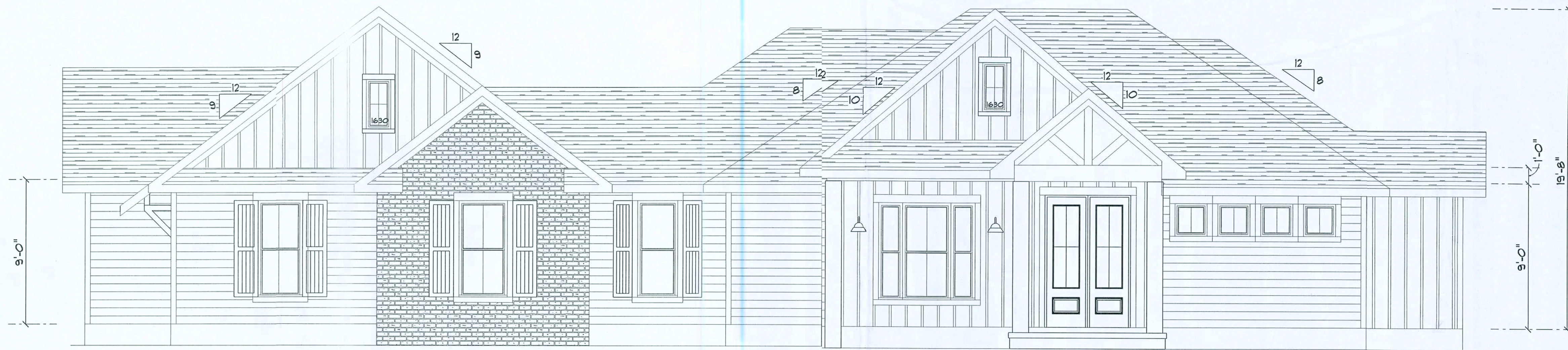
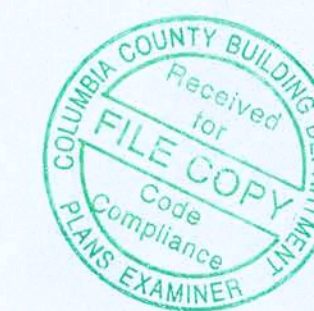


REAR ELEVATION  
SCALE: 1/4" = 1'-0"



FRONT ELEVATION  
SCALE: 1/4" = 1'-0"



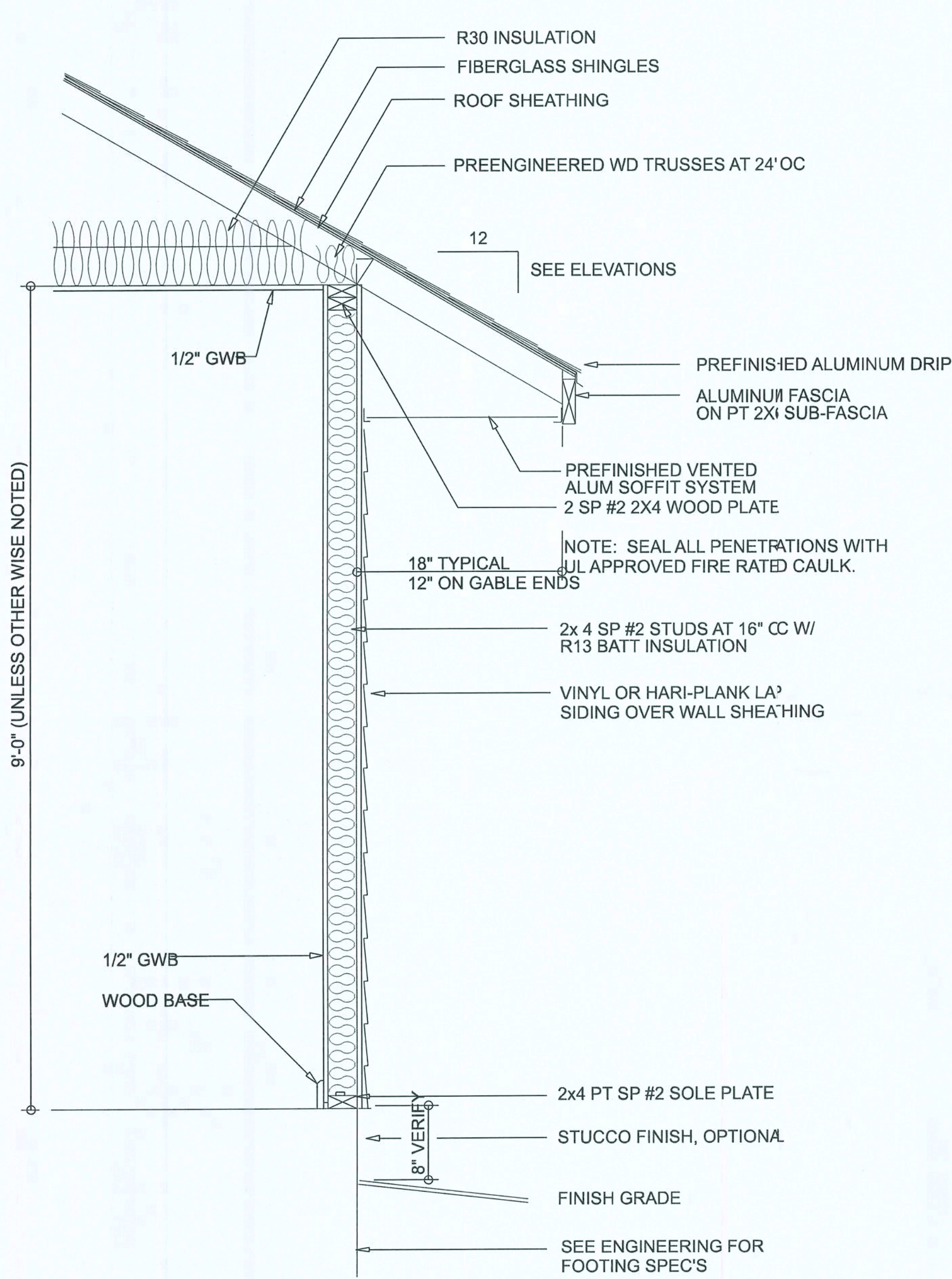
REVISIONS SCHEDULE			
SEPT. 19th, 2019	FINAL DRAWINGS		
NOV. 19th, 2019	ADDED BONUS RM.		

COLUNGA RESIDENCE  
LAKE CITY, FLORIDA

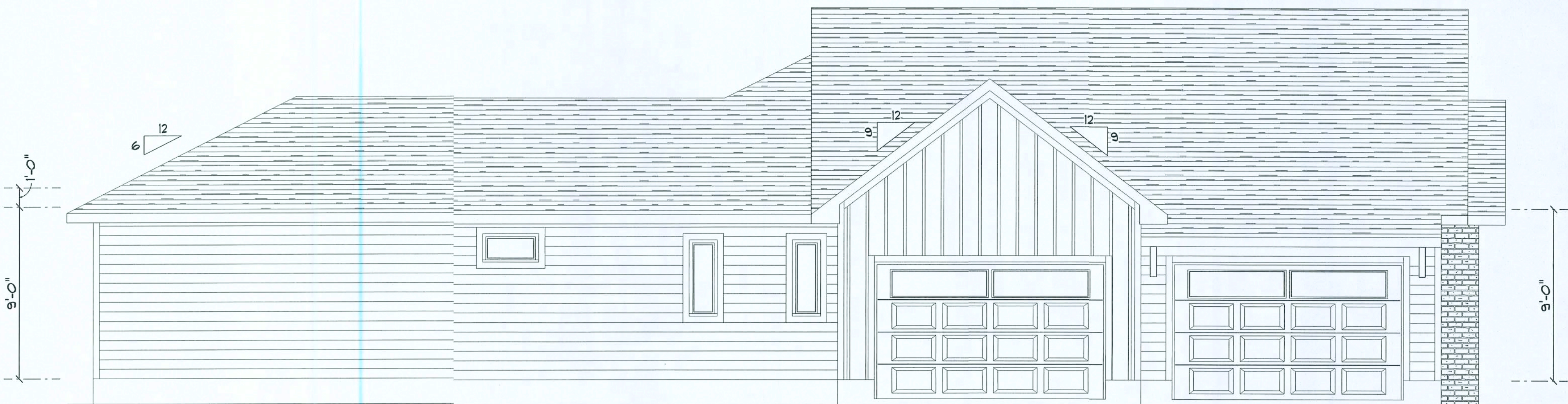
**RIDGEPOINT  
DESIGN**  
818 WEST DUVAL STREET, LAKE CITY, FLORIDA 32055  
P: 386-288-1188  
E: RIDGEPOINTDESIGN@GMAIL.COM

SHEET NUMBER  
  
OF 4 SHEETS

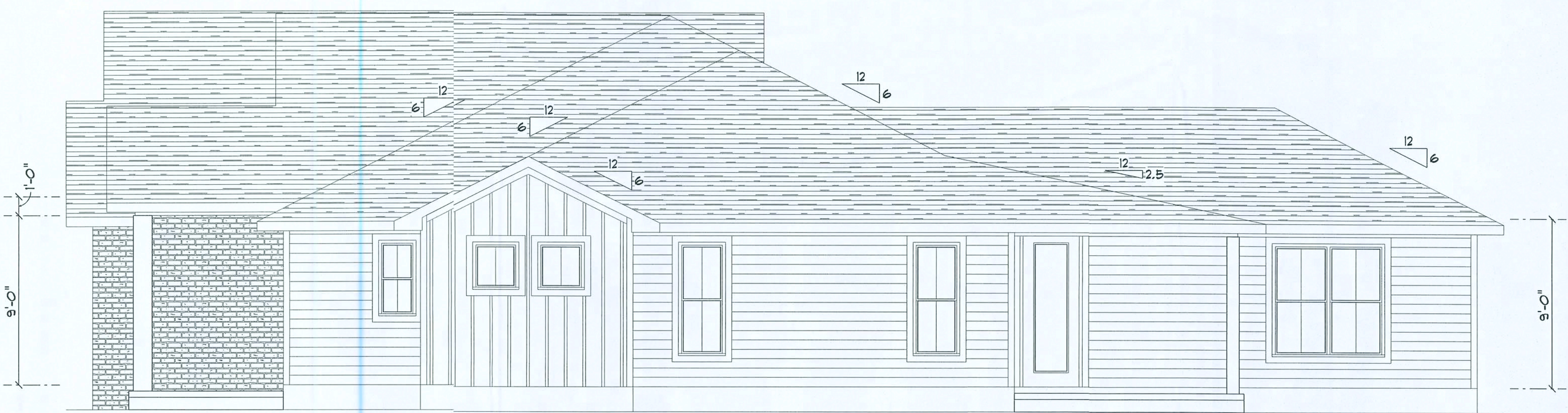




**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"




**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"

REVISIONS SCHEDULE		
SEPT. 19th, 2019	FINAL DRAWINGS	
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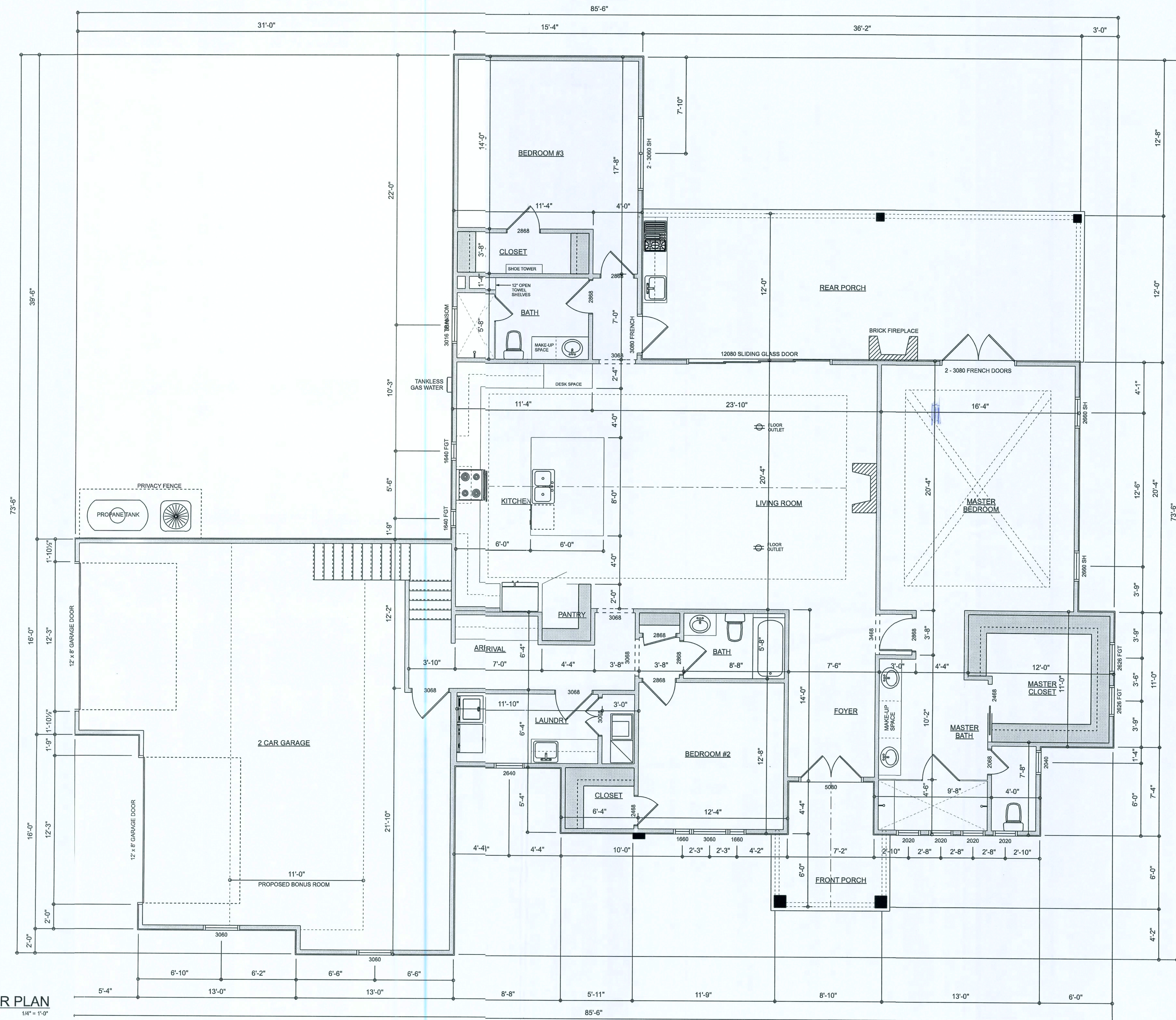
**RIDGEPOINT  
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# AREA SUMMARY

