

Alpine Engineered Products, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 567
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1SUL487-Z0410150625

Truss Fabricator: Anderson Truss Company
Job Identification: 6-004-GLENWOOD KING - BULLARD
Truss Count: 89
Model Code: Florida Building Code 2004
Truss Criteria: ANSI/TPI-2002(STD)/FBC
Engineering Software: Alpine Software, Version 7.22.
Structural Engineer of Record:
Address:
Minimum Design Loads: Roof - 32.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 110 MPH ASCE 7-02 -Closed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR487

Details: BRCLBSUB-CNBRGBLK-A11015EE-GBLLETIN-PIGBACKB-

Seal Date: 02/10/2006

-Truss Design Engineer-
Arthur R. Fisher
Florida License Number: 59687
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	86561--A1		06041029	02/10/06
2	86562--A2		06041030	02/10/06
3	86563--A3		06041031	02/10/06
4	86564--A4		06041032	02/10/06
5	86565--A5		06041033	02/10/06
6	86566--A6		06041034	02/10/06
7	86567--B1		06041035	02/10/06
8	86568--B2		06041036	02/10/06
9	86569--B3		06041037	02/10/06
10	86570--B4		06041038	02/10/06
11	86571--B5		06041039	02/10/06
12	86572--B6G		06041040	02/10/06
13	86573--B7		06041041	02/10/06
14	86574--B8G		06041042	02/10/06
15	86575--C1-GE		06041043	02/10/06
16	86576--C2		06041044	02/10/06
17	86577--C3G		06041008	02/10/06
18	86578--C4		06041095	02/10/06
19	86579--C5		06041009	02/10/06
20	86580--C6		06041045	02/10/06
21	86581--C7		06041046	02/10/06
22	86582--C8		06041047	02/10/06
23	86583--C9G		06041048	02/10/06
24	86584--CC4		06041010	02/10/06
25	86585--CC5		06041049	02/10/06
26	86586--CC6		06041050	02/10/06
27	86587--CC7		06041051	02/10/06
28	86589--D2		06041053	02/10/06
29	86590--D3		06041054	02/10/06
30	86591--D4		06041055	02/10/06
31	86592--D5G		06041056	02/10/06
32	86593--FGB		06041057	02/10/06
33	86594--FGC		06041058	02/10/06
34	86595--FGCC		06041059	02/10/06
35	86596--FGX		06041060	02/10/06
36	86597--HJ7		06041061	02/10/06
37	86598--EJ7		06041011	02/10/06
38	86599--J5		06041012	02/10/06

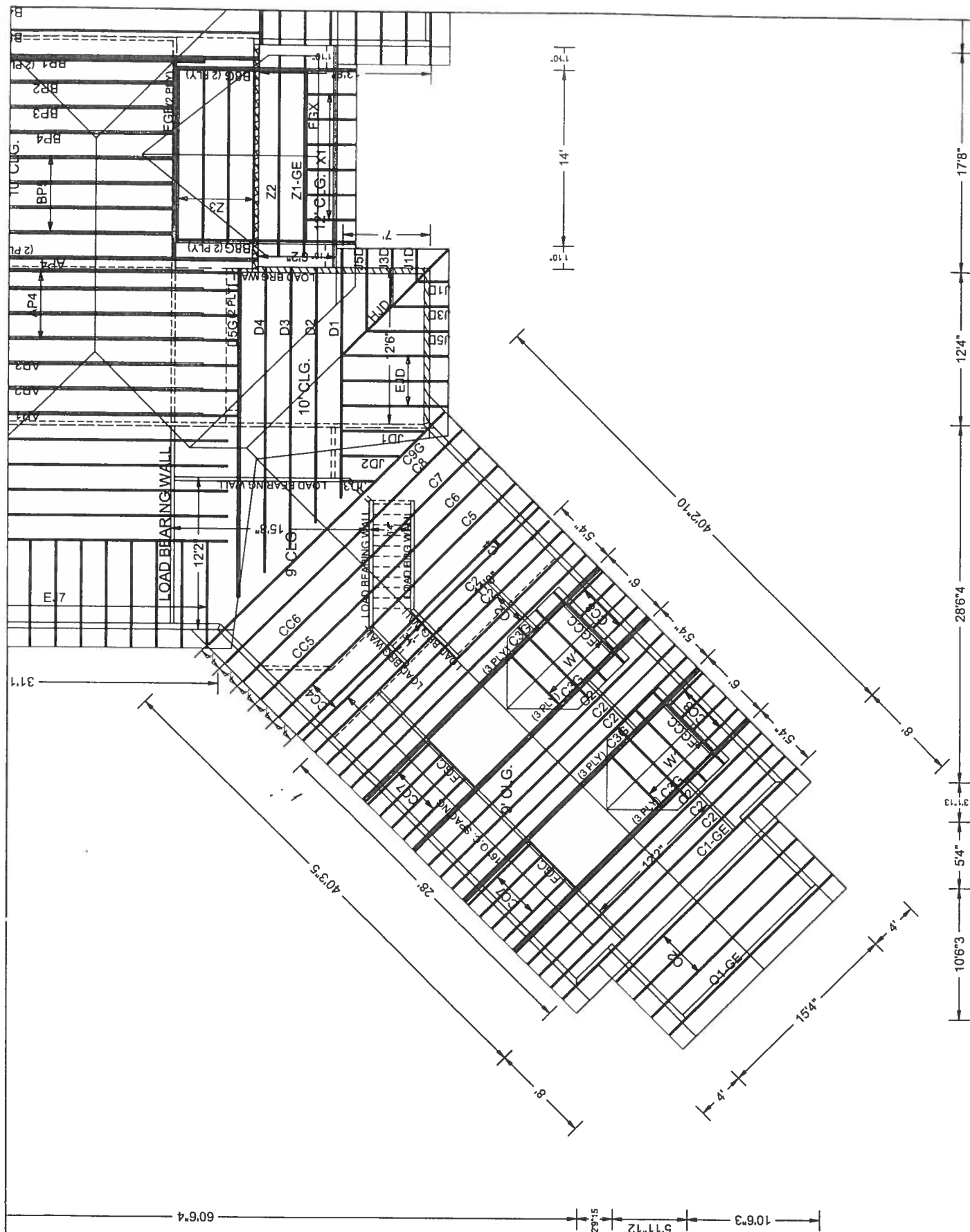
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39	86600--J3		06041013	02/10/06
40	86601--J1		06041062	02/10/06
41	86602--CC8		06041063	02/10/06
42	86603--HJD		06041064	02/10/06
43	86604--EJD		06041014	02/10/06
44	86605--J5D		06041065	02/10/06
45	86606--J3D		06041015	02/10/06
46	86607--J1D		06041066	02/10/06
47	86608--JD1		06041016	02/10/06
48	86609--JD2		06041017	02/10/06
49	86610--JD3		06041018	02/10/06
50	86611--HJK		06041067	02/10/06
51	86612--EJK1		06041068	02/10/06
52	86613--EJK2		06041069	02/10/06
53	86614--J5K		06041070	02/10/06
54	86615--J3K		06041071	02/10/06
55	86616--J1K		06041072	02/10/06
56	86617--JR1		06041019	02/10/06
57	86618--JR2		06041020	02/10/06
58	86619--JR3		06041021	02/10/06
59	86620--JR4		06041073	02/10/06
60	86621--JR5		06041074	02/10/06
61	86622--X1		06041022	02/10/06
62	86623--K1		06041075	02/10/06
63	86624--K2		06041076	02/10/06
64	86625--K3		06041077	02/10/06
65	86626--K4		06041096	02/10/06
66	86627--K5		06041078	02/10/06
67	86628--O1-GE		06041079	02/10/06
68	86629--O2		06041023	02/10/06
69	86630--AP1		06041080	02/10/06
70	86631--AP2		06041097	02/10/06
71	86632--AP3		06041082	02/10/06
72	86633--AP4		06041081	02/10/06
73	86634--BP1		06041083	02/10/06
74	86635--BP2		06041084	02/10/06
75	86636--BP3		06041085	02/10/06
76	86637--BP4		06041086	02/10/06

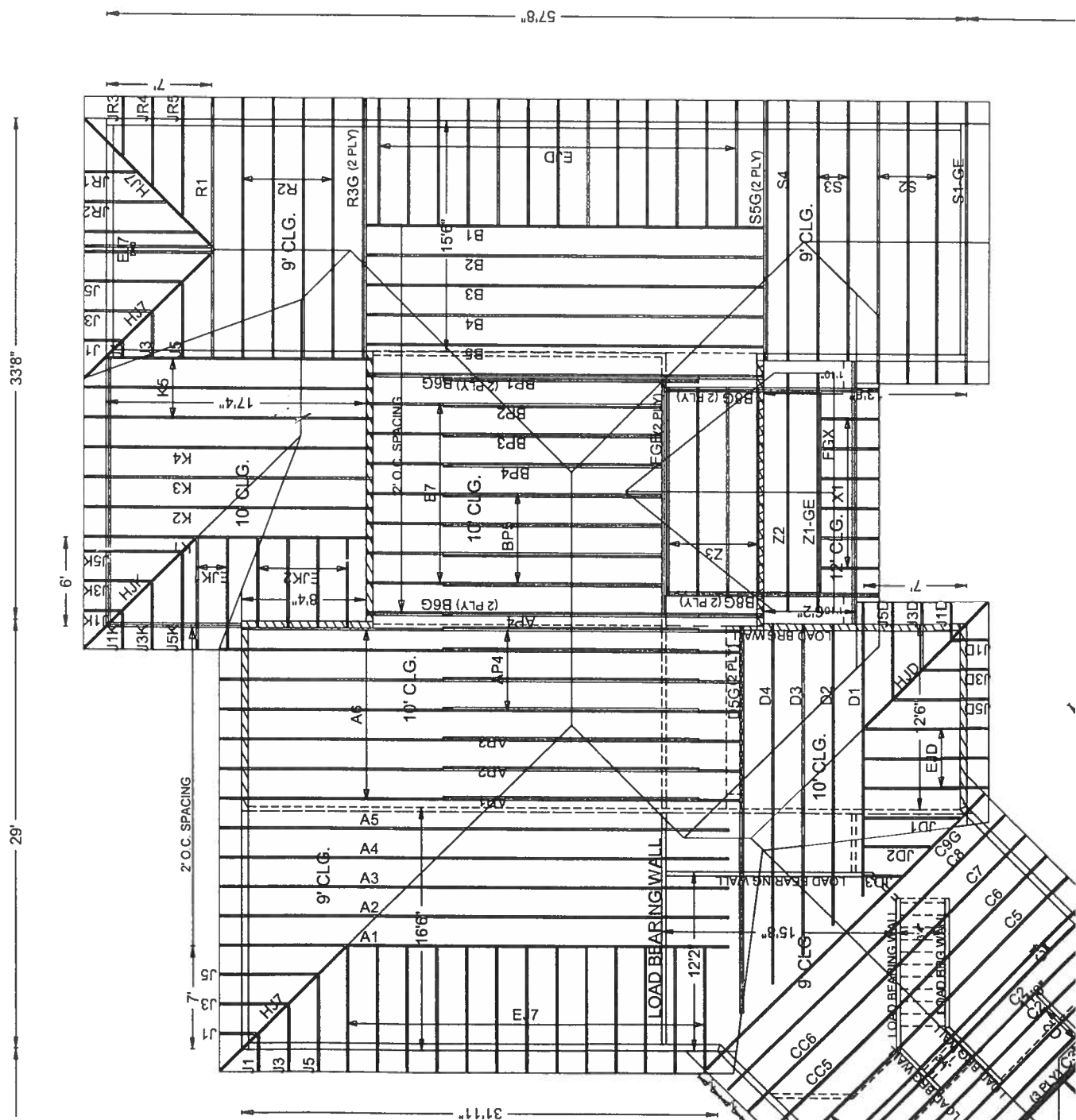
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77	86638--BP5		06041087	02/10/06
78	86639--R1		06041088	02/10/06
79	86640--R2		06041024	02/10/06
80	86641--R3G		06041089	02/10/06
81	86642--S1-GE		06041090	02/10/06
82	86643--S2		06041025	02/10/06
83	86644--S3		06041026	02/10/06
84	86645--S4		06041027	02/10/06
85	86646--S5G		06041091	02/10/06
86	86647--W1		06041028	02/10/06
87	86648--Z1-GE		06041092	02/10/06
88	86649--Z2		06041093	02/10/06
89	86650--Z3		06041094	02/10/06





755-6030
397-4708 Cell





Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

(B) 2x6 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

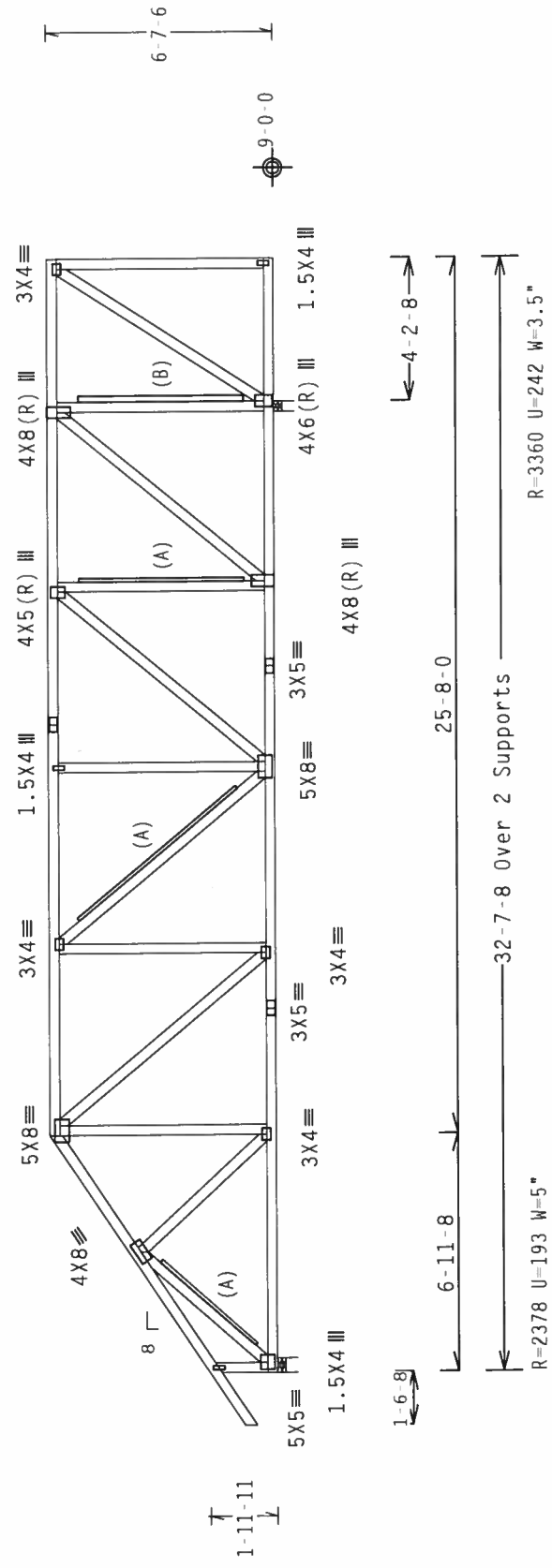
Trusses or components connecting to this girder have been modified by the truss designer. The loading for this girder requires verification for accuracy.

Right end vertical not exposed to wind pressure.

#1 hip supports 7-0-0 jacks with no webs.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

3X5



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

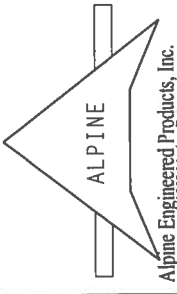
Scale = .1875"/Ft.

PLT TYP. Wave

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING CODES, SAFETY STANDARDS, ETC.) AND TPI TRUSS PLATE INSTITUTE, 503 HANLON, MI 48119, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/106GA (W/H/5/K) ASTM A653 GRADE 40/60 (W. K/H/5) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 10GA 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF 1711 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

TC LL	20.0 PSF	REF	R487 --	86561
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUR487	06041029
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	82249	
DUR.FAC.	1.25			



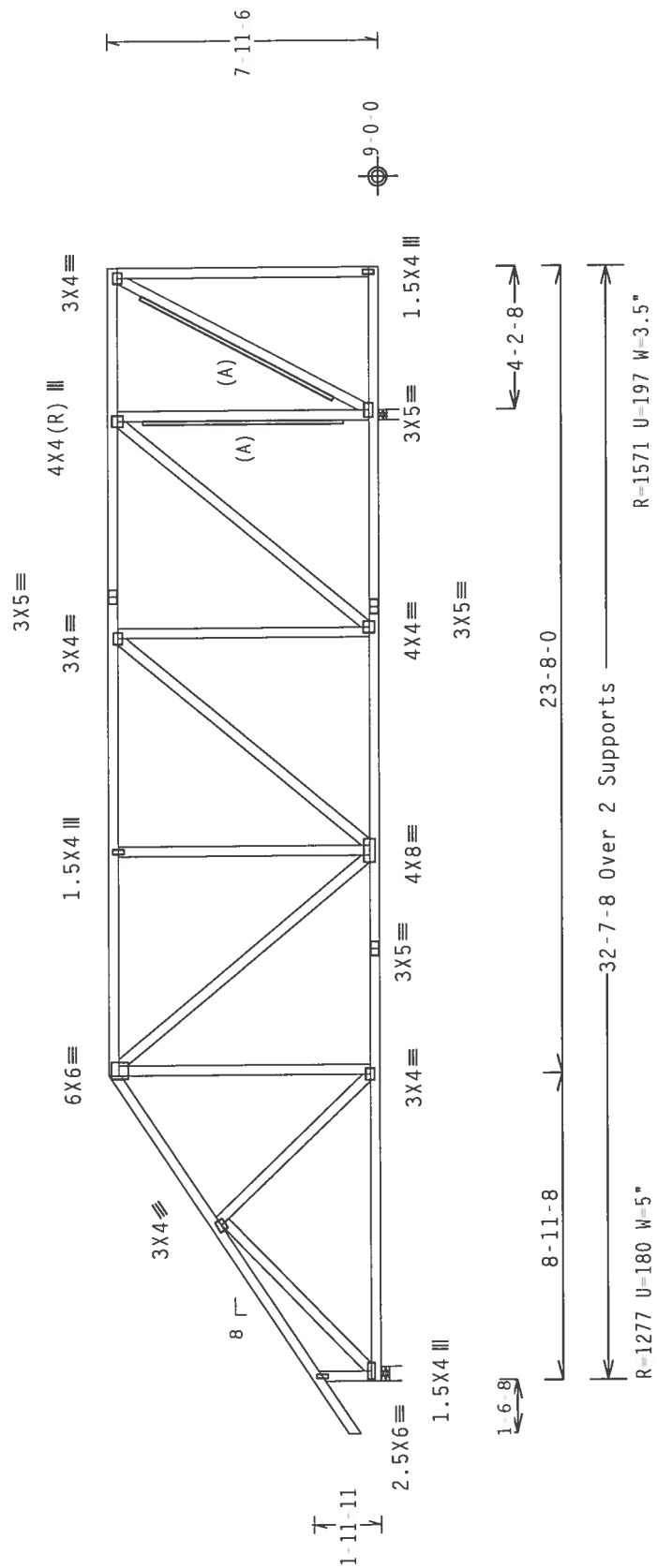
Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(A) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

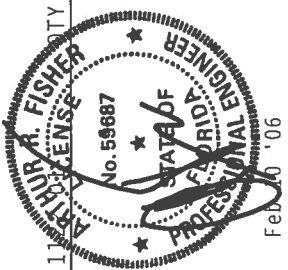
Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead loads 1.50.

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/

FL/4/-/R/- Scale = .1875"/Ft.

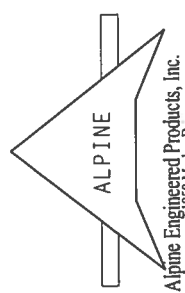
TC LL	20.0	PSF	REF R487 - 86562
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCUR8487 06041030
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 82260
DUR.FAC.	1.25		



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING TO PREVENT DAMAGE TO THE TRUSS. ALL TRUSSES SHALL BE DESIGNED TO MEET THE FOLLOWING INSTANT LOADS:
1. DEAD LOAD PER AREA OF 10 PSF.
2. LIVE LOAD PER AREA OF 60 PSF.
3. WIND LOAD PER AREA OF 18 PSF.
MADISON, MI 52719, AND VICA, LATED TRUSS COMPANY OF AMERICA, 6300 ENTERPRISE BL.,
MADISON, MI 52719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED,
TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED
RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING THUSSES, DESIGN COMPLAINS WITH APPLICABLE PROVISIONS OF THE MINNESOTA STATE CODE 40.60 (S.F. 4. K) S.F. STEEL ANALYSIS TO EACH FACE OF TRUSSES AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS LEGA 2.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMNIX AS OF TP11-2002 SEC.3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR TRUSS COMPONENT



Alpine Engineered Products, Inc.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 130 PLF at 0.00 to 130 PLF at 26.67
BC - From 50 PLF at 0.00 to 50 PLF at 26.67

(A) Continuous lateral bracing equally spaced on member.

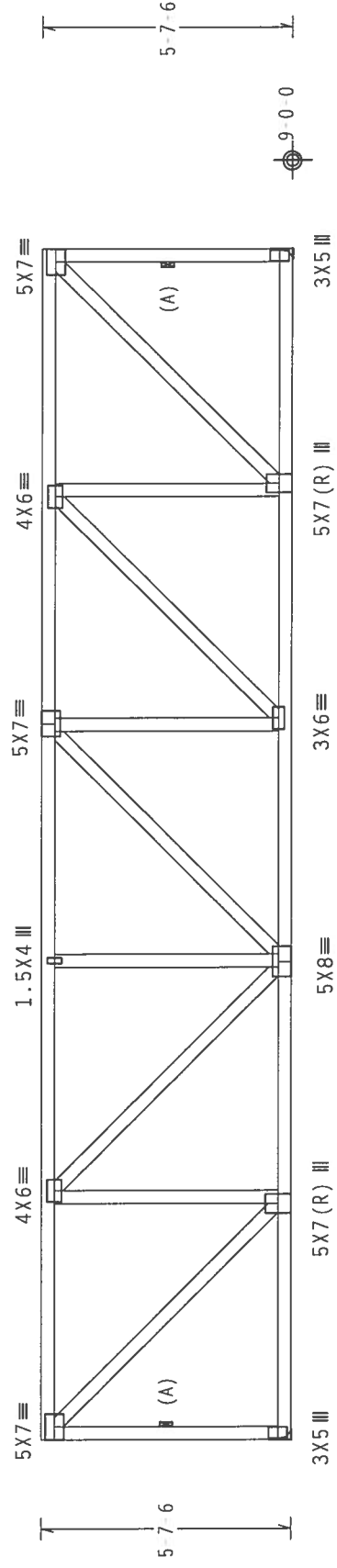
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

End verticals not exposed to wind pressure.

Max JT VERT DEFL: LL: 0.11" DL: 0.17" recommended camber 1/4"

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Truss must be installed as shown with top chord up.



26-8-0 Over 2 Supports

R-2400 U-257 H-Simpson HUS26
w/ (6) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

R-2400 U-257 H-Simpson HUS26
w/ (6) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

PLT TYP. Wave

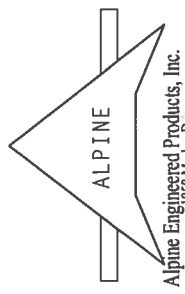
FL/-/4/-/-/R/-

Scale = .25"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REPAIRS MUST BE MADE IN ACCORDANCE WITH THE TPI (TRUSS PLATE INSTITUTE) 503 STANDARD SPECIFICATION FOR STEEL TRUSSES. ALL TRUSSES MUST BE DESIGNED, MANUFACTURED, SHIPPED, HANDLED, STORED, AND INSTALLED IN ACCORDANCE WITH THE TPI (TRUSS PLATE INSTITUTE) 503 STANDARD SPECIFICATION FOR STEEL TRUSSES. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR ON FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (40/60) ASTM A653 GRADE 40/60 (40 K/IN-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

TC LL	20.0 PSF	REF	R487 --	86567
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUSR487	06041035
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN-	146698	REV
DUR.FAC.	1.25			



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.95 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

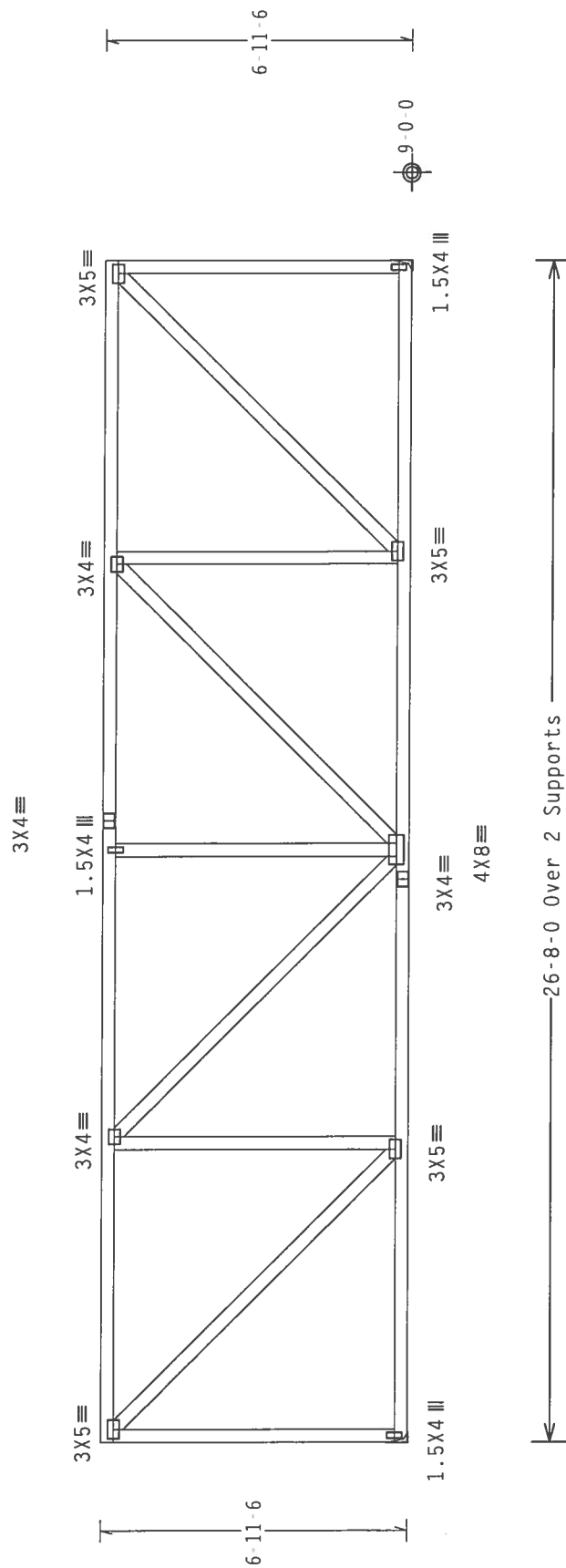
End verticals not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide for complete drainage of roof.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Truss must be installed as shown with top chord up.



R=1067 U=180 H=Simpson LUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

R=1067 U=180 H=Simpson LUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/10(0) 7.22.11

Scale = .25"/Ft.

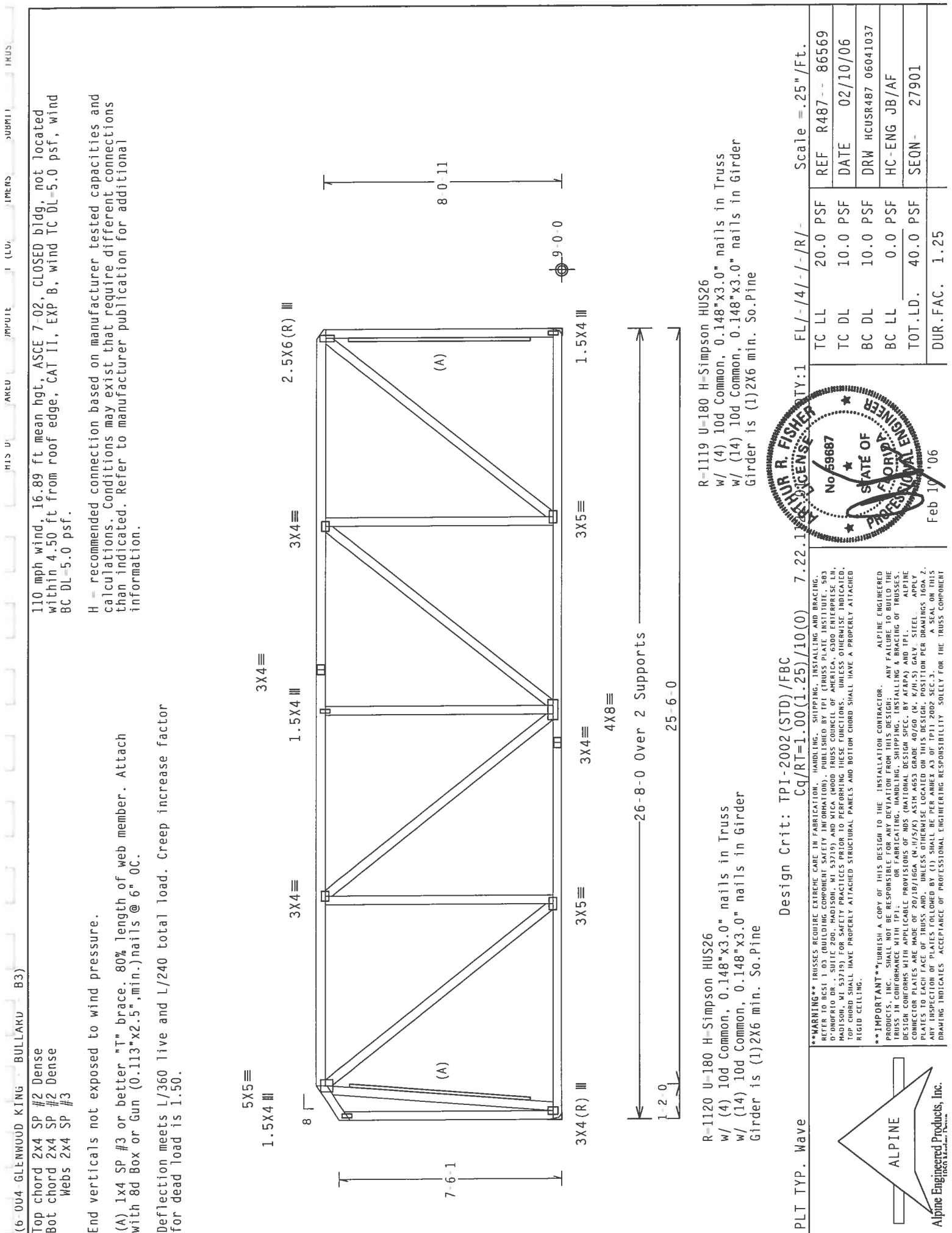
REF	R487	86568
TC LL	20.0 PSF	
TC DL	10.0 PSF	02/10/06
BC DL	10.0 PSF	DRW HCUSR487 06041036
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEON- 82847
DUR.FAC.	1.25	

ALPINE
Engineered Products, Inc.
19th Marlow Drive

BRIGGS R. FISHER
No. 59687
STATE OF ALABAMA
PROFESSIONAL ENGINEER
Feb 10 '06

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D'ONDRIE DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC. BY ACPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (4.0/5.0) ASTM A563 GRADE 40/60 (IN. 4.0/5.0) GALV. STEEL. APPLY TO ALL TRUSSES LOCATED ON THIS DESIGN. ANY DEVIATION FROM THIS DESIGN SHALL BE PERMITTED BY THE DESIGNER. ANY INSPECTION OF PLATES FOLLOWED BY TPI SHALL BE PERMITTED BY THE DESIGNER. ANY DEVIATION FROM THIS DESIGN SHALL BE PERMITTED BY THE DESIGNER. ANY INSPECTION OF PLATES FOLLOWED BY TPI SHALL BE PERMITTED BY THE DESIGNER. ANY DEVIATION FROM THIS DESIGN SHALL BE PERMITTED BY THE DESIGNER.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

End verticals not exposed to wind pressure.

(B) 1x4 SP #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(C) 2x4 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 17.56 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

(A) 2x6 SP #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

8

1.5X4 III

5X5 III

1.5X4 III

3X4 III

3X4 III

1.5X4 III

5X5 III

8

(A)

(B)

(C)

3X4 III

4X8 III

3X4 III

3X4 III

3X4 (R) III

7-6-1

8-0-11

3-2-0

21-2-0

2-4-0

26-8-0 Over 2 Supports

3-2-0

21-2-0

2-4-0

R=1120 U=180 H-Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

R=1120 U=180 H-Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002 (STD) /FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

Scale = .25" /Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

Feb 0 '06

Professional Engineer
No. 59687
State of Florida

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25

REF R487 -- 86570
DATE 02/10/06
DRW HCUSR487 06041038
HC-ENG JB/AF
SEON- 27890

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 18.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

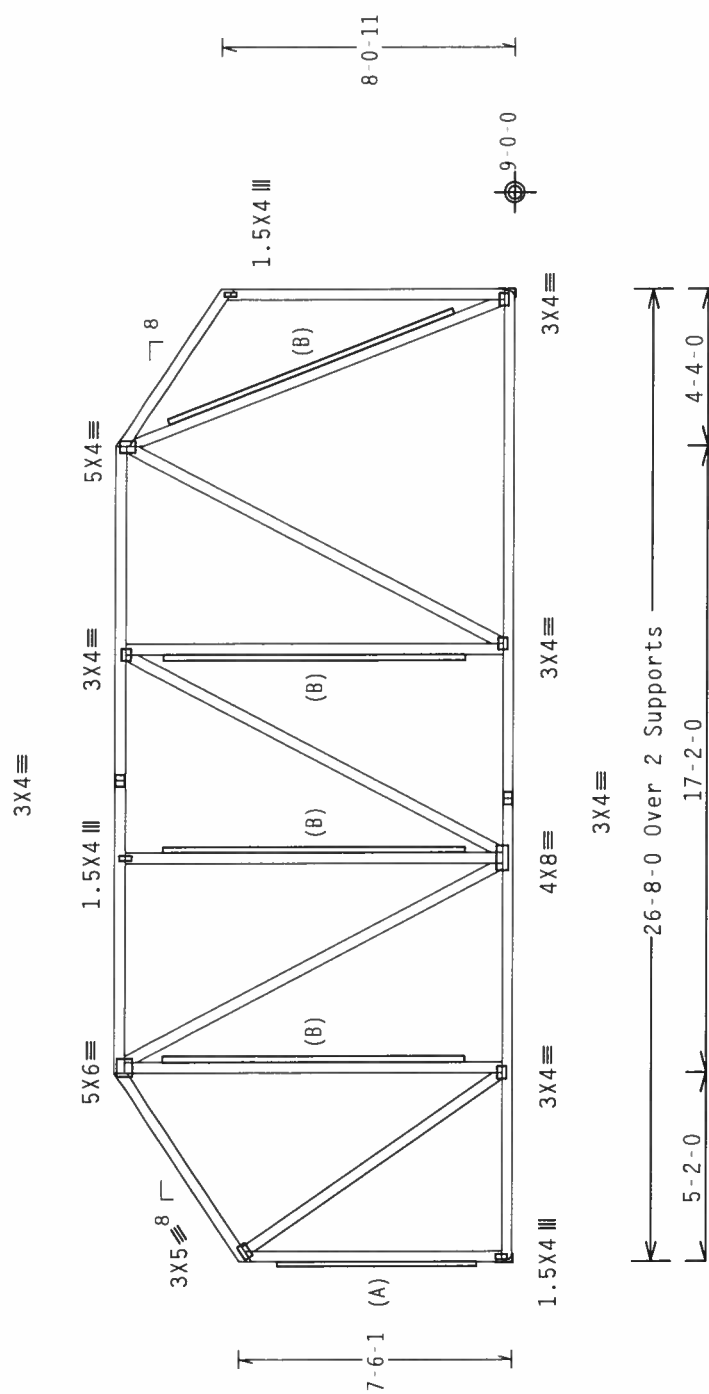
End verticals not exposed to wind pressure.

