINDOW HEADER HEIGHTS SHALL BE FRAMED DOWN 1'-2" FROM T.O.P., U.N.O.
- 7/16" O.S.B. AND HOUSEWRAP REQUIRED.
- DIMENSIONS ARE TO SHEATHING EXTERIOR; SUBTRACT 1/2" FROM DIMENSIONS FOR EXTERIOR WINDOW AND DOOR FRAMING LOCATION IF OPENINGS ARE FRAMED BEFORE SHEATHING INSTALLATION.
- ALL INTERIOR DOORS ARE EITHER CENTERED ON WALLS OR ROUGH OPENING STARTED MINIMUM OF 4" FROM ADJOINING WALL UNLESS OTHERWISE DIMENSIONED.
- NUMBER OF STAIR TREADS AND RISERS MAY VARY AS A RESULT OF LOCAL BUILDING CODES, STANDARDS AND FINAL GRADE.
- ALL SHELVES TO BE 12" DEEP U.N.O.
- HEIGHT OFF FLOOR TO BE:
 - SINGLE 68"
 - DOUBLE 42" AND 84"
- ALL PLUMBING FIXTURES SHOWN ARE A REPRESENTATION OF SIZE AND LOCATION ONLY. ACTUAL STYLE AND BRAND OF FIXTURES MAY VARY PER OFFICE LOCATION.
- ALL TUBS / SHOWERS TO HAVE NAILERS AT FLANGE.
- INSTALL A 24" WIDE WALKWAY FROM ATTIC ACCESS TO FURNACE PLATFORM.
- PORCH, STOOP, & DECK HANDRAILS NOT INCLUDED W/ SLAB FOUNDATION.
- RAILINGS ARE A FORCED OPTION WHEN PORCH IS OVER 30" HIGH FROM FINISHED GRADE.
- SCHLUTER DITRA UNCOUPLING AND WATERPROOFING MEMBRANE TO BE USED ON ALL TILE SHOWER INSTALLATIONS

SPECIAL NOTES:

