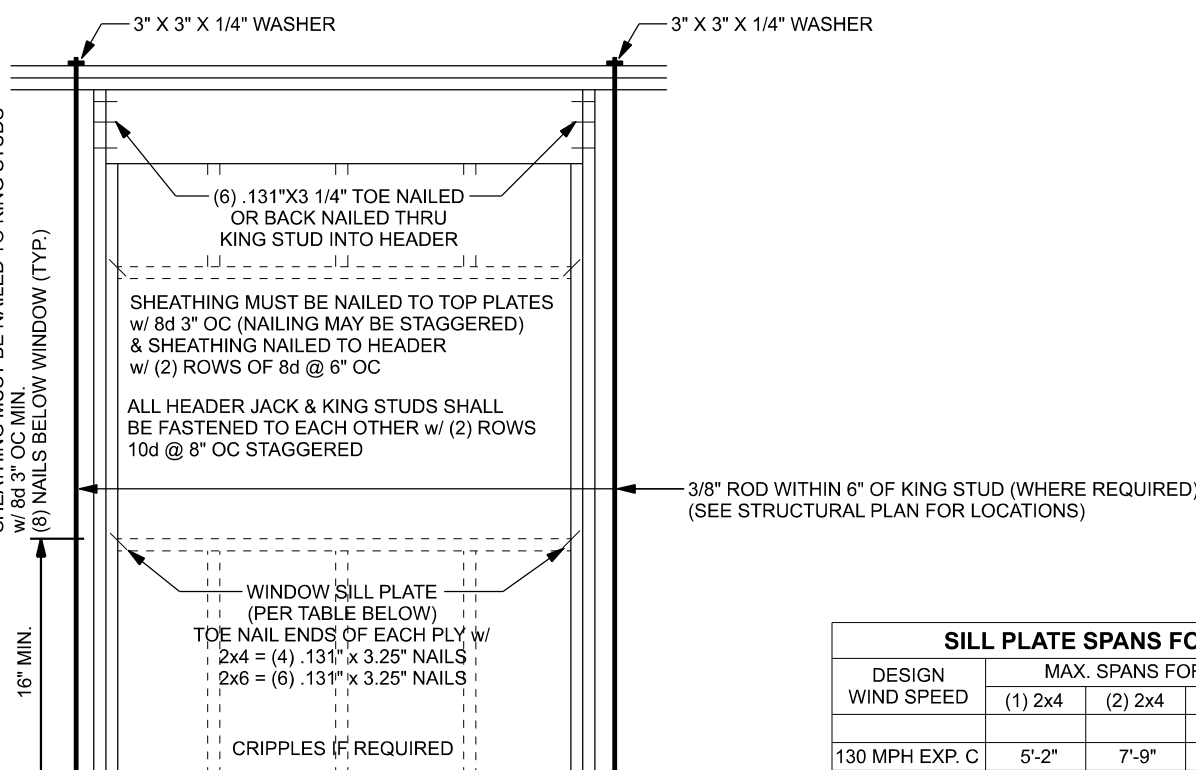


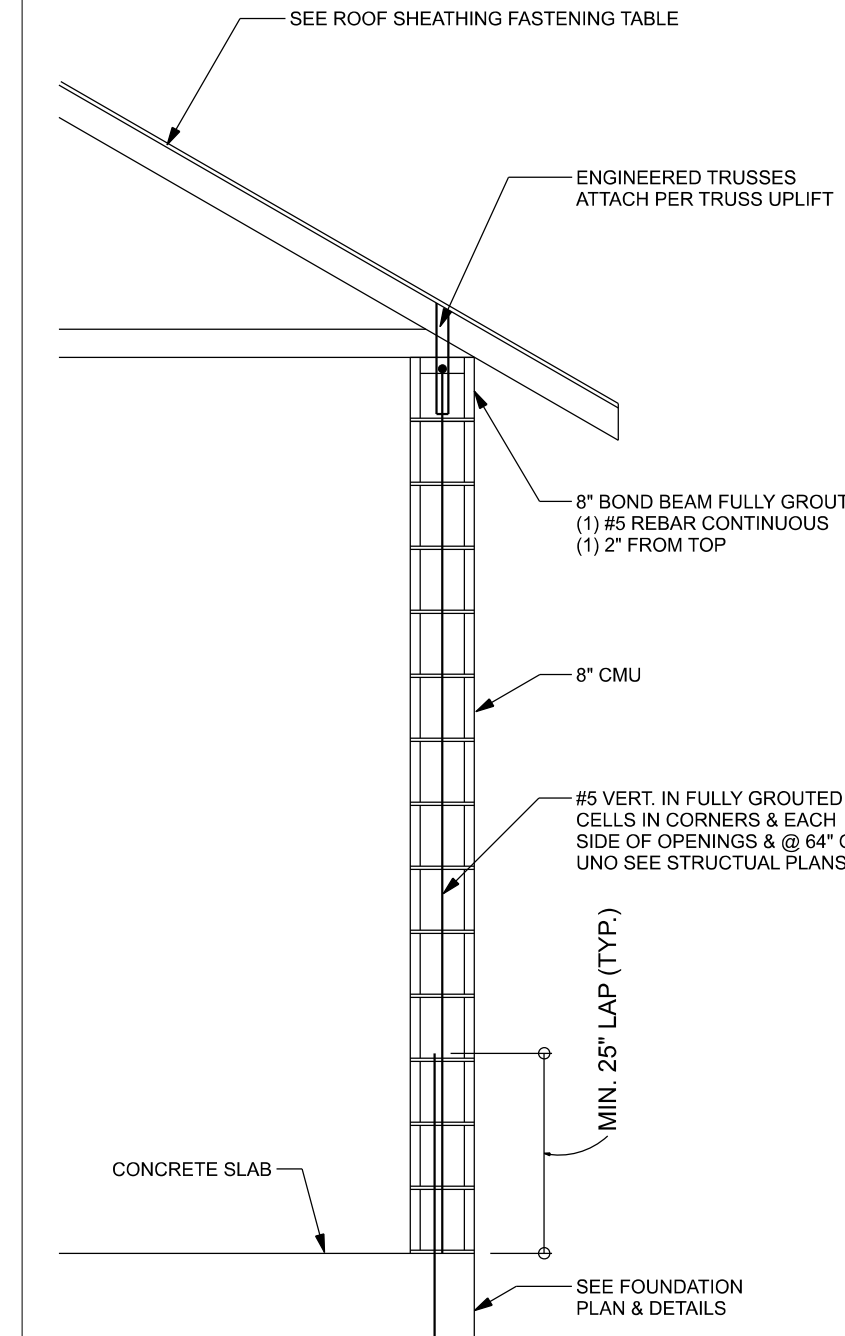
ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)					
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field	
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	12" oc	
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
130 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc	
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc	

Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

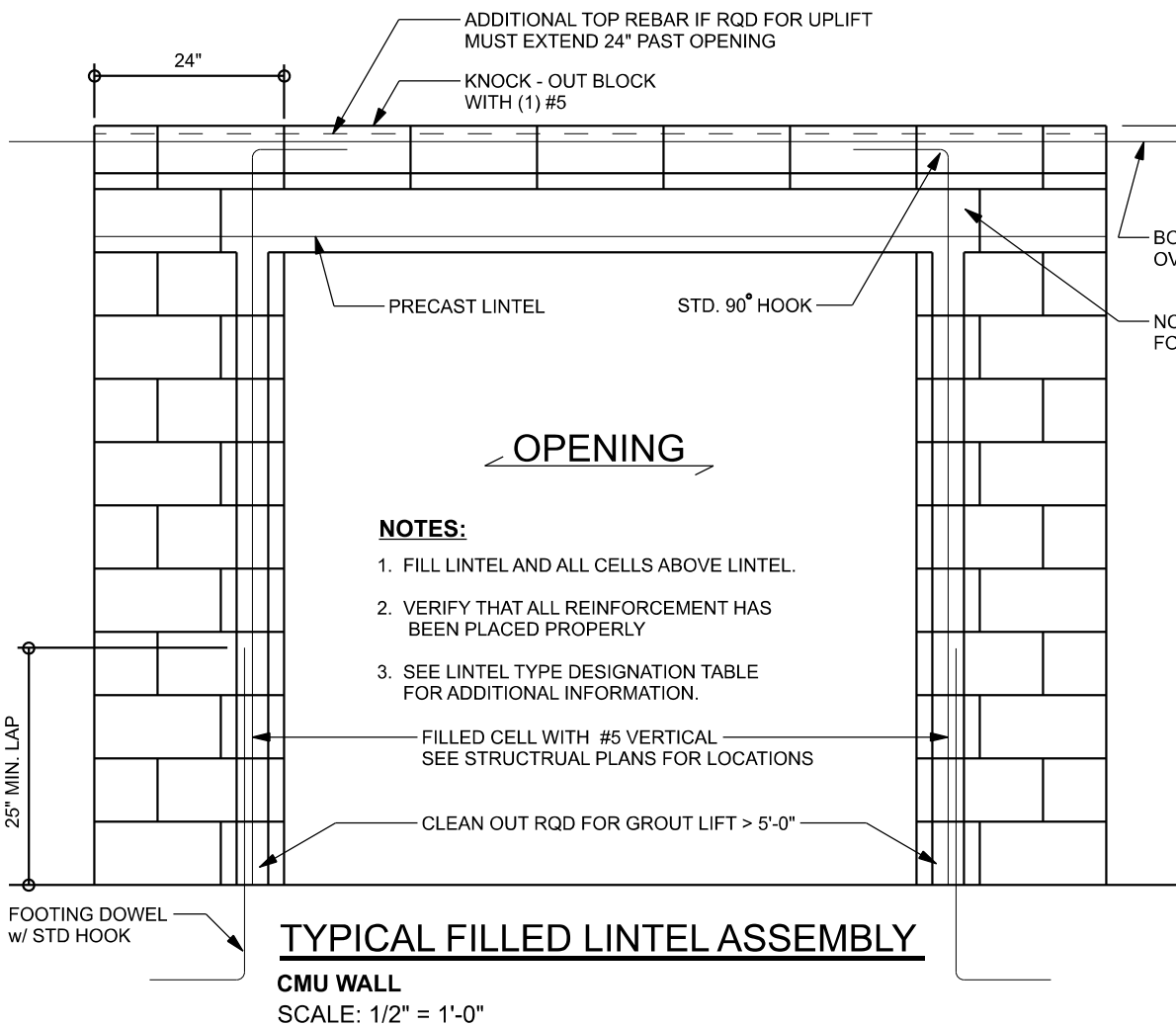


SILL PLATE SPANS FOR 10'-0" WALL HEIGHT				
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.2(9)		
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"

FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)



TYP. EXTERIOR WALL
ONE STORY CMU
SCALE: 1/2" = 1'-0"



- NOTES:
1. FILL LINTEL AND ALL CELLS ABOVE LINTEL. ADD PLATE AND 2x4 SPF#2 VERTICAL KICKER (8\"/>

2. VERIFY THAT ALL REINFORCEMENT HAS BEEN PLACED PROPERLY.

3. SEE LINTEL TYPE DESIGNATION TABLE FOR ADDITIONAL INFORMATION.

FILLED CELL WITH #5 VERTICAL. SEE STRUCTURAL PLANS FOR LOCATIONS.

CLEAN OUT ROD FOR GROUT LIFT > 5'-0"

FOOTING DOWEL w/ STD HOOK.

MIN. 25\"/>

SEE FOUNDATION PLAN & DETAILS.

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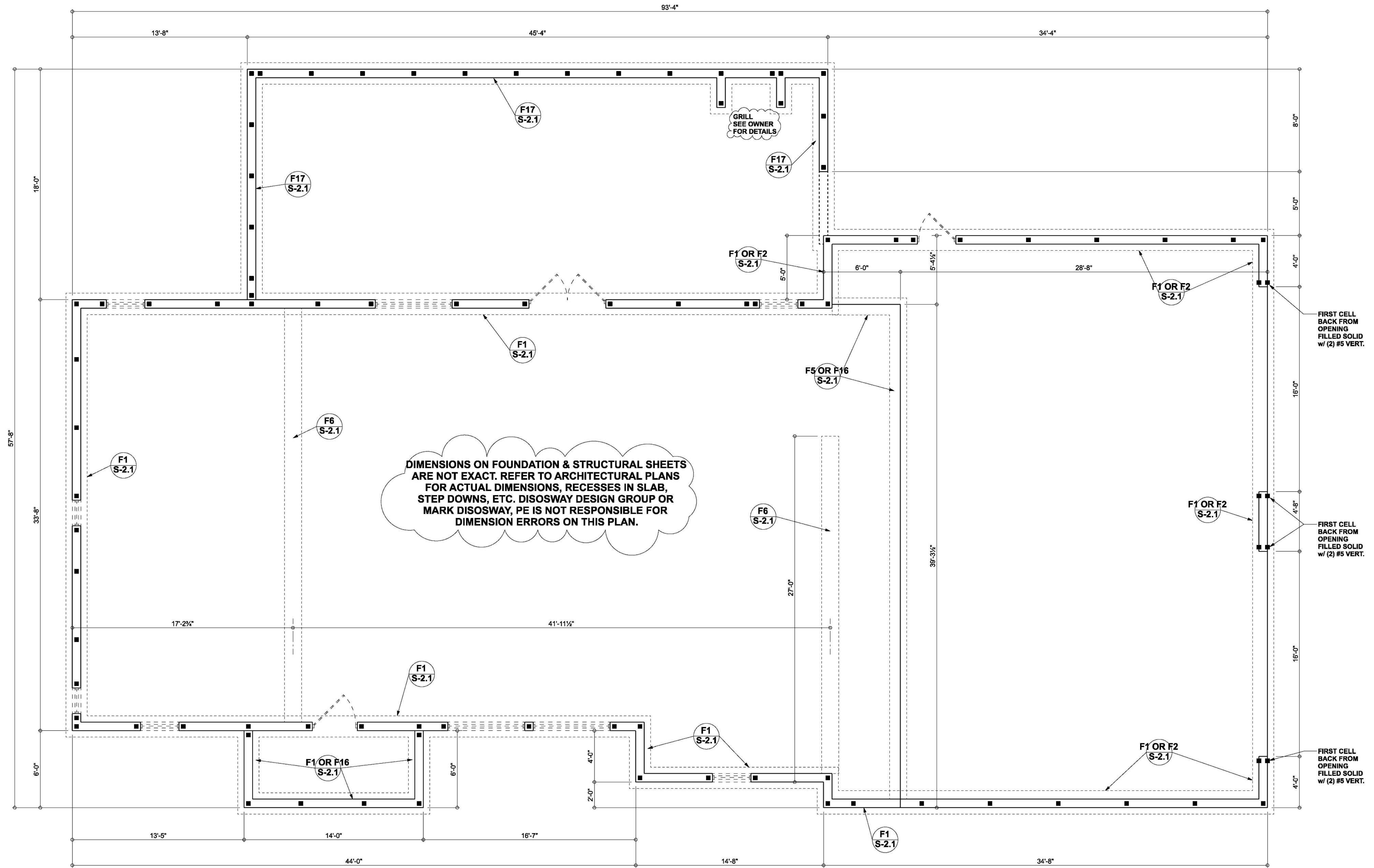
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FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL



Blake Construction

Ronnie Stuman Res

PROJECT ADDRESS:
205 SW Madison Court
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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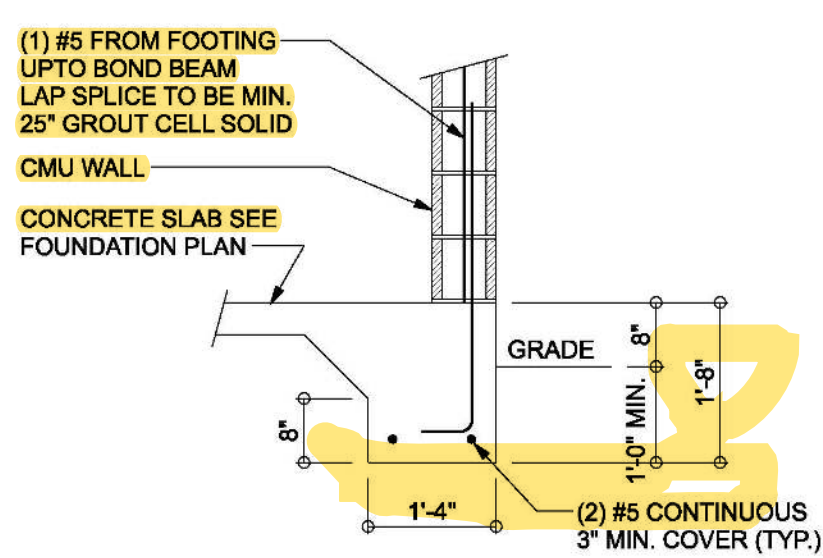