HEATED & COOLED	2,343	S.F.
BONUS ROOM	352	S.F.
LIVING AREA	2,695	S.F.
GARAGE	898	S.F.
FRONT PORCH	86	S.F.
REAR PORCH	434	S.F.
TOTAL AREA	4,113	S.F.

**DIMENSIONED FLOOR PLAN**  
SCALE: 1/4" = 1'-0"



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SEPT. 19th, 2019	FINAL DRAWINGS
NOV. 19th, 2019	ADDED BONUS RM.

## COLUNGA RESIDENCE

LAKE CITY, FLORIDA



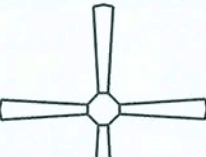


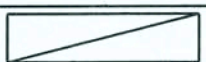



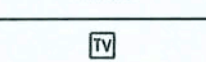







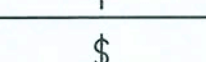
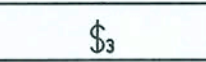




010 WEST DUVAL STREET, LAKE CITY, FLORIDA 32033  
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SHEET NUMBER

**A.3**

OF 4 SHEETS



ELECTRICAL LEGEND		
ELECTRICAL	COUNT	SYMBOL
CEILING FAN	6	
CAN LIGHT 6inch	31	
CHANDELIER	1	
FLUORESCENT LIGHT 1x4	4	
PENDANT LIGHT	2	
EXTERIOR SCENCE	3	
MOTION SECURITY LIGHT	5	
ELECTRIC PANEL	1	
CABLE TV OUTLET	5	
EXHAUST FAN	3	
FLOOR OULET DUPLEX	2	
LAN CONNECTION	5	
OUTLET	42	
OUTLET 220v	4	
OUTLET GFI	17	
OUTLET WP	4	
SMOKE DETECTOR	6	
STANDARD LIGHT	4	
SWITCH	26	
SWITCH 3 WAY	19	
VANITY BAR LIGHT - SMALL	5	

**ELECTRICAL PLAN NOTES:**

INSTALLATION SHALL BE PER 2017 NAT'L ELECTRIC CODE.

WIRE ALL APPLIANCES, HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS

CONSULT WITH THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED

ALL SMOKE DETECTORS SHALL BE 120v W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE, AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS

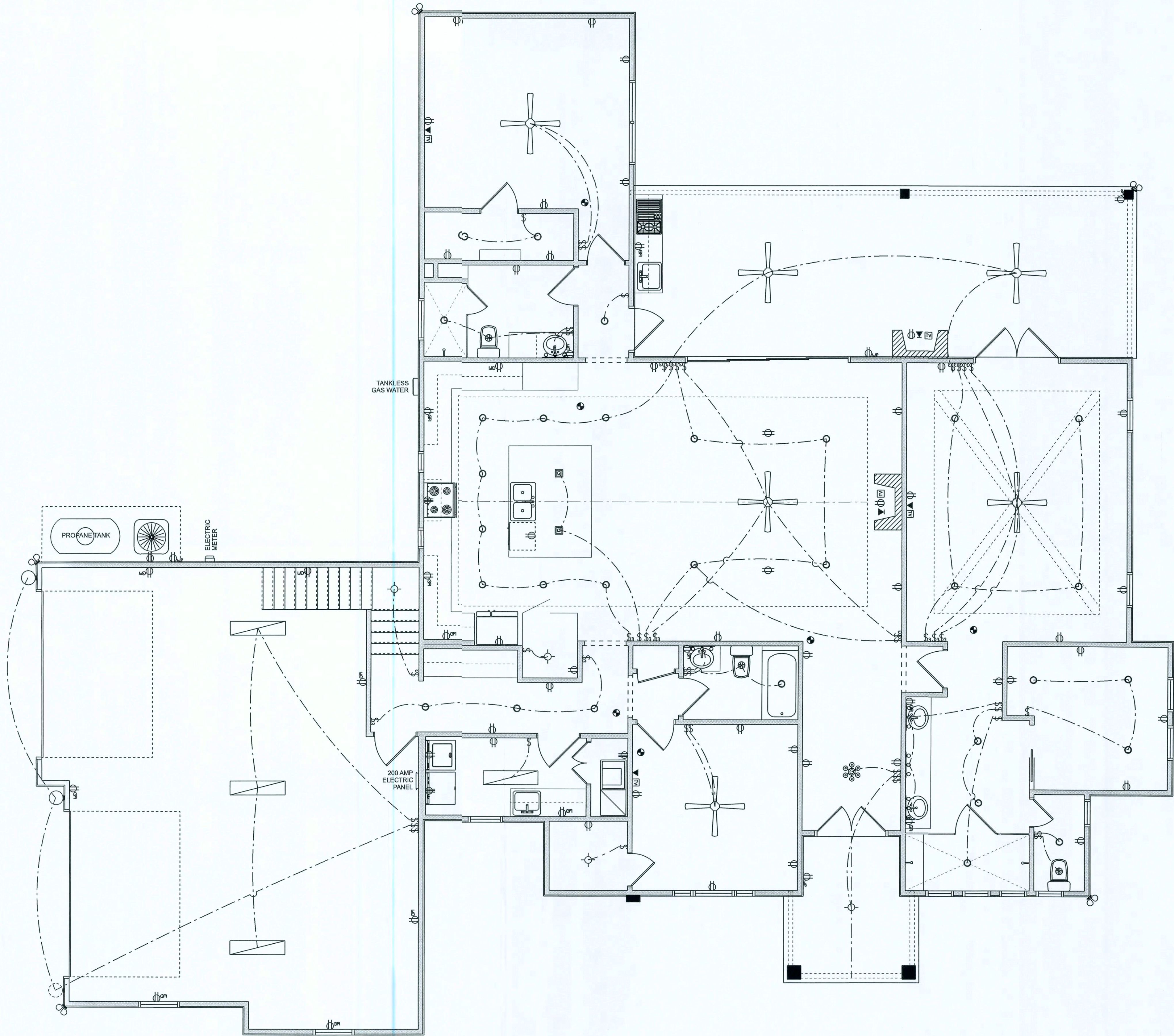
TELEPHONE, TELEVISION AND OTHER LOW VOLTAGE DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S DIRECTIONS. & IN ACCORDANCE W/ APPLICABLE SECTIONS OF NEC-LATEST EDITION.

ALL RECEPTALS, NOT OTHERWISE NOTED, SHALL BE ARC FAULT INTERRUPTER TYPE, EXCEPT DEDICATED OUTLETS

ALL RECEPTALS IN WET AREAS SHALL BE GROUND FAULT INTERRUPTER TYPE (GFI)

ALL EXTERIOR RECEPTALS SHALL BE WEATHERPROOF GROUD FAULT INTERRUPTER TYPE (WP/GFI)

NOTE:  
ELECTRICAL CONTR SHALL PREPARE "AS-BUILT" SHOP DWGS INDICATING ALL ELECTRICAL WORK, INCLUDING ANY CHANGES TO THE ELEC. PLAN, ADD'NS TO THE ELEC. PLAN, RISER DIAGRAM, AS-BUILT PANEL SCHEDULE W/ ALL CKTS IDENTIFIED W/ CKT Nr. DESCRIPTION & BRKR. SERVICE ENT. & ALL UNDERGROUND WIRE LOCATIONS/ROUTING / DEPTH. RISER DIA. SHALL INCLUDE WIRE SIZES/TYPE & EQUIPMENT TYPE W/ RATINGS & LOADS.  
CONTRACTOR SHALL PROVIDE 1 COPY OF AS-BUILT DWGS TO OWNER & 1 COPY TO THE PERMIT ISSUING AUTHORITY



**ELECTRICAL PLAN**  
SCALE: 1/4" = 1'-0"

REVISIONS SCHEDULE		
SEPT. 19th, 2019	FINAL DRAWINGS	
NOV. 19th, 2019	ADDED BONUS RM.	