(A) 1x4 SP #3 or better "I" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5", min.) nails @ 6" OC.

(B) 2x6 SP #3 or better "I" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5", min.) nails @ 6" OC.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

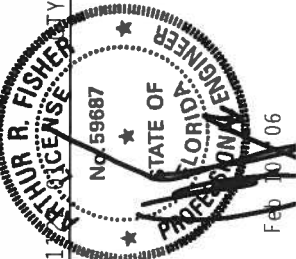
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



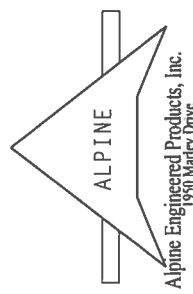
R=1120 U=180 H=Simpson HUS26
w/ (4) 10d Common, 0.148"x3.0" nails in Truss
w/ (14) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

Scale = .1875"/Ft.



PLT TYP. Wave	Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.22.1	FL/-/4/-/1/-/R/-	Scale = .1875"/Ft.
REF	R487 --	TC LL	20.0 PSF
DATE	02/10/06	TC DL	10.0 PSF
DRW	HCUSR487 06041039	BC DL	10.0 PSF
HC-ENG	JB/AF	BC LL	0.0 PSF
SEQN	27879	TOT.LD.	40.0 PSF
DUR.FAC.	1.25		



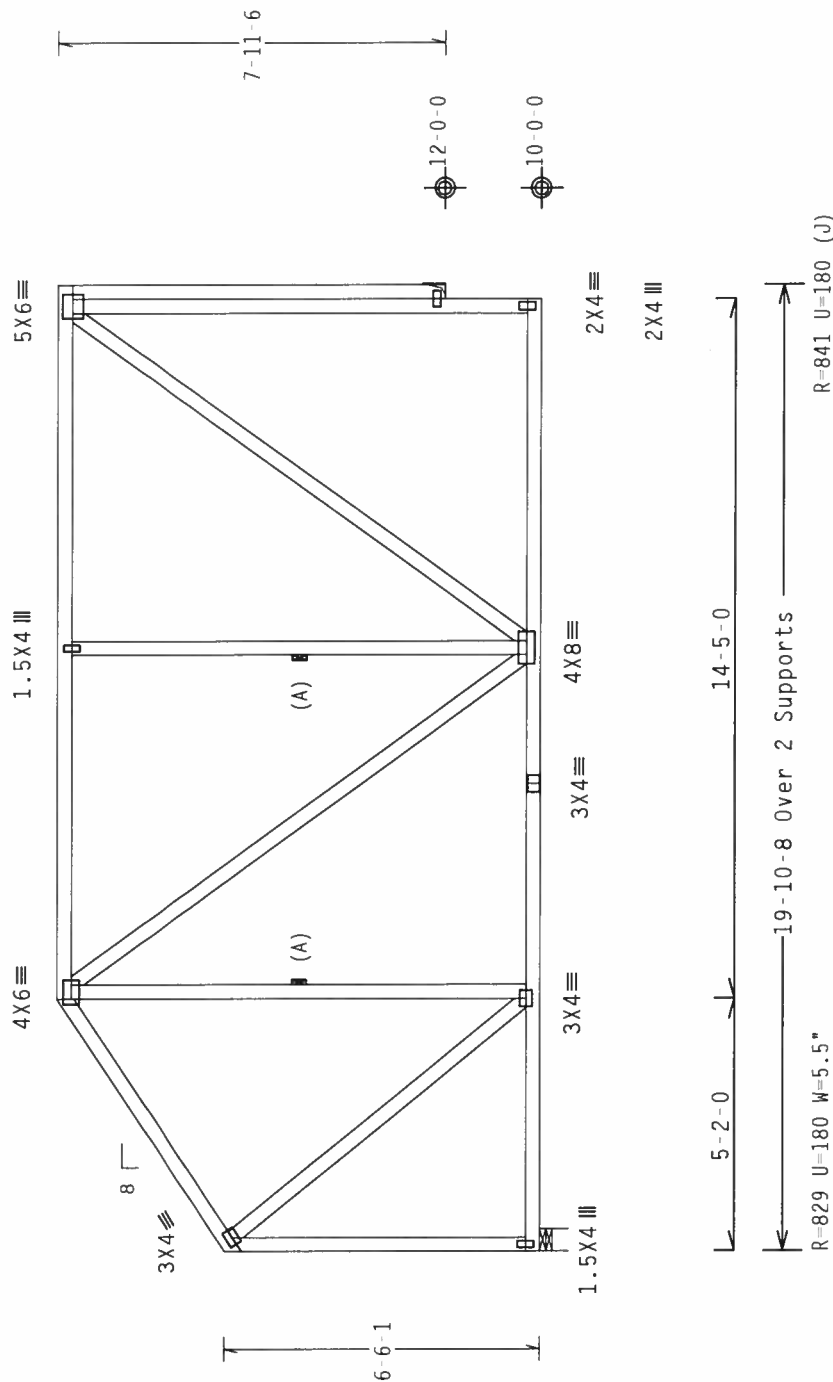
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Rt Bearing Leg 2x4 SP #3:

110 mph wind, 18.22 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

(J) hanger connection not found in inventory file for this condition.
Provide connection.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Design Crit: $\text{TPI}-2002(\text{STD})/\text{FBC}$ $Cq/RT=1.00(1.25)/10(0)$

7.22.11

TY:7

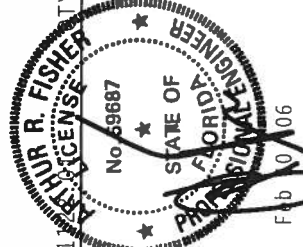
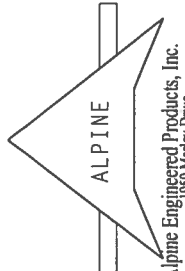
FL/-/4/-/-/R/-

Scale = .25"/Ft.

WARNING: THUSSES REQUIRE EXTREME CARE IN FABRICATION, SHIPPING, INSTALLING AND BRACING. REFER TO BCS 1.03 (BUILDING COMPONENT SAFETY), PUBLISHED BY IPI (IRVING PLATE INSTITUTE, 593 D'ONTARIO DR., SUITE 200, MADISON, WI 53719), AND NCA (NATIONAL TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE BL., MADISON, WI 53719) FOR SAFE FABRICATION PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID SEALING.

*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS DESIGNED HEREON WITHOUT FABRICATING, HANDLING, SHIPPING, INSTALLING OR BRACING AS TRUSSES, OR OTHERWISE CONFORMING WITH ALL APPLICABLE PROVISIONS OF THE AISC STEEL EDITION SPECIFICATIONS FOR STRUCTURAL STEEL CONNECTIONS WILL BE AT THE RISK OF THE USER.

CONNECTOR PLATES ARE MADE OF 20X18/16GA (W 1/2"x1/2") ASTM A563 GRADE 40/50 (F 40 / K 50) CARBON STEEL PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWINGS LE0A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF IPII 2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING INSPECTION SOLELY FOR THE TRUSS COMPONENT



TC LL	20.0 PSF	REF R487 - - 86573
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUR487 06041041
BC LL	0.0 PSF	HC-ENG JB/AF
TOT. LD.	40.0 PSF	SEQN- 27748
DUR. FAC.	1.25	

Top chord 2x4 SP #2 Dense :T2 2x6 SP #2:
Bot chord 2x6 SP #2
Webs 2x4 SP #3
:Lt Slider 2x4 SP #3: BLOCK LENGTH = 1.500'

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at -1.50 to 64 PLF at 12.71
PLF - From 312 PLF at 6.08 to 312 PLF at 12.71
BC - From 5 PLF at -1.50 to 5 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 12.71
TC - 240 LB Conc. Load at 2.38
PLB - 579 LB Conc. Load at (2.44,12.04)

Right end vertical not exposed to wind pressure.

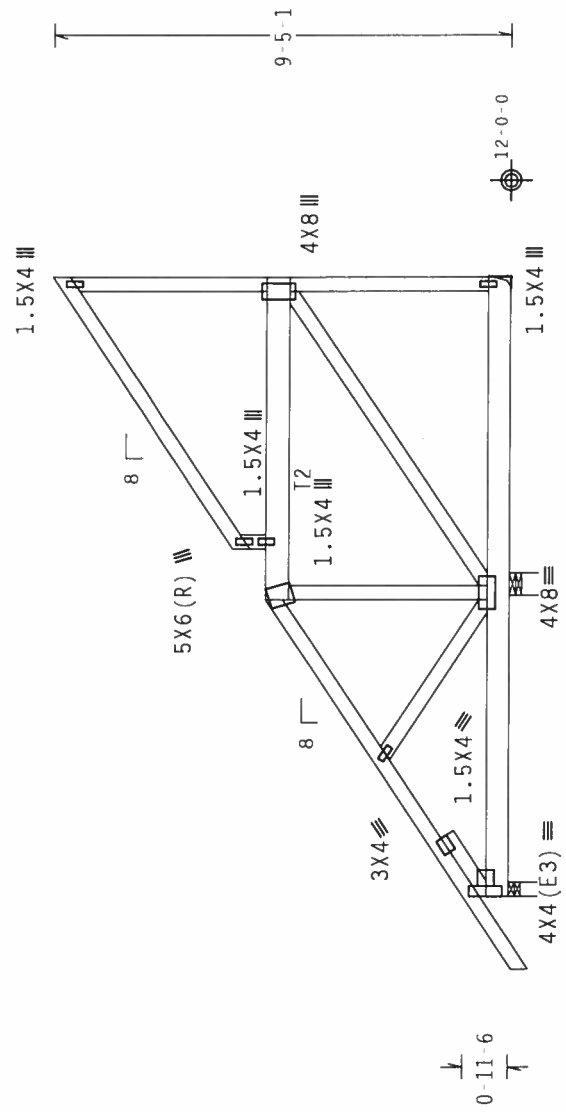
H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @ 9.25" o.c.
Bot Chord: 1 Row @ 12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 16.68 ft mean hgt. ASCE 7-02. CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, Exp B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



2.5X8 (E3) III

11-6-0
6-4-12
6-0-15
6-7-9

12-8-8 Over 3 Supports
R=780 U=180 W=3.5" R=2179 U=290 W=5.5"

R=1074 U=180 H=Simpson LUS26-2
w/ (4) 16d, 0.162"x2.5" nails in Truss
w/ (4) 16d, 0.162"x2.5" nails in Girder
Girder: 2x6 min. So.Pine

Design Crit: TPI-2002 (STD) / FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

PLT TYP. Wave

Scale = .25" / Ft.

ALPINE
Engineered Products, Inc.
1950 Marley Drive

REF	R487	--	86574
DATE	02/10/06		
DRW	HCUSR487		05041042
HC-ENG	JB/AF		
SEQN	27678		

TC LL	20.0	PSF
TC DL	10.0	PSF
BC DL	10.0	PSF
BC LL	0.0	PSF
TOT.LD.	40.0	PSF
DUR.FAC.	1.25	

ALPINE
Engineered Products, Inc.
1950 Marley Drive

Top chord 2x4 SP #2 Dense :T2, T3 2x8 SP SS:
Bot chord 2x8 SP SS :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

SPECIAL LOADS

- TC - From 65 PLF at -1.51 to 65 PLF at 5.58
- TC - From 82 PLF at 5.58 to 82 PLF at 9.82
- TC - From 65 PLF at 9.82 to 65 PLF at 11.67
- TC - From 65 PLF at 11.67 to 65 PLF at 13.51
- TC - From 82 PLF at 13.51 to 82 PLF at 17.75
- TC - From 65 PLF at 17.75 to 65 PLF at 20.89
- TC - From 65 PLF at 20.89 to 65 PLF at 24.84
- PLT - From 13 PLF at 10.58 to 13 PLF at 12.75
- BC - From 3 PLF at -1.51 to 3 PLF at 0.00
- BC - From 13 PLF at 0.00 to 13 PLF at 5.58
- BC - From 80 PLF at 5.58 to 80 PLF at 17.75
- BC - From 13 PLF at 17.75 to 13 PLF at 23.33
- BC - From 3 PLF at 23.33 to 3 PLF at 24.84
- BC - 60 LB Conc. Load at 5.58, 17.75

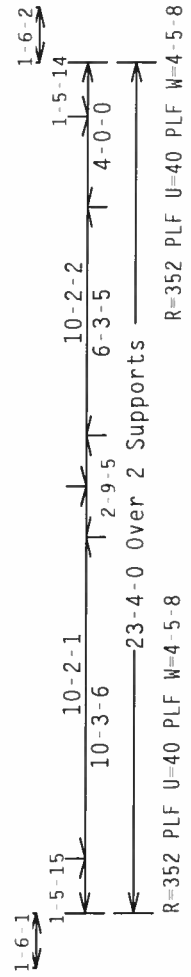
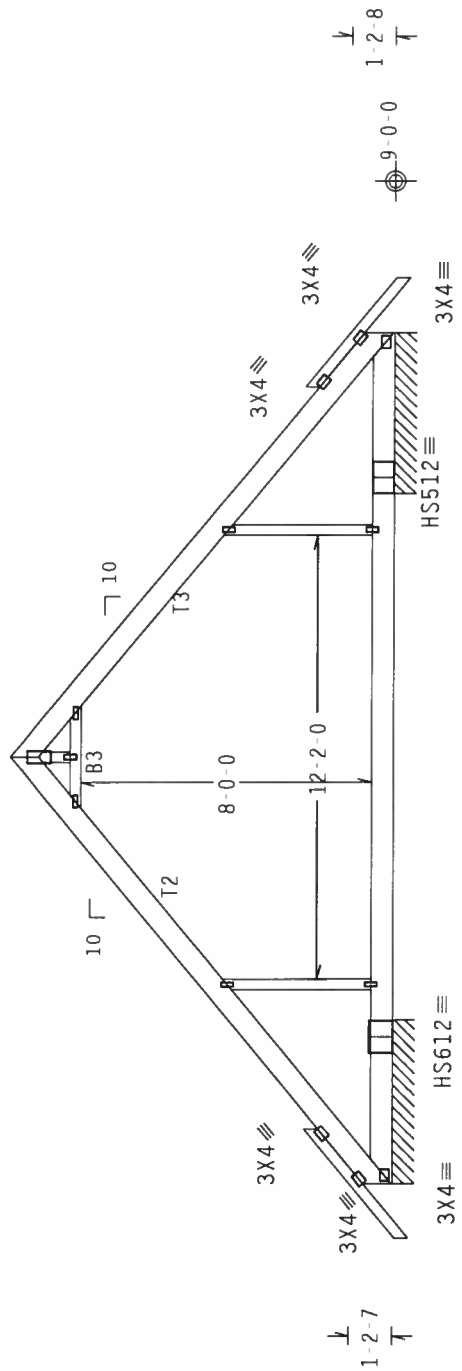
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Trusses to be spaced at 16.0" OC maximum.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

4X8(R) III



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS.Wave

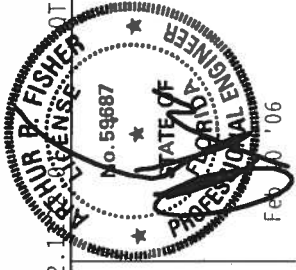
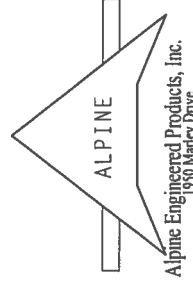
Cq/RT=1.00(1.25)/10(0)

7.22.1

Scale = .1875"/Ft.

WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 543 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT**TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE ENGINEERED PRODUCTS ARE NOT RESPONSIBLE FOR THE DESIGN OF THE TRUSS OR THE STRUCTURE TO WHICH IT IS APPLIED. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SECTION 10.00. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



TC LL	20.0 PSF	REF	R487 --	86575
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUSR487	06041043
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN	82829	REV
DUR.FAC.	1.25			

3 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)

Top Chord: 1 Row @ 6.25" o.c.

Bot Chord: 1 Row @ 8.00" 0.c.

Webs : 1 Row @ 4" o.c.

Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.04 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.18" due to live load and 0.28" due to dead load.

Trusses to be spaced at 42.5" OC maximum.

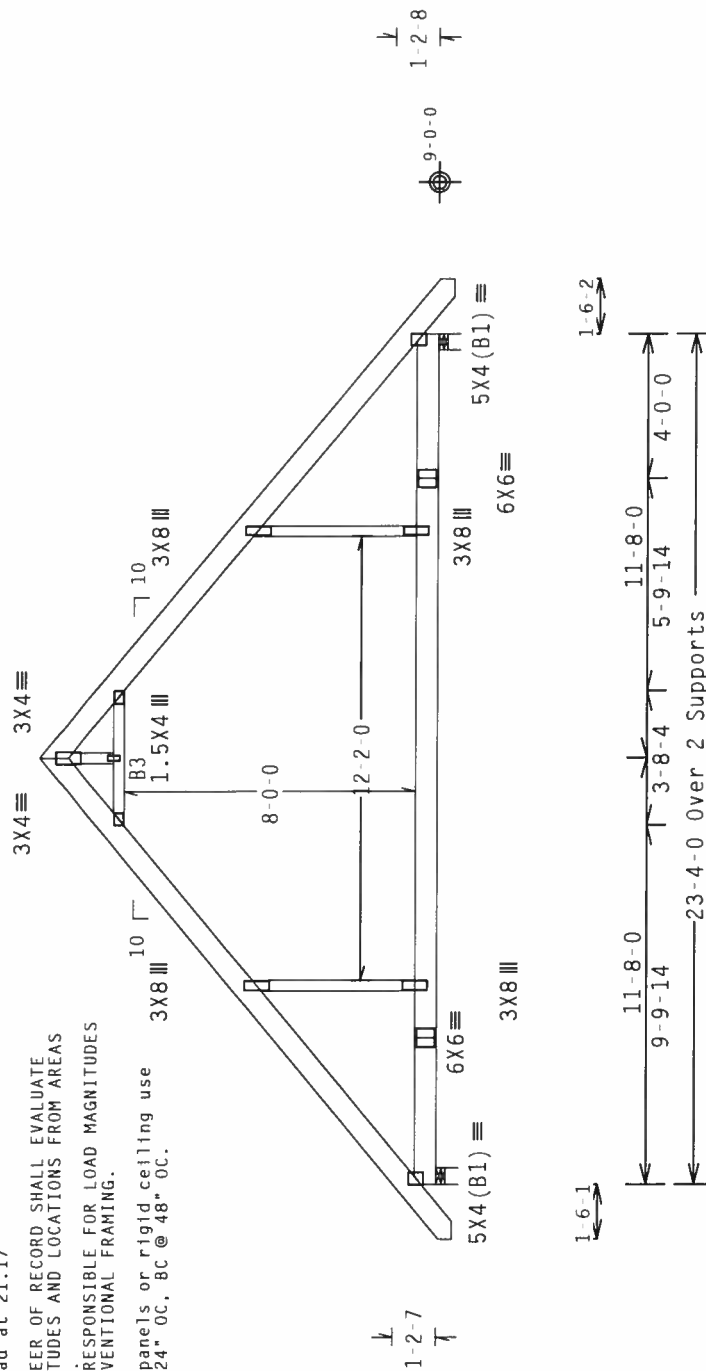
Collar-tie braced with continuous lateral bracing at 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

THE ARCHITECT OR ENGINEER OF RECORD SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS FROM AREAS FIELD FRAMED BY OTHERS.

TRUSS ENGINEER IS NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS FROM CONVENTIONAL FRAMING.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, 8C @ 48" OC.



R=6465 U=691 W=5.395"

R=4795 U=513 W=5.43"

Design Crit: $\text{TPI} - 2002(\text{STD})/\text{FBC}$
$$Ca/RT=1.00(1.25)/10(0)$$

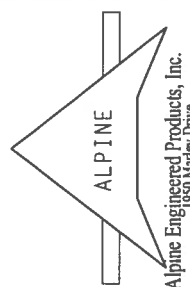
Scale = .1875"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN THE FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSSING PLATE INSTITUTE, 583 S. O'HOFER DR., SUITE 200, MADISON, WI 53719, AND METACORD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPHE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THE DESIGN REQUIREMENTS WILL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC NATIONAL DESIGN SPEC. FOR STEEL AND AISC CONNECTION PLATES ARE MADE OF 20/18/1868 (W-16X54) ASTM A663 GRAD 40/50 (F43 K155) GALV. STEEL PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, 2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AINX A3 OF 1911 2002 SEC 3 A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

TC LL	20.0	PSF	REF R487 - - 86577
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCURSR487 06041008
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 82719
DUR.FAC.	1.25		

Feb 19.06



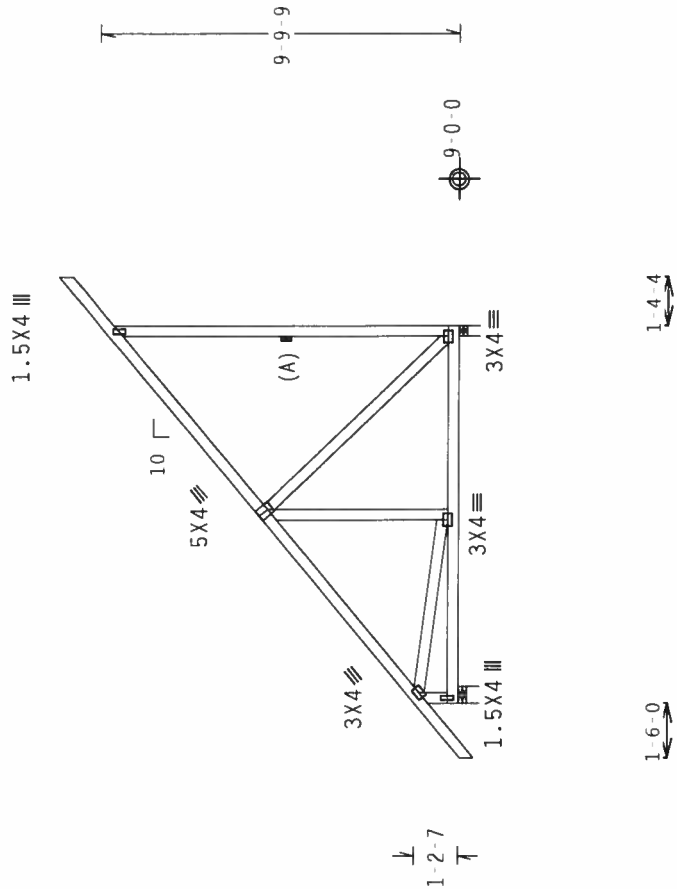
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

(A) Continuous lateral bracing equally spaced on member.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor
for dead load is 1.50.



10-3-12 over 2 Supports
R-552 U=180 W=5.5" R-531 U=180 W=3.45"

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.11

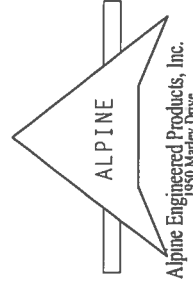
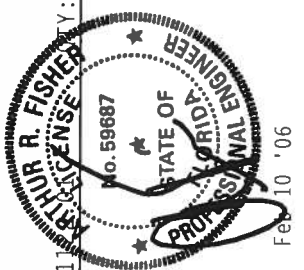
PLT TYP. Wave

Scale = .1875"/Ft.

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 583 O'DONOHIO DR., SUITE 200, MADISON, WI 53719, AND NIEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONECTOR PLATES ARE MADE OF 2018/16GA (N.I./S/S) ASTM A653 GRADE 40/60 (N. K/H-S) GALV. STEEL. APPLY AN EYE-BALL CHECK OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT

TC LL	20.0	PSF	REF	R487 --	86579
TC DL	10.0	PSF	DATE	02/10/06	
BC DL	10.0	PSF	DRW	HCUSR487	06041009
BC LL	0.0	PSF	HC-ENG	JB/AF	*
TOT.LD.	40.0	PSF	SEQN	82729	
DUR.FAC.	1.25				

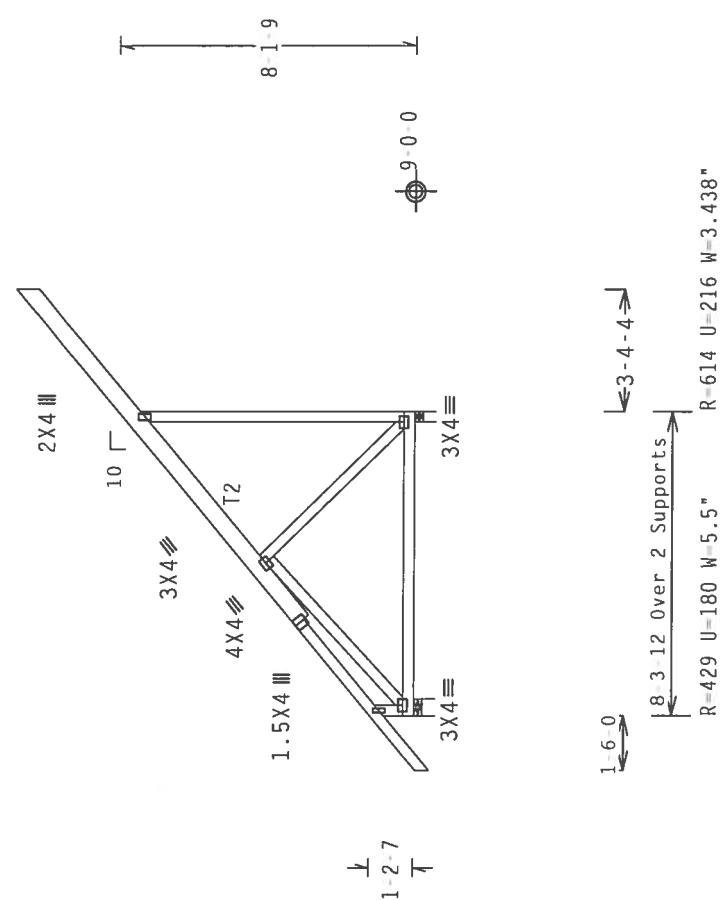


Top chord	2x4	SP #2	Dense	:T2	2x6	SP #2:
Bot chord	2x4	SP #2	Dense			
webs	2x4	SP #3				

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Right end vertical not exposed to wind pressure.



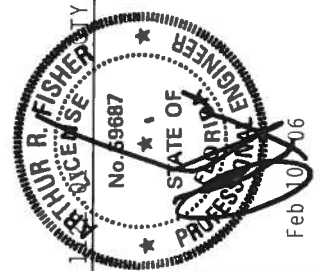
Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

 $Cq/RT=1.00(1.25)/10(0)$

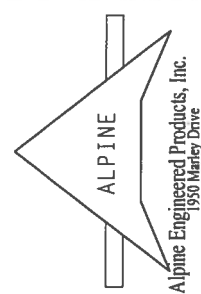
FL / - / 4 / - / R / - Scale = .1875" / Ft.

TC LL	20.0	PSF	REF R487 - - 86580
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCUSR487 06041045
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 82744
DUR.FAC.	1.25		



Feb 10 2006

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BECS 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PAPER INSTITUTE, 583 D'OONIFORD RD., SUITE 200, MADISON, WI 53719, AND WICA WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

[illegible]

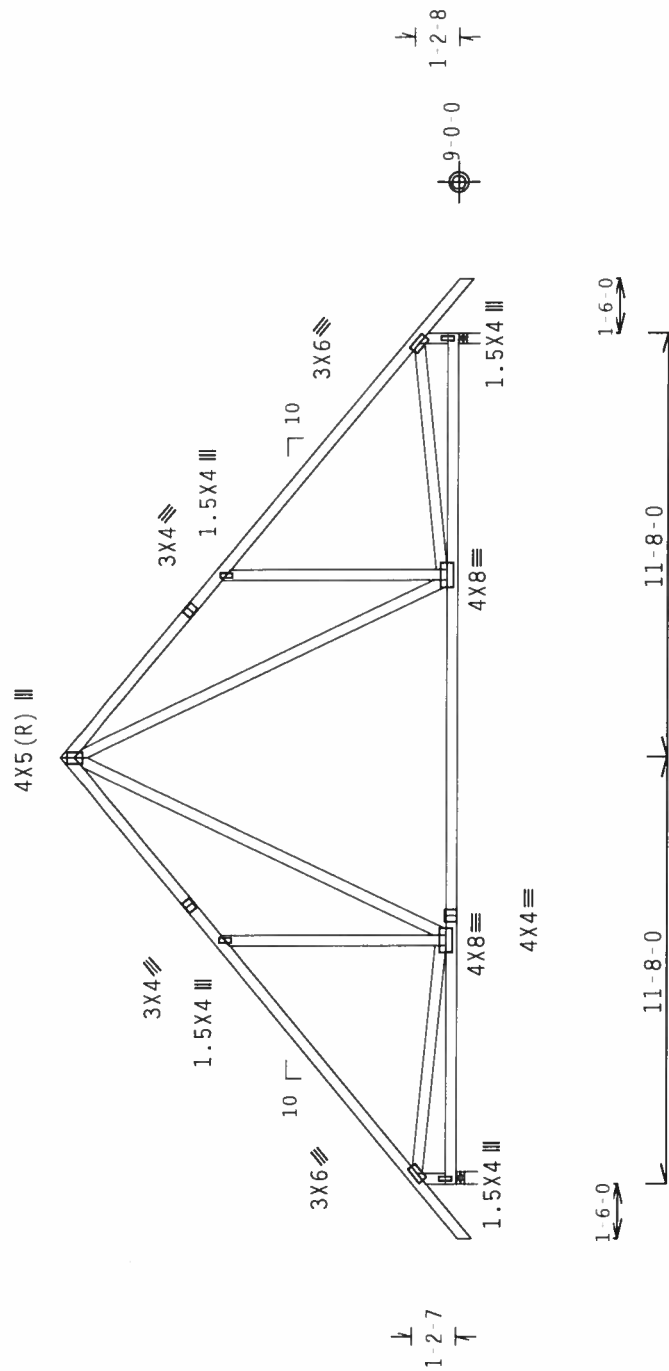
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor
for dead load is 1.50.

SPECIAL LOADS

- TC - From 66 PLF at -1.50 to 66 PLF at 24.83
- BC - From 5 PLF at -1.50 to 5 PLF at 0.00
- BC - From 20 PLF at 0.00 to 20 PLF at 23.33
- BC - From 5 PLF at 23.33 to 5 PLF at 24.83
- BC - 274 LB Conc. Load at 0.79
- BC - 190 LB Conc. Load at 3.61
- BC - 106 LB Conc. Load at 6.44



R=1612 U=180 W=4.104"
R=1178 U=180 W=3.393"

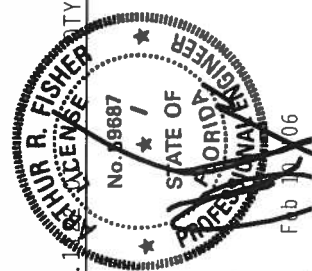
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

Scale = .1875" / Ft.

REF R487 -- 86583
DATE 02/10/06
DRW HCUSR487 06041048
HC-ENG JB/AF
SEON- 82797
DUR.FAC. 1.25

ALPINE
Alpine Engineered Products, Inc.
1000 Marlow Drive



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

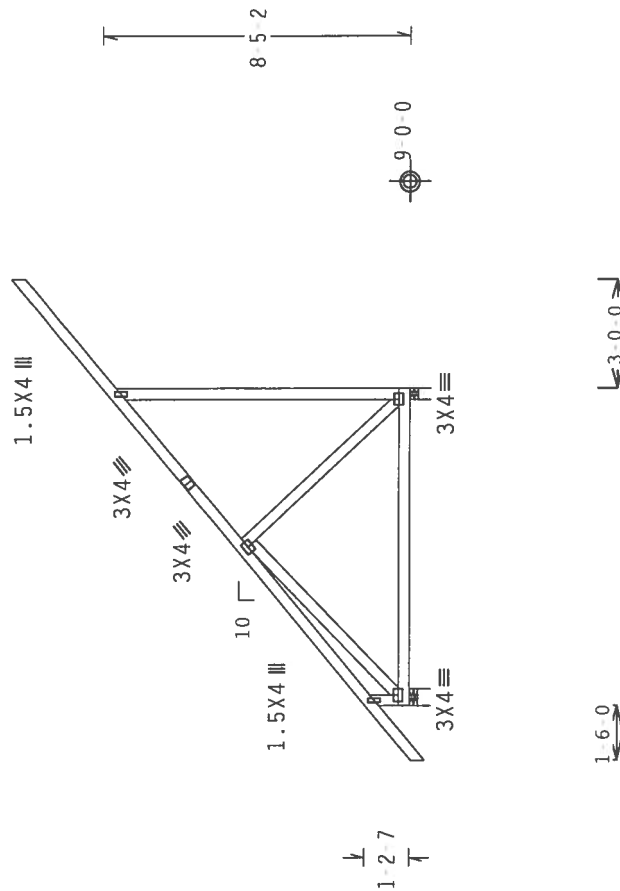
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2010/160A (4-11/16") ASTM A503 GRADE 40/60 (4-11/16") GALV. STEEL. APPLY ANY INSPECTION OF PLATES FOLLOWED BY SIGNATURE OF SEALING OFFICIALS. ANY DEVIATION FROM THESE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Right end vertical not exposed to wind pressure.