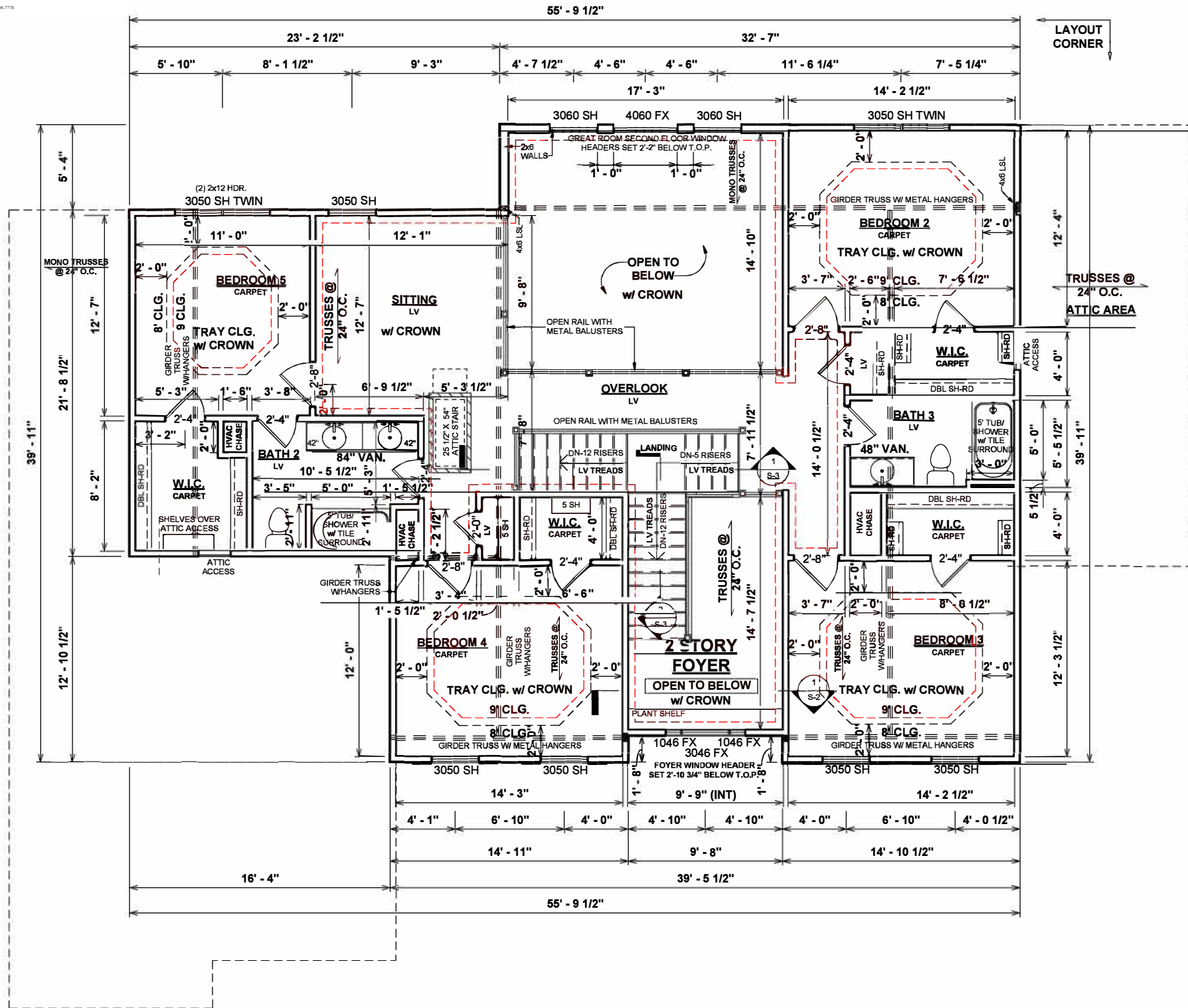
- UPGRADE UPPER CABINET TO BE 42" HIGH (UP TO CEILING)
- BY OWNER APPLIANCES:
 - DISHWASHER
 - COOKTOP
 - VENTHOOD



3 891.1 36" VANITY ELEVATION
3/8" = 1'-0"

PRELIMINARY CONSTRUCTION PLANS

SHEET #		PROPERTY OF:	DRAWN BY:	JOB #	THE:	AREAS:					REVISION SCHEDULE		
F-2		 © COPYRIGHT - 2024	CLAUDIO S.	59-25-020		SOUTHAMPTON 'A'	FIRST FLOOR HEATED	2,321 SF					
			CHECKED BY:	2x4 EXTERIOR WALLS	FOR:	REGINALD & WENDY DAVIS	SECOND FLOOR HEATED	1,499 SF					
			C.D.				FRONT PORCH	3,821 SF					
			PRINTED:	FOUNDATION TYPE:			GARAGE	128 SF					
			8/22/2025	MONOSLAB			REAR PORCH	648 SF					
			9:16:13 AM				TOTAL UNDER ROOF	164 SF					
					OFFICE:	SOLD BY:	4,761 SF						
FIRST FLOOR PLAN					GAINESVILLE, FL		L. HALLER						

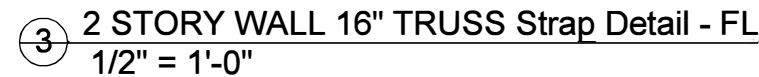
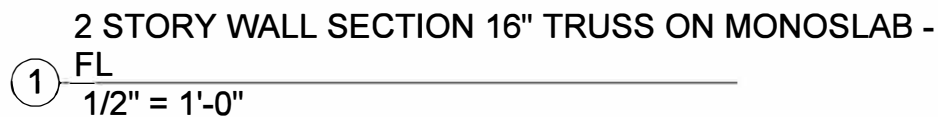