RIDGEPOINT  
DESIGN

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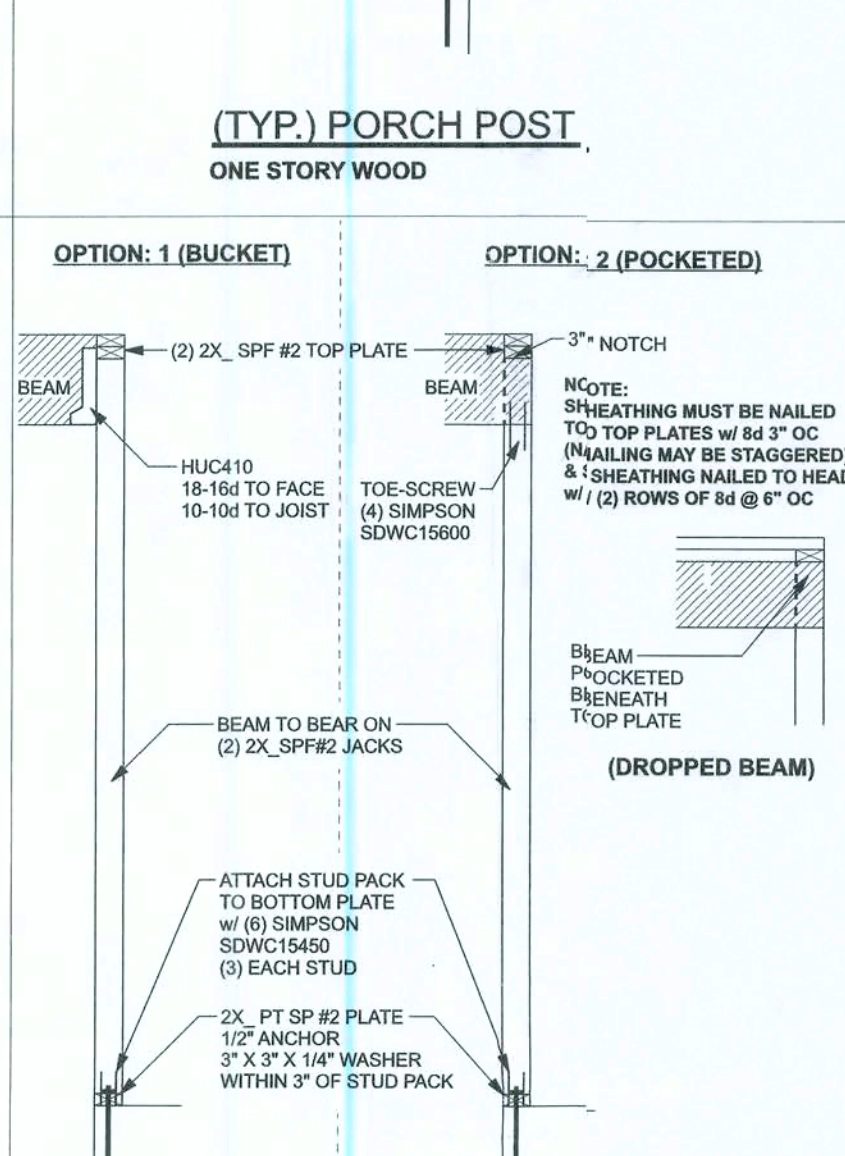
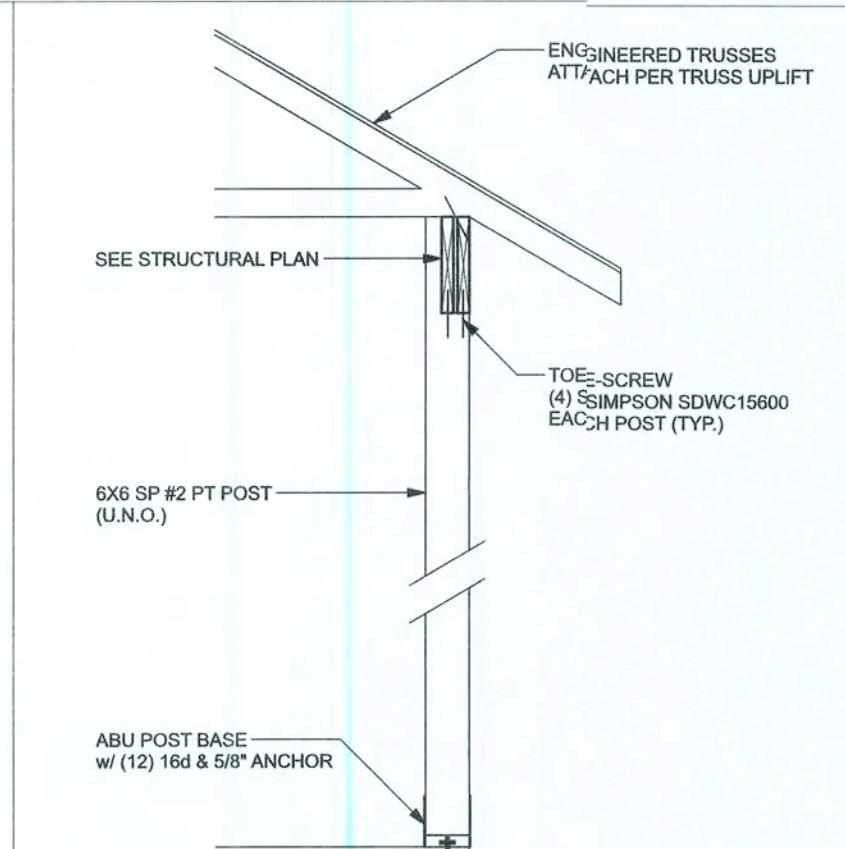
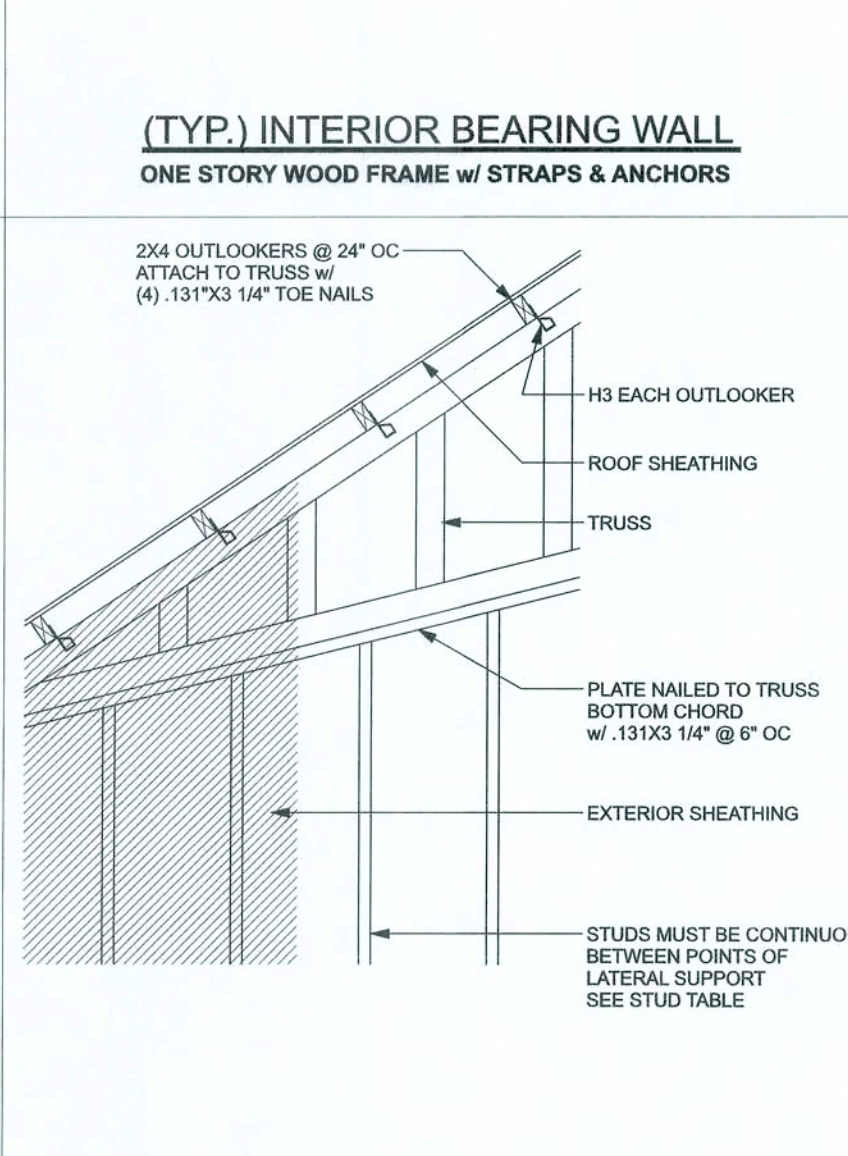
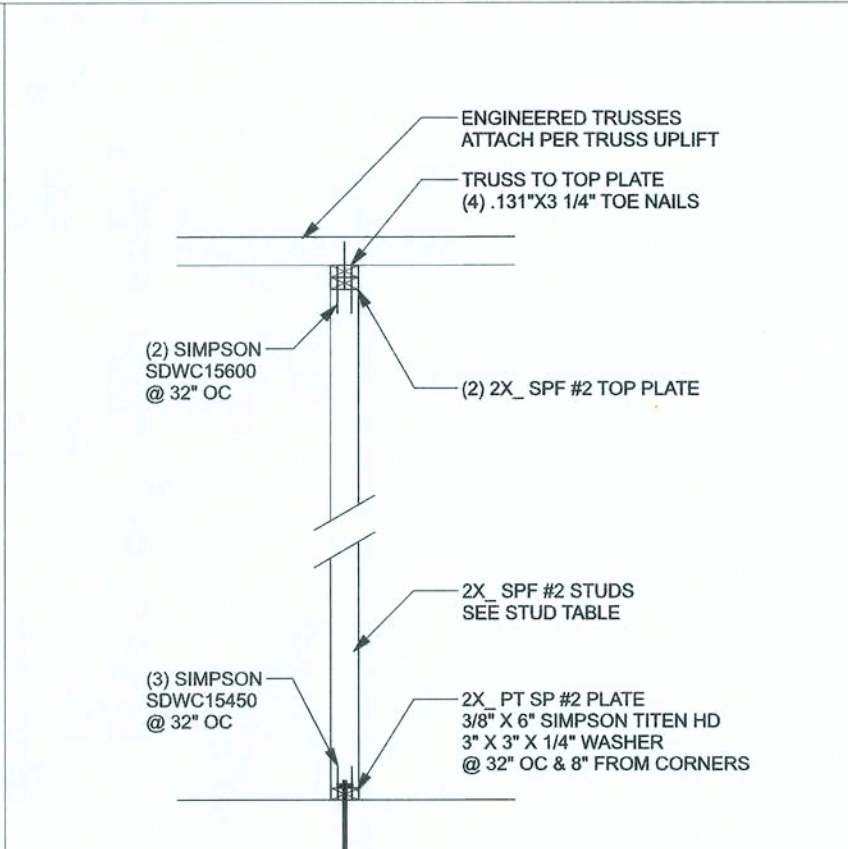
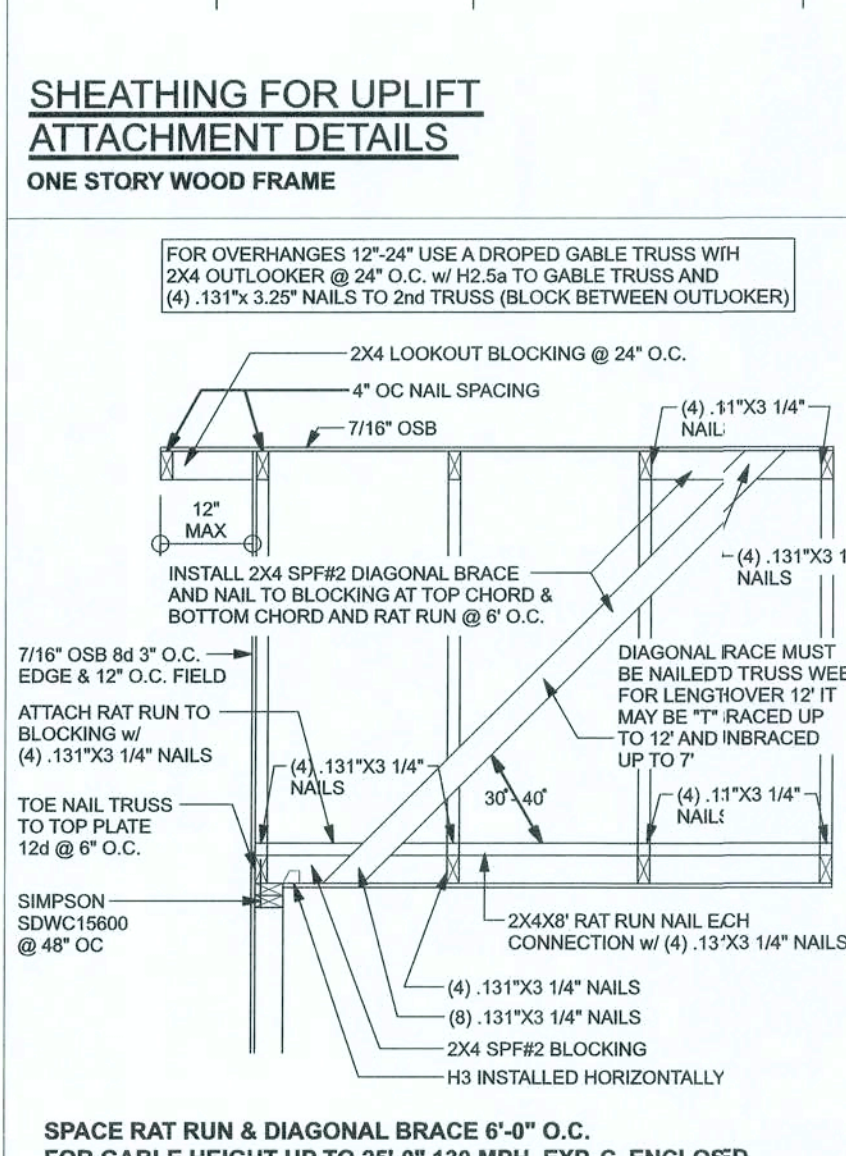
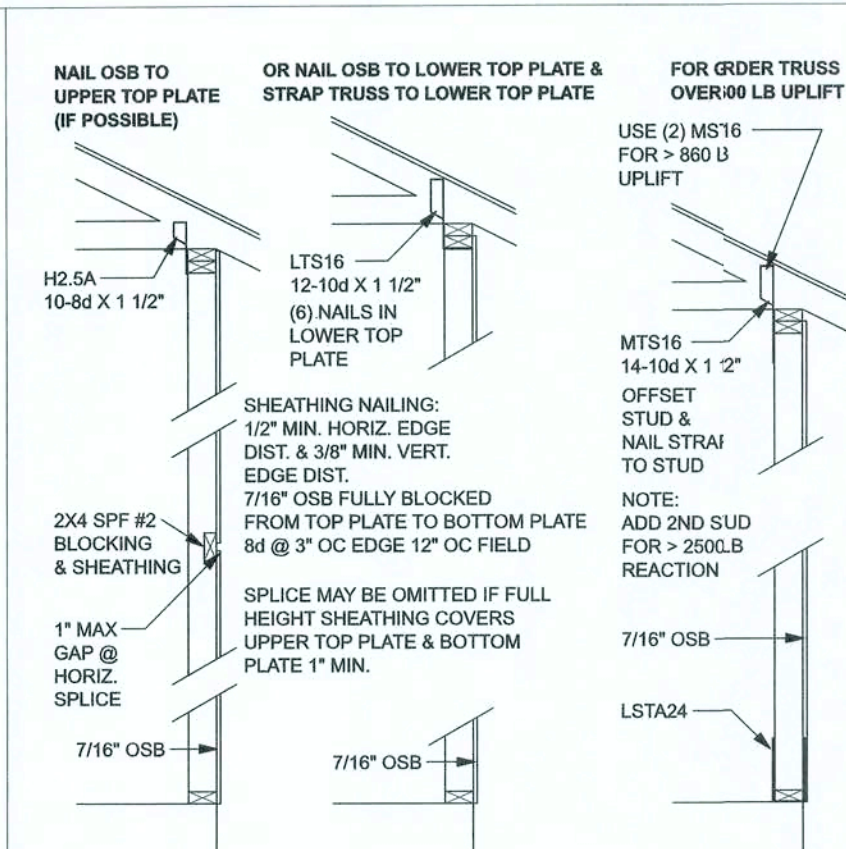
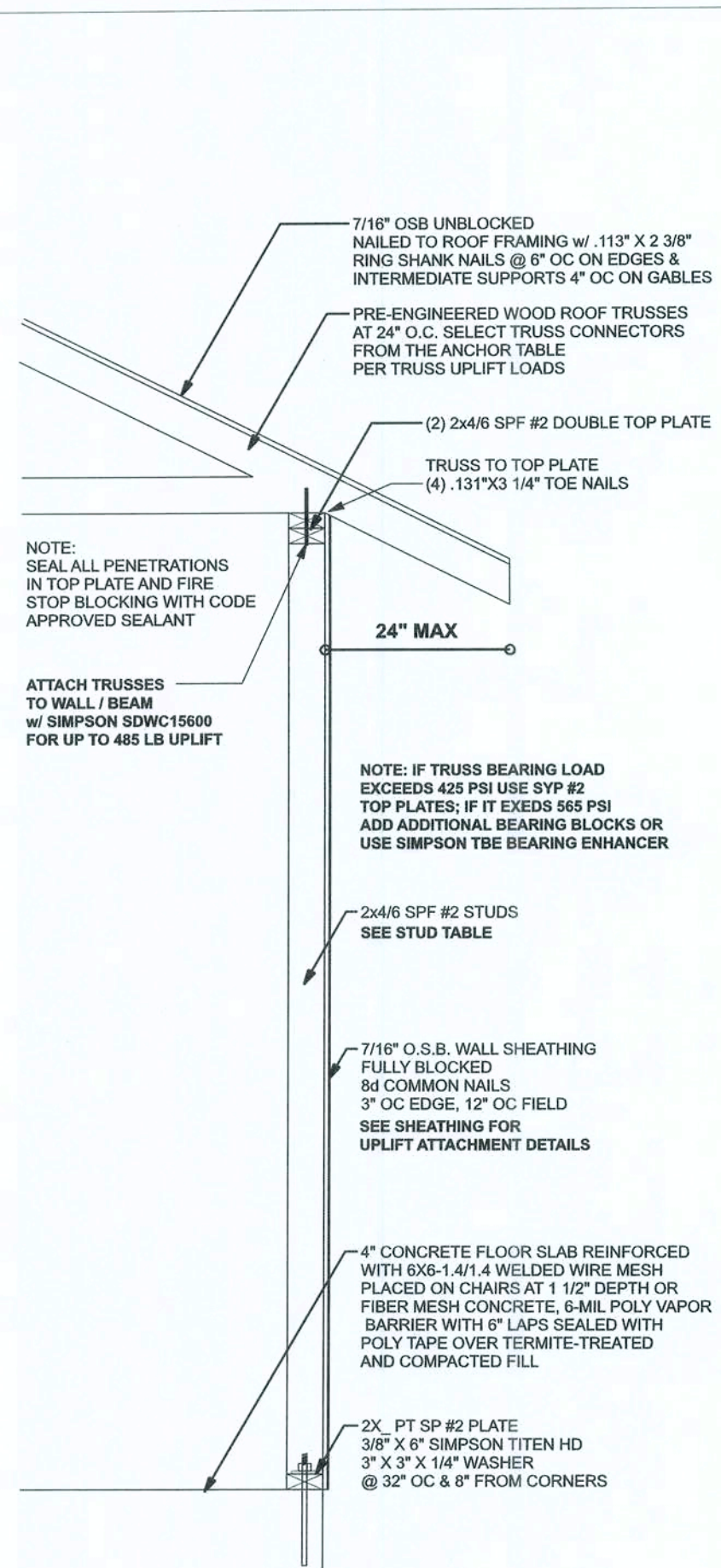
COLUNGA RESIDENCE  
LANE CITY, FLORIDA

