#### ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TED8228Z0222102934

Truss Fabricator: Anderson Truss Company

Job Identification: 7-278--GARY JOHNSON Shoup -- , \*\*

Truss Count: 1

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002 (STD) / FBC

Engineering Software: Alpine Software, Version 7.36.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-

#	Ref	Description	Drawing#	Date
1	33303-	-FT1	08022001	01/22/08

4.564

Seal Date: 01/22/2008

-Truss Design Engineer-James F. Collins Jr. Florida License Number: 52212 1950 Marley Drive Haines City, FL 33844



FII ( 7-278--GARY JOHNSON Shoup

SP #2 Dense SP #2 SP #3 Top chord 2x4 S Bot chord 2x6 S Webs 2x4 S

/ PLATE DUR.FAC.=1.25) to 60 PLF at 9.63 to 20 PLF at 9.63 1.92, 3.92, 5.92, 7. 5.92, SPECIAL LOADS

(LUMBER DUR.FAC.=1.25 / PLATE
TC - From 60 PLF at 0.00 to 6
BC - From 20 PLF at 0.00 to 7
BC - A74 LB Conc. Load at 1.92,

7.92

Wind reactions based on MWFRS pressures.

24" The TC of this truss shall be braced with attached spans at OC in lieu of structural sheathing. THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS").TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

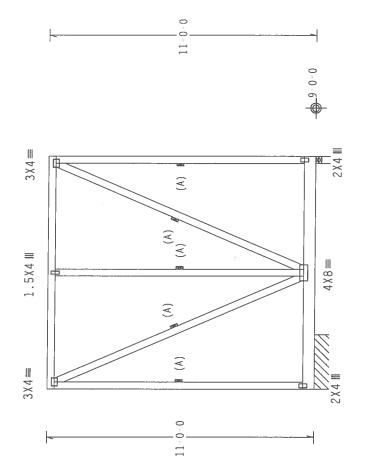
110 mph wind, 20.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.25 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

End verticals not exposed to wind pressure

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50.\,$ 

dn. Truss must be installed as shown with top chord



0 R=650 PLF U=425 PLF W=2-3-

R-1203 U-777 W-3.5"

2 Over -9-7-8

Supports

7.36.042

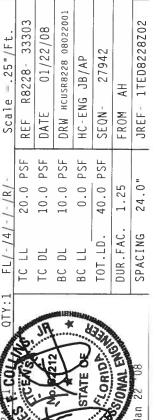
Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

PLT TYP. Wave

ITW Building Components Group, Inc. Haines City, FL 33844 FL Certificate of Authorization # 0 278 ALP I NE

\*\*WARNING\*\* IRUSSES REQUIRE EXTREME CARE IN FABRICATION, MANDELHG. SHIPPING, INSTALLING AND BRACEING. RETER TO DO SCST. (WILLDING COMPONENT SAFETY OF DEBLISHED SPETIFIES THE LISTITUTE. 218 NORTH LEE STREET. SHITE 312. ALEXANDRIA, "Y. 22314) AND VITCA (MODO TRUSS COUNCIL OF AMERICA, GADO TEMERRALE LAME, MADION, MI 53719) FOR SAFETY PRACEITES PRIOR TO PERFORMING INESS FUNCTIONS. UNILESS OFHIGHIST HOLICATED OF CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

Jan



# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

#### NOTES

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING. ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING

WEB MEMBER	SPECIFIED CLB	ALTERNATIV	ALTERNATIVE BRACING L-BRACE SCAB BRACE
SIZE	BRACING	T OR L-BRACE	
OR 2X4	1 ROW	2X4	1-2X4
OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

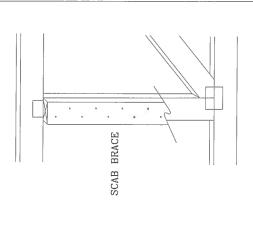
T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB \*

L-BRACE I-BRACE T-BRACE L-BRACE OR APPLY TO EITHER SIDE OF WEB NARROW FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A
MINIMUM 80% OF WEB
MEMBER LENGTH L-BRACING: T-BRACING

# SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB. NO MORE THAN (1) SCAB PER FACE. ATTACH WITH 10d BOX OR GUN (0.128"x 3."MIN) NAILS. AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH





\*\*\*WARNING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND MEACHGA. FERF TO GEST GRULLING COMPANIANCE, 218 NORTH LEE STR, SULTE 312, ALEXANDRIA, VA 22314) AND VITA (VOID TRUSS COUNCIL RETRICA, 208 CHTERPRIELS, LI, MADISON, WI 337195 TRE SAFETY REACTICES PRIOR TO PERFORMED FUNCTIONS. UNLESS OTHERWISE INDICATED, TIP CHIND SHALL HAVE PROPERLY ATTACHED STRUCTURE. WHORPTATATE FIRMUSH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BEG, INC., (MALL MOTE RESPONSIBLE FIRM ANY DEVIATION FROM THIS DESIGNA WAY FAILURE IT DBLICH THE FRUSS WING CORP. COMP. CONTRACTOR OF TRACTOR OF TRA

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA ALPINE

JAPACIING

SONAL ENGT

	TC LL	PSF REF	REF	CLB SUBST.
	TC DL	PSF	PSF DATE	2/23/07
	BC DL	PSF	PSF DRWG	BRCLBSUB0207
	BC LL	PSF		-ENG MLH/KAR
	TOT. LD.	PSF		
11314000	DUR. FAC.			
•••	SDACING			

THIS DRAWING REPLACES DRAWING 579,640

### 26316

#### ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TB28228Z0126145201

Truss Fabricator: Anderson Truss Company

Job Identification: 7-278--GARY JOHNSON GARY JOHNSON / SHOUP -- , \*\*

Truss Count: 28

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Versions 7.36, 7.37.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

#### Notes

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2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

22

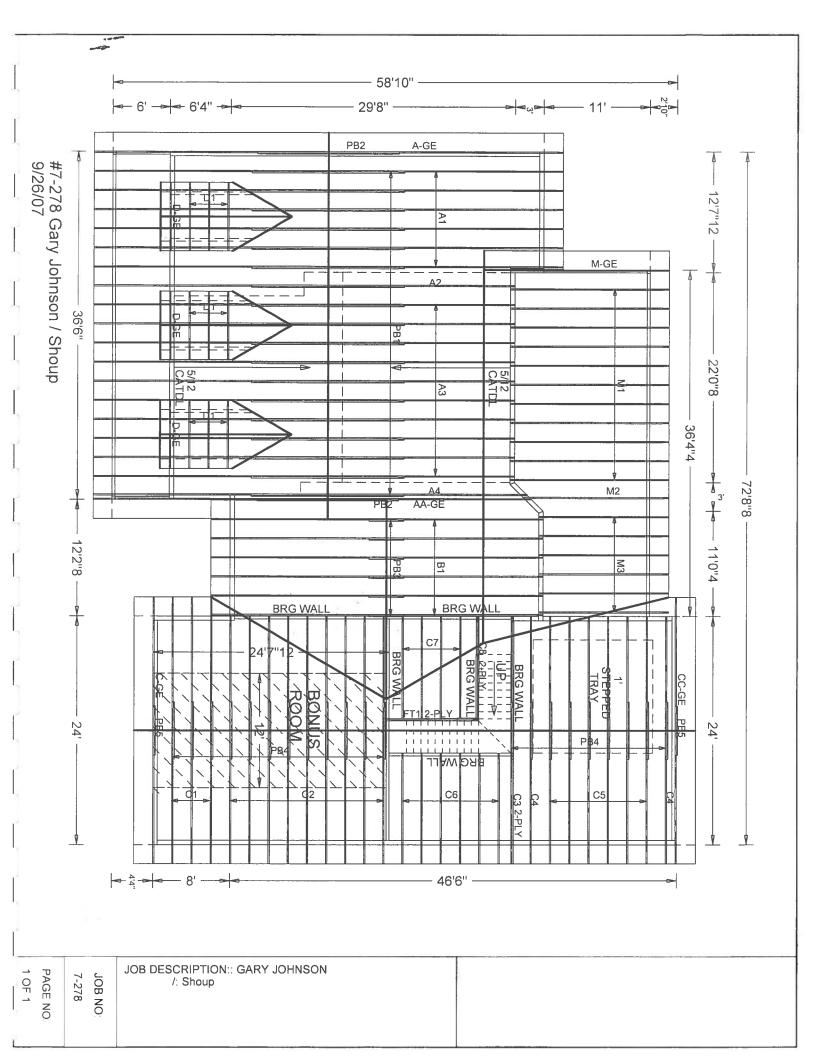
Seal Date: 09/26/2007

-Truss Design Engineer-Doug Fleming Florida License Number: 66648 1950 Marley Drive Haines City, FL 33844

Details: BRCLBSUB-140GC-CNBRGBLK-A11015EE-GBLLETIN-A11015EC-140PB-PIGBACKA-PIGBACKB-A11030EE-140GS-A10030EC-GBLBR

1 54791A1 07269021 09/26/07 2 54792A-GE 07269022 09/26/07 3 54793A3 07269023 09/26/07 4 54794A2 07269006 09/26/07 5 54795A4 07269007 09/26/07 6 54797B1 07269009 09/26/07 7 54798C5 07269001 09/26/07 8 54799C4 07269010 09/26/07 9 54800CC-GE 07269028 09/26/07 10 54801C3 07269011 09/26/07 11 54802C6 07269012 09/26/07 12 54803C7 07269013 09/26/07 13 54804C8 07269014 09/26/07 14 54805C2 07269015 09/26/07 15 54806C1 07269016 09/26/07 16 54807C-GE 07269029 09/26/07 17 54808FT1 07269029 09/26/07 18 54809M-GE 07269005 09/26/07 19 54810M1 07269002 09/26/07 20 54811M2 07269003 09/26/07 21 54812M3 07269004 09/26/07 22 54813PB4 07269017 09/26/07 23 54814PB5 07269004 09/26/07 24 54815PB3 07269018 09/26/07 25 54816PB1 07269020 09/26/07 26 54817PB2 07269020 09/26/07 27 54818DL 07269020 09/26/07 28 54819D-GE 07269027 09/26/07	#	Ref Description	Drawing#	Date
3 54793A3 07269023 09/26/07 4 54794A2 07269006 09/26/07 5 54795A4 07269007 09/26/07 6 54797B1 07269009 09/26/07 7 54798C5 07269001 09/26/07 8 54799C4 07269010 09/26/07 9 54800CC-GE 07269028 09/26/07 10 54801C3 07269011 09/26/07 11 54802C6 07269012 09/26/07 12 54803C7 07269013 09/26/07 13 54804C8 07269014 09/26/07 14 54805C2 07269015 09/26/07 15 54806C1 07269016 09/26/07 16 54807C-GE 07269016 09/26/07 17 54808FT1 07269029 09/26/07 18 54809M-GE 07269029 09/26/07 19 54811M2 07269005 09/26/07 20 54811M2 07269000 09/26/07 21 54812M3 07269004 09/26/07 22 54813PB4 07269017 09/26/07 23 54814PB5 07269018 09/26/07 24 54815PB3 07269019 09/26/07 25 54816PB1 07269020 09/26/07 26 54817PB2 07269020 09/26/07			07269021	09/26/07
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14       54805C2       07269015       09/26/07         15       54806C1       07269016       09/26/07         16       54807C-GE       07269029       09/26/07         17       54808FT1       07269024       09/26/07         18       54809M-GE       07269005       09/26/07         19       54810M1       07269002       09/26/07         20       54811M2       07269003       09/26/07         21       54812M3       07269004       09/26/07         22       54813PB4       07269017       09/26/07         23       54814PB5       07269025       09/26/07         24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07	12	54803C7	07269013	09/26/07
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16       54807C-GE       07269029       09/26/07         17       54808FT1       07269024       09/26/07         18       54809M-GE       07269005       09/26/07         19       54810M1       07269002       09/26/07         20       54811M2       07269003       09/26/07         21       54812M3       07269004       09/26/07         22       54813PB4       07269017       09/26/07         23       54814PB5       07269025       09/26/07         24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07	14	54805C2	07269015	09/26/07
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18       54809 M-GE       07269005       09/26/07         19       54810 M1       07269002       09/26/07         20       54811 M2       07269003       09/26/07         21       54812 M3       07269004       09/26/07         22       54813 PB4       07269017       09/26/07         23       54814 PB5       07269025       09/26/07         24       54815 PB3       07269018       09/26/07         25       54816 PB1       07269019       09/26/07         26       54817 PB2       07269026       09/26/07         27       54818 D1       07269020       09/26/07	16	54807 C - GE	07269029	09/26/07
19       54810M1       07269002       09/26/07         20       54811M2       07269003       09/26/07         21       54812M3       07269004       09/26/07         22       54813PB4       07269017       09/26/07         23       54814PB5       07269025       09/26/07         24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07			07269024	
20       54811M2       07269003       09/26/07         21       54812M3       07269004       09/26/07         22       54813PB4       07269017       09/26/07         23       54814PB5       07269025       09/26/07         24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07				
21       54812M3       07269004       09/26/07         22       54813PB4       07269017       09/26/07         23       54814PB5       07269025       09/26/07         24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07			07269002	
22       54813PB4       07269017       09/26/07         23       54814PB5       07269025       09/26/07         24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07				
23       54814PB5       07269025       09/26/07         24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07			07269004	
24       54815PB3       07269018       09/26/07         25       54816PB1       07269019       09/26/07         26       54817PB2       07269026       09/26/07         27       54818D1       07269020       09/26/07				
25 54816PB1 07269019 09/26/07 26 54817PB2 07269026 09/26/07 27 54818D1 07269020 09/26/07			07269025	
26 54817PB2 07269026 09/26/07 27 54818D1 07269020 09/26/07				
27 54818D1 07269020 09/26/07				
28 54819D-GE 07269027 09/26/07				
	28	54819D-GE	07269027	09/26/07

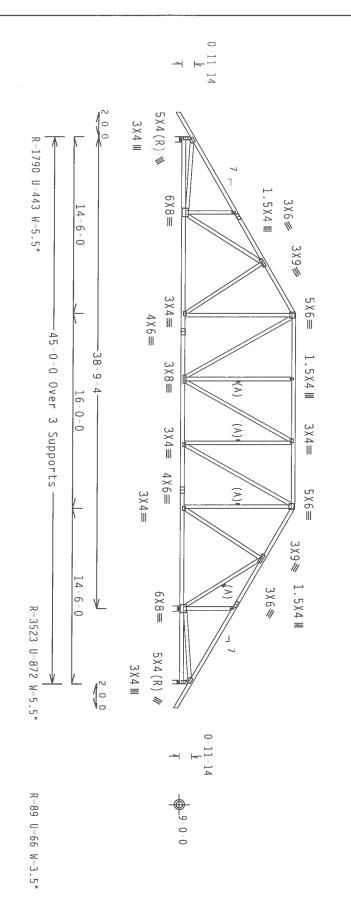




Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP SPECIAL LOADS From From From rom From From From From From From From (LUMBER LB Conc. 63 DUR.FAC PLF #2 Dense #2 Dense #3 at at аt at Load a t j p 30 32 38 39 10 10 45 39.00 0.1 to tο 0.1 0.1 ίO to 0.1 0.1 0.1 63 63 282 287 63 20 20 5 1.25) 1.6.49 1.14.50 1.30.50 1.32.42 1.38.51 1.39.00 1.47.00 1.45.00 1.45.00 1.45.00 IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT. WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels use purlins to brace all flat TC  $24\,^{\circ}$  OC. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, iw=1.00 GCpi(+/-)=0.55 (A) Continuous lateral bracing equally spaced on member. BUILDING DESIGNER AND shipping

Wind

reactions based on MWFRS pressures.



\*\*MARNING\*\* REUSISE REQUIRE EXTREM CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
HETER TO BEST. (BUILDING COMPONENT SALETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 718
HORTH LE SHELT, SHILT 317, ALEXANDRIA, VA, 2214) AND NICA (ADOD TRUSS COMUCIL OF ARRICA, 5000
HITERPRISE (LAKE, MADISON, MI 53719) FOR SALETY PRACTICES PRIOR TO PERFORMING INFSET FUNCTIONS. UNLESS
WHITERPRISE (LORGIED TOP COMED SMALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGID CEILING. TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

PLT

TYP.

Wave

Design Crit:

\*\*IMPORTANT\*\*\*(IRRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, THG, SHALL HOT BE RESPONSEDE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH IPT; OR FARRICATHING, SAMEDYING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROYESTOMS OF HOS (MATONAL DESIGN SPIC, BY AFAPA) AND TPT.

CONNECTION PLATES ARE HADE OF 20/18/16GA (M.H/SS/K) ASTH A653 GRADE 40/50 (M.K/M.SS) GAVE. SEECL. APPLY

HERE BUILDING DESIGNER PER ANSI/TPI I SEC. DESIGN SHOWN, I PLATES TO EACH TACE OF TRUSS AND, UNICESS OTHERWISE LOCATED ON THIS TOSSIGN, POSITION PER BRAHINGS 160A-Z, ANY INSPECTION OF PLATES FULLOWED BY (1) SHALL BE PER ANNEX AS OF FPIL 2007 SEC. 3. A SEAL ON THIS DRAWLING HIDICALES ACCEPTANCE OF PROFESSIONAL RESOURCES. THE SULFABILITY AND TPI1 2002 SEC.3. A SEAL ON THIS BILLITY SOLELY FOR THE TRUSS COMPONENT BUTLDING IS THE RESPONSIBILITY OF THE

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ALPINE



40.0 10.0 10.0 20.0 24.0" 1.25 0.0 PSF PSF PSF PSF PSF REF DATE FROM SEQN-HC-ENG DRW HCUSR8228 07269021 JRFF-R8228-1TB28778Z01 JB/DF 51561 09/26/07 54791

Scale = .125"/Ft

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubbed Wedge 2x4 SP #3::Rt Stubbed Wedge 2x4

SP

Truss spaced at 24.0" OC designed to support 2 0 0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace all flat TC  $24\,^{\prime\prime}$  OC. **@** 

MARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, and installation of trusses. See "WARNING" note below. shipping

SEE DRW HCUSROO1 02086015 FOR GABLE DETAILS

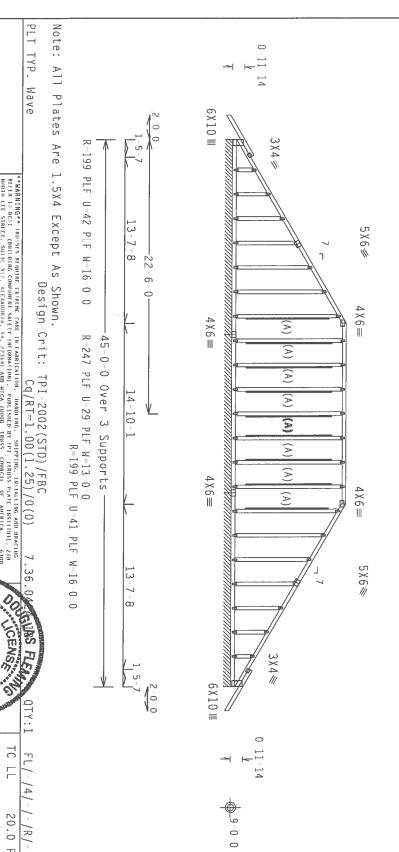
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55 wind

Wind reactions based on MWFRS pressures

with (A) 1x4 #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" 0C.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



PLT TYP.