1 SECOND FLOOR PLAN
1/8" = 1'-0"

GENERAL NOTES:

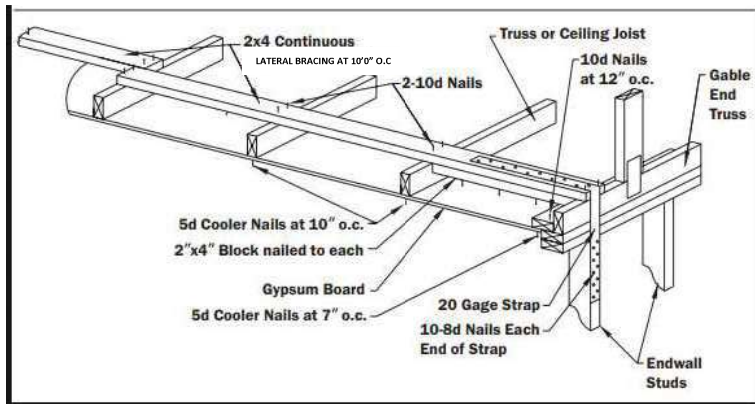
- MINIMUM 9'-1 1/8" CEILING HEIGHT ON FIRST FLOOR U.N.O.
- ALL LOAD BEARING WALLS AND EXTERIOR OPENINGS TO HAVE (2) 2x10 HEADERS U.N.O.
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- WINDOW HEADERS FOR WINDOWS LOCATED ABOVE KITCHEN SINK SHALL BE FRAMED DOWN 1'-5" FROM T.O.P., U.N.O.
- STANDARD SECOND FLOOR WINDOW HEADER HEIGHTS SHALL BE FRAMED DOWN 1'-2" FROM T.O.P., U.N.O.
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- RAILINGS ARE A FORCED OPTION WHEN PORCH IS OVER 30" HIGH FROM FINISHED GRADE.
- SCHLUTER DITRA UNCOUPLING AND WATERPROOFING MEMBRANE TO BE USED ON ALL TILE SHOWER INSTALLATIONS

REVISION SCHEDULE		REV #	DESCRIPTION	DATE
AREAS:		FIRST FLOOR HEATED	2,321 SF	
		SECOND FLOOR HEATED	1,499 SF	
		FRONT PORCH	3,821 SF	
		GARAGE	128 SF	
		REAR PORCH	648 SF	
		TOTAL UNDER ROOF	164 SF	
			940 SF	
			4,761 SF	
THE:		SOUTHAMPTON 'A'		
FOR:		REGINALD & WENDY DAVIS		
JOB #		59-25-020		
DRAWN BY:		CLAUDIO S.		
CHECKED BY:		C.D.		
PRINTED:		8/22/2025 9:16:14 AM		
PROPERTY OF:		AMERICA'S HOME PLACE		
SHEET #		F-3		
		© COPYRIGHT - 2024		
		OFFICE: GAINESVILLE, FL		
		SOLD BY: L. HALLER		
		242 SW GOLDBOLD AVE. LAKE CITY, FL 32024		
		FOUNDATION TYPE: MONOSLAB		
		SECOND FLOOR PLAN		