RIDGEPOINT  
DESIGN

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COLUNGA RESIDENCE  
LANE CITY, FLORIDA





CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	-	-	
415	290	H3	4-8dX1 1/2"	4-8dX1 1/2"	
575	495	H2.5A	5-8dX1 1/2"	5-8dX1 1/2"	
1340	1015	H10A	9-10d1 1/2"	9-10d1 1/2"	
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"	
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA21	8-10d	8-10d	
1640	1455	MSTA24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Holdowns @ Stemmwall	To Stud	To Plate	
585	535	SP1	6-10d	4-10d	
1065	605	SP2	6-10d	6-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stemmwall	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	18-18dX2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	18-18dX2 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stemmwall	To Post	Anchor	
2200	ABU44	ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300	ABU66	ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200	ABU44	ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300	ABU66	ABU66	12-16d	5/8"x7" Drill & Epoxy	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:					
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 12" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)					
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT				
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT				
(1) 2x6 @ 16" OC	TO 10'-1" STUD HEIGHT				
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT				

GRADE & SPECIES TABLE			
2x8	SP #2	Fb	E
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED WITH THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS,  $f'_c = 2500$  PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 86KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A186, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT: FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH/WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS,  $F_y = 40$  KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS), UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.O.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH .113" X 2 3/8" RING SHANK NAILS @ 6" OC ON EDGES & INTERMEDIATE SUPPORTS 4" OC ON GABLES

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

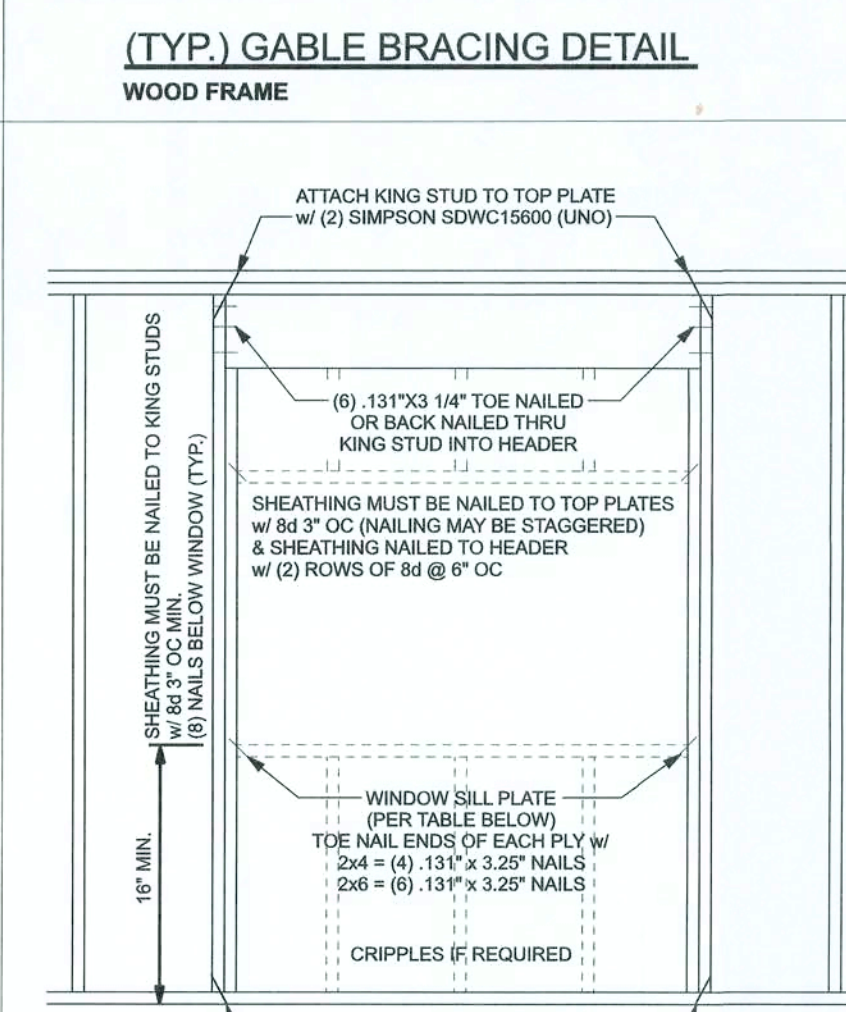
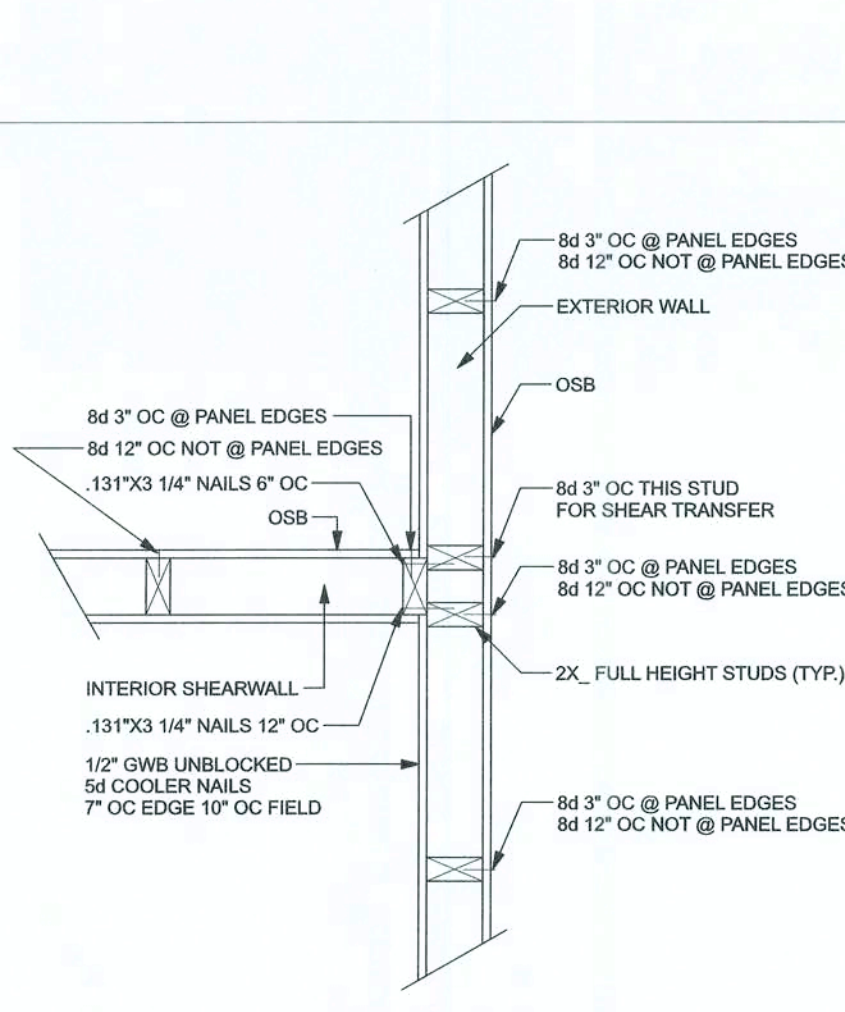
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.