Wave

Haines City, FL 33844
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ALPINE

\*\*IMPORTANT\*\* LIBRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, THC. SHALL NOT BE RESENOUGHE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE, TO BUILD THE TRUES IN COMPORMANCE WITH IPT. ON FARRESCAIRE, HANDLING, SHIPPIC, HISAALING A BRACLING OF TRUESES.

DESIGN COMPORTS HITH APPLICABLE PROPYSIONS OF THIS CRAFTONIAL DESIGN SPEC. BY ATERA, AND TPI.

ITH BCG. COMMICCIONE PLATES ARE ALOR OF 707/E91/BCGA, PLATESSEY) ASTH A AGAS GRADE 405/BC (M. K.M. SS.) GALV. SIEEL, APPLY PARTS TO TACH FACE OF TRUES AND, UMLES OTHERNISC LOCATED ON THIS DESIGN, POSITION PER BRAHINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER AMERY AS OF TPIL-2007 SCC.3. A STAL ON THIS DESIGN SOLD AND TRUESES OCCUPANCE OF PROPESSIONAL LIGHTEENING RESPONSIBILITY SOLELY FOR THE TRUES COMPONENT OF SHALL SHALL AND STREAM SOLE OF THIS COMPONENT OF SHALL SHA

CORIONAL ENGINEER

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PSF PSF

JB/DF 51520

TOT.LD.

SEQN FROM

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DRW HCUSR8228 HC-ENG

07269022

PSF

DATE REF

09/26/07

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SPACING DUR.FAC.

24.0" 1.25 40.

JRFF-

11828228701

\*\*MARNING\*\* IRUSE'S REQUIRE EXIRTHE CARE IN FARRICATION, MANDELING, SHIPPING, HISTALLING AND BRACING RETER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TIP (TRISS PLATE INSTITUTE, ZIB MORTH LEE SIRTEL, SUITE 312. ALEXANDRAL, VA, 22314) AND HEAT (AUGOD TRUSS COUNCELL OF AMERICA, 6300 ENTERPRISE LAME, MADISON, 41 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITESS DIHERUSE LIAME, MADISON, 41 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNITESS DIHERUSE LIAME, MADISON, 41 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THE SAFETY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PHOPERTY ATTACHED REGISTRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE

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Scale =

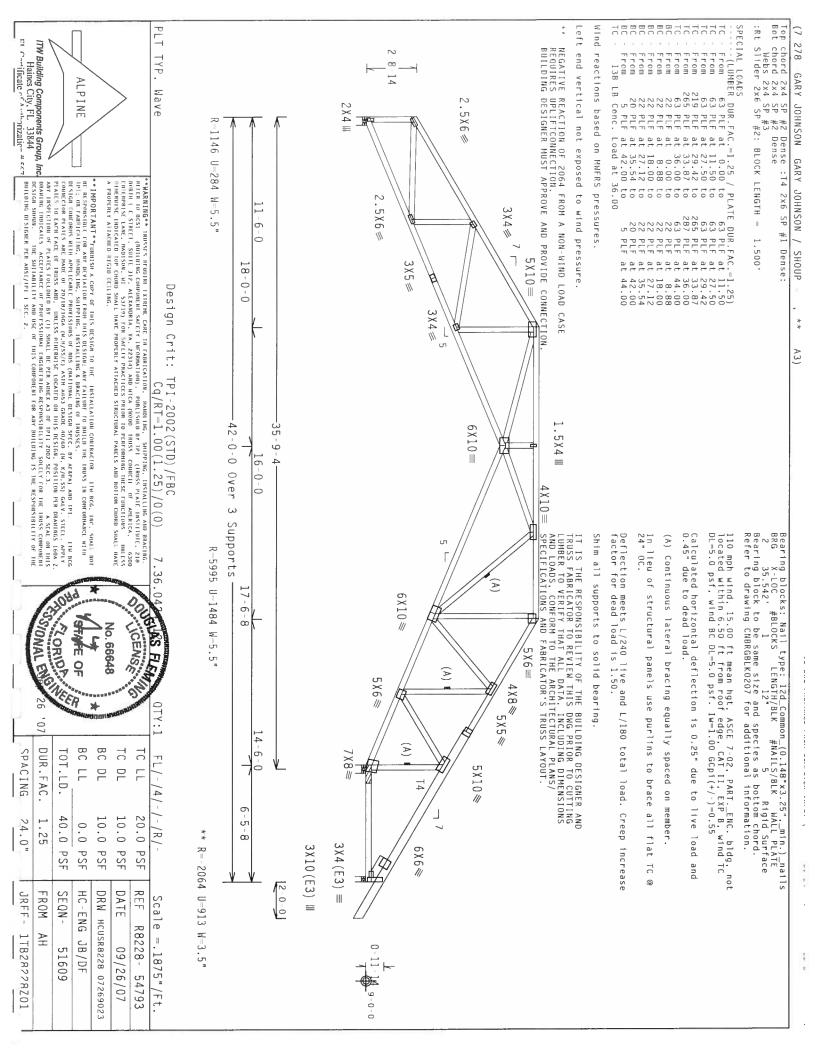
.125"/Ft. 54792

20.0

PSF

R8228-

CENSE No. 66646



(7-278 GARY JOHNSON GARY JOHNSON / SHOUP \* A2)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :W16 2x4

SP #2 Dense:

Left end vertical not exposed to wind pressure

Calculated horizontal deflection is 0.26" due to live load 0.43" due to dead load. and

(A) Continuous lateral bracing equally spaced on member

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

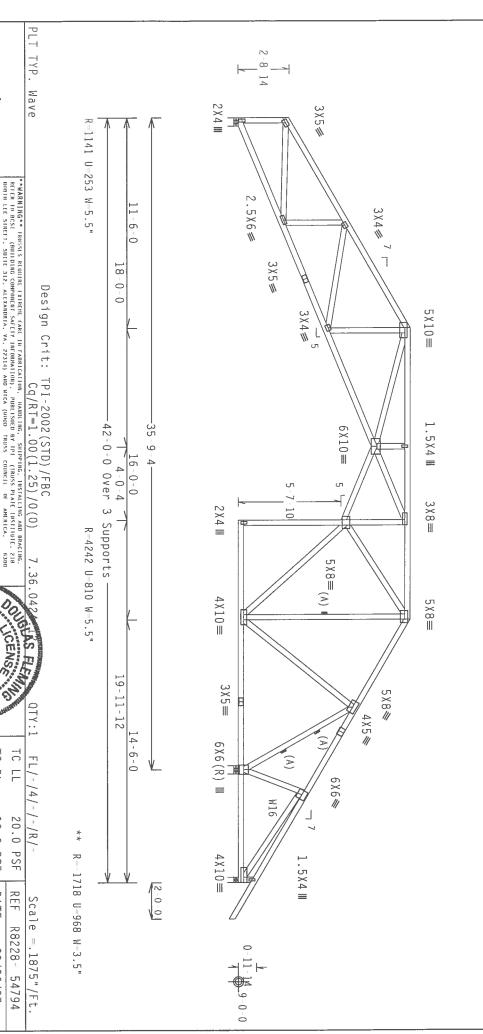
NEGATIVE REACTION OF 1718 FROM A NON-WIND LOAD CASE REQUIRES UPLIFTCONNECTION.
BUILDING DESIGNER MUST APPROVE AND PROVIDE CONNECTION.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, r located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures

In lieu 24" OC. of structural panels use purlins to brace all flat

Shim all supports to solid bearing



L. 1.1

ITW Building Components Group, Inc.
Haines City, FL 33844
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THE BCG SHOWN. THE SELECTION OF PLATES FOLLOWED BY (1) SHALL BE FUR AMEX AND THE SHAMEL FOR THE SHAME SHOWN. STEEL APPLY DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERS HAVE A SHOWN. THE SULFAMELT AND USE OF THIS COMPONENT OF SHOWN. THE SULFAMELT AND USE OF THIS COMPONENT OF SHOWN. THE SULFAMELT AND USE OF THIS COMPONENT OF SHOWN.

\*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE NCG, THE SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THEN DESIGN; ANY FEATURE TO BELLO THE RUSSES IN CONTORNAMICE WITH THE OR TABLECATING, HANDLING, SUIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONTORNS WITH APPLICABLE PROVISIONS OF MOS (MATONAL DESIGN SPEC, BY ALAPA) AND TPI.

114 BCG
CONNECTOR PLATES ARE HADE OF 20/18/16GA (M.H/SS/R) ASTH A653 GRADE 40/60 (M.K.H.SS) GALV. STELL, APPLY
CONNECTOR PLATES ARE HADE OF 20/18/16GA (M.H/SS/R) ASTH A653 GRADE 40/60 (M.K.H.SS) GALV. STELL, APPLY

\*

\*

BC DL TC DL TC LL

10.0 PSF

DRW HCUSR8228 07269006

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DATE

09/26/07

PSF

REF

54794

Scale = .1875"/Ft. R8228-

SIONAL ENGINEE

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DUR.FAC

FROM SEQN-

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51551

SDACING

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JRFF-

1TB28228Z01

\*\*HARNING\*\* INUSKIS REDUIRE IXIBERE CARE IN FABRICATION, HARDLING, SHIPPING, INSTALLING AND BRACING. RELEE TO BEST (BUILDING COMPONICH SAFELY IMPORATION), POBLISHED BY IPT (BUSS PLAFE INSTITUTE, 218 HORTH LEE SHREIT, SUITE 312, ALEXANDRA, VA, 22314) AND MICA (MOND TRUSS COUNCIL OF AMERICA, 6300 ERHERPASE LANE, HADSON, MI \$5718) FOR SAFELY PRACTICES PRIOR TO PEFFORMING HISS FUNCTIONS. UNLESS OTHERWISE HOLDSCALED POR COMES SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

7.36.042

QTY:1

FL/-/4/-/-/R/-

CENSO

Vo. 66648

ALPINE

TYP.

Wave

A4)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Calculated horizontal deflection is 0.11" due to live load 0.17" due to dead load.

(A)  $1\times 4$  #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(C) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" 0C.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

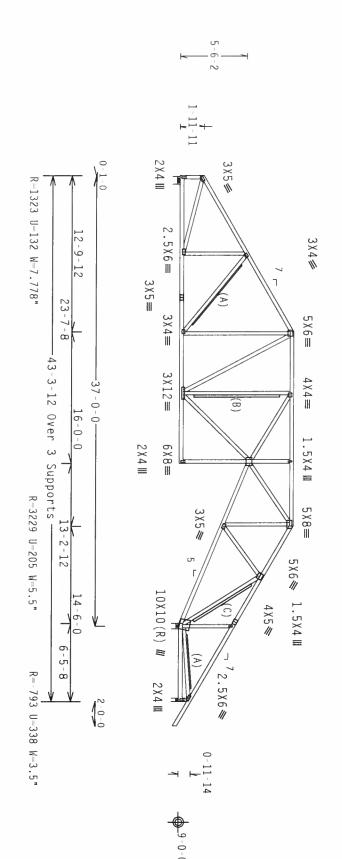
Negative reaction(s) of -792# MAX. (S load case requires uplift connection. -792# MAX. (See below) from a non-wind

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" 0C.

In lieu of structural panels use purlins to brace all flat TC



\*\*HARNING\*\* RRISSES REORIER LETREM "ARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING RETER FOR THE ROLLING COMPONENT SAFETY HERBMANDON, PUBLISHED BY THE (TRUSS PLATE INSTITUTE, ZIB MONTH LE SIREE, SUITE JEZ ALEXANDRIA, WA, ZZZJA) AND NICA (MODO TRUSS COUNCIL DE AMERICA, 6300 LINIERPESE LIAME, HADISON, HI 52/10) FOR SAFETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS, UNILESS OFFICIALIST HOLDINGS SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING. TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

Design Crit:

TYP.

Wave

\*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BEG, THE, SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY BEATLE OF BRILD THE RUSS IN CONTORNANCE WITH THIS DESIGN FARECALTHE, UNBUILD, SHIPPING, INSTALLING A BRACHE OF TRUSSES.

DESIGN CONTORNS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI.

CONNECTOR PLATES ARE HADO OF 20/18/16GA (M. H/SS/K) ASTH A663 GRADE 40/60 (M. K/M.SS) GAVE STEEL, APPLY

PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION OF PLATES FOLLOWED BY (1) SHALL DE PER ANNEX A3 OF TPIL 2002 SEC... THE STATE OF THE S

DRAWING INDICATES ACCEPTANCE OF PROF DESIGN SHOWN. THE SULFABILITY AND BUILDING DESIGNER PER ANSI/TPI I SEC THE SULFABILITY AND USE OF THIS COMPONENT FOR 22 SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT HE IS THE RESPONSIBILITY OF THE

Haines City, FL 33844

ALPINE

CENS No. 66646 107 BC LL BC DL TC DL TC LL SPACING DUR.FAC TOT.LD. FL/-/4/-/-/R/-24.0" 40.0 1.25 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF

PSF

HC-ENG

JB/DF 51636

DRW HCUSR8228 07269007

FROM SEQN-

JRFF-

1TB2R77RZ01

DATE REF

09/26/07

Scale =.125"/Ft.

R8228- 54795

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

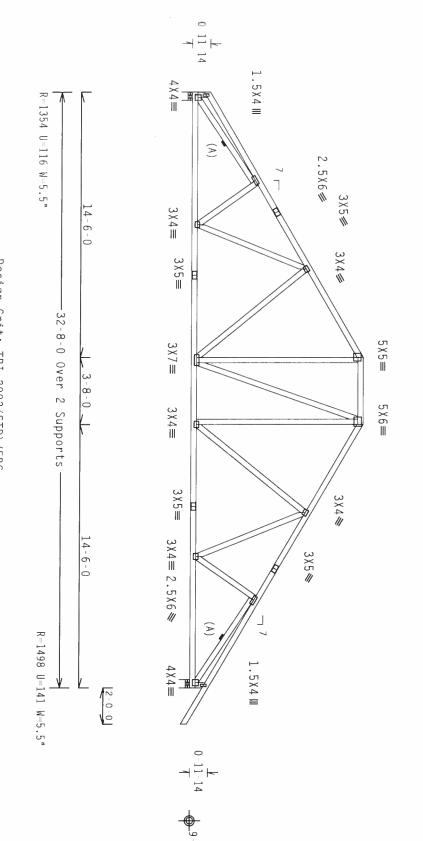
(A) Continuous lateral bracing equally spaced on member.

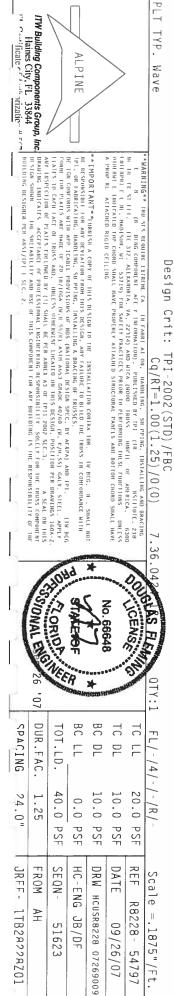
In lieu of structural panels use purlins to brace all flat TC  $24\mbox{\ensuremath{^{"}}}\ 0C.$ 

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





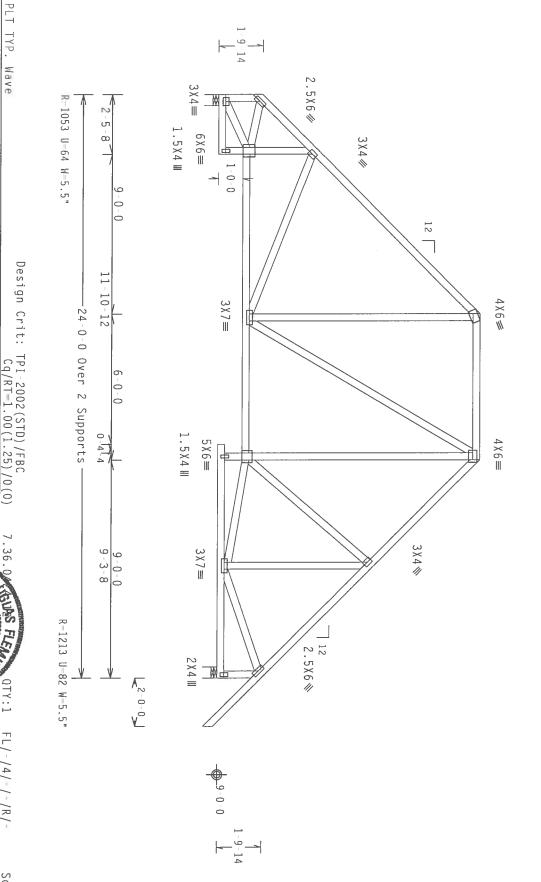
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC  $24\ensuremath{^{\prime\prime}}$  OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