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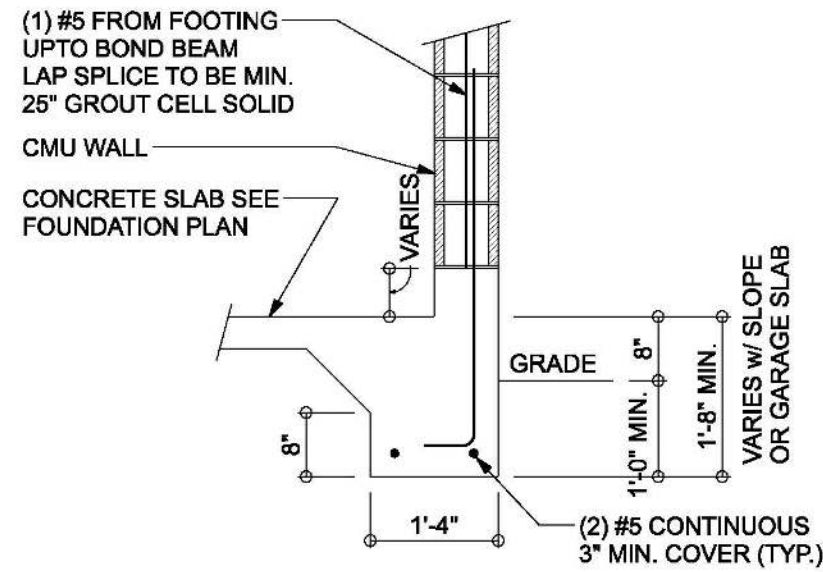
Mark Disosway P.E.
163 SW Midtown Place
Suite 103
Lake City, Florida 32025
386.754.5419
disoswaydesign@gmail.com

JOB NUMBER:
211179

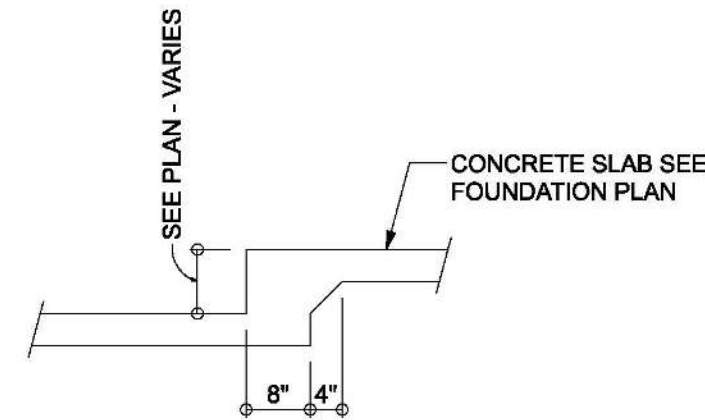
S-2
OF 4 SHEETS



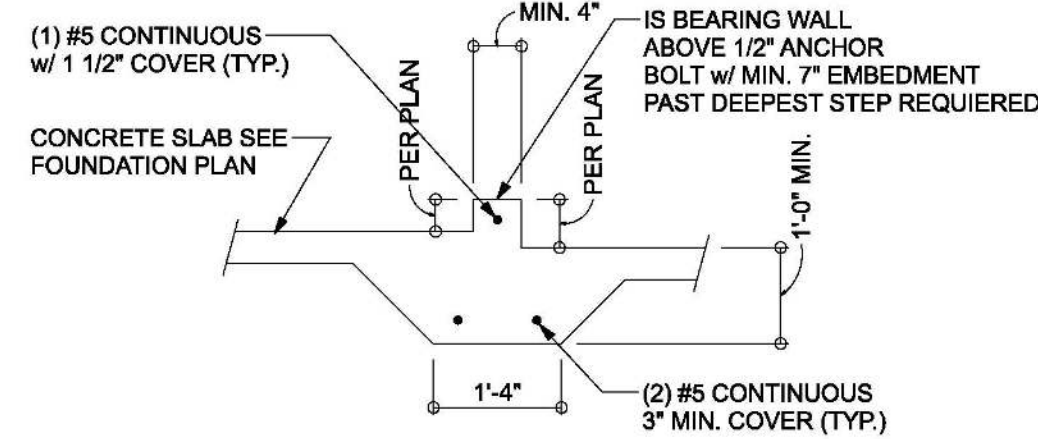
F1 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



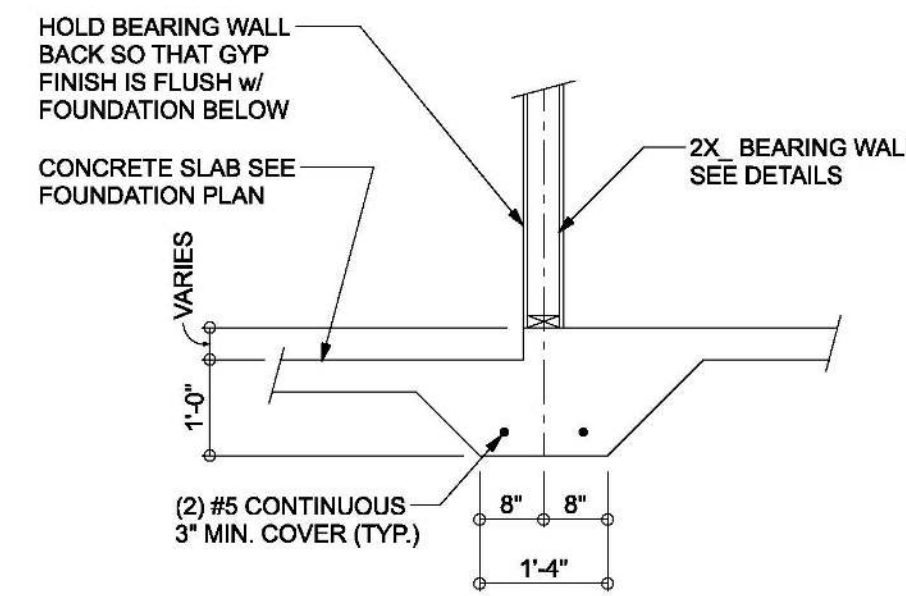
F2 GARAGE CURB FOOTING
SCALE: 1/2" = 1'-0"



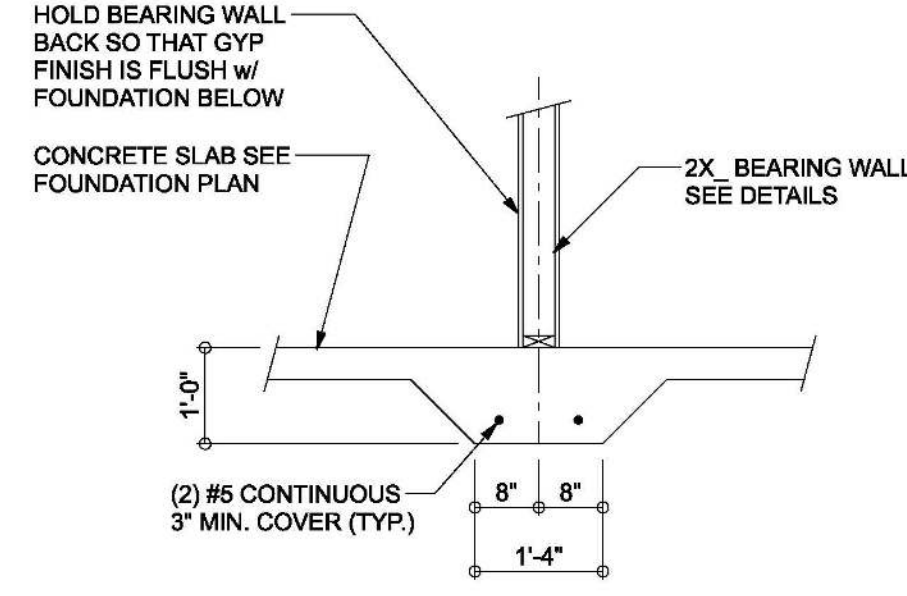
F3 NON-BEARING STEP DOWN
SCALE: 1/2" = 1'-0"



