

Job L287710	Truss CJ1	Truss Type JACK	Qty 2	Ply 1	CASH ACCT. - STAPE RES. L287710001 Job Reference (optional)
Builders FirstSource, Lake City, Fl 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:44:58 2008 Page 1		

Scale = 1:15.0

Plate Offsets (X,Y): [5:0-4-12,0-1-8]									
LOADING (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	0.00	5	>999	360
TCDL 7.0	Lumber Increase	1.25	BC	0.04	Vert(TL)	0.00	5	>999	240
BCLL 10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)						
					PLATES	GRIP			
					MT20	244/190			
					Weight: 10 lb				

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3	BRACING TOP CHORD Structural wood sheathing directly applied or 1-9-10 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
--	--

REACTIONS (lb/size) 5=186/0-5-8, 4=-4/Mechanical, 3=13/Mechanical
 Max Horz 5=166(load case 6)
 Max Uplift 5=-114(load case 6), 4=-7(load case 7), 3=-31(load case 7)
 Max Grav 5=186(load case 1), 4=20(load case 2), 3=21(load case 4)

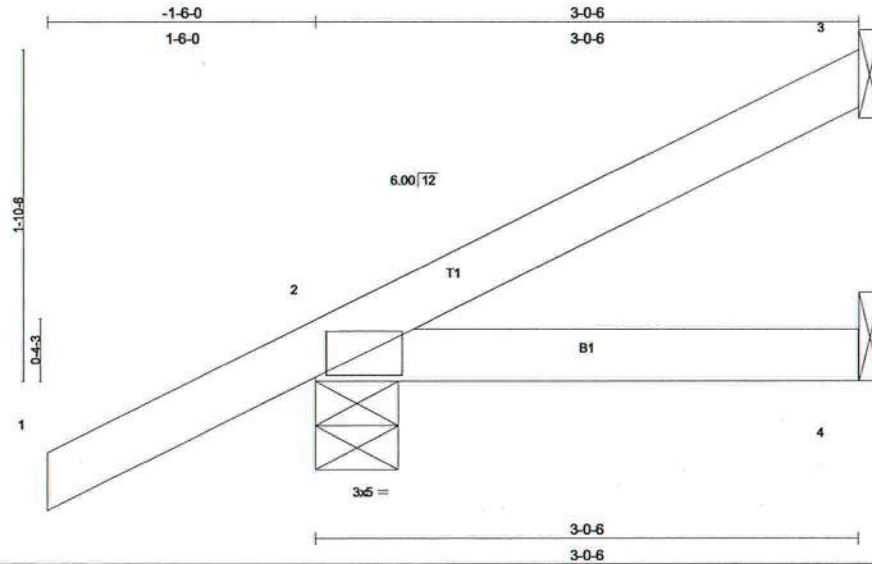
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 2-5=-165/149, 1-2=0/57, 2-3=49/12
 BOT CHORD 4-5=0/0

JOINT STRESS INDEX
 2 = 0.00 and 5 = 0.54

NOTES (5)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 5, 7 lb uplift at joint 4 and 31 lb uplift at joint 3.
 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss CJ3	Truss Type JACK	Qty 3	Ply 1	CASH ACCT. - STAPE RES. L287710002 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:00 2008 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.18	in (loc) l/def L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.05	Vert(LL) -0.00 2-4 >999 360		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.01 2-4 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2004/TPI2002			Weight: 12 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-6 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=45/Mechanical, 2=211/0-5-8, 4=14/Mechanical
Max Horz 2=116(load case 6)
Max Uplift 3=35(load case 6), 2=158(load case 6)
Max Grav 3=45(load case 1), 2=211(load case 1), 4=41(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/35, 2-3=50/14
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 3 and 158 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ6	Truss Type JACK	Qty 3	Ply 1	CASH ACCT. - STAPE RES. L287710003 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:00 2008 Page 1		

Scale = 1";19.8'

LOADING (psf) TCDL 20.0 TCDL 7.0 BCCL 10.0 BCDL 5.0	SPACING 2'-0" Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.37 BC 0.23 WB 0.00 (Matrix)	DEFL in (loc) l/defl L/d Vert(LL) -0.06 2-4 >999 360 Vert(TL) -0.10 2-4 >702 240 Horz(TL) -0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 22 lb
--	---	---	---	--

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins. BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.
---	---

REACTIONS (lb/size) 3=140/Mechanical, 2=290/0-5-8, 4=29/Mechanical
 Max Horz 2=185(load case 6)
 Max Uplift 3=127(load case 6), 2=169(load case 6)
 Max Grav 3=140(load case 1), 2=290(load case 1), 4=86(load case 2)

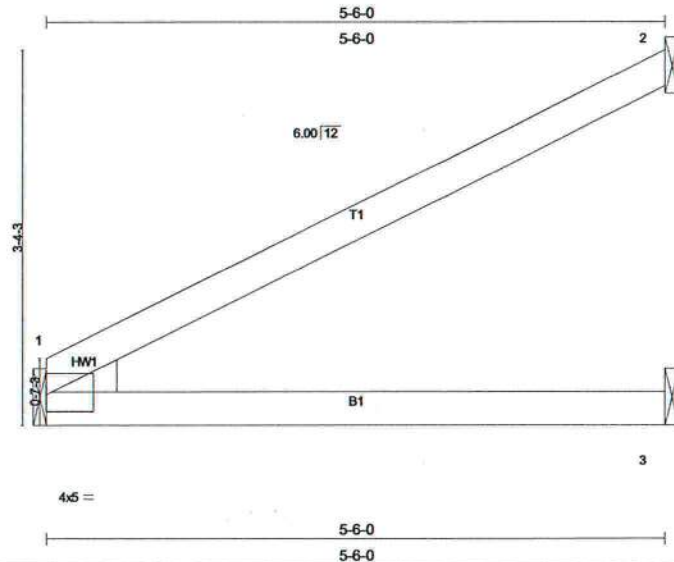
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/35, 2-3=116/51
 BOT CHORD 2-4=0/0

JOINT STRESS INDEX
 2 = 0.17

NOTES (5)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 127 lb uplift at joint 3 and 169 lb uplift at joint 2.
 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ6A	Truss Type JACK	Qty 1	Ply 1	CASH ACCT. - STAPE RES. L287710004 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:01 2008 Page 1		



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.43	Vert(LL)	-0.04	1-3	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.21	Vert(TL)	-0.08	1-3	>841	240		
BCLL 10.0 *	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2004/TPI2002								
								Weight: 18 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEDGE
Left 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=145/Mechanical, 3=27/Mechanical, 1=172/Mechanical
Max Horz 1=139(load case 6)
Max Uplift2=140(load case 6), 1=46(load case 6)
Max Grav 2=145(load case 1), 3=81(load case 2), 1=172(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=127/57
BOT CHORD 1-3=0/0

JOINT STRESS INDEX

1 = 0.15 and 1 = 0.00

NOTES (5)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 140 lb uplift at joint 2 and 46 lb uplift at joint 1.
- 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ7	Truss Type MONO TRUSS	Qty 9	Ply 1	CASH ACCT. - STAPE RES. L287710005 Job Reference (optional)
----------------	--------------	--------------------------	----------	----------	---

Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MITek Industries, Inc. Fri Sep 12 13:45:02 2008 Page 1

Scale = 1/32
Camber = 1/16 in

Plate Offsets (X,Y): [2-0-3-8,Edge]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	0.13	2-5	>655	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.32	Vert(TL)	-0.14	2-5	>588	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.05	4	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 38 lb										

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
SLIDER Left 2 X 6 SYP No.1D 4-8-1

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=169/Mechanical, 2=312/0-5-8, 5=45/Mechanical
Max Horz 2=239(load case 6)
Max Uplift 4=145(load case 6), 2=42(load case 6)
Max Grav 4=169(load case 1), 2=312(load case 1), 5=88(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=190/6, 3-4=170/83
BOT CHORD 2-5=0/0

JOINT STRESS INDEX
2 = 0.61, 2 = 0.06, 2 = 0.06 and 3 = 0.00

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 145 lb uplift at joint 4 and 42 lb uplift at joint 2.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ7A	Truss Type SPECIAL	Qty 7	Ply 1	CASH ACCT. - STAPE RES. L287710006 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6,300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:03 2008 Page 1		

Scale = 1:37.7

Plate Offsets (X,Y): [2-0-3-4,0-0-12]										
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/def	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.24	Vert(LL)	0.03	7-8	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.45	Vert(TL)	-0.04	7-8	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.13	Horz(TL)	-0.02	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
Weight: 41 lb										

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except* B2 2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3 SLIDER Left 2 X 6 SYP No.1D 1-10-1	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
--	---

REACTIONS (lb/size) 5=99/Mechanical, 2=312/0-5-8, 6=114/Mechanical
 Max Horz 2=239(load case 6)
 Max Uplift 5=84(load case 6), 2=42(load case 6), 6=60(load case 6)
 Max Grav 5=99(load case 1), 2=312(load case 1), 6=124(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/36, 2-3=276/0, 3-4=220/0, 4-5=113/48
 BOT CHORD 2-9=208/142, 8-9=31/56, 4-8=16/140, 7-8=381/297, 6-7=0/0
 WEBS 4-7=313/401

JOINT STRESS INDEX
 2 = 0.19, 2 = 0.14, 3 = 0.00, 4 = 0.67, 7 = 0.13, 8 = 0.68 and 9 = 0.60

NOTES (5)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 5, 42 lb uplift at joint 2 and 60 lb uplift at joint 6.
 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ7B	Truss Type MONO HIP	Qty 4	Ply 1	CASH ACCT. - STAPE RES. L287710007 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MITek Industries, Inc. Fri Sep 12 13:45:04 2008 Page 1		

Scale = 1:41.8

Plate Offsets (X,Y): [2-0-5-10,Edge], [4-0-3-8,Edge]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL 20.0	Plates Increase	1.25	TC 0.61	Vert(LL) 0.06	2-5 >999 360
TCCL 7.0	Lumber Increase	1.25	BC 0.28	Vert(TL) -0.08	2-5 >997 240
BCCL 10.0	Rep Stress Incr	YES	WB 0.00	Horz(TL) -0.00	5 n/a n/a
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)		
			PLATES		GRIP
			MT20		244/190
			Weight: 46 lb		

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 SLIDER Left 2 X 6 SYP No.1D 4-5-12	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
--	---

REACTIONS (lb/size) 5=210/Mechanical, 2=309/0-5-8
 Max Horz 2=237(load case 6)
 Max Uplift 5=142(load case 6), 2=42(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/36, 2-3=176/0, 3-4=130/72, 4-5=155/227
 BOT CHORD 2-5=44/35

JOINT STRESS INDEX
 2 = 0.54, 2 = 0.05, 2 = 0.05, 3 = 0.00, 4 = 0.61 and 5 = 0.84

NOTES (5)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCCL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip
 DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 142 lb uplift at joint 5 and 42 lb uplift at joint 2.
 5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ7C	Truss Type MONO HIP	Qty 3	Ply 1	CASH ACCT. - STAPE RES. L287710008 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055					6,300 s Apr 19 2006 MITek Industries, Inc. Fri Sep 12 13:45:05 2008 Page 1

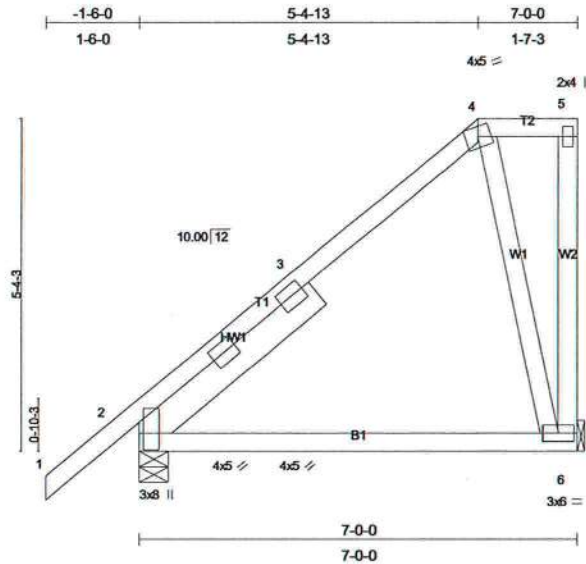


Plate Offsets (X,Y): [2-0-5,0-0-13]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.26	Vert(LL)	-0.04	2-6	>999	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.16	Vert(TL)	-0.08	2-6	>999	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.10	Horz(TL)	-0.00	6	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 49 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 3-7-8

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 6=210/Mechanical, 2=309/0-5-8
Max Horz 2=199(load case 6)
Max Uplift 6=96(load case 6), 2=71(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=188/0, 3-4=88/9, 4-5=19/0, 5-6=74/42
BOT CHORD 2-6=87/67
WEBS 4-6=215/318

JOINT STRESS INDEX

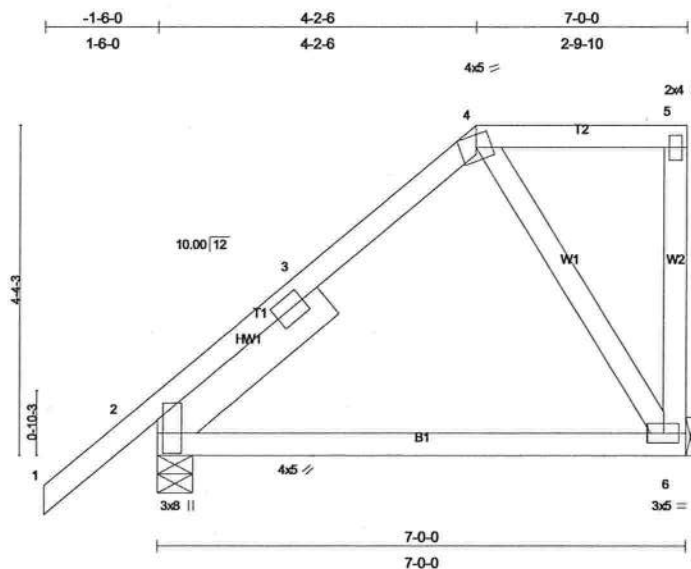
2 = 0.72, 2 = 0.05, 2 = 0.05, 3 = 0.00, 4 = 0.54, 5 = 0.29 and 6 = 0.68

NOTES (6)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 96 lb uplift at joint 6 and 71 lb uplift at joint 2.
- 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ7D	Truss Type MONO HIP	Qty 3	Ply 1	CASH ACCT. - STAPE RES. L287710009 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:06 2008 Page 1		



Scale = 1:29.3

Plate Offsets (X,Y): [2-0-5-6,0-0-13]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.28	Vert(LL) -0.04 2-6 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.17	Vert(TL) -0.07 2-6 >999 240		
BCLL 10.0	Rep Stress Incr YES	WB 0.06	Horz(TL) -0.00 6 n/a n/a		
BCDL 5.0	Code FBC2004/TP12002	(Matrix)			
				Weight: 45 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-10-2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 6=210/Mechanical, 2=309/0-5-8
Max Horz 2=167(load case 6)
Max Uplift 6=74(load case 5), 2=88(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/36, 2-3=205/34, 3-4=120/45, 4-5=24/0, 5-6=54/47
BOT CHORD 2-6=113/92
WEBS 4-6=146/206

JOINT STRESS INDEX

2 = 0.75, 2 = 0.11, 3 = 0.00, 4 = 0.33, 5 = 0.22 and 6 = 0.47

NOTES (6)

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 74 lb uplift at joint 6 and 88 lb uplift at joint 2.
- 6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss EJ7E	Truss Type MONO HIP	Qty 2	Ply 1	CASH ACCT. - STAPE RES. L287710010 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:07 2008 Page 1		

Scale: 1/2"=1'

Plate Offsets (X,Y): [2:0-1-9,0-1-1]									
LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.33	Vert(LL)	-0.00	5-6	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(TL)	-0.01	5-6	>999	240		
BCLL 10.0	Lumber Increase 1.25	WB 0.13	Horz(TL)	0.00	5	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								
									Weight: 46 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
SLIDER Left 2 X 8 SYP No.1D 2-0-0	

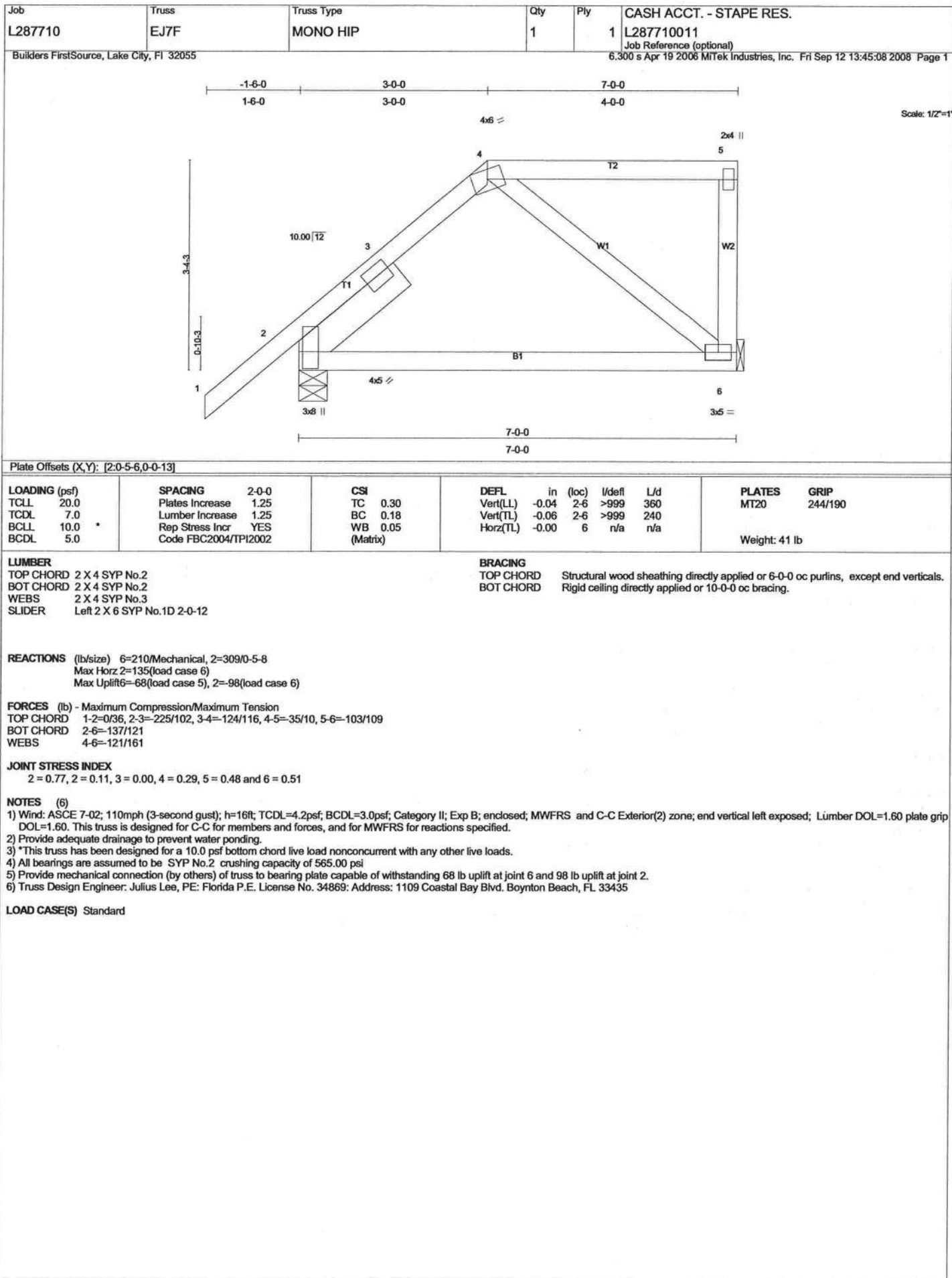
REACTIONS (lb/size) 5=440/Mechanical, 2=451/0-5-8
Max Horz 2=136(load case 5)
Max Uplift 5=247(load case 4), 2=208(load case 5)

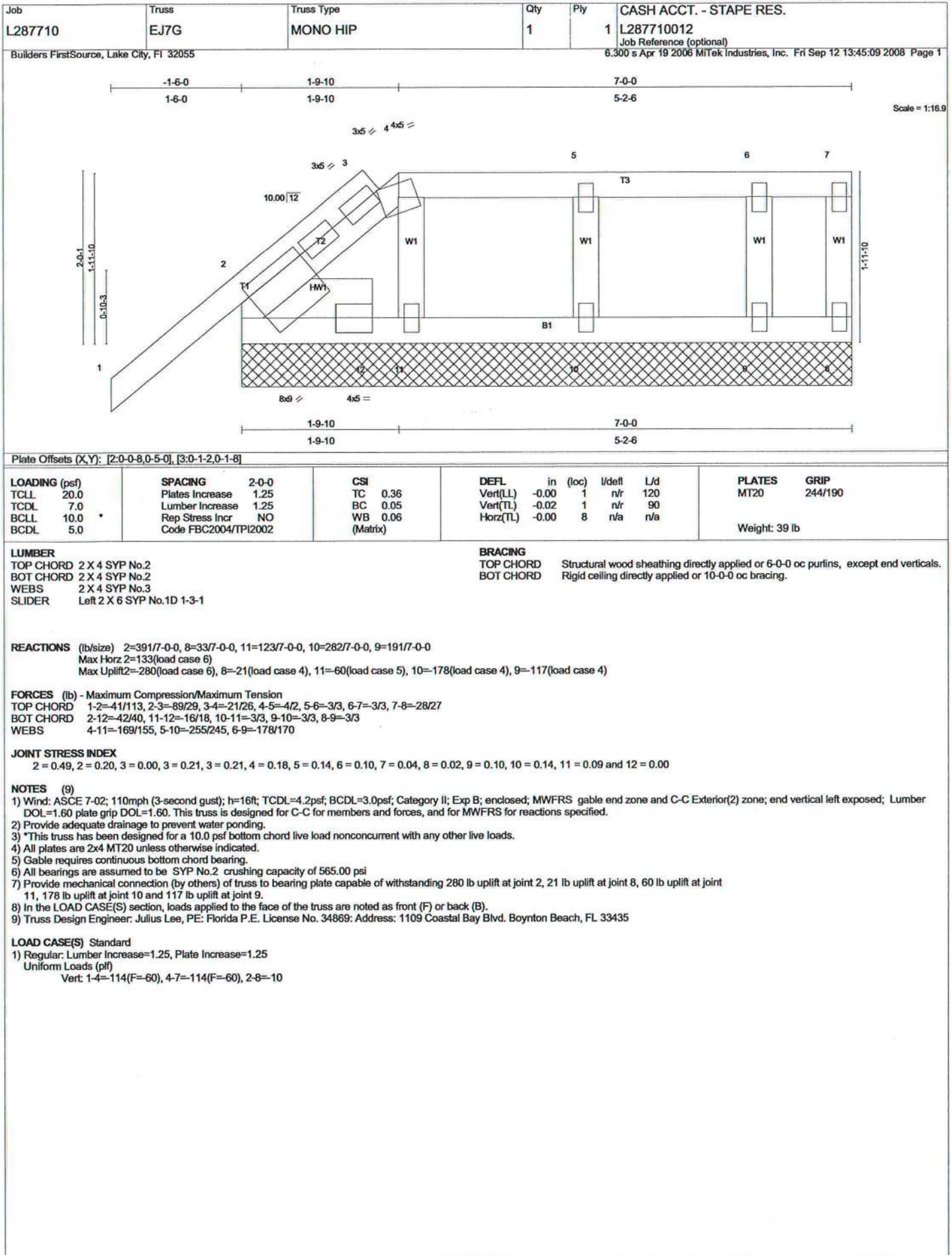
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=438/183, 3-4=43/26, 4-5=198/158
BOT CHORD 2-7=170/282, 6-7=170/282, 5-6=173/290
WEBS 3-6=64/180, 3-5=315/191

