

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS. TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOLINDATION: CONFIRM THAT THE FOLINDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI. WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4. FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185: LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 \* DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY

	ACI530.1-02 Section	Specific Requirements				
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi				
2.1	Mortar	ASTM C 270, Type N, UNO				
2.2	Grout	ASTM C 476, admixtures require approval				
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block				
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"				
2.4	Reinforcing bars, #3 - #11 ASTM 615, Grade 40, Fy = 40 ksi, I splices min 40 bar dia. (25" for #5)					
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS				
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS				
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.				
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.				

#### **BUILDER'S RESPONSIBILITY:** THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS. FOUNDATION BEARING CAPACITY, GRADE AND BACKELL HEIGHT WIND SPEED AND DEBRIS ZONE AND ELOOD ZONE PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY. VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS

DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

**ROOF SYSTEM DESIGN:** 

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN. PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE 1 REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED

	FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
WINDLOADS	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	С
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
<b>C&amp;C DESIGN PRESSURES</b>	SEE TABLE
FLOOR LOADING	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD

20 PSF LIVE LOAD

16 PSF LIVE LOAD

12 PSF LIVE LOAD

THIS BUILDING IS NOT IN THE FLOOD ZONE

+25.6(Vasd) -27.8(Vasd) +25.6(Vasd) -34.2(Vasd)

+42.6(Vult) -46.2(Vult) +42.6(Vult) -57(Vult)

+22.6(Vasd) -25.5(Vasd)

+21.7(Vasd) -24.1(Vasd)

END 4' FROM ALL

OUTSIDE CORNER

Thursday, August 26, 2021

DIMENSIONS:

Stated dimensions supercede scaled

dimensions. Refer all questions to

Mark Disosway, P.E. for resolution

Do not proceed without clarification

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CERTIFICATION: I hereby certify that I have

examined this plan, and that the applicable portions of the plan, relating to wind engineering