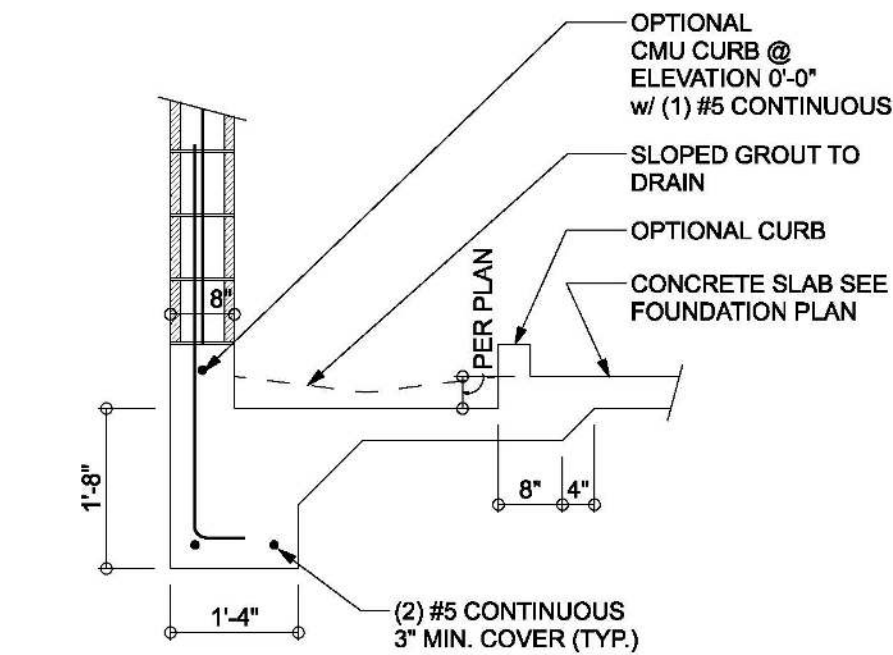
F4 DOUBLE STEP FOOTING
SCALE: 1/2" = 1'-0"



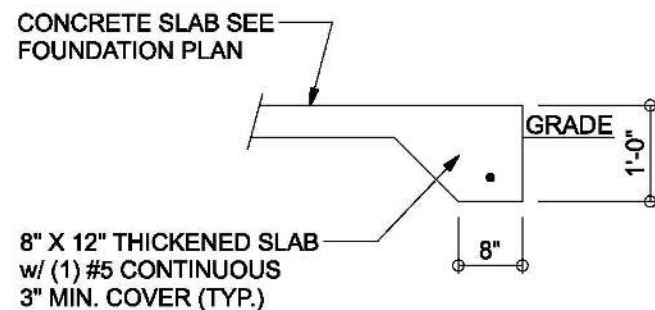
F5 STEP FOOTING BEARING
SCALE: 1/2" = 1'-0"



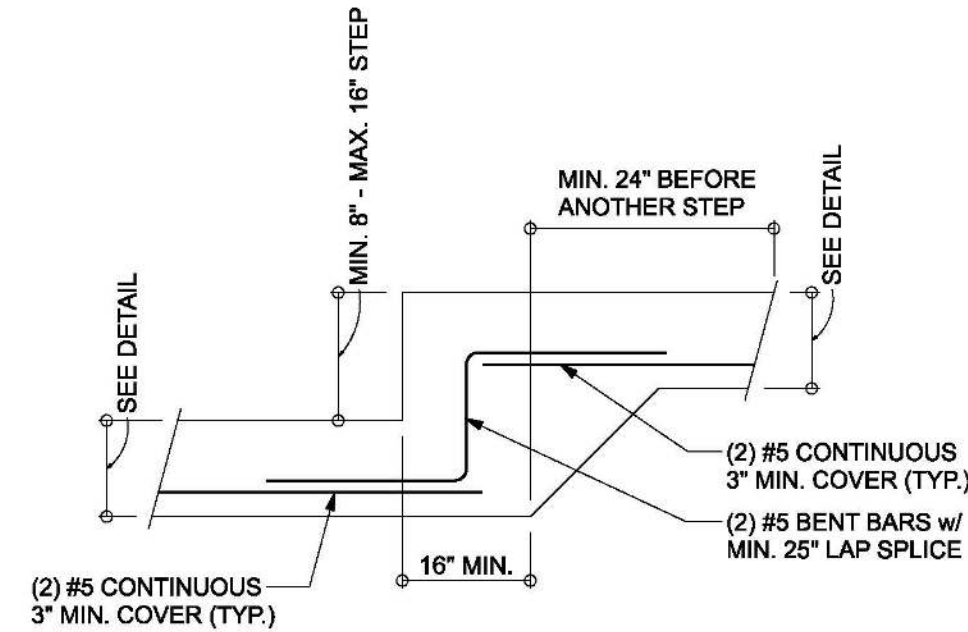
F6 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



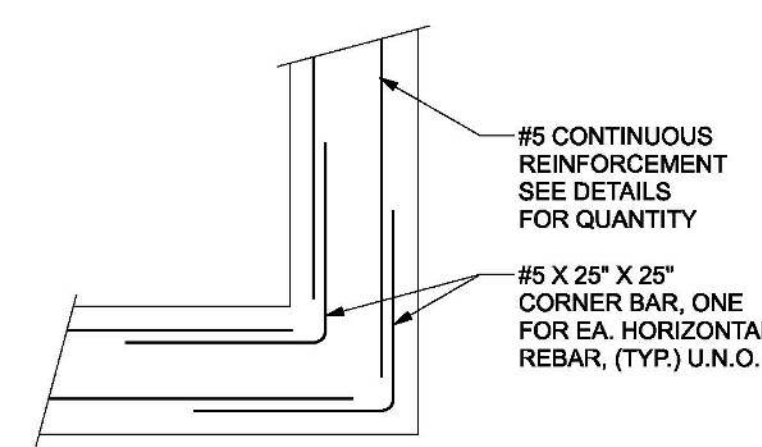
F7 FOOTING @ SHOWER @ MASONRY
SCALE: 1/2" = 1'-0"



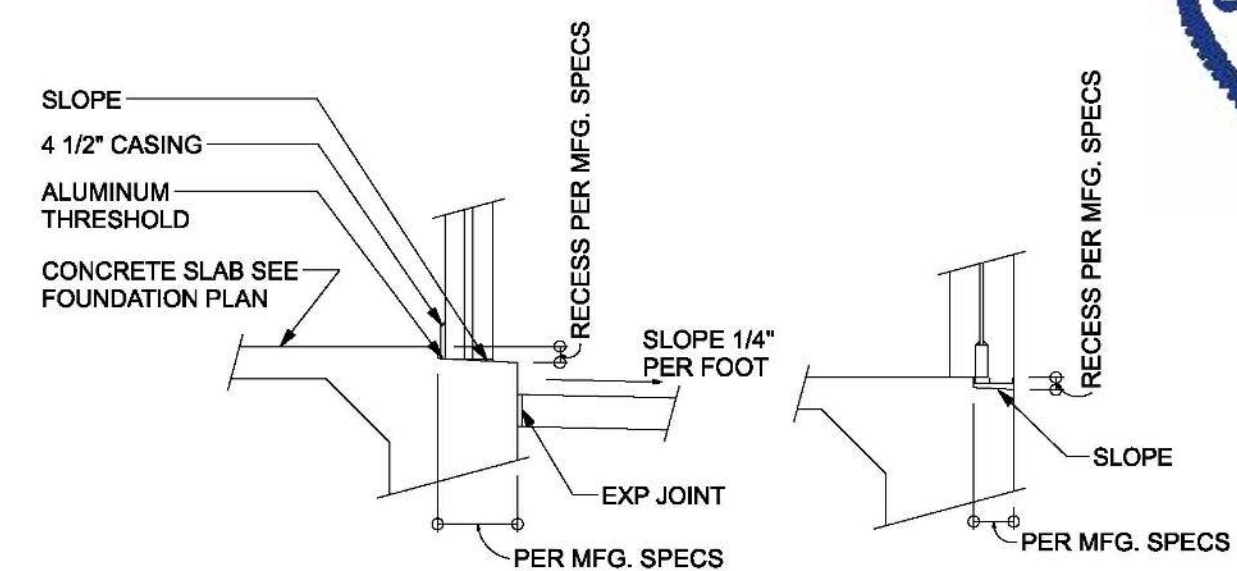
F8 THICKEND EDGE
SCALE: 1/2" = 1'-0"