JOINT STRESS INDEX
2 = 0.89, 2 = 0.11, 3 = 0.42, 4 = 0.89, 5 = 0.30, 6 = 0.13 and 7 = 0.00

NOTES (9-10)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Provide adequate drainage to prevent water ponding.
3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 5 and 208 lb uplift at joint 2.
6) Girder carries hip end with 0-0-0 right side setback, 3-0-0 left side setback, and 6-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 144 lb down and 112 lb up at 3-0-0 on bottom chord.
The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
10) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=54, 3-4=104(F=50), 2-6=10, 5-6=19(F=9)
Concentrated Loads (lb)
Vert: 6=144(F)





Job L287710	Truss EJ7H	Truss Type SPECIAL	Qty 1	Ply 1	CASH ACCT. - STAPE RES. L287710013 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:10 2008 Page 1		

Scale = 1:35.1

Plate Offsets (X,Y): [2-0-3-1,0-1-13]					
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/defl
TCLL 20.0	2-0-0	TC 0.18	Vert(LL) 0.03	7-8	>999
TCDL 7.0	Plates Increase 1.25	BC 0.11	Vert(TL) -0.01	7-8	>999
BCLL 10.0	Lumber Increase 1.25	WB 0.21	Horz(TL) -0.03	7	n/a
BCDL 5.0	Rep Stress Incr YES	(Matrix)			
	Code FBC2004/TPI2002				
			PLATES	GRIP	
			MT20	244/190	
			Weight: 50 lb		

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 SLIDER Left 2 X 6 SYP No.1D 3-0-2	BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 6-7-5 oc bracing.
---	--

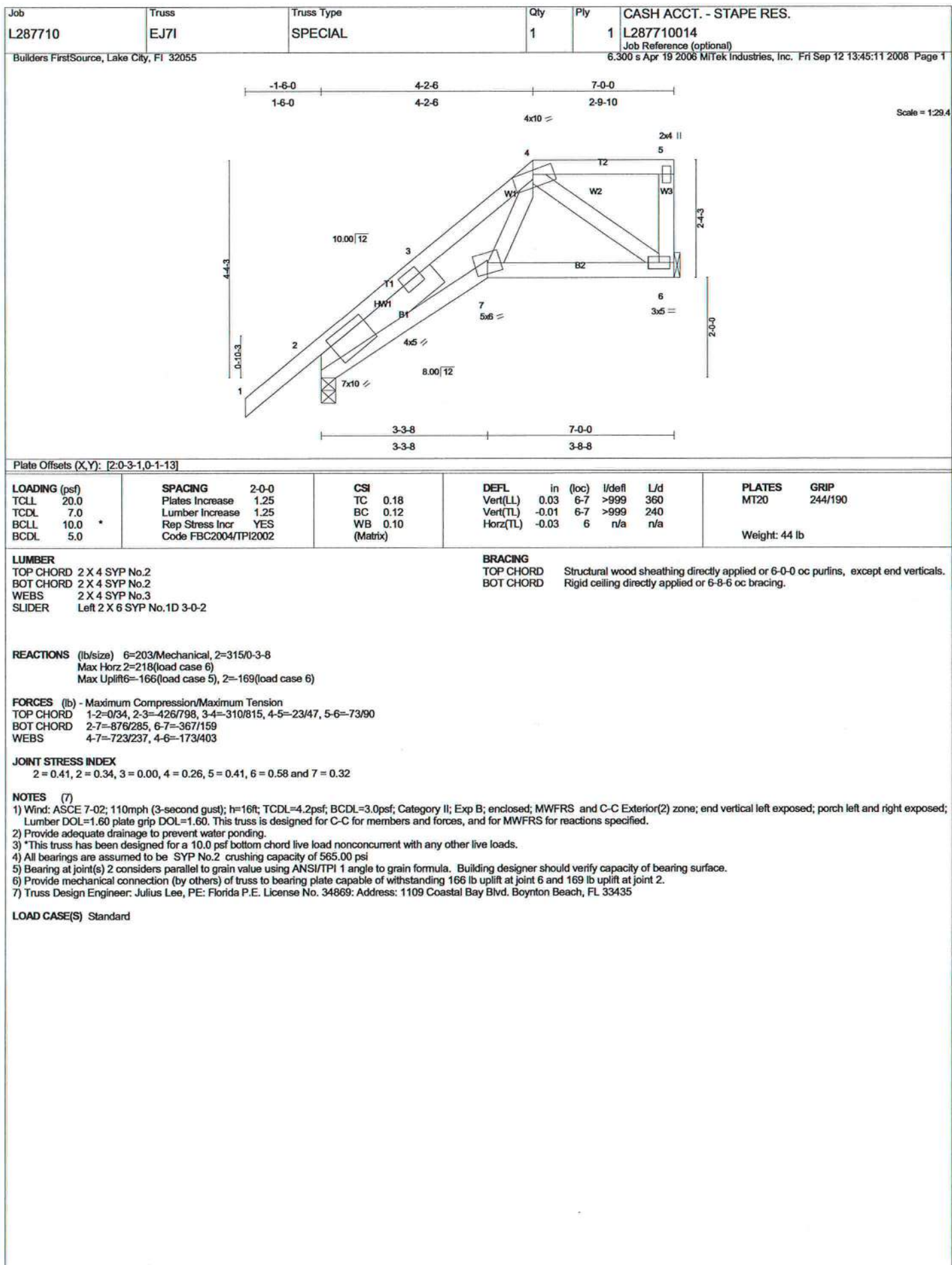
REACTIONS (lb/size) 7=203/Mechanical, 2=315/0-3-8
 Max Horz 2=250(load case 6)
 Max Uplift 7=188(load case 6), 2=151(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/34, 2-3=425/761, 3-4=309/774, 4-5=331/821, 5-6=10/21, 6-7=24/21
 BOT CHORD 2-8=902/285, 7-8=182/81
 WEBS 4-8=73/64, 5-8=886/282, 5-7=172/373

JOINT STRESS INDEX
 2 = 0.38, 2 = 0.33, 3 = 0.00, 4 = 0.05, 5 = 0.27, 6 = 0.17, 7 = 0.46 and 8 = 0.29

NOTES (7)
 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
 2) Provide adequate drainage to prevent water ponding.
 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 188 lb uplift at joint 7 and 151 lb uplift at joint 2.
 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job L287710	Truss EJ7J	Truss Type SPECIAL	Qty 1	Ply 1	CASH ACCT. - STAPE RES. L287710015 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:12 2008 Page 1		

Scale = 1:23.7

The diagram illustrates a truss structure with members numbered 1 through 7. Plates are labeled T1, T2, T3, B1, B2, W1, W2, and W3. Dimensions include horizontal spans of 10.00/12, 8.00/12, 3-3-8, and 7-0-0; vertical heights of 3-4-3, 0-10-3, 1-4-3, and 2-0-0; and member lengths such as 1-6-0, 3-0-0, 7-0-0, 4-0-0, 4x5, 3x8, 5x6, 7x10, and 2x4. The truss is supported at joint 1.

Plate Offsets (X,Y): [2-0-3-1,0-1-13]									
LOADING (psf) TCDL 20.0 BCDL 7.0 BCLL 10.0 BCDL 5.0		SPACING Plates Increase 2-0-0 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002		CSI TC 0.33 BC 0.12 WB 0.16 (Matrix)		DEFL in (loc) l/defl L/d Vert(LL) 0.03 2-7 >999 360 Vert(TL) -0.01 7 >999 240 Horz(TL) -0.03 6 n/a n/a		PLATES GRIP MT20 244/190 Weight: 41 lb	
LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 SLIDER Left 2 X 6 SYP No.1D 3-0-2					BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 6-10-10 oc bracing.				
REACTIONS (lb/size) 6=203/Mechanical, 2=315/0-3-8 Max Horz 2=186(load case 6) Max Uplift 6=160(load case 5), 2=179(load case 6)									
FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/34, 2-3=443/811, 3-4=363/824, 4-5=289/753, 5-6=173/309 BOT CHORD 2-7=-829/305, 6-7=-170/97 WEBS 4-7=-465/167, 5-7=609/200									
JOINT STRESS INDEX 2 = 0.41, 2 = 0.35, 3 = 0.00, 4 = 0.24, 5 = 0.59, 6 = 0.84 and 7 = 0.25									
NOTES (7) 1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 2) Provide adequate drainage to prevent water ponding. 3) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 160 lb uplift at joint 6 and 179 lb uplift at joint 2. 7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435									
LOAD CASE(S) Standard									

Job L287710	Truss HJ4	Truss Type JACK	Qty 1	Ply 1	CASH ACCT. - STAPE RES. L287710016 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055					6.300 s Apr 19 2006 MITek Industries, Inc. Fri Sep 12 13:45:13 2008 Page 1

Scale = 1:16.9

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.56	Vert(LL) -0.02	5-6	>999	360		MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.13	Vert(TL) -0.03	5-6	>999	240			
BCLL 10.0 *	Lumber Increase 1.25	WB 0.04	Horz(TL) -0.00	3	n/a	n/a			
BCDL 5.0	Rep Stress Incr NO	(Matrix)							
	Code FBC2004/TPI2002								

Weight: 25 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 4-6-2 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

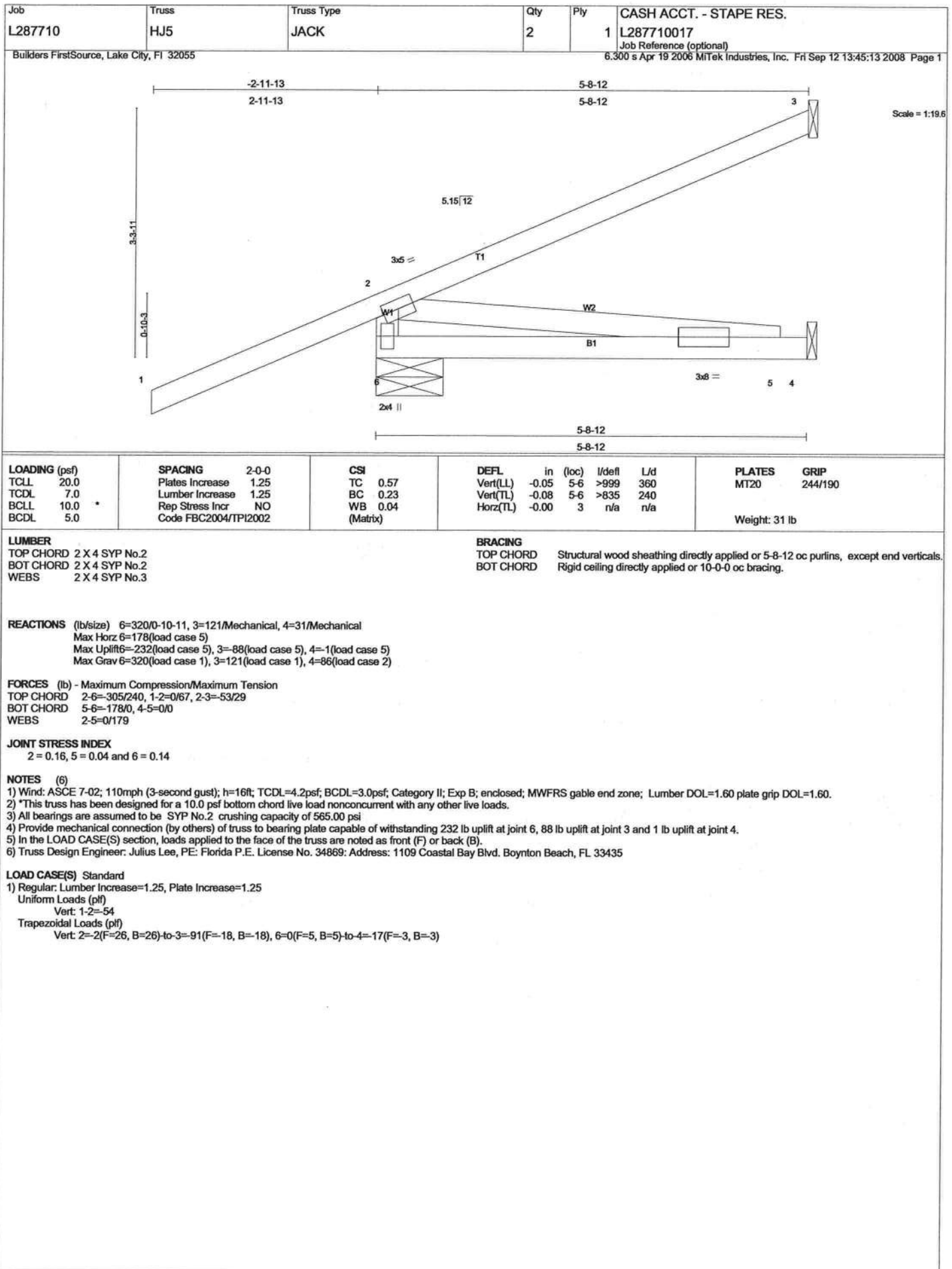
REACTIONS (lb/size) 6=298/0-10-3, 3=50/Mechanical, 4=20/Mechanical
Max Horz 6=144(load case 5)
Max Uplift 6=241(load case 5), 3=34(load case 6), 4=8(load case 5)
Max Grav 6=298(load case 1), 3=50(load case 1), 4=63(load case 2)

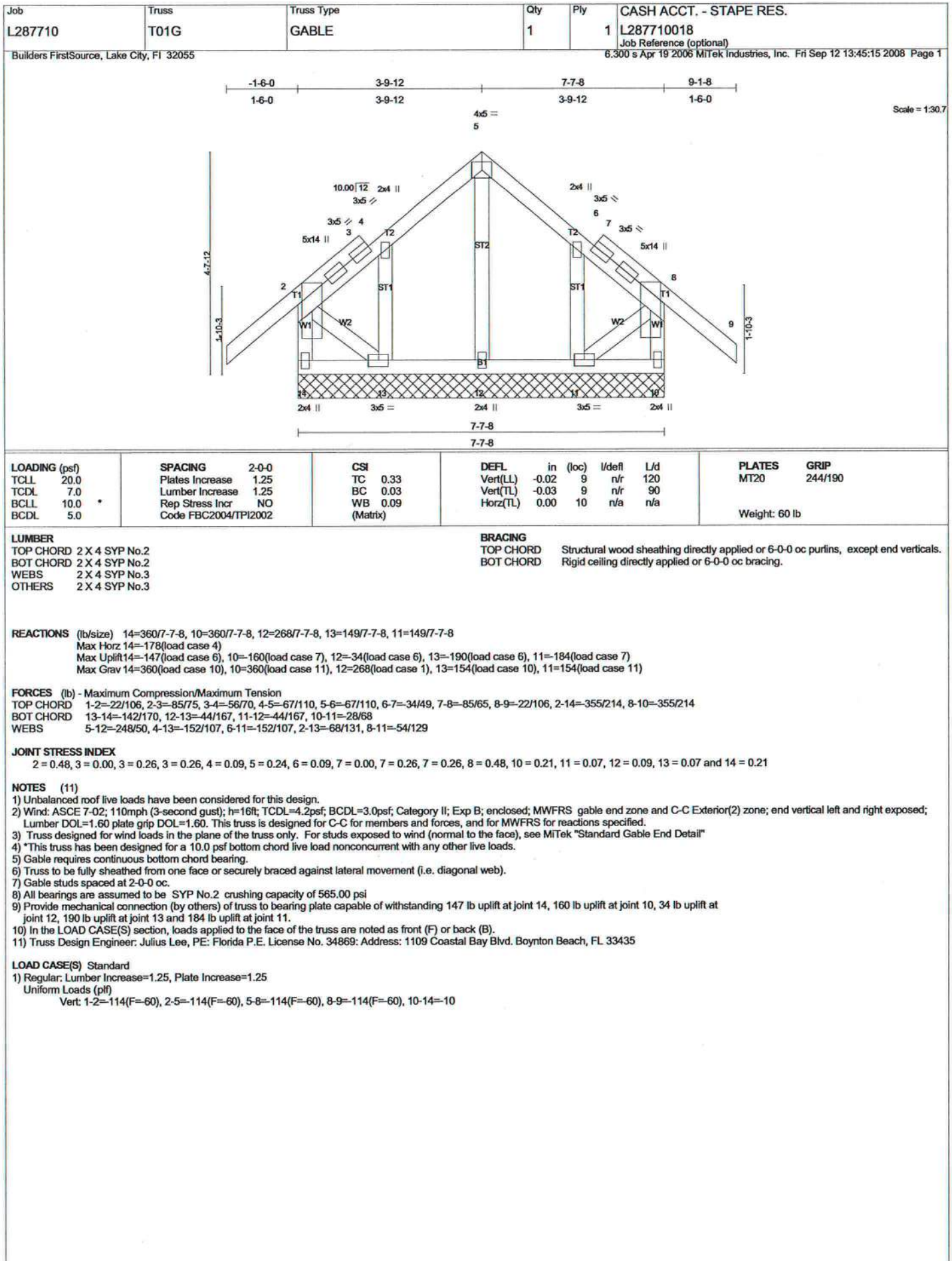
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-6=-288/245, 1-2=0/66, 2-3=-47/7
BOT CHORD 5-6=144/0, 4-5=0/0
WEBS 2-5=0/146

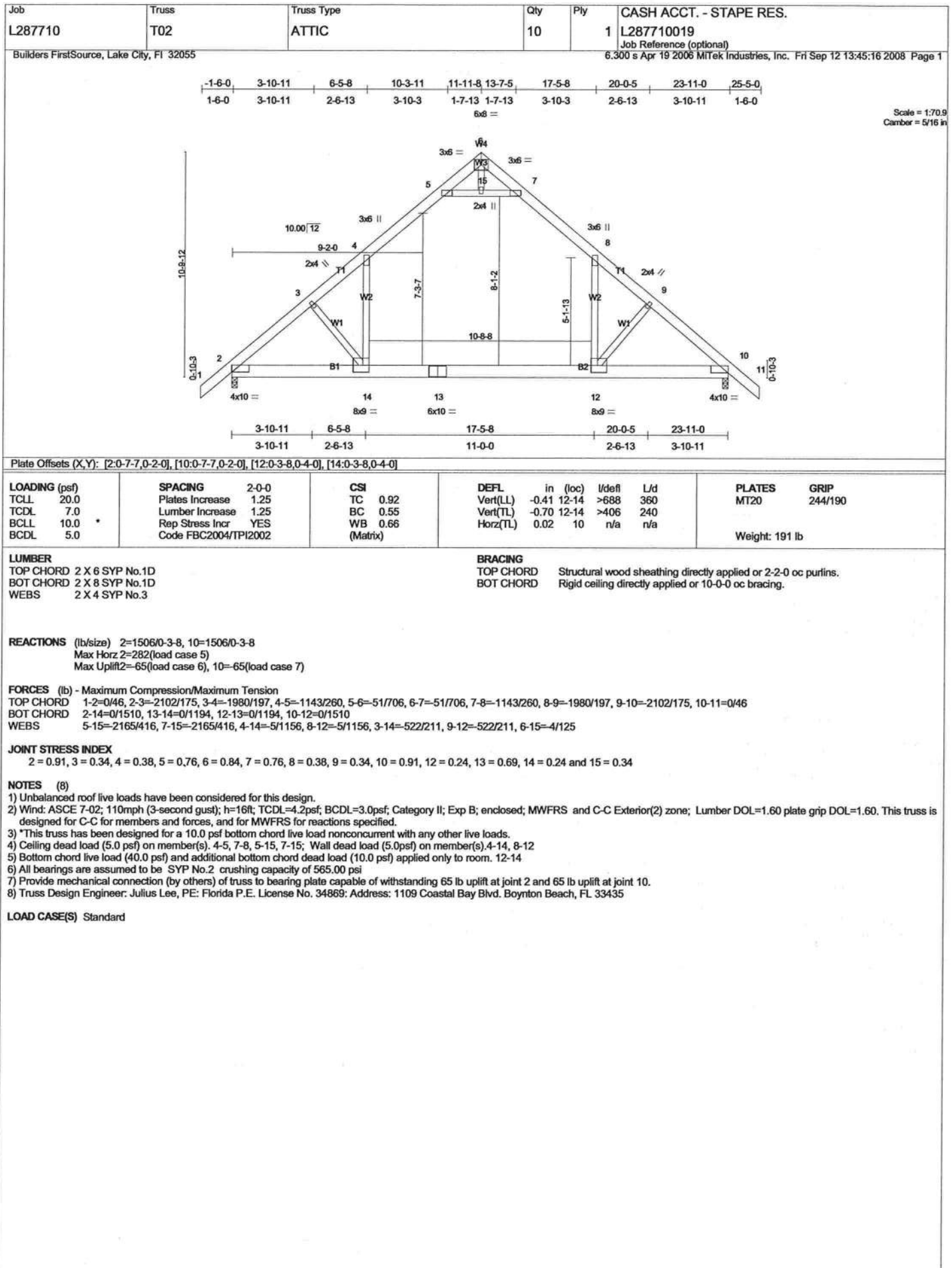
JOINT STRESS INDEX
2 = 0.17, 5 = 0.05 and 6 = 0.14