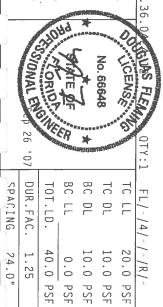
M			
DESTAG CONFORMS WITH APPLICABLE PROVISIONS OF HIDS (MALHOMA) DESTAGA SPEC, BY ARRAYA AND PRI. IT HE BCG COUNTRICTOR PALES ARE OF ZOLVENICA (MELACA) CHARLES OF THE STAGE OF TH	**IMPORTANT**JUBRISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. IT BEG, THE SHALL NOT BE RESONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE HITH TPI: OR FARBLEATING. ANY DEVIATION, STREAM AND THE STREAM AND T	ENTERPRISE CARE, ANDESME, ME 5379) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERMISE, INDICATED TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARTS. AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGED CELLING.	**WARNING** INUSERS NIONIRE CYTERIN CARCINI ABBRICATION, HANDLING, SHIPPING, INSTACLING AND BRACING. NITE TO BEST (RUILDING COMPORTH SACTIY INFORMATION), PUBLISHED BY TEL (RUISE PLAIE INSTITUIE, 218 HANTE TO ESTRETE, SUITE 312, ALTANDRIA, *A. 27314) AND MICK HOOD TRUSS CONNECTE OF ANTERCAS, 6300
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REF

09/26/07

Scale = .25"/Ft. REF R8228- 54798

HC-ENG JB/DF

51327

DRW HCUSR8228 07269001

SEQN-FROM JRFF-

1TB28228Z01

SPACING

24.0"

JREF -

1TB28228Z01

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 :M1, M13 2x4 SP

#2 Dense

110 mph wind, 15.00 ft mean hgt, ASCE 7–02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS All015EE0207 & GBLLETIN0207 for more requirements

(A)  $1x4\ \#3$  or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

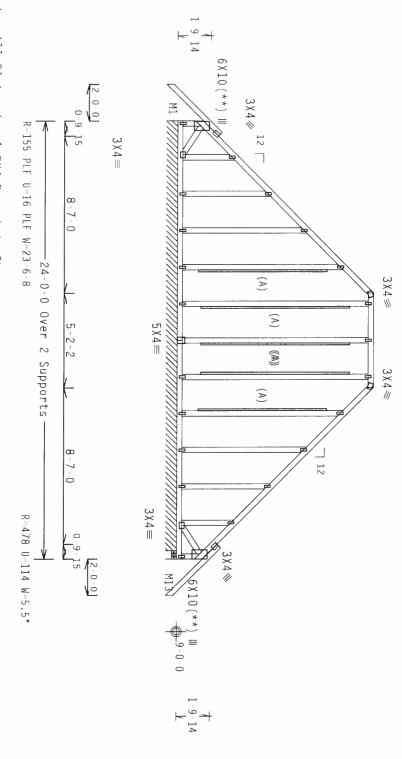
(\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Wind reactions based on MWFRS pressures

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace all flat TC @  $24\mbox{\ensuremath{^{\prime\prime}}}$  OC.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



Note: All Plates Are 1.5X4 Except As Shown. Design Crit:

PLT TYP. Wave

\*\*MARNING\*\* IRUSELS RIQUIRE LITEREE CARE IN FABRICATION. HANDLING. SHIPPIDG. HISJALING AND BRACHIG MUDINI ITE STREET, SUITE 312. ALEXANDRIA, VA. Z2314) AND MICH. (2000) TRUSE COUNCIL OF AMERICA. 6300 LINESPEISE LAIL, HANDSHI, M. 35719) FOR SAFETY PRACTICES PRIOR TO PERFORM HG HESS FUNCTIONS. WHILE SEQUENCES FOR TO PERFORM HG HESS FUNCTIONS. WHILE SEQUENCES FOR TO PERFORM HG HESS FUNCTIONS. WHILE SEQUENCES FOR TO PERFORM HG HESS FUNCTIONS. Cq/RT=1.00(1.25)/0(0) 7.37

TPI-2002 (STD)

\*\*IMPORTANT\*\*\*URBHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR THE BCG, INC. SHALL NOT BE RYSOMSTBLE FOR ANY OUVALION FROM HILS DESIGN, ANY TAILURE (D BULLD HE RNSS) IN COMPORNANCE WITH THE DESIGN CONTROLLING, NUMBEL BOWN LING, LINGALLING & BRACING OF HRUSSES BOSALE AND THE BCG COUNCE ON THE ARRICAL REAL AND TOT 20/14/1664 (H.H/SS) RAW AND DESIGN SULCE ON THE SECOND OF THE SECOND COUNCELOR PLATES AND THAT OF THE SECOND COUNCES WITH APPLICABLE PROVISIONS OF HIS (MATHONAL DESIGN SULCE, BY AFAPA) AND THE BCG COUNCELOR PLATES AND THE ARRIVAL SULCESS OF THE BCALLING AND THE SECOND COUNCES WITH A SECOND

BUILDING DESIGNER PER AUSI/IPI 1 SEC. 2

ITW Building Components Group, Inc. Haines City, FL 33844 Ft Conficate of Authorization with Confication of the Confication of

ALPINE

SPACING 24.0" JRFF-FROM 1TB28778Z01

GOOGLUAS FLE CENS No. 66648 07 BC LL DUR.FAC. BC DL TC DL TC LL FOT.LD. FL/-/4/-/-/R/-40.0 1.25 10.0 PSF 10.0 PSF 20.0 0.0 PSF PSF

HC-ENG JB/DF

SEQN-

2467

DRW HCUSR8228 07269028

DATE REF

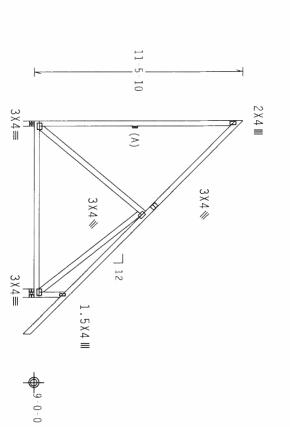
09/26/07

54800

Scale =.1875"/Ft. R8228-

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Left end vertical not exposed to wind pressure (A) Continuous lateral bracing equally spaced on member. (7 278 GARY JOHNSON GARY JOHNSON / SHOUP \*\* (6) 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18 Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=410 U=169 W=3.5" ▲ 9-7-12 Over 2 Supports R=589 W=5.5" <u>V</u> 2-0-0

\*\*WARNING\*\* RHASSIS RIGHER CYRRHE FARE IN FARRICATION, HARRING, SHIPPING, INSTALLING AND BRACHE RETHE TO BCSI. (BUILDING COMPONENT SALETY HUROHANITON), POBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LLISTREE, SHITE 312. ALEXANDRIA, VAA, ZZZIA) AND HITA (MODD. TRUSS COUNCIL OF AMERICA. GOOD LIHIERRISC LAME, HADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HERSE FUNCTIONS. DHICESS OTHERWIST HOLICATED HOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING. 

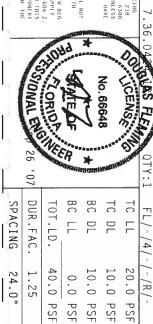
TYP.

Wave

\*\* IMPORTANT\*\* "numish a copy of this design; and the installation contractor. It were its shall not be accounted to the contractor. The same properties of the contractor of the contractor in the contractor of the contractor of

ALPINE

Haines City, FL 33844 DESIGNED DESIGNED DESIGNED FOR ARST/TP 1 STC. DESIGN COMPORMS WITH APPLICABLE PROVISIONS OF HDS (MATURAL DESIGN SPEC, BY AFRY) AND IPI. I'H BCG CONNECTOR PLATES ARE HADE OF ZO/IB/166A (R.H./SSK) ARAN A653 GRADE 40/60 (R. Z/M.SS) GALV. SIEEL. APPLY PLATES ID EACH FACE OF TRISS AND. UNICESS OTHERNISE (DCATED ON THIS DESIGN, POSITION PER DRAWHROS 160A Z. ANY INSPECTION OF TRAITS FOLLOWED BY (I) SMALL BE FER ANNEX AS OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWHROE OF PROFESSIONAL INCHEMENT AND THE SOLITY FOR THE HOSS COMPONENT DESIGN SHOWN. IN ESTIMATION OF THE STORMS AND ADDITIONAL SHOWN. THE SULFABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE



REF

Scale =.1875"/Ft. R8228- 54802

DATE

09/26/07

HC-ENG

JB/DF 51373

DRW HCUSR8228 07269012

JREF-

1TB28228Z01

FROM SEQN-

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP \* C7)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

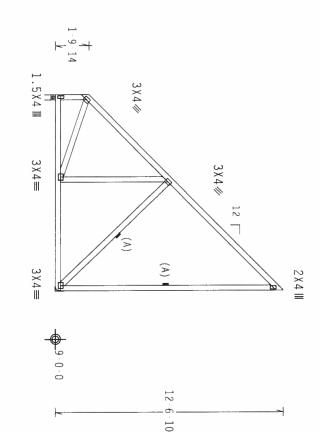
 $\geq$ Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.19 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.



▲10-8-12 Over 2 Supports >

R=474 W=3.5" Design Crit: R=474 U=199 TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

Haines City, FL 33844 TYP. ALPINE Wave

\*\*HARNING\*\* TRUSSES BEON BE CEMENT CARE IN FABRICATION, IMMOULDE, SHIPPING, HSTAI ING AND BRACHIG RETER TO BEST (MUTLING COMPORTER SALLEY INFORMATION), PUBLISHED BY PI (TRUSS PLATE INSTITUE, 218 INBIT LET SHEET, SUITE 312, ALEXANDRIA, VA. 22314) AND WICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ER LERGYSEL LANE, MADISON, WI 53719) FOR SALETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNILESS OTHERWISE INDIVIALED FOR COMED SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

\*\*IMPORTANT\*\*THRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG. THC. SMALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM HIS DESIGN. ANY FAILURE TO BRILLD THE TRESS IN COMMONMENT OF THE PLANT THE PROPERTY OF THE PROPERTY O



SPA	LEN 26 '07 DUR	DRION INC.	OF BC LL	0	TC DL
SPACING	DUR.FAC.	TOT.LD.	F	DL	DL
24.0"	1.25	40.0 PSF	0.0 PSF	10.0 PSF	10.0 PSF
JRFF 1TB28728Z01	FROM AH	SEQN- 51393	HC-ENG JB/DF	DRW HCUSR8228 07269013	DATE 09/26/07

20.0 PSF

REF

Scale = .1875"/Ft. R8228- 54803

COMPLETE

TRUSSES

REQUIRED

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 16.19 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.25 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures

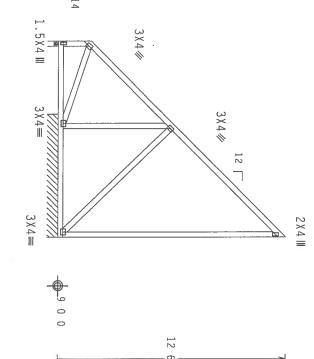
Deflection factor for meets L/240 live and L/180 total load. Creep increase dead load is 1.50.

t o lieu of structural panels or rigid ceiling use purlins brace TC @ 24" OC, BC @ 24" OC.

# Nailing Schedule: (12d Common\_(0.148"x3.25",\_min.)\_nails) Top Chord: 1 Row @12.00" o.c. Bot Chord: 1 Row @12.00" o.c. Webs : 1 Row @ 4" o.c. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 34.5" OC maximum



10

R=352 W=3.5" R=150 PLF U=46 ▲10-8-12 Over 2 Supports > PLF M=6ω

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

PLT

TYP.

Wave

\*\*MARNING\*\* IRUSSES BEQUIRE CEREME CARE UN FAMBICATION, IMABLIAND, SHIPPING, HISTAILING AND BRACHIG. REFER TO BOST (RRUSS PLATE HISTITUTE, ZUB HUNDIH LEE STREE, SUITE 312, ALEXANDRIA, VA, ZZ313) AND HICA (ROOD TRUSS COUNCIL OF AVERICA, 6300 ERITERPENSE LANE, HADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING HUESE FUNCTIONS. UNLESS OTHERWISE HOLDSCALED FOR PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

7.36.042 \* ON LICENS SONAL SIGNAL No.66648 \* 07 BC DL BC LL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-40.0 10.0 20.0 10.0 PSF 0.0

PSF

HC-ENG

JB/DF 51399

DRW HCUSR8228 07269014

PSF

SEQN-

PSF PSF

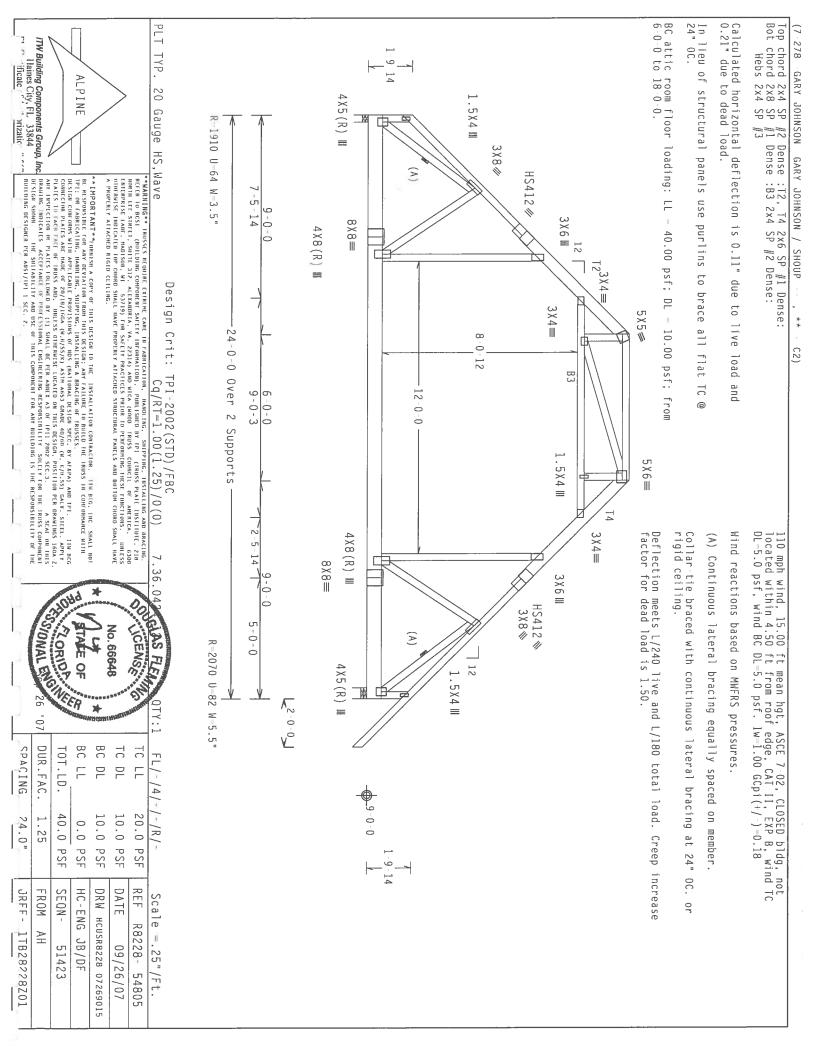
DATE

09/26/07

Scale = .1875"/Ft. R8228= 54804

34.5" 1.25 JREF-FROM

1TB28228701



rizatio

SPACING

24.0"

JRFF-

1TB28228Z01

The building designer is responsible for the design of the roof and celling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer. Top chord 2x4 SP #2 Dense :T3, T5 2x6 SP #1 Dense: Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense: Webs 2x4 SP #3 Note: All Plates Are 1.5X4 Except As Shown See DWGS Al1015EC0207 & GBLLETIN0207 for more requirements must not be cut or notched. Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord Wind reactions based on MWFRS pressures PLT TYP. rigid ceiling. Collar-tie braced with continuous lateral bracing at 24" OC. Haines City, FL 33844
FI Carificate City, FL 37844 1-9-14 ALPINE Wave L2-0-0 3 X 4 ≡ 6X10(\*\*) Ⅲ 0 3X4 / 9 =261 \*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BUT RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY FALLURE TO BHILD THE TRUSS IN COMPORNANCE WITH FILE OR FARRENCHING, NAMOLIGE, SHIPPING, HISTALLING & BRACHING OF TRUSSES.

DESIGN COMPORIS WITH APPLICABLE PROPYSIONS OF HIS (NATIONAL DISIGN SPEC, BY ATAPA) AND FPI. HIS BCG COMMICTION PLAIRS ARE HADE OF ZO/149186A, BH-14552, KNATIONAL DISIGN SPICE, BY ATAPA) AND FPI. HIS BCG COMMICTION PLAIRS ARE HADE OF ZO/149186A, BH-14552, AND HIS DESIGN FOSTION FROM BRAHINGS 160A Z.

PLAIRS TO EACH FACE OF TRUSS AND. HINESS OTHERWISE LOCATED ON HIS DESIGN, POSITION FROM BRAHINGS 160A Z.

ANY HISSECTION OF PLAIRS TOLLOWED BY (1) SHALL BE FER AIMER AS OF FPI. ZOOZ SEC.3.