**(TYP.) GABLE WALL w/ VAULTED CEILING**  
WOOD FRAME

2X4 OUTLOOKERS @ 24" OC ATTACH TO TRUSS w/ (4) .131"x3 1/4" TOE NAILS

H3 EACH OUTLOOKER

ROOF SHEATHING

TRUSS

PLATE NAILED TO TRUSS BOTTOM CHORD w/ .131"x3 1/4" @ 6" OC

EXTERIOR SHEATHING

STUDS MUST BE CONTINUOUS BETWEEN POINTS OF LATERAL SUPPORT SEE STUD TABLE

2X PT SP #2 PLATE 1/2" ANCHOR 3" X 3" X 1/4" WASHER WITHIN 3" OF STUD PACK

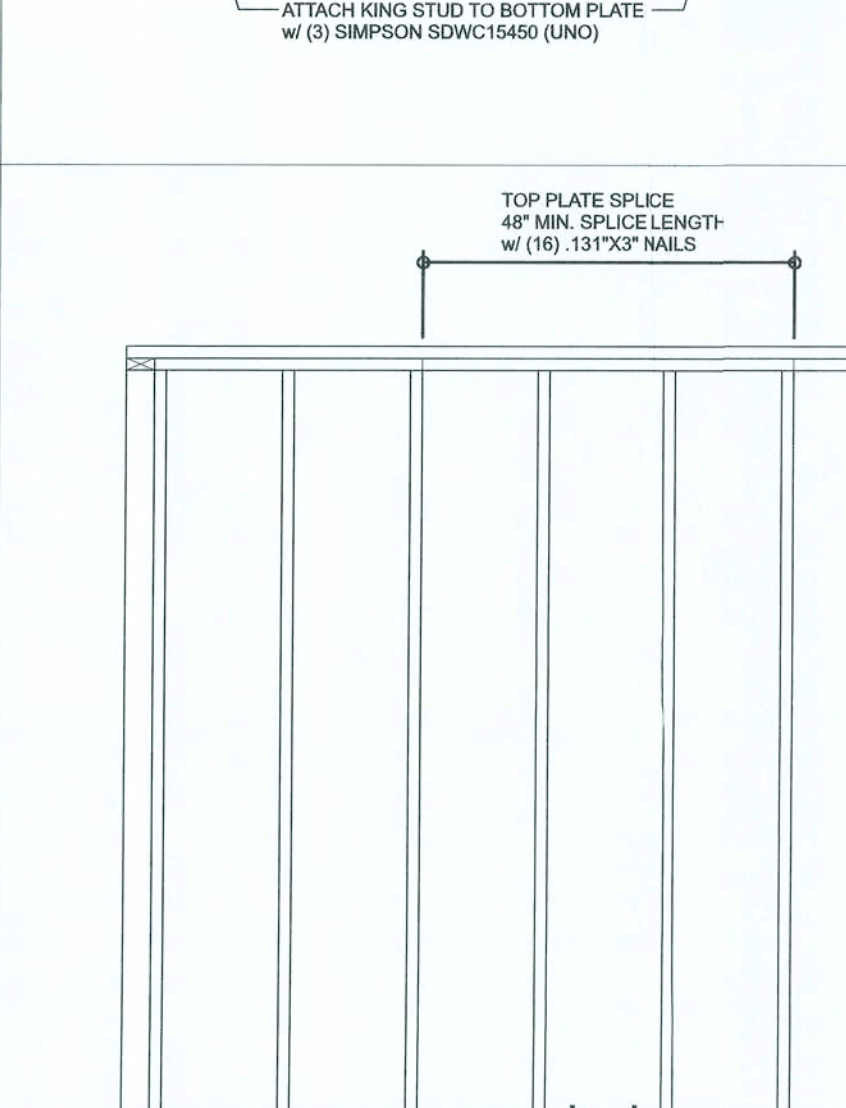
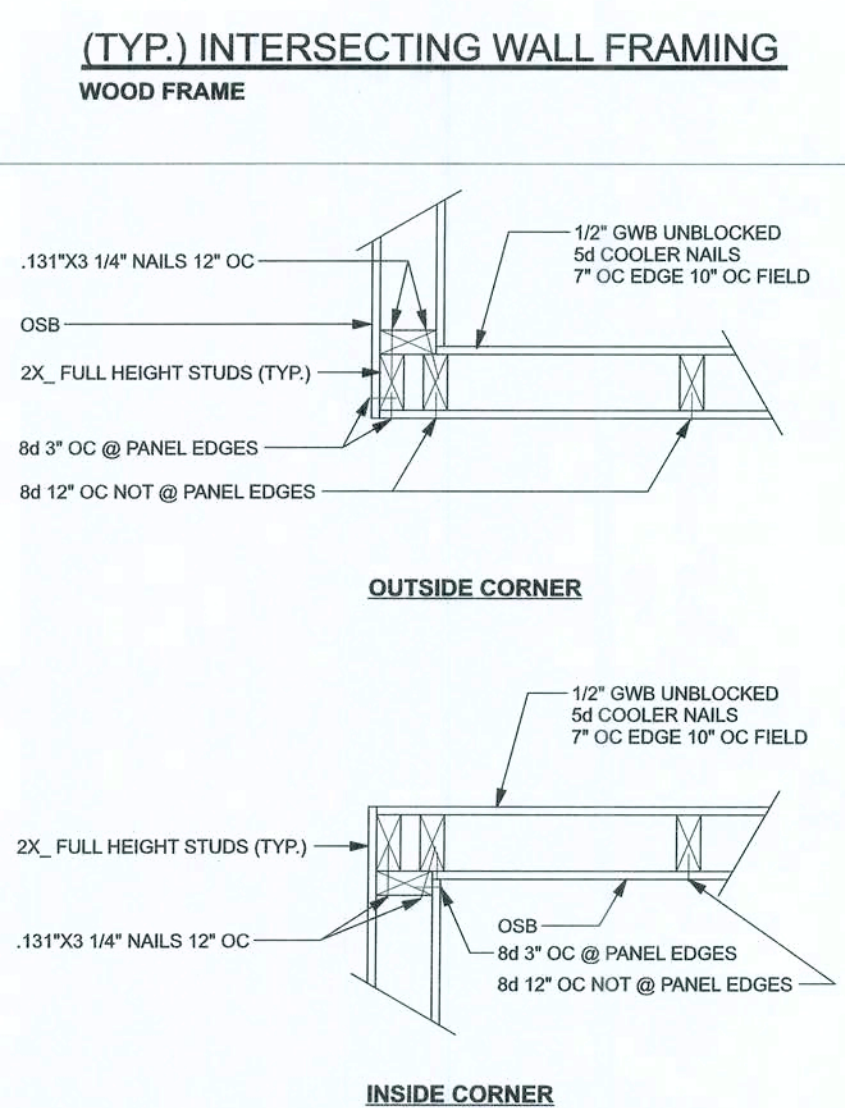
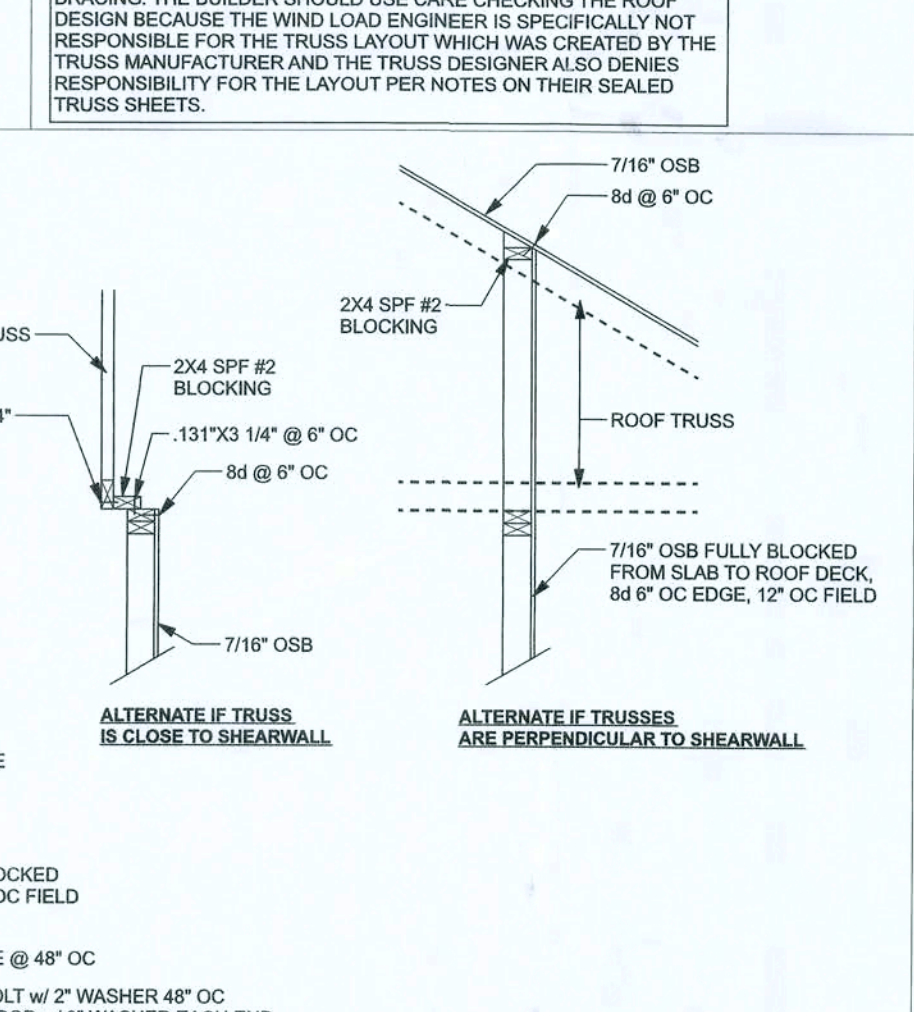
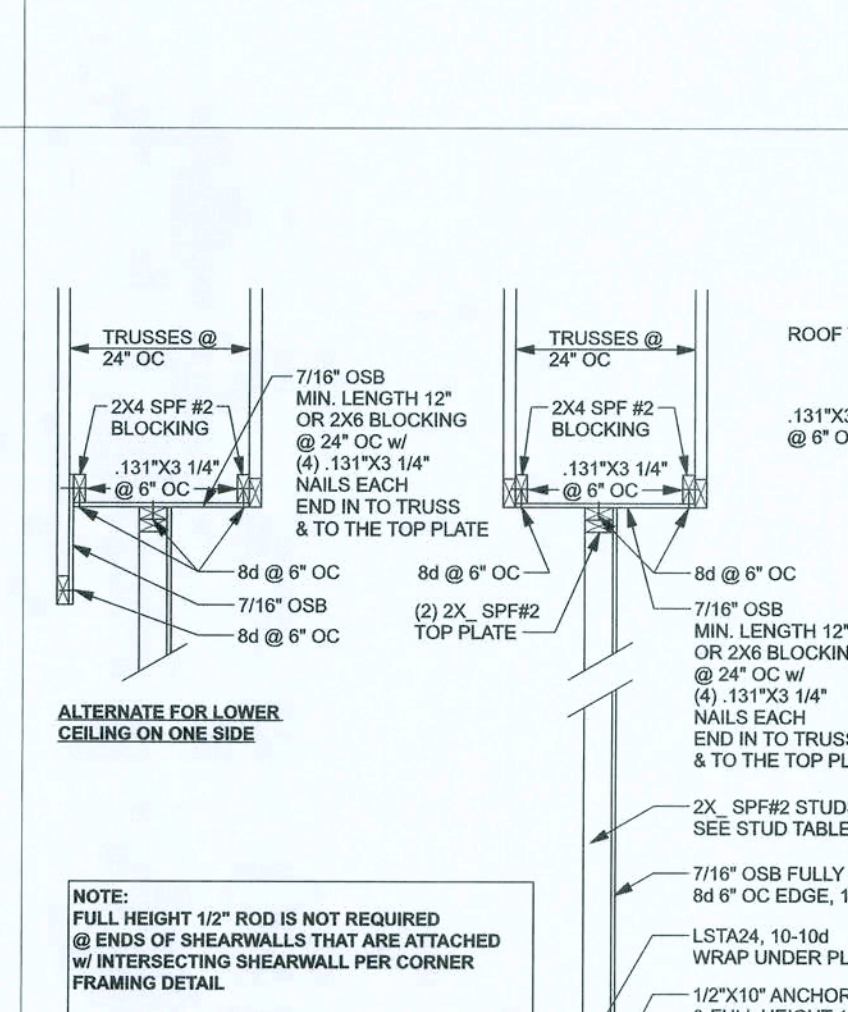
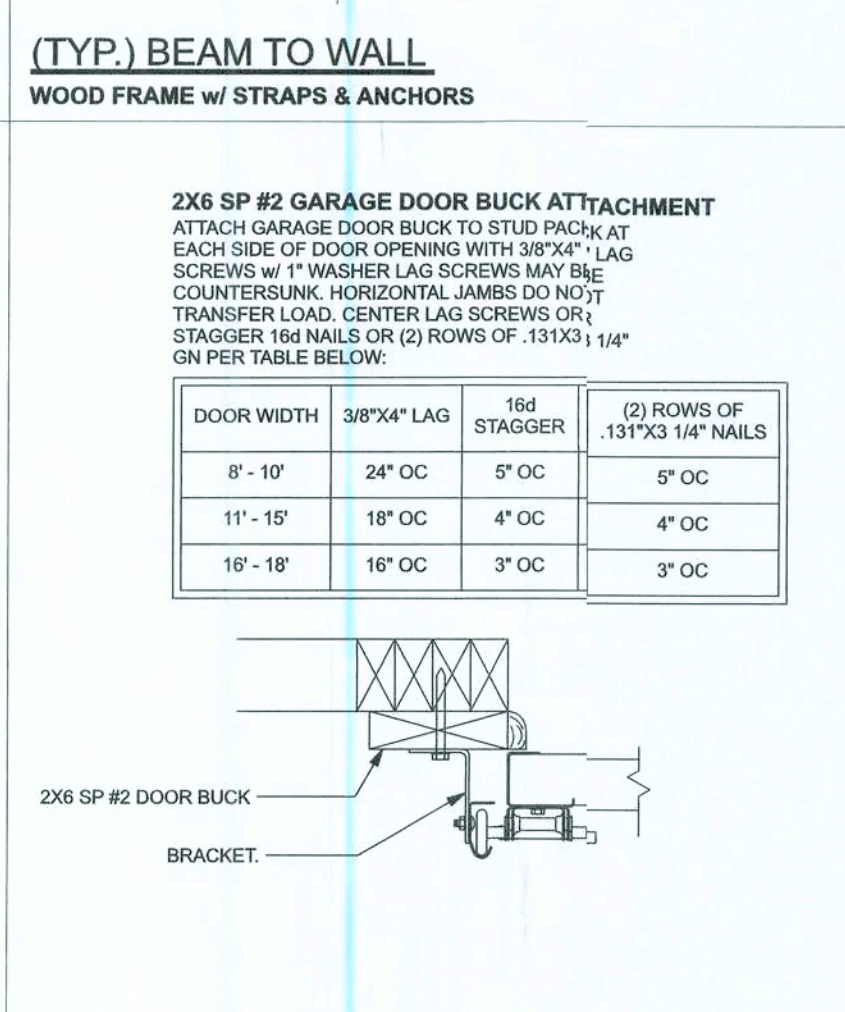
BEAM TO BEAR ON (2) 2X SPF#2 JACKS