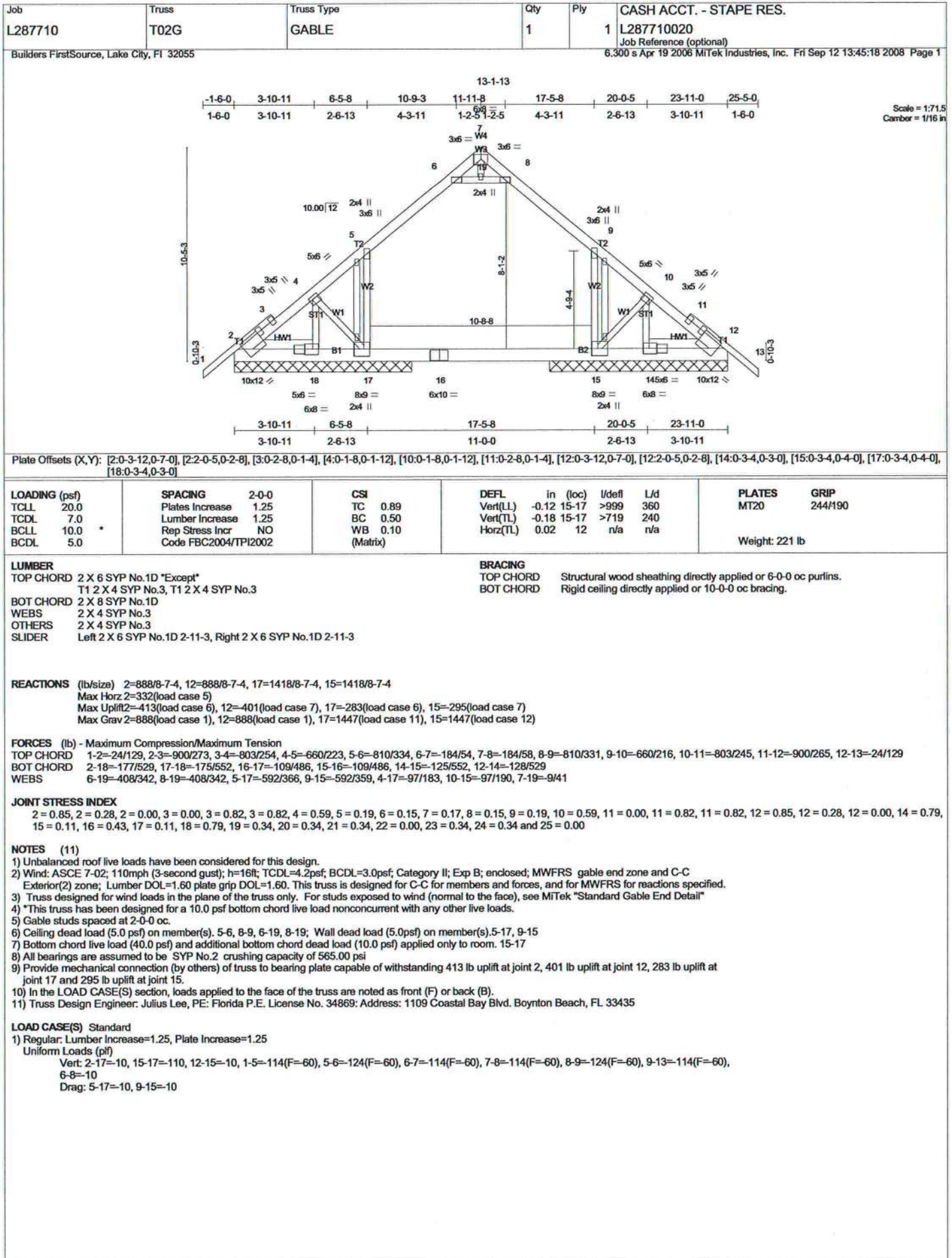
NOTES (6)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 241 lb uplift at joint 6, 34 lb uplift at joint 3 and 8 lb uplift at joint 4.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
6) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

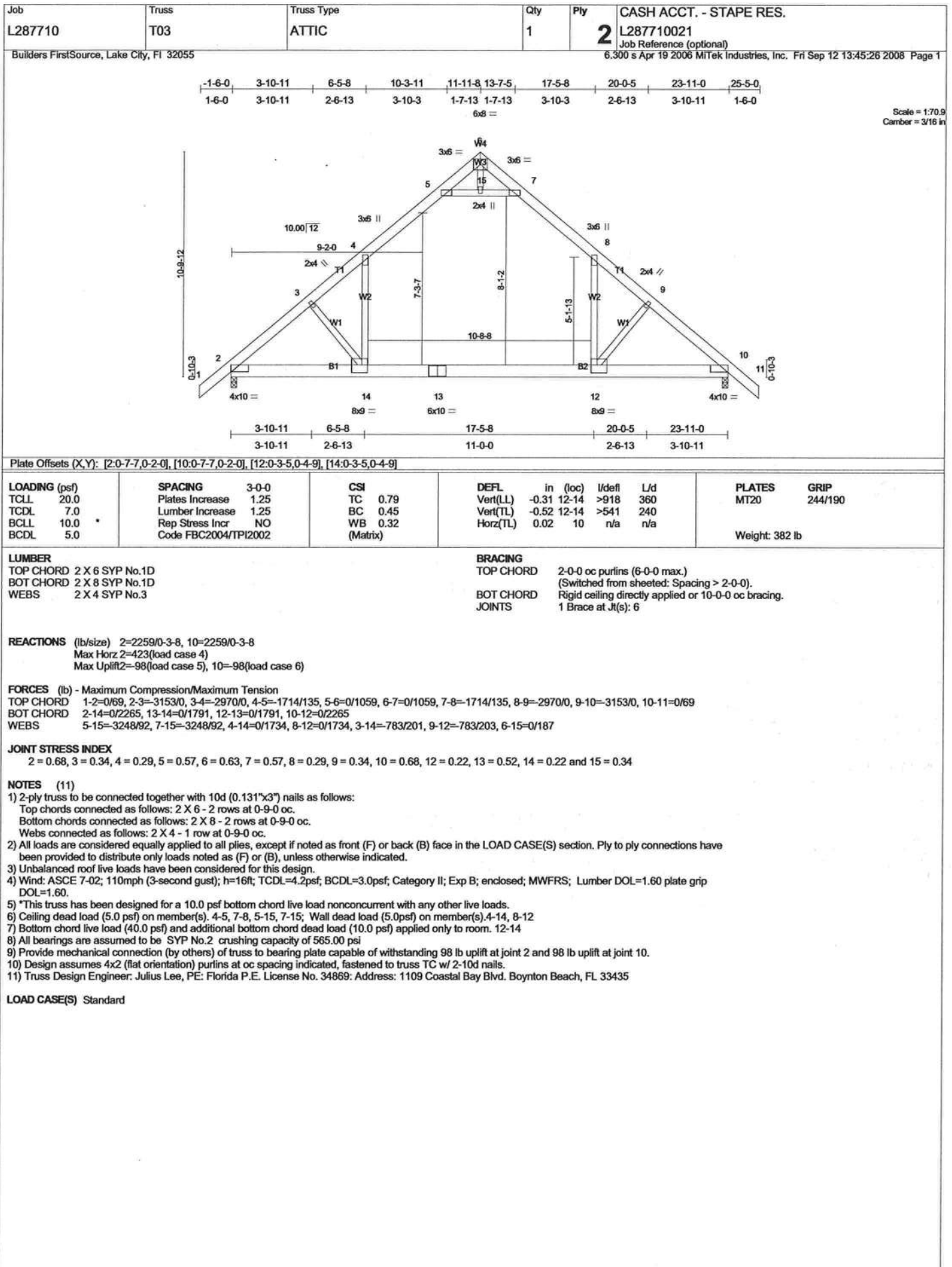
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=54
Trapezoidal Loads (plf)
Vert: 2=2(F=26, B=26)-to-3=-76(F=-11, B=-11), 6=0(F=5, B=5)-to-4=-14(F=-2, B=2)











Job L287710	Truss T04	Truss Type MONO TRUSS	Qty 1	Ply 1	CASH ACCT. - STAPE RES. L287710022 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:27 2008 Page 1		

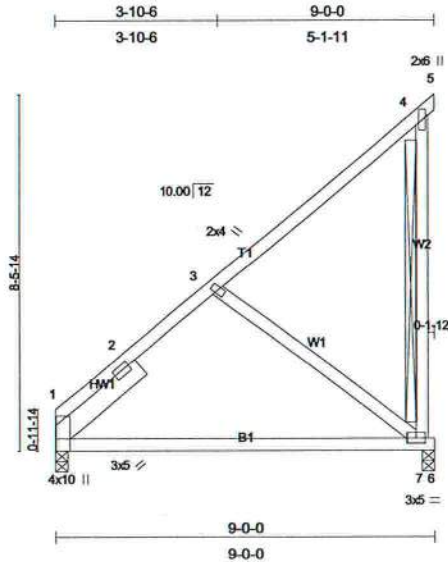


Plate Offsets (X,Y): [1:Edge,0-0-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.37	Vert(LL)	-0.11	1-7	>989	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.31	Vert(TL)	-0.18	1-7	>577	240		
BCLL 10.0	Rep Stress Incr	YES	WB 0.14	Horz(TL)	-0.00	7	n/a	n/a		
BCDL 5.0	Code FBC2004/TPI2002		(Matrix)							
										Weight: 58 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 2-7-11

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 4-7
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 1=279/0-3-8, 7=286/0-3-8
Max Horz 1=260(load case 6)
Max Uplift 7=201(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=277/0, 2-3=170/0, 3-4=119/63, 4-5=3/0, 4-7=118/151
BOT CHORD 1-7=247/176, 6-7=0/0
WEBS 3-7=205/300

JOINT STRESS INDEX
1 = 0.73, 1 = 0.15, 2 = 0.00, 3 = 0.17, 4 = 0.39 and 7 = 0.61

NOTES (5)
1) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 201 lb uplift at joint 7.
5) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd, Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job L287710	Truss T04G	Truss Type MONO TRUSS	Qty 1	Ply 2	CASH ACCT. - STAPE RES. L287710023 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055					6.300 s Apr 19 2006 Mitek Industries, Inc. Fri Sep 12 13:45:27 2008 Page 1

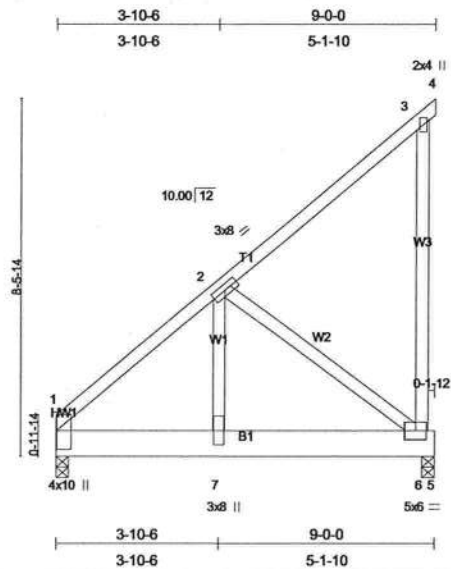


Plate Offsets (X,Y): [1:0-5-5,0-0-1]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.19	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.29	Vert(LL) -0.02 6-7 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.40	Vert(TL) -0.04 6-7 >999 240		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 6 n/a n/a		
	Code FBC2004/TPI2002			Weight: 141 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 8 SYP No.1D
WEBS 2 X 4 SYP No.3
WEDGE
Left: 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)

6=2546/0-3-8, 1=2392/0-3-8
Max Horz 1=255(load case 5)
Max Uplift 6=863(load case 5), 1=584(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=2292/543, 2-3=116/51, 3-4=3/0, 3-6=112/103
BOT CHORD 1-7=563/1624, 6-7=563/1624, 5-6=0/0
WEBS 2-7=699/2505, 2-6=2018/695

JOINT STRESS INDEX

1 = 0.45, 1 = 0.00, 2 = 0.76, 3 = 0.40, 6 = 0.24 and 7 = 0.41

NOTES (8)

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 863 lb uplift at joint 6 and 584 lb uplift at joint 1.
- Girder carries tie-in span(s): 32-4-4 from 0-0-0 to 9-0-0
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=54, 3-4=14, 1-5=496(F=486)

Job: L287710 Truss: T06 Truss Type: SPECIAL Qty: 1 Ply: 1 CASH ACCT. - STAPE RES. L287710025 Job Reference (optional) 6,300 s Apr 19 2006 Mitek Industries, Inc. Fri Sep 12 13:45:30 2008 Page 1

Builders FirstSource, Lake City, FL 32055

Scale = 1/8" = 1'-0" Camber = 3/16" in

Plate Offsets (X,Y): [9:0-8-0,0-0-6]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.47	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.51	Vert(LL) -0.15 10-12 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.38	Vert(TL) -0.33 10-12 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.09 9 n/a n/a		
	Code FBC2004/TPI2002				

Weight: 217 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-4 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 6-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

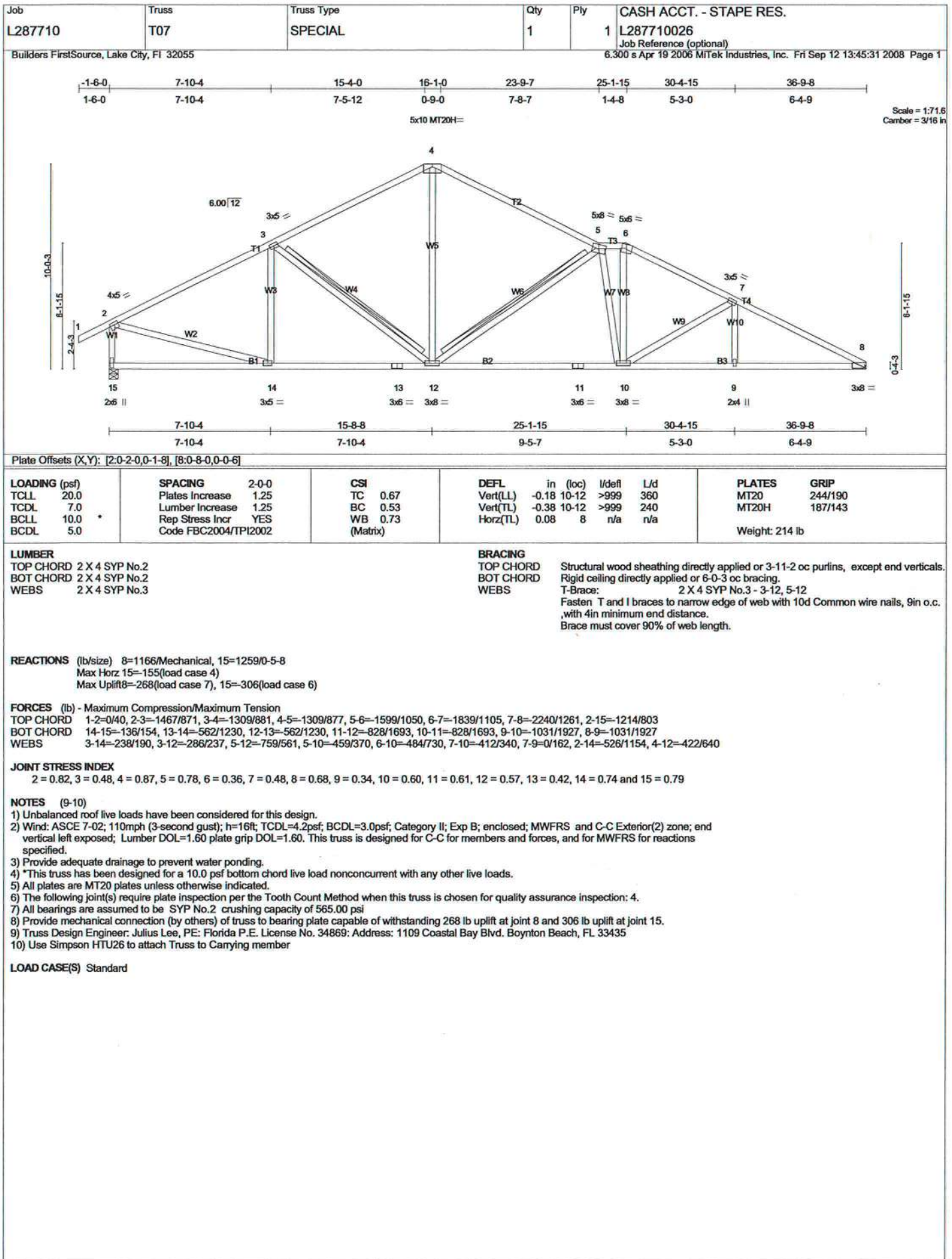
REACTIONS (lb/size) 9=1166/Mechanical, 16=1259/0-5-8
Max Horz 16=-131(load case 4)
Max Uplift 9=253(load case 7), 16=-284(load case 6)

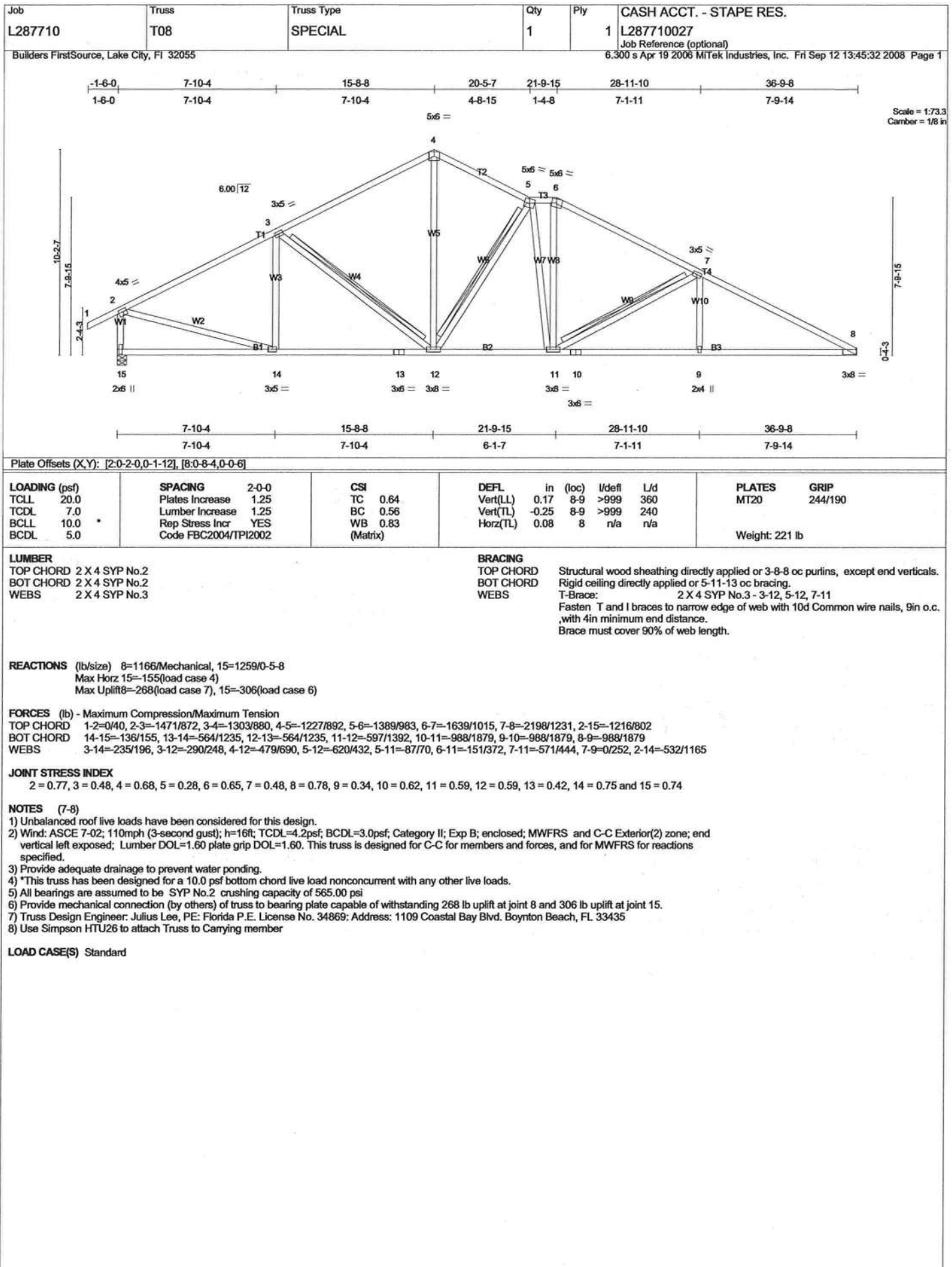
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-1319/779, 3-4=-1406/879, 4-5=-1355/952, 5-6=-1598/967, 6-7=-1818/1104, 7-8=-2057/1157, 8-9=-2276/1299, 2-16=-1231/781
BOT CHORD 15-16=96/126, 14-15=-520/1127, 13-14=-451/1189, 12-13=-451/1189, 11-12=-1027/2004, 10-11=-1027/2004, 9-10=-1097/1986
WEBS 3-15=-392/257, 3-14=-63/200, 4-14=-44/180, 4-12=-136/368, 5-12=-110/352, 6-12=-741/551, 6-10=-663/484, 7-10=-497/806, 8-10=-218/240, 2-15=-572/1182