 $\leq 8-8-0$ Over 2 Supports $\rightarrow$

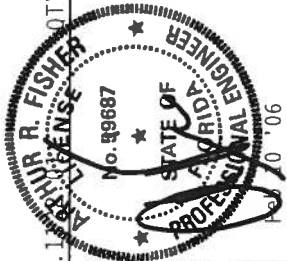
R=455 U=180 W=5.5"

 $R=596 \text{ l}=208 \text{ W}=3.5''$

Design Crit: TPI-2002(STD)/FBC

7.22.10
FL - 4 - R -
Scale = .1875" / Ft.

TC LL	20.0 PSF	REF R487 - - 86584
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUSR487 06041010
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 82819
DUR.FAC.	1.25	



Alpine Engineered Products, Inc.
1950 Maple Drive

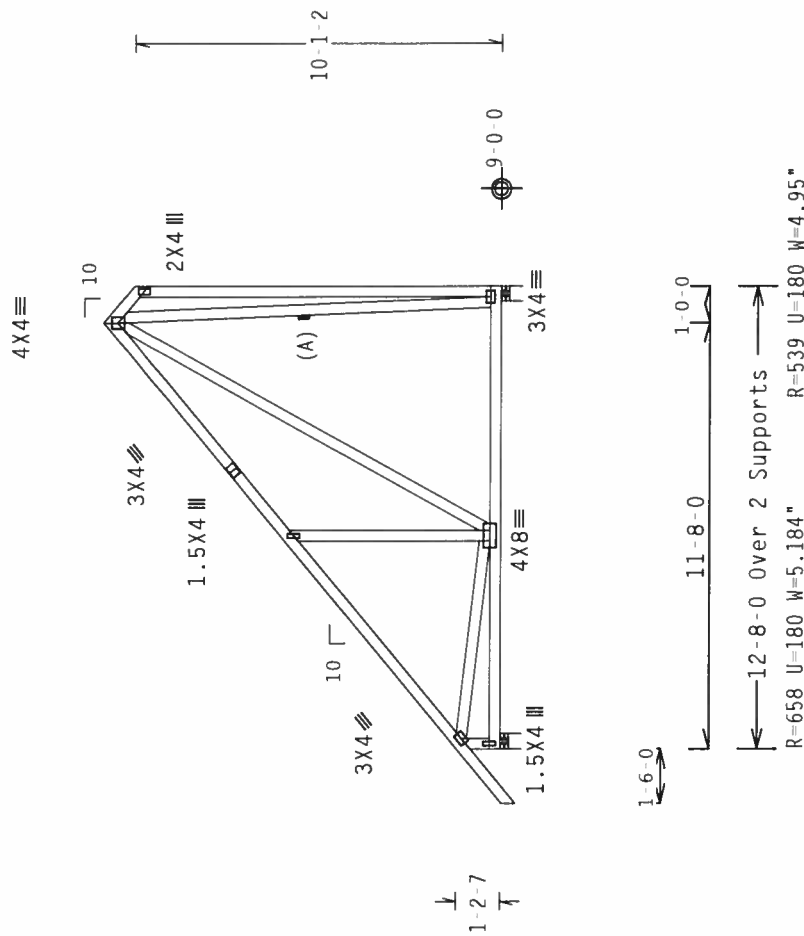
IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THE DESIGN, MANUFACTURING, AND ERECTION OF ALL TRUSS COMPONENTS IN CONFORMANCE WITH THE FOLLOWING SPECIFICATIONS. ANY FAILURE TO BUILD THESE TRUSSES IN CONFORMANCE WITH THE FOLLOWING SPECIFICATIONS SHALL BE THE RESPONSIBILITY OF THE TRUSS CONTRACTOR. THE TRUSS CONTRACTOR SHALL BE RESPONSIBLE FOR THE FABRICATING, HANDLING, SHIPPING, INSTALLING, AND BRACING OF THE TRUSSES. THE TRUSSES SHALL BE DESIGNED TO MEET THE FOLLOWING REQUIREMENTS: 1. THE TRUSSES SHALL BE DESIGNED TO MEET THE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AFPA) AND TPI (TECHNICAL PROVISIONS FOR ALPINE CONNECTOR PLATES ARE MADE OF 2018/1614 (N/A) 4053 GRADE 40/60 (N/A) 5015 GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPI 2002 SEC.3. 3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR TRUSS COMPONENT

	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

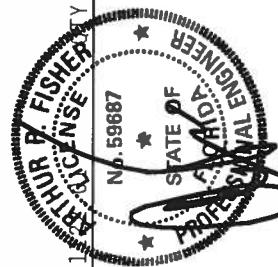
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

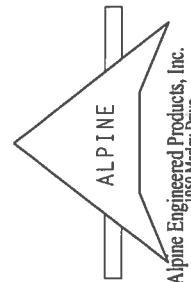
Design Crit: TPI-2002(STD)/FBC
$$Cq/RT = 1.00(1.25)/10(0)$$

Scale = .1875"/Ft.

TC LL	20.0 PSF	REF R487 - - 86586
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUSR487 06041050
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 82804
DUR.EAC.	1.25	



Feb. 06



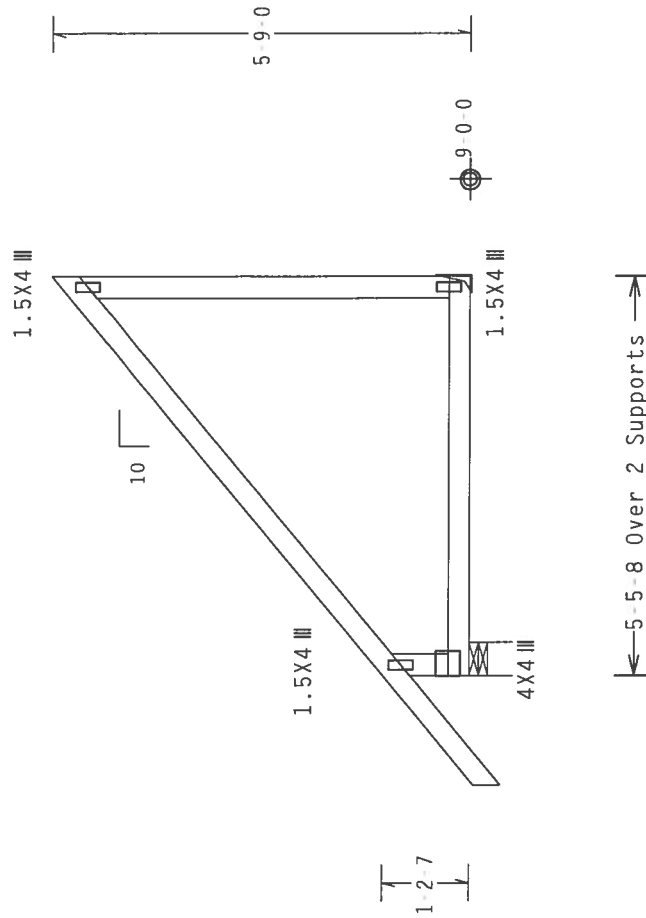
	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



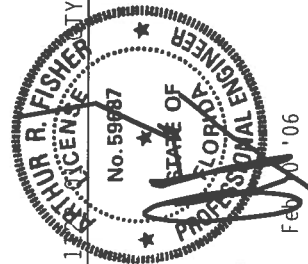
R-356 U=180 W=5.5"

R-220 U-180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

Design Crit: TPI-2002(STD)/FBC

Scale = .375"/Ft.

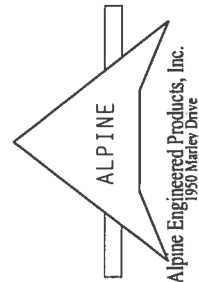
TC LL	20.0 PSF	REF R487 - 86587
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUR487 06041051
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN - 82572
DUR.EAC.	1.25	



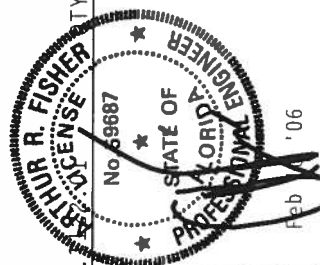
PLT TYP. Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI TRUSS PUBLICATIONS, PUBLISHED BY IPT (FIBRUS PLASTIC INSTITUTE) 563 1/2 CONIFER DR., SUITE 200, MADISON, WI 53719, AND WEBA, 100 MADISON, WI 53715, FOR SUFFICIENT PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, ALL TRUSSES MUST BE PROPERLY ATTACHED TO THE TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED TOP CHORD SHALL.

IMPORTANTURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED CONNECTIONS SHALL BE CONSIDERED AS A DESIGN DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FABRICATION AND ERECTION DRAWINGS, ALL PARTS OF THE ALPINE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY AIA) AND TPI (TECHNICAL PROVISIONS FOR THE DESIGN AND CONSTRUCTION OF STEEL TRUSS ROOFING SYSTEMS). ALL STEEL CONNECTOR PLATES ARE MADE OF 20/18/16GA (W-/J-S/E) ASH A663 GRADE 40/50 (A. K/V-I-S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A.2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT

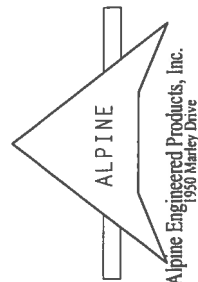


(**) Plate relocated as shown.

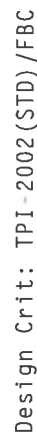


FL/-/4/-/-R/-		Scale = .3125"/Ft.
TC LL	20.0 PSF	REF R487-- 86589
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUSR487 06041053
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 82189
DUR.FAC.	1.25	

*** IMPORTANT:** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
THESE PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATIONS FROM THE
DESIGN IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES,
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC., BY AFAPA) AND TPI. ALPINE ENGINEERED
CONNECTOR PLATES ARE MADE OF 20/18/106GA (W-15) ASTM A563 GRADE 40/60 (N/A, E/H-SI) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS/UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 16GA Z
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMEHC AS OF TPI# 2002 SEC. 3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR C TRUSS COMPONENT



NOTE: IN THE CALCULATION OF DEFLECTION AT CANTILEVER END, CREEP WAS NOT CONSIDERED.

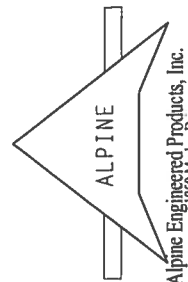
 $Cq/RT=1.00(1.25)/10(0)$

100% COTTON

Scale = .25"/Ft.

A circular professional engineer seal for the State of Florida. The outer ring contains the text "PROFESSIONAL ENGINEER" at the top and "STATE OF FLORIDA" at the bottom, separated by stars. Inside the ring, the name "R. R. FISHER" is written in a large arc, and "LICENSE" is written in a smaller arc below it. In the center, the text "No. 53687" is displayed. To the right of the center, the expiration date "Feb 17, '06" is printed. A signature is written across the bottom half of the seal.

* IMPORTANT - FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE CONTRACTOR SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE PRODUCTS, INCLUDING FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE 1989 (AISC) AND THE 1989 (AISC) STEEL CONSTRUCTION MANUAL. THE TRUSSES AND CONNECTOR PLATES ARE MADE OF 20X18/16GA (H-W)X35 45TH A463 GR40C 40/16 GA. THE TRUSSES ARE WELDED TO PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER PARAGRAPHS 16.0, 2.0 AND 3.0 OF THE 1989 (AISC) STEEL CONSTRUCTION MANUAL. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT.



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 : W1 2x6 SP #2:

(A) SP #3 or better scab brace. Same size & 80% length of web member.
Attach with 10d Box or Gun (0.128"x3", min.) nails @ 6" OC.

(B) Continuous lateral bracing equally spaced on member. Or 2x6 SP #3 or
better "T" brace. 80% length of web member. Attached with 16d Box or Gun
(0.135"x3.5", min.) nails @ 6" OC.

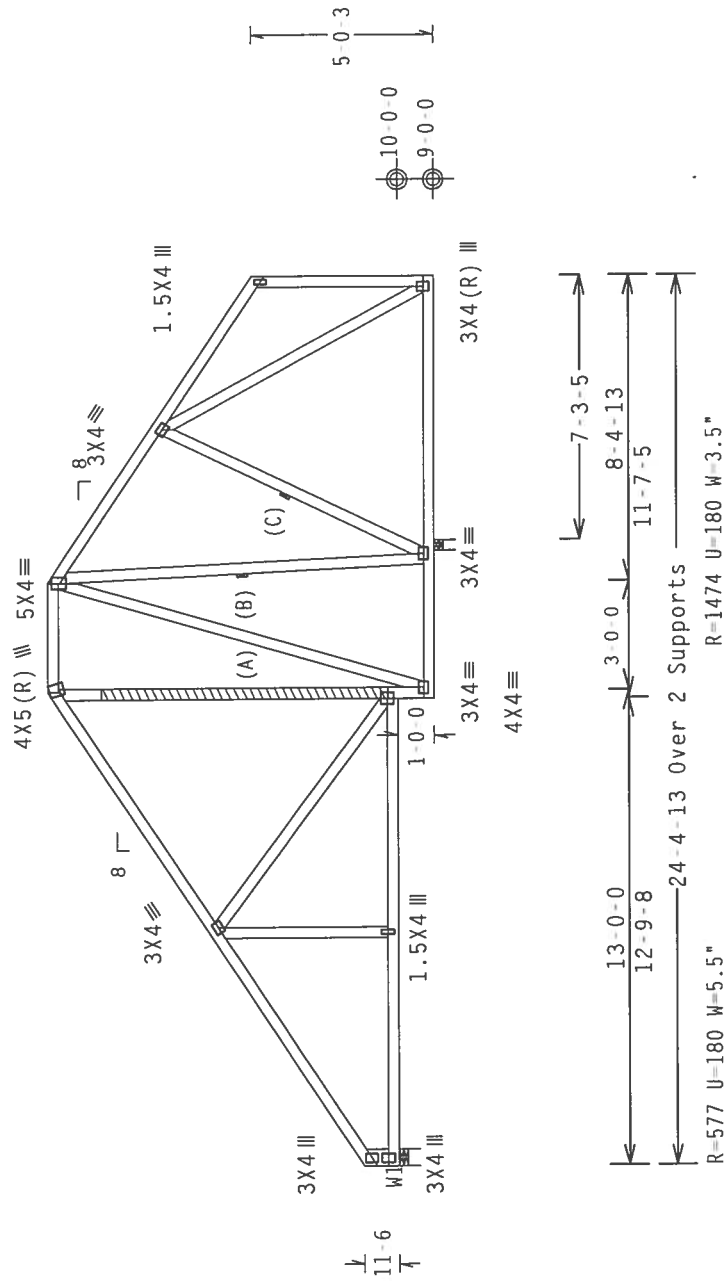
NOTE: IN THE CALCULATION OF DEFLECTION AT CANTILEVER END,
CREEP WAS NOT CONSIDERED.

110 mph wind, 15.28 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL-5.0 psf, wind
BC DL-5.0 psf.

Right end vertical not exposed to wind pressure.

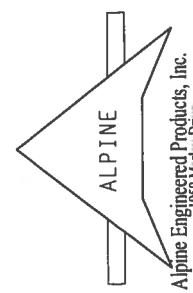
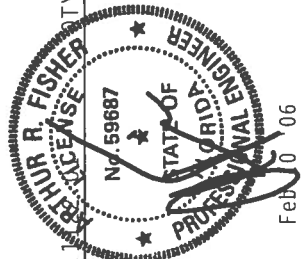
(C) Continuous lateral bracing equally spaced on member. Or 1x4 SP #3
or better "T" brace. 80% length of web member. Attached with 8d Box or
Gun (0.113"x2.5", min.) nails @ 6" OC.

Deflection meets L/360 live and L/240 total load. Creep increase
factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

TC LL	20.0 PSF	FL/-4/-/R/-	Scale = .1875"/Ft.
TC DL	10.0 PSF		REF R487 - 86591
BC DL	10.0 PSF		DATE 02/10/06
BC LL	0.0 PSF		DRW HCUSR487 06041055
TOT.LD.	40.0 PSF		HC-ENG JB/AF
DUR.FAC.	1.25		SEQN - 82236



PLT TYP. Wave

2 COMPLETE TRUSSES REQUIRED

Top chord 2x4 SP #2 Dense
Bot chord 2x8 SP SS : B2 2x6 SP #2:
Webs 2x4 SP #3 : W3, W5 2x4 SP #2 Dense:
: Lt Slider 2x6 SP #2: BLOCK LENGTH = 1.911:

SPECIAL LOADS
----- (LUMBER DUR. FAC. = 1.25 / PLATE DUR. FAC. = 1.25)
TC - From 64 PLF at 0.00 to 64 PLF at 26.24
BC - From 20 PLF at 0.00 to 20 PLF at 26.24
BC - 1402 LB Conc. Load at 0.35, 1.77, 3.77, 5.77, 7.77

110 mph wind, 15.84 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

Calculated horizontal deflection is 0.20" due to live load and 0.31" due
to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor
for dead load is 1.50.

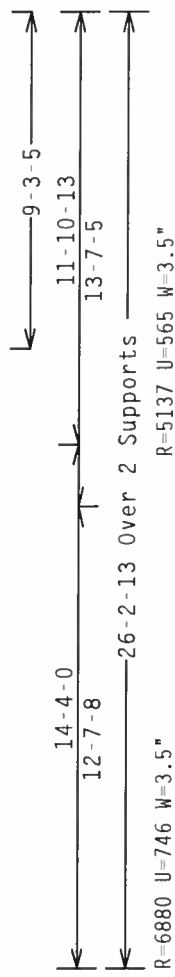
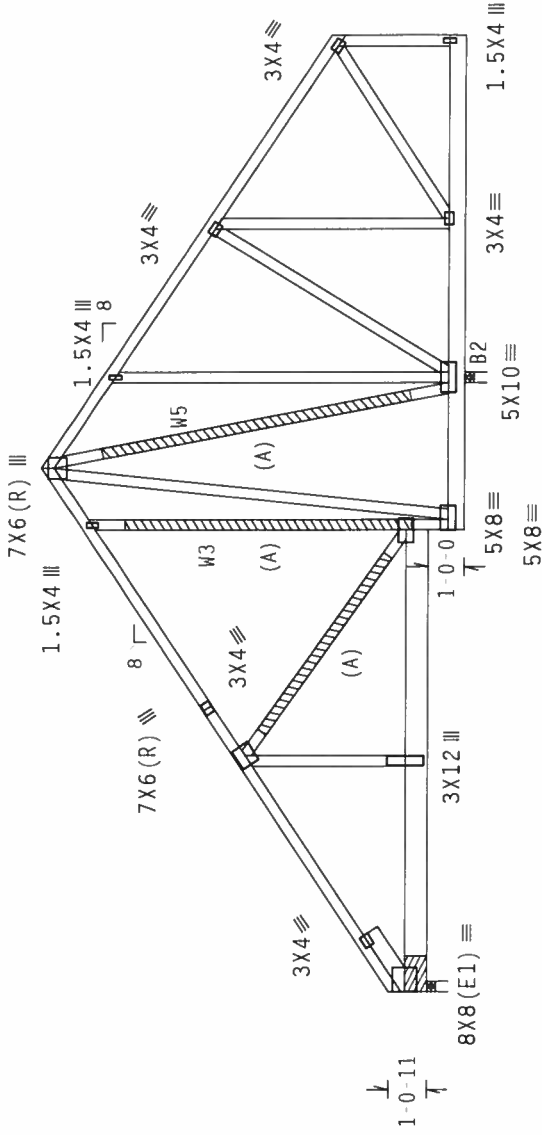
Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 1 Row @ 5.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

Bearing blocks: Nail type: 12d Common (0.148"x3.25", min.)_nails
BRG X-LOC #BLOCKS LENGTH/BLK #NAILS/BLK WALL PLATE
1 0.000' 1 12" 8 Match Truss
Bearing block to be same size and species as bottom chord.
Refer to drawing CBRGBLK1103 for additional information.

Right end vertical not exposed to wind pressure.

(A) SP #3 or better scab brace. Same size & 80% length of web member.
Attach with 10d Box or Gun (0.128"x3", min.)_nails @ 6" OC.

NOTE: IN THE CALCULATION OF DEFLECTION AT CANTILEVER END,
CREEP WAS NOT CONSIDERED.



R=6880 U=746 W=3.5"

R=5137 U=565 W=3.5"

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

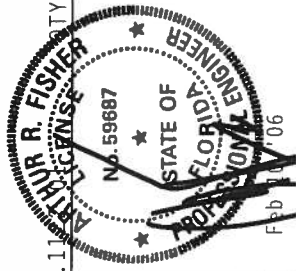
7.22.11

FL/-/4/-/R/-

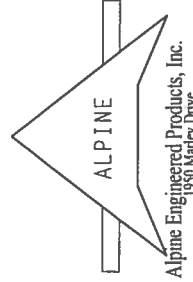
Scale = .1875"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE), 543 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719, AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTORS), 530 N. DEARBORN AVE., MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTORS), 530 N. DEARBORN AVE., MADISON, WI 53719, AND AISC (AMERICAN INSTITUTE OF STEEL CONSTRUCTORS), 530 N. DEARBORN AVE., MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED CEILING.



PLT TYP. Wave



10 004 GLENWOOD KING BULLARKU FG6J

TOP CHORD 2x4 SP #2 DENSE
BOT CHORD 2x6 SP #2
WEBS 2x4 SP #3

END VERTICALS NOT EXPOSED TO WIND PRESSURE.

GIRDER SUPPORTS 5-5-8 SPAN TO BC ONE FACE AND 6-1-0 SPAN TO TC OPPOSITE FACE.

DEFLECTION MEETS L/360 LIVE AND L/240 TOTAL LOAD. CREEP INCREASE FACTOR FOR DEAD LOAD IS 1.50.

110 MPH WIND, 15.00 FT MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP B, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

H = RECOMMENDED CONNECTION BASED ON MANUFACTURER TESTED CAPACITIES AND CALCULATIONS. CONDITIONS MAY EXIST THAT REQUIRE DIFFERENT CONNECTIONS THAN INDICATED. REFER TO MANUFACTURER PUBLICATION FOR ADDITIONAL INFORMATION.

TRUSS MUST BE INSTALLED AS SHOWN WITH TOP CHORD UP.

THE TC OF THIS TRUSS SHALL BE BRACED WITH ATTACHED SPANS AT 24" OC IN LIEU OF STRUCTURAL SHEATHING.

1.5X4 III

3X4 III

5-9-0

5-9-0

1.5X4 III

3X4 III

1.5X4 III

9-0-0

←5-3-0 OVER 2 SUPPORTS→

R-619 U=180 H=Simpson LUS24
W/ (2) 10d Common, 0.148"x3.0" NAILS IN TRUSS
GIRDER IS (1)2X4 MIN. SO.PINE

R-619 U=180 H=Simpson LU26
W/ (4) 10d, 0.148"x1.5" NAILS IN TRUSS
W/ (6) 10d Common, 0.148"x3.0" NAILS IN GIRDER
GIRDER IS (1)2X6 MIN. SO.PINE

PLT TYP. WAVE

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.11

Scale = .375"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

ARTHUR R. FISHER
No. 59887
STATE OF FLORIDA
PROFESSIONAL ENGINEER
Feb 10 '06

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED STRUCTURAL PANELS OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED RIGID CEILING, SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED STRUCTURAL PANELS OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED RIGID CEILING, SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED STRUCTURAL PANELS OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED RIGID CEILING, SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR.

10-004- GLENWOOD KING BULLAKU FGCU

TOP CHORD 2x4 SP #2 DENSE
BOT CHORD 2x4 SP #2 DENSE
WEBS 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Girder supports 2-2-0 span to BC one face and 2-0-0 span to TC/BC split opposite face.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

Trusses or components connecting to this girder have been modified by the truss designer. The loading for this girder requires verification for accuracy.

End verticals not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Provide for complete drainage of roof.

Truss must be installed as shown with top chord up.

1.5X4 III

3X4 III

3X4 III

1.5X4 III

3-0-2

3-0-2

9-0-0

9-0-0

5-3-0 Over 2 Supports

R=219 U-180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X4 min. So.Pine

R=219 U-180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.11

Scale = .5" / Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

LEATHUR R. FISHER
No. 59887
STATE OF FLORIDA
PROFESSIONAL ENGINEER

Feb 10 '06

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25

REF R487 -- 86595
DATE 02/10/06
DRW HCUSR487 06041059
HC-ENG JB/AF
SEON - 82701
FROM JP

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense
webs	2x4	SP	#3	

End verticals not exposed to wind pressure.

Girder supports 2-4-8 span to BC one face and 2-0-0 span to TC/BC split opposite face.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

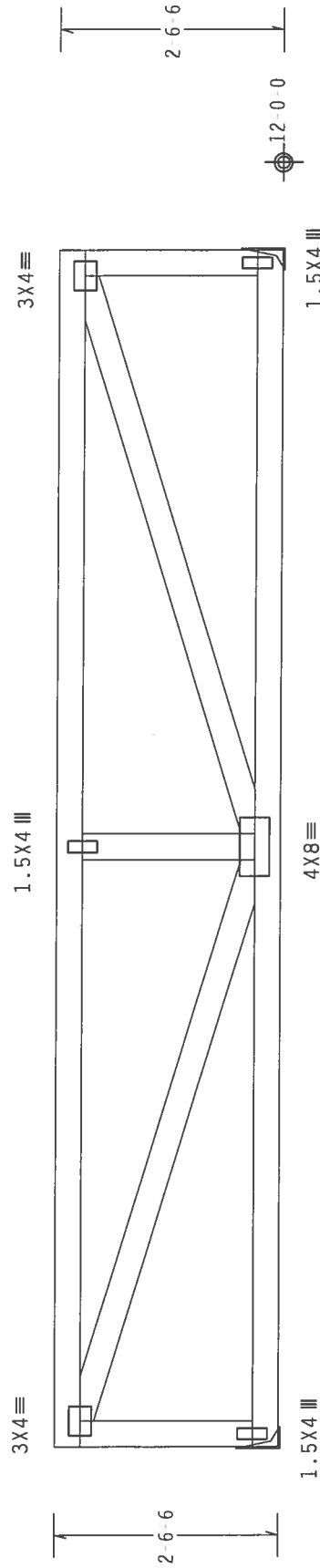
The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

110 mph wind, 15.00 ft mean hgt, ASCE 7 02, CLOSED bldg, Located anywhere in roof, CAT I, EXP C, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Provide for complete drainage of roof.

Truss must be installed as shown with top chord up.



-13-6-0 Over 2 Supports

R=579 U-180 H=Simpson LU26
w/ (4) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

R-579 U-180 H-Simpson LU26
w/ (4) 10d, 0.148"x1.5" nails in Truss
w/ (6) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1.25) 6x11in. So Pine

Design Crit: TPI-2002(STD)/FBC

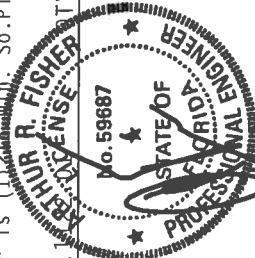
 $Cq/RT=1.00(1.25)/10(0)$

7.22.14

PROPERTY: 1 EL/14/1-1R/1-

Scale = 5" / Ft.

TC LL	20.0	PSF	REF	R487 --	86596
TC DL	10.0	PSF	DATE	02/10/06	
BC DL	10.0	PSF	DRW	HCUSR487	06041060
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	82052	
DUR.FAC.	1.25				



Alpine Engineered Products, Inc.
1050 Marlow Drive

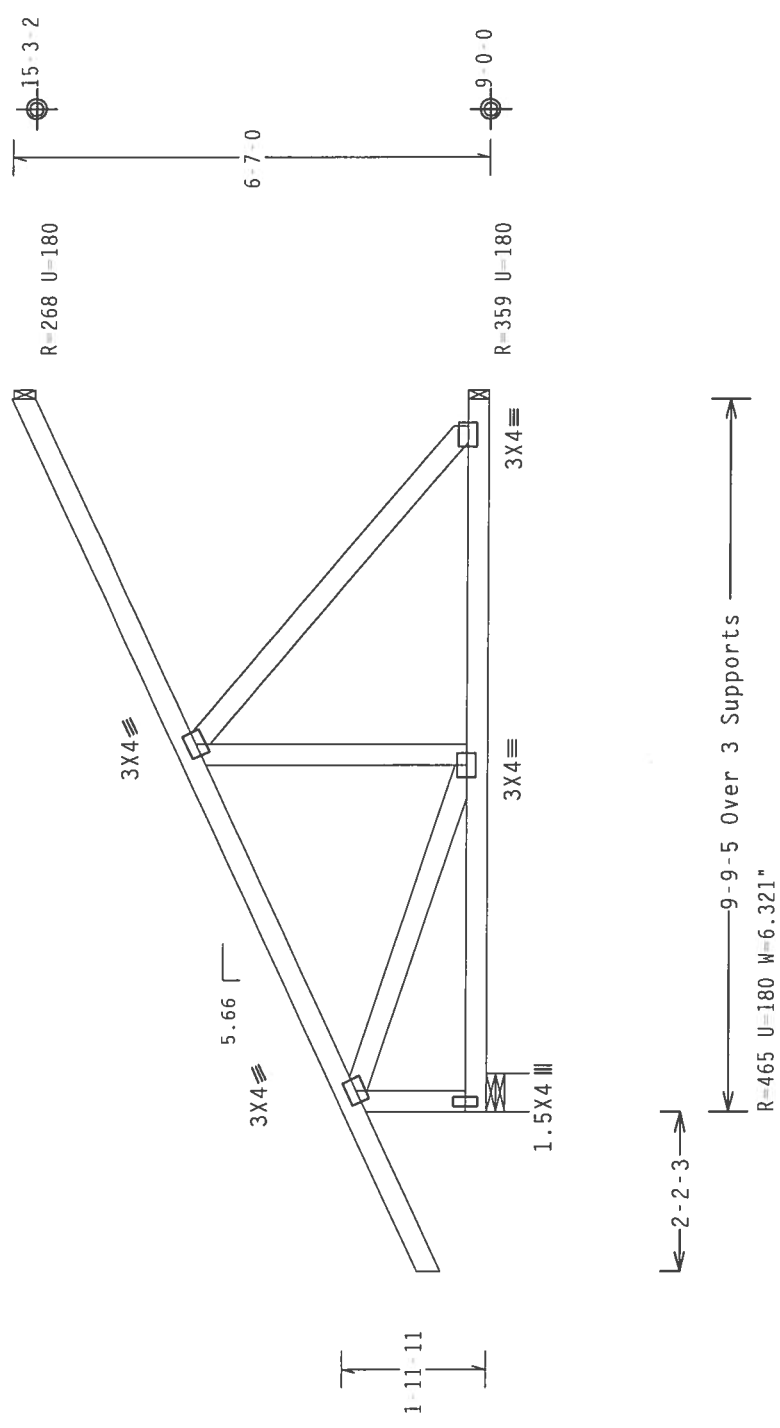
	Top	Bot	Dense
#1	chord	chord	SP
#2	chord	chord	SP
#3	webs	webs	SP

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 7-0-0 setback jacks with no webs.

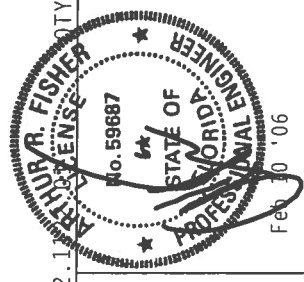
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (3) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



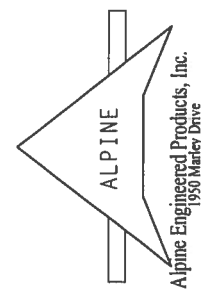
Design Crit: TPI-2002(STD)/FBC

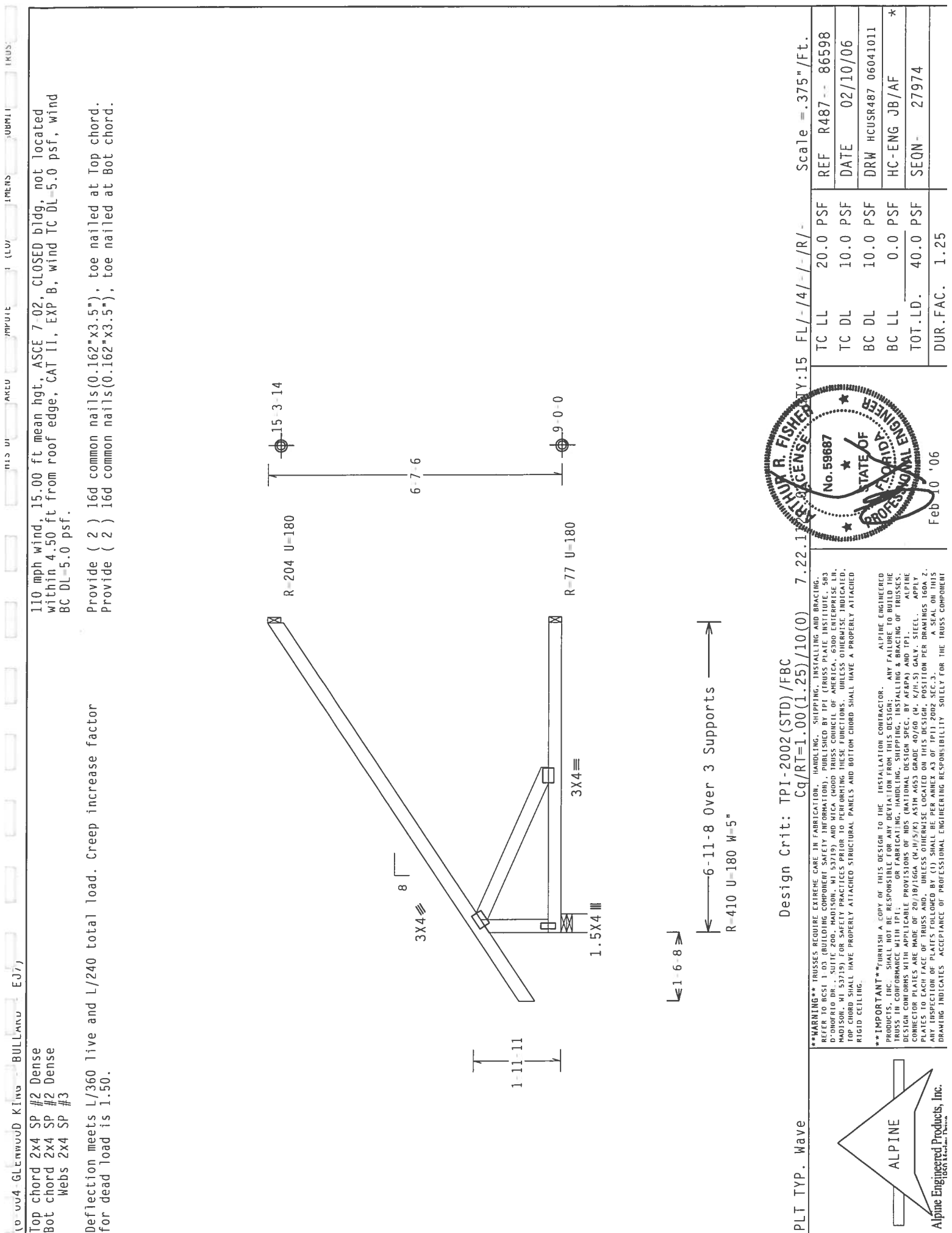
FL / - / 4 / - / R / -		Scale = .375" / Ft.
TC LL	20.0 PSF	REF R487 - 86597
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUSR487 06041061
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 28045
DUR.FAC.	1.25	



*****WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BGS 1 03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY IPTI TRUSS PACT INSTITUTE, 583 D'ORFIO DRIVE, SUITE 200, MADISON, WI 53719) AND WICA CHORD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED, RIGID CEILING.**

****IMPORTANT****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
THESE TRUSS SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO THESE SPECIFICATIONS WILL BE AT THE RISK OF THE USER. THE FOLLOWING LISTING OF THE TRUSS DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC.) BY ALUMINUM ASSOCIATION. CONNECTOR PLATES ARE MADE OF 20/18/16GA (.015X) ASH A653 GRADE 40/50 (M, K/H/S) GALV. STEEL. ALL PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, PER DRAWINGS 100A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF IP11 2002 SEC.3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT





110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0)

7.22.11

Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 --	86598
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUSR487	06041011
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	40.0 PSF	SEQN-	27974	
DUR.FAC.		1.25		

ALPINE

Engineered Products, Inc.