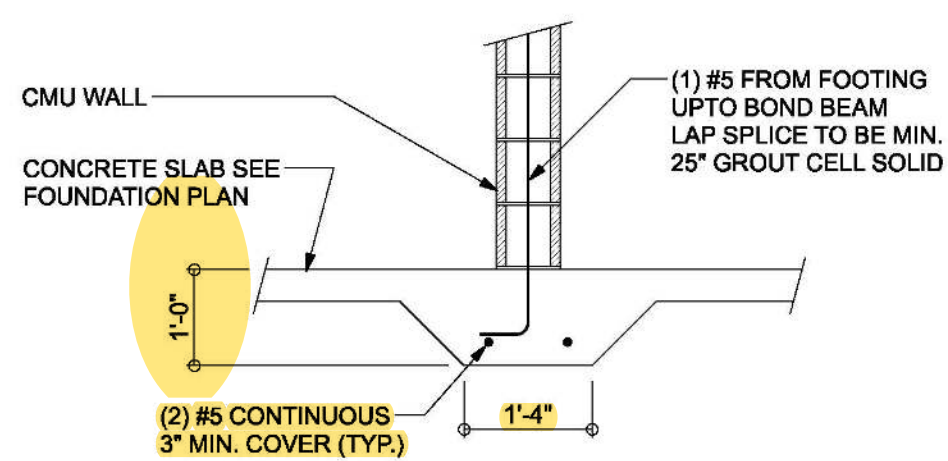
F9 (TYP.) STEP FOOTING DETAIL
SCALE: 1/2" = 1'-0"



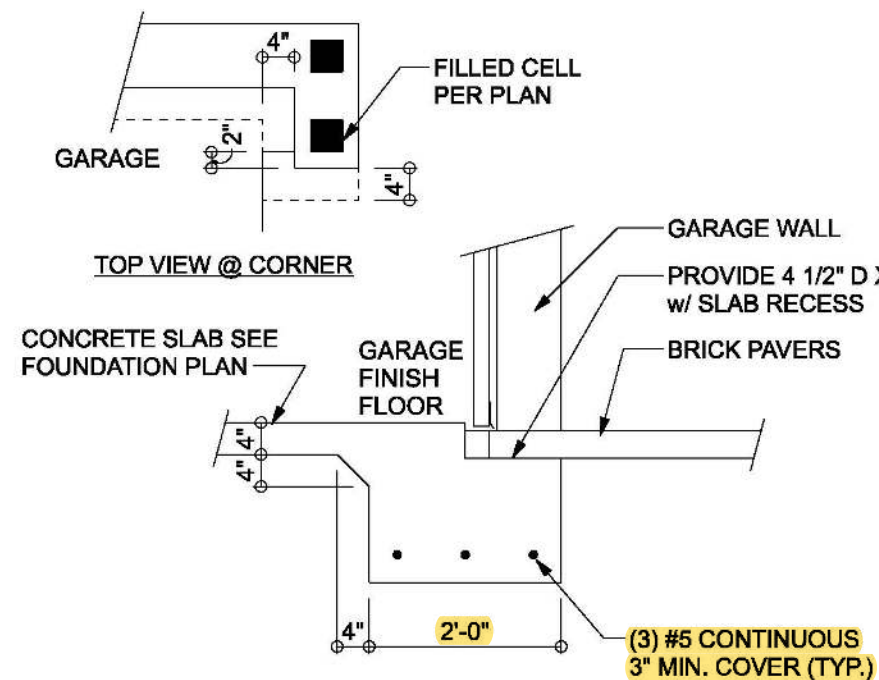
F10 (TYP.) CORNER BAR DETAIL
SCALE: 1/2" = 1'-0"



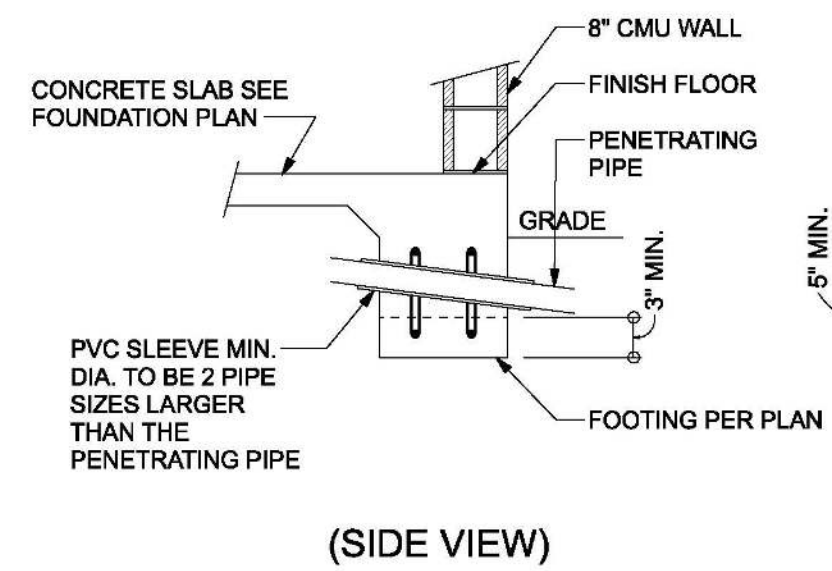
F11 EXTERIOR DOOR POURED SILLS
SCALE: 1/2" = 1'-0"



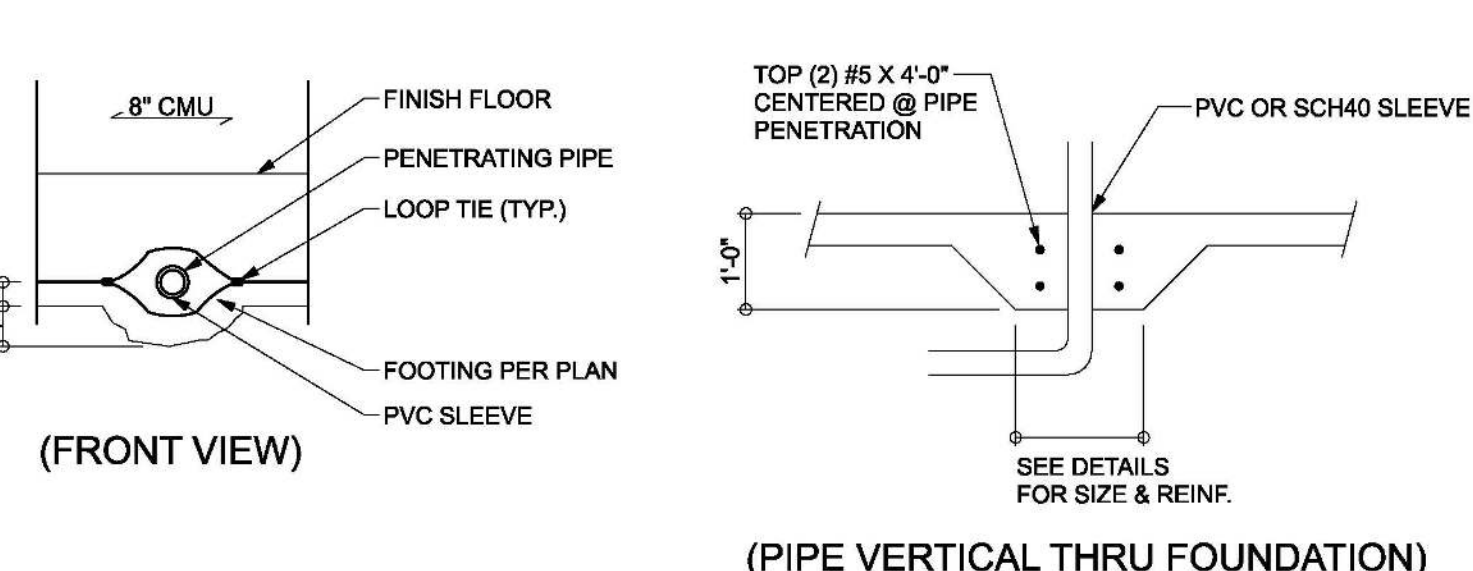
F12 INTERIOR BEARING FOOTING w/ CMU WALL
SCALE: 1/2" = 1'-0"