AS STANDARD OF PLAIRS OF THE TRUSS COMPONENT FOR ANY BRILLIANG IS HIS PROPRISED. BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2. DESIGN SHOWN. I \*\*WARNING\*\* HOUSES, BEQUIRE CYRENE CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, RELEA TO BEST (BRISS PLATE INSTITUTE, 218 MORTH OF BEST CHAIS SPLATE INSTITUTE, 218 MORTH OLE SIREE, SUITE 312. ALEXANDRIA, VA. 22314) AND HICA (HOOD TRUSS COUNCIL OF AMERICA, 6300 CHILEBRISE LANE, MADISON, BI 53719) FOR SAFELY PRACTICES PRIDE TO PERFORMING HIESE FUNCTIONS. AND HICA (HOOD SINCE INVESTMENTS IN THE STATE OF THE STALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE 12 PLF 3 X 4 ∥ U=3 PLF THE SUITABILITY AND USE OF THIS COMPONENT FOR -10-14 **€**8×8 ₽=5 8-7-0 4X416T3 Design Crit: 12-0-0 -0-0 =229 3X4/ 3 X 4 ≡ PLF U=10 PLF W=14-0-0 TPI-2002(STD) (A)8-€X6= 24-0-0 Cq/RT=1.00(1.25)/0(0) 3 X 4 Ⅲ 0 -**1** 2(A) œ В 0ver 5-2-2 8-2-5 + DZ SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT WG IS THE RESPONSIBILITY OF THE ω (A) Supports 3 X 4 III 5X8 + MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS. 3 X 4 ≡  $\widehat{\mathbb{A}}$ BC attic room floor loading: LL = 40.00 psf; DL  $6\cdot 0\text{-}0$  to  $18\cdot 0\text{-}0$  . 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. (\*\*) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. In lieu of structural panels use purlins to brace all flat TC 24" OC. (A) Continuous lateral 3X4// 2-10-14 SOURCE FLE =259 3 X 4 ≡ 6 X 6 ≡ 4X4// STONAL ENGINEE PLF CICENS No. 66648 <u>U</u>=3 3 X 4 // bracing equally spaced on PLF 5-0-0 M=5-0-0 QTY:1 6X10(\*\*) 12 3 X 4 // 07 -9-15 BC DL DUR.FAC. BC LL TC DL SPACING TC LL TOT.LD. K2-0-0V FL/-/4/-/-/R/-3 X 4≡ member 10.00 psf; from 40.0 10.0 10.0 24.0" 1.25 20.0 0.0 PSF PSF PSF PSF PSF FROM DATE REF SEQN-HC-ENG DRW HCUSR8228 JRFF-Scale = R8228-1TB28228Z01 JB/DF 09/26/07 .25"/ft. 54807 07269029 REV

l op Bot p chord 2x4 SP #2 I t chord 2x6 SP #2 Webs 2x4 SP #3 Dense

SPECIAL LOADS --(LUMBER DUR.FAC.=1.25) PLATE DUR.FAC.=1.25) From 60 PLF at 0.00 to 60 PLF at 9.63 From 20 PLF at 0.00 to 20 PLF at 9.63 474 LB Conc. Load at 1.92, 3.92, 5.92, at 9.63

7.92

Wind reactions based on MWFRS pressures

End verticals not exposed to wind pressure

Truss must be installed as shown with top chord up

# COMPLETE TRUSSES REQUIRED

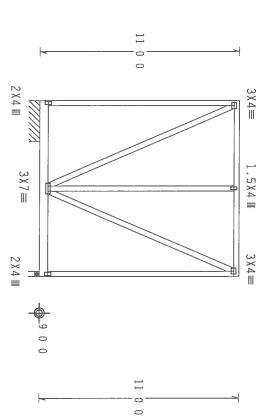
הממווז וירם מו וואספס ווו אי

Nailing Schedule: (12d\_Common\_(0.148"x3.25",\_min.)\_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 9.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

110 mph wind, 20.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.25 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\cdot$ 

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.





Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

P TYP. Wave

\*\*MARNING\*\* HOUSEK, REQUIRE CERTREME CARE IN FABRICATION, HANDING, SHIPPING, HISTAILING AND BRACHG.
RETER TO BEST (BUILDING COMPONENT SAFETY HATOBRATION), PUBLISHED BY TET (THUSE PLATE HISTITUIT, 218
HOWLIN LEE SHREE, SUITE 312, ALEXANDRIA, WA, 22314) AND MICA (MODD TRUSS COUNCIL OF MICHICA, 6300
THIERPRIST LANL, MAUSON, HI \$5218) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS
OTHERMIST HOUSENING TO GROUND SMALL HAME PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTON CHORD SMALL HAME
A PROPERLY ATTACHED REGID CELLING.

7.36. GOUGUES FLE CENS No. 66648 107 BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-10.0 24.0" 1.25 40.0 20.0

10.0 PSF

DRW HCUSR8228 07269024

0.0

PSF

HC-ENG

JB/DF 51410

PSF

SEQN-

FROM

JRFF-

11828228201

-/R/-

Scale =.1875"/Ft.

R8228- 54808

PSF

PSF

DATE REF

09/26/07

\*\*IMPORTANT\*\*\*URBLES A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY TATURES TO BUILD THE TRUSS IN CHAPMENDER WITH PET ON FARRESTING. ANALUGE, SHEPTHOL, INSTALLING A BRACHING OF TRUSSES, WARRAN, AND TELL PROVISIONS OF THIS SCHAPTHOL, STANLING AND THE STREET OF THE STANLING AND THE STREET OF THE ACCOUNT OF THIS DESIGN. SHEET, APPLY THAT IS TO LACH FACE OF TRUSS. AND. DRIES OF THE SHEET OF THIS DESIGN. POSITION FOR DIAMINGS LOCAL ON THIS DESIGN OF PARTES OF THE PROPESS OF THE SHEET OF THE STREET OF THE STANLING SHOWS OF THE STA

Haines City, FL 33844

Fit Conficate Contact Annual City of the State of the State

"orizatic" " era

ALPINE

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched. Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense: Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie-plates 24" o.c. Center plate on stacked/dropped chord. Note: All Plates Are 1.5X4 Except As Shown. PLT TYP. designed by the building The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous chord in notchable area using 3x6. interface, plate length perpendicular to chord length. Splice top lateral restraint to the gable end. All connections to be designed by the building designer. Haines City, FL 33844
Haines City, FL 31844
Haines City and Anticality of the Company of the Com 278 GARY JOHNSON GARY JOHNSON / SHOUP ALPINE Wave -2-0-0-R=170 PLF U=48 PLF \*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE HISTALLATION CONTRACTOR. THE NGG, THE SHALL NOT BE RESPONDED FOR ANY DEVIATION FROM THIS DESIGN. ANY TAILURE TO UNITED THE TRUSS IN COMPTONHANCE WITH PIECES FOR THE SHALL HE ADDRESS OF THE ADDRESS DRAWING INDICATES ACCEPTANCE OF PROFESS DESIGN SHOWN. THE SUITABILITY AND USL BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. \*\*WARNING\*\* IRUSSES REQUIRE EXTREPL CARE IN LARRICATION. DANDLING, SHIPPTING, 1951ALLING AND BRACHG. RELEAS OF CHILDING COMPORDER SAFETY IMPORATION, PUBLISHED BY FET (TRUSS PLATE INSTITUTE, 210 month LEE SHEFT, SHIFE 315, ALEXANDRAL, NA, 27313) AND MICA (MODO) TRUSS COUNCIL OF MICHICA. GOOD FRIENDRALE HAVE AND SON, MI 53/19) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. MILESS OFHIRMALE HOPECHANALISTO FOR FORMS SMALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD STALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD STALL HAVE 4-0-0 2X4 (C5)  $2.5 \times 6 (C5) =$ THE SUITABILITY AND USE OF THIS 0-0 (NNL) ₩=14-5 Design Crit: M GE) TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /0(0) 3X4≡ ф SOLELY FOR THE TRUSS COMPONENT 14-5-8 w 0ver  $\bigoplus$ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55 Right end vertical not exposed to wind pressure. Wind reactions based on MWFRS pressures SEE DRW HCUSROO1 02086015 FOR GABLE DETAILS Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. In lieu of structural panels use purlins to brace Continuous 7.36.04 12-5-SOUCENSE Support  $\Rightarrow$  $\Phi$ io. 66648 ģ  $\Box$ BC DL DUR.FAC. BC LL TC DL TC LL TOT.LD. FL/-/4/-/-/R/-10.0 40.0 20.0 ф 1.25 10.0 PSF 0.0 TC @ PSF PSF PSF PSF 24" REF SEQN-FROM DRW HCUSR8228 07269005 DATE HC-ENG Scale =.5"/Ft. wind  $\Phi$  $\Rightarrow$ R8228-JB/DF 09/26/07 51454 54809 9-0-0

SPACING

24.0"

JRFF-

1TB28778Z01

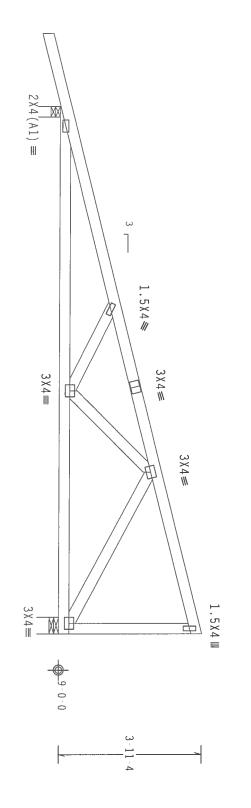
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART.\_ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures

Right end vertical not exposed to wind pressure.



R=729 U=164 W=3.5"

14-5-8

0ver

2 Supports

**←2**-0-0->

R=566 U=148 W=5.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

PLT

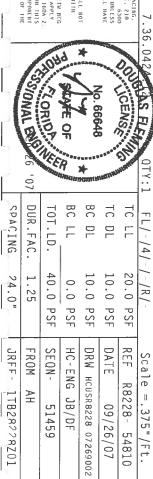
TYP.

Wave

\*\*IMPORTANT\*\*FURBLES A CEPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEGG, THE, SMALL NOT BE RESPONDED IN THE TRUE OF THE TRUE O BUILDING DESIGNER PER AUSI/IPI 1 SEC. 2. SUITABILITY AND USE OF THIS COMP RESPONSIBILITY OF THE

Haines City, FL 33844

ALPINE



JB/DF 51459

1TB28228Z01

Scale = .375"/Ft. R8228 54810

09/26/07

24.0"

JRFF-

1TB28228Z01

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP PB4)

> IT AS ONG THE MICE ואוי האוו חודע זעו מו לרמעתם מ מזנורעוסזמעם למחנודוורה מו נעמסים ווועי

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 21.32 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,.$ 

 SPECIAL LOADS

 ------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

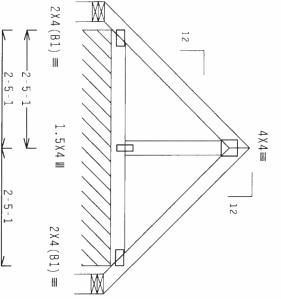
 TC - From 68 PLF at 0.00 to 68 PLF at 3.00

 TC - From 68 PLF at 3.00 to 68 PLF at 6.00

 RC - From 4 PLF at 0.00 to 4 PLF at 6.00

Wind reactions based on MWFRS pressures.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.



13 U=80 ₩=4.95" =89 PLF U=30 PLF W=4-10-2 6-0-0 Over 3 Supports 13 U=21 W=4.95"

Design Crit: TPI-2002(STD) Cq/RT=1.00(1.25)/0(0)

PLT TYP.

Wave

\*\*MARNING\*\* IGUSCES BEQUIRE TYREME CARE IN FAMRICATION. HANDING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BULLDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IT! (1805S PLATE INSTITUIT, 218 HOUTH LEE SHREET, SUITE 312, ALEXANDRIA, VA, 22314) AND BECA (MODD TRUSS COUNCIL OF AUTRICA, 6300 ELHERPRISE LAME, MADSON, HI 55719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. DHEESS OFHERHYSE INDICATED BY CHORD SHALL HAME PROPERLY ATTACHED STRUCTURAL PAURLS AND BOTTON CHORD SHALL HAME

DRAWING INDICATES ACCEPTANCE OF PROFESS OF SIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/IPI 1 SEC. 2.

Haines City, FL 33844
Fit Contificate of A anti-prization # con

ALPINE

7.37.05 OS LICENSE No. 66648 QTY:1 07 BC DL BC LL SPACING DUR.FAC. TC DL TC LL TOT.LD. FL/-/4/-/-/R/-40.0 24.0" 1.25 10.0 PSF 10.0 PSF 20.0 0.0

PSF PSF

HC-ENG

JB/DF 2442

SEQN-

DRW HCUSR8228 07269017

JRFF-FROM

1TB28778Z01

PSF

Scale =.5"/Ft.

R8228- 54813

DATE REF

09/26/07

PB5)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

110 mph wind, 21.12 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 GCpi (+/-)=0.18

Wind reactions based on MWFRS pressures

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

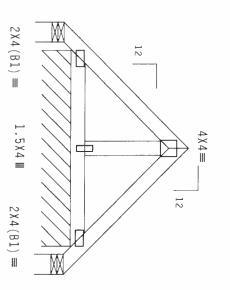
SPECIAL LOADS
\_----(LUMBER DUR.FAC.=1.25 / From From 68 PLF at 0.00 to 68 PLF at 2.59 to 4 PLF at 0.00 to / PLATE DUR.FAC.=1.25)
to 68 PLF at 2.59
to 68 PLF at 5.18
to 4 PLF at 5.18

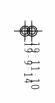
Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

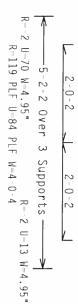
In lieu of rigid ceiling use purlins to brace BC @

Refer to DWG PIGBACKAO2O7 or PIGBACKBO2O7 for piggyback details.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.







-2 - 0 - 2

Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

FL/-/4/-/-/R/-

20.0

PSF

REF

54814

Scale =.5"/Ft. R8228-

TYP.

Wave

\*\*WARNING\*\* IRNSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RELER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY IPI (TRUSS PLAIE INSTITUTE, 218 HORIN LEE STREET, SUID 317. ALEXANDRIA, VA. 22-214) AND WITCA (MODD TRUSS COUNTEL OF ANHREA. 6300 ENURS PLAIE, SHADISON, H. 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THEST FUNCTIONS. UNILESS OTHERNISE LINDLEAGLED FOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION CHILD.

ALPINE

Haines City, FL 33844 \*\* IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE DEG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY JALIURE TO BUILD THE TRUSS IN COMPORMANCE WITH THE TO FARRICATHO. HANDLING. SHEPPING, INSTALLING A BRACHING OF TRUSSES.

PESIGN COMMENS WITH APPLICABLE PROVISIONS OF DROS (MATIONAL DESIGNE SPEC, BY ATARA) AND TPI CONTROLLED PROVISIONS OF DROS (MATIONAL DESIGNE SPEC, BY ATARA) AND TPI CONTROLLED PROVISIONS OF DROS (MATIONAL DESIGNE SPEC, BY ATARA) AND TPI CONTROLLED AND TRUSS OF THE SPECIAL PROVISION FOR BRAINED OF THE SPECIAL PROVISION FOR THE SPECIAL PROVISION FOR THE SPECIAL DROST OF THIS DESIGN. POSITION FOR DRAWHINGS 160A Z. ANY INSPECTION OF PLATES OLICIABLE BY () SHALL BE PER AMERZ AS OF TPI 2002 SEC 3. A SLA. ON THIS DESIGN SHOULDED OF THE SPECIAL PROPERTY OF THE SPECIAL PROPERTY OF THE SPECIAL PROPERTY OF THE SPECIAL SHOWN.

BRAINED INSTALLS ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPORED THE SPECIAL SHOWN.

DESIGN SHOWN IN SUITABLE PER AMERZ AS OF THE SPECIAL PROPERTY OF THE SPECIAL SHOWN.

21 2

COUNTY FLA 과 SSIDNAL ENGINEE No. 66648 07 BC LL BC DL DUR.FAC. TC OL TOT.LD.

SPACING 40.0 24.0" 10.0 PSF 10.0 PSF 0.0 PSF PSF DATE JRFF-HC-ENG DRW HCUSR8228 07269025 1TB2R22RZ01 JB/DF 51352 09/26/07

(7-278 GARY JOHNSON GARY JOHNSON / SHOUP PB3)

Top chord 2x4 SP Bot chord 2x4 SP #2 Dense

110 mph wind, 18.98 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/ )=0.18

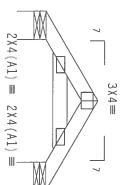
Wind reactions based on MWFRS pressures

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.

SPECIAL LOADS From (LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
rom 63 PLF at 0.00 to 63 PLF at 1.83
rom 63 PLF at 1.83 to 63 PLF at 3.67
rom 4 PLF at 0.00 to 4 PLF at 3.67

In lieu of rigid ceiling use purlins to brace BC @ 24"

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





R-104 U-30 W-6.946" ▲3-8-0 Over 2 Supports > R-104 U=30 W=6.946"

Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

PLT

TYP.

Wave

WHARNING\*\* REQUEST REQUEST CATREME CARE IN FARRICATION, IMMRILIAN EN PIPPING, INSTALLING AND BRACING.
RETER TO BESS. BUILDING COMPOUNDERS ACTEVE HOWARDLEN, PRINCIPED BY RE PHYLICE INSTITUTE. ALL MARKET.
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\*\*IMPORTANT\*\*TURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEPLACEMENT BOOK AND THE SELECTION FROM THE SELECTION FOR THE SELECTION FROM THE SELECTION FOR THE SELECTI

Haines City, FL 33844

ALPINE

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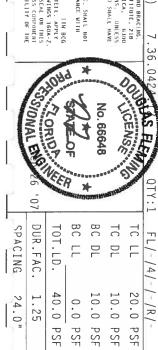
SEQN-

HC-ENG

JB/DF 51490

DRW HCUSR8228 07269018

FROM



DATE

09/26/07

REF

R8228- 54815

Scale =.5"/Ft.

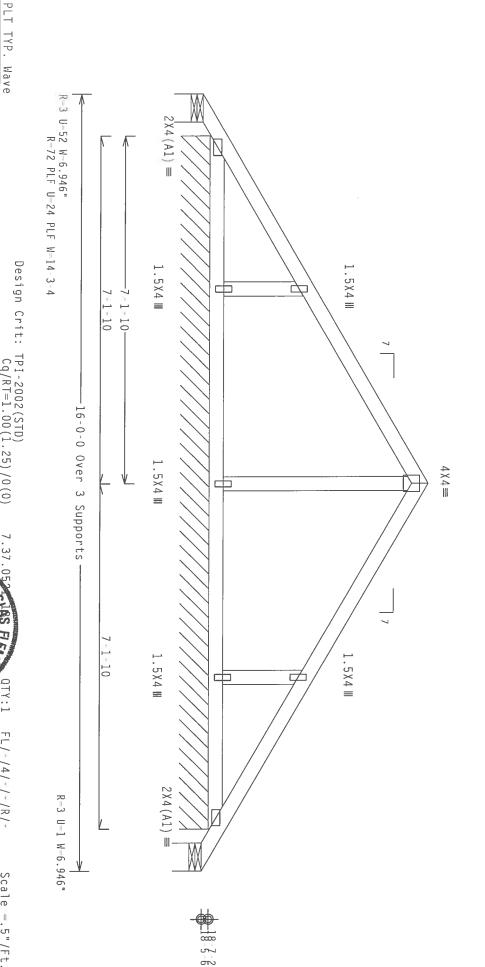
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 110 mph wind, 20.78 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is  $1.50\,\mathrm{.}$ 

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details

SPECIAL LOADS ----- (LUMBER I TC - From 6: TC - From 6: 63 PLF at 63 PLF at 4 PLF at DUR.FAC.=1.25 / 0.00 / PLATE DUR.FAC.=1.25)
) to 63 PLF at 8.00
) to 63 PLF at 16.00
) to 4 PLF at 16.00 to

Wind reactions based on MWFRS pressures



ALPINE

\*\*MARNING\*\* INUSES REQUIRE CARE IN FABRICATION, IMMOLING, SHIPPING, INSTALLING AND BRACING, RETER DO SEST. (BUILDING COMPONENT SACETY INCOMENTARY), PUBLISHED BY THE CHRUSS PLANE INSTITUTE, 718 MORTH ELE SHREEL, SUITE IZE, ALEXANDRIA, VA, 2221A) AND HEACA (MOOD TRUSS COUNCIL OF AMERICA. 6300 CHICEMENSE LANE, MADISON, H. 53719) FOR SACETY PRACTICES PRIOR TO PERFORMING INESE FUNCTIONS. DULESS OTHERWISE, INDICATED FOR CHROST SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED REGION STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

Cq/RT=1.00(1.25)/0(0)

7.37.05

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FL/-/4/-/-/R/-

Scale = .5"/Ft.