ATTORNEY R. FISHER

STATE OF FLORIDA

PROFESSIONAL ENGINEER

No. 59687

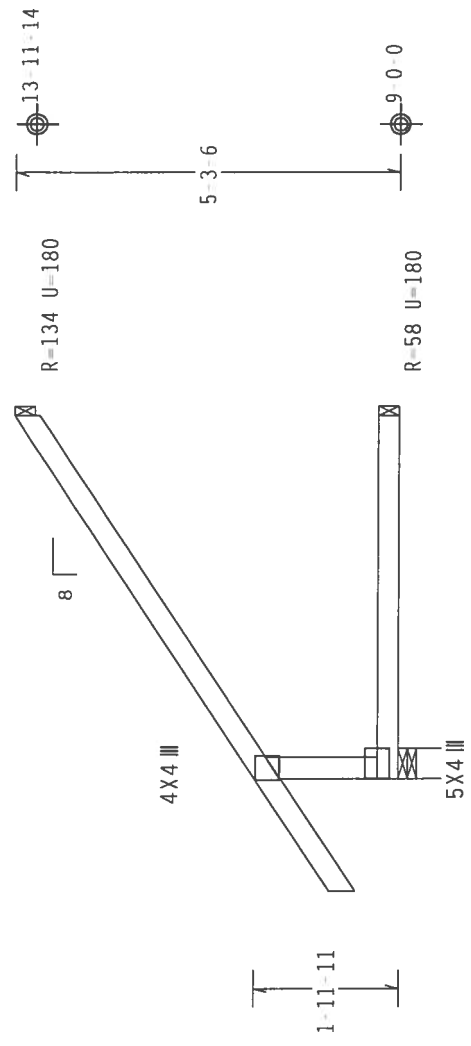
Feb 10 '06

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#2	Dense

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



1687

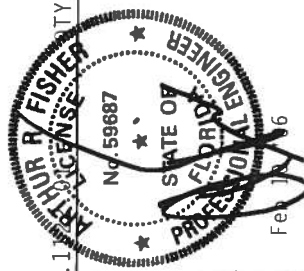
4-11-8 Over 3 Supports
R=331 U=180 W=5"

Design Crit: TPI-2002(STD)/FBC

$\text{FL} / - / 4 / - / - / \text{R} / -$

$\text{Scale} = .375''/\text{ft.}$

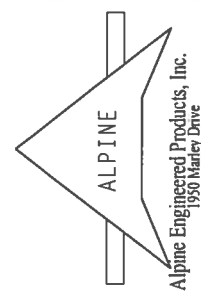
TC LL	20.0 PSF	REF R487 -- 86599
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUSR487 06041012
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 28005
DUR.FAC.	1.25	



Feb 18 1966

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BECS 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PAPER INSTITUTE, 583 D'OROFF RD., SUITE 200, MADISON, WI 53719), AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED, RIGID CEILING.

* IMPORTANT *—TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. AIRLINE ENGINEERED TRUSSES, INC., HAS RESPONSIBILITY FOR THE DESIGN AND CONSTRUCTION OF ALL TRUSSES. ANY FABRICATING OR DRUING IN CONFORMANCE WITH TPI: PROVISIONS OF NDS NATIONAL DESIGN SPEC. BY AFAPA AND TPI. THE DESIGN CONFORMS WITH APPLICABLE REQUIREMENTS OF AISC 880-90 STEEL EDITION 1990. ALL TRUSSES ARE MADE OF 20/18/16GAL (.015/.015/.015) ASTM A663 GRADE 40 (N. K/J-H/S) GALV. STEEL. PLATES TO EACH FACE OF BRUSS AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. TRUSSES SHALL BE DESIGNED TO SUPPORT THE WEIGHT OF THE TRUSS AND THE WEIGHT OF THE TRUSS COMPONENTS. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS. SEE DETAILING FOR THE USE OF THE ENGINEERING AND THE MATERIALS. THE MATERIALS SHALL BE AS SHOWN.

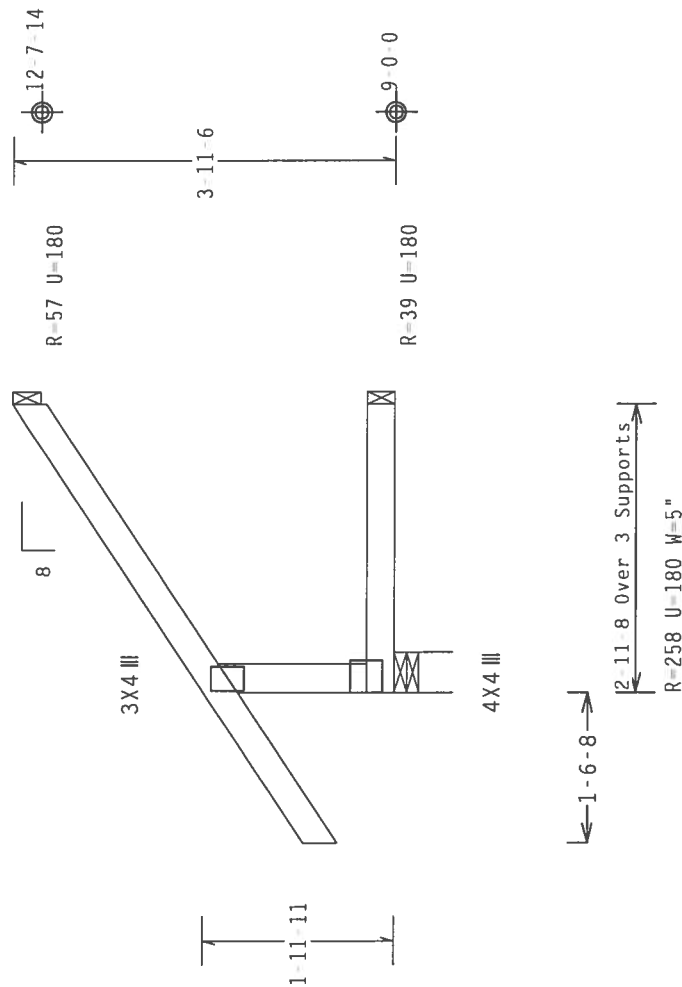


	#1	#2	Dense
Top chord	2x4	SP	Dense
Bot chord	2x4	SP	Dense
Webs	2x4	SP	Dense

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



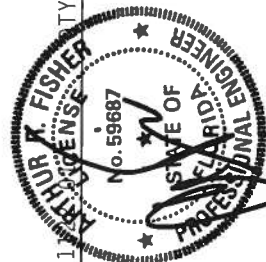
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

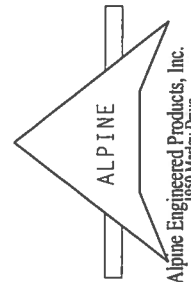
$$C_0/RT = 1.00(1.25)/10(0)$$

Scale = .5" / Ft.

TC LL	20.0 PSF	REF R487-- 86600
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUR487 06041013
BC LL	0.0 PSF	HC-ENG JB/AF *
TOT.LD.	40.0 PSF	SEQN- 28012
DUR.EAC.	1.25	



Feb 10, '06

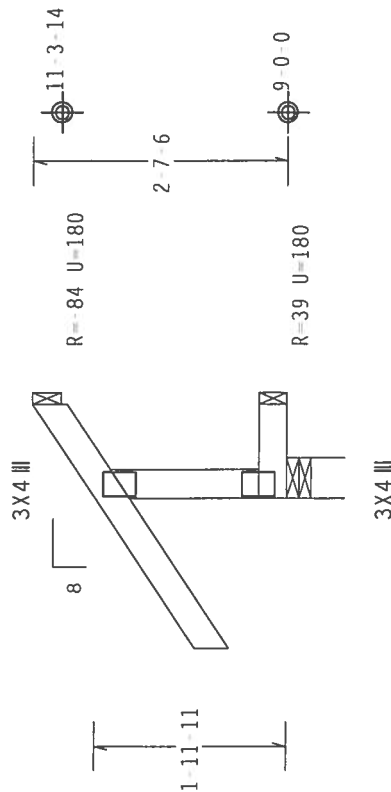


	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



$\leftarrow 1-6-8 \rightarrow$
 0-11-8 Over 3 Supports
 $\leftarrow 1-6-8 \rightarrow$
 $R=232 \quad U=180 \quad W=5$

Design Crit: TPI-2002(STD)/FBC

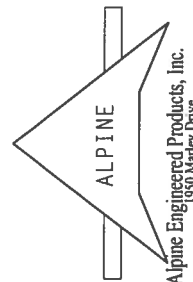
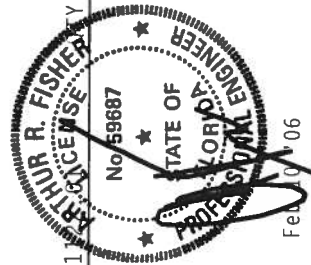
PLT TYP. Wave

$$Ca/RT=1.00(1.25)/10(0) \quad 7.22 \cdot 10^3$$

RTTB SHEET 4 EL - A / - IR / -

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R487-- 86601
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUR487 06041062
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 28019
DUR.FAC.	1.25	



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 101 (BUILDING CONSTRUCTION) PUBLISHED BY THE PRESS PAPER INSTITUTE, 583 D'ORFORD DR., SUITE 200, MADISON, WI 53719, AND VTC (VERTICAL TRANSPORTATION) PUBLISHED BY THE PRESS PAPER INSTITUTE, 583 D'ORFORD DR., SUITE 200, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE SPECIFIED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

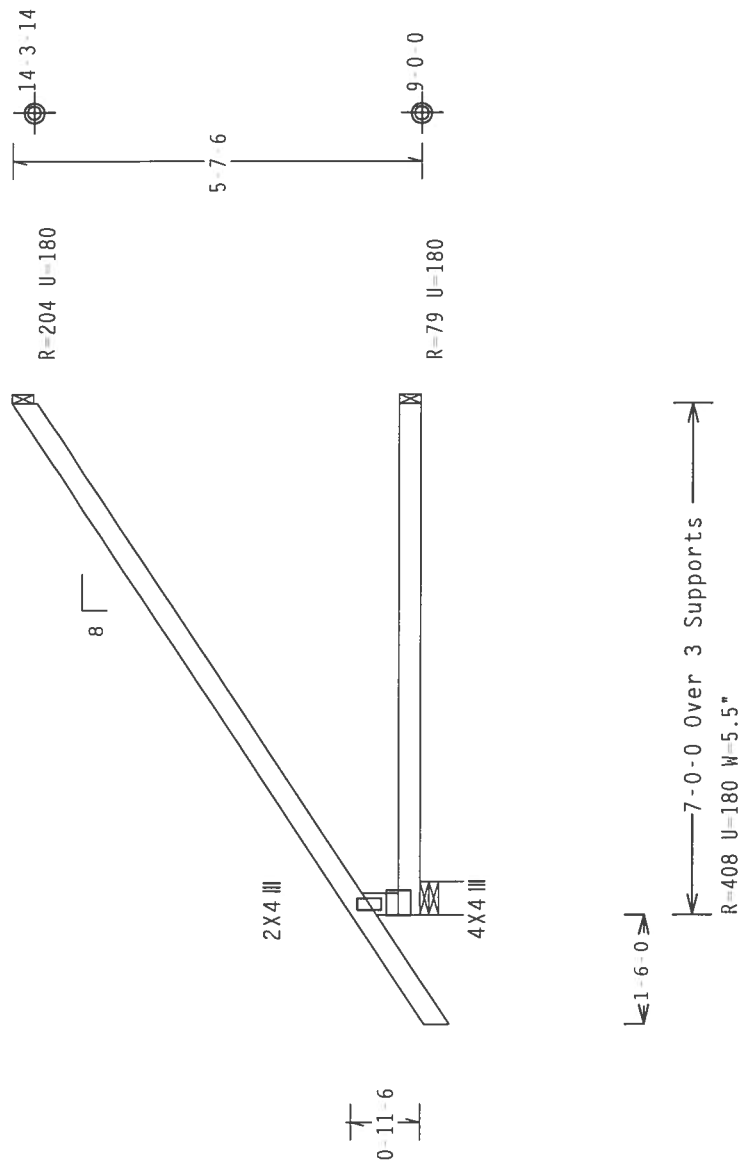
IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING PROVISIONS OF THIS DESIGN, SHALL BE THE RESPONSIBILITY OF THE DESIGNER. THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., IN KENPA AND NDS) AND THE CONNECTOR PLATES ARE MADE OF 2010/1816 (M 2X1/8) ASTM A653 GR50 40Y/60 (4 K/1/8) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TP11 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

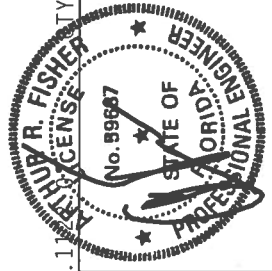
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



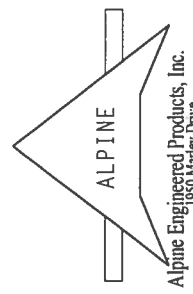
Design Crit: TPI-2002(STD)/FBC

7.22.11
TY: 16 FL / - 4 / - - R / -
Scale = .375" / Ft.

TC LL	20.0	PSF	REF R487 - 86604
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCUSR487 06041014
BC LL	0.0	PSF	HC-ENG JB/AF *
TOT.LD.	40.0	PSF	SEQN 82875
DUR.FAC.	1.25		



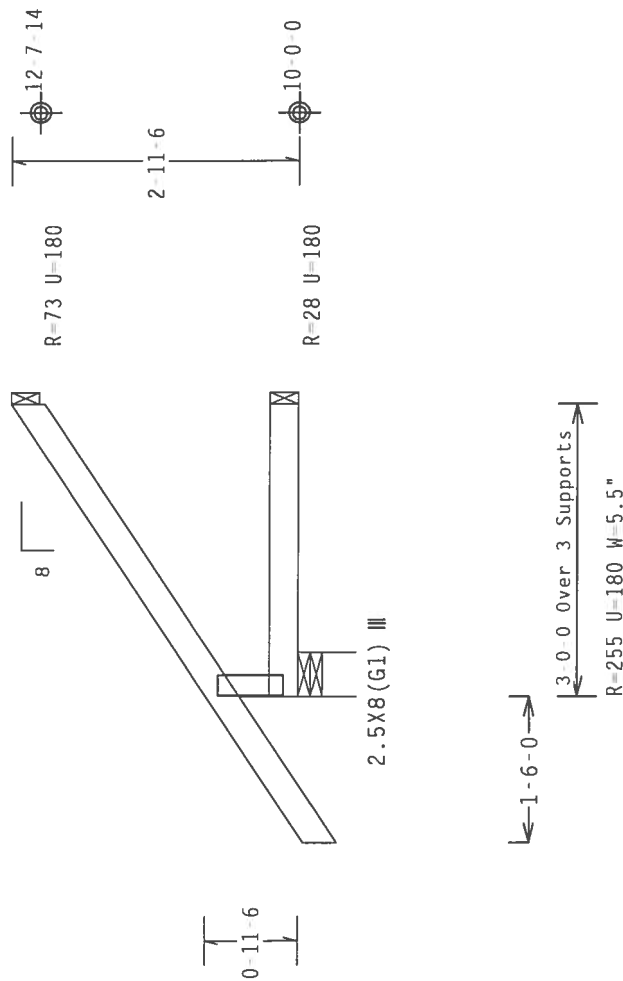
Feb 10 '06



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
:LT Stubbed Wedge 2x6 SP #2:
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located
anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0
psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

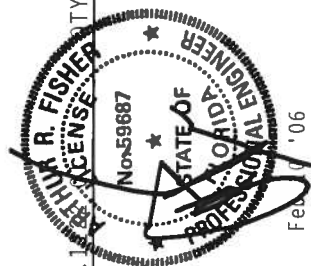
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



Design Crit: TPI-2002(STD)/FBC

Scale = .5"/Ft.

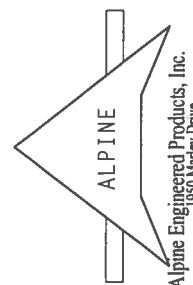
TC LL	20.0	PSF	REF R487-- 86606
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCUSR487 06041015
BC LL	0.0	PSF	HC-ENG JB/AF *
TOT.LD.	40.0	PSF	SEQN- 82136
DUR.FAC.	1.25		



Feb 07, '06

WARNING—TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO PCSI TRUSS BUILDING MANUAL FOR SAFETY PRECAUTIONS OF STEEL DECK PLATE INSTITUTE, 383 WEST O'DONNELL RD., SUITE 200, MADISON, WI 53719, AND WEA (WOOD WORKS COUNCIL OF AMERICA), 600 N. LAKE ST., SUITE 200, MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED CEILING.

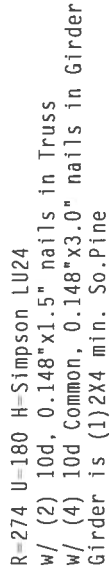
IMPORTANT-TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE PRODUCTS IN ACCORDANCE WITH THE MANUFACTURING, INSTALLING AND BRACING OF TRUSSES. THE FOLLOWING PROVISIONS OF THE NATIONAL DESIGN SPECIFICATION FOR STEEL BUILDINGS SHALL APPLY TO ALL TRUSS CONNECTOR PLATES ARE MADE OF 70/18/16GA. 14W/25/5/5 ASTM A663 GRADE 40/60 (H. K/15) GALV. STEEL. APPLICABLE TO ALL TRUSS PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, INSTALLATION PERMANENT 1604.2. ALL TRUSS PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, PERMANENT 1604.2. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT.



H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

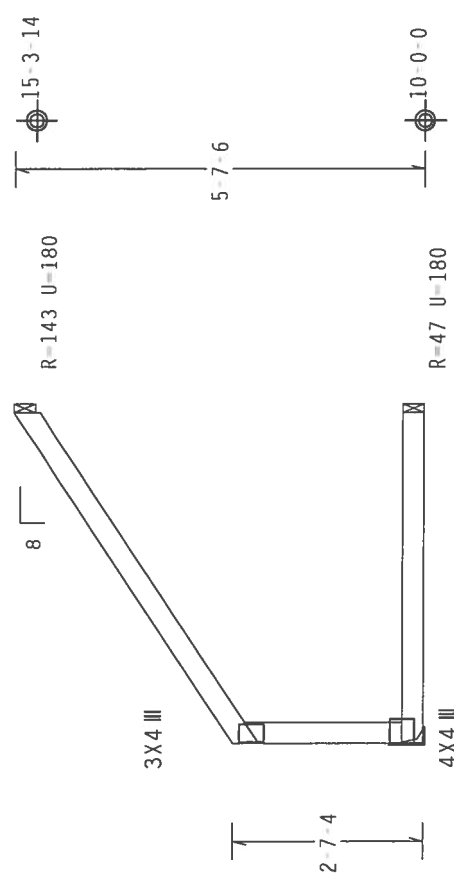
Left end vertical not exposed to wind pressure.

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Calculated horizontal deflection is 0.26" due to live load and 0.17" due to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



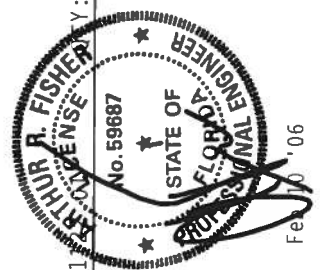
4-6-3 Over 3 Supports

R-190 U-180 H-Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X4 min. So.Pine

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

PLT TYP. Wave

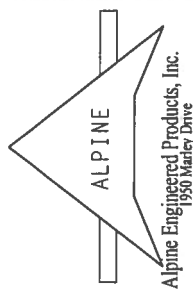
Scale = .375"/Ft.



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DECKING, CONNECTIONS, OR PANELS, PROVIDING OR USING UNDESIGNED OR UNTESTED ALPINE CONNECTOR PLATES ARE MADE ON OR 20/18/16GSS UNLESS OTHERWISE INDICATED. ALPINE SHALL BE RESPONSIBLE FOR ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN ONLY.

TC LL	20.0 PSF	REF	R487 --	86609
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUSR487	06041017
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	40.0 PSF	SEON-	82115	
DUR.FAC.	1.25			



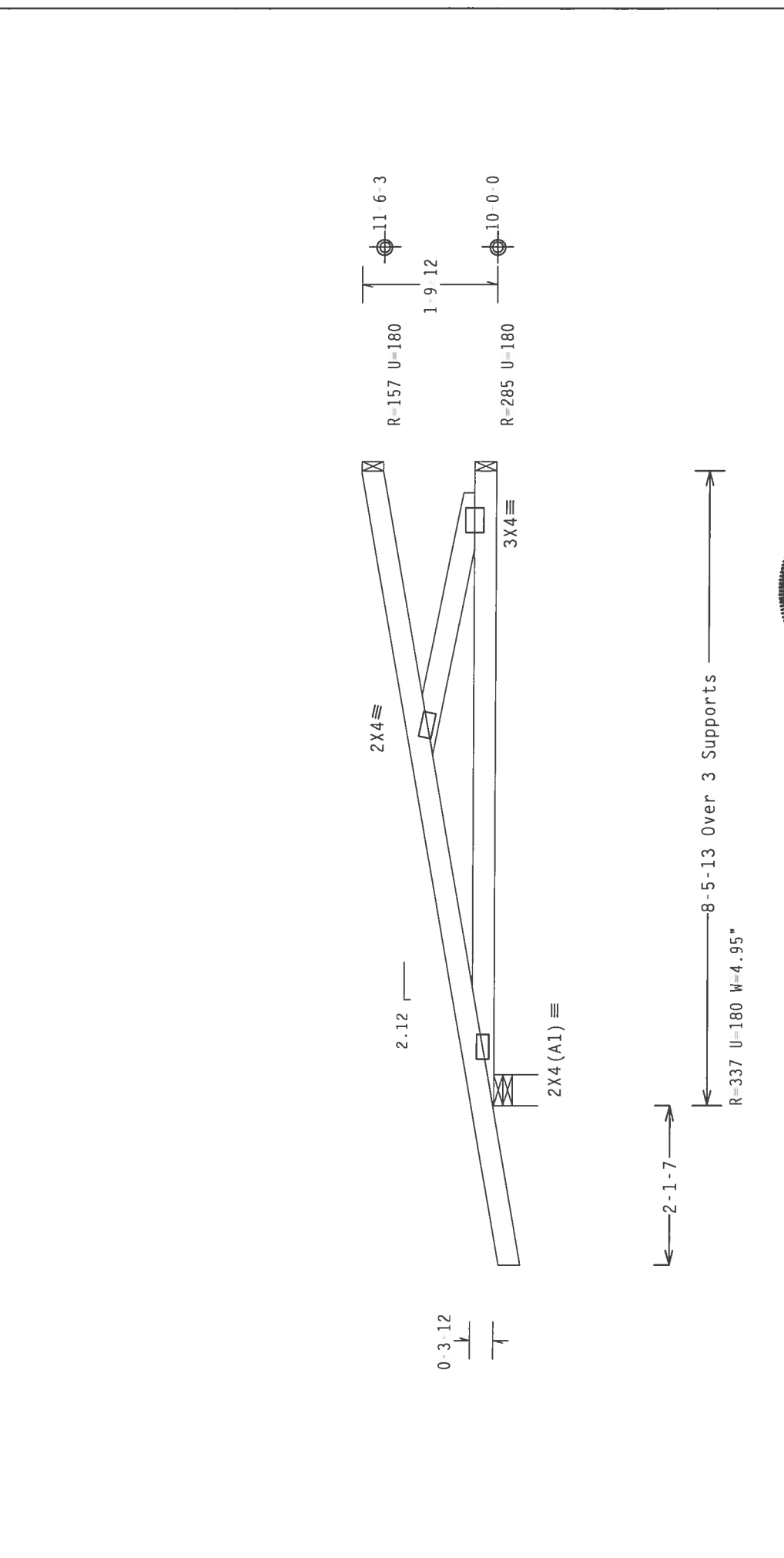
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Hipjack supports 6-0-0 setback jacks with no webs.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

Scale = .5"/Ft.

TC LL	20.0 PSF	REF	R487 --	86611
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUSR487	06041067
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEQN	28098	
DUR.FAC.	1.25			

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

Professional Engineer
R. FISHER
No. 59687
STATE OF ALABAMA
Professional Engineer
Feb 10 '06

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

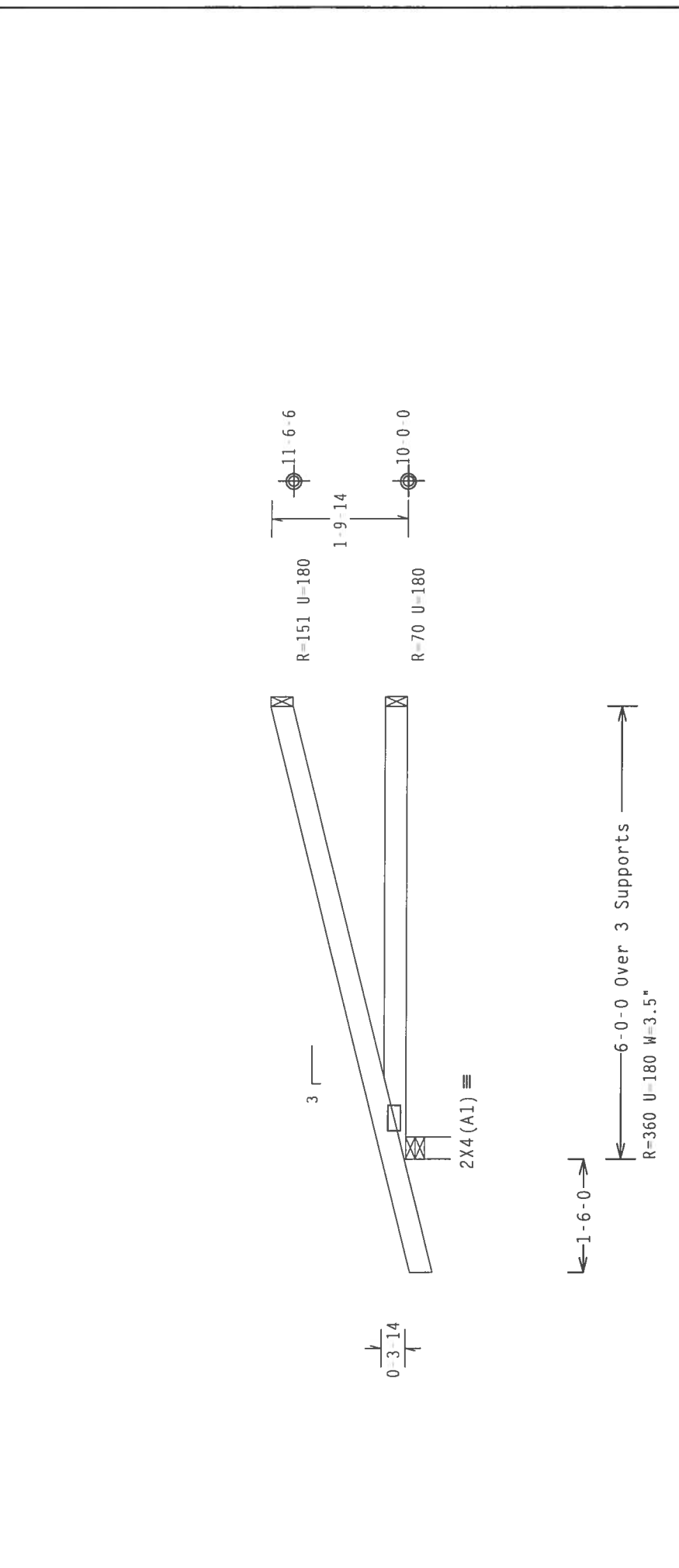
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS TO CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED PANELS OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED RIGID CEILING SHALL BE THE RESPONSIBILITY OF THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY FAILURE TO BUILD THE TRUSS TO CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED PANELS OR TO PROVIDE THE TRUSS WITH PROPERLY ATTACHED RIGID CEILING.

ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC 3.3. POSTION PER DRAWING INDICATES. ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

Scale = .5" / Ft.

TC LL	20.0 PSF	REF	R487 -- 86612
TC DL	10.0 PSF	DATE	02/10/06
BC DL	10.0 PSF	DRW	HCUSR487 06041068
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	28076
DUR.FAC.	1.25		

ARTHUR R. FISHER
No. 59887
STATE OF
PROFESSIONAL ENGINEER
Feb 11 '06

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive

ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

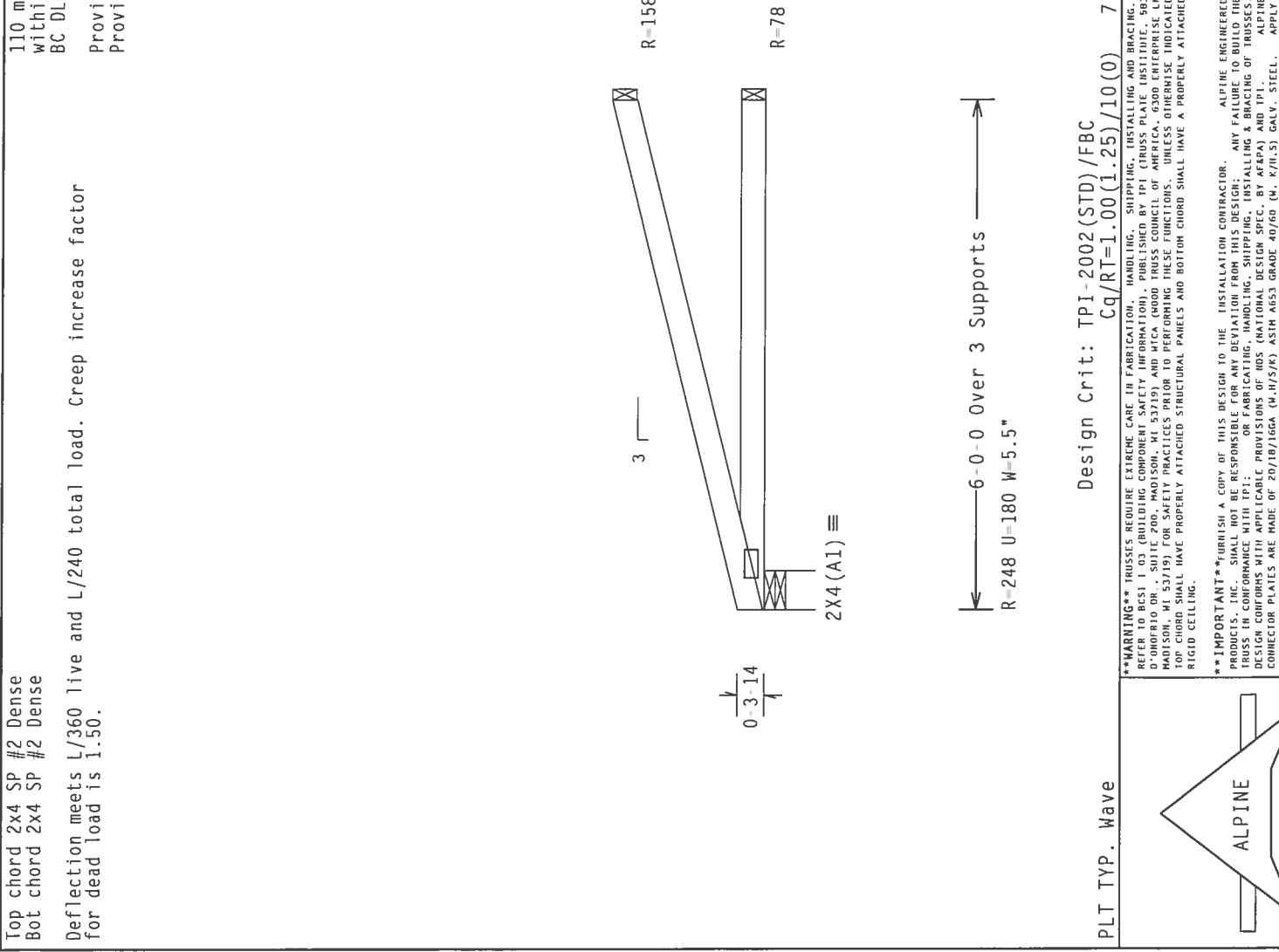
IMPORTANT: FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI-2002 (BUILDING COMPONENT SAFETY INFORMATION) - PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND NCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL 5.0 psf, wind BC DL=5.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

Scale = .5"/Ft.