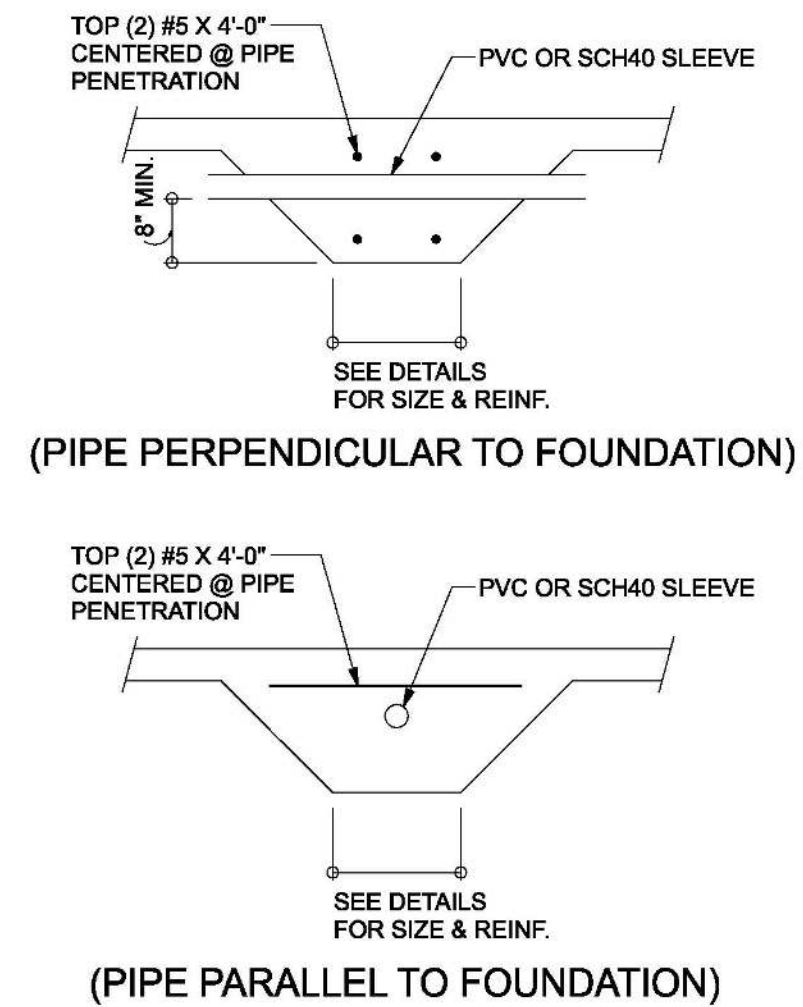
F13 THICKENED EDGE @ GARAGE DOOR
SCALE: 1/2" = 1'-0"



F14 TYPICAL FOUNDATION PENETRATIONS
SCALE: 1/2" = 1'-0"



F15 TYPICAL FOUNDATION PENETRATIONS
SCALE: 1/2" = 1'-0"



F16 TYPICAL FOUNDATION PENETRATIONS
SCALE: 1/2" = 1'-0"

Blake Construction

Ronnie Shuman Res

PROJECT ADDRESS:
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DIMENSIONS:
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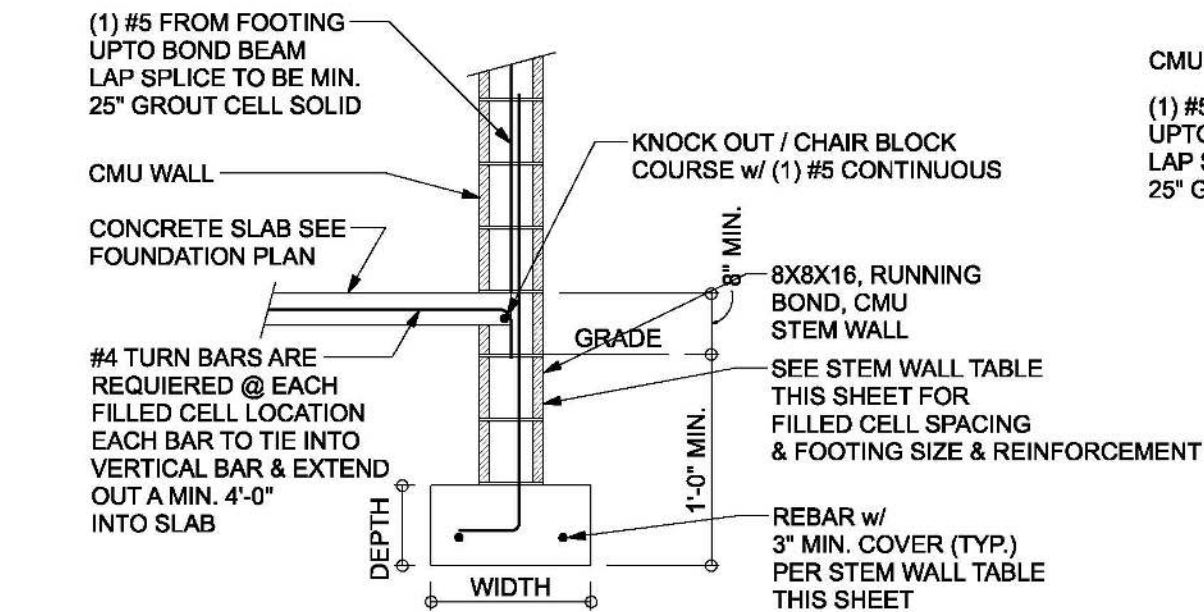


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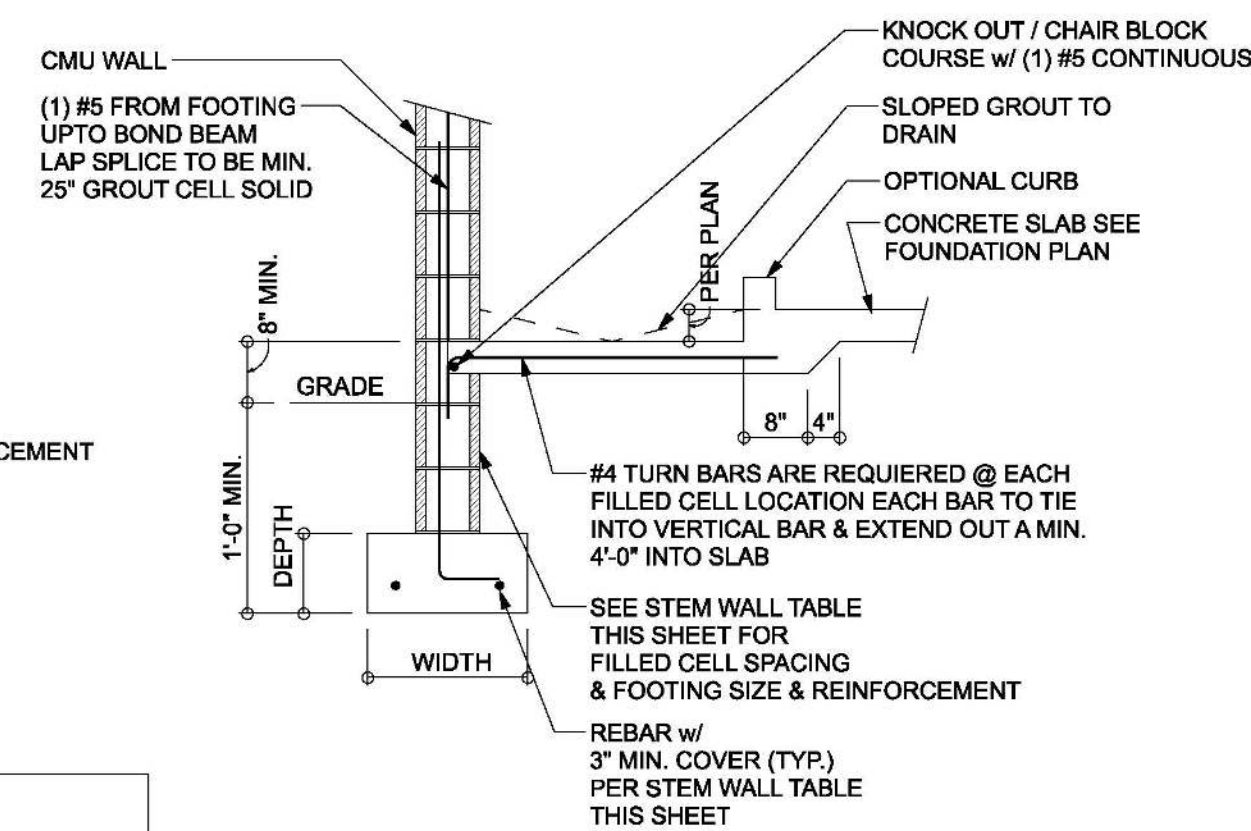
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JOB NUMBER:
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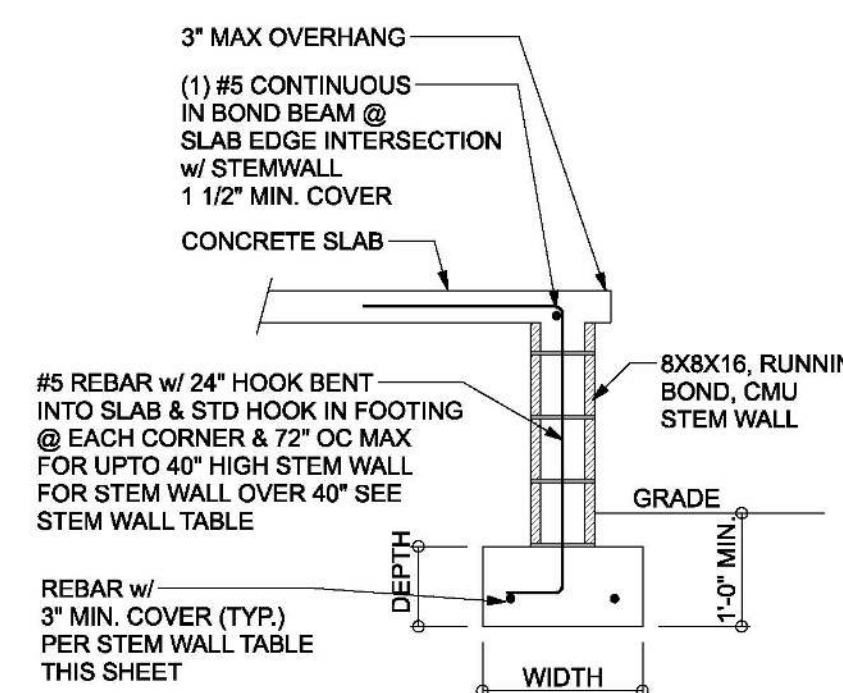
S-2.1
OF 4 SHEETS