R8228- 54816

20.0 10.0 PSF

PSF

DATE REF

09/26/07

SOUCENSE

No. 66648

BC LL

0.0 PSF

HC-ENG JB/DF

DRW HCUSR8228 07269019

TOT.LD.

40.0

PSF

SEQN-

2447

REV

BC DL TC DL TC LL

10.0 PSF

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BLE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE FROMS IN COMPORMANCE WITH IP: OR FARREACHING, HANDLING, SHIPPIDE, HISTALLING A BRACILLO OF TRUSSES, DESIGN. ANTERNALLING, SHAPPIDE, THE SHALLING A BRACILLO OF TRUSSES, WATERNA, AND IP!. IH BCG CONNECTOR PLATES ARE HADE OF ZO/IN/16/66 (H.1/55/R). ASIH A653 GRADE 40/60 (H. K/M.55) GALV. SIETEL APPLY PLATES TO LACH FACE OF TRUSS AND. BURLESS OHIERISE LOCATED ON HIS DESIGN. POSITION PER BRAHINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FOR NAMEX AS OF THI 2002 SEC. 3. A SEAL ON HIS ENGALD. BURLES AND ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FOR NAMEX AS OF THI 2002 SEC. 3. A SEAL ON HIS ENGALD. BURLES AND AND ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FOR NAMEX AS OF THI 2002 SEC. 3. A SEAL ON HIS ENGALD. BURLES AND ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FOR NAMEX AS OF THIS CONFORMER AND ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FOR NAMEX AS OF THIS CONFORMER AND ANY INSPECTION OF THE THESE COMPORER IN THE PLATES FOR THE THESE FOR THE THESE

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Haines City, FL 33844 ITW Building Components Group, Inc.

STONAL ENGINEE 07 DUR.FAC. SPACING 24.0" 1.25 JRFF-FROM 1TB2R22RZ01

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 110 mph wind, 20.61 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. lw=1.00 GCpi(+/ )=0.18

See DWGS All030EE0207 & GBLLETIN0207 for more requirements

Wind reactions based on MWFRS pressures

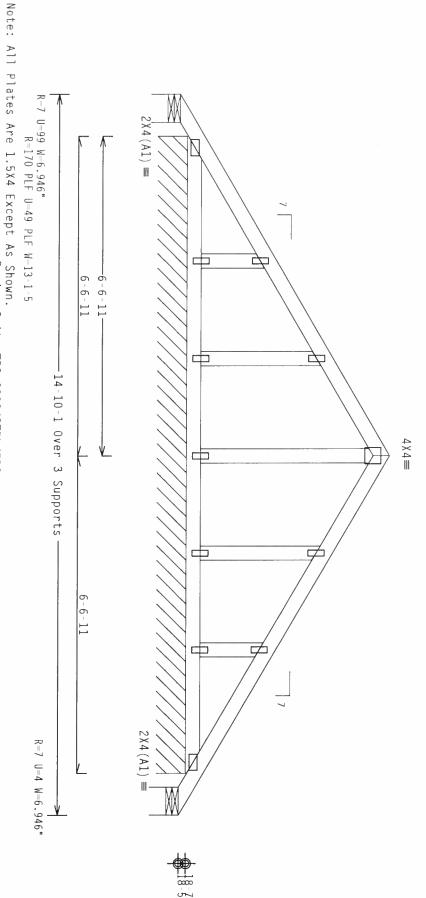
Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.

SPECIAL LOADS -- (LUMBER DUR.FAC.=1.25 / From 63 PLF at 0.00 1 From 63 PLF at 7.42 From 4 PLF at 0.00 0.00 to 7.42 to 0.00 to PLATE TE DUR.FAC.=1.25)
63 PLF at 7.42
63 PLF at 14.84
4 PLF at 14.84

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of rigid ceiling use purlins to brace BC @ 24"

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

7.36.

FL/-/4/-/-/R/-

Scale =.5"/Ft.

R8228- 54817

DATE REF

09/26/07

HC-ENG

JB/DF

51507

DRW HCUSR8228 07269026

\*\*WARRING\*\* IRUSSIS REQUIRE EXTREME CARL IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, BYLER TO BEST (BUILDING CHOPOREMI SAFETY INFORMATION), PUBLISHED BY FPI (TRUSS PLATE INSTITUE, 218 MORTH LET STREET, SUITE 377 ALEXANDRIA, NA, 22314) AND HEAT ACK (MOOD TRUSS COUNCEL OF AMERICA, 6300 ENTERPOIS LAWE, MADISON, 41 53719) FOR SAFETY PRACTICES PHIOR TO PERFORMING THESE FUNCTIONS. UNLESS OHIGHNIST INDICATED THE CHARD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANIELS AND BOTTOM CHORD SMALL HAVE A PROPERLY ATTACHED RIGHT CHIRDS SMALL HAVE

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE DCG. THC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN CONTORNANCE WITH FIT OR FARREACHING, HANDLURG, SHEPPIG, HISTALLIGA BRACHING OF TRUSSES. BY AFRAYA, AND FIT.

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HALES TO EACH FACE OF TRUSS AND. UNICESS OTHERMISE LOCAFFD ON THIS DESIGN, POSITION PER DRAWHINGS 160A Z.

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ANY INSPECTION OF TRAITS FOLLOWID BY (I) SHALL BE FIR ANIEX 30 FIFLI 2002 SEC. 3.

ORACHING INDICALES ACCEPTANCE OF PROFESSIONAL ENGLITER BIOR EXPONSIBILITY OF THE DESIGN SHOWN.

HE SULFAMILIES TO CONFORMER THE SECONDARY BUILDING IS THE RESPONSIBILITY OF THE

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Haines City, FL 33844

ALPINE

SOUNCENS, No. 66648 707 DUR.FAC. BC LL BC DL TC DL TC LL SNISAGS TOT.LD. 24.0" 1.25 40.0 20.0 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF

JRFF-FROM SEQN-

11B2x22RZ01

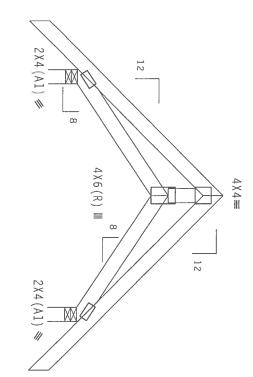
AH

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.08 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/ )=0.18

Wind reactions based on MWFRS pressures



7 10 5

L1-0-0-V R = 312U=33 W=3.5" 5-2-0 Over 2 Supports 2-7-0 R-312 U-33 W-3.5" L1 0 0 V

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

TYP.

Wave

\*\*MARNING\*\* IRUSSES BEQUIRE LETERE CARE IN FARRICATION, IMADELING, SHIPPING, HISTAILING AND BRACING, BELLE TO REST, CHAILDING COMPORENT SAFETY INFORMATION, PUBLESHOLD BY PT (TRUSS PLATE INSTITUTE, 2018 MORTH LE STREE, SUITE 312, ALEXANDRIA, VA. 27314) AND HEA (MODD TRUSS COUNCEL O AMERICA, 6300 CHILERENSE LAKE, MADISON, HI 53/19) FOR SAFETY PRACTICES PRIOR TO PERFORMING HIESE FUNCTIONS. UNLESS OTHERWISE HALD, ALEXANDRO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE

\*\*IMPORTANT\*\*JURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE SHALL HOT BE HIS PURSUANCE ANY OCYTATON FROM THIS DESIGN ANY CALLING TO BUILD THE FRUSS IN COMPORMANCE WITH THE REPORT OF TRUSSES.

TO BE A AMERICATION, MADILING, SHIPPING, INSTALLING A BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPIC, BY ATAPA) AND THI. THE BCG CONNICTION PLATES ARE HADE OF 20/10/166A (M.H.MSS/M.) ASTH A653 GRADE 40/00 (M.K/M.SS) GAV. SHEEL, APPLY

ALPINE

PRANTS (D EASH FACE OF TRUSS AND. HHLESS OHNERHSE LOCATED ON HIS DESIGN, POS); ANY HISPECTION OF PLAIES FOLLOWED BY (1) SMALL BY PER ANNEX AS OF IPTI 2002 SEC.3. DRAWHG INDICALES ACCEPTANCE OF PROFESSIONAL ENGIRETHING RESPONSIBILITY SOLELY P AFRPA) AND TPI. ITH BCG , K/H.SS) GALV. STEEL, APPLY POSITION PER DRAHINGS 160A Z.

THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY IPI1 2002 SEC.3. A SEAL ON THIS BILLITY SOLELY FOR THE TRUES COMPONENT BUILDING IS THE RESPONSIBILITY OF THE



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HC-ENG JB/DF

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PSF

SEQN-

51304

FROM

JRFF-

1TB28778Z01

PSF PSF

DATE REF

09/26/07

Scale = .5"/Ft.

R8228- 54818

GE)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3

Wind reactions based on MWFRS pressures

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

MEMBER TO BE LATERALLY BRACED FOR WIND LO. PERPENDICULAR TO TRUSS. BRACING SYSTEM TO DESIGNED AND FURNISHED BY OTHERS. LOADS

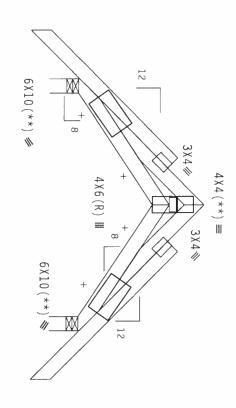
See for more requirements. DWGS A10030EC0207, GBLLETIN0207, & GBLBRSTC0207

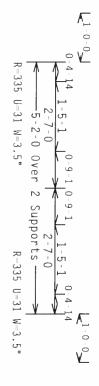
(\*\*) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 18.88 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.





Design Crit: TPI-2002 (STD) Cq/RT=1.00(1.25)/0(0)

0TY:1

FL/-/4/-/-/R/-

Scale =.5"/Ft. R8228-

PLT

TYP.

Wave

\*\*MARNING\*\* IRUSSES REQUIRE EXTREME CARE IN TABRECATION, HARDLING, SHIPPING, INSTALLING AND BRACING, BELTER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY FPI (TRUSS PLATE INSTITUTE, 218 HORRILLE SHITE SIZE, ALEXANDRAE, NA, 22214) AND MICK (MODD TRUSS COUNCIL OF ANRIGA. 6300 CHIERRES EL LAME, MADISON, HISTORIAN SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS CHILLRESSE INDICATED FOR DURBO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAHELS AND BOTTOM CHORD SIBALL HAVE A PROPERLY ATTACHED ATTACHED REGID CELLING.

\*\*IMPORTANT\*\* TURNISH A CUPY OF THIS DESIGN TO THE THISTALLATION CONTRACTOR. ITH BCG, THE, SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRUSS. IN COMPORTANCE WITH THE TO BE RAMETICATING. AMBILTOR. SHIPPING. INSTALLING A BRACING OF TRUSSES.

DESIGN CONTROLLES AND THE TABLE TO THE THISTON OF HOS (MATIONAL DESIGN SPEC, BY ALAPA) AND THE LAPPL CANCEL PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY ALAPA) AND THE LAPPLY AND THE TOP THE ATAPA) AND TP1 ITW BCG. K/M.5S) GALV. STEEL. APPLY PCS ITION PER DRAHINGS 160A Z.

ALPINE

OF TP11 2002 SEC.3.

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CENSO 107 BC LL BC DL DUR.FAC. TC DL SPACING TC LL TOT.LD. 40.0 20.0 10.0 24.0" 1.25 10.0 PSF 0.0

PSF

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REV

JRFF-FROM

1TB28228Z01

PSF PSF

DATE REF

9

/26/07 54819

# CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

#### NOTES

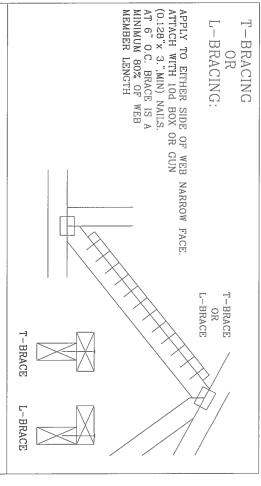
THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

2-2X6(*)	2X6	2 ROWS	2X8
1-2X8	2X6	1 ROW	2X8
2-2X4(*)	2X6	2 ROWS	2X6
1-2X6	2X4	1 ROW	2X6
2-2X4	2X6	2 ROWS	OR
1-2X4	2X4	1 ROW	2X3 OR 2X4
SCAB BRACE	T OR L-BRACE	BRACING	SIZE
E BRACING	ALTERNATIVE BRACING	SPECIFIED CLB	WEB MEMBER

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(\*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.



# SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.

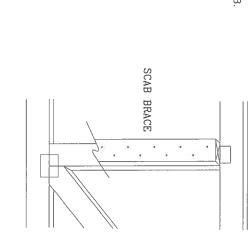
NO MORE THAN (1) SCAB PER FACE.

ATTACH WITH 10d BOX OR GUN

(0.128"x 3.",MIN) NAILS.

AT 6" O.C. BRACE IS A MINIMUM

BO% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640



\*\*\*MARHING\*\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDILING, SHEPPING, HISTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONAIT SAFETY INTORNATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND VICA (VUIDI TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, VI 53719) FIR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL

WHENDER/WHITE FLEWISH COPY OF THIS DESIGN TO INSTALLATION CONFRACTOR ITY BGG, INC. SIMILARD REPORTANTE FLEWISH COPY OF THIS DESIGN, ANY FAILURE ID BUILD THE FRUSS IN THE RESPONSIBLE FOR ANY DEVICE FOR THE STREET OF THE RESPONSIBLE FOR ANY DESIGN STREET OF THE RESPONSIBLE FOR ANY DESIGN STREET OF THE RESPONSIBLE FOR ANY DESIGN STREET OF THE RESPONSIBLE FOR THE PROPERTY OF THE RESPONSIBLE FOR THE STREET OF THE RESPONSIBLE FOR THE STREET OF THE RESPONSIBLE FOR THE STREET OF THE ST



TC LL	PSF REF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

# BEARING BLOCK NAIL SPACING DETAIL

MAXIMUM NUMBER OF NAIL LINES

PARALLEL

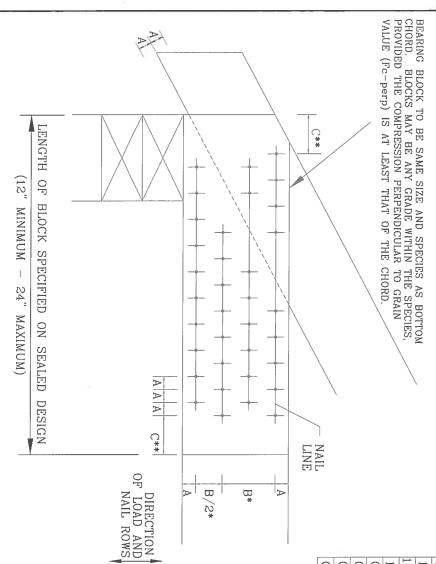
T0

GRAIN

MINIMUM SPACING NAILING FOR TWO FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- CBA EDGE DISTANCE AND SPACING BETWEEN STAGGERED IN A ROW (12 NAIL DIAMETERS) ROWS OF NAILS (6 NAIL DIAMETERS)
- END SPACING OF NAILS
- DISTANCE (15 NAIL DIAMETERS)

Ę NAIL HOLES ARE PREBORED,
 SPACING MAY BE REDUCEI AIL HOLES ARE PREBORED, SOME SPACING SPACING MAY BE REDUCED BY 50% SPACING MAY BE REDUCED BY 33% MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:



		CHC	CHORD SIZE	ZE	
NAIL TYPE	2X4	2X6	2X8	2X10	2X12
8d BOX (0.113"X 2.5",MIN)	ယ	9	6	12	15
10d BOX (0.128"X 3.",MIN)	ယ	S	7	10	12
12d BOX (0.128"X 3.25",MIN)	ω	5	7	10	12
16d BOX (0.135"X 3.5", MIN)	ω	5	7	10	12
20d BOX (0.148"X 4.",MIN)	N	4	5	6	8
8d COMMON (0.131"X 2.5", MIN)	ω	ວາ	7	10	12
10d COMMON (0.148"X 3.",MIN)	N	4	6	8	10
12d COMMON (0.148"X 3.25", MIN)	N	4	6	8	10
16d COMMON (0.162"X 3.5", MIN)	N	4	6	8	10
GUN (0.120"X 2.5",MIN)	ယ	6	8	11	14
GUN (0.131"X 2.5",MIN)	ယ	Q	7	10	12
GUN (0.120"X 3.",MIN)	3	6	8	11	14
GUN (0.131"X 3.", MIN)	ω	27	~2	10	12

# MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	A		B*		C**
8d BOX (0.113"X 2.5",MIN)	3/4"	1	1 3/8"	<u>-</u>	1 3/4"
10d BOX (0.128"X 3.",MIN)	7/8"	1	5/8"		ญู
12d BOX (0.128"X 3.25", MIN)	7/8"	1	1 5/8"		ಬ್
16d BOX (0.135"X 3.5",MIN)	7/8"	1	5/8"	N	1/8"
20d BOX (0.148"X 4.",MIN)	1"	1	7/8"	N	2 1/4"
8d COMMON (0.131"X 2.5", MIN)	7/8"	Ţ	5/8"		νĩ
10d COMMON (0.148"X 3.", MIN)	1"	ī	7/8"	8	2 1/4"
12d COMMON (0.148"X 3.25", MIN)	1"	<b>-</b> →	7/8"	Ø	1/4"
16d COMMON (0.162"X 3.5", MIN)	1,		2,	N	1/2"
GUN (0.120"X 2.5", MIN)	3/4"	-	1/2"	<u> </u>	1 7/8"
GUN (0.131"X 2.5", MIN)	7/8"	1	5/8"		ਨ੍ਹ
GUN (0.120"X 3.",MIN)	3/4"	1	1/2"	1	1 7/8"
GUN (0.131"X 3.",MIN)	7/8" 1 5/8"	-	5/8"		ಬ್

THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK0699

REF

BEARING BLOCK

WALHPERFANTAM FURNISH COPY OF THIS DESIGN TO INSTALLATION COMPRACTOR. ITY BGG, INC., SHALL
NOT BE RESPONSUBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE OF BUILD THE FRUSS. IN
CONTIDENANCE WITH 1P1, DR FABRICATING, HANDLING, SHPPPING, INSTALLING & BRACING TOF RUSS.