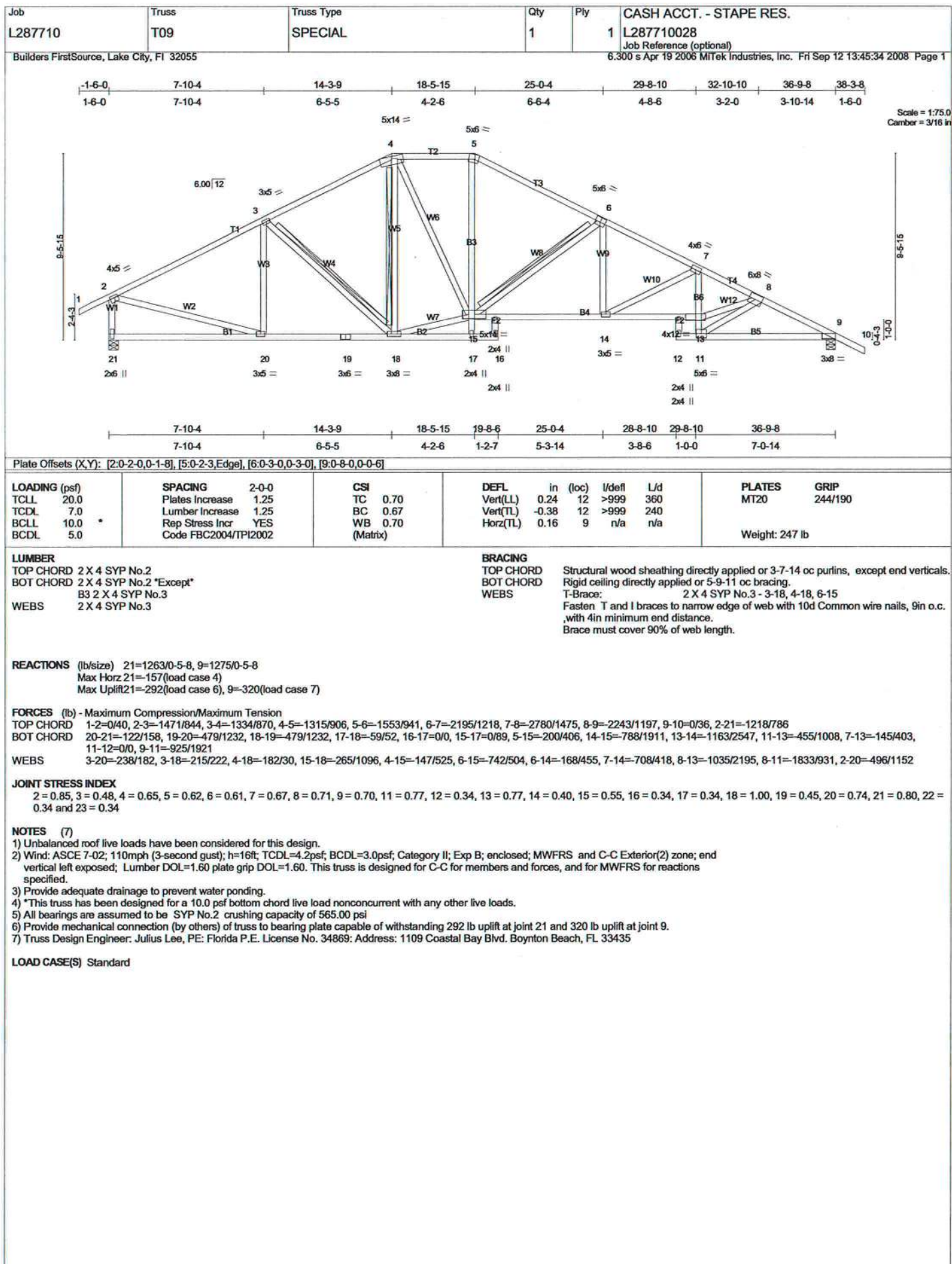
JOINT STRESS INDEX
2 = 0.49, 3 = 0.48, 4 = 0.80, 5 = 0.73, 6 = 0.77, 7 = 0.38, 8 = 0.34, 9 = 0.69, 10 = 0.61, 11 = 0.74, 12 = 0.57, 13 = 0.40, 14 = 0.40, 15 = 0.77 and 16 = 0.54

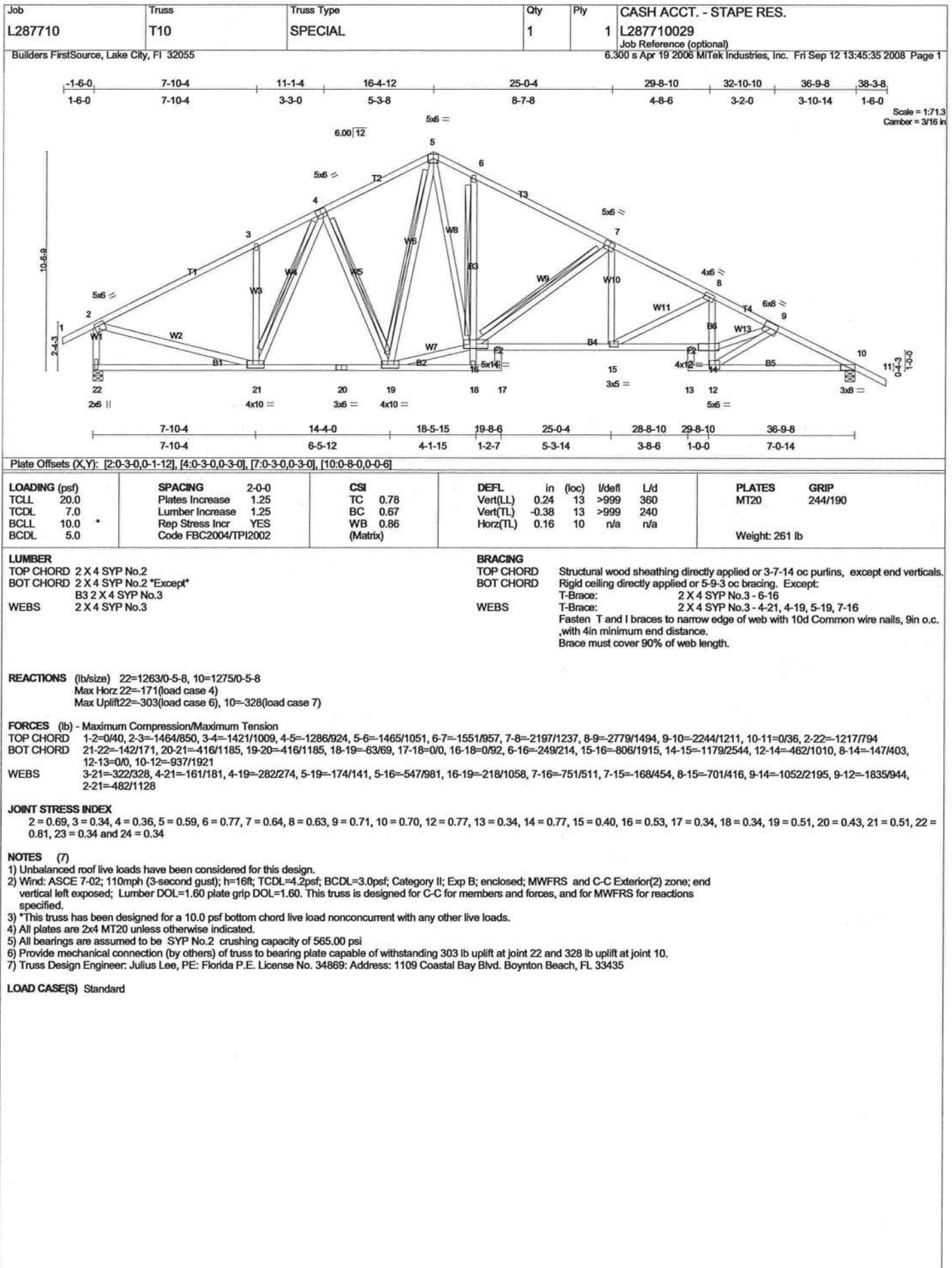
NOTES (7-8)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 253 lb uplift at joint 9 and 284 lb uplift at joint 16.
7) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
8) Use Simpson HTU26 to attach Truss to Carrying member