LIMITATION: This design is valid for one

MARK DISOSWAY P.E. 53915

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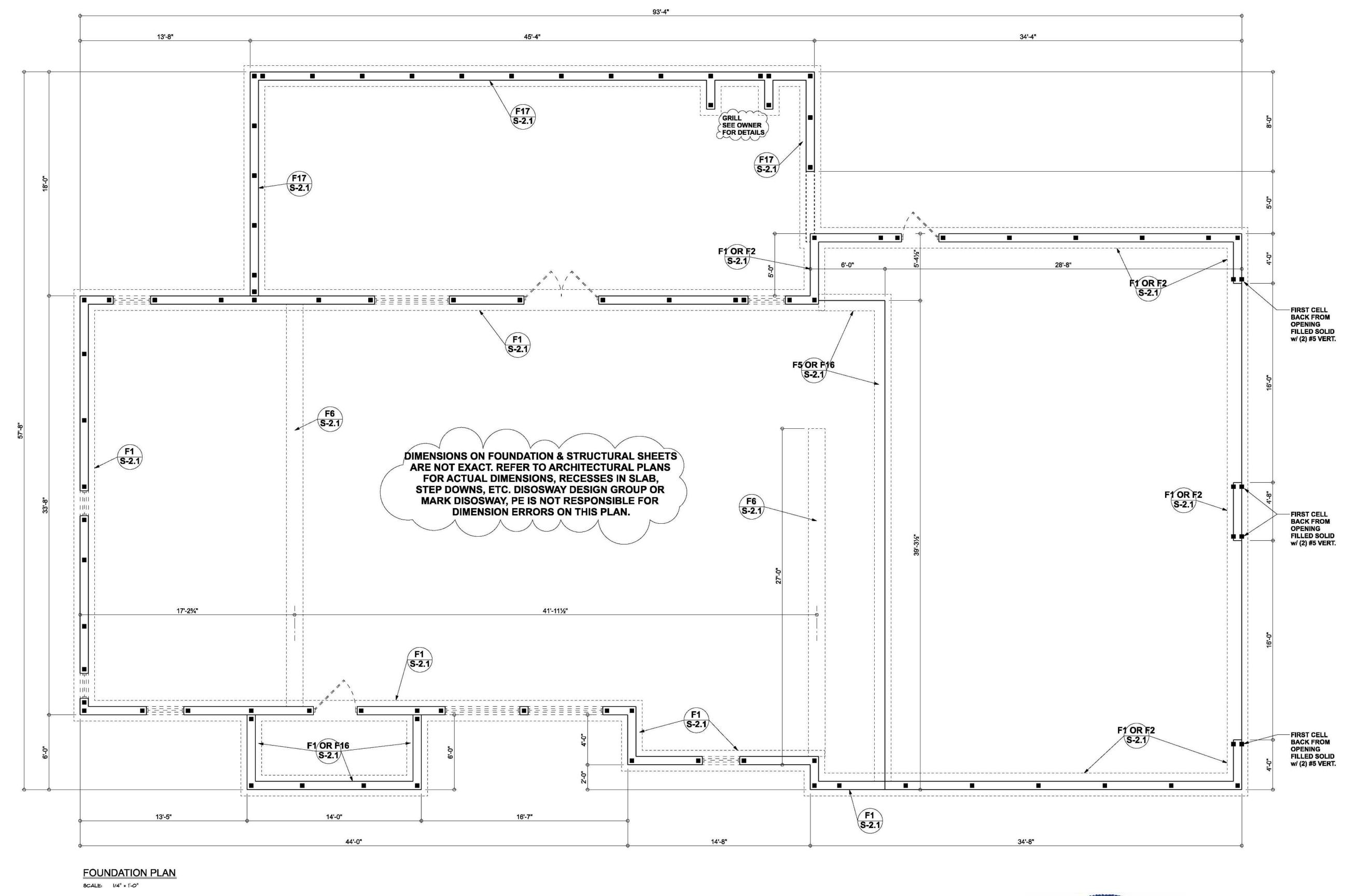
to the best of my knowledge.

building, at specified location.

Mark Disosway P.E. **163 SW Midtown Place** Suite 103 Lake City, Florida 32025 386.754.5419 disoswaydesign@gmail.com

> JOB NUMBER: 211179 **S-1**

> > OF 4 SHEETS



## FOUNDATION NOTES

FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.

CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING FN - 2 IN ALL AREAS BY REVIEWINGTHE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN

FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/
6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS
@ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL
POLY VAPOR BARRIER w/ 6" LAPS SEALED w/
POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL



Ronnie Shuman Res
PROJECT ADDRESS:
205 SW Madison Court
Lake City, FL 32024

DIMENSIONS: Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915
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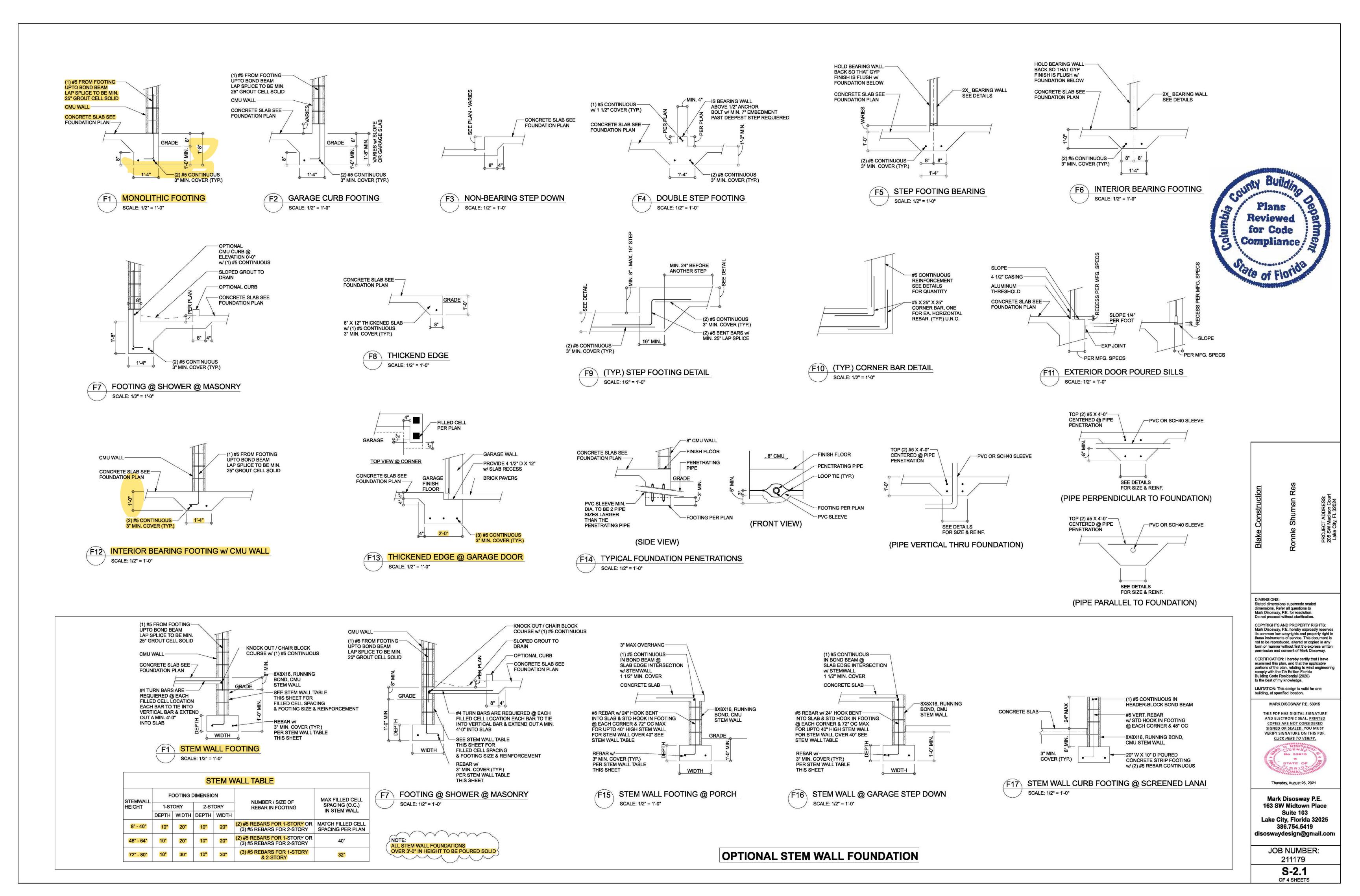
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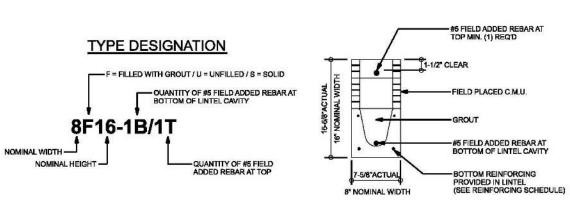
Thursday, August 26, 2021

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JOB NUMBER: 211179

S-2 OF 4 SHEETS





**MATERIALS**  fc 8" precast lintel = 3500 psi
 fc prestressed lintel = 6000 psi
 Grout per ASTM C476 fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump 4. Concrete Masonry Units (CMU) per ASTM C90 w/minimum net area compressive strength = 1900 psi 5. Rebar per ASTM A615 grade 60

 Prestressing strand per ASTM A415 grade 270 low relaxation
 Mortar per ASTM C270 type M or S **GENERAL NOTES** 1. Provide full mortar bed and head joints. Shore filled lintels as required.
 Installation of lintel must comply with the architectural

and/or structural documents.
4. U-Lintels are manufactured with 5 1/2" long notches at the ends to accommodate vertical cell reinforcing and grouting.