OPTION: 1 (BUCKET)

OPTION: 2 (POCKETED)

NOTE: SHEATHING MUST BE NAILED TO TOP PLATES w/ 8d 3" OC (NAILING MAY BE STAGGERED) 4" SHEATHING NAILED TO HEADER w/ (2) ROWS OF 8d @ 6" OC

BEAM POCKETED BENEATH TOP PLATE (DROPPED BEAM)



# TYPICAL HEADER STRAPING DETAIL

## ONE STORY WOOD FRAME w/ STRAPS & ANCHORS

NAILING @ TOP PLATE TO STUD

END NAIL OR TOE NAIL

13"X3" 14" NAILS

(3) FOR 2X4

(4) FOR 2X6

(5) FOR 2X8

(6) FOR 2X10

SPH @ CHANGE INSTALLED

NAILING @ SILL PLATE TO STUD

END NAIL OR TOE NAIL

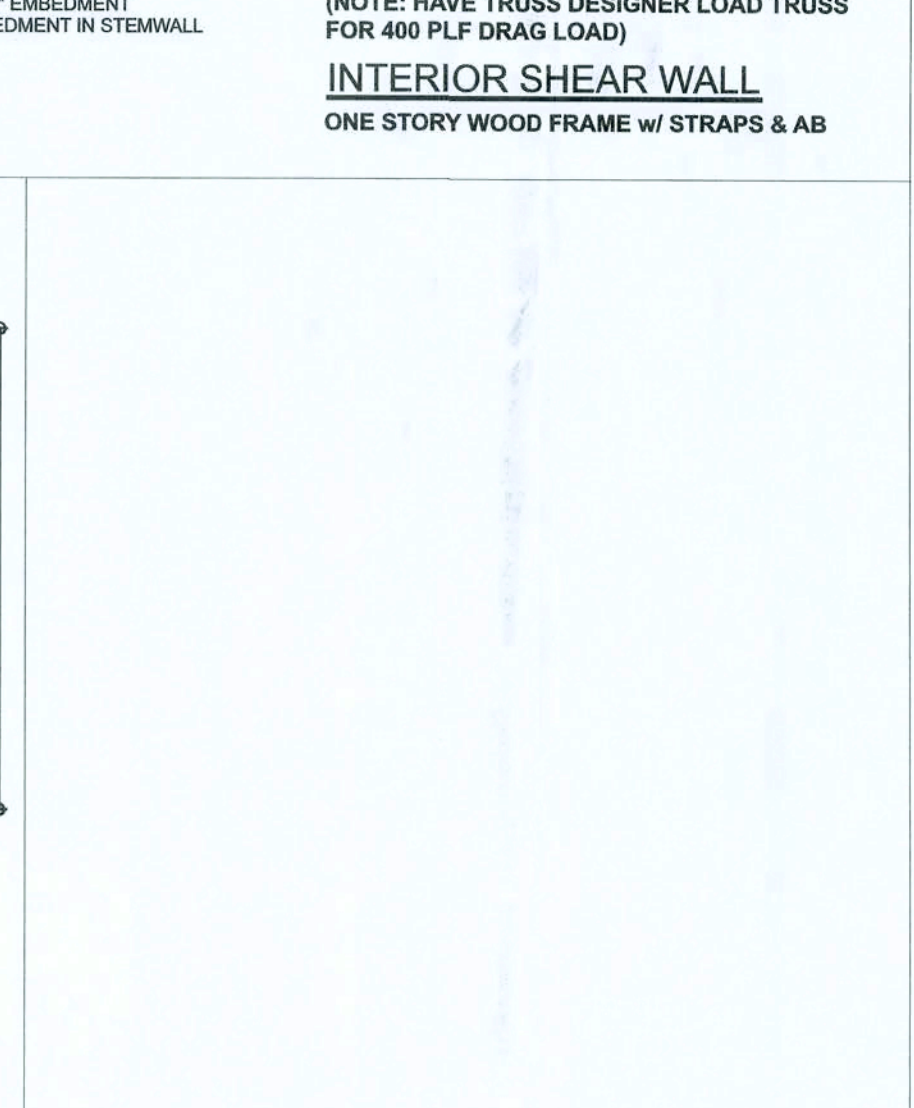
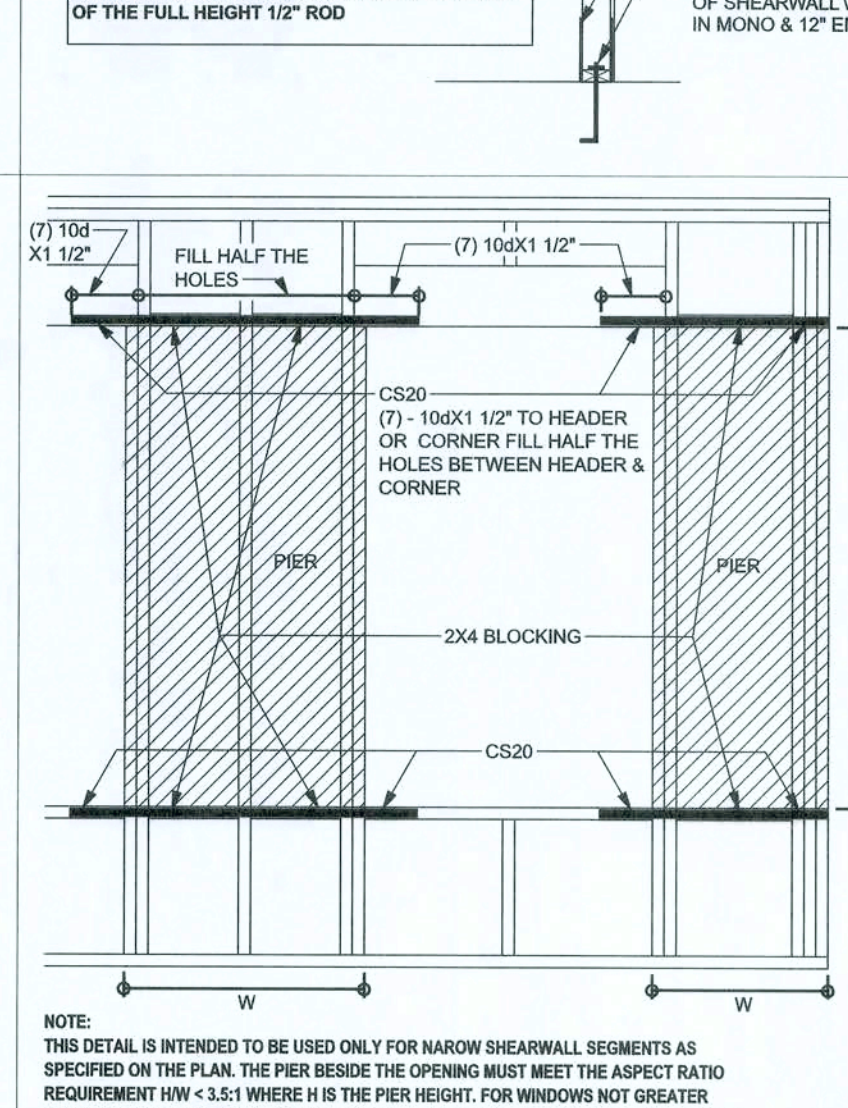
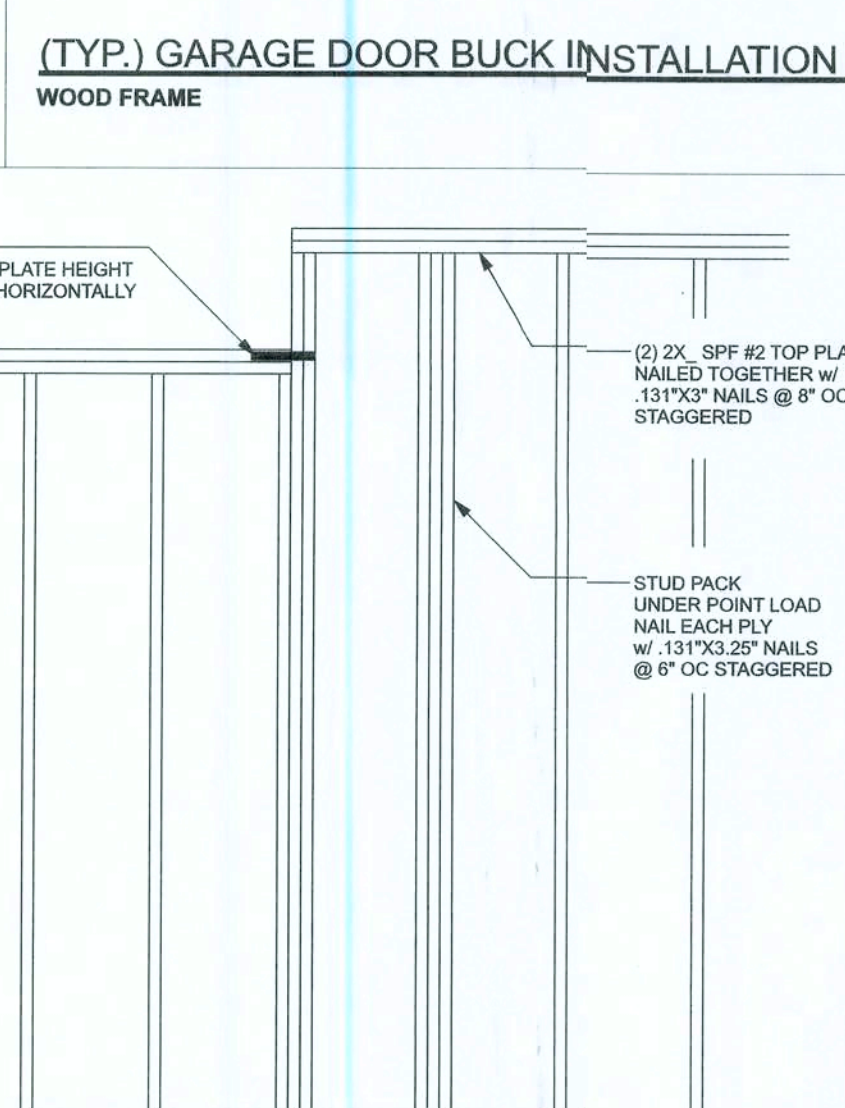
13"X3" 14" NAILS