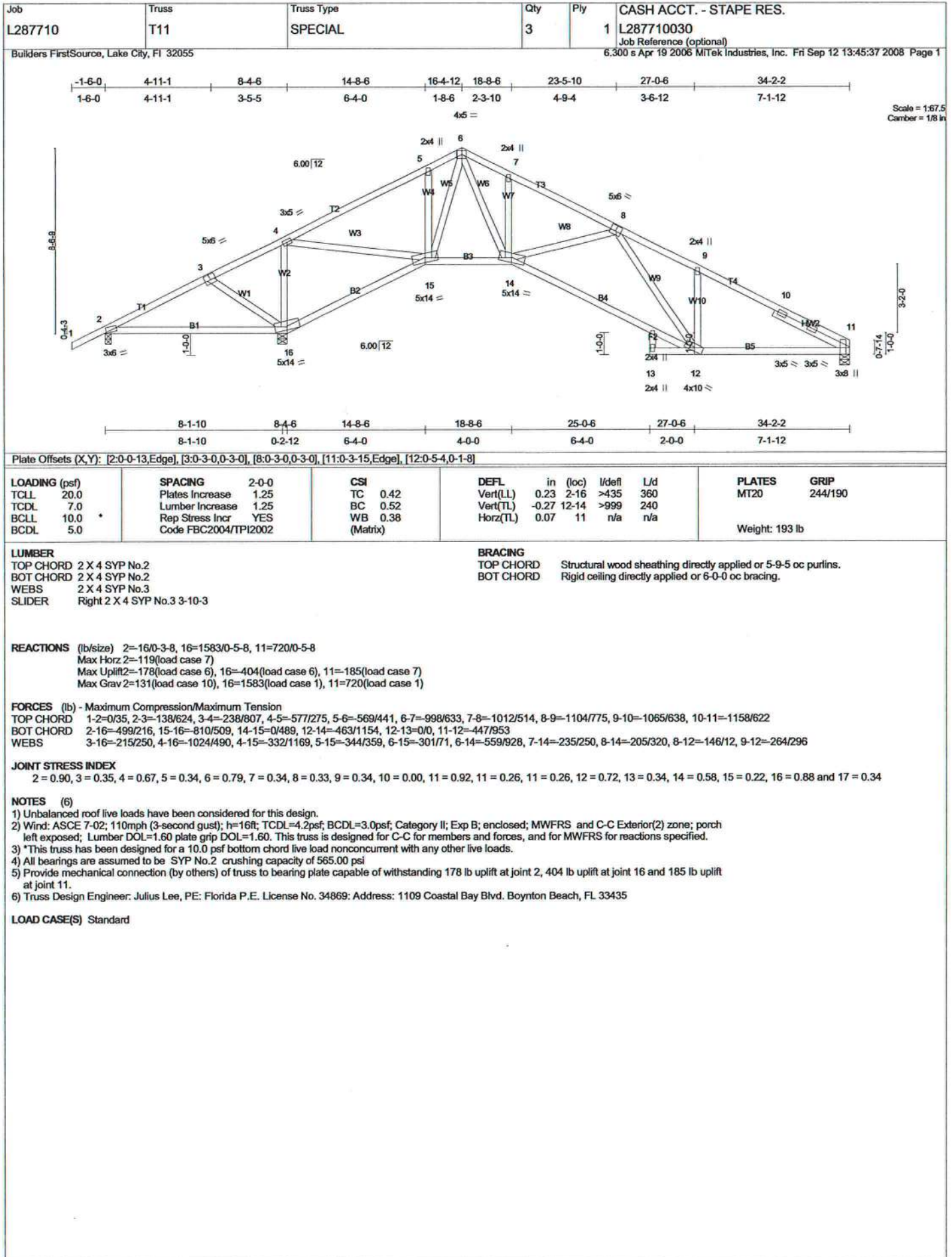
LOAD CASE(S) Standard

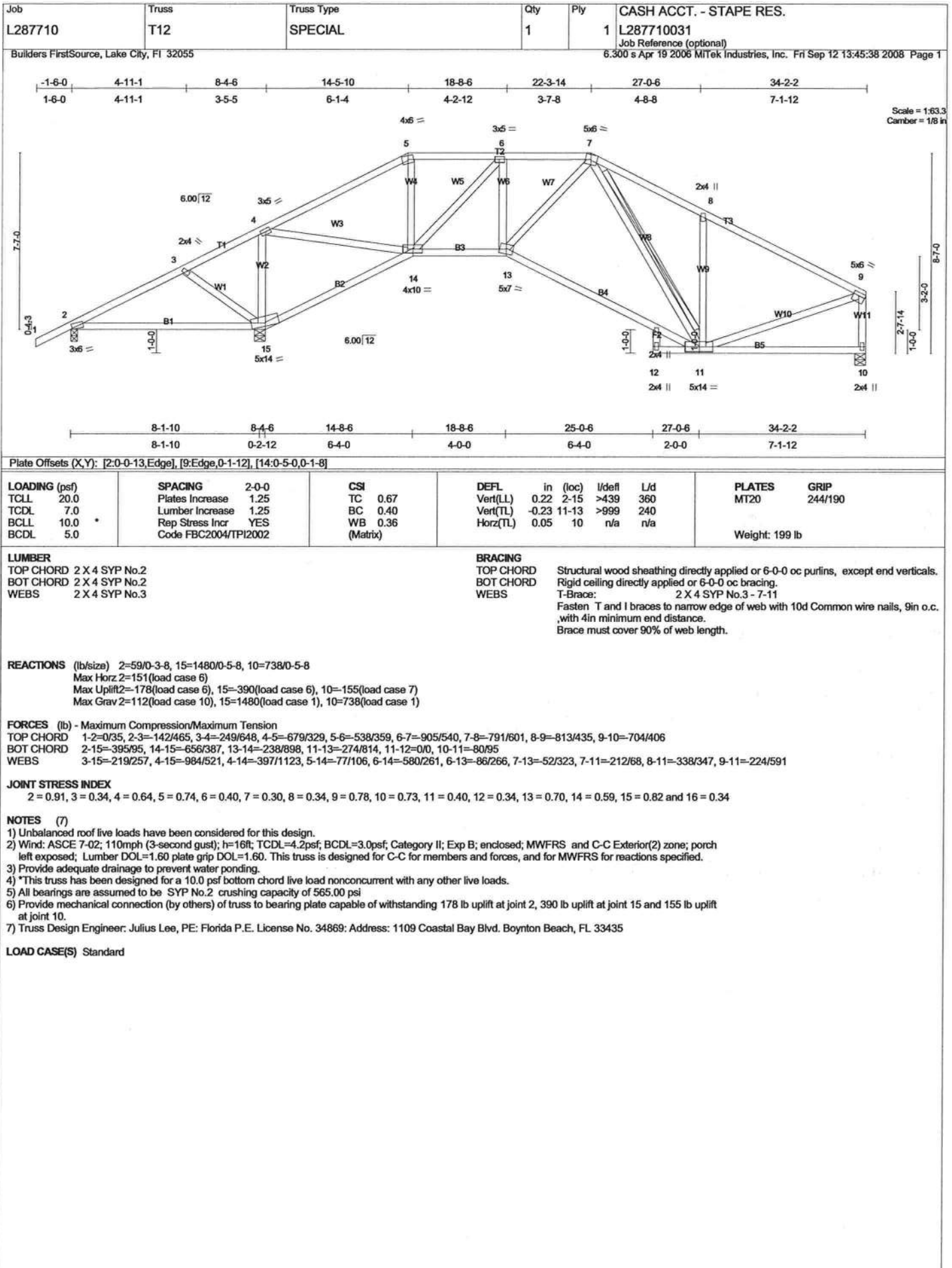




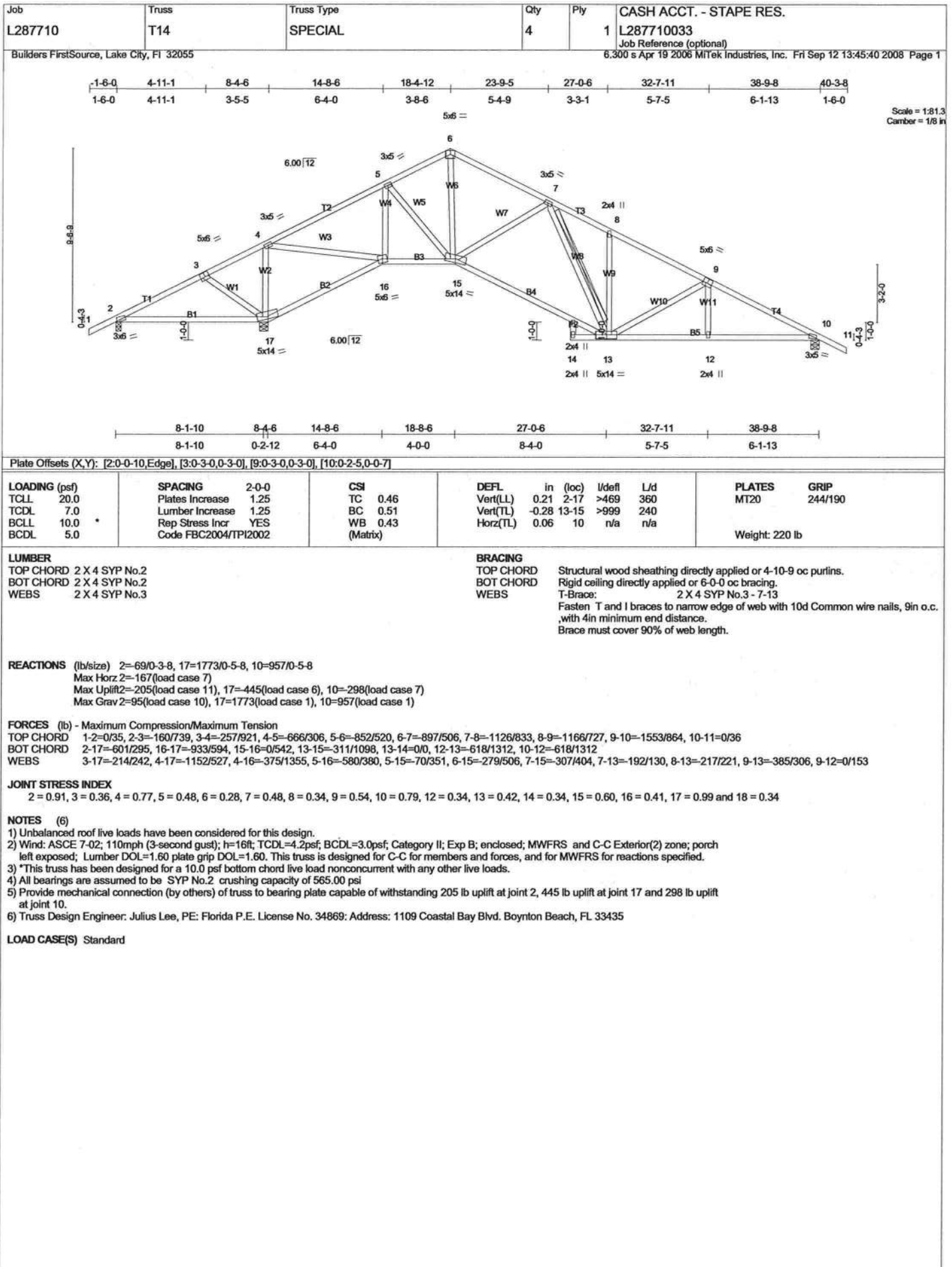


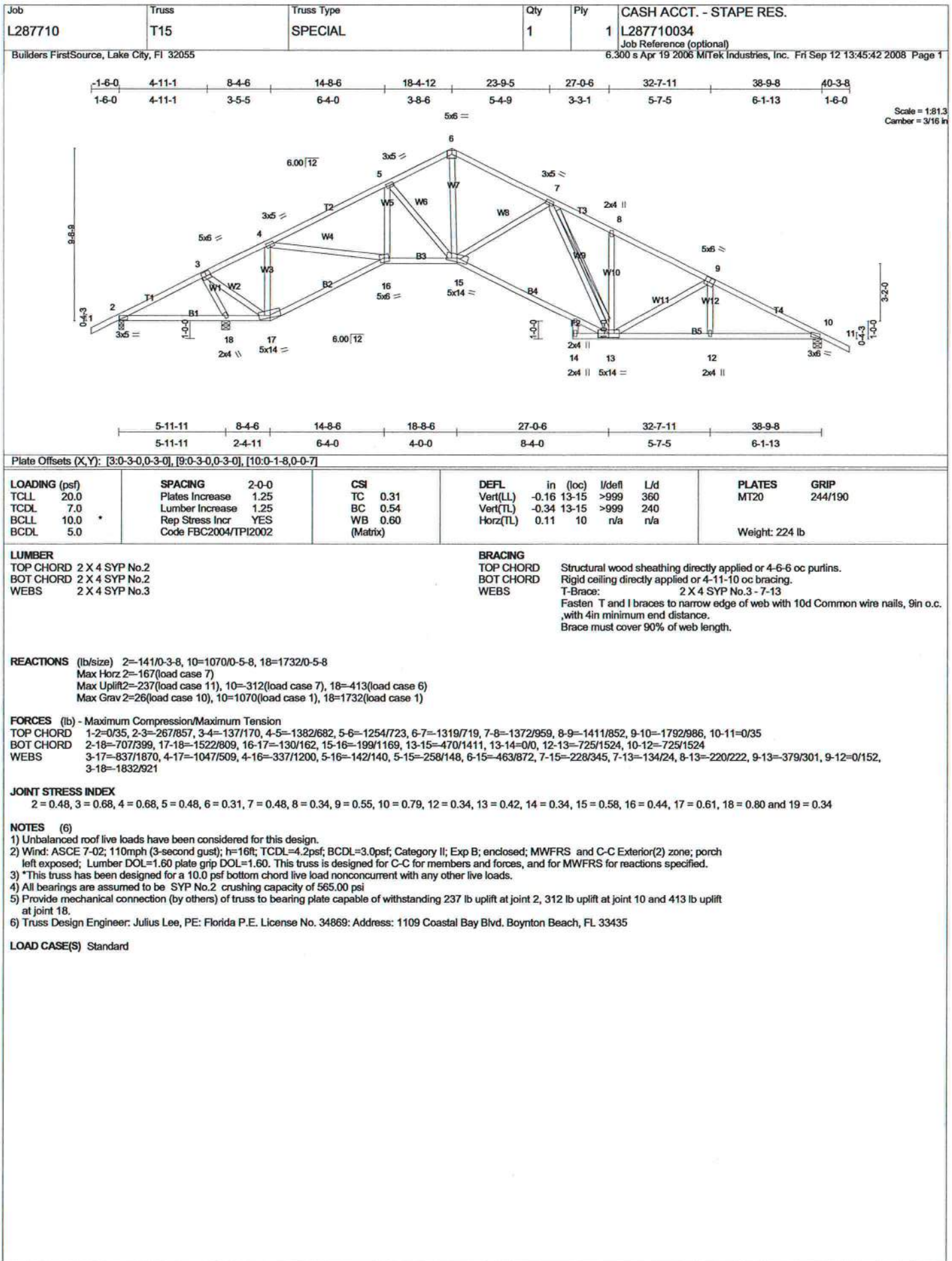


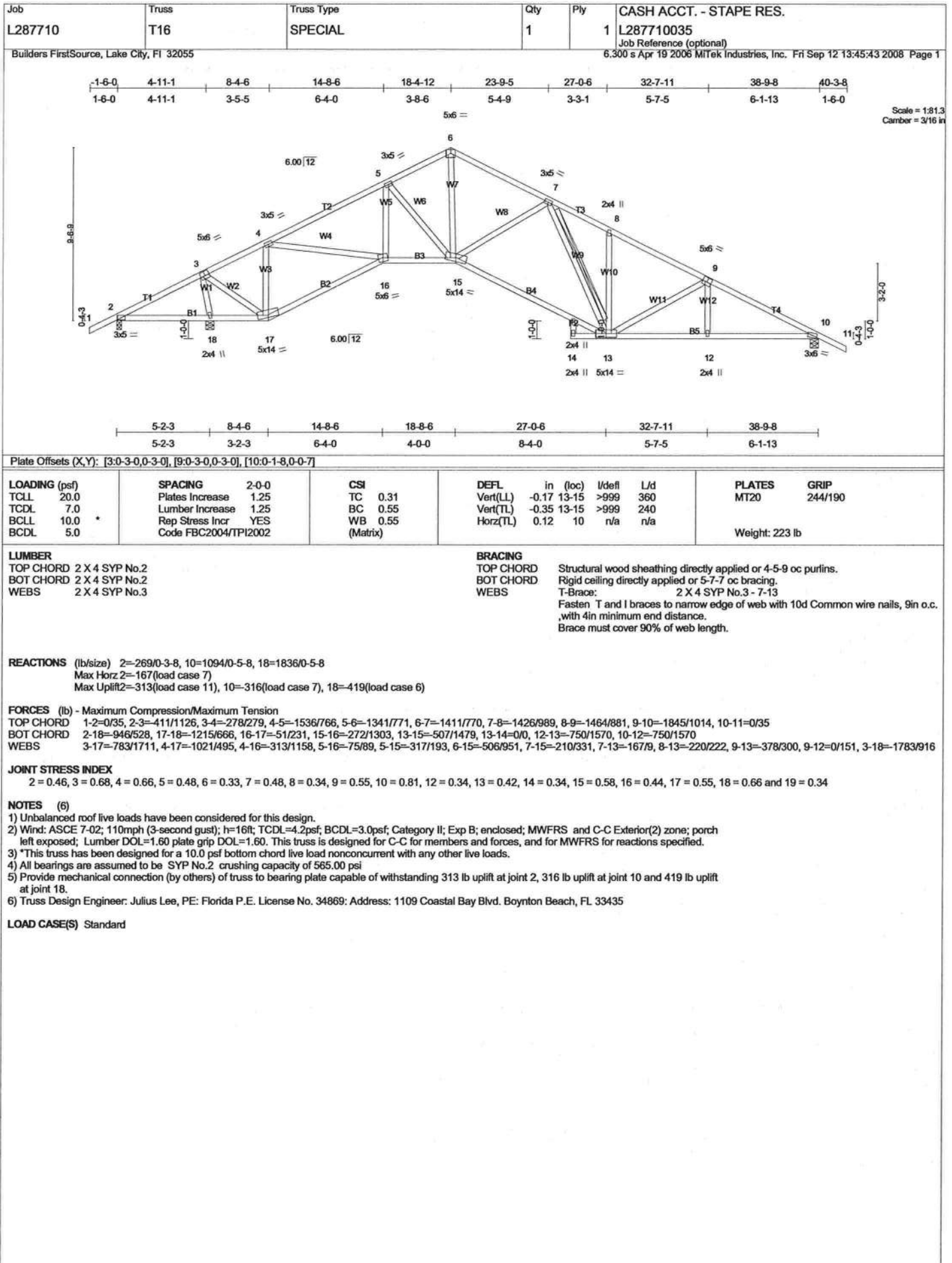


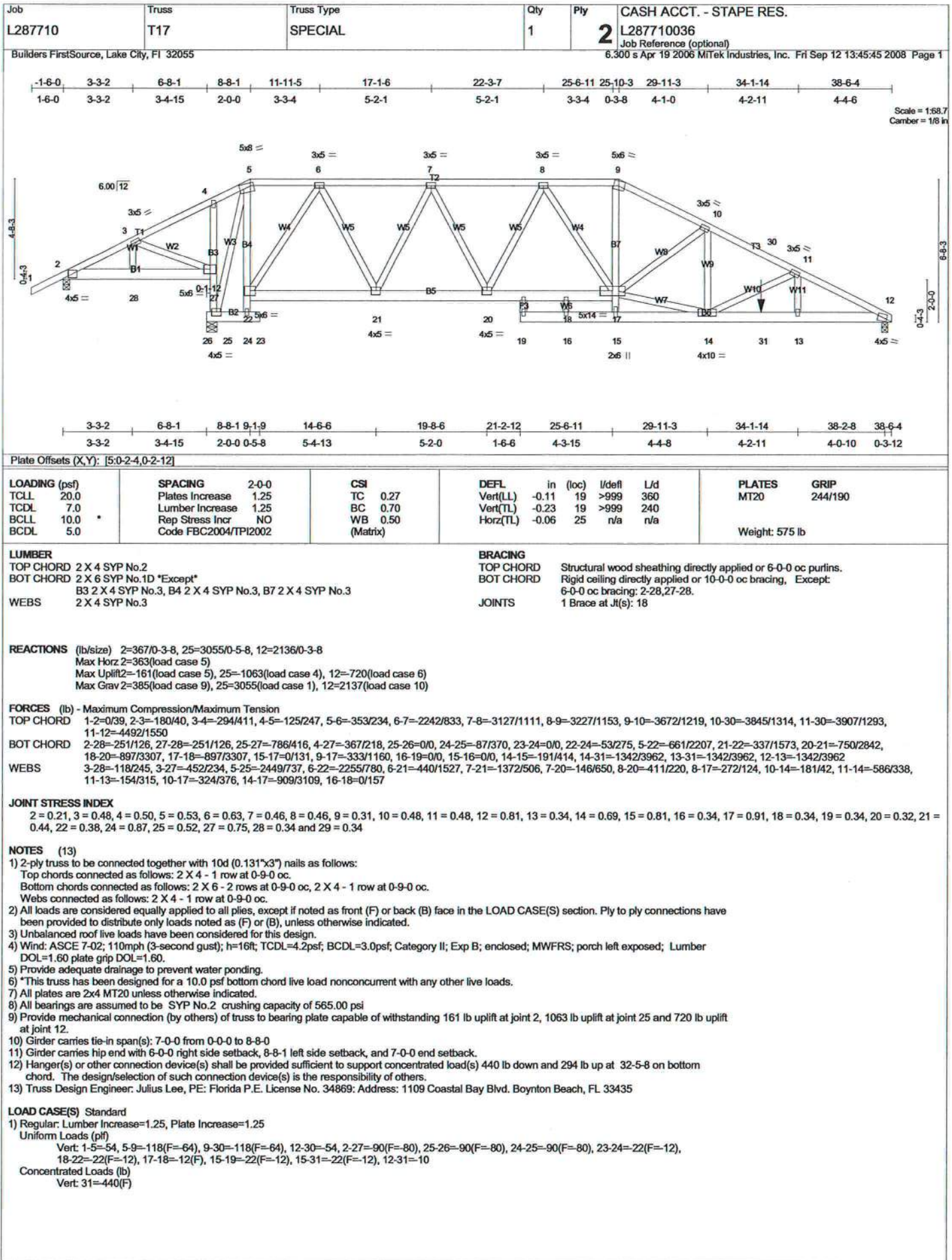


Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:39 2008 Page 1





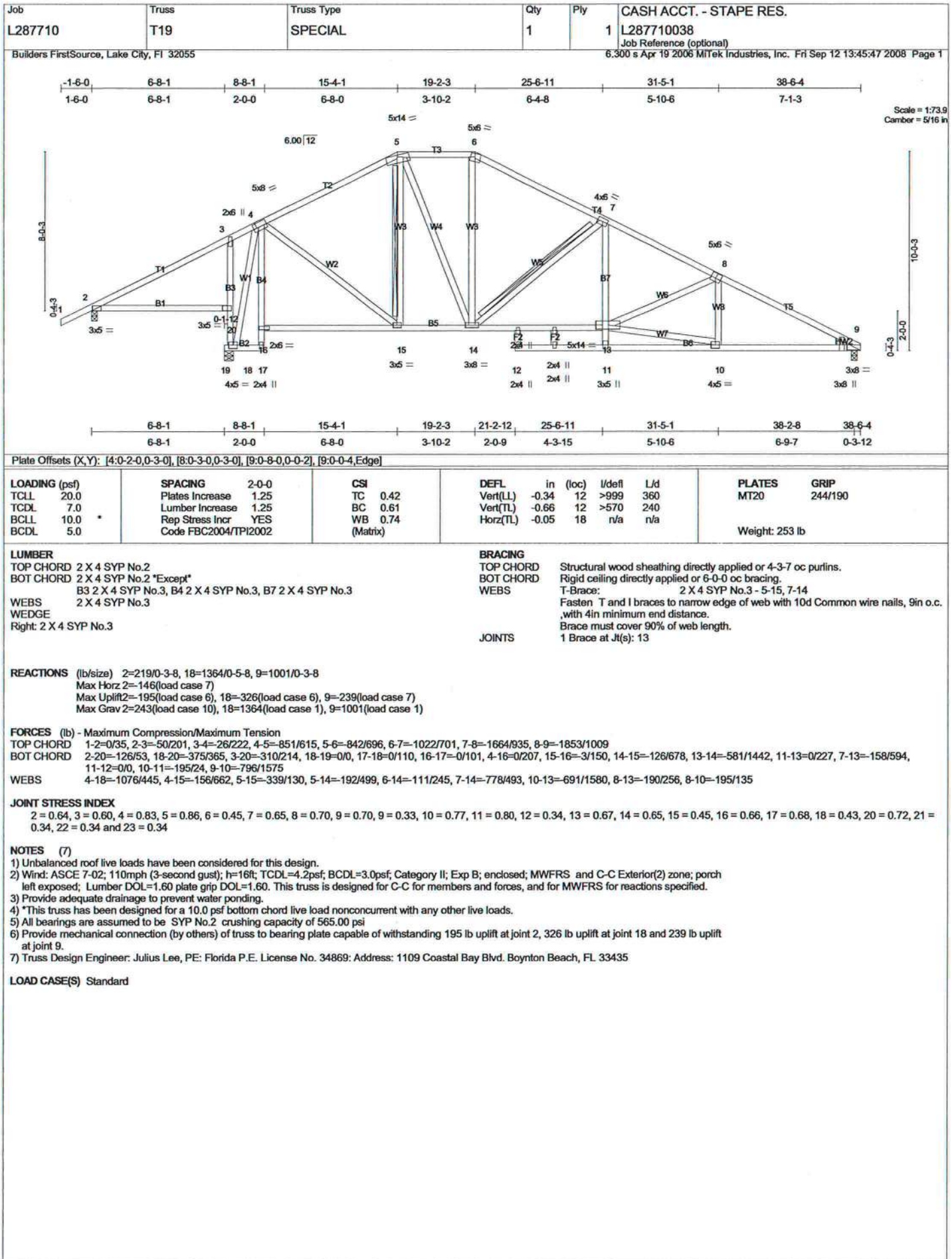


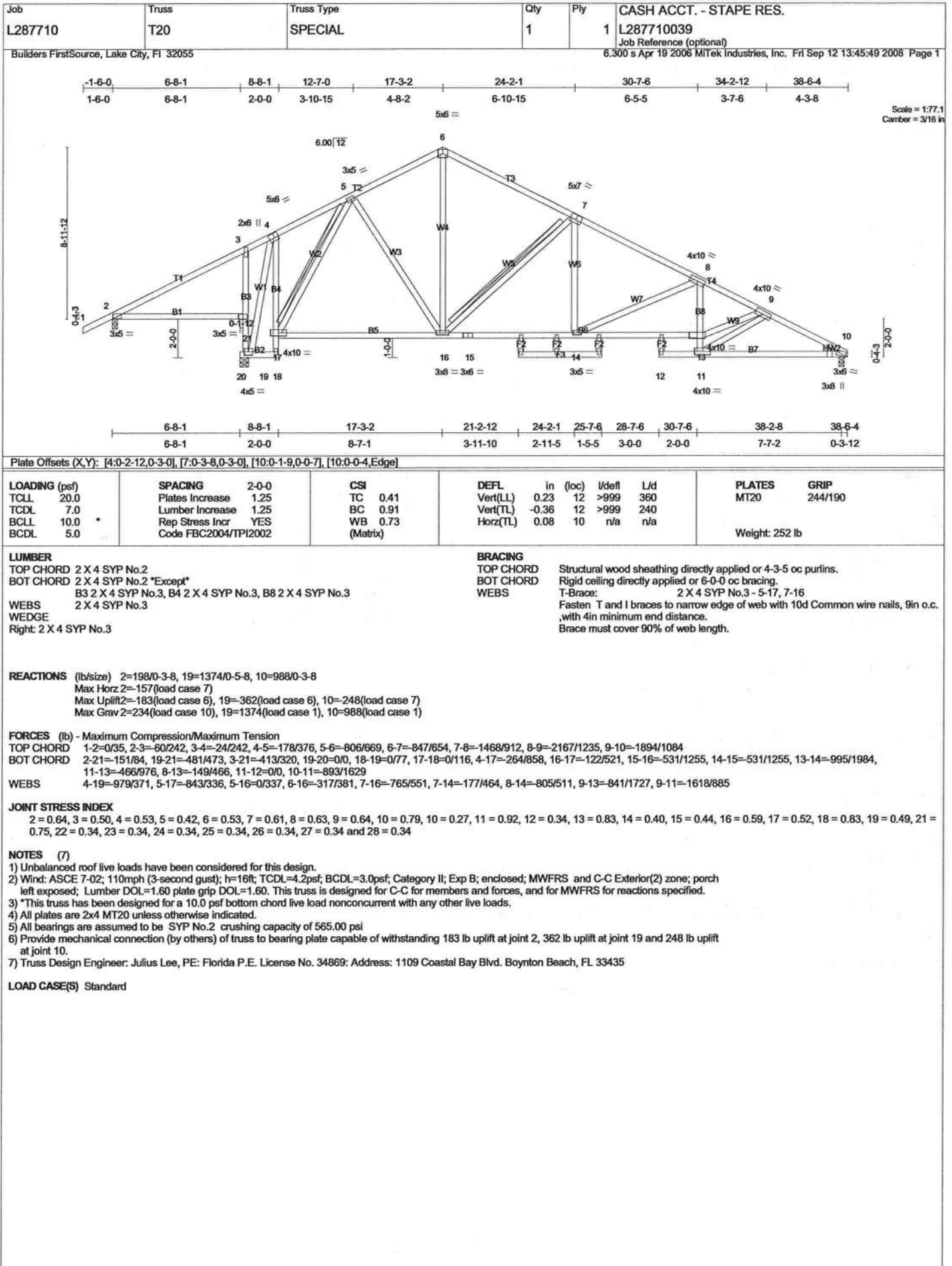


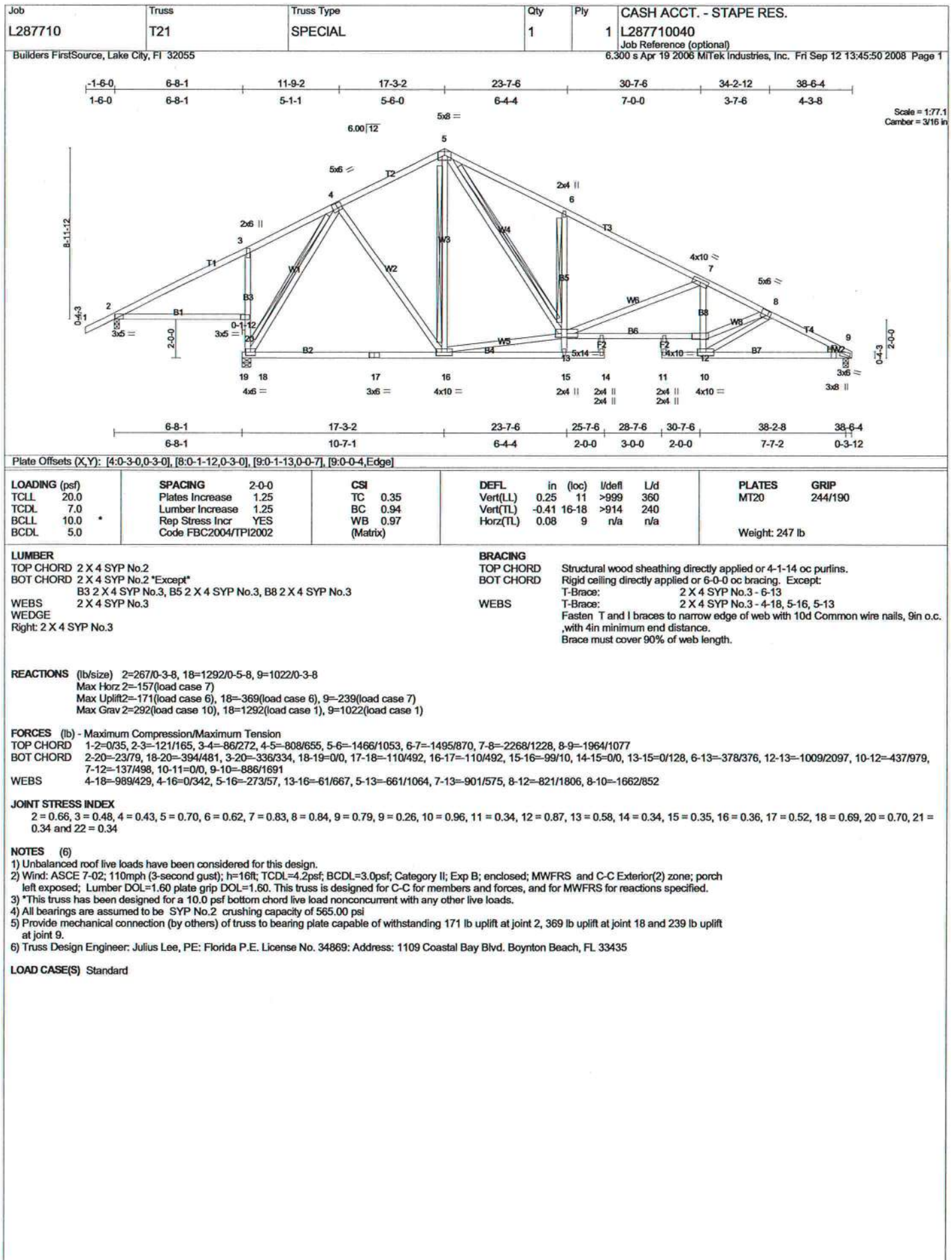
Job L287710 Truss T18 SPECIAL 1 Ply 1 CASH ACCT. - STAPE RES. L287710037 Job Reference (optional) 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:46 2008 Page 1