TC LL 20.0 PSF
TC DL 10.0 PSF
BC DL 10.0 PSF
BC LL 0.0 PSF
TOT.LD. 40.0 PSF
DUR.FAC. 1.25

REF R487 -- 86613
DATE 02/10/06
DRW HCURS487 06041069
HC-ENG JB/AF
SEQN 28073

ALPINE
ENGINEERED
PRODUCTS, INC.
1950 Marlev Drive

ALPINE

ALPINE ENGINEERED PRODUCTS, INC.
SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS, NATIONAL DESIGN SPEC. BY AISC, AND TPI. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS AND THE INSTALLATION PER DRAWING. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

ALPINE ENGINEERED PRODUCTS, INC.
1950 Marlev Drive

ALPINE ENGINEERED PRODUCTS, INC.
1950 Marlev Drive

ALPINE ENGINEERED PRODUCTS, INC.
1950 Marlev Drive

ALPINE ENGINEERED PRODUCTS, INC.
1950 Marlev Drive

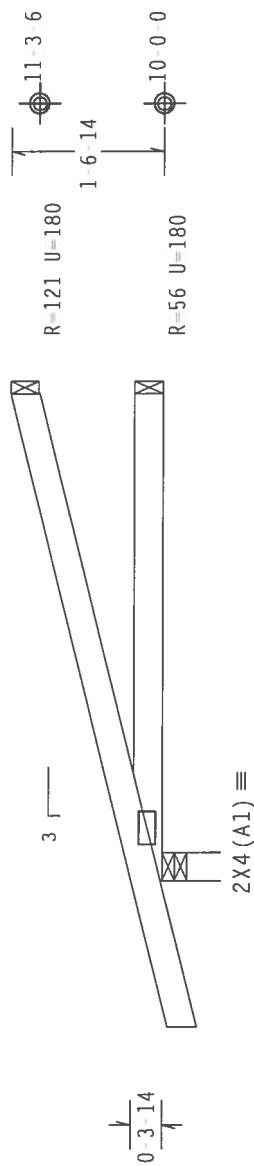
ALPINE ENGINEERED PRODUCTS, INC.
1950 Marlev Drive

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP	#2	Dense

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



1-6-0

← 5-0-0 Over 3 Supports →
R=323 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC

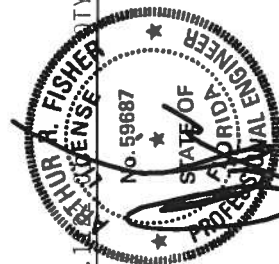
PLT TYP. Wave

FI-2002 (S10) / FBC	Cq/RT=1.00(1.25) / 10(0)	7.22.1

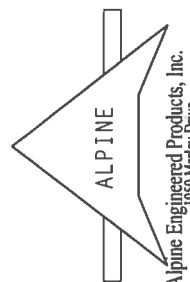
FL/-4/-1-R/-

Scale = .5"/Ft.

TC LL	20.0 PSF	REF R487 - - 86614
TC DL	10.0 PSF	DATE 02/10/06
BC DL	10.0 PSF	DRW HCUR487 06041070
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	40.0 PSF	SEQN- 28078
DUR.FAC.	1.25	



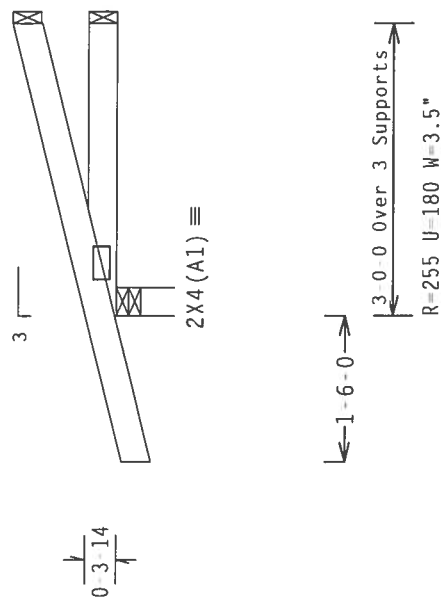
Feb 10, 06



Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.11

Scale = .5"/Ft.

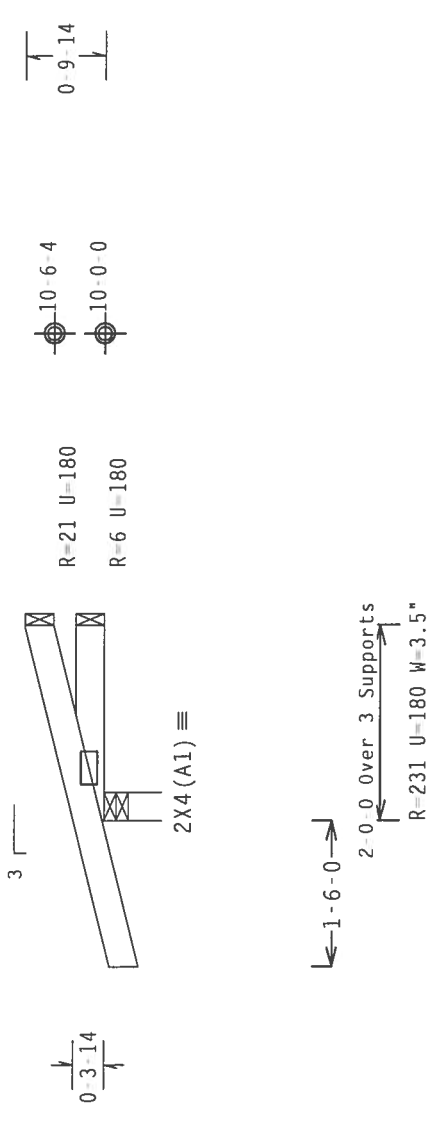
FL/-/4/-/R/-

TC LL	20.0 PSF	REF	R487 --	86615
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUSR487	06041071
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	40.0 PSF	SEON-	28081	
DUR.FAC.	1.25			

Feb 10 '06

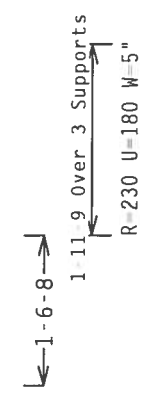
ALPINE ENGINEERED PRODUCTS, INC.
1950 Marley Drive

Top chord 2x4 SP #2 Dense	110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Bot chord 2x4 SP #2 Dense	Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.	



PLT TYP. Wave	Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/10(0) 7.22.1 FL / - / 4 / - / R / - Scale = 5" / Ft.	REF R487 -- 86616 DATE 02/10/06 DRW HCUSR487 06041072 HC-ENG JB/AF SEQN- 28088
WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BES1 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'AMORIO DR., SUITE 200, MADISON, WI 53719) AND WTEA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.	TC LL 20.0 PSF TC DL 10.0 PSF BC DL 10.0 PSF BC LL 0.0 PSF TOT.LD. 40.0 PSF DUR.FAC. 1.25	STATIONER R. FISHER PROFESSIONAL ENGINEER STATE OF ILLINOIS No. 59867 Feb 01 '06
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF AISC (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 2018/166A (W-11/5X) ASTM A653 GRADE 40/60 (M. K/H-S) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMER A3 OF TPI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT	ALPINE Alpine Engineered Products, Inc.	

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



ARTHUR R. FISHER
LICENSE
No. 59687
STATE OF
FLORIDA
PROFESSIONAL ENGINEER
Feb 10 '06

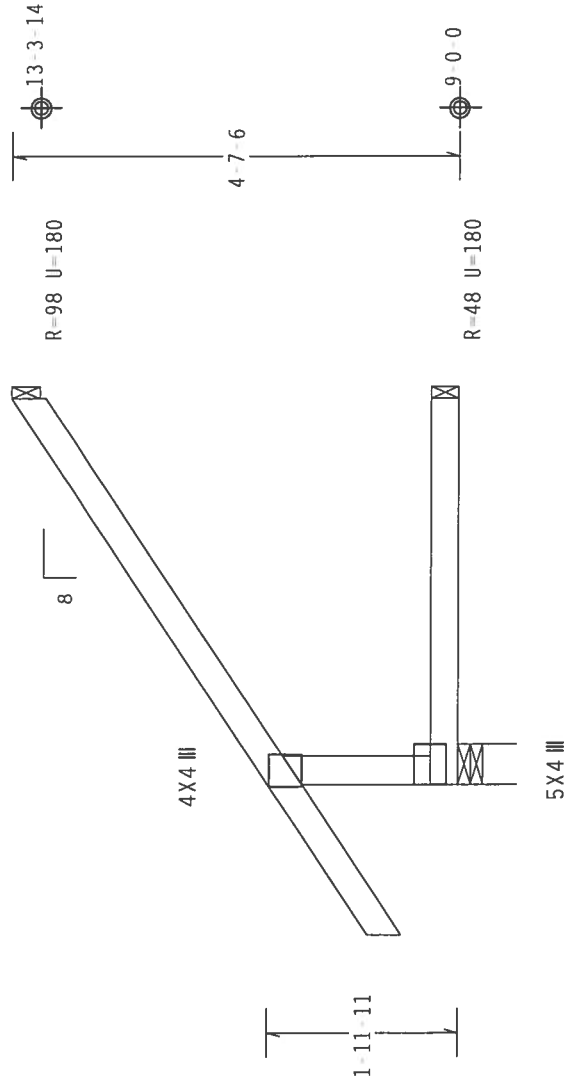
	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#2	Dense

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.17" due to live load and 0.10" due to dead load.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



1-6-8

←3-11-9 Over 3 Supports →
R=293 U=180 W=5"

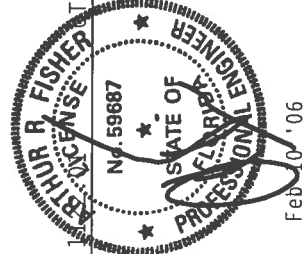
Design Crit: TPI-2002(STD)/FBC
C₀/RT=1.00(1.25)

PLT TYP. Wave

FL/4/1/R/

Scale = .5"/Ft.

TC LL	20.0	PSF	REF	R487 --	86618
TC DL	10.0	PSF	DATE	02/10/06	
BC DL	10.0	PSF	DRW	HCUSR487	06041020
BC LL	0.0	PSF	HC-ENG	JB/AF	*
TOT.LD.	40.0	PSF	SEQN-	27993	
DUR.FAC.	125				



Alpine Engineered Products, Inc.
1950 Marley Drive

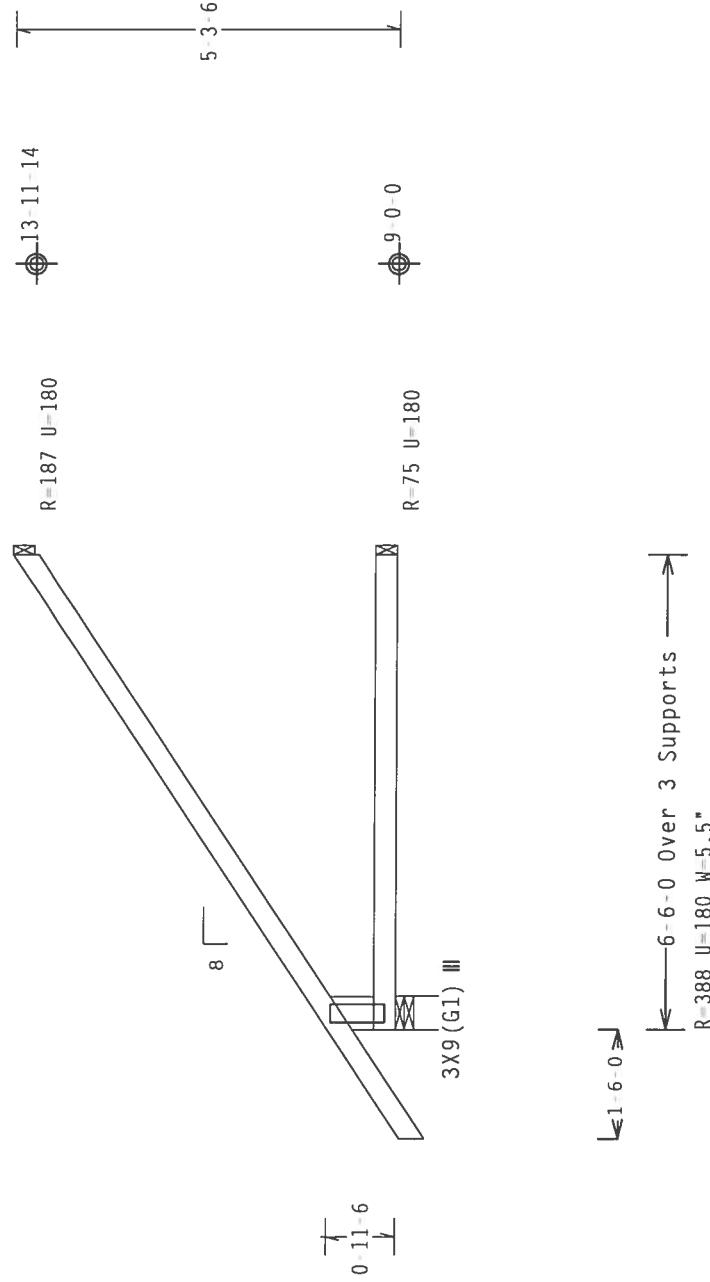
ALPINE

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
:Lt Stubbed Wedge 2x8 SP SS:

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord.
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

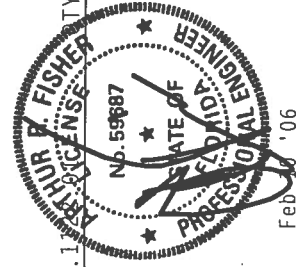
7 22 13

TV: 1

E11-1A1-1-1D1

Scale = .375"/Ft.

TC	LL	20.0	PSF	REF	R487 --	86621
TC	DL	10.0	PSF	DATE	02/10/06	
BC	DL	10.0	PSF	DRW	HCUSR487	06041074
BC	LL	0.0	PSF	HC-ENG	JB/AF	
TOT.	LD.	40.0	PSF	SEQN-	28037	
DIR	EAC	125				



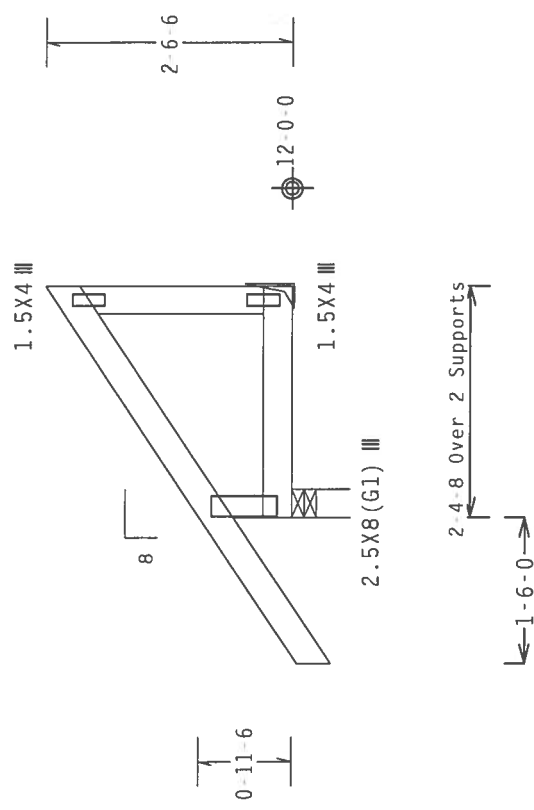
Feb 90, '06



Alpine Engineered Products, Inc.
1950 Marley Drive

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

H = recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.



R-236 U-180 W-3.5"
R-67 U-180 H=Simpson LU24
w/ (2) 10d, 0.148"x1.5" nails in Truss
w/ (4) 10d Common, 0.148"x3.0" nails in Girder
Girder is (1)2X6 min. So.Pine

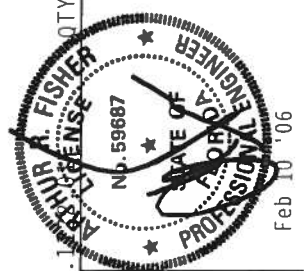
Design Crit: $\text{TPI-2002(STD) / FBC}$
 $\text{Cq/RT}=1.00(1.25) /$

PLT TYP. Wave

QTY:6 FL/-/4/-/-/R/-

Scale = .5"/Ft.

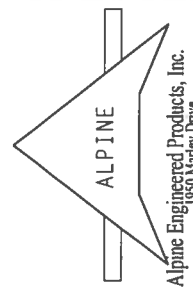
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	TC DL	10.0	PSF	DATE 02/10/06
	BC DL	10.0	PSF	DRW HCUSR487 06041022
	BC LL	0.0	PSF	HC-ENG JB/AF *
	TOT.LD.	40.0	PSF	SEQN- 82043
	DUR.FAC.	1.25		



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BCSI 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 1420 DUNFORD DR., SUITE 200, MADISON, WI 53719, AND MICA (MODUL TRUSS COMPANY OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE DESIGN CONFORMS WITH APPLICABLE PROVIDER FABRICATING, HANDLING, SHIPPING, INSTALLATION OF TRUSSES. THE CONNECTOR PLATES ARE MADE OF 2018/16GA (H 1/2X3/4) ASTM A575 GRADE 40/60 (H 1/2X3/4) STEEL. PLATES TO EACH FACE OF TRUSSES AND UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER A913 AX3 OF 1/11/2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

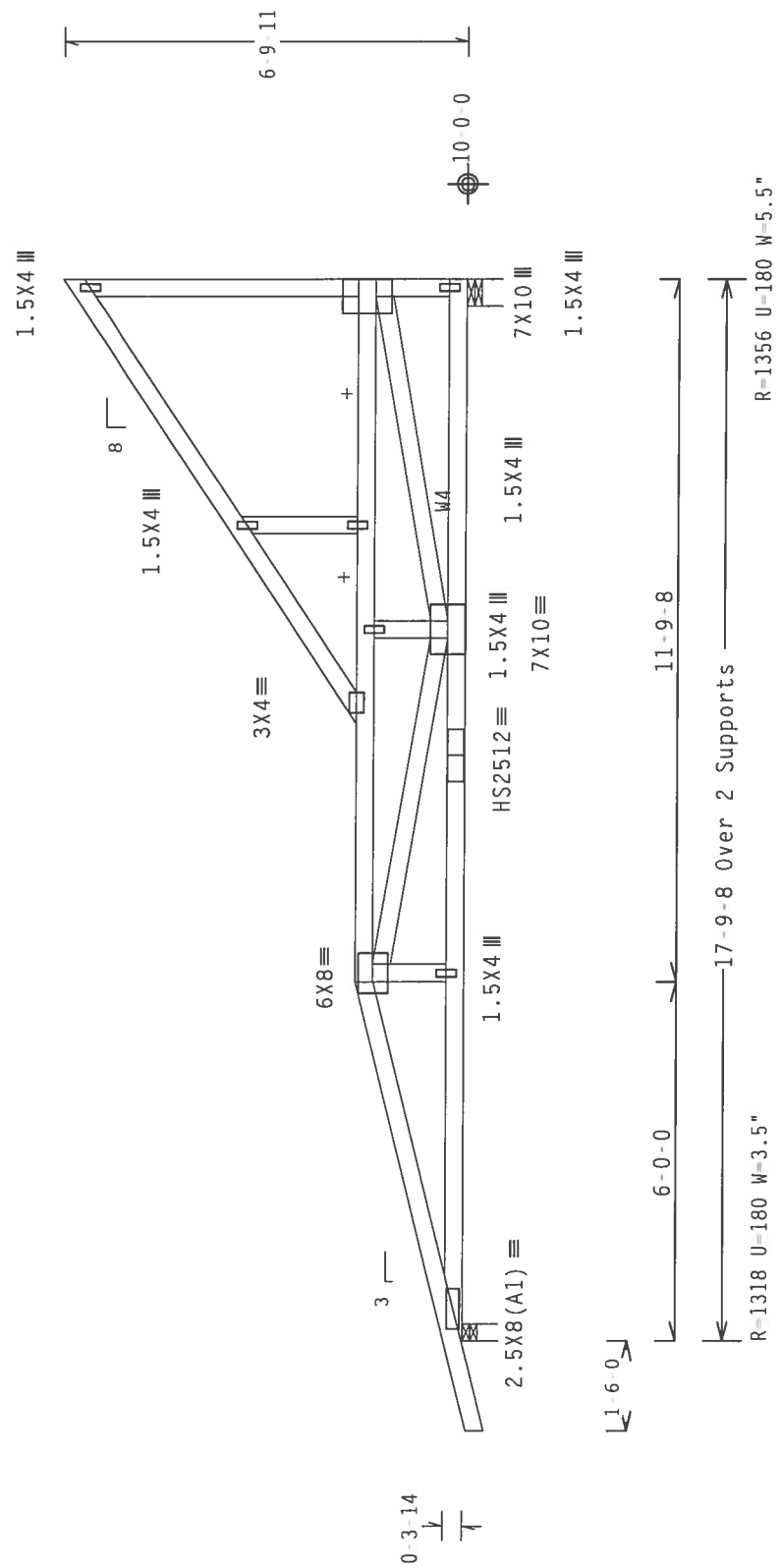
Calculated horizontal deflection is 0.13" due to live load and 0.20" due to dead load. Right end vertical not exposed to wind pressure.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

+ MEMBER TO BE Laterally Braced for Horizontal Wind Loads.
Bracing System to be Designed and Furnished by Others.

CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS.
SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END.
ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.



PLT TYP. 20 Gauge HS.Wave

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.22.11

Scale = .3125" / Ft.

REF R487 - 86623

DATE 02/10/06

DRW HCUSR487 06041075

HC-ENG JB/AF

SEQN 28105

PROPERTY: 1 FL/- /4/- /- /R/-

TC LL 20.0 PSF

TC DL 10.0 PSF

BC DL 10.0 PSF

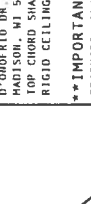
BC LL 0.0 PSF

TOT.LD. 40.0 PSF

DUR.FAC. 1.25

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETURN TO BEST T.O. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 DUFFIN RD., SUITE 200, MADISON, WI 53719) AND MICA (MIDWEST COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF IBCS (NATIONAL DESIGN SPEC. BY AISC) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (N-H/S/K) ASTM A653 GRADE 40/60 (N. K/H-S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPII 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

Feb 10 2006

Top chord 2x4 SP #2 Dense

Bot chord 2x4 SP #2 Dense

Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Right end vertical not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

0-3-14

3

4X5 (R)

5X5

1.5X4 III

8

6-9-11

10-0-0

3X4 (A1)

3X4

3X4

3X5

3

6-8-12

3-0-12

8-0-0

1-6-0

17-9-8 Over 2 Supports

R-829 U-180 W-3.5"

R-725 U-180 W-5.5"

TY:1

FL/-/4/-/-/R/-

Scale = .3125"/Ft.

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0)

7.22.1

PLT TYP. Wave

REF R487 -- 86624

DATE 02/10/06

DRW HCUSR487 06041076

HC-ENG JB/AF

SEQN- 28113

DUR.FAC. 1.25

ALPINE

Alpine Engineered Products, Inc.

1950 Marley Drive

ENGINEER

PROFESSIONAL ENGINEER

STATE OF ARIZONA

NO. 59687

FEEL 10 '06

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTC (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

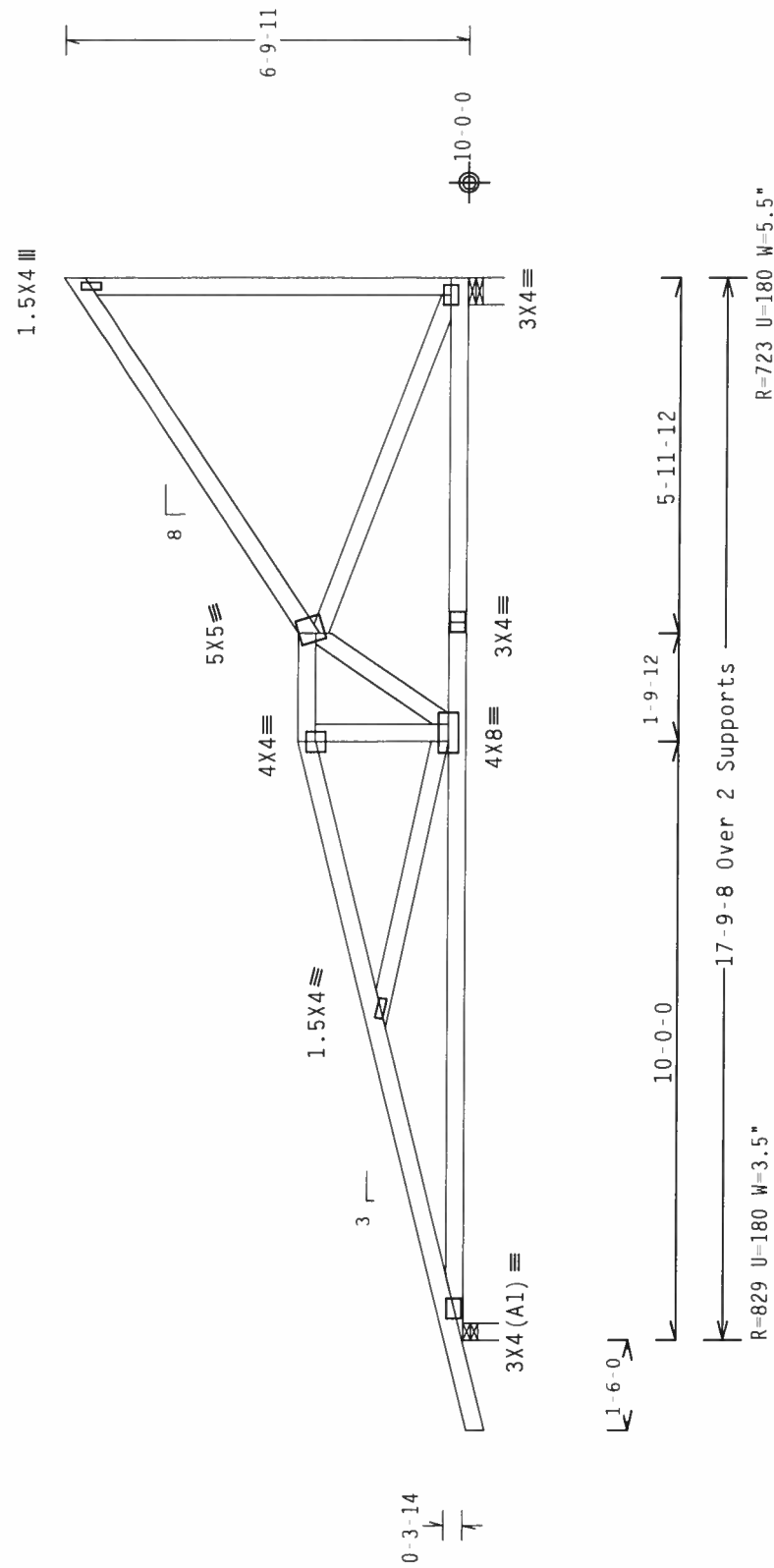
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF WDS (NATIONAL DESIGN SPEC. BY AIA/AIA) AND TPI. APPLICABLE CODES AND STANDARDS SHALL BE USED. UNLESS OTHERWISE INDICATED, ALL STEEL SHALL BE A36 STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, ALL PLATES SHALL BE 1/2" THICK. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC.3. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Right end vertical not exposed to wind pressure.

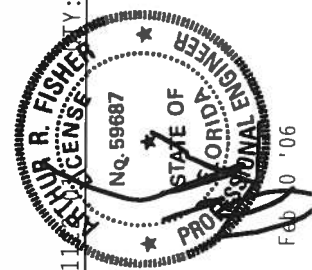


PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

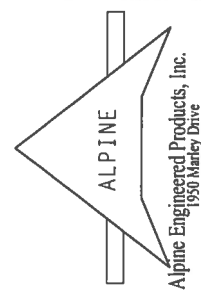
$$\text{FL} / - / 4 / - / - / \text{R} / - \quad \text{Scale} = .3125" / \text{Ft.}$$

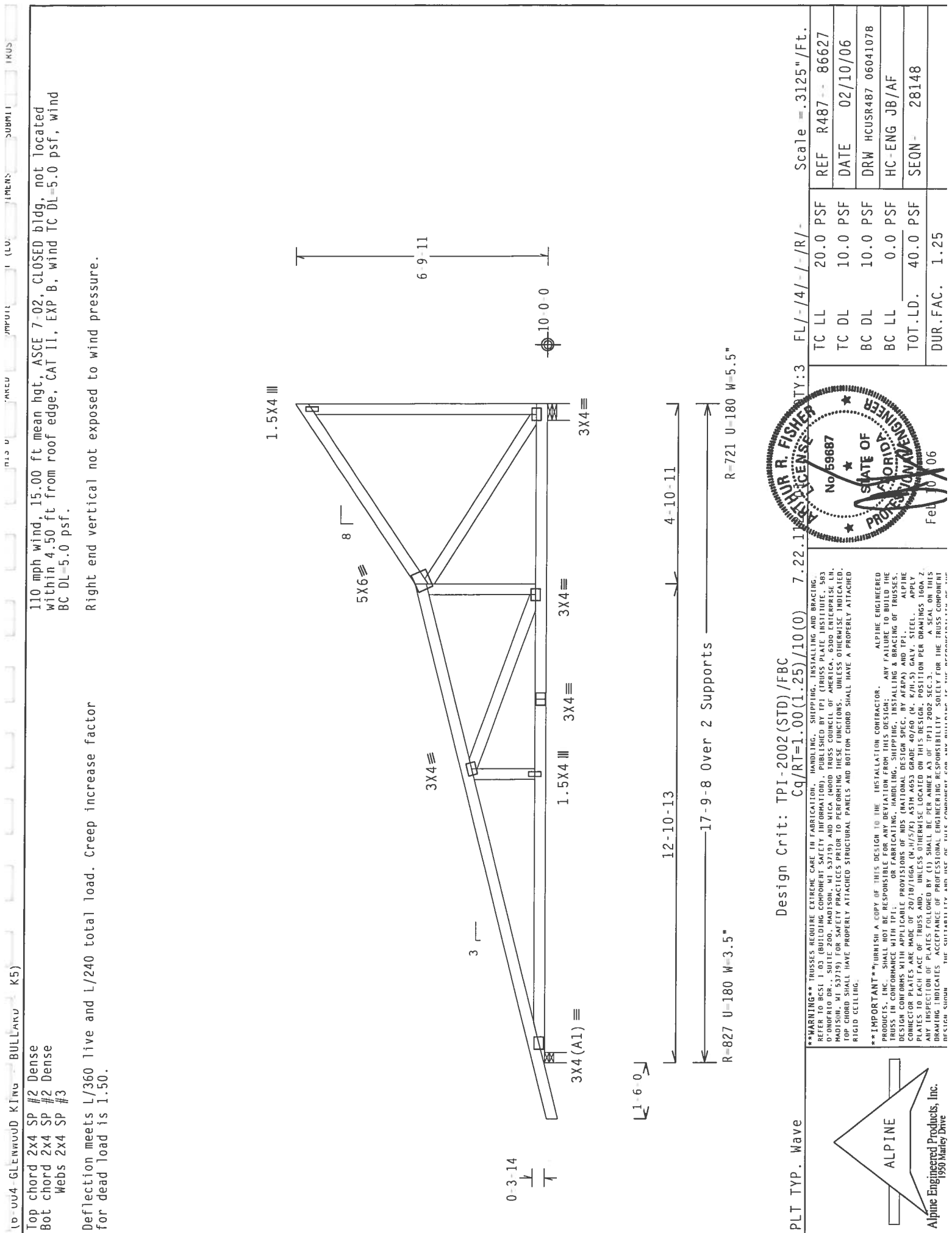
TC LL	20.0	PSF	REF R487 - - 86625
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCUSR487 06041077
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 28130
DUR.FAC.	1.25		



****IMPORTANT****TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI'S PROVISIONS FOR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES OR THE USE OF AN UNDESIGNATED MATERIAL WILL BE AT THE USER'S RISK AND WITHOUT LIABILITY TO ALPINE ENGINEERED PRODUCTS, INC.

CORRUGATOR PLATES ARE MADE OF 20/18TH GAUGE (.01875") ASTM A653 GRADE 40/90/10 C/PMS ONLY STEEL PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMNH AS OF 1/1/11 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

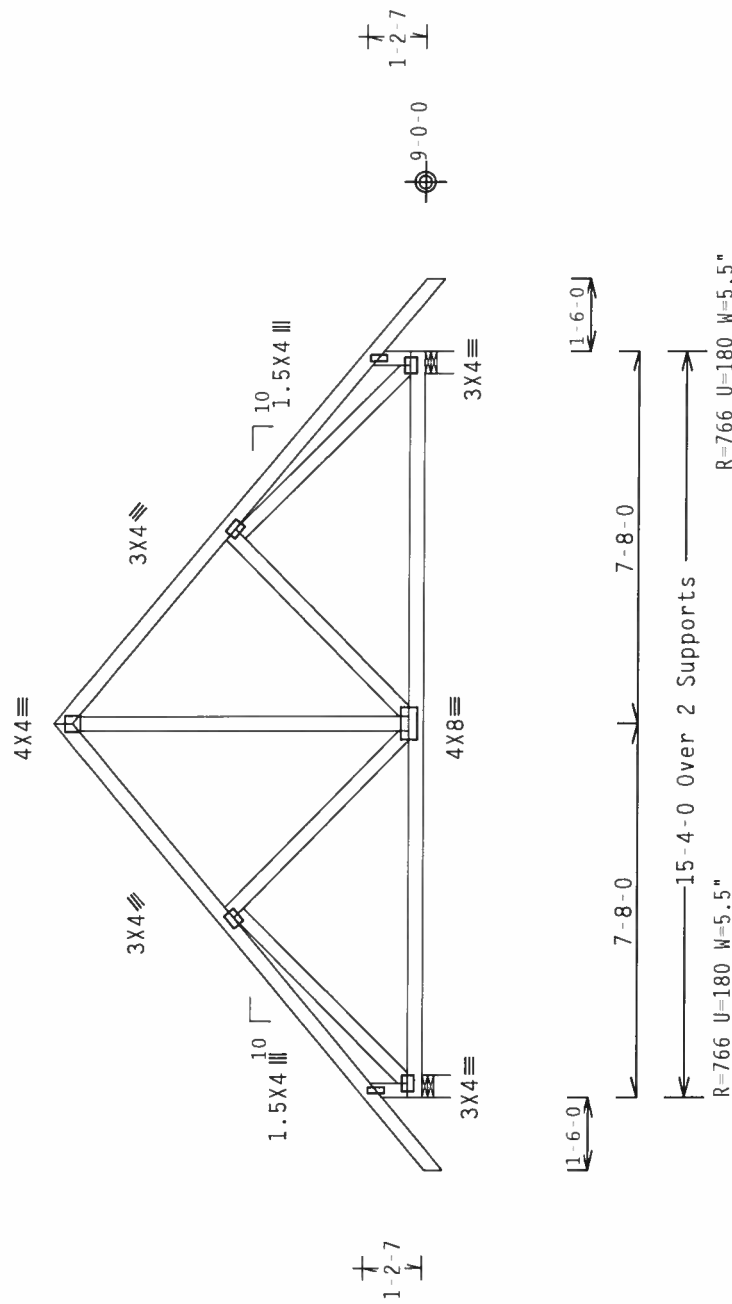




	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



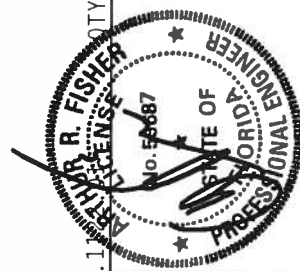
Design Crit: TPI-2002(STD)/FBC

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REPLY TO: SAFETY COMPONENTS, PUBLISHED BY TPI (TRUSS PAPER INSTITUTE), 583 D'ONOFIO RD., SUITE 100, WESTFIELD, MA 01095. FOR SAFETY INFORMATION, THESE FUNCTIONS ARE INDICATED BY THE FOLLOWING: 1. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED RIGID CEILING.