5. All lintels meet or exceed L/360 deflection, except lintels 17"-4" and longer with a nominal height of 8" meet or exceed L/180 deflection.

6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the

bottom steel for mechanical anchorage.

8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units. Safe load rating based on rational design analysis per ACI 318 and ACI 530 10. Product Approvals: Miami-Dade County, Florida No. 03-0606.05

11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:

Applied vertical load Safe vertical load Safe vertical load Safe horizontal load ≤1.0 13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right: SAFE LOAD TABLE NOTES 1. All values based on minimum 4 inch

nominal bearing.
Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 6 1/2 inches. 2. N.R. = Not Rated Safe loads are superimposed allowable loads
 Safe loads based on grade 40 or grade 60

5. One #7 rebar may be substituted for two #5 rebars in 8" lintels only 6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at d-away from face of support. For composite lintel heights not shown, use safe load from next lower height shown.

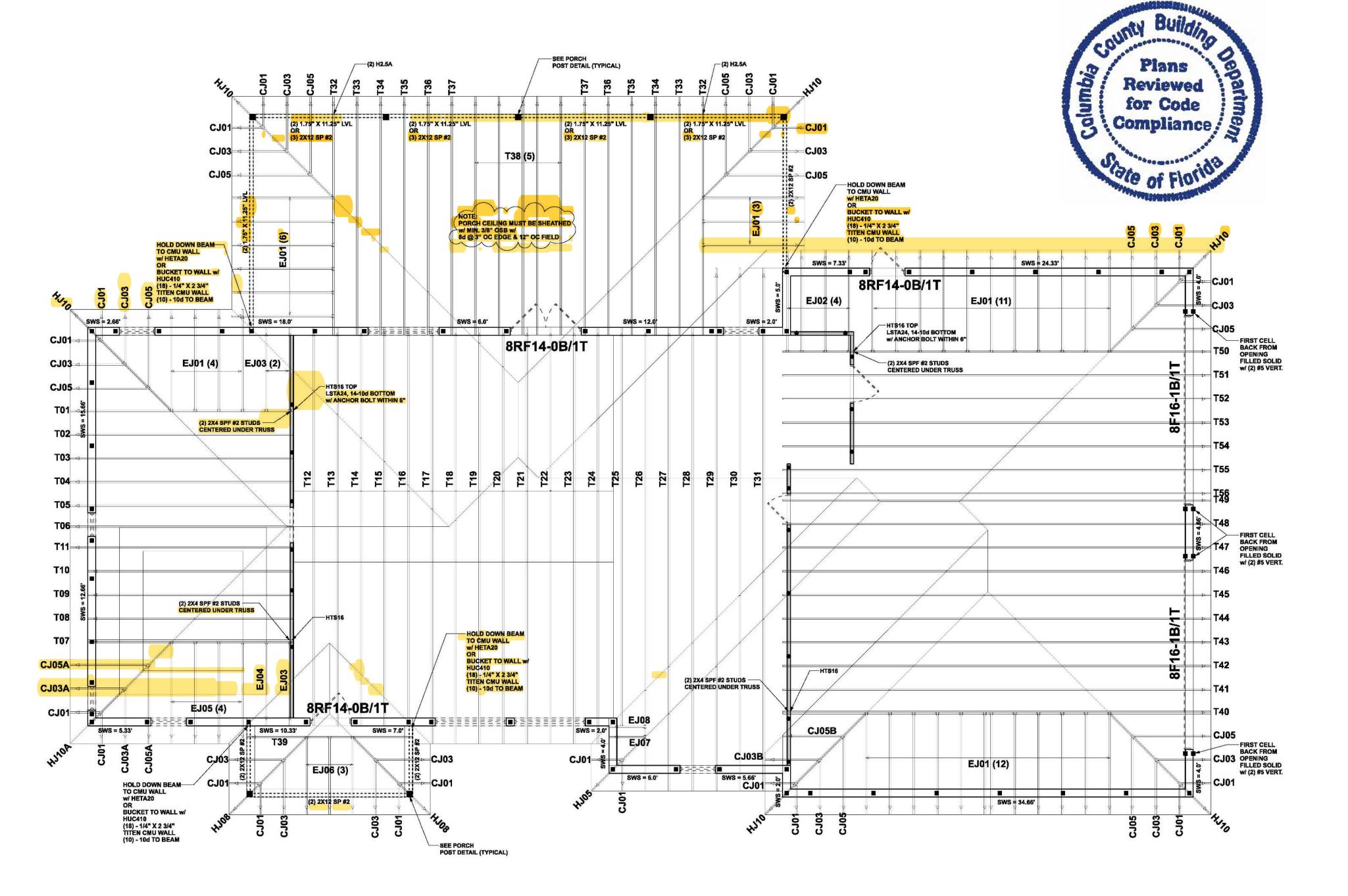
8. For lintels lengths not shown, use safe load from next longest length shown