DESIGN COMPRETION PLATES, ARE HADE OF 20/18/16GA OF HANSSON, STATH, ASS3 GRADE 40/60 OF MAYASS.

ITV. BGG, CONNECTION PLATES, ARE HADE OF 20/18/16GA OF HANSSON, STATH, ASS3 GRADES AND HASS

GAV, STEEL, APPLY PAIRS TO EACH FACE OF TRUSS. AND JAILESS DIFFERENTS DESIGN SHALL BE PER

BESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTING OF MAINS DIFFERENTS ACCEPTANCE OF PROFESSIONAL

BENION PROFILE OF THE CONTROLL OF THE TRUSS COMPONENT DESIGN SHAPP. THE SUITABILITY OF THE BUILDING DESIGNER, PER

ANSIVER AS OF THE CONTROLL OF ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER

ANSIVER AS OFFICE OF THE BUILDING DESIGNER, PER \*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY (NYTOMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STE, SUITE 312 ALEXANDRIA, VA. 22314) AND VTGA (VOODD TRUSS COLONGLIO MAERICA, 6300 ENTERBRISE LN, HADISON, VI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERISES INDEXISED STORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE



-ENG	DRWG	DATE
SJP/KAR	CNBRGBLK0207	2/23/07

# ASCE 7-02: 110 MPH WIND SPEED 5 MEAN HEIGHT, ENCLOSED, ||1.00, EXPOSURE $\bigcirc$

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		1	2	,,		0	. (	ζ.			1	6	,,		0	. (	J.			2	4	,,		0	) . (	C	•	SPACING	GARLE
			1	<u>٧</u> .	<u>)</u>	TIT	I	OKI				1	C.	}	TIT	I I	JII				j j	<u>ر</u>	)	TIT	I I	ひてコ		SPECIES	2X4 CABLE VERTICAL
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#22	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	4' 11"	5′ 0"	5,0"	5' 3"	5,4,	4' 9"	4' 9"	4' 9"	4' 11"	4' 5"	4 6"	4 6"		4' 10"	4 4"	4' 4"	4 4"	4' 5"	3' 10"	"	4' 0"	4' 2"	4' 3"	3, 9,	3, 9,	3, 9,	3' 10"	BRACES	N O
	7' 5"	8, 5,	8 5	1	80,	7' 3"	8, 5,	8, 5,	8 5	6, 2,	7' 6"	7' 7"		7' 8"	6, 4,"	7' 4"	7' 4"	7' 8"	ڻ ن	6' 1"	6' 2"	6, 8,	6' 8"	5	6' 0"	6' 0"	6'8"	GROUP A	(1) 1X4 "L"
	7' 5"	8' 7"	8, 5,	9' 1"	9' 1"	7' 3"	8, 5,	1		6, 5,	7' 6"	7' 7"	"	8,3,		7' 4"	7' 4"	7' 10"	5 3	6' 1"	6' 2"	7' 2"	7' 2"	ر ا ا	6' 0"	6' 0"	6' 10"	GROUP B	" BRACE *
	9' 10"	10' 0"	10' 0"	10' 0"	10, 0,	9' 7"	10' 0"	10' 0"	10, 0,	8' 6"	9' 1"	9' 1"	l	9' 1"	8' 4"	9' 1"	9' 1"	9' 1"	6' 11"	7' 11"	7'11"	7' 11"	7' 11"	6'9"	7' 11"	7' 11"	7' 11"	GROUP A	(1) 2X4 "L"
G MMAS	9' 10"	10' 6"	10' 6"	10' 9"	10' 9"	9' 7"	10' 0"	10' 0"	10' 3"	8' 6"	9' 6"	9' 6"	9′9"	9' 9"	8' 4"	9' 1"	9′ 1"	9' 4"	6' 11"	8'0"	8' 1"	8' 6"	8' 6"	6'9"	7' 11"	7' 11"	8' 1"	GROUP B	" BRACE *
	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	9' 4"	9' 5"	9' 5"	9' 5"	9' 5"	9' 1"	9' 5"	9' 5"	9' 5"	GROUP A	(2) 2X4 "L"
	12' 3"	12' 6"	12' 6"	12' 10"	12' 10"	11' 11"	11' 11"	11' 11"	12' 3"	11' 1"	11' 4"	11' 4"	11' 8"	11' 8"	10′ 10″	10' 10"	10' 10"	11' 1"	9' 4"	9' 11"	9' 11"	10' 2"	10, 5	9' 1"	9' 5"	9' 5"	9' 8"	GROUP B	BRACE **
	14' 0"	14'0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	10' 10"	12' 5"			12' 5"		-	12' 4"	12' 5"	GROUP A	(1) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"		12' 6"	12' 8"		13′ 5″	10' 7"	12' 3"	12' 4"	12' 9"	GROUP B	BRACE *
	14' 0"	14′0"	"	14' 0"	14′0″	14' 0"	14' 0"	14' 0"					14' 0"		14' 0"		14' 0"	14' 0"	14' 0"	14' 0"		14'0"	14' 0"	14' 0"	- 1	14'0"	14' 0"	GROUP A	(2) 2X6 "L"
	14' 0"		14' 0"	14'0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14'0"	14' 0"		14' 0"		14' 0"	14' 0"	14' 0"		14' 0"	- 1		14' 0"		14' 0"	14' 0"	GROUP B	BRACE **

DOUGLAS FIR-LARCH
#3
STUD

SOUTHERN PINE #3

STANDARD

STUD

GROUP

₽

#1 & BTR #1

HEM-FIR

SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD

#3

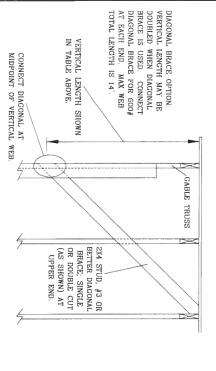
STANDARD

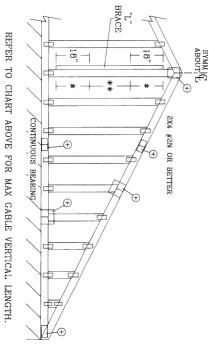
HEM-FIR

BRACING GROUP SPECIES AND GRADES:

GROUP

Α





LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

#2

GABLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12"
PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.

\* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.

\* FOR (2) "L" BRACE: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

PLATE SIZES    NO SPLICE     1X4 OR 2X3     BUT   2X4     2.5X4     EEL PLATES	REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
NO SPLICE X4 OR 2X3 2X4 2X4 2.5X4 DESIGN FOR	RUSS EEL PI	L	TUB	_		PLATE
	DESIGN FOR	2.5X4	2X4	X4 OR 2X3	NO SPLICE	SIZES

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BULDING COMPONENT SAFETY (MEDRANTIDA), PUBLISHED BY TPI CRUSS PLATE INSTITUCE, 218 MODERN LEE SER, SUITE 122, ALEXAMBRIA, VA. 252145 AND VICA CYODIO TRUSS COUNCIL MARRICA, 6300 ENTERPRISE LE, HADISON, WI 537195 FOR SAFETY PRACTICES PRIBE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TIP CHIRD SHALL HAVE PRIBERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CEILING.

WHORPKANITH FURNASH COPY OF THIS DESIGN TO INTALIATION CONTRACTOR TITY BCG, INC. SHALL NOT BE RESERVASIBLE OR ANY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRUSS IN CONFIDENCE WITH 1911 DR FABRICATION, HANDLING, SHEPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFIDENCE WITH APPLICABLE PROVISIONS OF MIS CHATIDHAL DESIGN SPEC, BY AFRAN AND THIS DECISION SPEC, BY AFRAN AND THIS DESIGN CONFIDENCE WITH APPLICABLE PROVISIONS OF MIS CHATIDHAL DESIGN AND ASSOCIATION PLATES ARE MADE OF ENTIRE STATE AND MISSES AND MISSES OF MIRRY SEC DANE 40/06 (W,KMASS) CALL SPECIAL SPEC

ITWBUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

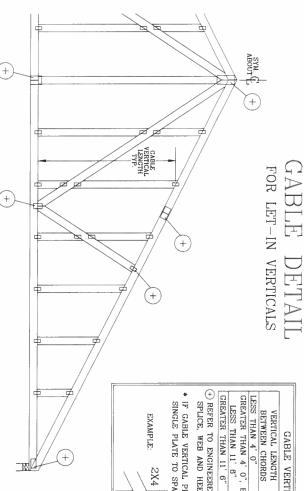
No. 66648

MAX. TOT.

SPACING

24.0"

LD.				
60				
LD. 60 PSF				
	-ENG	DRWG	DATE	REF
		A11015EE0207	2/23/07	ASCE7-02-GAB11015



_						
	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' 0"	BETWEEN CHORDS	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
	2.5X4	2X4	1X4 OR 2X3	SIZE	PLATE	L PLATE SI
	2.5X8	2X8	2X8	OVERLAP*	IF PLATES	ZES

TOENAIL

TOENAIL

2X4 "T" REINFORCING MEMBER

2X6 "T" REINFORCING MEMBER







ATTACH EACH "T" REINFORCING MEMBER WITH PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN

HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AN AND BOTTOM CHORD

GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C.
(4) TOENAILS IN TOP AND BOTTOM CHORD. PLUS

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

"T" REINFORCING-MEMBER

4 TOENAILS

RIGID SHEATHING

GABLE. TRUSS

TOENAILS SPACED AT 4" O.C.

ASCE 7-93 GABLE DETAIL DRAWINGS

ASCE 7-98 GABLE DETAIL DRAWINGS A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

ASCE 7-02 GABLE DETAIL DRAWINGS A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207 A08530EC0207

ASCE 7-05 GABLE DETAIL DRAWINGS A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207, A10030EE0207, A10050EE0207, A10050EE0207, A10050EE0207, A10050EE0207, A1 A08530EE0207

A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207 A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

4 TOENAILS

CEILING

								_
EXAMPLE: 2X4	* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.	(+) REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' 0"	BETWEEN CHORDS	VERTICAL LENGTH	CEPTO APPLIABLE PROPERTY
2XX	S OVERLAP, HE WEB.	RUSS DESIGN LATES.	2.5X4	2X4	1X4 OR 2X3	SIZE	PLATE	L LINIE OF
8XX	USE A	FOR PEAK,	2.5X8	2X8	2X8	OVERLAP*	IF PLATES	650
					_		_	_

VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD. APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR

WEB LENGTH INCREASE W/ BRACE

	WIND SPEED	T. REINE.	CBCCI	d Cal	
	AND MRH	MBR. SIZE	ישטינין	AJUE	
	110 MPH	2x4	10 %	2 01	
	15 FT	2x6	40 %	50 %	
	110 MPH	2x4	10 %	10 %	
	30 FT	2x6	50 %	50 %	
	100 MPH	2x4	10 %	10 %	
	15 FT	2x6	30 %	50 %	
	100 MPH	2 <b>x4</b>	10 %	10 %	
-	30 FT	2x6	40 %	40 %	
	90 MPH	2x4	20 %	10 %	
_	15 FT	2 <b>x</b> 6	20 %	40 %	
	90 MPH	2x4	10 %	10 %	
	30 FT	2 <b>x</b> 6	30 %	50 %	
	80 MPH	2 <b>x4</b>	10 %	20 %	
	15 FT	2 <b>x</b> 6	10 %	30 %	
	80 MPH	2x4	20 %	10 %	
	30 FT	2x6	20 %	40 %	
	70 MPH	2x4	2,0	20 %	
	15 FT	2x6	0 %	20 %	
	70 MPH	2x4	10 %	20 %	
	30 FT	2x6	2 01	30 %	
-	EVALUE E				

#### EXAMPLE:

MEAN ROOF HEIGHT = 30 FT GABLE VERTICAL = 24" O.C. SP #3 "T" REINFORCING MEMBER SIZE = 2X4 "T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10 (1) 2X4 "L" BRACE LENGTH = 6' 7" ASCE WIND SPEED = 100 MPH MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH 1.10 x 6' 7" = 7' 3"



\*\*EVARAHING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACKING. REFER TO BESS (GUILDING COMPENENT SAFT WEIGHANTIN), POBLISHED BY TEY CIRUSS PLATE INSTITUTE, 218 MORTH LEE STR., SUITE 312, ALEXAMIRA, VA. 2234) AND WTCA CAUDID TRUSS COUNCIL RABRICLA, 6300 ENTERPRISE LH, HADISON, WI 53719) FOR SAFETY PRACTICES PRIGR TO PERFORMING THESE FUNCTIONS. UNIESSS DITHEMSES INDICATOR, TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL

WHINDERWAITH FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITY BEG, INC. SHALL

ROTE OF RESPONSIBLE COP AND SECURITY FROM THIS DESIGN ANY FAILURE TO BUILD THE TRUSS IN

ODHEDWANCE VITH, IPF; DR FARRICATING, HAND BUILS, SHIPPING, INSTALLING & BRACING OF FRISSAS,

DESIGN CONNECTOR PLATES ARE HADE OF PROVISIONAL OF SHIPPING, INSTALLING & BRACING OF FRISSAS,

LEVEN CONNECTOR PLATES ARE HADE OF PROVISIONAL OF THE SHIPPING AND THE SHIPPING OF THE SHIPPIN



MAX SPACING DUR. FAC.

ANY 24.0"

DLJ/KAR	-ENG	MAX TOT. LD. 60 PSF	ED.	TOT.	MAX	A CHIMINISTING
GBLLETIN0207	DRWG					
2/23/07	DATE					
LET-IN VERT	REF					

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719

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HC26294035

# ASCE ~2 -98: 110 MPH WIND SPEED, 15 MEAN HEIGHT, ENCLOSED, 1.00, EXPOSURE

		_				_																							
			M	A	X	7	(	J.	4]	B	L	E		V	E	F	Γ.	Ί	C	A	L		I	Æ	'N	J(	7. J.	ГΗ	
		1	2	,,		0	. (	C	•		1	6	,,		0	. (	<u> </u>			2	4	,,		С	) . (	С	٠	SPACING	GABL
		DHIL	ן ן	7	3	111	I,		N D I		LH.	1	C.	3	TTT	I I	SIL				]	\(\frac{1}{2}\)	) J	TTT		777	Q J J	SPACING SPECIES	2X4 GABLE VERTICAL
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
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ลิ	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	11' 11"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	10' 10"	9' 4"	9' 5"	9' 5"	9, 5,"	9' 5"	9' 1"	9' 5"	9' 5"	9' 5"	GROUP A	(2) 2X4 "L"
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	14' 0"	14' 0"	14' 0"	14' 0"	14′0″	14' 0"	14' 0"	14′0"	14' 0"	13' 3"	- 1	14' 0"	14' 0"		12' 11"		14'0"	14' 0"		12, 5"	- 1	12' 5"	12′5″		12' 3"	12' 4"	12' 5"	GROUP A	(1) 2X6 "L"
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DOUGLAS FIR-LARCH #3 STUD

SOUTHERN PINE #3

STANDARD

STANDARD

STUD

GROUP

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HEM-FIR #1

SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD

#3

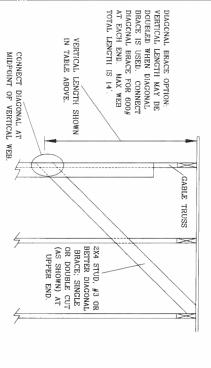
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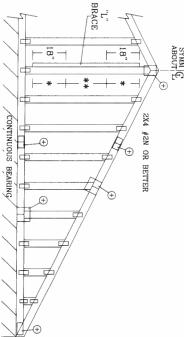
HEM-FIR

BRACING GROUP SPECIES AND GRADES:

GROUP

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# GABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

#2

12 1

GABLE END SUPPORTS LOAD FROM 4' 0"
OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD). LIVE LOAD DEFLECTION CRITERIA IS L/240. PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH IOD NAILS

\* FOR (I) "L" BRACE: SPACE NAILS AT 2" O.C.

\*\* FOR (I) "L" BRACE: SPACE NAILS AT 2" O.C.

\*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C.