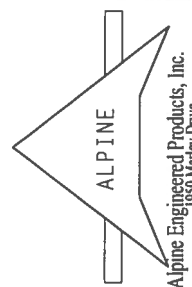
IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED DESIGN, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF THE AISC STEEL CONSTRUCTION MANUAL (13TH EDITION) AND THE AISC STEEL DESIGN GUIDE NO. 1. THE STEEL CONNECTOR PLATES ARE MADE OF 20T/18T/16G (1/2X24) ASTM A563 GRADE 40/50 (40 KSI) STEEL. PLATES TO EACH FACE OF TRUSS AND WELLS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A, 2 AND 160B. IF ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF IPTI 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

$$\text{Scale} = .25''/\text{Ft.}$$

TC LL	20.0	PSF	REF R487--	86629
TC DL	10.0	PSF	DATE	02/10/06
BC DL	10.0	PSF	DRW HCUSR487	06041023
BC LL	0.0	PSF	HC-ENG JB/AF	*
TOT.LD.	40.0	PSF	SEQN-	82547
DUR.EAC.	1.25			



Feb 10 '06



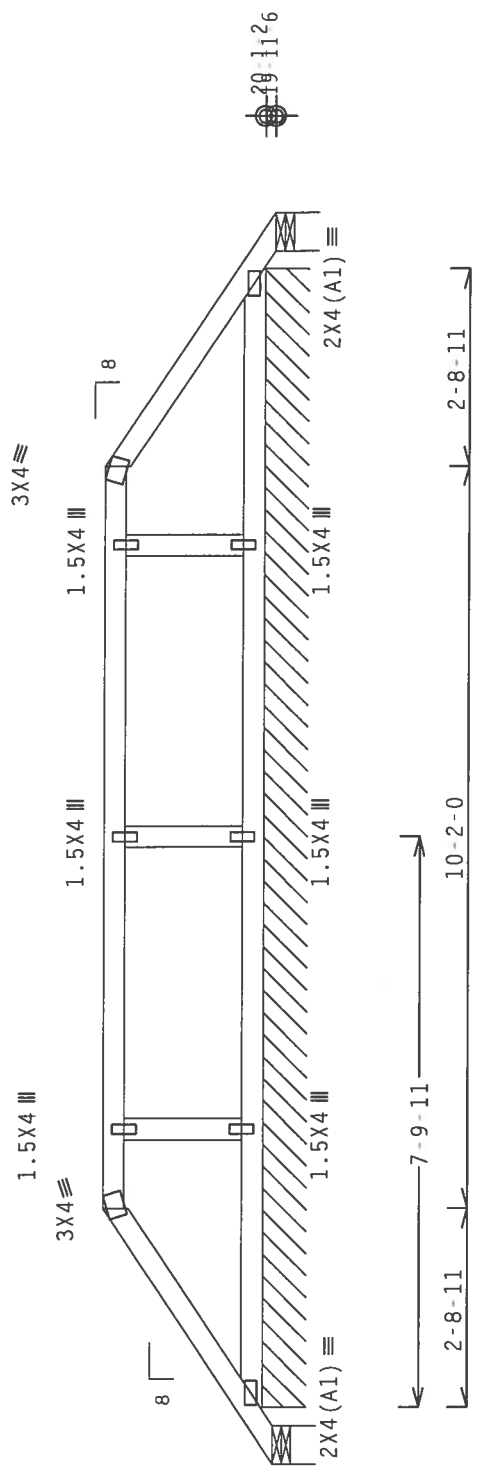
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.11 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

REFER TO DWG PIGBACK80405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED



R-16 U-180 W-6.31"
R-75 PLF U-26 PLF W-15-7-6

17-2-0 Over 3 Supports

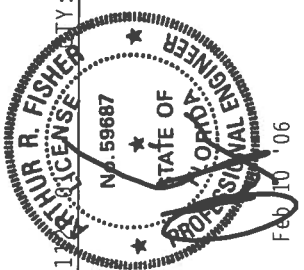
R-16 U-180 W-6.309"

Design Crit: TPI-2002(STD)/FBC

Cq/RT=1.00(1.25)/10(0) 7.22.1

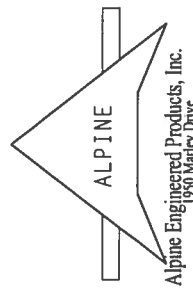
Scale = .375" / Ft.

TC LL	20.0 PSF	REF	R487 --	86630
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	2.0 PSF	DRW	HCUSR487	06041080
BC LL	0.0 PSF	HC-ENG	JB/AF	
TOT.LD.	32.0 PSF	SEQN	82205	
DUR.FAC.	1.25			



****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSE 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 O'CONNOR DR., SUITE 200, MADISON, WI 53719) AND MICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

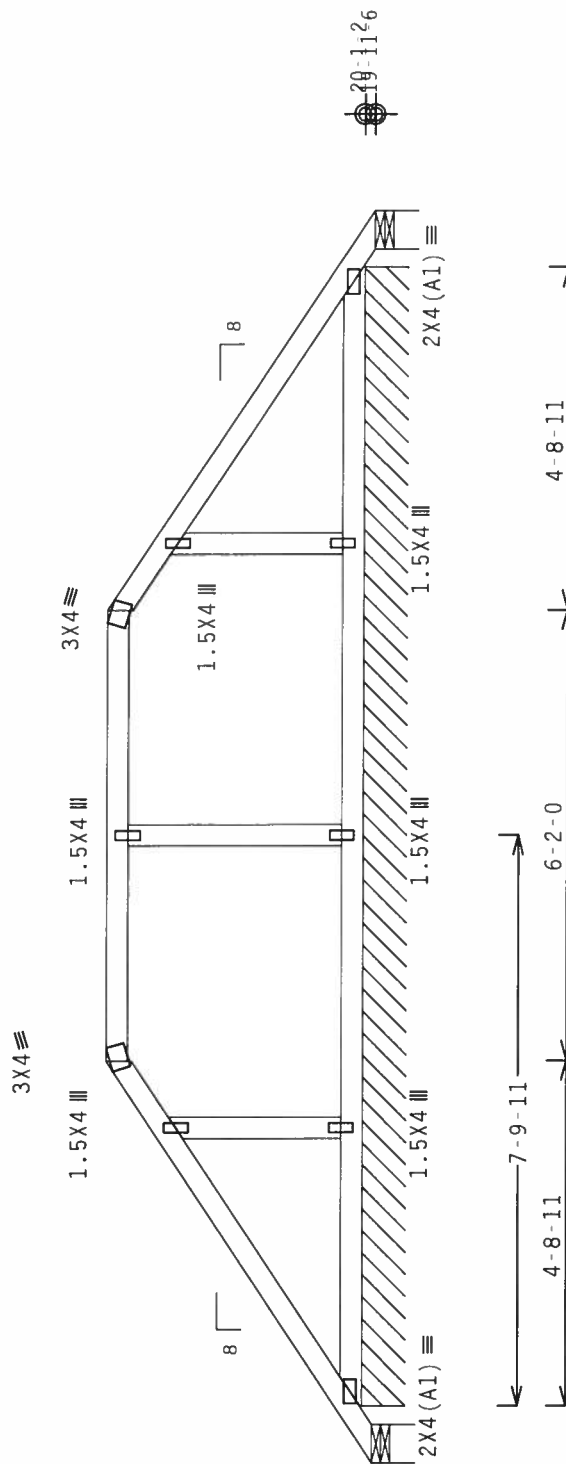
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTION PLATES ARE MADE OF 2010T1000 (4.175X3) ASH A663 GRADE 40/60 (4.175X3) GALV. STEEL. APPLY PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. UNLESS OTHERWISE INDICATED, POSITION PER DRAWINGS 100A Z. ANY INSPECTION OF PLATES FOLLOWED BY TPI SHALL BE REFERENCED TO TPI. UNLESS OTHERWISE INDICATED, DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY. SOLELY FOR THE TRUSS COMPONENT



110 mph wind, 21.78 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

REFER TO DWG PIGBACK0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED

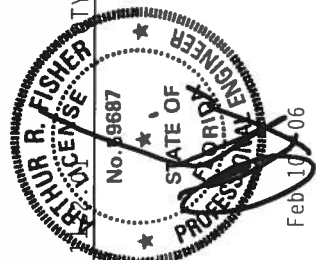


$R=36$ $U=180$ $W=6.31''$
 $R=77$ $PLF=29$ PLF $W=15.7-6$
 17-2-0 Over 3 Supports $R=36$ $U=180$ $W=6.31''$

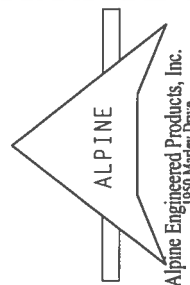
Design Crit: $\text{TPI-2002(STD)}/\text{FBC}$
 $\text{Cq}/\text{RT}=1.00(1.25)/10(0)$ 7.22

Scale = .375"/Ft.

TC LL	20.0 PSF	REF R487 - - 86631
TC DL	10.0 PSF	DATE 02/10/06
BC DL	2.0 PSF	DRW HCUR487 06041097
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	32.0 PSF	SEQN- 146694 REV
DUR.FAC.	1.25	



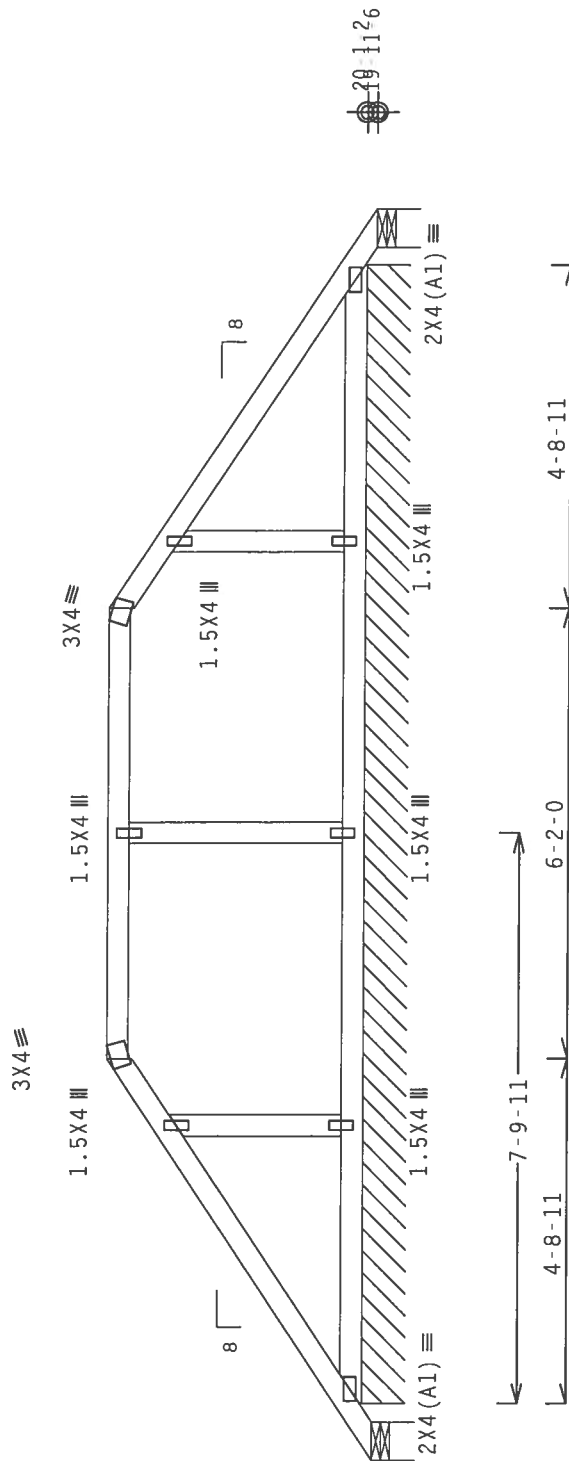
Feb 1995-06



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 21.78 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL-5.0 psf, wind BC DL-1.2 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R-36 U-180 W-6.31"

 $R = -36 \quad U = 180 \quad W = 6.31''$

R=77 PLF U=29 PLF W=15-7-6

17-2-0 Over 3 Supports

PLT TYP. Wave

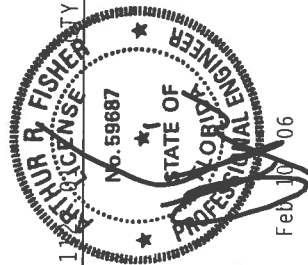
Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

FL/-/4/-/-/R/-

Scale = .375"/Ft.

WARNING TRUSSES REQUIRE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLATION AND BRACING. REFER TO BC51 1 03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 583 D-ONFORD RD., SUITE 200, MADISON, WI 53719, AND NCA WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719, FOR SAFETY PRACTICES PRIOR TO ERECTING THESE STRUCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

[illegible]

Feb 10 06

DUR.FAC. 1.25

SEQN - 146689 REV

HC - ENG JB/AF

DRW HCUSR487 06041081

DATE 02/10/06

REF R487 86633

Scale = 375"/ft

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

	Top	Bot	Dense
#1	chord	chord	SP
#2	chord	chord	SP
#3	webs	webs	SP

SPECIAL LOADS

TC	-	From	64 PLF at 0.00 to	64 PLF at 17.16
BC	-	From	4 PLF at 0.00 to	4 PLF at 17.16

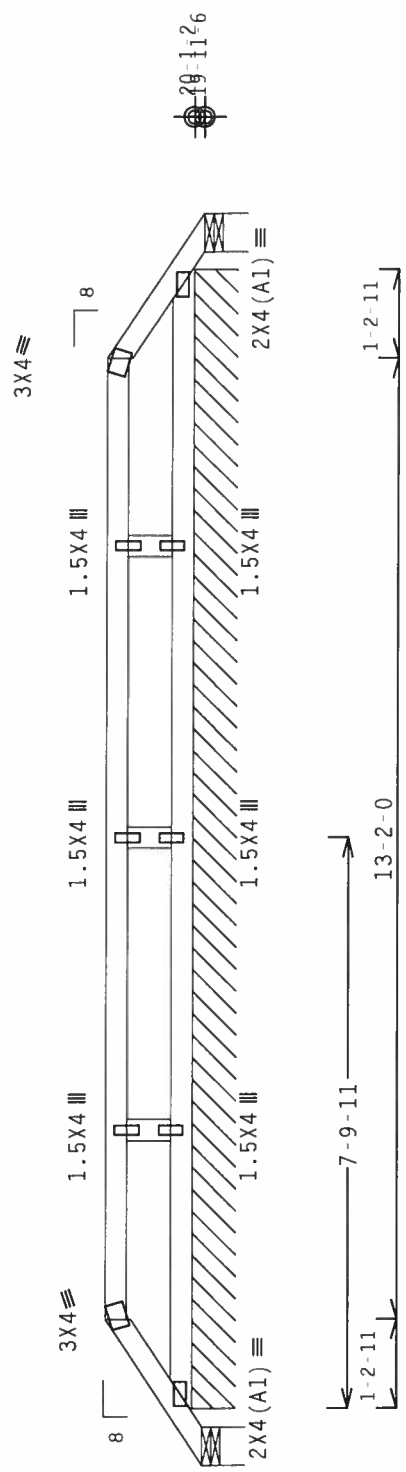
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24"O.C., UNLESS OTHERWISE SPECIFIED

Trusses or components connecting to this girder have been modified by the truss designer. The loading for this girder requires verification for accuracy.

110 mph wind, 20.61 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.



$\overbrace{\hspace{10em}}^{17-2-0 \text{ Over } 3 \text{ Supports}}$
 $R=3 \quad U=180 \quad W=6.31''$
 $R=72 \text{ PLF} \quad U=16 \text{ PLF} \quad W=15-7.6$

Design Crit: $\text{TPI-2002(STD) / FBC}$
 $\text{Cq/RT} = 1.00(1.25) /$
$$FL / - / 4 / - / - / R / - / - /$$

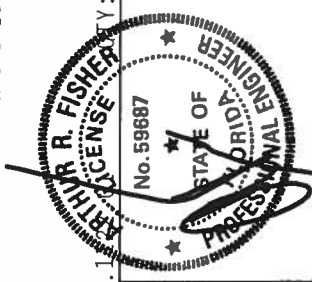
Scale = .375" / Ft.

TC LL	20.0 PSF	REF R487 --	86634
TC DL	10.0 PSF	DATE	02/10/06
BC DL	10.0 PSF	DRW	HCUSR487 06041083
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN-	27861
DUR.FAC.	1.25		

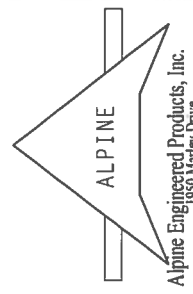
PLT TYP. Wave

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'AMORE DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE IN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED, RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
PROVIDE RESPONSIBILITY FOR THE FOLLOWING DESIGN: ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE SUPPLYING MANUFACTURER'S DESIGN SHALL BE THE RESPONSIBILITY OF THE FABRICATOR. THE TRUSS SHALL BE DESIGNED TO MEET THE REQUIREMENTS OF THE NATIONAL DESIGN SPEC. BY AFPM AND TPI. THE TRUSS CONFORMS WITH ALL APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AFPM) AND TPI. ALL CONNECTOR PLATES ARE MADE OF 20/18/16GA (H/1/5/16) ASTM A653 GRADE 40/60 (H/1/5/16) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 100A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A OF TPII 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



Feb 10, '06



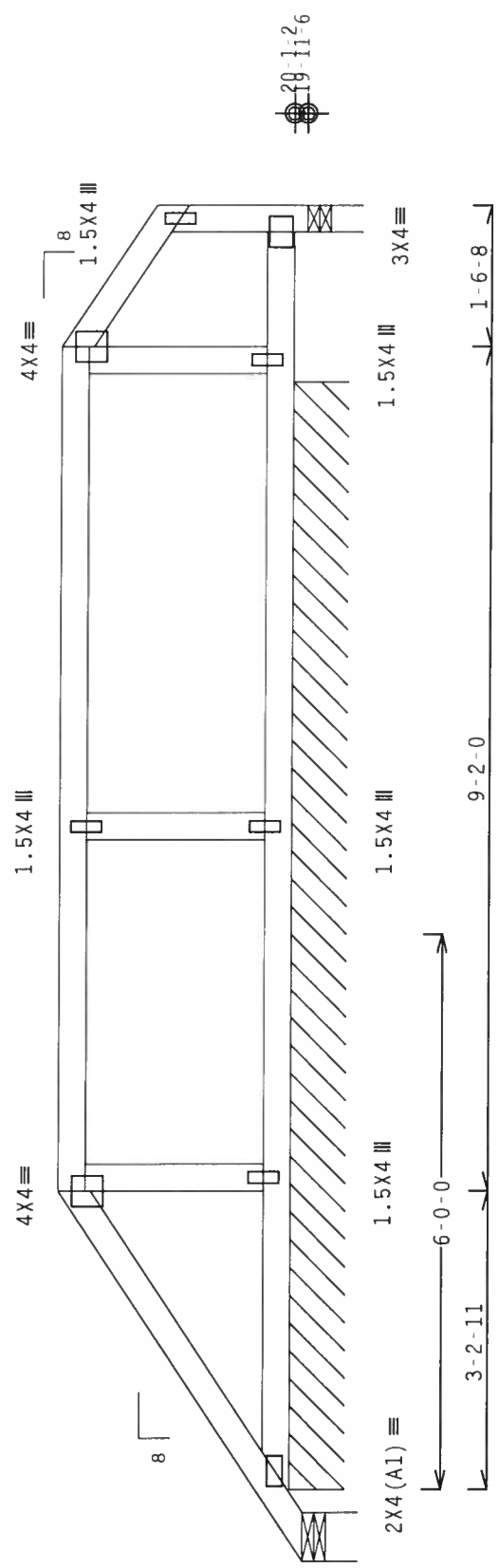
Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 21.28 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

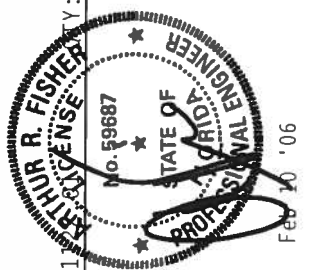


$R=28 \quad U=180 \quad W=6.31"$
 $R=82 \quad PLF \quad U=31 \quad PLF \quad W=12.0-0$
 14-8-8 Over 3 Supports
 $R=29 \quad U=180 \quad W=3.5"$

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.11

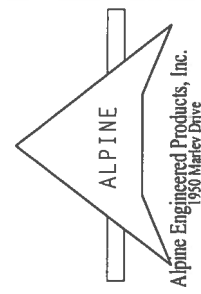
Scale = .5"/Ft.

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TC DL	10.0 PSF	DATE 02/10/06
BC DL	2.0 PSF	DRW HCUJR487 06041084
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	32.0 PSF	SEUN- 27849
DUR.FAC.	1.25	



*****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1.03 (BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TP1 (TRUSS PACE INSTITUTE, 583 D'ORFIO DR., SUITE 200, MADISON, WI 53719) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 CUIPERVILLE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERWISE INDICATED. TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT****—FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED
DESIGN PROVIDES FORTY PERCENT DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE
TRUSS IN CONFORMANCE WITH THE PROVISIONS OF THIS DESIGN, INCLUDING THE BUILDING OF THE
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC., BY ASEA) AND IPI
CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/45/30) ASTM A653 GRADE 40/50 (W/ 45/30) GALV. STEEL. APPLY
PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, PER DRINKINGS 160A Z.
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF IPI11 2007 SEC.3.
DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



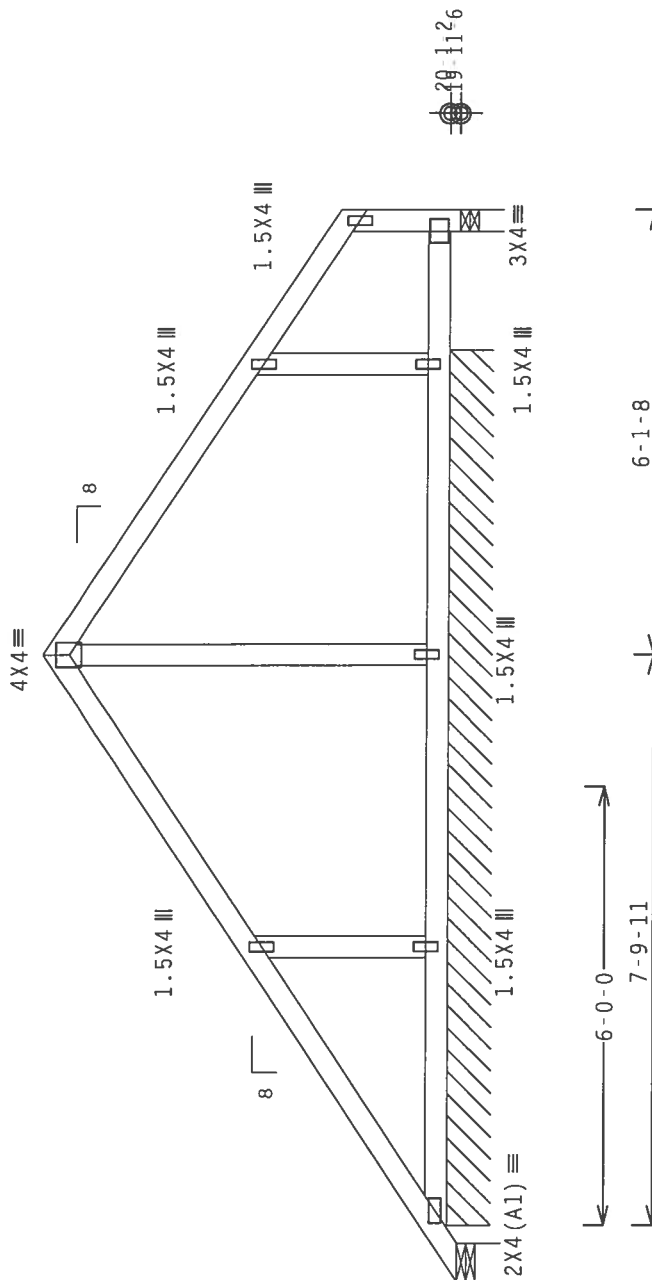
	Top	chord	2x4	SP	#2	Dense
	Bot	chord	2x4	SP	#2	Dense
		webs	2x4	SP	#3	

1110 mph wind, 22.81 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

REFER TO DWG PIGBACKB0405 FOR PIGGYBACK DETAILS.
TOP CHORD OF SUPPORTING TRUSS UNDER PIGGYBACK TO
BE BRACED @ 24" O.C., UNLESS OTHERWISE SPECIFIED



← 14-8-8 Over 3 Supports →

R-36 U=180 W=6.31"
R-82 PLF U=33 PLF W=12-0-0

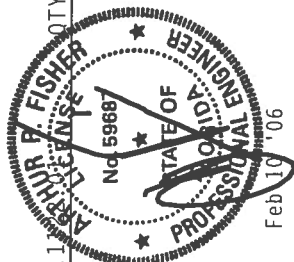
R-33 U=180 W=3.5"

Design Crit: TPI-2002(STD)/FBC $Cq/RT=1.00(1.25)/10(0)$ 7.22.13

QTY: 4 FL/-/4/-/-/R/-

Scale = .375"/Ft.

TC LL	20.0 PSF	REF R487 - 86638
TC DL	10.0 PSF	DATE 02/10/06
BC DL	2.0 PSF	DRW HCUR487 06041087
BC LL	0.0 PSF	HC-ENG JB/AF
TOT.LD.	32.0 PSF	SEQN - 27823
DUR.FAC.	1.25	



Feb 10 '06

Alpine Engineered Products, Inc.
1950 Marlow Drive

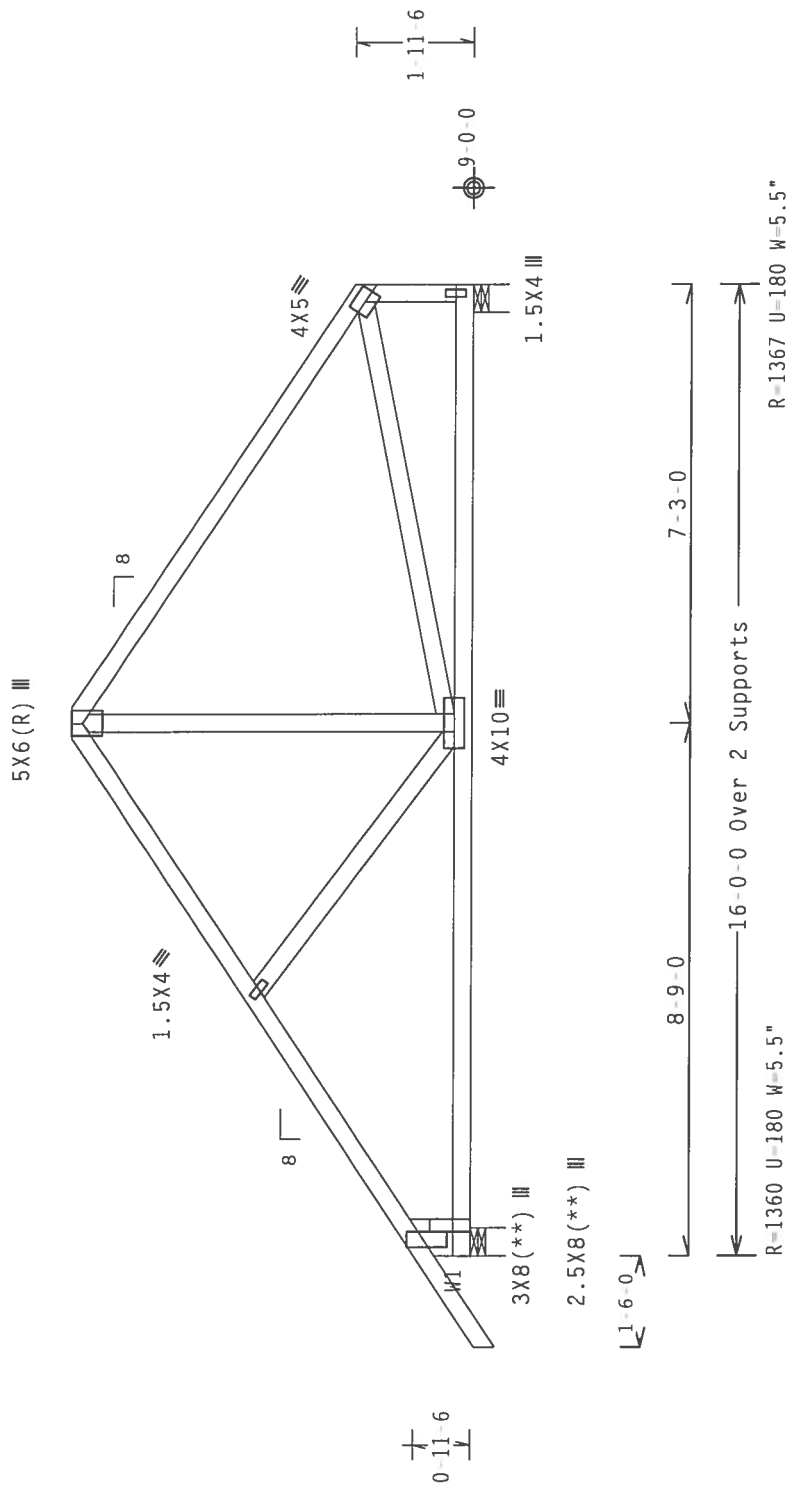
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W1 2x8 SP SS:
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located
within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind
BC DL=5.0 psf.

(**) Plate relocated as shown.

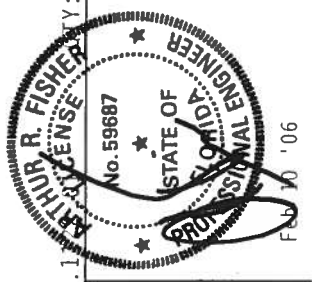
SPECIAL LOADS

		DUR.FAC. = 1.25	PLATE	DUR.FAC. = 1.25
TC	From	64 PLF at 1.50 to	64 PLF at 16.00	
BC	From	5 PLF at 1.50 to	5 PLF at 0.00	
BC	From	20 PLF at 0.00 to	20 PLF at 16.00	
TC	204 LB Conc.	Load at 8.56,	8.94	
BC	436 LB Conc.	Load at 8.50,	9.00	

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/

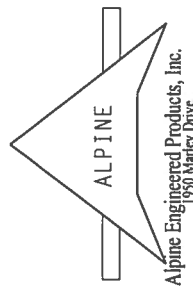
FL/-/4/-/R/-	Scale = 3125"/Ft.
TC LL 20.0 PSF	REF R487-- 86639
TC DL 10.0 PSF	DATE 02/10/06
BC DL 10.0 PSF	DRW HCUSR487 06041088
BC LL 0.0 PSF	HC-ENG JB/AF
TOT.LD. 40.0 PSF	SEQN- 28070
DUR.FAC. 1.25	



7.2.
design criteria: TPI-2002(SID)/FBC
 $Cq/RT=1.00(1.25)10(0)$

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PACE INSTITUTE, 583 D'ORFORD DR., SUITE 200, MADISON, WI 53719) AND WICA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

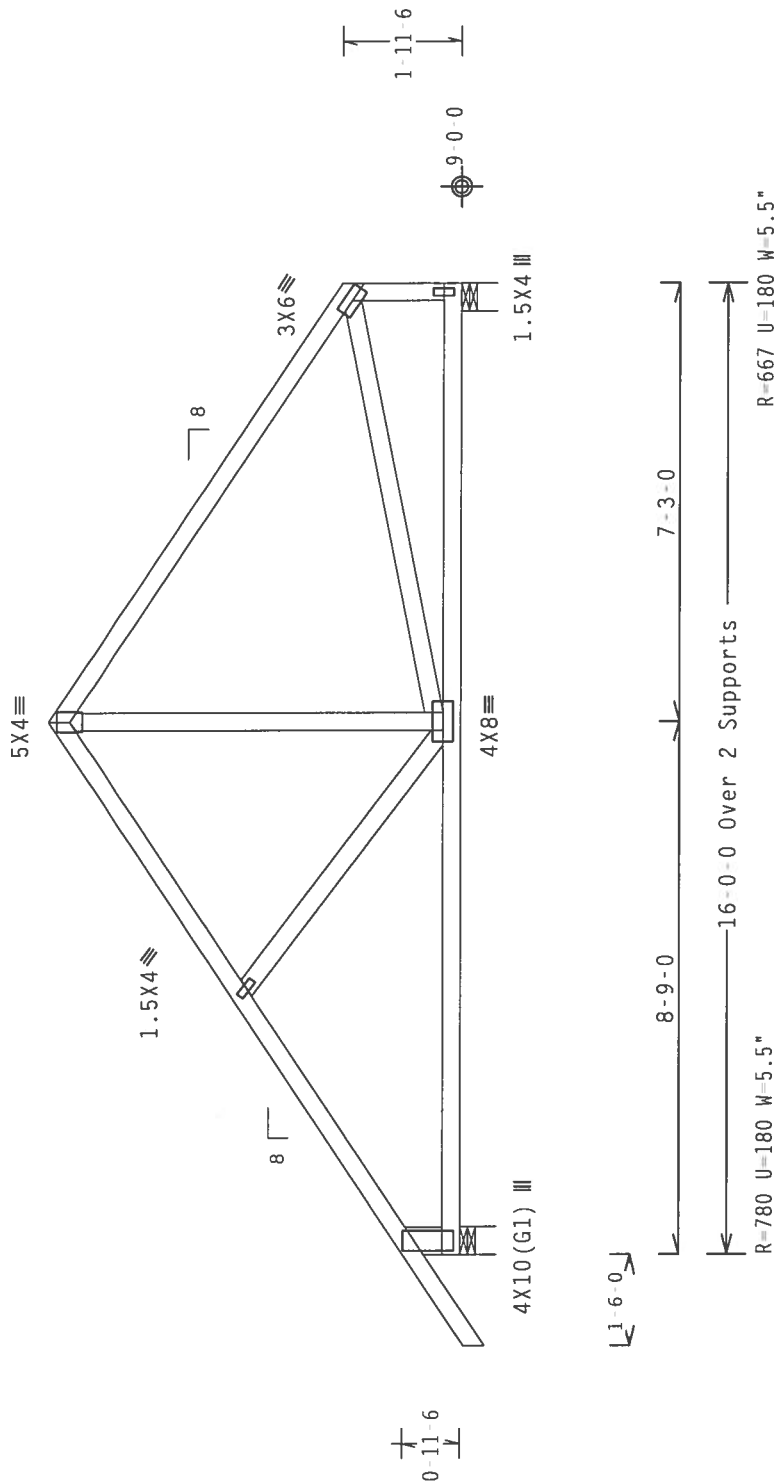
****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACING OF TRUSSES DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. CONNECTOR PLATES ARE MADE OF 2018/16GA (N/12/5) ASTM A653 GRADE 40/60 (M. K/H-5) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A 2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER AMX 3 OF TPII 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENTS



Top	chord	2x4	SP	#2	Dense
Bot	chord	2x4	SP	#2	Dense
	webs	2x4	SP	#3	

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

 $Cq/RT=1.00(1.25)/10(0)$

FL/-/4/-/-/R/-

Scale = .3125"/Ft.