9. All safe loads in units of pounds per linear 10. All safe loads based on simply supported span.

11. The number in the the parenthesis

5'-10" (70") PRECAST 810 1133 2342 4242 638 (10) 8060 (10) 8700 (1 indicates the percent reduction for grade 40 

	14	<b></b> !'	X	В	OTTOM OF	LINTELC	AVITY			
		7-5/8"ACTU	-φ	P	OTTOM RE ROVIDED SEE REINF	N LINTEL		E)		
100	A	FE GRAVITY LO	DADS F	OR 8" PI	RECAST	& PRES	TRESS	ED U-LIN	ITELS	
<	Çast	-Coxene		SAF	E LOAD	- POUN	DS PER	LINEAR	FOOT	
-		TYPE		8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0
LENGTH			8U8	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	- PCDO - VVCDO -
2'-10" (3-	(34")	") PRECAST	2231	3069	4605	6113	7547	8974	10394	1180
- 10	<b>(</b> )			3069	4605 3719	6113	7547	8974	10394	1180
3'-6"	(42")	PRECAST	2231	3069	4605	5163 6113	6607 7547	8054 8974	9502 10394	1095
4'-0"		PRECAST	1966	2561	2751	3820	4890	5961	7034	8107
	(48")			2693	4605	6113	7547	8974	10394	1180
4'-6"	(54")	PRECAST PRECAST PRECAST	1500	1969	2110	2931	3753	4576	5400	6224
	(0.)		1599	2189	4375	6113	7547 (7	-	10294	1180
5'-4"	(64")		1217	1349	1438	1999	2560	3123	3686	4249
5000			315000	1663	3090 1173	5365 1631	7547 <sub>(36</sub>	7342(19	3009 3009	3470
5'-10"	(70")		1062	1451	2622	4360	7168 (45		100000000	100000
	1674 1476		y/outersec	1238	2177	3480	3031	3707	4383	506
6'-6"	(78")		908	1238	2177	3480	5381	8360	10394(37	
	2002	C-6200052	100	1011	1729	2632	2205	2698	3191	3689
7'-6"	(90")	PRECAST	743	1011	1729	2661	3898	5681	8467(44	6472
9'-4"	(112")	PRECAST		699	1160	1625	2564	3486	2818	3302
~ ~	1.12.)	PRECASI	554	752	1245	1843	2564	3486	4705(37	6390
10'-6"	(126")	PRECAST PRECAST	475 362	535	890	1247	2093	2777	2163	2536
.0-0	( )			643	1052	1533	2093	2781	3643 (38	1000000
11'-4"	(136")			582	945	1366	1846	2423	3127	4006
337 54	Hard Clark	00 15 400 15 40 TO	0.848860	582 540	945 873	1366 1254	1846 1684	2423 2193	3127 2805	3552
12'-0"	(144")	PRECAST	337	540	673	1254	1684	2193	2805	3552
	88 99		296	471	755	1075	1428	1838	2316	2883
3'-4"	(160")	PRECAST		471	755	1075	1428	1838	2316	2883
naraa	672233	PRECAST	279	424	706	1002	1326	1697	2127	2630
14'-0"	(168")			442	706	1002	1326	1697	2127	2630
14'-8"	(176")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
14-0	(170)			458	783	1370	1902	2245	2517	2712
15'-4"	(184")	PRESTRESSED	N.R.	NR	NR	NR	NR	NR	NR	NR
22-11-30	•			412	710 ND	1250	1733 ND	2058	2320	251
17'4"	(208")	PRESTRESSED PRESTRESSED PRESTRESSED	N.R. N.R. N.R.	300	NR 536	NR	1326	1609	1849	2047
	02 88			NR.	NR	950 NR	NR	NR	NR	NR
19'-4"	(232")			235	418	750	1037	1282	1515	1716
241.48	(250)1			NR	NR	NR	NR	NR	NR	NR
21'-4"	(256")			180	340	598	845	1114	1359	1468
22'-0"	(264")			NR	NR	NR	NR	NR	NR	NR
	85			165	315	550	784	1047	1285	1399
24'-0"	(288")	PRESTRESSED	N.R.	NR 120	NR	NR 450	NR	NR	NR 1002	NR 1920
_	AFE	DAVITY LOAD	C FOR	129	250	450	854 CEOC D	00 B LI	1092	1222
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4'-4"	i i i	PRECAST	1635	8RF6-1B 1749	8RF10-1B 3355	8RF14-1B 3280	8RF18-1B 4349	8RF22-1B 5421	8RF26-1B 6493	8RF30- 7567
	(52")			1891	3699	5206	6639	8060	9479	10893
	16_300			1596	3063	2992	3988	4946	5924	6904
4'-6"	(54")	PRECAST	1494	1756	3899	5206	6639	8060	9479	10893
5'-8"	/ep=\	PRECAST	000	920	1770	1716	2277	2839	3402	3966
,-0	(68")	I KECASI	866	1107	2404	4507	6200	poen	7047	0244