IN 18" END ZONES AND 6" O.C. BETWEEN ZONES. MEMBER LENGTH. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	GREATER THAN 11' 6"	CREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' 0"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
PLATES.	2.5X4	2 <b>X4</b>	1X4 OR 2X3	NO SPLICE	TE SIZES

DATE REF

DRWG

A11015EC0207 2/23/07 ASCE7-98-GAB11015

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ANSI/TPI I SEC. 2.	"". ANNEX A3 OF TP1 1-2002 SEC. 3. A SEAL ON THIS BRAVING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE IRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND LIVE OF THIS CONTROLLY FOR THE FROM THE PROFESSION OF THE SUITABILITY AND LIVE OF THE SUITABILITY AND LIVE OF THE SUITABILITY AND LIVE OF THE SUITABILITY OF THE SUITABILITY OF THE SUITABILITY AND LIVE OF THE SUITABILITY AND LIVE OF THE SUITABILITY OF THE SUITABIL	GALV. STEEL. APPLY PLATES TO EACH FACE OF TRISSS AND UNLESS OTHERWISE LOCATED ON THIS GALV. STEEL PROPERTY OF THE TRISS AND UNLESS OTHERWISE LOCATED ON THIS GALVE. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER	DESIGN CONFIDENTS WITH APPLICABLE PROPUSIONS OF MOST WATER AND AFTER AND THE LITTLE PROPUSIONS OF MOST WATER AND THE LITTLE PROPUSIONS OF MOST WATER AND AFTER AND THE LITTLE PROPUSIONS OF MOST WATER AND AFTER AND THE LITTLE PROPUSIONS OF MOST WATER AND THE LITTLE PROPUSIONS OF MOST WATER AND THE LITTLE PROPUSIONS OF MOST WATER AND THE	**IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. IT'V BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN	FUNCTIONS. UNLESS DTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL. PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.	INSTITUTE, 218 URTH LEES STR. SUITE 312, ALEXANDRIA, VA. 22314) AND ATCA (VOID TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WEST STREET STR. SUITE 312, ALEXANDRIA, VA. 22314) AND ATCA (VOID TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WE 33719) FOR SAFETY PRACTICES PRIOR TO PERETORNING THESE	**WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND
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MAX. SPACING 24.0"		MAX. TOT. LD. 60 PSF					
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ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

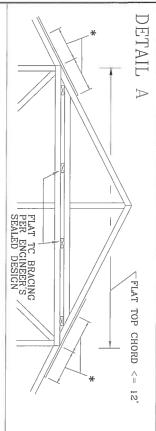
# PIGGYBACK DETAIL

100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

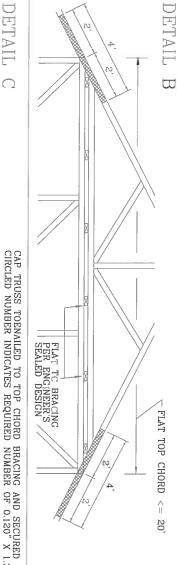
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATLY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

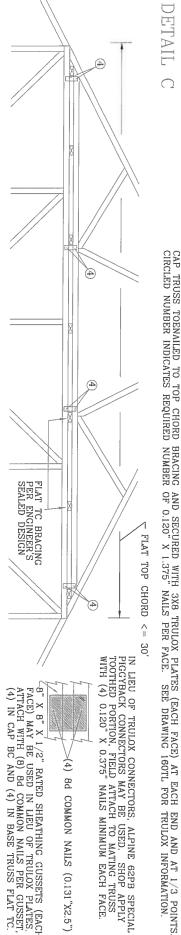


PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.

\* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"X3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.



IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO NATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.

Z8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 8d COMMON NAILS PER GUSSET, (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

(4) 8d COMMON NAILS (0.131"X2.5")

THIS DRAWING REPLACES DRAWINGS 581,670 & 961,860



\*\*AVARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFEET TO BEST (BULLDING COMPINENT SAFETY INFORMATION), PUBLISHED BY TEI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STE, SUITE 312, ALEXANDRIA, VA. 22314) AND WITCA (AVOIDD TRUSS COUNCIL, BUSINGER, 184 AGENICA, 6300 ENTERPRISE LN, HANDSON, WI 53719) FOR SAFETY PRACTICES PRIBE TO PERFORMING THESE FUNCTIONS. UNIESS DIHERVISE INDICATED. TOP CHORD SMALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERRY ATTACHED STRUCTURAL

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	TOT. LD. MAX 60 PSF	-ENG	DRWG -ENG	DL PSF DRWG LL PSF — ENG T. LD. MAX 60 PSF

TOP CHORD BOT CHORD 2X4 2X4 2X4 ##2

# PIGGYBACK

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To

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. TRUSS TOP CHORD WITH 1.5X3 PLATE. ATTACH VERTICAL WEBS TO

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS. REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX. 110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

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MAX SIZE OF 2X12 #2 OR BETTER

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20' FLAT TOP CHORD MAX SPAN

EITHER PLATE LOCATION IS ACCEPTABLE

SPLICE

(<del>4</del>) 6d BOX (0.099"X 2.", MIN) NAILS.

C8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET. (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC

JOINT Α [Z] D a В 1.5X3 4X6 5X4 4X6 2X4 30 OR 3X6 TRULOX AT 4' ROTATED VERTICALLY 2.5X4 5X5 5X6 34 1.5X4 2.5X4 5X5 5X6 38 5X6 252 20

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

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	10' TO 14'	7'9" TO 10'	7'9"	WEB LENGTH	
	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND BOZ LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC		0' TO 7'9" NO BRACING	REQUIRED BRACING	WEB BRACING CHART

# \* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.

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SPACING 24.0"	1.15 DUR. FAC.	47 PSF AT		50 PSF AT	1.33 DUR. FAC.		MAX LOADING
				-ENG	DRWG	DATE	REF
				ENG DLJ/KAR	DRWG PIGBACKB0207	2/23/07	PIGGYBACK

PANELS AND BOTTOM CH AMERICA, 6300 ENTERP BRACING REFER TRUSSES REQUIRE EXTREME CARE IN CARRICATING, HANDLING, SHIPPING, INSTALLING AND TER TO BESS (SUIL DING CHAPPINAT SAFETY INFORMATION), PUBLISHED BY, PI CHARLS RAATE BARRET LEE SIR, SUITE 312, ALEXANDRIA, VA. 22314) AND VITCA CADD TRUSS COUNCIL OF LEHERRISE LN, HANDSON, VI 537159 FIRE SAFETY PRACTICES PRIOR TO PERFORMING THESE WALSS OTHERWISE INDICATED, TIP CHARD SIALL, HAVE PROPERLY ATTACHED STRUCTURAL

PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.

ITWBUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA GALV. STEEL. APPLY PI DESIGN, POSITION PER DI ANNEX A3 OF TPI 1-200 ENGINEERING RESPONSIBI USE OF THIS COMPONENT ANSI/TPI 1 SEC. 2. DESIGN CONFORMS WITH \*\*\*IMPORTANT\*\* FURNI HOT BE RESPONSIBLE F CONFORMANCE WITH TPI

#### ASCE 7 - 02: 110 MPHWIND SPEED, 30, MEAN HEIGHT, ENCLOSED, 1.00, EXPOSURE $\bigcirc$

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	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
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	6' 9"	7' 9"	7' 11"	8' 0"	1	6' 7"	7' 8"	7' 8"	8, 0,	5' 10"	6' 9"	6' 10"	7' 3"	7, 3,	5, B	8, 0,	6, 8,	7' 3"	4' 9"	5 6"	5' 7"	6' 4"	6' 4"	4' 8"	5, 5,	5, 5,	6' 4"	GROUP A	(1) 1X4 "L"
	6' 9"	7' 9"	7' 11"	8' 7"	8' 7"	6' 7"	7' 8"	7' 8"	හ <sub>.</sub> ස	5' 10"	6' 9"	6' 10"	7' 9"	7' 9"	5, 8,	8' 0"	6. 8.	マ, 5"	4' 9"	5' 6"	5' 7"	6' 10"	6' 10"	4' 8"	5' 5"	5, 5,	6' 6"	GROUP B	" BRACE .
	8' 10"	9' 5"	9, 5,	9' 5"	9' 5"	8,8	9' 5"	9' 5"	9' 5"	7' 8"	8' 7"	8' 7"	8' 7"	8' 7"	7' 6"	8' 7"	8' 7"	8' 7"	6' 3"	7' 3"	7' 4"	7' 6"	7' 6"	6'1"	7' 1"	7' 2"	7' 6"	GROUP A	(1) 2X4 "L"
MAS	8' 10"	9' 11"	9' 11"	10' 2"	10' 2"	8' 8"	9' 5"	9' 5"	9' 8"	7' 8"	8' 11"	9' 0"	9' 3"	9' 3"	7' 6"	8' 7"	8' 7"	8' 10"	6' 3"	7' 3"	7' 4"	8' 1"	8' 1"	6' 1"	7' 1"	7' 2"	7' 8"	GROUP B	BRACE *
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	11' 7"	11' 10"	11' 10"	12' 1"	12' 1"	11' 3"	11' 3"	11' 3"	11' 7"	10' 4"	10' 9"	10' 9"	11' 0"	11' 0"	10′ 1″	10' 3"	10' 3"	10' 6"	8' 5"	9'5"	9, 5,	9' 7"	9' 7"	8' 3"	8' 11"	8' 11"	9' 2"	GROUP B	BRACE **
- 1	13′ 10″	14' 0"	14' 0"	14'0"	14' 0"	13' 6"	14' 0"	14' 0"	14' 0"	11' 11"	13' 5"	13′ 5″	13′ 5″	13' 5"	11' 8"	13' 5"	13' 5"	13' 5"	9' 9"	11' 4"	11' 5"	11, 9,,	11' 9"	9' 6"	11' 1"	11' 2"	11' 9"	GROUP A	(1) 2X6 "L"
- 1	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	13' 6"	14' 0"	14' 0"	14' 0"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	11' 8"	13' 5"	13' 5"	13' 10"	9' 9"	11' 4"	11' 5"	12' 8"	12' 8"	9' 6"		11' 2"	12' 1"	GROUP B	," BRACE *
	- 1	14' 0"	14' 0"	14' 0"	14' 0"					14' 0"	14' 0"			14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 3"	14' 0"	- 1	- 1	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	GROUP A	(2) 2X6 "L"
- 1	- 1	14′ 0"	14' 0"	14' 0"	14' 0"	14' 0"	- 1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	12' 11"	14' 0"	14' 0"	14' 0"	GROUP	BRACE

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE #3 STUD

STANDARD

GROUP B:

HEM-FIR

SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD

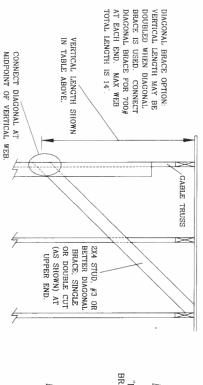
#3

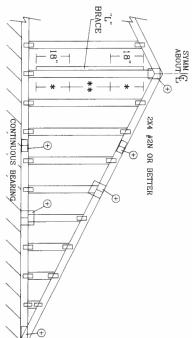
STANDARD

HEM-FIR

BRACING GROUP SPECIES AND GRADES:

GROUP A:





MEMBER LENGTH.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB

REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

LIVE	_
LIVE LOAD	GABLE
	H
DEFLECTION	TRUSS
SE	
CRITERIA	DETAIL
S	Ţ
L/240.	NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG. PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

- ATTACH EACH "L" BRACE WITH 10d NAILS.

  \* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
  IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

  \*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

+			_	_	
+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.	GREATER THAN 11' 6"	GREATER THAN 4' 0", BUT LESS THAN 11' 6"	LESS THAN 4' O"	VERTICAL LENGTH	GABLE VERTICAL PLATE SIZES
RUSS		TUE	-		PLATE
DESIGN FOR	2.5X4	2X4	1X4 OR 2X3	NO SPLICE	SIZES
L .					

	MUSICIPI I SEL. C.
•	USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER
	ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND
Ā	'  ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL
	, DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER
a	GALV STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS
7	ITW, BCG COMMECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W,K/H,SS)
	DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AFRPA) AND TPI.
•	CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
ī	NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN
-	**IMPORTANT** FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL
4	
-	PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.
	FUNCTIONS: UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL
	AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE
	INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF
	BRACCING. REFER TO BOSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE
_	*** CARLING **   TROUDED REGULAR EXIZENC CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND

ITWBUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA

ALPINE

SONAL ENGINEE 26 '0≱ SUAS FIEL No. 66648 RENSE \* MAX. MAX. Ţ SPACING

	OT. LD. 60 PSF				
		-ENG	DRWG	DATE	REF
			A11030EE0207	2/23/07	ASCE7-02-GAB11030

24.0"

## ASCE 7 - 98: 100 MPH WIND SPEED, 30 MEAN HEIGHT, ENCLOSED, ||1.00, **EXPOSURE** $\Box$

				_		_					_	_	_	_	_		_	_	_		_	_	_		65				
		]	M	A	X	-	(	J.	\]	3]	 [_]	E		V	E	R	Τ:	Ί	С	A	L		L	E	N		רר	Ή	
		1	2	,,		0	. (	ζ.			1	6	,,		0	. (	ζ.	,		2	4	,,		0	. (	C .		SPACING	CARL
	1	<u> </u>	1	<u>V.</u>	)	TIT	I I	7	2 7 7			j j	<u>ر</u>	)	TTT	I I I	נילו	2 7 7 7			1	<i>V.</i>	)	TTT	I I	ひてコ	Ω J	SPECIES	2X4
	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	#2	#1	STANDARD	STUD	#3	#1 / #2	STANDARD	STUD	#3	## Z2	#1	STANDARD	STUD	#3	#1 / #2	GRADE	BRACE
	5' 0"	5' 1"	5, 1,"	5, 4,	5' 6"	4' 10"	4' 10"	4' 10"	5,0,"	4' 6"	4' 8"	4' 8"	4' 10"	5.0"	4 5"	4. 5."	4. 5."	4' 6"	3' 11"	4' 1"	4' 1"	4' 3"	4' 4"	3' 10"	3' 10"	3' 10"	3' 11"	BRACES	N C
	7' 7"	8' 7"	8' 7"	8' 7"	8' 7"	7' 5"	8' 7"	8' 7"	8' 7"	6' 7"	7' 7"	7' 8"	7' 10"	7' 10"	6, 5,	7' 5"	7' 6"	7' 10"	5 4	6, 5,	6' 3"	6' 10"	6' 10"	5 3"	6' 1"	6'1"	6' 10"	GROUP A	(1) 1X4 "L"
	7' 7"	8' 9"	8' 10"	9' 3"	9' 3"	7' 5"	8' 7"	8' 7"	8' 10"	6' 7"	7' 7"	7' 8"	1 -	8, 5,	`	7' 5"	7' 6"	8' 0"	5, 4,	6, 5,	6' 3"	7' 4"	7' 4"	5 3	6' 1"	6' 1"	7'0"	GROUP B	" BRACE *
	10' 0"	10' 2"	10' 2"	10' 2"	10' 2"	9' 9"	10' 2"	10' 2"	10' 2"	8' 8"	9' 3"	9' 3"	9' 3"	9' 3"	8, 5,		9' 3"	9' 3"	7' 1"	8' 1"	8'1"	8'1"	8' 1"	6' 11"	8' 0"	8' 1"	8' 1"	GROUP A	(1) 2X4 "I
OI MRAS	10' 0"	10'9"	10' 9"	11' 0"	11' 0"	9' 9"	10' 2"	10' 2"	10' 6"	8' 8"	9' 9"	9' 9"	10' 0"	10' 0"	8, 5,	9' 3"	9' 3"	9' 6"	7.	8' 2"	8' 3"	8' 9"	8' 9"	6' 11"	8' 0"	8' 1"	8' 4"	GROUP B	"L" BRACE *
2	12' 2"	12' 2"	12' 2"	12, 2,	12' 2"	1	12' 2"		12' 2"	11' 1"	11' 1"	11' 1"	11' 1"	11' 1"	11' 1"	11' 1"	11' 1"	11' 1"	9' 6"	9' 8"	9'8"	9' 8"	9' 8"	9' 3"	9' 8"	9' 8"	9' 8"	GROUP A	(2) 2X4 "L"
	12' 6"	12' 10"	12' 10"	13' 1"	13' 1"	1 1	12' 2"	12' 2"	12' 6"	11' 4"	11' 8"	11' 8"	11' 11"	11' 11"	11' 1"	11' 1"	11' 1"	11' 4"	9' 6"	10' 2"	10' 2"	10' 5"	10' 5"	9' 3"	9′ 8″	9' 8"	9' 11"	GROUP B	BRACE ** (1)
	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	13' 5"	14' 0"	14' 0"	14'0"	14' 0"	13' 2"	14' 0"	14' 0"	14' 0"	11' 0"	12' 8"	12' 8"	12'8"	12' 8"	10' 9"	12' 6"	- 1	12' 8"	GROUP A	(1) 2X6 "L"
	14' 0"	14' 0"	14' 0"	14′0″	14'0"	14' 0"	14' 0"	14' 0"	14' 0"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"	13' 2"	14' 0"	14' 0"	14' 0"	11' 0"	12' 9"	12' 10"	13′ 8″	13′ 8″	10' 9"	12' 6"	12' 7"	13' 1"	GROUP B	BRACE *
	14' 0"	14' 0"	14' 0"	- 1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14′0″	- 1	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14' 0"	14' 0"		14' 0"	GROUP A	(2) 2X6 "L"
	14' 0"	14' 0"	14' 0"		14, 0,	14' 0"	14' 0"	14'0"		14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14'0"	14'0"			14' 0"	14' 0"	14' 0"	14' 0"	14′0"	14' 0"	GROUP B	BRACE **

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE #3

STANDARD

STUD

GROUP HEM-FIR

₩

SPRUCE-PINE-FIR
#1 / #2 STANDARD
#3 STUD

#1

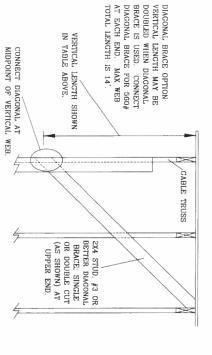
BRACING GROUP SPECIES AND GRADES:

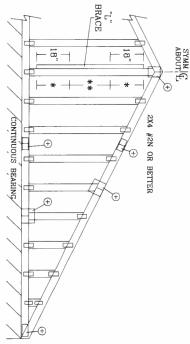
GROUP

A # 15

HEM-FIR

STANDARD STUD





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

# GABLE TRUSS DETAIL NOTES:

SOUTHERN PINE

DOUGLAS FIR-LARCH

75 7

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PROVIDE UPLIFT CONNECTIONS FOR 75 PLF OVER LIVE LOAD DEFLECTION CRITERIA IS L/240. PLYWOOD OVERHANG CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

- ATTACH EACH "L" BRACE WITH 10d NAILS.

  \* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C.
  IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.

  \*\* FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. "L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.
- GREATER THAN 11' 6" VERTICAL LENGTH
  LESS THAN 4' 0"
  GREATER THAN 4' 0", BUT REFER TO COMMON TRUSS DESIGN FOR GABLE VERTICAL PLATE SIZES ESS THAN II' 6' NO SPLICE 1X4 OR 2X3 2.5X4 2X4

PEAK, SPLICE, AND HEEL PLATES.