WARNING: TRUSSES REQUIRING EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI TRUSSING PLATE INSTITUTE, 583 D'ONOFIO DR., SUITE 200, MADISON, WI 53719; AND NCA (NORTH BRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

TC LL	20.0	PSF	REF R487 - - 86640
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCUSR487 06041024
BC LL	0.0	PSF	HC-ENG JB/AF *
TOT.LD.	40.0	PSF	SEQN - 28051
DUR.FAC.	1.25		

Alpine Engineered Products, Inc.

Feb 19, '06

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #1 Dense
Webs 2x4 SP #3
:lt Stubbed Wedge 2x8 SP SS:

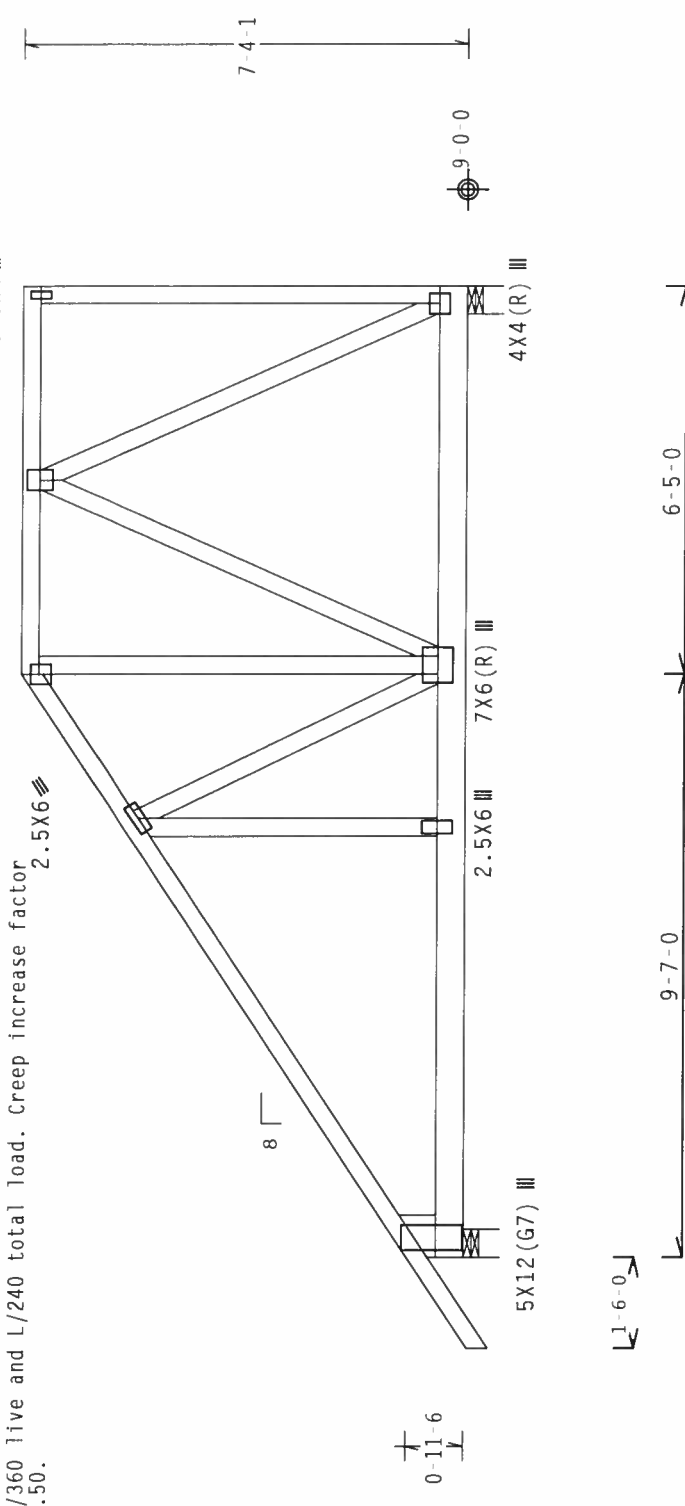
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 4.50" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Trusses or components connecting to this girder have been modified by the truss designer. The loading for this girder requires verification for accuracy.

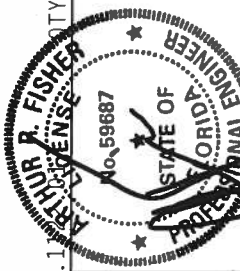
SPECIAL LOADS
----- (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 64 PLF at -1.50 to 64 PLF at 16.00
BC - From 5 PLF at -1.50 to 5 PLF at 0.00
BC - From 20 PLF at 0.00 to 20 PLF at 8.00
BC - From 532 PLF at 8.00 to 532 PLF at 16.00
BC - 2400 LB Conc. Load at 7.00

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave
Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.11
Scale = .3125"/Ft.

TC LL	20.0 PSF	FL / - / 4 / - / R / -	REF	R487 - -	86641
TC DL	10.0 PSF		DATE	02/10/06	
BC DL	10.0 PSF		DRW	HCUSR487	06041089
BC LL	0.0 PSF		HC-ENG	JB/AF	
TOT.LD.	40.0 PSF		SEQN-	146703	REV
DUR.FAC.	1.25				

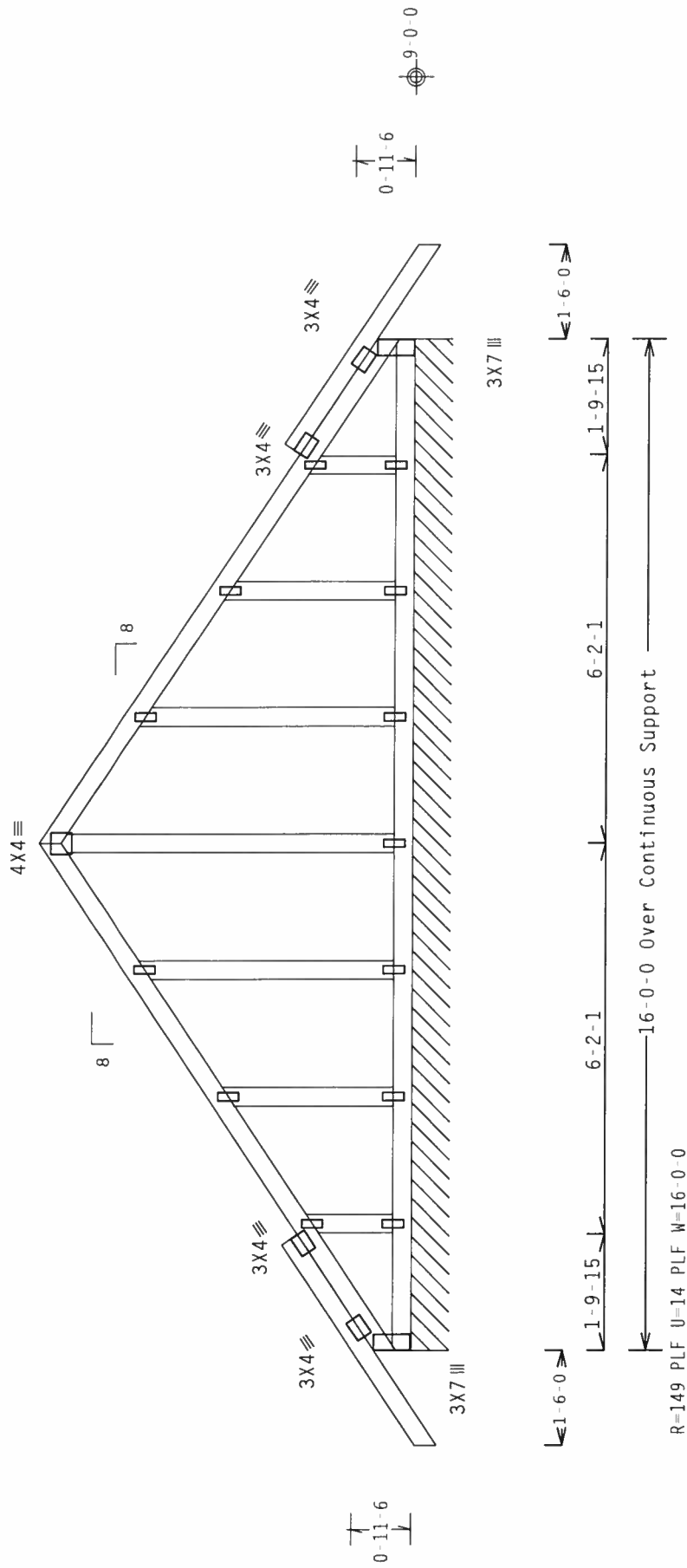


110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS A11015EE0405 & GBLLETIN0405 for more requirements.

Truss spaced at 24.0" OC designed to support 1-4-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

FL/-/4/-/-/R/-

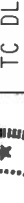
Scale = .375"/Ft.



ALPINE
Engineered Products, Inc.

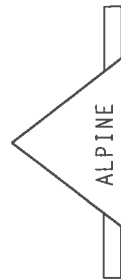
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 593 D'ONOFRIO DR., SUITE 200, MADISON, WI 53719) AND WTA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN ACCORDANCE WITH THIS DESIGN, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGNED IN CONJUNCTION WITH TPI, PROHIBITS THE DESIGNER'S LIABILITY (PER TPI AND ALPINE). ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA. (4-JULY-95) ASTM A656 GR. 60/55 IN. TPI STEEL. ALPINE TRUSS PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A, Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF IPT11 2002 SEC-3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



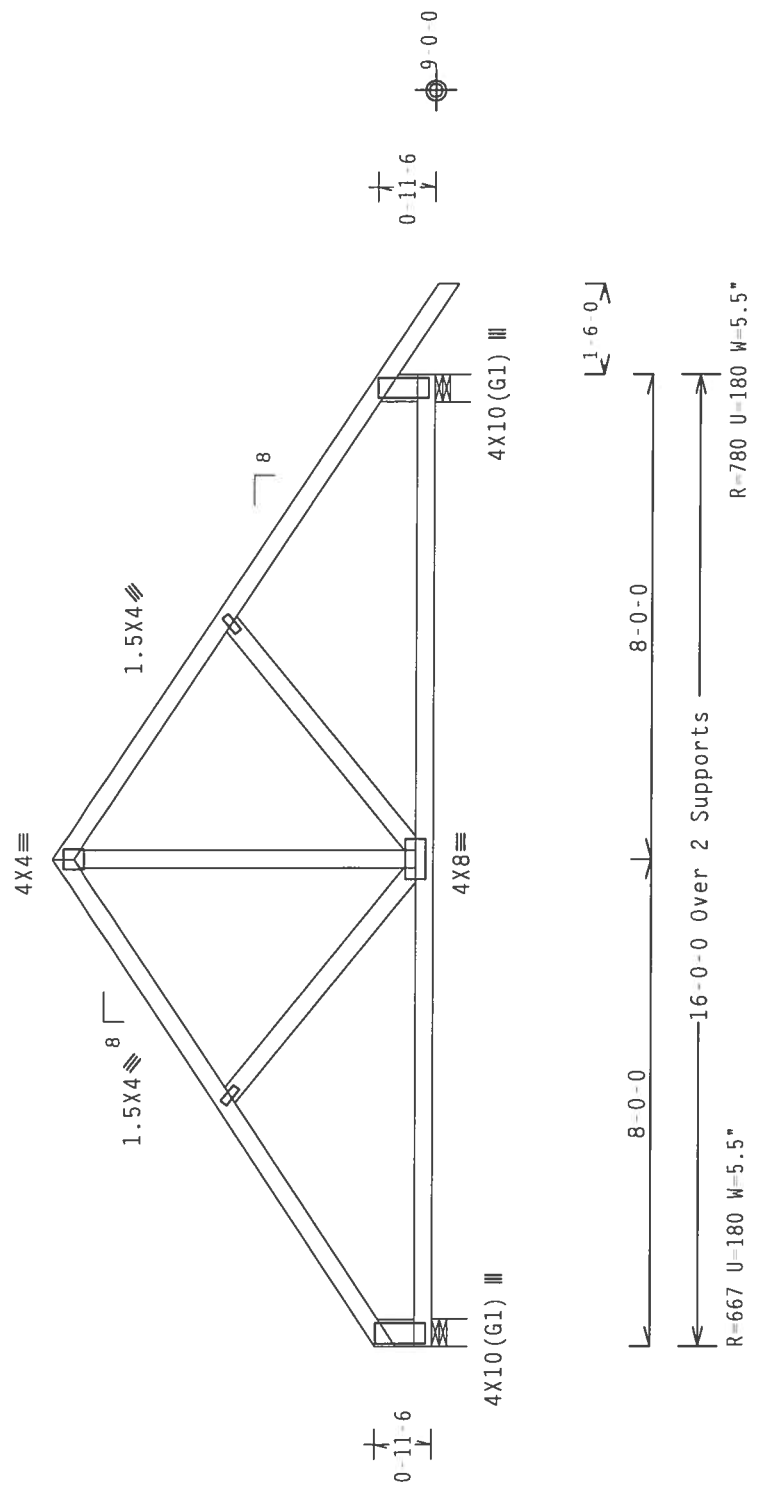
TC LL	20.0	PSF	REF	R487 --	86642
TC DL	10.0	PSF	DATE	02/10/06	
BC DL	10.0	PSF	DRW	HCUR487	06041090
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	27954	REV
DUR.FAC.	1.25				

Feb 10, '06



Alpine Engineered Products, Inc.
1950 Marley Drive

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubbcd Wedge 2x8 SP SS::Rt Stubbcd Wedge 2x8 SP SS:
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002 (STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22.1

Scale = .3125" / Ft.

TC LL	20.0 PSF	REF	R487 - -	86644
TC DL	10.0 PSF	DATE	02/10/06	
BC DL	10.0 PSF	DRW	HCUSR487	06041026
BC LL	0.0 PSF	HC-ENG	JB/AF	*
TOT.LD.	40.0 PSF	SEQN	27946	
DUR.FAC.	1.25			

ALPINE

Alpine Engineered Products, Inc.
1950 Marlow Drive

SEAL

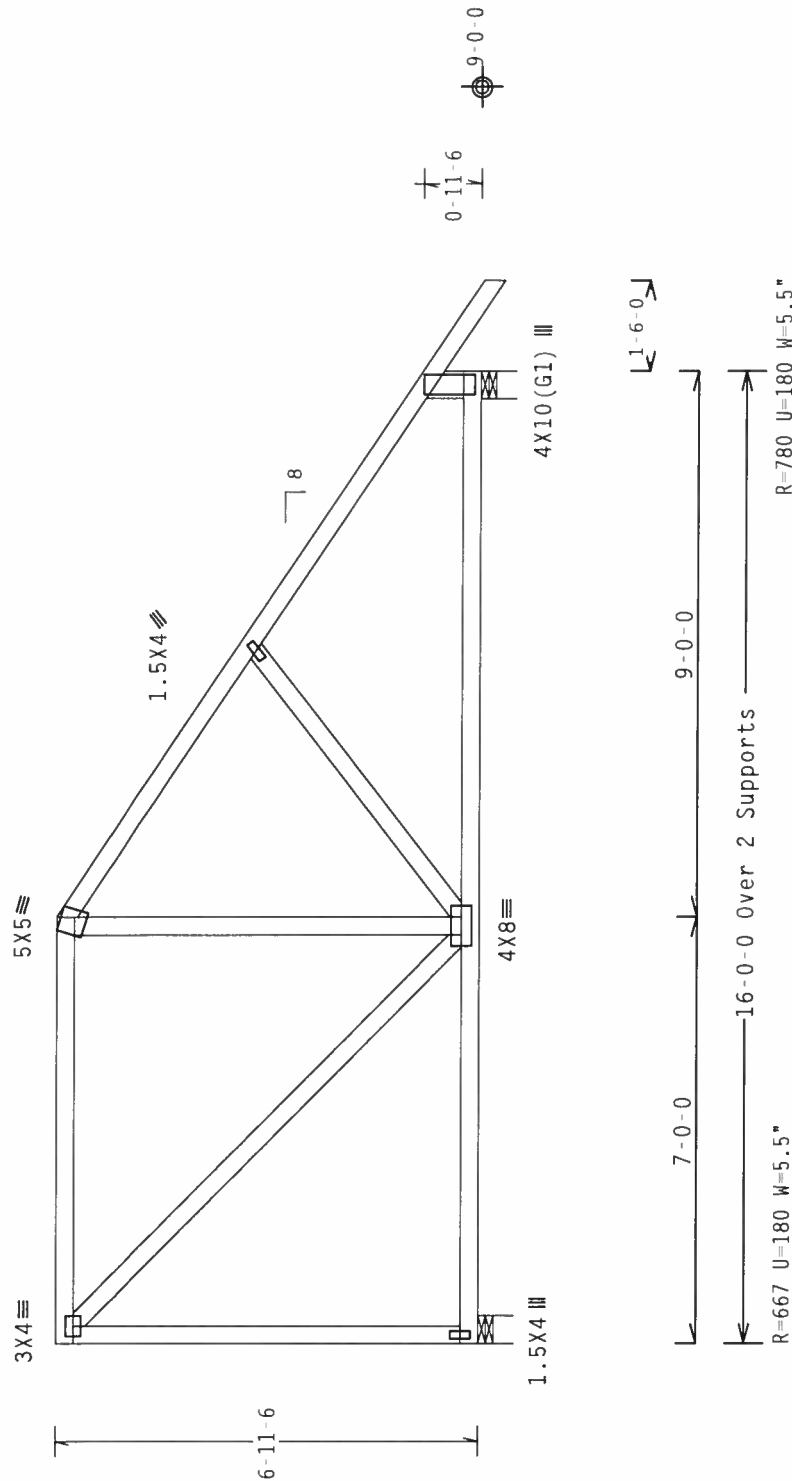
PROFESSIONAL ENGINEER
STATE OF FLORIDA
No. 59687
R. FISHER
FEB 10 '06

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 503 D'ONOFIO DR., SUITE 200, MADISON, WI 53719), AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN., MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE TRUSSES ARE DESIGNED TO MEET THE REQUIREMENTS OF THE 2001 IBC (INTERNATIONAL BUILDING CODE) AND THE 2002 IBC (INTERNATIONAL BUILDING CODE). ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF THE 2002 IBC. (2) SHALL BE PER ANNEX A3 OF THE 2002 IBC. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

110 mph wind, 15.00 ft mean hgt., ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT 1I, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

$$C_a/RT = 1.00(1.25)/10(0)$$

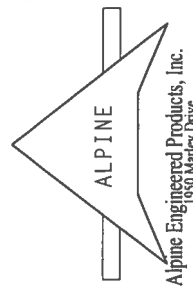
Scale = .3125"/Ft.

TC LL	20.0	PSF	REF	R487--	86645
TC DL	10.0	PSF	DATE	02/10/06	
BC DL	10.0	PSF	DRW	HCUSR487 06041027	
BC LL	0.0	PSF	HC-ENG	JB/AF *	
TOT.LD.	40.0	PSF	SEQN-	27960	
DUR.FAC.	1.25				

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSP 1.03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC., 530 NORTH DEARBORN, SUITE 200, MADISON, WI 53719, AND WCA (WOOD TRUSS COUNCIL), 1000 W. MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO BRACING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

*****IMPORTANT***** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE DESIGNER SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE DESIGN SHALL BE THE RESPONSIBILITY OF THE INSTALLING CONTRACTOR. THE TRUSS IS CONFORMANT WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY NAPA) AND TP/ACCS (TRUSS PRODUCTION/ACCEPTANCE CRITERIA SPECIFICATION) AND SHALL BE IDENTIFIED BY THE MANUFACTURER'S MARKING. THE TRUSS CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/5/8"X) ASTM A653 GRADE 40/60 (4" K/1/8") GALV. STEEL. APPLICABLE PLATES TO EACH FACE OF TRUSS AND, OTHERWISE, LOCATED ON THE DESIGN POSITION PER DRAWINGS 160A, 2, 3, 4, 5, 6, 7, 8, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 21, 22, 23, 24, 25, 26, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 47, 48, 49, 50, 51, 52, 53, 54, 55, 56, 57, 58, 59, 60, 61, 62, 63, 64, 65, 66, 67, 68, 69, 70, 71, 72, 73, 74, 75, 76, 77, 78, 79, 80, 81, 82, 83, 84, 85, 86, 87, 88, 89, 90, 91, 92, 93, 94, 95, 96, 97, 98, 99, 100, 101, 102, 103, 104, 105, 106, 107, 108, 109, 110, 111, 112, 113, 114, 115, 116, 117, 118, 119, 120, 121, 122, 123, 124, 125, 126, 127, 128, 129, 130, 131, 132, 133, 134, 135, 136, 137, 138, 139, 140, 141, 142, 143, 144, 145, 146, 147, 148, 149, 150, 151, 152, 153, 154, 155, 156, 157, 158, 159, 160, 161, 162, 163, 164, 165, 166, 167, 168, 169, 170, 171, 172, 173, 174, 175, 176, 177, 178, 179, 180, 181, 182, 183, 184, 185, 186, 187, 188, 189, 190, 191, 192, 193, 194, 195, 196, 197, 198, 199, 200, 201, 202, 203, 204, 205, 206, 207, 208, 209, 210, 211, 212, 213, 214, 215, 216, 217, 218, 219, 220, 221, 222, 223, 224, 225, 226, 227, 228, 229, 230, 231, 232, 233, 234, 235, 236, 237, 238, 239, 240, 241, 242, 243, 244, 245, 246, 247, 248, 249, 250, 251, 252, 253, 254, 255, 256, 257, 258, 259, 260, 261, 262, 263, 264, 265, 266, 267, 268, 269, 270, 271, 272, 273, 274, 275, 276, 277, 278, 279, 280, 281, 282, 283, 284, 285, 286, 287, 288, 289, 290, 291, 292, 293, 294, 295, 296, 297, 298, 299, 300, 301, 302, 303, 304, 305, 306, 307, 308, 309, 310, 311, 312, 313, 314, 315, 316, 317, 318, 319, 320, 321, 322, 323, 324, 325, 326, 327, 328, 329, 330, 331, 332, 333, 334, 335, 336, 337, 338, 339, 340, 341, 342, 343, 344, 345, 346, 347, 348, 349, 350, 351, 352, 353, 354, 355, 356, 357, 358, 359, 360, 361, 362, 363, 364, 365, 366, 367, 368, 369, 370, 371, 372, 373, 374, 375, 376, 377, 378, 379, 380, 381, 382, 383, 384, 385, 386, 387, 388, 389, 390, 391, 392, 393, 394, 395, 396, 397, 398, 399, 400, 401, 402, 403, 404, 405, 406, 407, 408, 409, 410, 411, 412, 413, 414, 415, 416, 417, 418, 419, 420, 421, 422, 423, 424, 425, 426, 427, 428, 429, 430, 431, 432, 433, 434, 435, 436, 437, 438, 439, 440, 441, 442, 443, 444, 445, 446, 447, 448, 449, 450, 451, 452, 453, 454, 455, 456, 457, 458, 459, 460, 461, 462, 463, 464, 465, 466, 467, 468, 469, 470, 471, 472, 473, 474, 475, 476, 477, 478, 479, 480, 481, 482, 483, 484, 485, 486, 487, 488, 489, 490, 491, 492, 493, 494, 495, 496, 497, 498, 499, 500, 501, 502, 503, 504, 505, 506, 507, 508, 509, 510, 511, 512, 513, 514, 515, 516, 517, 518, 519, 520, 521, 522, 523, 524, 525, 526, 527, 528, 529, 530, 531, 532, 533, 534, 535, 536, 537, 538, 539, 540, 541, 542, 543, 544, 545, 546, 547, 548, 549, 550, 551, 552, 553, 554, 555, 556, 557, 558, 559, 560, 561, 562, 563, 564, 565, 566, 567, 568, 569, 570, 571, 572, 573, 574, 575, 576, 577, 578, 579, 580, 581, 582, 583, 584, 585, 586, 587, 588, 589, 590, 591, 592, 593, 594, 595, 596, 597, 598, 599, 600, 601, 602, 603, 604, 605, 606, 607, 608, 609, 610, 611, 612, 613, 614, 615, 616, 617, 618, 619, 620, 621, 622, 623, 624, 625, 626, 627, 628, 629, 630, 631, 632, 633, 634, 635, 636, 637, 638, 639, 640, 641, 642, 643, 644, 645, 646, 647, 648, 649, 650, 651, 652, 653, 654, 655, 656, 657, 658, 659, 660, 661, 662, 663, 664, 665, 666, 667, 668, 669, 670, 671, 672, 673, 674, 675, 676, 677, 678, 679, 680, 681, 682, 683, 684, 685, 686, 687, 688, 689, 690, 691, 692, 693, 694, 695, 696, 697, 698, 699, 700, 701, 702, 703, 704, 705, 706, 707, 708, 709, 710, 711, 712, 713, 714, 715, 716, 717, 718, 719, 720, 721, 722, 723, 724, 725, 726, 727, 728, 729, 730, 731, 732, 733, 734, 735, 736, 737, 738, 739, 740, 741, 742, 743, 744, 745, 746, 747, 748, 749, 750, 751, 752, 753, 754, 755, 756, 757, 758, 759, 760, 761, 762, 763, 764, 765, 766, 767, 768, 769, 770, 771, 772, 773, 774, 775, 776, 777, 778, 779, 780, 781, 782, 783, 784, 785, 786, 787, 788, 789, 790, 791, 792, 793, 794, 795, 796, 797, 798, 79

PLT TYP. Wave



Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3
:Rt Stubbed Wedge 2x8 SP SS:

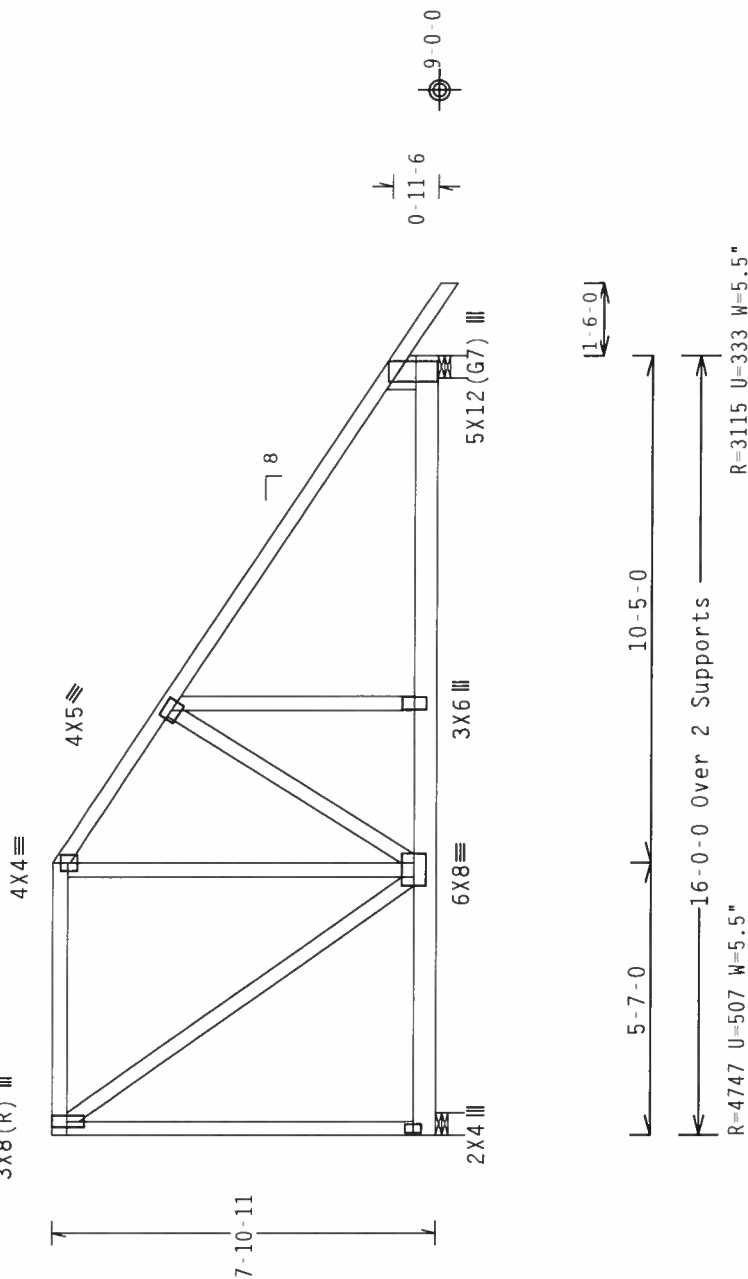
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.)_nails)
 Top Chord: 1 Row @12.00" o.c.
 Bot Chord: 1 Row @ 4.50" o.c.
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails
 in each row to avoid splitting.

SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)			
TC - From	64 PLF at 0.00 to	64 PLF at 17.50	
BC - From	20 PLF at 0.00 to	20 PLF at 0.05	
BC - From	532 PLF at 0.05 to	532 PLF at 7.85	
BC - From	264 PLF at 7.85 to	264 PLF at 7.92	
BC - From	20 PLF at 7.92 to	20 PLF at 16.00	
BC - From	5 PLF at 16.00 to	5 PLF at 17.50	
BC - 2400 LB Conc.	Load at 8.93		

2400 LB COLL. Load at 8.53
3X8(R) III



R=4747 U=507 W=5.5"

16-0-0 Over 2 Supports

 $R=3115 \quad U=333 \quad W=5.5''$

PLT TYP. Wave

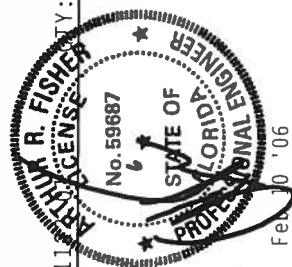
Design Crit: TPI-2002(STD)/FBC

$$Cq/RT=1.00(1.25)/10(0)$$

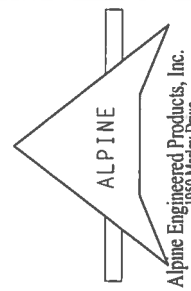
FL/-/4/-/-/R/-

Scale = .25"/Ft.

TC LL	20.0	PSF	REF	R487--	86646
TC DL	10.0	PSF	DATE	02/10/06	
BC DL	10.0	PSF	DRW	HCUSR487 06041091	
BC LL	0.0	PSF	HC-ENG JB/AF		
TOT.LD.	40.0	PSF	SEQN-	146706 REV	
DUR.FAC.	1.25				

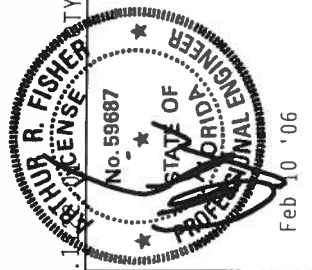


Feb 10 '06



Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

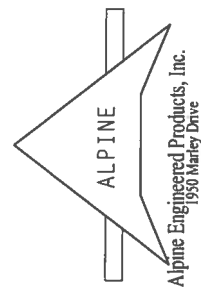
Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



8 FL / - 4 / - / - / R / -		Scale = .5" / Ft.	
TC LL	20.0 PSF	REF	R487 -- 86647
TC DL	10.0 PSF	DATE	02/10/06
BC DL	10.0 PSF	DRW	HCUSR487 06041028
BC LL	0.0 PSF	HC-ENG	JB/AF *
TOT.LD.	40.0 PSF	SEQN-	82453
DUR.FAC.	1.25		

10(0) 7.22.1

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. PROVIDE THE TRUSS RESPONSIBILITY TO THE TRUSS MANUFACTURER. THE TRUSS DESIGN CONFORMS WITH THE FOLLOWING PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ALPINE ENGINEERED PRODUCTS) AND THE TRUSS MANUFACTURER'S DESIGN SPEC. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH THE FOLLOWING PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY ALPINE ENGINEERED PRODUCTS) SHALL BE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER. THE TRUSS CONNECTOR PLATES ARE MADE OF 20/18/16GA (W/45/50) ASTM A653 GRADE 40/50 (IN. K/H/S) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THE DRAWINGS 16GA Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPII 2002 SEC.3. DRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT



110 mph wind, 20.18 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

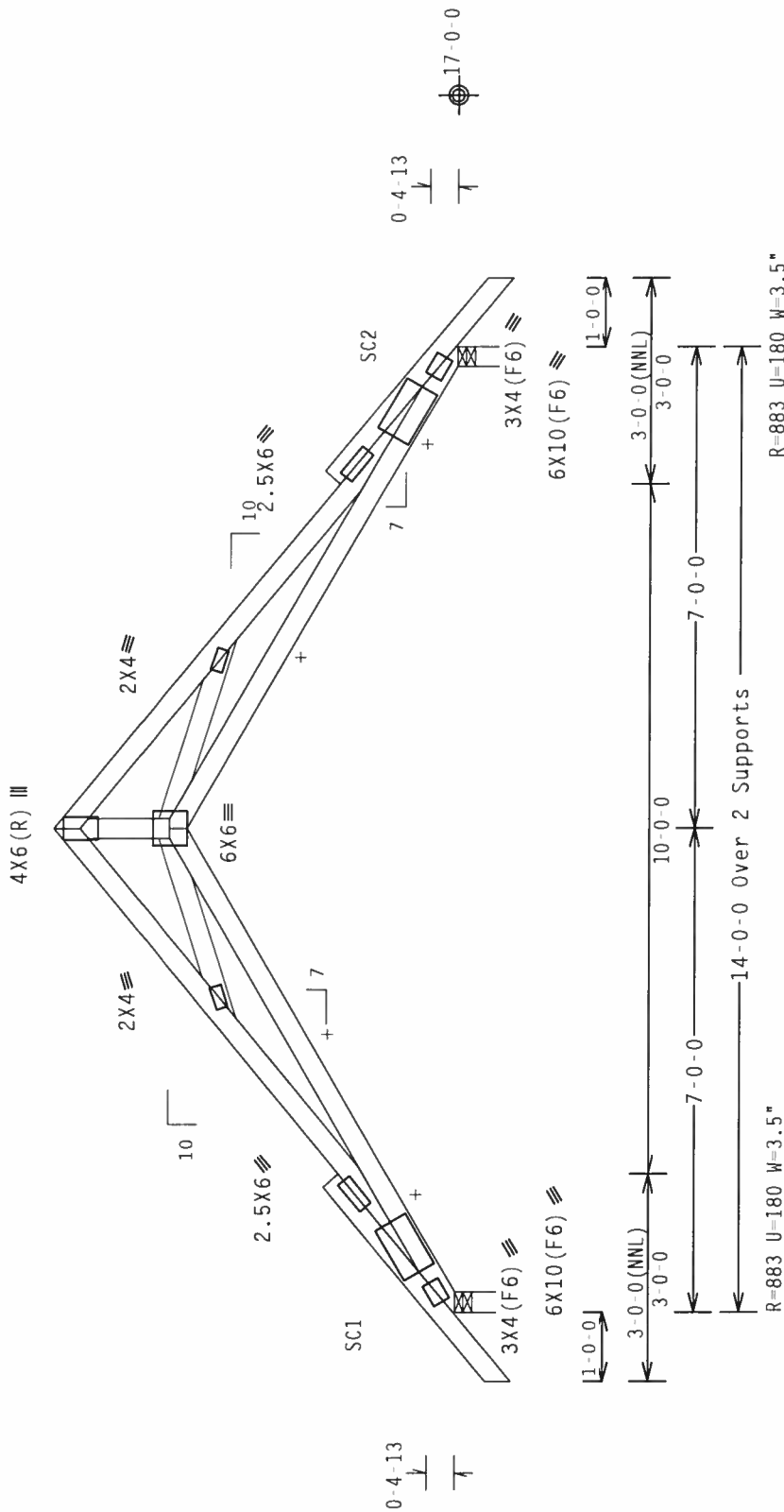
Calculated horizontal deflection is 0.27" due to live load and 0.41" due to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE DESIGN OF ROOF AND CEILING DIAPHRAGMS, GABLE END SHEAR WALLS, SUPPORTING SHEAR WALLS. SHEAR WALLS MUST PROVIDE CONTINUOUS LATERAL RESTRAINT TO GABLE END. ALL CONNECTIONS TO BE DESIGNED BY THE BUILDING DESIGNER

Stacked top chord must NOT be notched or cut in area (NWL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6

+ MEMBER TO BE Laterally Braced For Horizontal Wind Loads.
Bracing System To Be Designed And Furnished By Others.