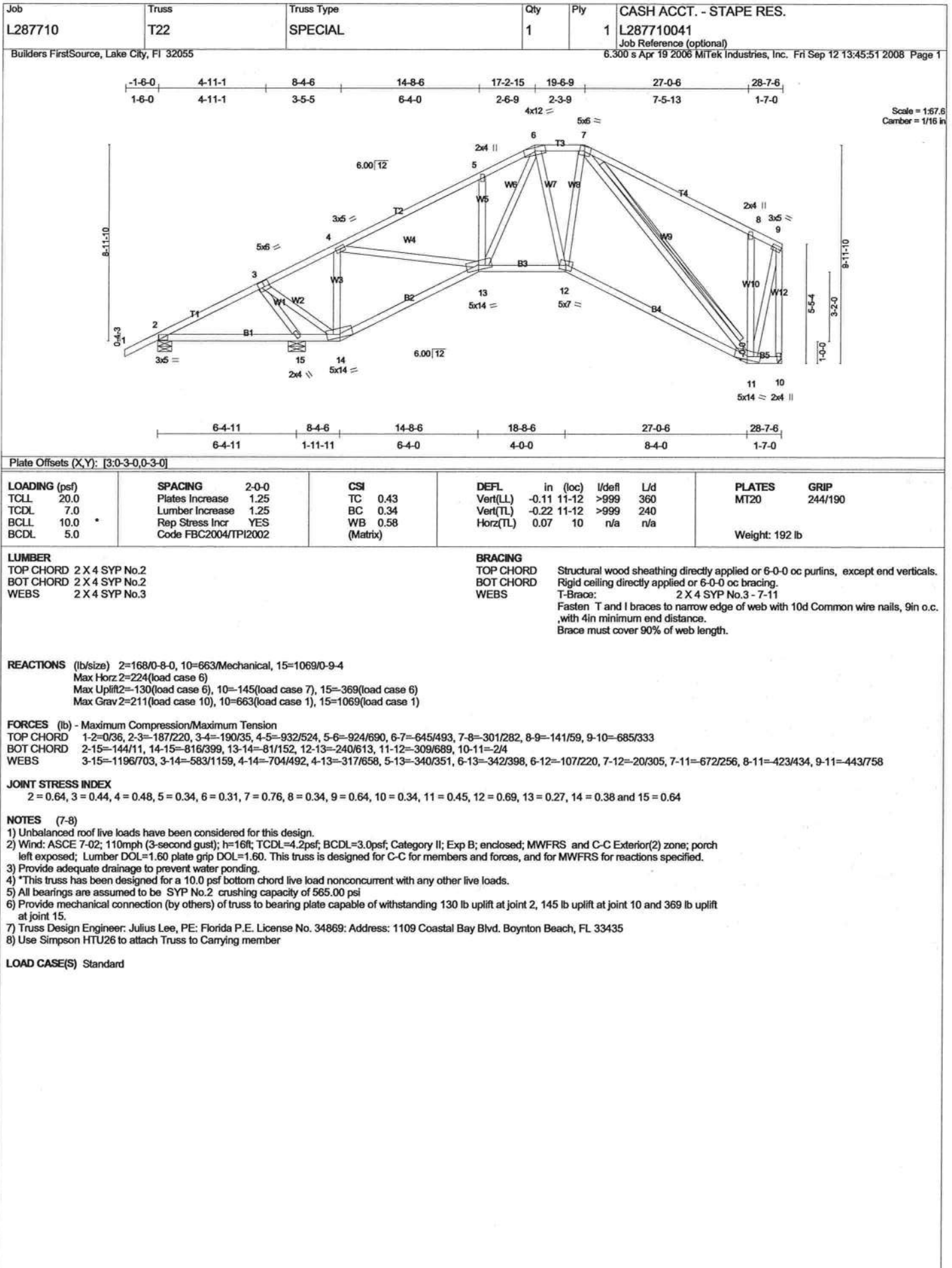
Builders FirstSource, Lake City, FL 32055

Scale = 1/68.8
Camber = 5/16 in









Job L287710	Truss T23	Truss Type SPECIAL	Qty 1	Ply 1	CASH ACCT. - STAPE RES. L287710042 Job Reference (optional)
-----------------------	---------------------	------------------------------	-----------------	-----------------	--

Builders FirstSource, Lake City, FL 32055

6.300 s Apr 19 2008 MiTek Industries, Inc. Fri Sep 12 13:45:53 2008 Page 1

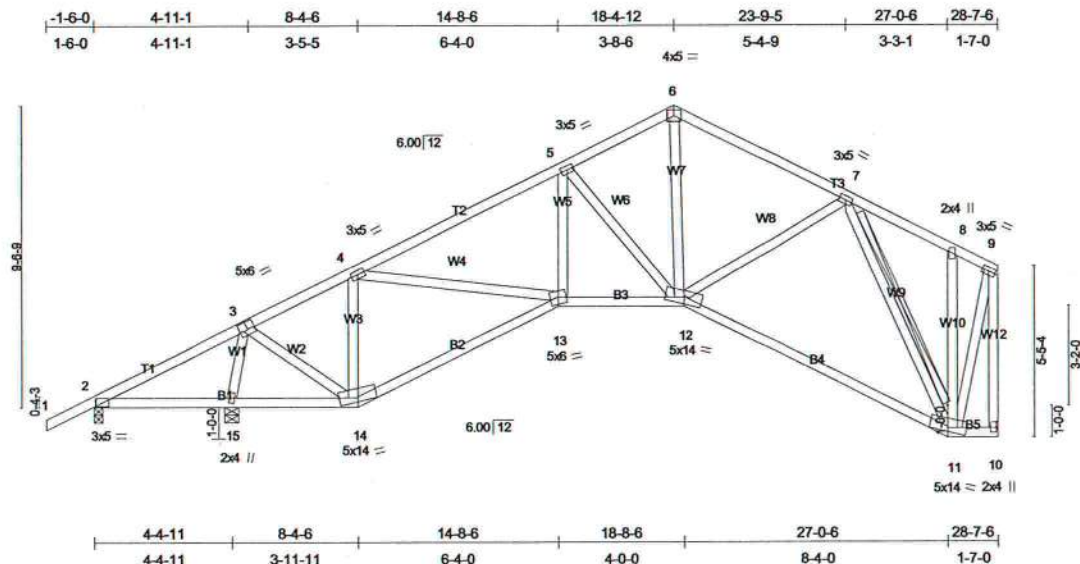


Plate Offsets (X,Y): [3-0-3-0,0-3-0]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.44	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.37	Vert(LL) -0.11 11-12 >999 360		
BCLL 10.0	Lumber Increase 1.25	WB 0.31	Vert(TL) -0.21 11-12 >999 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.07 10 n/a n/a		
	Code FBC2004/TPI2002			Weight: 187 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-6-1 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 7-11
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=17/0-3-8, 10=726/Mechanical, 15=1192/0-5-8

Max Horz 2=230(load case 6)
Max Uplift 2=93(load case 4), 10=147(load case 7), 15=375(load case 6)
Max Grav 2=35(load case 10), 10=726(load case 1), 15=1192(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/35, 2-3=437/584, 3-4=524/268, 4-5=1159/682, 5-6=805/569, 6-7=848/558, 7-8=210/169, 8-9=199/111, 9-10=738/377
BOT CHORD 2-15=468/229, 14-15=250/90, 13-14=316/483, 12-13=515/958, 11-12=308/545, 10-11=6/8
WEBS 3-14=463/844, 4-14=641/455, 4-13=246/550, 5-13=76/217, 5-12=450/344, 6-12=325/465, 7-12=42/272, 7-11=755/450, 8-11=94/100, 9-11=311/637, 3-15=1155/739

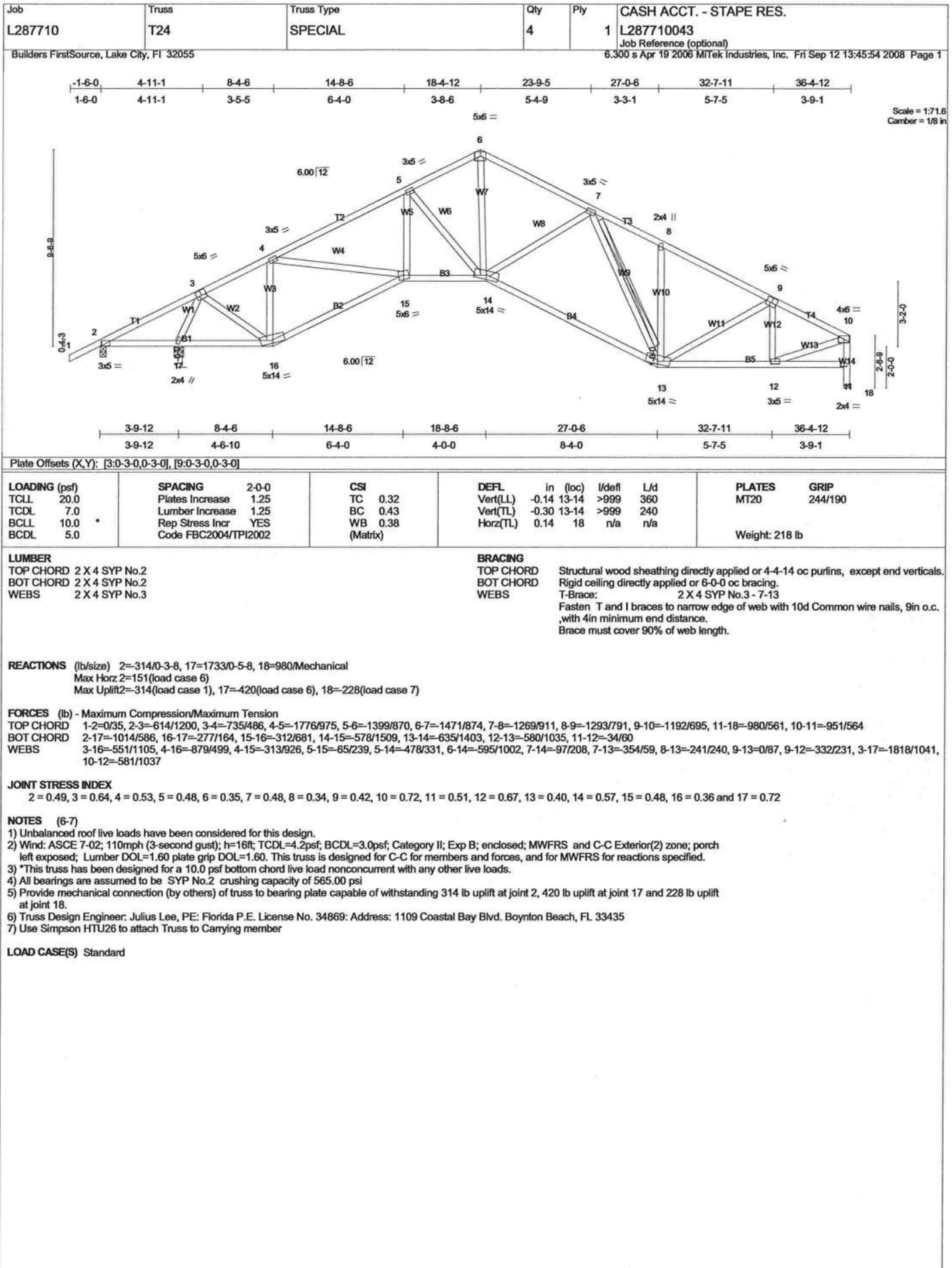
JOINT STRESS INDEX

2 = 0.28, 3 = 0.42, 4 = 0.48, 5 = 0.48, 6 = 0.53, 7 = 0.48, 8 = 0.34, 9 = 0.54, 10 = 0.34, 11 = 0.38, 12 = 0.56, 13 = 0.40, 14 = 0.27 and 15 = 0.43

NOTES (6-7)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 110mph (3-second gust); h=16ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- *This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 93 lb uplift at joint 2, 147 lb uplift at joint 10 and 375 lb uplift at joint 15.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



Builders FirstSource, Lake City, FL 32055 6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:55 2008 Page 1

Job

L287710

Truss

T25G

Truss Type

GABLE

Qty

1

Ply

1

CASH ACCT. - STAPE RES.

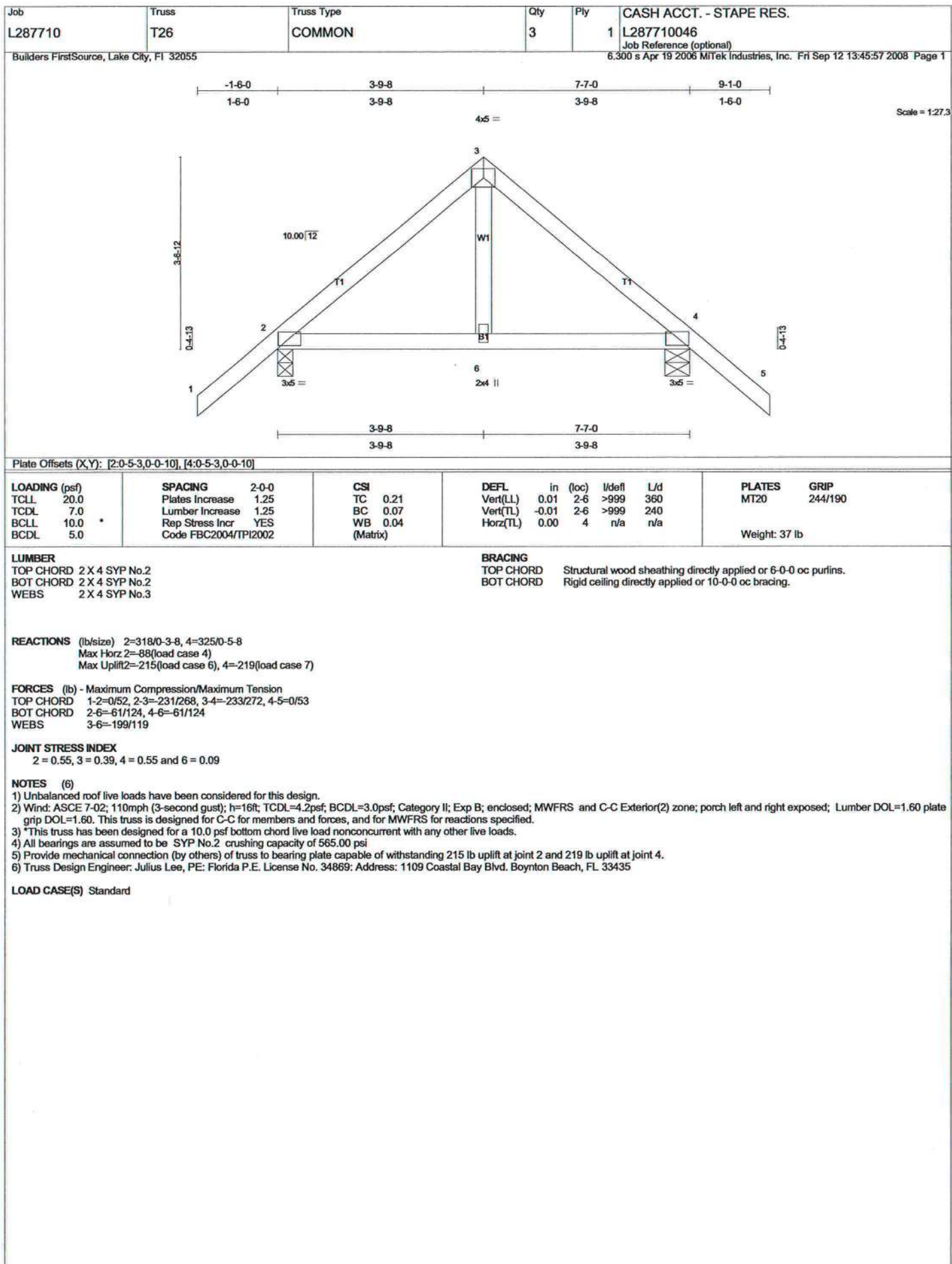
L287710045

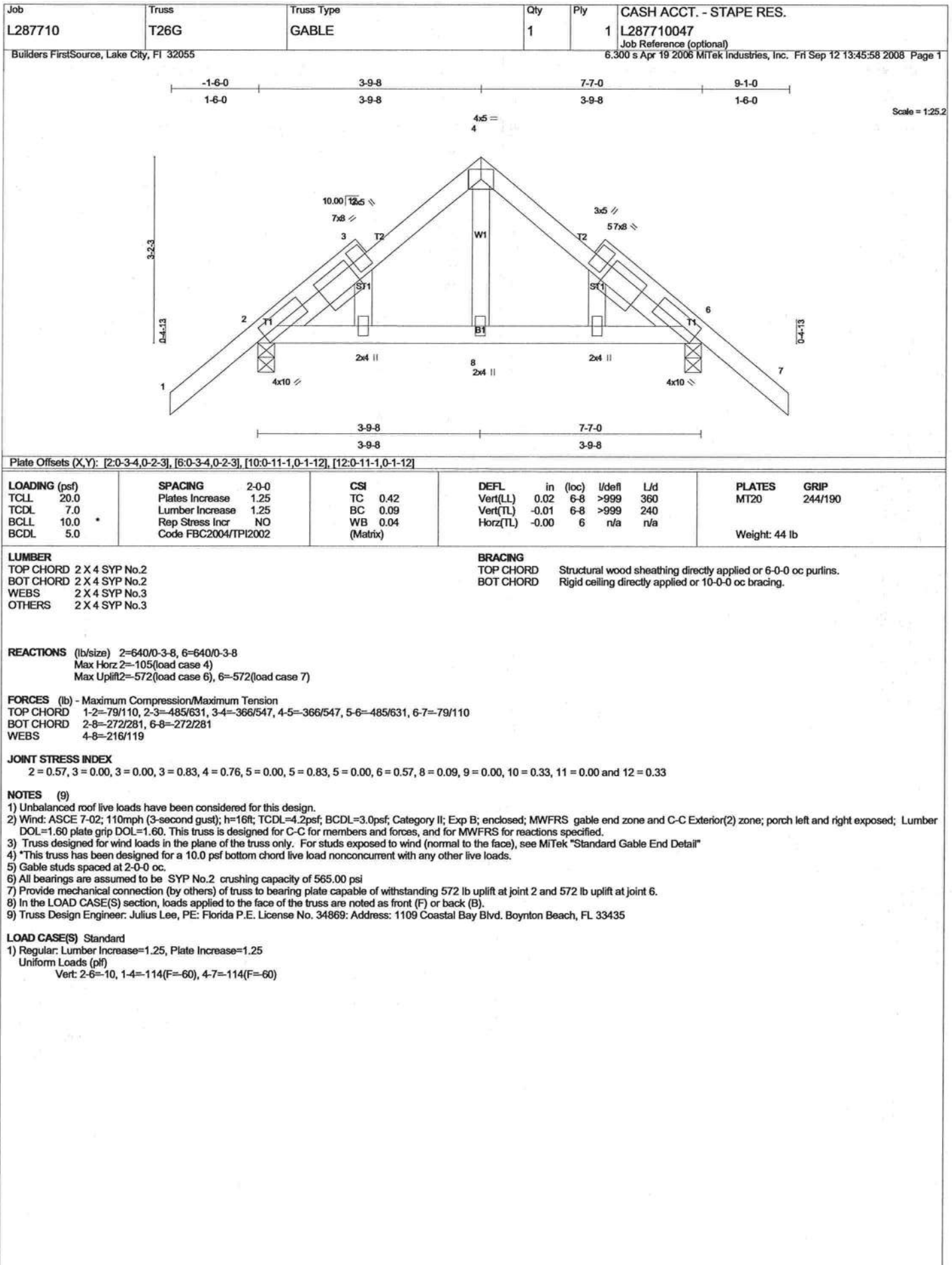
Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Fri Sep 12 13:45:56 2008 Page 1

Builders FirstSource, Lake City, FL 32055

Scale = 1/28.





Setback 7' or Less

UPLIFT: 400# or Less

400# or Less
*

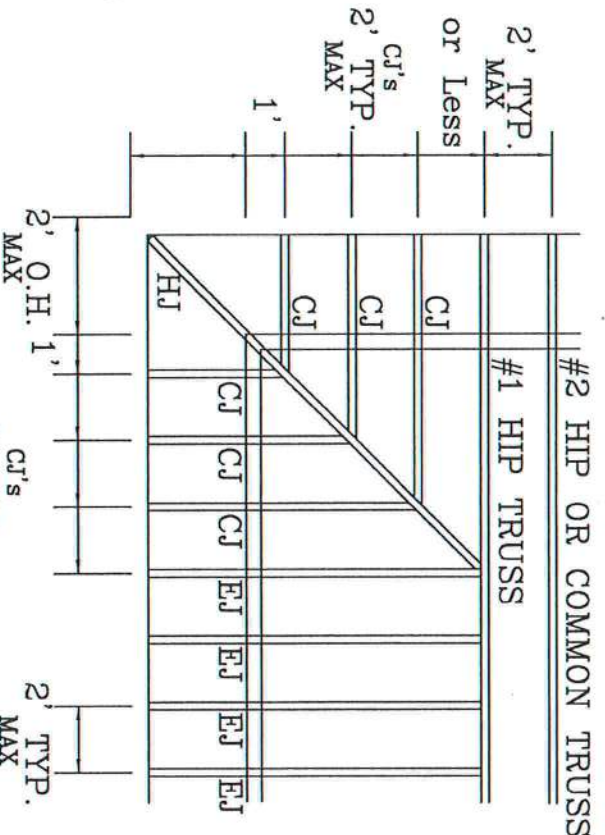
MAY

400# or Less

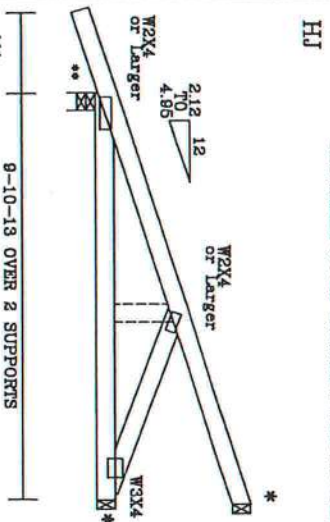
400# or Less
*

TIPJFY. 400# or Less

400# or Less
*



END AND CORNER JACKS



HIP JACK

SEE FOR THE DOWN

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET
SETBACK

7'0" MAX

UNLESS OTHERWISE INDICATED, ALL TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC1-1-03 BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS
PLATE INSTITUTE, 588 DUNDERRIDGE DR., SUITE 200, MADISON, WI 53719 AND VTOA CYCLOD TRUSS COUNCIL
OF AMERICA, 6300 ENTERPRISE BL, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING
THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED
STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED CEILING.

UNIMPORTANT: FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED
PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO
BUILD THE TRUSS IN CONFORMANCE WITH TPI OR APPLICATING, HANDLING, SHIPPING, INSTALLING &
BRACING OF TRUSSES. DESIGN CONDITIONS WITH TPI OR APPLICATING PROVISIONS OF NDS (NATIONAL DESIGN &
BY A/E/P) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 8018/8624 (A16/24) ASTM A575 GRADE
40/60 (A575) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND NESTED UNDER
ON THIS DESIGN, POSITION PER DRAWINGS ABOVE. ON THIS DRAWING INDICATES ACCEPTANCE OF
THE DESIGN AND ASSUMING RESPONSIBILITY AS A E/P. THE TRUSS COMPONENT DESIGN SHOWN, THE
SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING
DESIGNER. PER AWS1/PT1 SEC. 2.