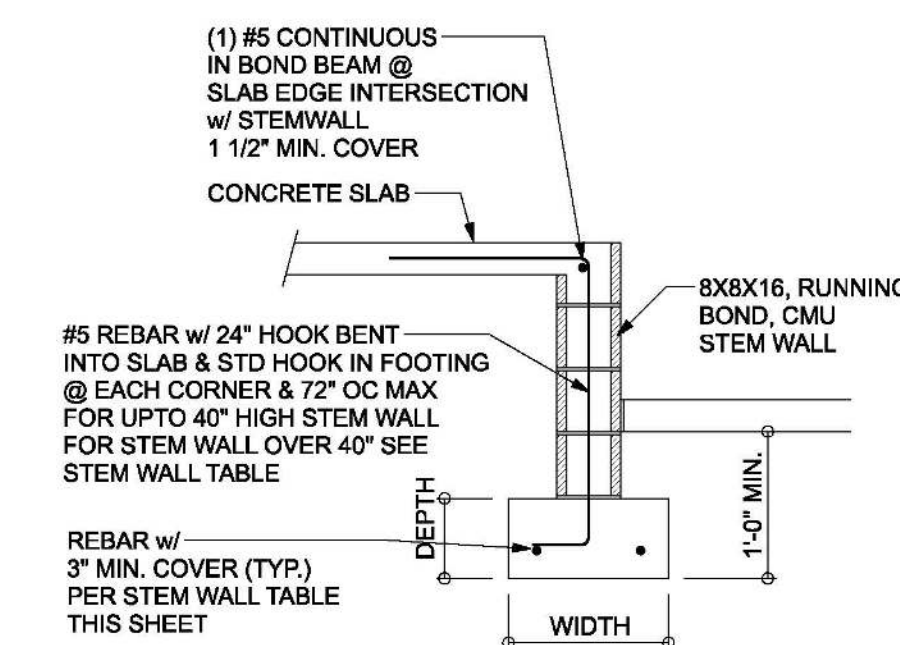
F1 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



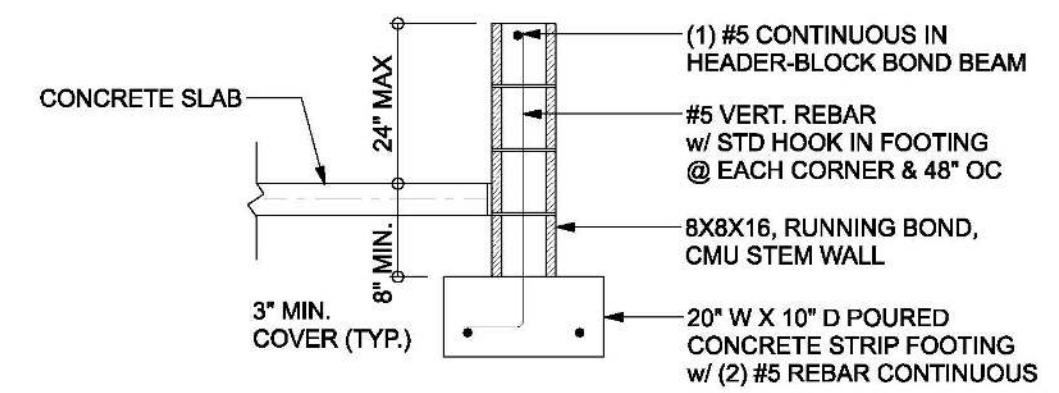
F7 FOOTING @ SHOWER @ MASONRY
SCALE: 1/2" = 1'-0"



F15 STEM WALL FOOTING @ PORCH
SCALE: 1/2" = 1'-0"



F16 STEM WALL @ GARAGE STEP DOWN
SCALE: 1/2" = 1'-0"



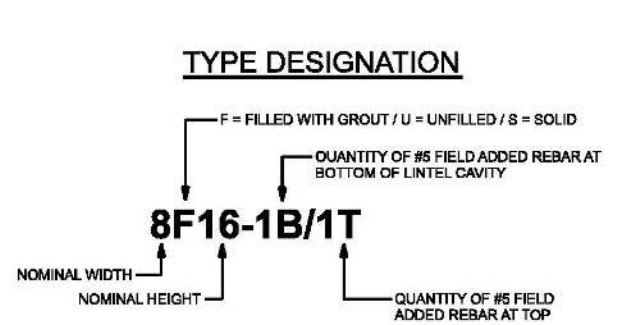
F17 STEM WALL CURB FOOTING @ SCREENED LANAI
SCALE: 1/2" = 1'-0"

STEM WALL TABLE

STEM WALL HEIGHT	FOOTING DIMENSION				NUMBER / SIZE OF REBAR IN FOOTING	MAX FILLED CELL SPACING (O.C.) IN STEM WALL
	1-STORY DEPTH	1-STORY WIDTH	2-STORY DEPTH	2-STORY WIDTH		
8" - 40"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	MATCH FILLED CELL SPACING PER PLAN
48" - 64"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	40"
72" - 80"	10"	30"	10"	30"	(3) #5 REBARS FOR 1-STORY & 2-STORY	32"

NOTE:
ALL STEM WALL FOUNDATIONS OVER 3'-0" IN HEIGHT TO BE POURED SOLID

OPTIONAL STEM WALL FOUNDATION



MATERIALS

1. Fc precast lintel = 3500 psi
2. Fc prestressed lintel = 6000 psi
3. Grout per ASTM C476 Fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1500 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