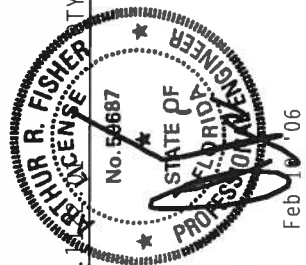


PLT TYP. Wave

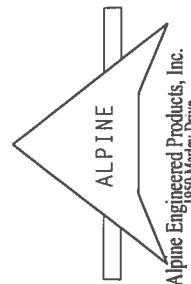
Design Crit: TPI-2002(STD)/FBC

 $Cq/RT=1.00(1.25)/10(0) \quad 7.22.1$
$$\text{Scale} = .375"/\text{Ft.}$$

TC LL	20.0	PSF	REF R487 - - 86648
TC DL	10.0	PSF	DATE 02/10/06
BC DL	10.0	PSF	DRW HCUR487 06041092
BC LL	0.0	PSF	HC-ENG JB/AF
TOT.LD.	40.0	PSF	SEQN- 82035
DUR.FAC.	1.25		



Feb 10, 06

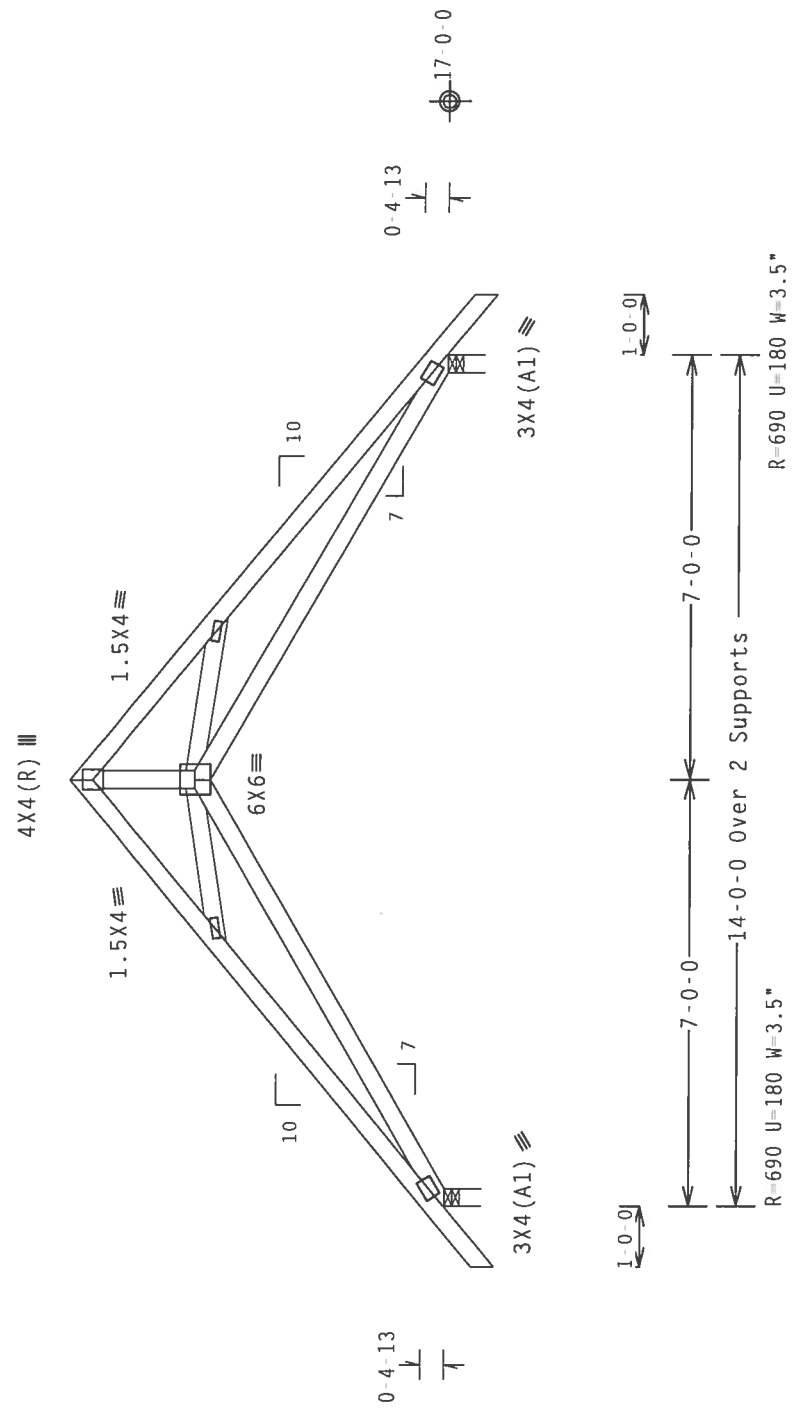


Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 19.90 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.11" due to live load and 0.19" due to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.

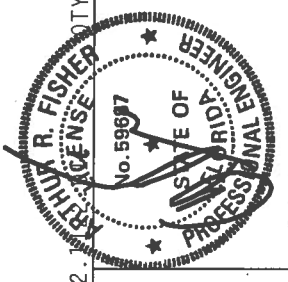


Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/10(0) 7.22

PLT TYP. Wave

Scale = .3125" / Ft.

TC LL	20.0 PSF	REF	R487 - 86649
TC DL	10.0 PSF	DATE	02/10/06
BC DL	10.0 PSF	DRW	HCUSR487 06041093
BC LL	0.0 PSF	HC-ENG	JB/AF
TOT.LD.	40.0 PSF	SEQN	27645
DUR.FAC.	1.25		



ALPINE
Alpine Engineered Products, Inc.
1950 Markey Drive

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST 1-03 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 583 D'ONOFIO DR., SUITE 200, MADISON, WI 53719) AND MTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

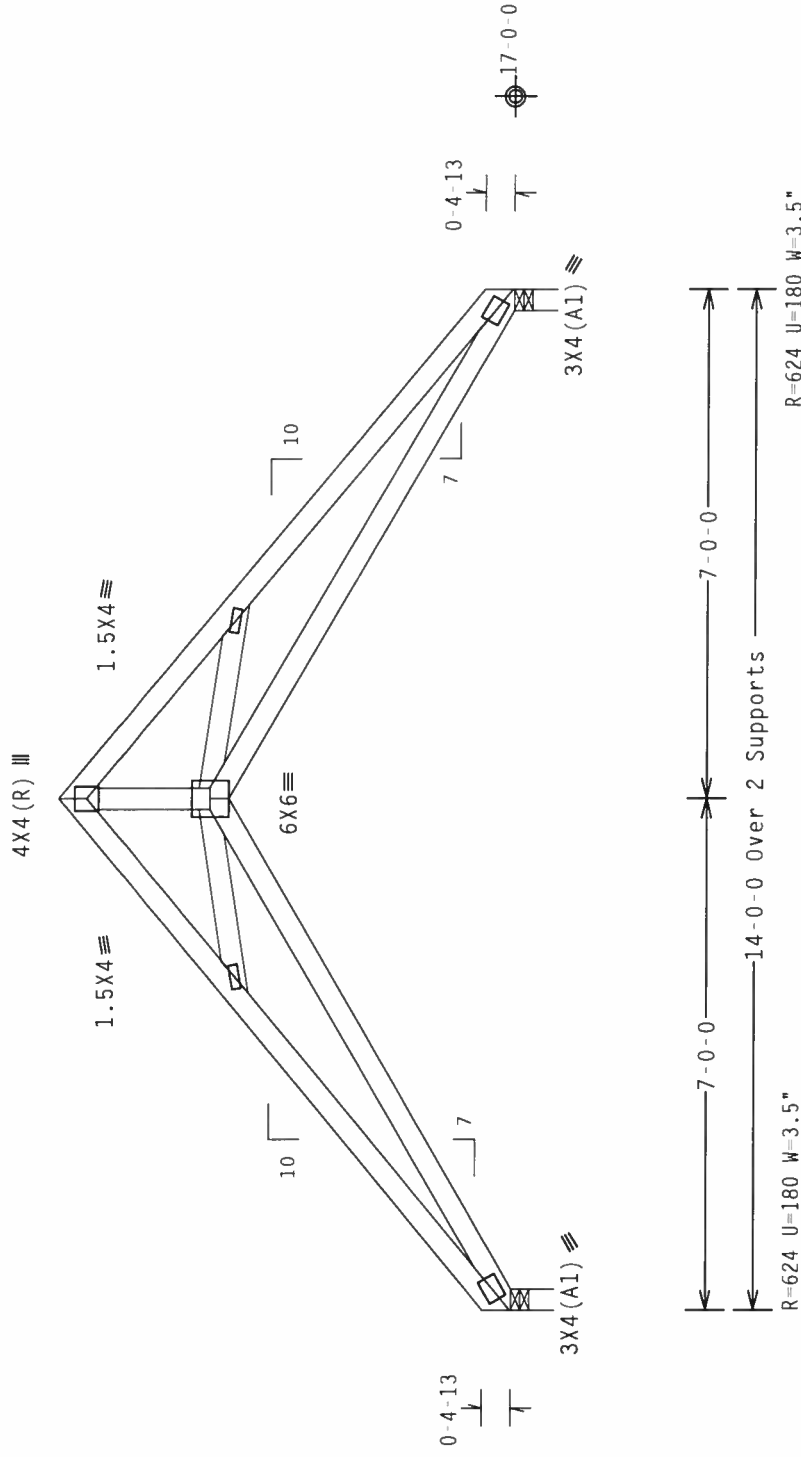
****IMPORTANT**** TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES, DESIGNER SHALL BE RESPONSIBLE. PROVISIONS OF AIA (NATIONAL DESIGN SPEC. BY AIA) AND TPI. ALPINE ENGINEERED PRODUCTS, INC. SHALL BE RESPONSIBLE FOR THE DESIGN OF THE TRUSS. APPLY ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI 2002 SEC. 1.1. DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

Top chord	2x4	SP	#2	Dense
Bot chord	2x4	SP <td>#2</td> <td>Dense</td>	#2	Dense
webs	2x4	SP	#3	

110 mph wind, 20.32 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Calculated horizontal deflection is 0.10" due to live load and 0.19" due to dead load.

Deflection meets L/360 live and L/240 total load. Creep increase factor for dead load is 1.50.



R=624 U=180 W=3.5"

R=624 U=180 W=3.5"

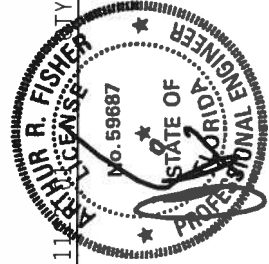
PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC

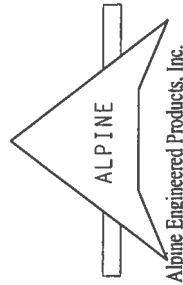
7.22.11

Scale = .375"/Ft.

TC LL	20.0	PSF	REF	R487--	86650
TC DL	10.0	PSF	DATE	02/10/06	
BC DL	10.0	PSF	DRW	HCUSR487	06041094
BC LL	0.0	PSF	HC-ENG	JB/AF	
TOT.LD.	40.0	PSF	SEQN-	27639	
DUR.FAC.	1.25				



Feb 10 '06



ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE.
FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE
BRACING.

WEB MEMBER SIZE	SPECIFIED CLB BRACING	T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

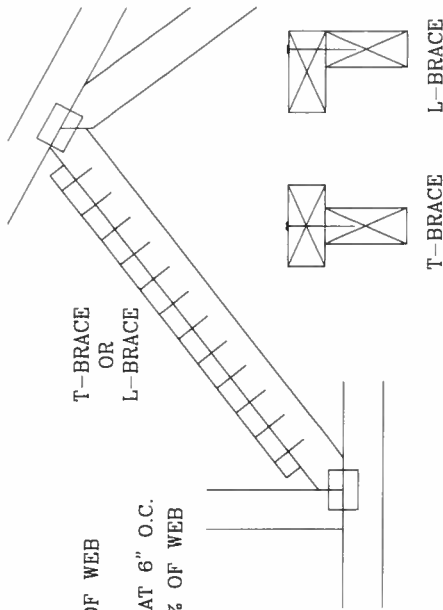
T-BRACING

OR
L-BRACING:

APPLY TO EITHER SIDE OF WEB
NARROW FACE

ATTACH WITH 16d NAILS AT 6" O.C.

BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH

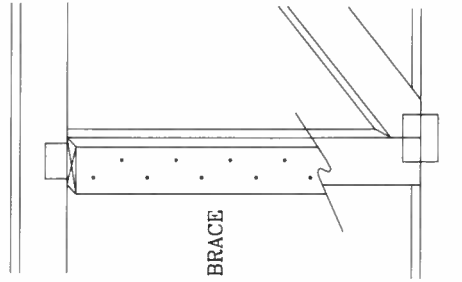


SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.

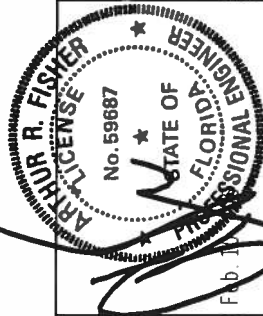
ATTACH WITH 10d OR .128"x3" GUN

NAILS AT 6" O.C. BRACE IS A MINIMUM
80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	BRCCLBSUB1103
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTING. ALWAYS FOLLOW THE SAFETY INFORMATION PUBLISHED BY TPI TRUSS COMPANY, INC. (TPI TRUSS COMPANY, INC., 500 WINDERMERE DR., WINDERMERE, IL 60093) AND THE TPI TRUSS COMPANY OF AMERICA, 6300 WINDERMERE DR., WINDERMERE, IL 60093, FOR SAFETY PRACTICES PRIOR TO PROCEEDING TO THE NEXT STEP. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT: FURNISH COPY OF THIS DESIGN FOR ANY DEVIATION FROM CONTRACTOR PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & MAINTAINING THE TRUSSES IN CONFORMANCE WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPEC. FOR WOOD TRUSSES, AND ANY FAILURE TO FOLLOW THE DESIGN AND CONSTRUCTION DETAILS LOCATED ON 40/60 (W/K/H/S), GUSSET PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON 40/60 (W/K/H/S), POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE IN ACCORDANCE WITH SECTION 3.2 OF THE 2008 IBC. A SEAL ON THE DRAWING INDICATES ACCEPTANCE OF THE PROFESSIONAL ENGINEER'S REVIEW OF THE TRUSSES FOR CONFORMANCE WITH THE DESIGN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING OWNER.



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

BEARING BLOCK NAIL SPACING DETAIL

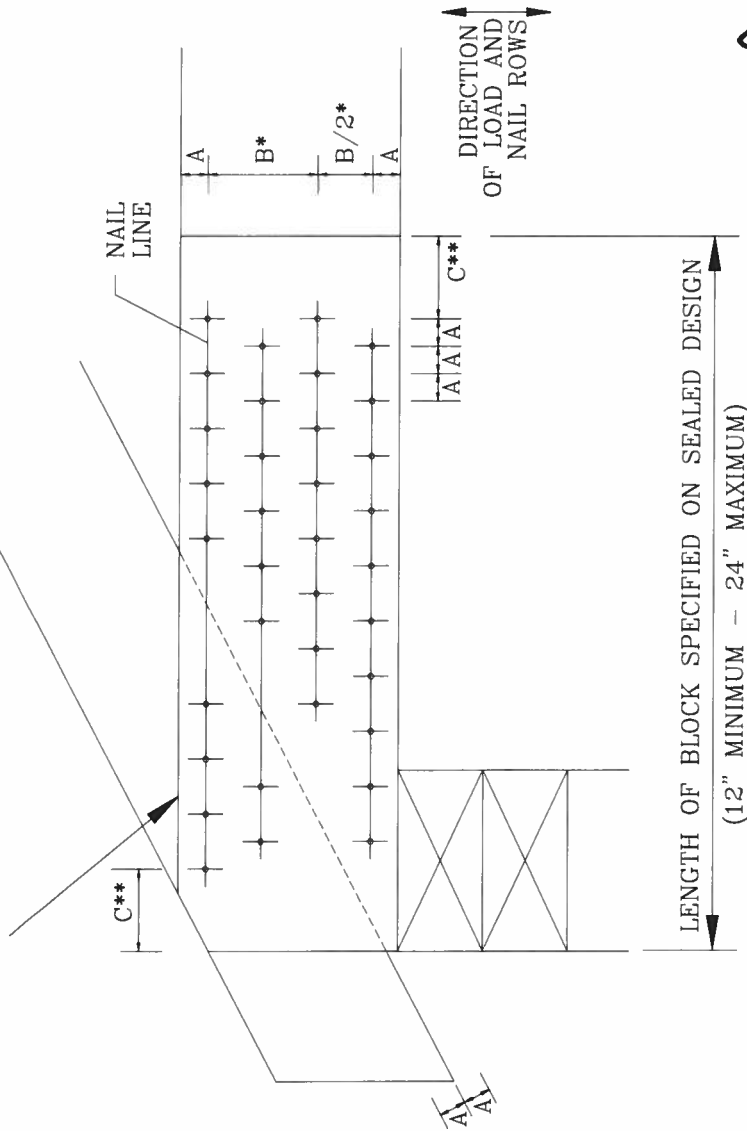
MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

- SPACING MAY BE REDUCED BY 50%
- SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (F_c-perp) IS AT LEAST THAT OF THE CHORD.



MAXIMUM NUMBER OF NAIL LINES PARALLEL TO GRAIN

NAIL TYPE	CHORD SIZE			
	2X4	2X6	2X8	2X10 2X12
8d BOX (0.113"x2.5")	3	6	9	12 15
10d BOX (0.128"x3")	3	5	7	10 12
12d BOX (0.128"x3.25")	3	5	7	10 12
16d BOX (0.135"x3.5")	3	5	7	10 12
20d BOX (0.148"x4")	2	4	5	6 8
8d COMMON (0.131"x2.5")	3	5	7	10 12
10d COMMON (0.148"x3")	2	4	6	8 10
12d COMMON (0.148"x3.25")	2	4	6	8 10
16d COMMON (0.162"x3.5")	2	4	6	8 10
0.120"x2.5" GUN	3	6	8	11 14
0.131"x2.5" GUN	3	5	7	10 12
0.120"x3.0" GUN	3	6	8	11 14
0.131"x3.0" GUN	3	5	7	10 12

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES		
	A	B*	C**
8d BOX (0.113"x2.5")	3/4"	1 3/8"	1 3/4"
10d BOX (0.128"x3")	7/8"	1 5/8"	2"
12d BOX (0.128"x3.25")	7/8"	1 5/8"	2"
16d BOX (0.135"x3.5")	7/8"	1 5/8"	2 1/8"
20d BOX (0.148"x4")	1"	1 7/8"	2 1/4"
8d COMMON (0.131"x2.5")	7/8"	1 5/8"	2"
10d COMMON (0.148"x3")	1"	1 7/8"	2 1/4"
12d COMMON (0.148"x3.25")	1"	1 7/8"	2 1/4"
16d COMMON (0.162"x3.5")	1"	2"	2 1/2"
0.120"x2.5" GUN	3/4"	1 1/2"	1 7/8"
0.131"x2.5" GUN	7/8"	1 5/8"	2"
0.120"x3.0" GUN	3/4"	1 1/2"	1 7/8"
0.131"x3.0" GUN	7/8"	1 5/8"	2"

THIS DRAWING REPLACES DRAWING B139 AND CNBRGK0699

REF	BEARING BLOCK
DATE	11/26/03
DRWG	CNBRGK1103
-ENG	SJP/KAR



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST PRACTICES (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS & BRACING INSTITUTE) AND THE NATIONAL TRUSS MANUFACTURERS ASSOCIATION (NTMA) FOR THE LATEST TRUSS DESIGN AND BRACING REQUIREMENTS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AIA/ASCE AND TPI). ALPINE CONNECTOR PLATES ARE MADE OF 2018/16GA (W/H/S/K) ASTM A653 GRADE 50. THIS DESIGN POSITION PER DRAWING 165-2. ON THIS DESIGN, THE SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SEAL AND TPI SEAL ARE NOT TO BE USED FOR ANY OTHER PURPOSE.

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																
2x4 GABLE VERTICAL SPACING		BRACE		NO BRACES	(1) 1x4 "L" BRACE •						(2) 2x4 "L" BRACE ••		(1) 2x6 "L" BRACE •		(2) 2x6 "L" BRACE •	
		GRADE	BRACE		GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF	#1 / #2		3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"	14' 0"	
	HF	#3		3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"	14' 0"	
		STUD		3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"	14' 0"	
		STANDARD		3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"	14' 0"	
24" O.C.	SP	#1		4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	
		#2		4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"	14' 0"	
		#3		4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"	14' 0"	
		STUD		4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"	14' 0"	
24" O.C.	DFL	STANDARD		3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	
		#1 / #2		4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3		4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		STUD		4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
16" O.C.	HF	STANDARD		4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"	14' 0"	
		#1		4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2		4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3		4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
16" O.C.	DFL	STUD		4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD		4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"	14' 0"	
		#1 / #2		4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3		4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.	HF	STUD		4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		STANDARD		4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#1		5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#2		5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.	SP			5' 0"	8' 5"	9' 1"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
		#3		5' 0"	8' 5"	9' 1"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
12" O.C.	DFL	STUD		4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	

MAX GABLE VERTICAL LENGTH

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	HEM-FIR
#1 / #2	#2
STANDARD	STUD
#3	STANDARD

DOUGLAS FIR-LARCH

#3	STUD
STANDARD	STANDARD

SOUTHERN PINE

#3	STUD
STANDARD	STANDARD

GROUP B:

HEM-FIR	#1 & BTR
#1	#1

DOUGLAS FIR-LARCH

#1	#2
----	----

SOUTHERN PINE

#1	#2
----	----

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0"

OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

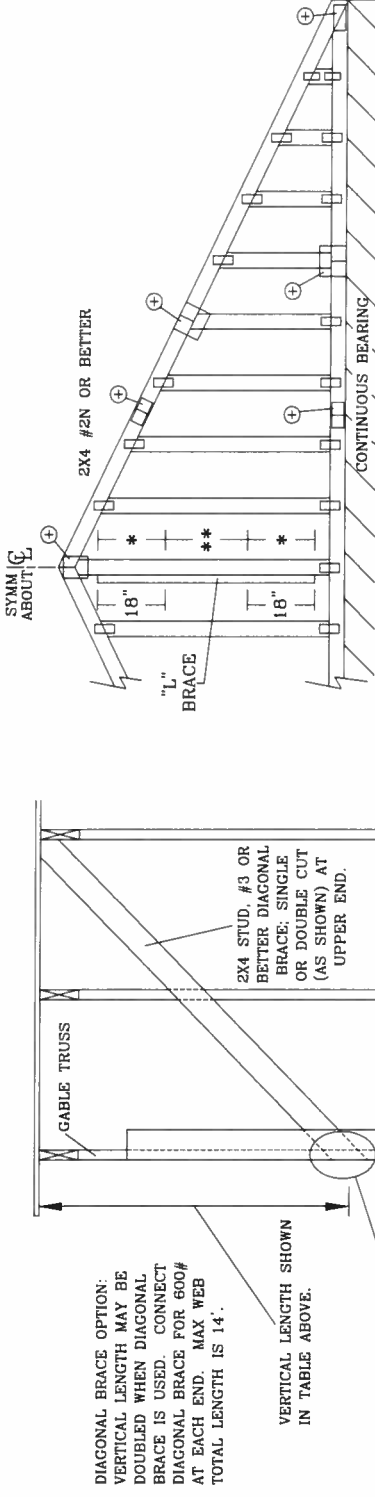
ATTACH EACH "L" BRACE WITH 10d NAILS:

* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH	NO SPLICE	
LESS THAN 4' 0"	1X4 OR 2X3	
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	
GREATER THAN 11' 6"	2.5X4	
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.		



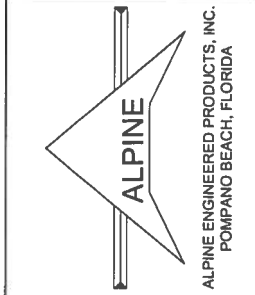
REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

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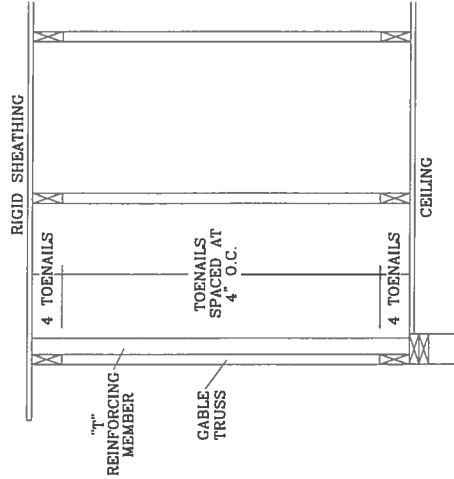
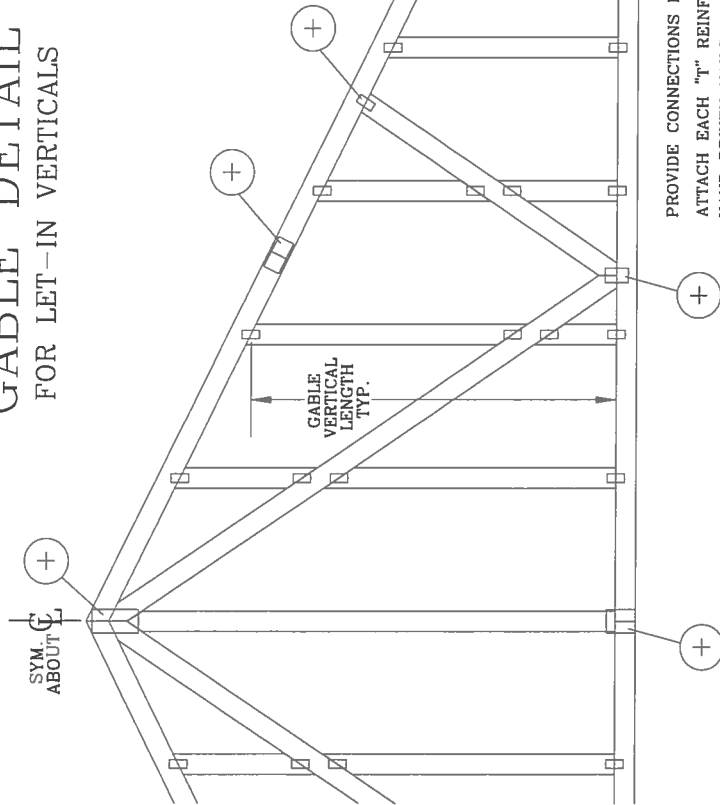
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REF	ASCE7-02-CAB11015
DATE	04/15/05
DRWG	A11015EE0405
-ENG	

MAX. TOT. LD. 60 PSF



GABLE DETAIL FOR LET-IN VERTICALS



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH

HAND DRIVEN NAILS:

- 10d COMMON (0.148" X 3" MIN) TOENAILS AT 4" O.C. PLUS
- (4) 16d COMMON (0.162" X 3.5" MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:

- 8d COMMON (0.131" X 2.5" MIN) TOENAILS AT 4" O.C. PLUS
- (4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

- ASCE 7-93 GABLE DETAIL DRAWINGS
 - A11015EN1103, A09015EN1103, A08015EN1103, A07015EN1103
 - A11030EN1103, A10030EN1103, A09030EN1103, A08030EN1103, A07030EN1103
- ASCE 7-98 GABLE DETAIL DRAWINGS
 - A13015EC1103, A12015EC1103, A11015EC1103, A08515EC1103
 - A13030EC1103, A12030EC1103, A11030EC1103, A08530EC1103
- ASCE 7-02 GABLE DETAIL DRAWINGS
 - A13015EE0405, A12015EE0405, A11015EE0405, A08515EE0405, A07015EE0405
 - A13030EE0405, A12030EE0405, A11030EE0405, A08530EE0405, A07030EE0405

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

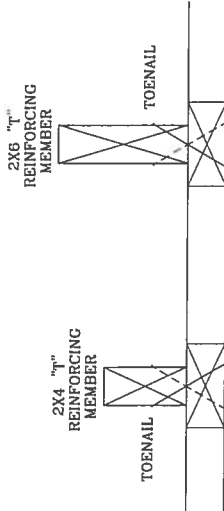
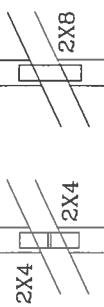
GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE:



TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	20 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:

ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT

GABLE VERTICAL = 24" O.C. SP #3

"T" REINFORCING MEMBER SIZE = 2X4

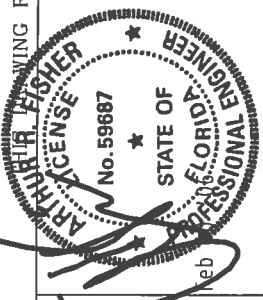
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10

(1) 2X4 "L" BRACE LENGTH = 6' 7"

MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"

REPLACES DRAWINGS GAB98117 876.719 & HC26294035

REF	LET-IN VERT
DATE	04/14/05
DRWG	GBLLETIN0405
-ENG	DLJ/KAR
MAX TOT. LD.	60 PSF
DUR. FAC.	ANY



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ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA

TOP	CHORD	2X4	#2 OR BETTER
BOT	CHORD	2X4	#2 OR BETTER
	WEBS	2X4	#3 OR BETTER

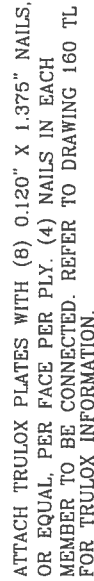
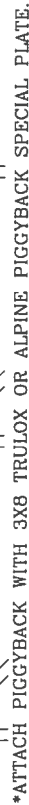
SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

130 MPH WIND, 30' MEAN HGT., ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE
PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

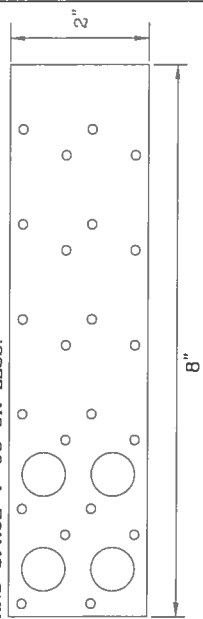


JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7' 9"	NO BRACING
7' 9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5" MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5" MIN) NAILS AT 4" OC.

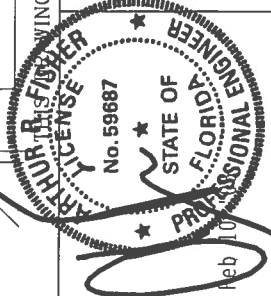
* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THOMPSON SUBMACHINE GUNS
REPAIR & REBUILDING
REPLACES DRAWINGS 634.016 634.017 & 847.045

MAX LOADING	REF	PIGGYBACK
55 PSF AT	DATE	04/14/05
1.33 DUR. FAC.	DRWG	PIGBACKB0405
50 PSF AT	-ENG	DLJ/KAR
1.25 DUR. FAC.		
47 PSF AT		
1.15 DUR. FAC.		



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND REMOVING. BECAUSE OF THE EXTREME STRENGTH OF THE TRUSSES, THE FOLLOWING INFORMATION IS PRESENTED FOR THE USER'S REFERENCE. PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 5820 PINEVIEW DR, SUITE 100, WESTMINSTER, CO 80540-1000, USA) AND THE NATIONAL ASSOCIATION OF TRUSS ROOFING CONTRACTORS OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719. FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, THE USER SHALL HAVE PROPERLY ATTACHED TO THE TRUSSES, THE FOLLOWING INFORMATION. THE TRUSSES SHALL HAVE PROPERLY ATTACHED RIGID CLING. STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CLING.

*****IMPORTANT***** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. DESIGNING AIRPLANE ENGINEERED PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING, OR BRACING THE TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, 2018 EDITION), AISC (STEEL ECTION, 2016 EDITION), AND 2018 IBC (INTERNATIONAL BUILDING CODE). THE TRUSS SHALL BE FABRICATED IN CONFORMANCE WITH THE 2018 IBC (INTERNATIONAL BUILDING CODE) AS THE AISC GRADE, 40/60 (A36/50) GALV. STEEL. APPLY PLATES TO EACH CHORD OF TRUSS. INSPECTION OF TRUSSES SHALL BE CONDUCTED IN CONFORMANCE WITH THE 2018 IBC (INTERNATIONAL BUILDING CODE) AND THE 2018 IBC (INTERNATIONAL BUILDING CODE) SHALL BE PER ANNEX A3 OF TPI 1-20002 SEC. 3. A SEAL ON THE DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE



ALPINE ENGINEERED PRODUCTS, INC.
POMPANO BEACH, FLORIDA