CONS. ENGINEERS, P.A.
1266 SW 41st Avenue
MIAMI BEACH, FL 33141-2101

	MC	IT	20	MAX	PSTF
	TC	DL	79	MAX	PSTF
	BC	TU	10*	MAX	PSTF
	BC	TL	5	MAX	PSTF
	BC	DL			

REF	7'MAX STBK
DATE	Jun./27/2008
DRWG	

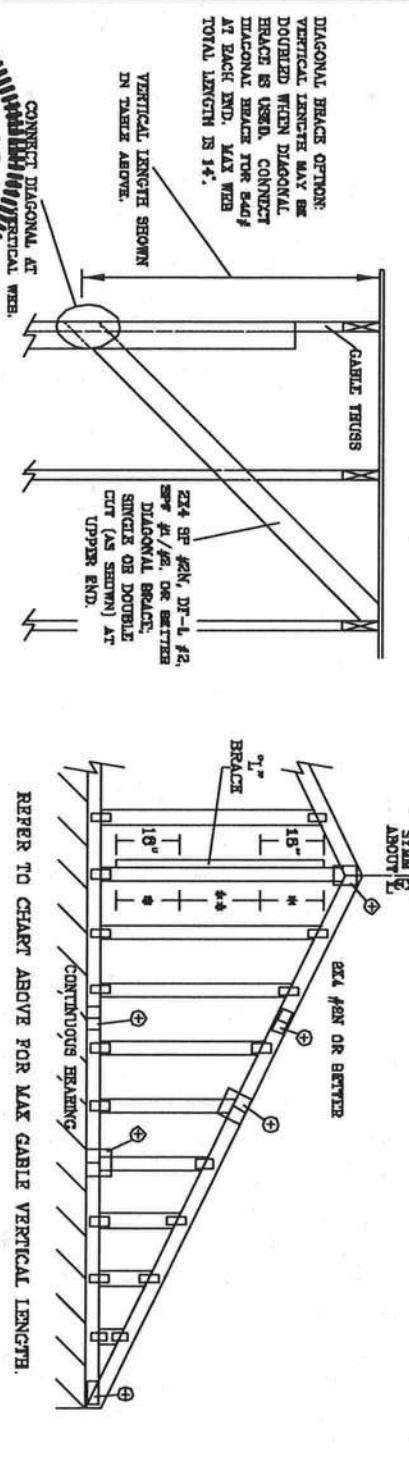
-ENG

REVIEWED

By Julius Lee at 10:52 am, Jun 27, 2008

ASCE 7-02: 130 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

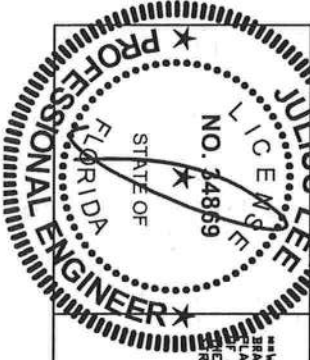
MAX GABLE VERTICAL LENGTH		BRACE		NO BRACES		(1) 1x4 "L" BRACE *		(1) 2x4 "L" BRACE *		(1) 2x6 "L" BRACE *		(2) 2x8 "L" BRACE *	
CABLE VERTICAL SPACING	SPECIES	GRADE	2x4	BRACE	NO	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
12" O.C.	SPF	#1 / #2	DFL	STUD	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	8' 3"	8' 3"	10' 10"	11' 2"
					3' 3"	4' 11"	4' 11"	6' 6"	8' 3"	8' 3"	10' 1"	10' 1"	12' 11"
					3' 3"	4' 11"	4' 11"	6' 5"	8' 3"	8' 3"	10' 0"	10' 0"	12' 11"
					3' 3"	4' 11"	4' 2"	5' 6"	7' 5"	8' 3"	8' 3"	11' 8"	11' 8"
16" O.C.	SPF	#1 / #2	DFL	STUD	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	8' 11"	10' 10"	12' 11"
					3' 7"	6' 10"	6' 3"	6' 11"	7' 6"	8' 3"	8' 11"	11' 8"	13' 11"
					3' 6"	5' 0"	6' 0"	6' 8"	8' 3"	8' 3"	10' 4"	12' 11"	13' 7"
					3' 4"	4' 3"	4' 3"	5' 8"	7' 8"	8' 3"	10' 3"	12' 11"	13' 7"
24" O.C.	SPF	#1 / #2	DFL	STUD	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	8' 6"	12' 6"	12' 9"	14' 0"
					3' 8"	6' 0"	6' 0"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"
					3' 8"	6' 0"	6' 0"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"
					3' 8"	6' 0"	6' 0"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"



CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPICE
LESS THAN 4' 0"	1x4 OR 2x3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2x4
GREATER THAN 11' 8"	2.5x4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPICE, AND BEEL PLATES.

BRACING GROUP SPECIES AND GRADES:	
GROUP A:	
SPICE-PINE-YR	SPICE-PINE-YR
#1 / #2 STANDARD	#2 STUD
#3 STUD	#3 STANDARD
GROUP B:	
SPICE-PINE-YR	SPICE-PINE-YR
#1 / #2 STANDARD	#2 STUD
#3 STUD	#3 STANDARD



REVIEWED By Julius Lee at 12:00 pm, Jun 11, 2008

STATE OF FLORIDA

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

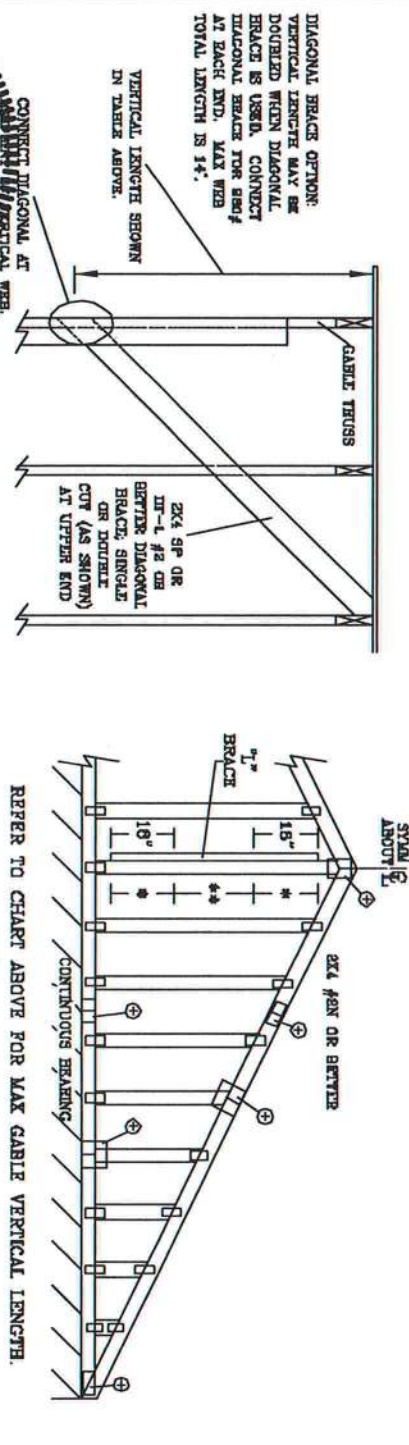
REF ASCE 7-02-CAB13015

DATE 11/26/03

DRWG MTRX STD CABLE IS E BT -ENG

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH															
CABLE VERTICAL SPACING	2x4 SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 "L" BRACE *											
				(1) 2x4 "L" BRACE *		(2) 2x4 "L" BRACE **		(1) 2x6 "L" BRACE *		(2) 2x6 "L" BRACE *		(2) 2x8 "L" BRACE **			
12" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"		
			#3	3' 1"	4' 5"	4' 5"	6' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	
		HF	STUD	3' 1"	4' 6"	4' 5"	5' 10"	6' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"	
			STANDARD	2' 11"	3' 9"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"	
		SP	#1	3' 6"	5' 8"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"	
	#2		3' 6"	5' 6"	5' 11"	6' 6"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"		
	#3		3' 3"	4' 6"	4' 8"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 6"		
	DFL	STUD	3' 3"	4' 8"	4' 6"	5' 11"	5' 11"	6' 11"	8' 0"	9' 3"	9' 3"	12' 3"	12' 6"		
		STANDARD	3' 0"	3' 10"	3' 10"	6' 1"	5' 1"	6' 11"	8' 11"	9' 0"	9' 0"	10' 10"	10' 10"		
		SPF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 6"	8' 11"	9' 2"	11' 6"	12' 1"	14' 0"	14' 0"	
#3			3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"		
HF		STUD	3' 7"	5' 6"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"		
	STANDARD	3' 7"	4' 6"	4' 8"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"	12' 11"			
	SP	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	8' 7"	11' 9"	12' 8"	14' 0"	14' 0"		
		#2	3' 11"	8' 4"	8' 4"	7' 6"	8' 1"	8' 11"	8' 7"	11' 9"	12' 8"	14' 0"	14' 0"		
	DFL	STUD	3' 8"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	8' 6"	11' 5"	11' 6"	14' 0"	14' 0"		
STANDARD		3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 5"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 8"	9' 8"	13' 3"	13' 3"		
		#3	4' 0"	6' 11"	7' 2"	6' 3"	6' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"	14' 0"		
HF		STUD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"	14' 0"		
	STANDARD	3' 11"	6' 3"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 10"	12' 10"	14' 0"	14' 0"			
	SP	#1	4' 5"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"		
		#2	4' 4"	6' 11"	7' 6"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"	14' 0"		
	DFL	STUD	4' 2"	6' 6"	6' 5"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	12' 11"	14' 0"	14' 0"		
STANDARD		4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	DFL	STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
STANDARD		4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
SPF		#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
		#3	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
HF		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"		
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"	14' 0"	14' 0"			
	SPF	#1 / #2	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	9' 9"	9' 9"	11' 4"	11' 4"				



BRACING GROUP SPECIES AND GRADES:		GROUP A:		GROUP B:	
SPRUCED PINE - FIR	#1 / #2	STUD	STUD	DOUGLAS FIR - LARCH	#1 / #2
	#3	STUD	STUD		#3
DOUGLAS FIR - LARCH	#1 / #2	STUD	STUD	SOUTHERN PINE	#1 / #2
	#3	STUD	STUD		#3

CABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS $L/240$.

PROVIDE UPLIFT CONNECTIONS FOR 160 PSF OVER CONTINUOUS BEAMING (6 PSF VC DEAD LOAD).

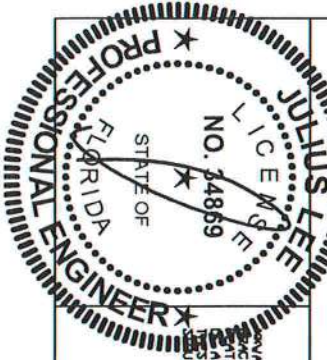
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

** FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.



REVIEWED By Julius Lee at 12:00 pm, Jun 11, 2008

STATE OF FLORIDA

JULIUS LEE'S CONS. ENGINEERS P.A.

1456 SW 4th AVENUE
ORLANDO, FL 32801

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

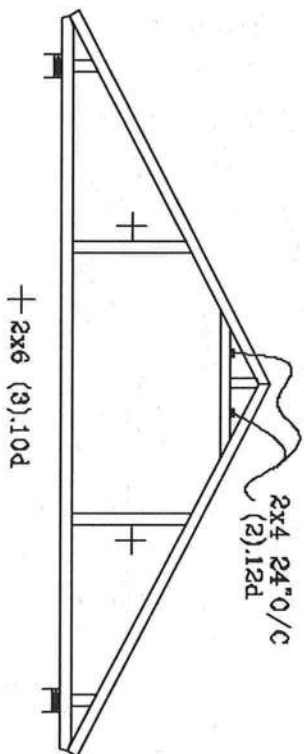
REF ASCE 7-02-CAB10030

DATE 11/26/03

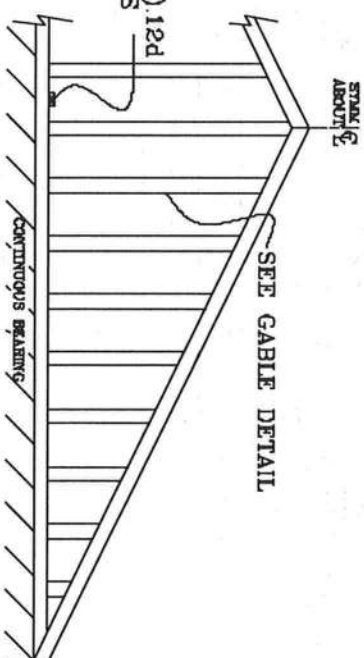
DWG BY: JLE

ENG

TYPICAL ATTIC TRUSS BRACING

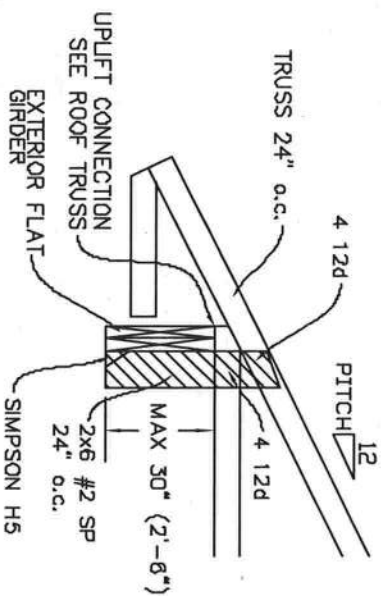


GABLE END TRUSS DETAIL

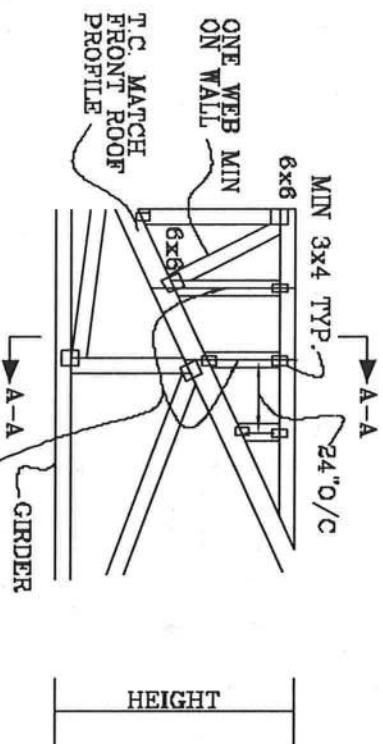


MINIMUM 800 BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL ALTERNATE BRACING DETAIL
FOR EXTERIOR FLAT GIRDER TRUSS

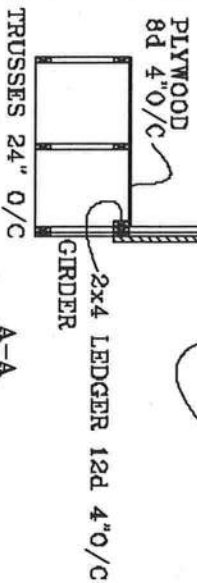


TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL

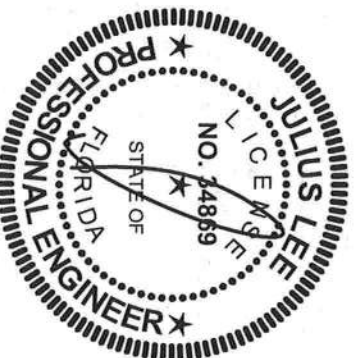


~~SEE ROOF TRUSSES
FOR UPLIFT~~ ROOF 24" O/C

SEE CABL. END DETAIL
FOR T-BRACE BEHIND
EACH VERTICAL _____



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008



**JULIUS LEE'S
CONS. ENGINEERS P.A.**
1455 SW 4th AVENUE
DUNBAR BEACH, FL. 33444-2161

No: 34869
STATE OF FLORIDA

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PIGGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG,

LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST

CAT 1, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

110 MPH WIND, 30' MEAN HGT, FBC

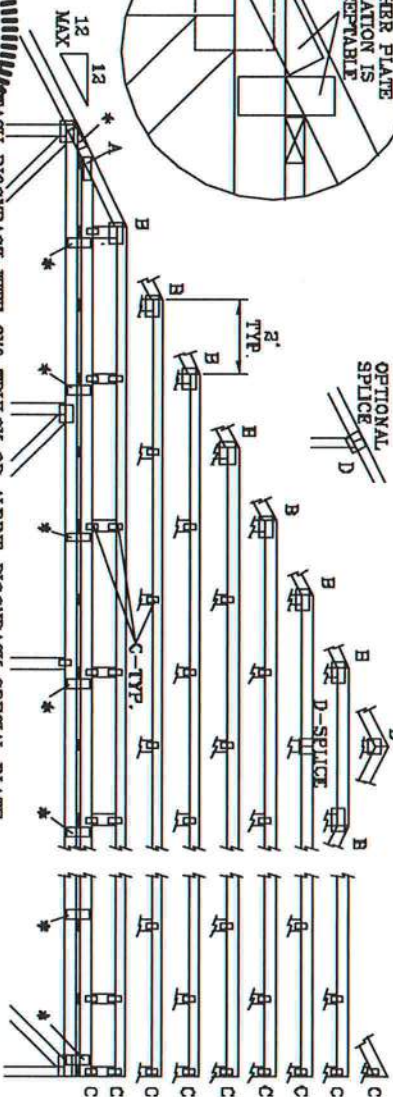
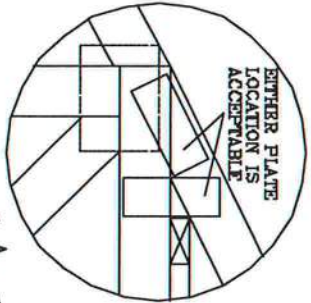
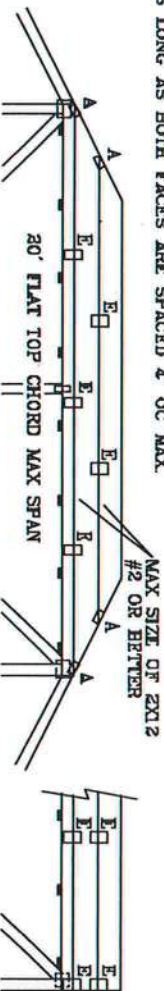
ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF

WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE

PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=6 PSF, WIND BC DL=6 PSF



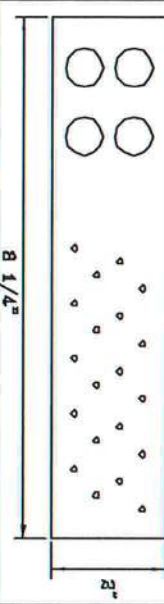
TRUSS PIGGYBACK WITH 3X6 TRUSS OR ALPINE PIGGYBACK SPECIAL PLATE.

JOINT TYPE	SPANS UP TO			
	30'	34'	38'	62'
A	2X4	2.6X4	2.6X4	3X6
B	4X8	6X8	6X8	5X8
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	6X5	6X5	5X6
E	4X8 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY			

ATTACH TRUSS PLATES WITH (6) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.

* PIGGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PIGGYBACK AT THE TUE OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



NOTATION: TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO WEST I-80 GUIDING COMPONENT SAFETY DEGRADATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 2880 BROADVIEW DR., SUITE 200, WASHINGTON, VA 22750 AND AISC TRUSS COUNCIL, 1100 N. 17TH ST., SUITE 100, NORTON, VA 22750 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIBBON CEILING.

JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 SW 42ND AVENUE
OCEAR BEACH, FL 33444-2161

THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

MAX LOADING

65 PSF AT

1.33 DUR. FAC.

50 PSF AT

1.25 DUR. FAC.

47 PSF AT

1.15 DUR. FAC.

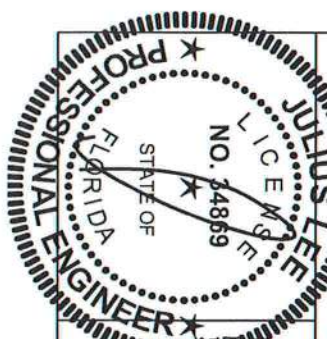
SPACING 24.0"

REF PIGGYBACK

DATE 09/12/07

DWG/MTK STD PIGGY

-ENG JL



REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

No. 34869
STATE OF FLORIDA

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
VEES 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH. ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15" MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=6 PSF.

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "I"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERTY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS
INSTALLATION

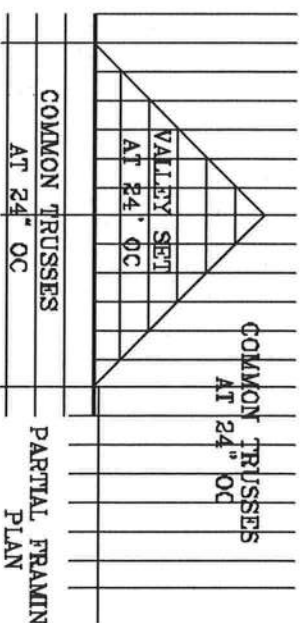
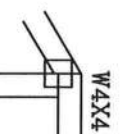
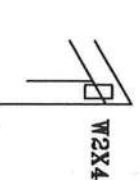
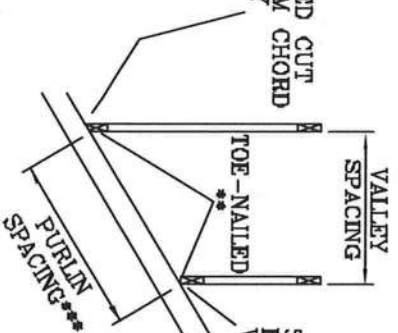
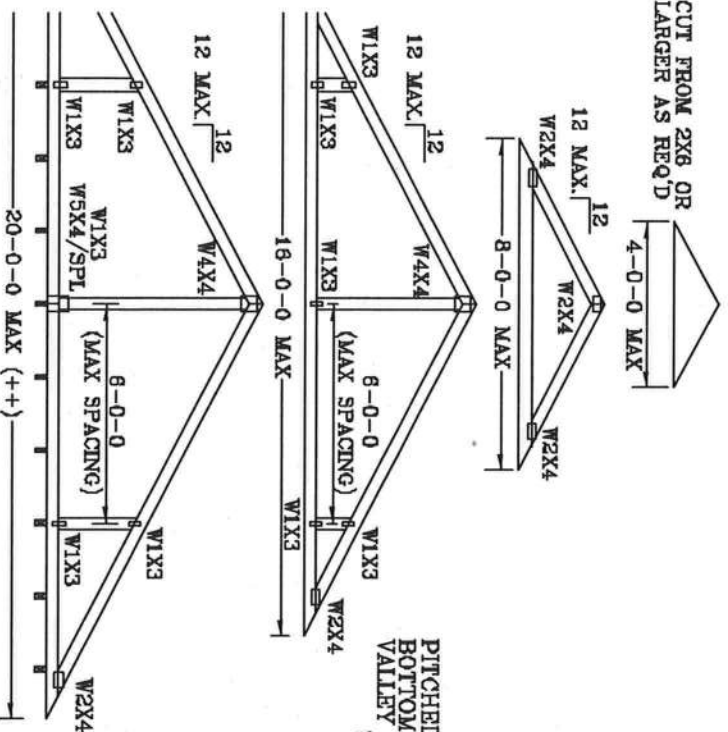
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN



TRUSSES AT 24" OC MAXIMUM SPACING.