2 STORY WALL SECTIONS - FL

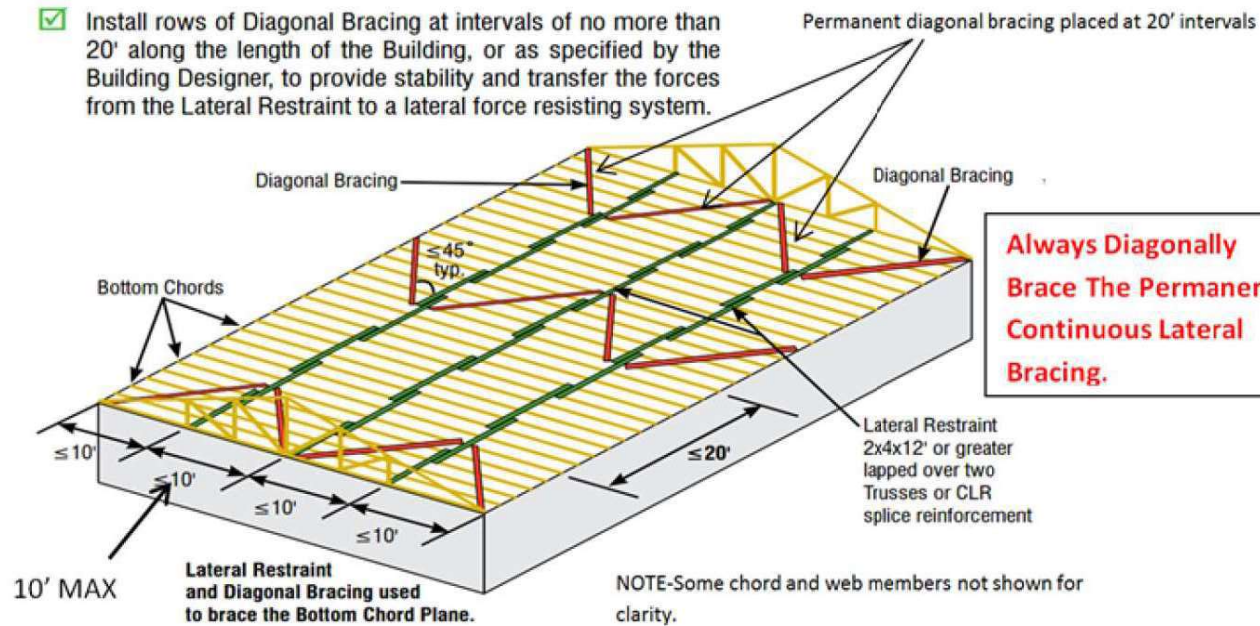
GABLE END BRACING



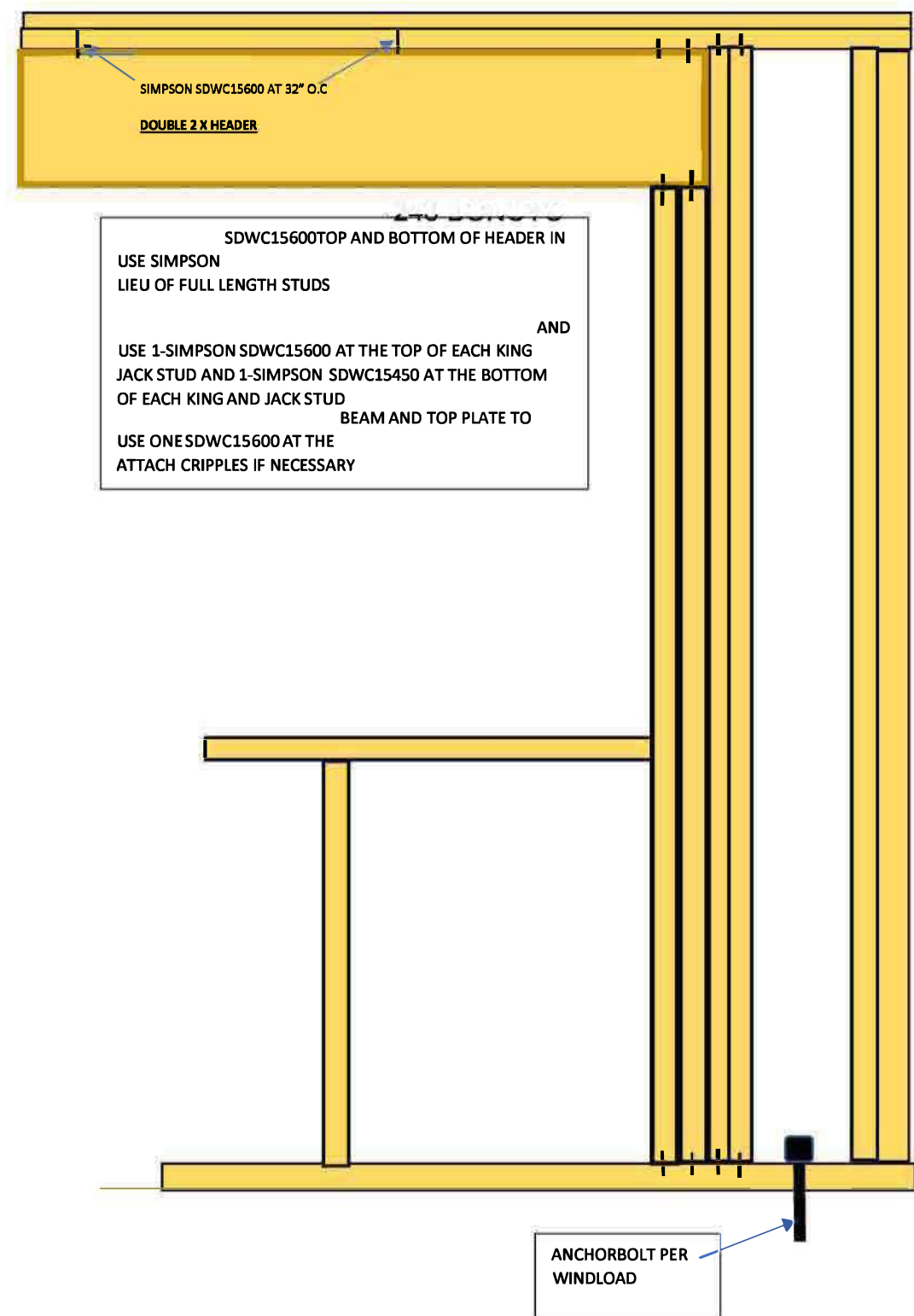
This item has been digitally signed and sealed by James A Zaleski on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies



- ✓ Install rows of Diagonal Bracing at intervals of no more than 20' along the length of the Building, or as specified by the Building Designer, to provide stability and transfer the forces from the Lateral Restraint to a lateral force resisting system.



Always Diagonally Brace The Permanent Continuous Lateral Bracing.

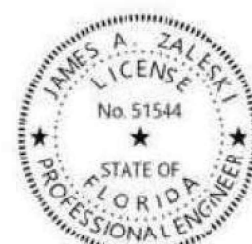


HEADER SIZE AND STRAPPING CHART

SPAN	HEADER SIZE	QUANTITY OF JACK STUDS AT EACH END	QUANTITY OF KING STUDS AT EACH END	STRAPPING TO JACK STUDS AT EACH END TOP AND BOTTOM	STRAPPING TO KING STUDS AT EACH END TOP AND BOTTOM
0'-0" TO 7'-6"	2 - 2X10" WITH ½" PLATE	1	1	1 SIMPSON MSTA24	1 SIMPSON SPH4
7'-6" – 11'-3"	2 - 2X12" WITH ½" PLATE OR 4-2 X 10" WITH ½" PLATE	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4
11'-3" – 14'-0"	2- 1 ¾" X 9 ¼" LVL	3	2	2 SIMPSON MSTA24	2 SIMPSON SPH4

IN LIEU OF STRAPPING USE A SDWC15600 AT THE TOP OF EACH JACK AND KING STUD AND ONE SDWC15450 AT THE BASE OF EACH JACK AND KING STUD

AT ALL CORNERS USE 3- SIMPSON SDWC1540 SCREWS AT BASE AND 3- SIMPSON SDWC15600 AT TOP – IF NECESSARY, USE 2-SCREWS IN ONE STUD AND ADD ONE MORE STUD MIDSPAN



FOR DIGITAL SIGNATURES/SEAL: This item has been digitally signed and sealed by JAMES ZALESKI P.E. 51544 on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on electronic copies.