(3) FOR 2X4

(4) FOR 2X6

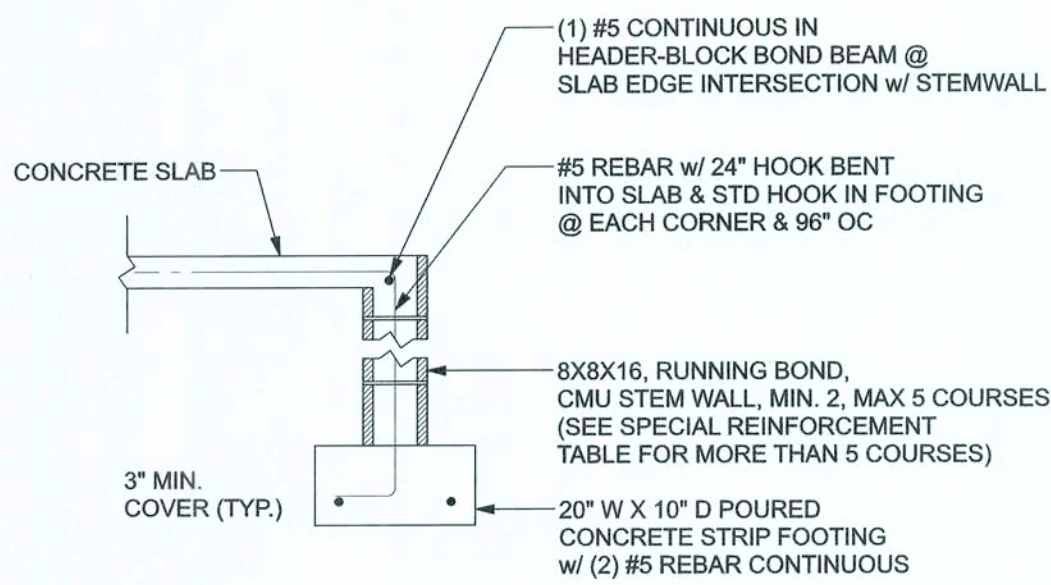
(5) FOR 2X8

(6) FOR 2X10

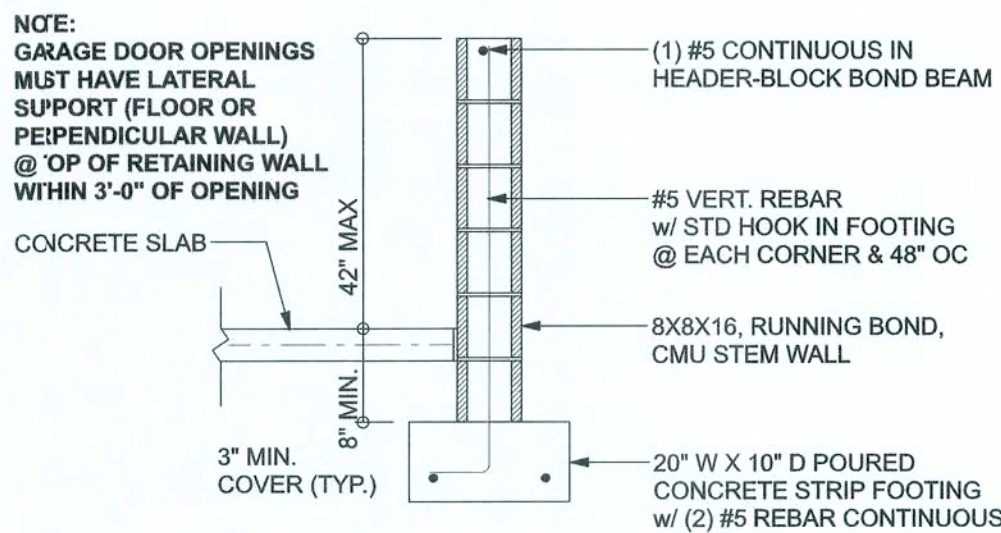


DESIGN CRITERIA & LOADS:		
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)	
CODE FOR DESIGN LOADS	ASCE 7-10	
<b>WINDLOADS</b>		
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	II	
RISK CATEGORY	ENCLOSED	
ENCLOSURE CLASSIFICATION	0.18	
INTERNAL PRESSURE COEFFICIENT	7-45 DEGREES	
ROOF ANGLE	30 FT	
MEAN ROOF HEIGHT	SEE TABLE	
<b>C&amp;D DESIGN PRESSURES</b>		
<b>FLOOR LOADING</b>		
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
<b>ROOF LOADING</b>		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
<b>SOIL BEARING CAPACITY</b>		
1500 PSF		
<b>FLOOD ZONE</b>	THIS BUILDING IS NOT IN THE FLOOD ZONE	

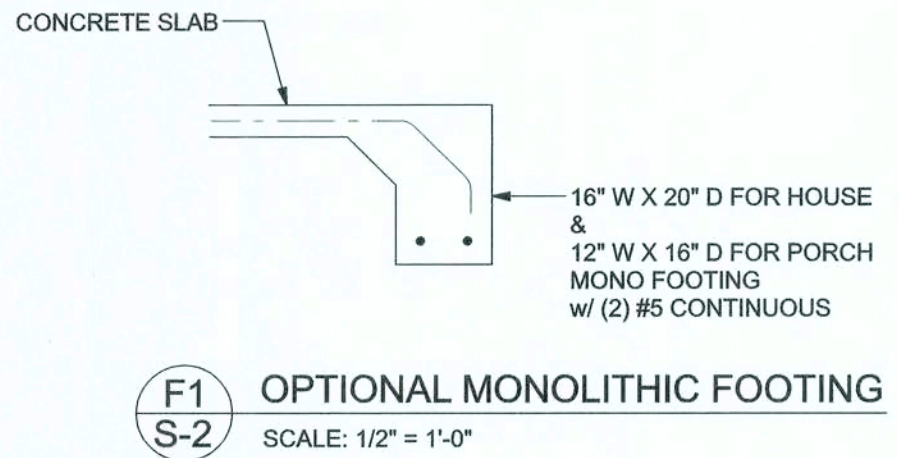




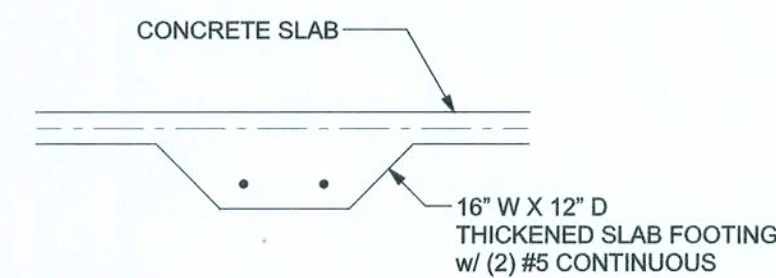
**F1 S-2** STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



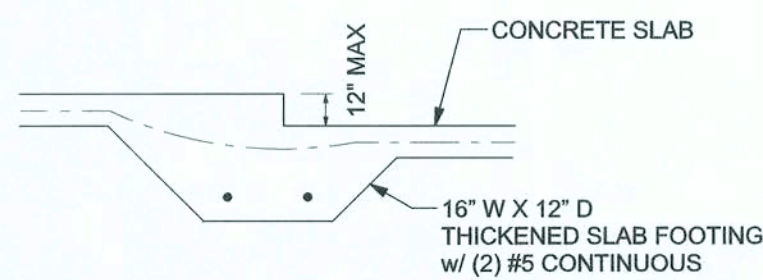
**F4 S-2** STEM WALL CURB FOOTING  
SCALE: 1/2" = 1'-0"



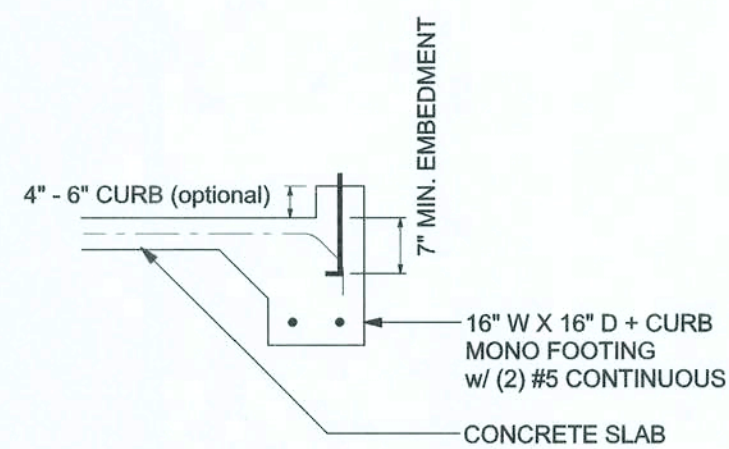
**F1 S-2** OPTIONAL MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



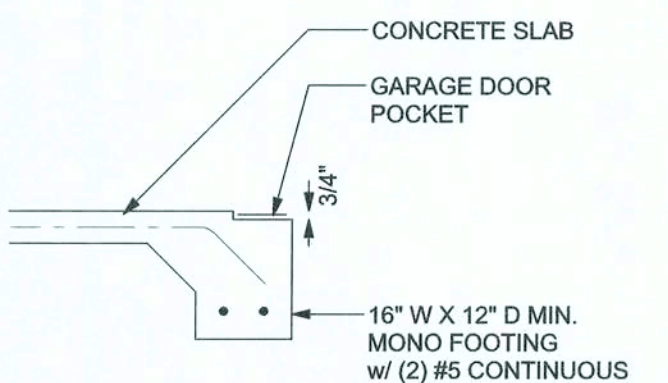
**F2 S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



**F3 S-2** INTERIOR BEARING STEP FOOTING  
SCALE: 1/2" = 1'-0"



**F4 S-2** OPTIONAL MONOLITHIC CURB FOOTING  
SCALE: 1/2" = 1'-0"



**F5 S-2** GARAGE DOOR POCKET FOOTING  
SCALE: 1/2" = 1'-0"

**TALL STEM WALL TABLE:**  
The table assumes 61 ksi reinforcing bars with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Drowall ladder reinforcement at 16" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEM WALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEM WALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEM WALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.1	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

**MASONRY NOTE:**  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

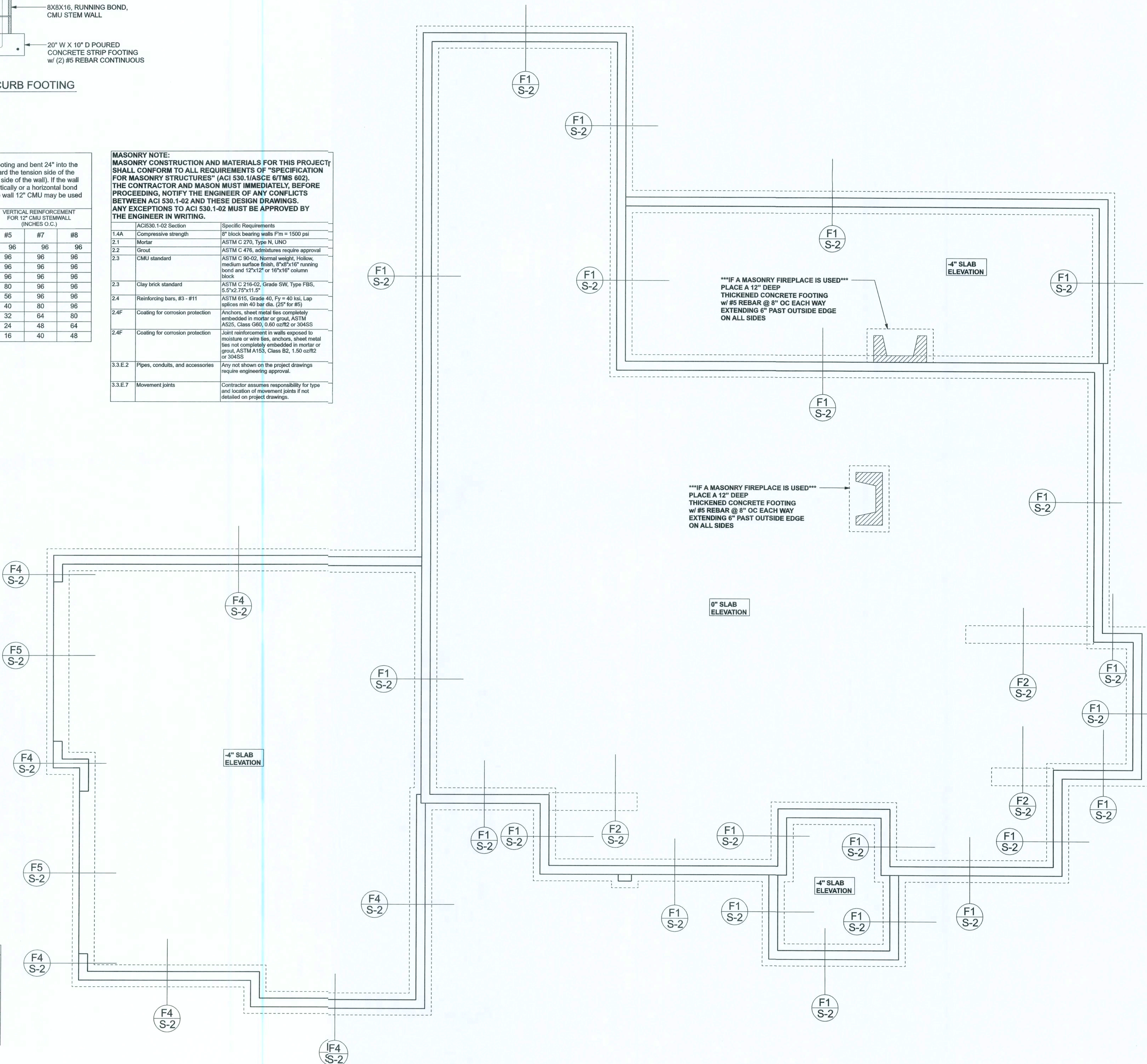
ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls P'm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, S-S-X-2.75"x11.8"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

#### FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

**FOUNDATION NOTES**

1. DIMENSIONS IN FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
2. CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
3. THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6X6-1 A14 WELDED WIRE MESH PLACED ON CHAIRS @ 1' 10" DEPTH OR FIBER MESH CONCRETE, 1/2" POLY VAPOR BARRIER W/ 6" LAPS SEALED W/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL



Aaron Simque Homes

Columbia Res.