# STRUCTURAL PLAN SCALE: 3/16" = 1'-0"

#### STRUCTURAL PLAN NOTES

DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS

PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED

#### FRAME HEADER LEGEND HEADER/BEAM CALL-OUT (U.N.O.) -NUMBER OF KING STUDS (FULL LENGTH) --- NUMBER OF JACK STUDS (UNDER HEADER) -SPAN OF HEADER SIZE OF HEADER MATERIAL

-NUMBER OF PLIES IN HEADER

### THREADED ROD LEGEND

INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD

**ACTUAL vs REQUIRED SHEARWALL** TRANSVERSE LONGITUDUNAL 51.9' 143.3' REQUIRED 35.0'

UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS 1. USE HETA16 CMU TO TRUSS 2. USE H2.5A FRAME TO TRUSS 3. ALL LENTELS TO BE: 8F16-0B/1T

4. ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SP #2 5. ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE

**ENGINEERED TRUSSES** ATTACH PER TRUSS UPLIFT TO CMU WALLS: **HETA16 FOR UP TO 1350 LB UPLIFT** (2) HETA16 FOR UP TO 2035 LB UPLIFT MGT FOR UP TO 3965 LB UPLIFT TO FRAME WALLS: (1) H2.5A FOR UP TO 495 LB UPLIFT OR (2) H2.5A FOR UP TO 990 LB UPLIFT SEE CONNECTOR TABLE OR SIMPSON BOOK FOR ADDITIONAL OPTIONS

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LIMITATION: This design is valid for one building, at specified location.

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> JOB NUMBER: **S-3**

> > OF 4 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2902489