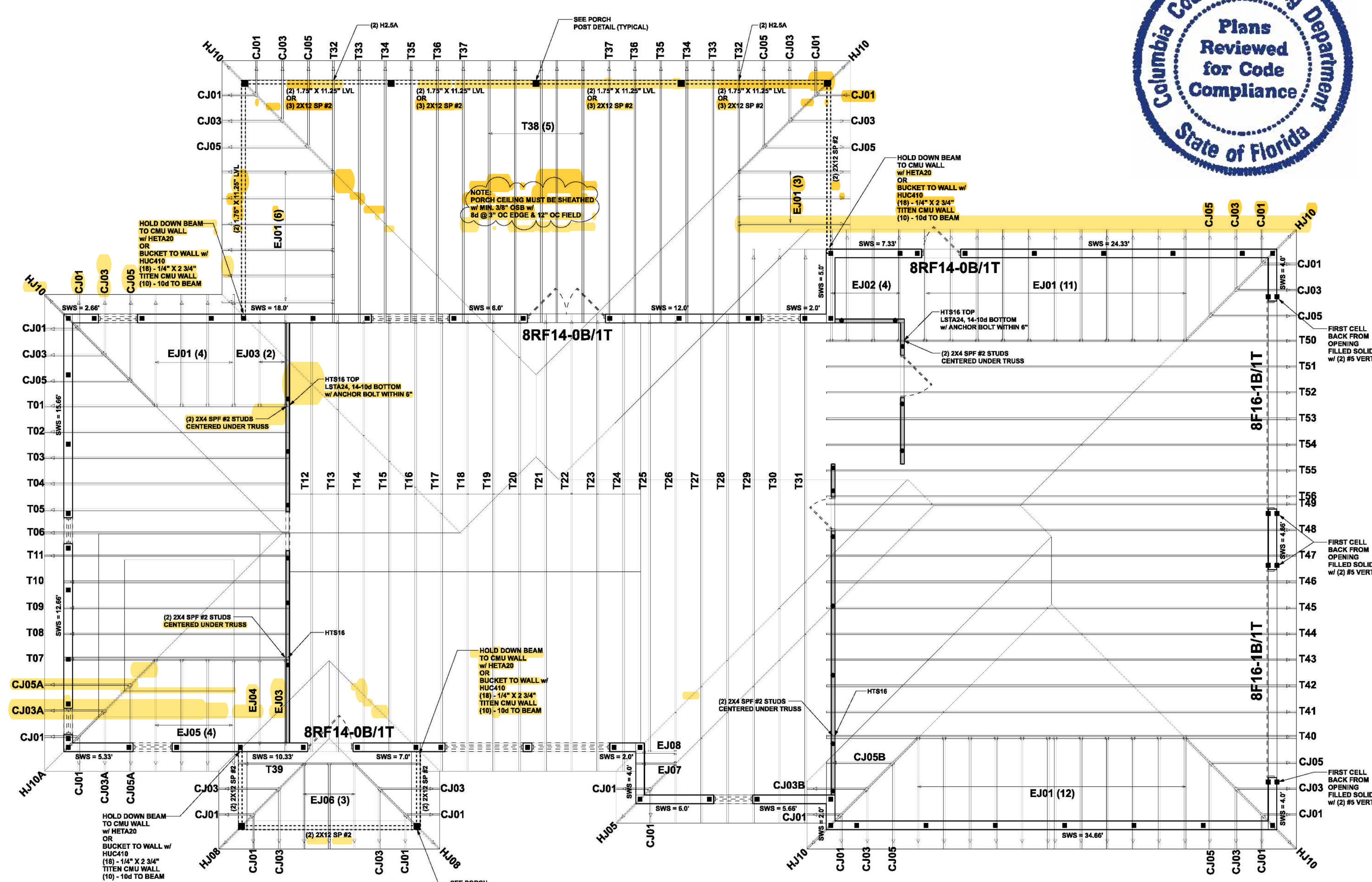
1. Provide full mortar bed and head joints.
2. Show field lintels as required.
3. Installation of lintel must comply with the architectural and structural documents.
4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical oil reinforcing and grouting.
5. All lintels meet or exceed L/500 deflection, except lintels 17'-4" and longer with a nominal height of 8" most or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
9. Safe load rating based on rational design analysis per ACI 318 and ACI 308.
10. Product Approvals: Manassas County, Florida No. 03-0006-05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-296 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:
Applied vertical load + Applied horizontal load
Safe vertical load + Safe horizontal load ≤ 1.0
13. Additional lateral load capacity can be obtained by the designer by providing additional mild steel reinforcement above the lintel. See detail at right.

SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
2. Exception: Safe loads for unfilled lintels must be reduced by 20% if bearing length is less than 5 1/2 inches.
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebar in 8" lintels only.
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at c-d away from face of support.
7. For composite lintel weight not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown.
9. All safe loads in units of pounds per linear foot.
10. All safe loads based on simply supported span.
11. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar.
Example: 7'-4" lintel type 8F32-1B safe gravity load = 6472(1.0468) (150)(0.071) w/ 15% reduction 6472 = (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-INTELS		SAFE LOAD - POUNDS PER LINEAR FOOT											
LENGTH	TYPE	8F8-8	8F12-8	8F16-8	8F20-8	8F24-8	8F28-8	8F32-8	8F36-8	8F40-8	8F44-8	8F48-8	8F52-8
2'-10" (84")	PRECAST	2231	3068	4065	5113	6213	7347	8524	9734	10964	12214	13484	14764
3'-4" (102")	PRECAST	2231	3068	4065	5113	6213	7347	8524	9734	10964	12214	13484	14764
4'-0" (120")	PRECAST	1966	2587	3375	4250	5163	6113	7094	8104	9134	10184	11254	12344
4'-4" (132")	PRECAST	1599	2089	2713	3375	4065	4794	5564	6374	7214	8084	8984	9914
5'-4" (162")	PRECAST	1217	1599	2089	2613	3163	3747	4364	5014	5694	6404	7144	7914
5'-10" (174")	PRECAST	1062	1387	1813	2250	2713	3194	3694	4214	4754	5314	5894	6484
6'-4" (192")	PRECAST	908	1173	1537	1913	2300	2694	3104	3534	3984	4454	4944	5444
7'-4" (222")	PRECAST	743	957	1250	1550	1863	2184	2514	2854	3214	3584	3964	4354
8'-4" (252")	PRECAST	584	757	1000	1250	1500	1750	2000	2250	2500	2750	3000	3250
10'-0" (300")	PRECAST	478	613	800	1000	1200	1400	1600	1800	2000	2200	2400	2600
10'-4" (312")	PRECAST	478	613	800	1000	1200	1400	1600	1800	2000	2200	2400	2600
11'-4" (342")	PRECAST	392	500	650	825	1000	1175	1350	1525	1700	1875	2050	2225
12'-0" (360")	PRECAST	337	425	550	688	825	963	1100	1238	1375	1513	1650	1788
13'-4" (402")	PRECAST	296	375	488	613	738	863	988	1113	1238	1363	1488	1613
14'-0" (420")	PRECAST	279	350	450	563	675	788	900	1013	1125	1238	1350	1463
14'-4" (428")	PRECAST	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
15'-4" (462")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
16'-4" (492")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
17'-4" (522")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
19'-4" (582")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
21'-4" (642")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
22'-0" (660")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
24'-0" (720")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-INTELS		SAFE LOAD - POUNDS PER LINEAR FOOT											
LENGTH	TYPE	8R16-8	8R20-8	8R24-8	8R28-8	8R32-8	8R36-8	8R40-8	8R44-8	8R48-8	8R52-8	8R56-8	8R60-8
4'-4" (132")	PRECAST	1635	1743	1850	1957	2064	2171	2278	2385	2492	2599	2706	2813
4'-8" (144")	PRECAST	1494	1587	1680	1773	1866	1959	2052	2145	2238	2331	2424	2517
5'-4" (162")	PRECAST	868	913	958	1003	1048	1093	1138	1183	1228	1273	1318	1363
5'-10" (174")	PRECAST	810	855	900	945	990	1035	1080	1125	1170	1215	1260	1305
6'-4" (192")	PRECAST	797	842	887	932	977	1022	1067	1112	1157	1202	1247	1292
7'-4" (222")	PRECAST	669	714	759	804	849	894	939	984	1029	1074	1119	1164
8'-4" (252")	PRECAST	411	436	461	486	511	536	561	586	611	636	661	686



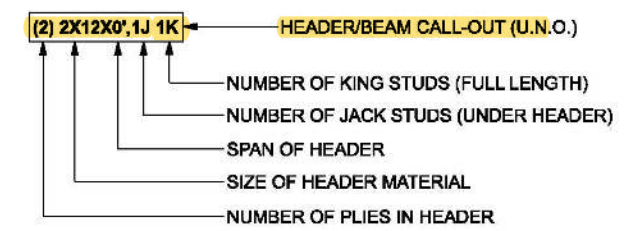
STRUCTURAL PLAN

SCALE: 3/16" = 1'-0"

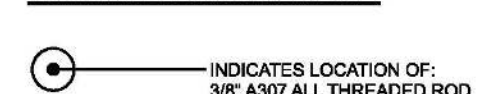
STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC51-103, BC51-81, BC51-82, & BC51-83. BC51-81, BC51-82, & BC51-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

FRAME HEADER LEGEND



THREADED ROD LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	91.9'	143.3'
REQUIRED	36.0'	28.0'

UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS

1. USE HETA16 CMU TO TRUSS
2. USE H2.5A FRAME TO TRUSS
3. ALL LINTELS TO BE: 8F16-0B/1T
4. ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SP #2
5. ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE

ENGINEERED TRUSSES ATTACH PER TRUSS UPLIFT TO CMU WALLS:

- HETA16 FOR UP TO 1350 LB UPLIFT
(2) HETA16 FOR UP TO 2035 LB UPLIFT
MGT FOR UP TO 3965 LB UPLIFT TO FRAME WALLS:
(1) H2.5A FOR UP TO 495 LB UPLIFT OR
(2) H2.5A FOR UP TO 990 LB UPLIFT
SEE CONNECTOR TABLE OR SIMPSON BOOK FOR ADDITIONAL OPTIONS

Blake Construction

Ronnie Shuman Ros

PROJECT ADDRESS:
205 SW Madison Court
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915
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Thursday, August 26, 2021

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JOB NUMBER:
211179

S-3
OF 4 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #282469