PROJECT ADDRESS:  
Columbia County, FL

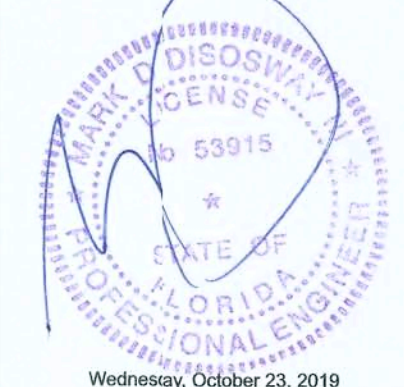
**DIMENSIONS:**  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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**CERTIFICATION:** I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

**LIMITATION:** The design is valid for one building, at specified location.

MARK DISOSWAY, P.E. 53915



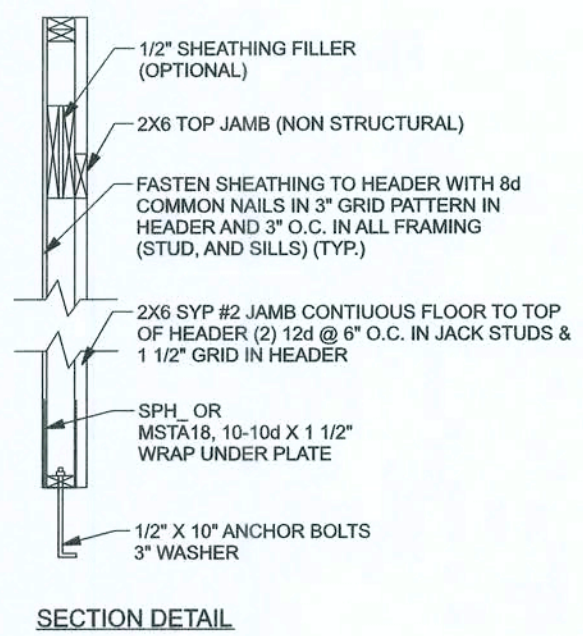
Mark Disosway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

**JOB NUMBER:**  
91095

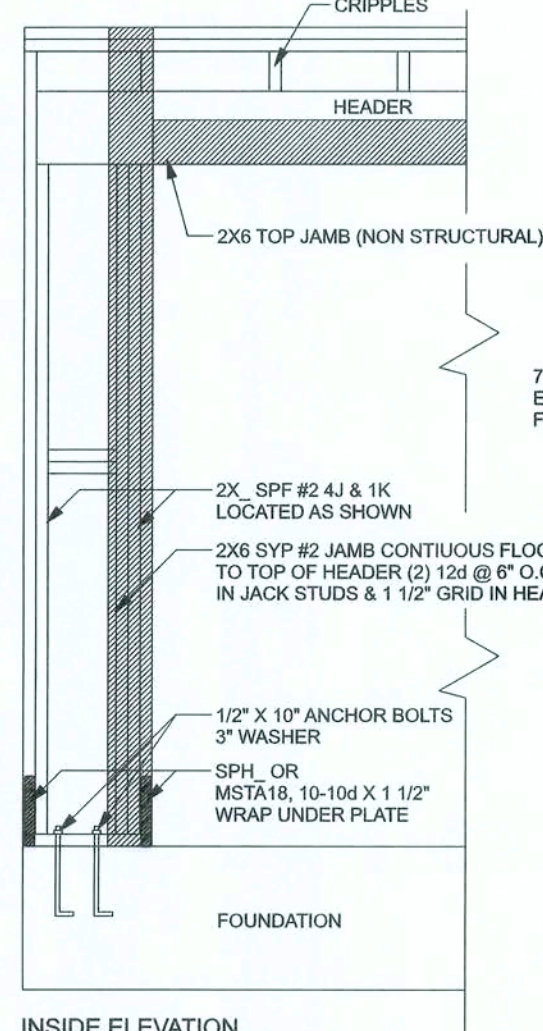
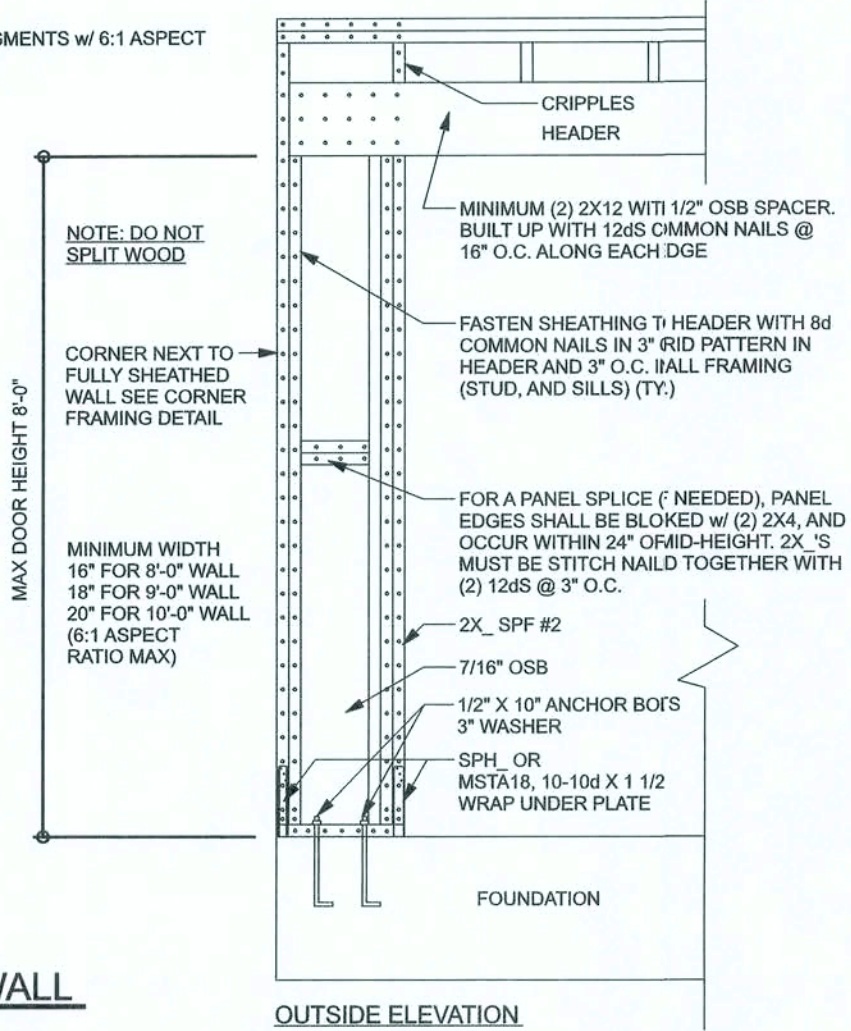
**S-2**  
OF 3 SHEETS



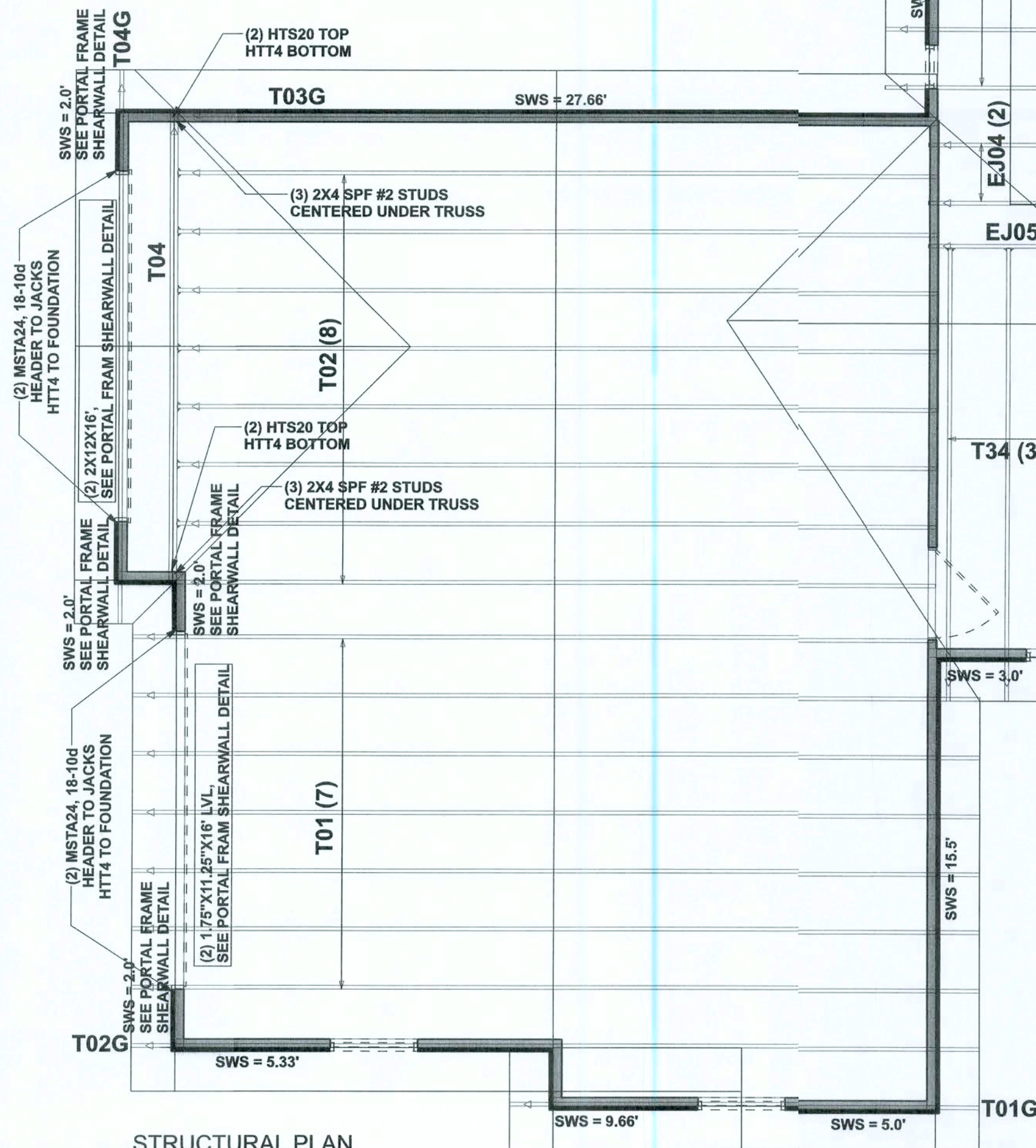
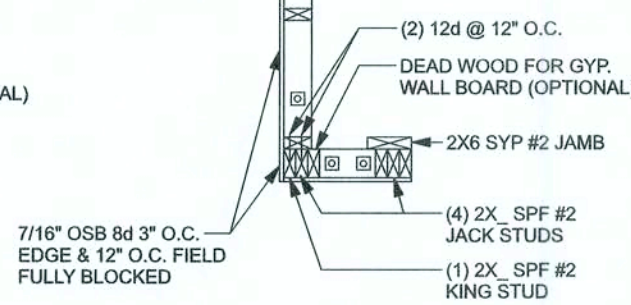
NOTE:  
THIS PORTAL FRAME IS DESIGNED FOR NARROW WALL SEGMENTS w/ 6:1 ASPECT  
RATIO MAX SUCH AS BESIDE FRONT LOAD GARAGE DOORS



(TYP.) PORTAL FRAME SHEARWALL  
ONE STORY WOOD FRAME



CORNER FRAMING DETAIL



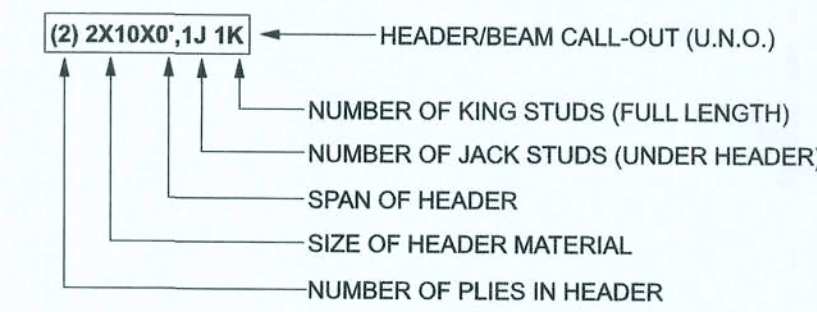
STRUCTURAL PLAN  
SCALE: 1/4" = 1'-0"

#### STRUCTURAL PLAN NOTES

- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- SN-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

- SN-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS18-01, BCS18-02, BCS18-03, BCS18-04, BCS18-05, BCS18-06, BCS18-07, BCS18-08, BCS18-09, BCS18-10, BCS18-11, BCS18-12, & BCS18-13 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

#### HEADER LEGEND



ACTUAL vs REQUIRED SHEARWALL		
	TRANSVERSE	LONGITUDINAL
ACTUAL	26436 LBF	26872 LBF
REQUIRED	18205 LBF	16655 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2097852

PRE-ENGINEERED ROOF TRUSSES  
(DESIGNED BY OTHERS)  
ATTACH TRUSSES  
TO WALL / BEAM  
w/ (1) SIMPSON SDWC15600  
FOR UP TO 485 LB UPLIFT  
OR  
(2) SIMPSON SDWC15600  
FOR UP TO 850 LB UPLIFT

SEE OPENING  
FORCE TRANSFER  
DETAIL ON SHEET S-1

Aaron Simque Homes

Colunga Res.

PROJECT ADDRESS:  
Colunga County, FL

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan and that the applicable portions of the plan relating to wind engineering comply with the 2017 Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915



Wednesday, October 23, 2019

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Suite 103  
Lake City, Florida 32025  
386754.5419  
disowaydesign@gmail.com

JOB NUMBER:  
191095

S-3  
OF 3 SHEETS