THIS DRAWING REPLACES DRAWING A105

PARTIAL FRAMING PLAN

SAVING MORE BUSINESS, REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND PACKAGING. REFER TO BEST-OF-BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 1000 W. 15TH AVE., SUITE 500, DENVER, CO 80202. FOR THE LATEST TRUSS DESIGN, AMERICAN DIFFERENCES, A HUNDRED OF SEVERAL TYPES OF TRUSSES, AND THE MANY DIFFERENT TYPES OF JOINTS AND FUNCTIONS. UNLESS OTHERWISE SPECIFIED, TOP CHORD SHALL HAVE PROPERLY ATTACHED MEMBERS, BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

ULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2161

TC LL	20	PSF	REF	VALLEY DETAIL
TC DL	7	PSF	DATE	11/26/03
BC DL	5	PSF	DRWG	VALTRUSS1103

-ENG JL

REVIEWED

By julius lee at 11:59 am, Jun 11, 2008

No: 34869
STATE OF FLORIDA

DUR.FAC. 1.2
SPACING:

--	--

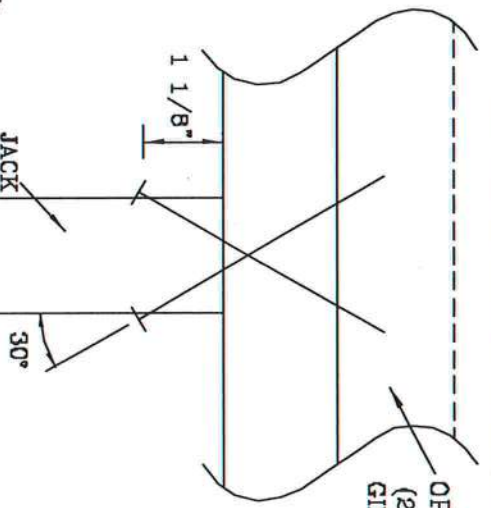
TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/APA NDS-2001 SECTION 12.4.1 – EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

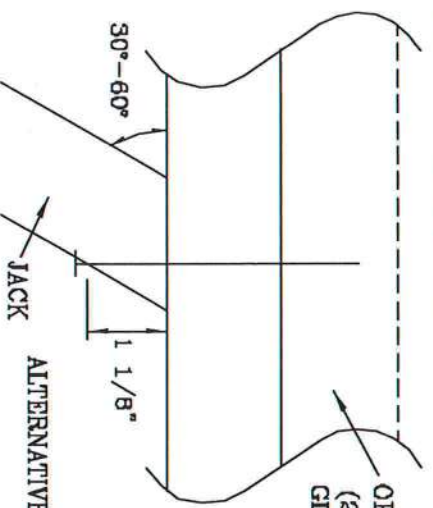
THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES
2	187#	256#	181#	234#	156#	203#	154#	189#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	639#	452#	585#	390#	507#	384#	496#

OPTIONAL
(2) PLY
GIRDER

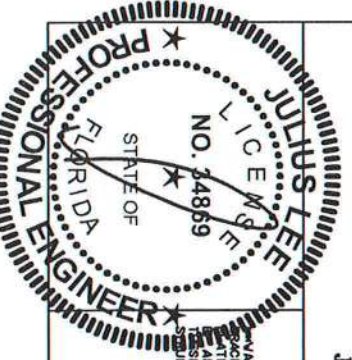


OPTIONAL
(2) PLY
GIRDER



ALTERNATIVE CONDITION

THIS DRAWING REPLACES DRAWING 784040



WARNING: TRIMERS REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, DRYING AND BURNING. REFER TO BEST PRACTICES FOR CONDUCTING SAFETY TRAINING. NEVER USE TRIMMER ON HARDENED SURF. ENTERPRISE, INC. (MILWAUKEE, WI 53015) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TRIP CORD SHALL HAVE PERSELY ATTACHED ELECTRICAL PANTS, AND BOTTOM CORD SHALL HAVE A PERSELY ATTACHED REEL DIALS.

REVIEWED
By Julius lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
CONS. ENGINEERS P.A.

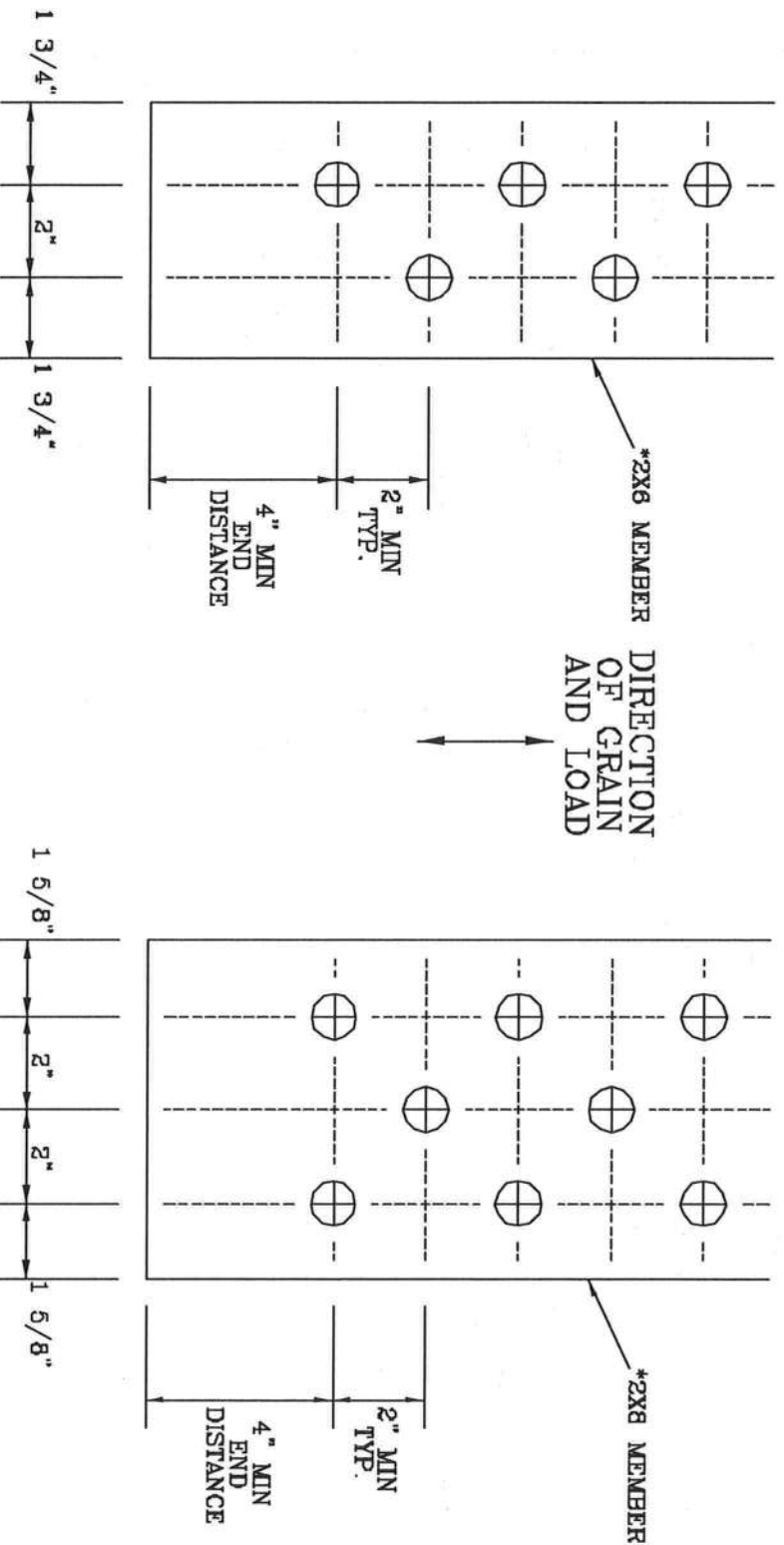
No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONALL103
BC LL	PSF	-ENG JL	
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A628.016

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTION. REFER TO POST 1-80 QUALITY DEPENDENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS LATE INSTITUTE, 580 OAKRIDGE DR., SUITE 200, MARIETTA, GA 30067 AND APCA CODED TRUSS COUNCIL, 10000 W. 11TH AVE., SUITE 100, DENVER, CO 80233 FOR SAFETY PRACTICES PRIOR TO PERFORMING REPAIRS OR MODIFICATIONS. ALL TRUSSES MUST BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER. STRUCTURAL PANELS AND ATTACHED CHORDS SHALL HAVE A PERMANENT ATTACHED RIGID CHORD.

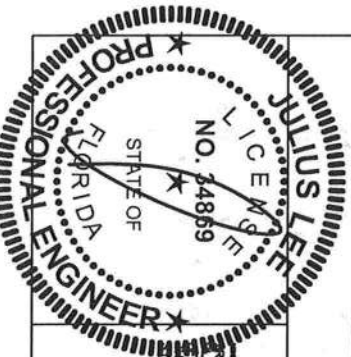
JULIUS LEE'S
CONS. ENGINEERS P.A.
1400 17th AVE. N.E.
DELUPT BRIDGE, FL 33444-2461

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTSPI103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		

REVIEWED

By Julius Lee at 11:59 am, Jun 11, 2008

No: 34869
STATE OF FLORIDA



TRULOX CONNECTION DETAIL

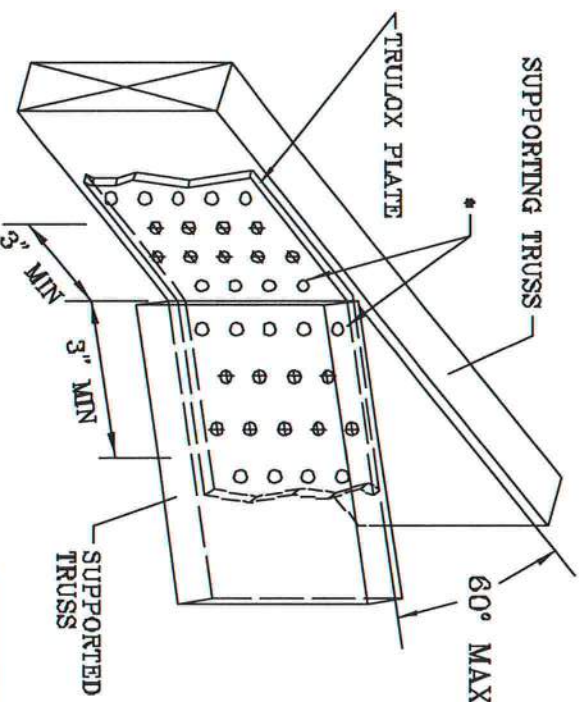
11 GAUGE (0.120" X 1.376") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

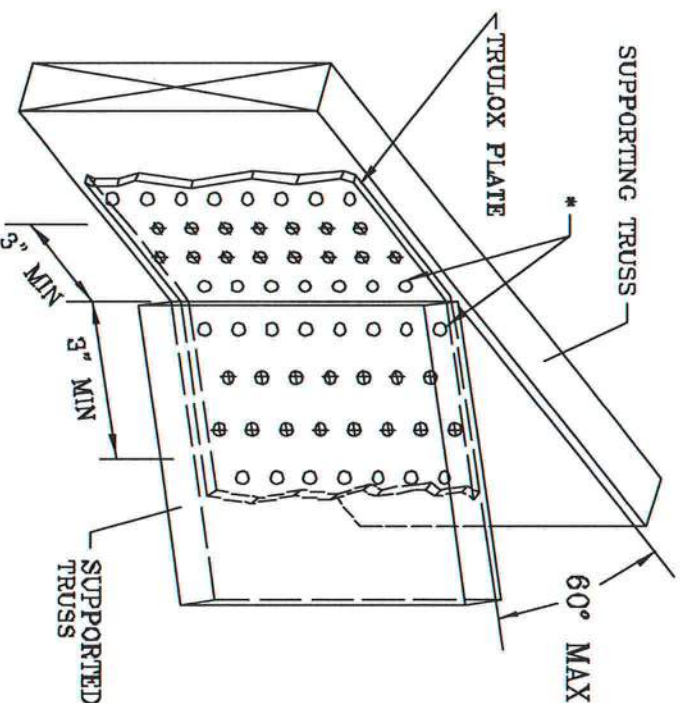
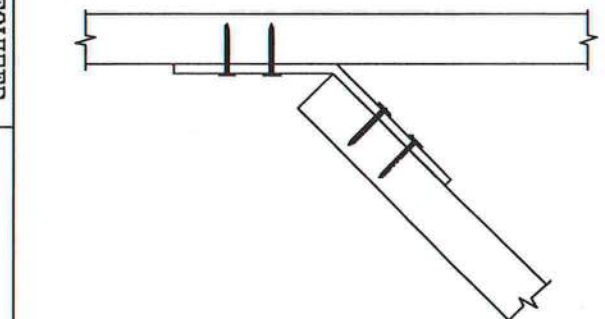
TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



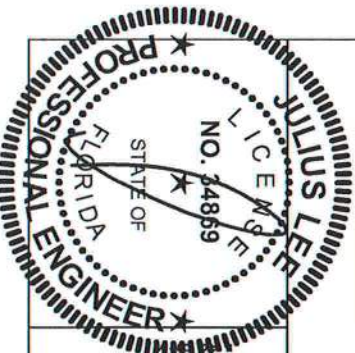
MINIMUM 3X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	990#



MINIMUM 5X6 TRULOX PLATE

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND MAINTAINING. REFER TO AISC 1-10 (BUILDING COMPONENT SAFETY DEFORMATION), PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 390 JONATHAN DR., SUITE 200, WASHINGTON, VA 22719 AND AISC TRUSS COUNCIL, 1100 N. 17TH ST., SUITE 100, DENVER, CO 80202. TRUSSES MUST BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE AISC TRUSS COUNCIL DESIGN MANUAL, 1ST EDITION, 1989. TRUSSES MUST BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE AISC TRUSS COUNCIL DESIGN MANUAL, 1ST EDITION, 1989. TRUSSES MUST BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH THE AISC TRUSS COUNCIL DESIGN MANUAL, 1ST EDITION, 1989.

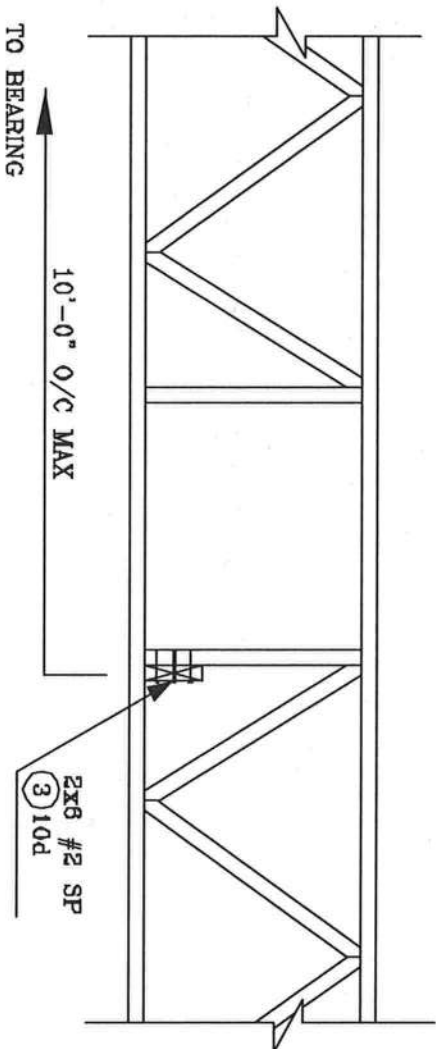
JULIUS LEE'S
CONS. ENGINEERS P.A.
1455 SW 4th AVENUE
DECATUR, GA 30030-2101

No. 34869
STATE OF FLORIDA

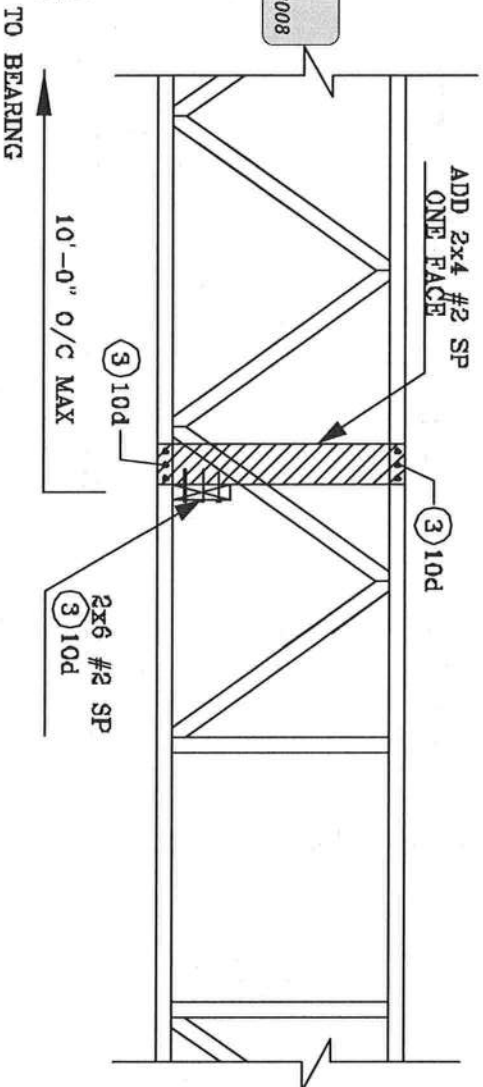
THIS DRAWING REPLACES DRAWINGS 1,158,989 1,158,989/R
1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

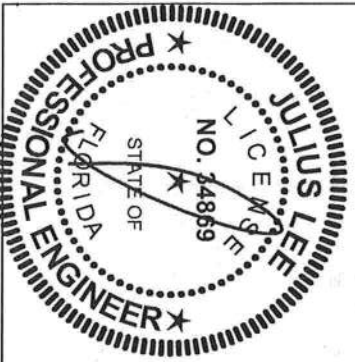
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008



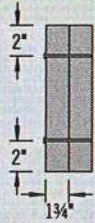
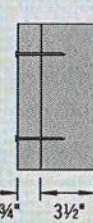
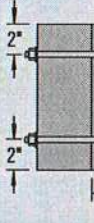
JULIUS LEE'S
CONS. ENGINEERS P.A.

1425 SW 4th AVENUE
DEER BEACH, FL 33444-2161

No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/8" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"	500	500	500	445	480	445
		19.2"	625	625	625	555	600	555
		16"	750	750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

(1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.

(2) Washers required. Bolt holes to be 1/16" maximum.

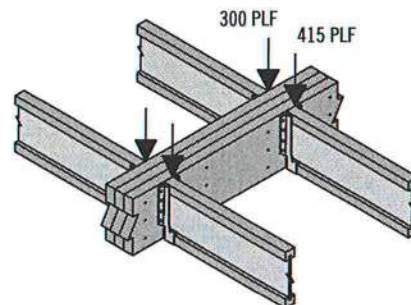
(3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

(4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



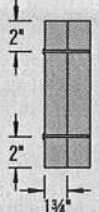
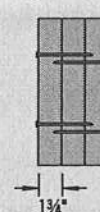
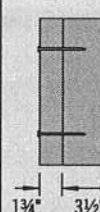
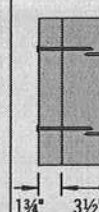
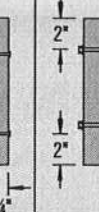
First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 1 3/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
							
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

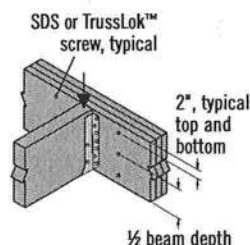
(2) 6" long screws required.

(3) 5" long screws required.

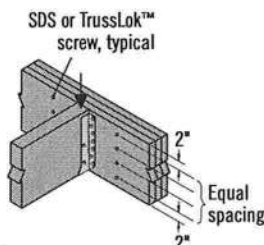
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections

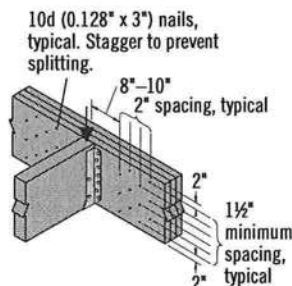
4 or 6 or Screw Connection



8 Screw Connection

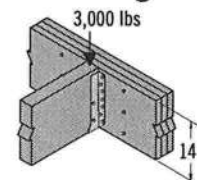


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d-16d (0.148"-0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

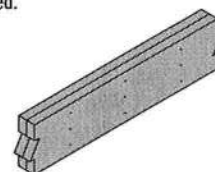
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7'

L6