CORIO	STATE OF SERVICE	PATATION NO.	26 Ng. 66648	LIGENS, US	
	MAX.		MAX.		2.
2002	MAX. SPACING 24.0"		MAX. TOT. LD. 60 PSF		
	ING		LD.		
	24		60		
	.0"		PSF		

ITWBUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

WHEREPRANTE FURNISH COPY OF THIS DESIGN TO INSTALLATION COPPRENCIOR. ITY BCG, NO. SANL
HOT BE RESPONSIBLE FOR ANY DEVALUED FROM THE SUSSIGN ANY FALLIRE IT BRILD THE FRUSS IN
COMPRIBMANCE WITH APPLIED FARBICATING HANDLING SHAPPING INSTALLING & BRACING OF PRISSES.

BESTON COLONICTION PLATES ARE HADE OF SOLVENING WITH APPLIED TO SOLVENING OF THE S

\*\*WARNING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSI GBULDING COMPURAT SAFETY INFORMATION, PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STR, SUITE 312, ALEXANDRIA, VA. 2-234) AND VTCA. CYODD TRUSS COLONGLE INSTITUTE, 218 MORTH LEE STR, SUITE 312, ALEXANDRIA, VA. 2-234) AND VTCA. CYODD TRUSS COLONGLE AMERICA, 6-300 ENTERPRISE LN, HADISON, WI 537(9) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS DIMENSISE INDICATED, TOP GROUPE SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL

ALPINE

	K. TOT. LD. 60 PSF				
	PSF	-ENG	DRWG	DATE	REF
			DRWG A10030EC0207	2/23/07	ASCE7-98-GAB10030

# COMMON RESIDENTIAL GABLE ASCE 7-98: END WIND EXPOSURE BRACING REQUIREMENTS STIFFENERS

100 MPH 3 SECOND GUST WIND, 30 FT MEAN HGT, A CLOSED BLDG, LOCATED ANYWHERE IN ROOF, I=1.00, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF. ASCE 7-), CAT II, -98

LATERAL CHORD BRACING REQUIREMENTS TOP: CONTINUOUS ROOF SHEATHING BOT: CONTINUOUS CEILING DIAPHRAGM

ON THIS DETAIL. ENGINEER'S SEALED DESIGN REFERENCING LUMBER, PLATES, AND OTHER INFORMATION THIS S DETAIL SHOWN

NAILS: 10d COMMON OR BOX (0.148"X 3.", MIN) NAILS GUN (0.125"X 3.", min) NAILS 0R

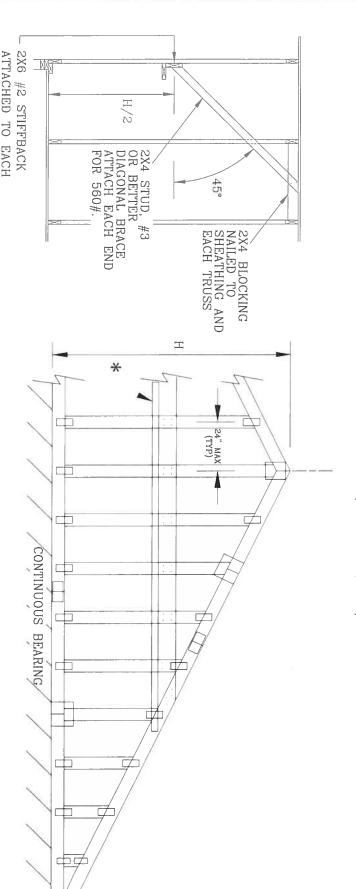
> LESS THAN 4'6" NO STUD BRACING REQUIRED

工  $\blacksquare$ 

GREATER THAN 4'6" TO 7'6" PROVIDE A 2X6 STIFFBACK AT MID-HEIGHT AND BRACE STIFFBACK TO ROOF DIAPHRAGM EVERY 6'0" (SEE DETAIL BELOW OR REFER TO DRAWING A10030EC1103). IN LENGTH

 $\blacksquare$ GREATER THAN 7'6" TO 12'0" MAX:
PROVIDE A 2X6 STIFFBACK AT MID-HEIGHT AND BRACE
TO ROOF DIAPHRAGM EVERY 4'0" (SEE DETAIL BELOW OR
REFER TO DRWG A10030EC1103).

\* OPTIONAL 2X L-REINFORCEMENT ATTACHED TO STIFFBACK WITH 10D BOX OR GUN (0.128" X 3", MIN.) NAILS @ 6" O.C.





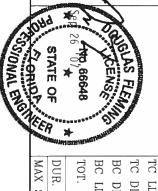
STUD W/

(4)

10 D BOX OR GUN (0.123" X 3", MIN.) NAILS

\*\*WARRHING\*\* TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS CAULDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TELCRUSS PLATE INSTITUTE, 218 NIBETH LEE STR., SUITE 312, ALEXANDRIA, VA. 2234) AND VETCA CAUDID TRUSS COUNCIL MARKICA, 4630 ENTERPRISE LN, HADISON, WI 53719) FOR SAFETY PRACTICES PRIGE TO PERFORMING THESE FUNCTIONS. UNLESS OTHERVISE INDICATED. TOP CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOH CHORD SHALL HAVE PROPERTY ATTACHED STRUCTURAL PANELS AND BOTTOH CHORD SHALL HAVE A PROPERTY ATTACHED STRUCTURAL

WHIPDERVANES FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR ITY BGG, INC., SMALL ONLY FROM THE PROPERTY OF THIS DESIGN, ANY FALURE T BRILD HE TRUSS. IN CONFIDENCIAL HTML OF FABRICATING HANDLING, SHIPPING, INSTALLINE SEED, BY AFRAM AND THIS DESIGN ANY FALURE TO BRILD HE TRUSSES. BESSION CONTRACTOR OF FARRICATING HANDLING, SHIPPING, INSTALLINE SEED, BY AFRAM AND THIS DESIGN SHIPPING HAS PROPERLY BELOW OF THIS DESIGN SHIPPING HAS SHIPPING HAS SHIPPING HAD AND THIS DESIGN SHIPPING HAVE DEFINED HANDLING HAD AND THIS DESIGN SHIPPING HAVE DEFINED HAD THE SHIPPING HAVE TO BE THIS COMPONENT BESIGN SHIPPING HE PROPERSIONAL BEFORE HAD FOR THIS COMPONENT BESIGN SHIPPING HAVE DEFINED SHIPPING HAVE DEFINED HE SHIPPING HAVE DEFINED HE SHIPPING HAVE DEFINED HAVE DEFINED



	TC LL	PSF REF	REF	GE WHALER
FIFE	TC DL	PSF	PSF DATE	2/23/07
NO. A	BC DL	PSF	DRWG	GBLBRSTC0207
anaug.	BC LL	PSF	-ENG	-ENG SJP/KAR
\$6648 	TOT. LD.	PSF		
	DUR. FAC.			
A SEE	MAX SPACING 24"	24"		

THIS DRAWING REPLACES

DRAWING 59469/GE

IN ROOF, CAT II, 140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC. BLDG, LOCATED ANYWHERE EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF

BRACING DEFINITIONS:

140 MPH WIND, 30. IN ROOF, CAT II, 30.0 FT MEAN HGT. ASCE 7-02, PART. ENC. BLDG, LOCATED ANYWHERE II. EXP C. WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

- FOR VERTICAL VERTICAL WEBS LESS THAN 4'0": W1X4
  WEBS GREATER THAN 4'0" BUT NO MORE THAN 11'6": WZX4
- SPLICE. PEAK. AND HEEL PLATES TO MATCH COMMON TRUSS
- 2X4 OR GREATER CHORDS

DROP GABLE WILL SUPPORT 4'0" SPACED 24" O.C., OR THE LOAD OUTLOOKERS WITH 2'0" OVERHANG (DROP HEEL GABLE) FROM 12" PLYWOOD OVERHANG (NOMINAL HEEL GABLE).

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

AND THE BUILDING DESIGNER IS RESPONSIBLE FOR THE GABLE SHEAR WALL DESIGN. CEILING ROOF SHEATHING DIAPHRAGM CONNECTIONS, AND ALL TRUSS TO WALL CONNECTIONS.

7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

R.1 NOTE: HIIM STEPS OF LADDER TRUSS ONTO 2:16D NAILS AT EACH END. 3HT OUTSIDE PIECES

NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED

> 16" 24 " STUD STUD 16" SPACING 8 NOTE: "END ZONE" EXISTS 18" 12" 12" 2X4 SP 0 (C) (B) (1) (2) (2) END (1) END SPACING / BRACING #3 ZONES; 2X6 SP ZONES; 2X6 SP ZONES; 2X4 SP ZONES; 2X4 SP L/360 L/360 L/180 L/180 L/180 DEFLEC CRITERIA NOIL #2 6"3 4 # 3 4 " #2 N "L" BRACE. OC. OC. BETWEEN ZONES OC. 0C. \_= N "L" BRACES. BETWEEN ZONES. BETWEEN ZONES BRACES. BETWEEN ZONES. BRACE. BRACE TABLE: NO ΑŢ ATTACH 80TH ATTACH EACH WITH 0.128"X3" NAILS ATTACH WITH 0.128"X3" ATTACH EACH WITH 0.128"X3" NAILS @ 3" 3'11" 4 ' 7 " 4'9" 3' 4" TYPE (1) 2X4 "L" BRACE ENDS HIIM (A) QF 0.128"X3" VERTICAL TYPE (B) "L" BRACE (2) οū 5 5 4 4 ω ‡ 7 2 2 X 4 NAILS WEB NAILS @ (1) 2X6 "L" BRACE TYPE (C) e 8'11" 9 7'10" 9 9 2 6, يب 0 2 oc. @ 0C. يى ļ., (2) TYPE (D) 11' 0" 17 11' 0" 9'11" 8 OC. BRACE 2 X 6 0C. 0 0 Z Z

STEPS @ 24" LADDER W/ FASCIA INCLUDES OVERHANG DETAIL 0C. MAX 0-0 24" 0.C TRUSSES (e) ++ R2: Over Continuous Support REVISED FOR ASCE 7-02. REFER TO TABLE FOR BRACING REQUIREMENTS. U=280 PLF

Note: All Plates Are

TYP.

ALT. GABLE SHAPES

2X4 Except As Shown.

Wave TPI-95 \*\*HARNING\*\* IRUSSES REQUIRE EXIRENE CARE IN FABRICATION, INAUDITNG, SHIPPING, INSTALLING AND BRACING. RETHE OF DEST, 103 (BUILDING COMPONENT SAFETY IN GRANDING, PUBLISHED BY FET (RISS PLATE HESTITUTE, 583 D'UNORFETO DE. SHITE ZOO, AND SOON ENTERPRISE LE, MADISON, H. 15.3710) AND HECK PRISE LE, MADISON, H. 15.3710) AND HECK PRISE LE, MADISON, H. 15.3710) FOR SAFETY PRACTICES PRIOR TO PREFORMING THESE FUNCTIONS. UNLESS OTHERNISE INDICATED TO PROPERTY ATTACHED SHALL HAVE A PROPERTY ATTACHED REGION SHALL HAVE PROPERTY ATTACHED REGION SHALL HAVE A PROPERTY ATTACHED REGION Design Crit: TPI-1995 (STD) R3: REVISED DIAPHRAGM NOTE. DLJ 02/27/200G

\*\*IMPORTANT\*\*FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPTHE ENGLICEED
PRODUCTS. THE. SHALL HOT BE RESPONSIBLE FOR ANY DEPLATION FROM THIS DESIGN. ANY FALLURE TO BUILD THE
PRODUCTS. THE SHALL HOT BE RESPONSIBLE FOR ANY DEPLATION, HANDLING, SHIPPHIG. (INSTALLING A BRACTHE OF BRUSSES,
DESIGN CONFURNS WITH APPLICABLE PROVISIONS OF 1005 (INATIONAL DESIGN SPEC, BY AFRA) AND PEL.

CHINECTON PLAIRS AND AND THE APPLICABLE OF 79/18/18/GA, QH, H/S/S), ASTH AGS GRADE GA/GA, VS. STEEL. APPLY

PLAIES TO EACH FACE OF TRUSS AND, HURLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION FOR BRAWHIGS 100A-Z.

ANY HISPECTION OF PLAIES FOLLOWED BY (1) SHALL BE PER ADMEX A 30F PEL-2002 SCC.3.

ASSA. ON HISS

PRAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLIFICATION RESPONSIBILITY SOURT OR HIS HERS COMPOUNT

PROMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGLIFICATION RESPONSIBILITY SOURT OR HIS HERS COMPOUNT

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PROFESSIONAL PROFESSIONAL ENGLIFICATION RESPONSIBILITY SOURT OR HIS HERS COMPOUNT

PROFESSIONAL P

DIAMING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE IRISS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ARY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/IPI I SEC. 2.

Alpine Engineered Products, Inc 1950 Marley Drive

ALPINE

ficate of t 2n # 567

GOUGLAS FLA URIDA SILE DLJ 09/30/2005 R1 REV 2-5-02 JWC HI/-/1/-/-/R/-BC LL BC DL TC DL SPACING DUR.FAC. TC LL TOT.LD. 47.0 30.0 PSF 10.0 PSF 0.0 7.0

PSF

HC-ENG

DLJ/DLJ

DRW HCUSROO1 02086015

PSF

SEQN-

24860

PSF

DATE REF

03/27/02

Scale = .375"/Ft.

R001--

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DETAIL: 140GC

24.0" 1.33 JRFF-1873001 R03

(PIGGYBACK

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

PER THIS DETAIL DESIGNED TO BE USED FOR THE FOLLOWING: 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC.BLDG, CAT II, EXP

С.

TRUSSES BUILT 140 MPH WIND,

T0P CHORD 2x4 SP CHORD 2x4 SP WEBS 2x4 SP SP #2N #3 OR # #2N

(4) 0.131"X1.375" SCOTCH NAILS OR EDUAL IN EACH TO BE APPLIED TO EACH FACE AT 2'0" O.C. MAXIMUM 142 FOR TRULOX INFORMATION. PLATES ON THE FRONT OFFSET FROM THE PLATES ON THE BACK FACE AS LONG 4'0" OC. MAX. OC. MAX. MEMBER. TRULOX PLATE SPACING. REFER TO DRAWING FACE OF TRUSS MAY BE AS PLATES ARE SPACED

NOTE: PIGGYBACK VERTICALS TO BE SPACED AT 4'0" O.C. MAXIMUM

\* MAXIMUM SIZE 9F 2X12, #2 HEM-FIR OR BETTER.

4X6 ALPINE. 3X6 TRULOX AT 2'0" OC. MAX.

+ PIGGYBACY BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH W1.5X3 ALPINE.

3X8 TRULOX PLATE OR ALPINE PIGGYBACK SPECIAL PLATE (SEE DRWG. 847,847)

140 MPH WIND. 30.0 FT MEAN HGT, ASCE 7-02, PART. ENC.BLDG, CAT II, EXP С.

NOTE: THIS DETAIL MAY ALSO BE USED FOR A MONO OR HIP-MONO PIGGYBACK USING A TYPE-C PLATE AT THE HIGH END. AND END VERTICAL WHICH IS GREATER THAN 6-0-0 IN LENGTH AND EXPOSED TO WIND MUST BE VERIFIED BY ALPINE ENGINEERED PRODUCTS.

NOTE: TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

2X4 CONTINUOUS LATERAL BRACING AT OF SUPPORTED TRUSS TOP CHORD WITH 24" OC. MAX SPACING. ATTACH TO TOP 2-16D NAILS IN EACH TRUSS. SIDE

124 CONTINUOUS LATERAL BRACING AT 24" OC. MAX. SPACING. ATTACH TO BOTTOM SIDE OF SUPPORTED TRUSS TOP CHORD WITH 2-16D NAILS IN EACH TRUSS. BOTTOM CHORD OF PIGGYBACK SHOULD REST DIRECTLY ON THE TOP CHORD OF THE SUPPORTED

NOTE: BRACING MATERIAL IS TO BE ATTACHED TO A SUITABLE SUPPORT AT EACH END AND MUST BE #3 HEM-FIR OR BETTER.

TYPE 

7 ' 9" WEB BRACING
7'9"
NO BRACING
TO 12'3"-1X4 "T" BRACE, SAME GRADE
AND LENGTH AS WEB, ATTACH WITH 8D
NAILS AT 6" OC.
TO 14'0"-2X4 "T" BRACE, SAME GRADE
NAUL ENGTH AS WEB, ATTACH WITH 16D
NAILS AT 6" OC.

30'0" W2X4 W5X4 W1X3 SPANS W2X + 0 S W1XX5 W5XX5 W3X4 W5X5 W5X5 W5X5 42.0" W3X5 W5X5 W1.5X3 12'3"

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SPLICE A > > В

12

MAX

3

TγP

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12

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(TYP)

R1: REVISED FOR ASCE 7-02.

TCDL BCDL TOTL

20 20 10 50

R

15 PSF 10 PSF 55 PSF 1.33

TCLL

ALTERNATE

LOADING:

30 PSF

09/30/2005

DETAIL: 140PB

TYP. High Strength, Wave TPI-95

Design Criteria: TPI (STD)

42

-0-0 MAXIMUM PIGGYBACK SPAN

\*\*#ARNING\*\* RUSSIS REQUIRE EXTREME CARE IN FABRICATION, HABBLING, SHIPPING, INSTALLING AND BRACING. RETER TO BEST I OJ (BUILDING COMPONENT SACTIVE INFORMATION), PUBLISHED BY FPI (TRUSS PLATE HISTITUTE, 583 D'UNDERFO BR., SUITE ZOO, HADISON, HI 53710) AND WITCA (MODO BRUSS COUNCEL O AMERICA, STODE ENTERPRISE LH, MADISON, HI 53710) FOR SACELY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNICESS OTHERNISE INDICATED, TOP CHORD SHALL HAVE A PROPERLY ATTACHED REGION CELLING.

\*\*IMPORTANT\*\*NURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.

ALPTHE ENGINEERLD PRODUCTS, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION ROOM THIS DESIGN: ANY FALLURE TO BUILD THE BROSS IN CONTRACTANCE WITH PIT:

BUSSIGN CONTRACTS IN CONTRACTS ARE AND TO THE PIT OF TABLECT AND THE PIT.

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Alpine Engineered Products, Inc 1950 Marley Drive

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> -/-/R/-30.0 7.0 PSF PSF PSF DATE REF HC-ENG DRW HCUSR001 02086006 R001--DLJ/DLJ 03/27/02 0

TOT.LD. SPACING DUR.FAC. 47. 24.0" 10.0 1.33 0.0 0 PSF PSF JRFF-SEQN ISQVOOT 24938

