ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID: ITED8228Z0222102934

Truss Fabricator:	Anderson Truss Company
Job Identification:	7-278--GARY JOHNSON Shoup -- , **
Truss Count:	1
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.36.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 01/22/2008

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-

#	Ref	Description	Drawing#	Date
1	33303--FT1		08022001	01/22/08

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 60 PLF at 0.00 to 60 PLF at 9.63
BC - From 20 PLF at 0.00 to 20 PLF at 9.63
BC - 474 LB Conc. Load at 1.92, 3.92, 5.92, 7.92

Wind reactions based on MWFRS pressures.

The TC of this truss shall be braced with attached spans at 24" OC in lieu of structural sheathing.

THE BUILDING DESIGNER SHALL EVALUATE AND APPROVE LOAD MAGNITUDES AND LOCATIONS AS SHOWN ("SPECIAL LOADS").TRUSS ENGINEER & FABRICATOR ARE NOT RESPONSIBLE FOR LOAD MAGNITUDES AND LOCATIONS.

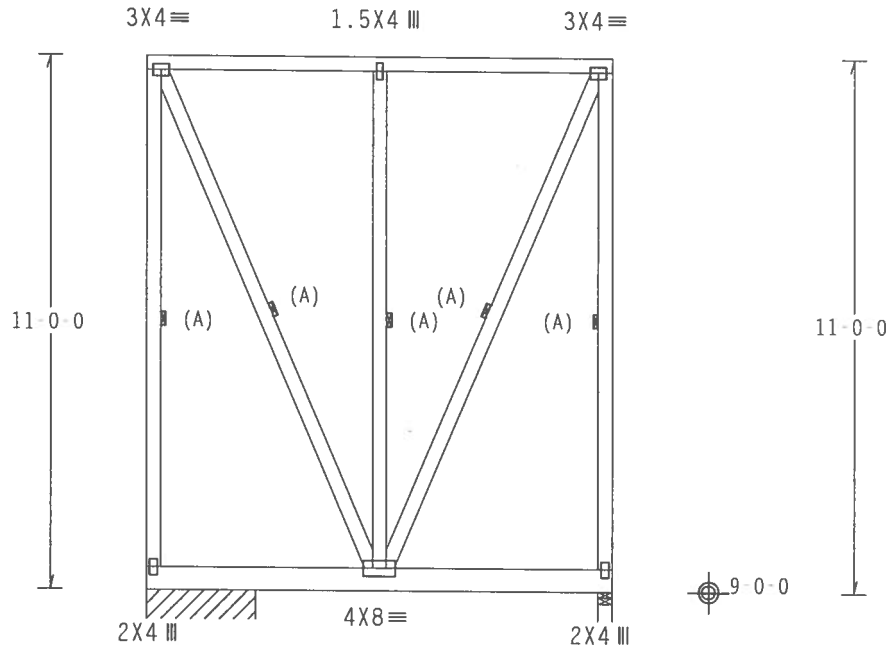
110 mph wind, 20.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.25 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/)=0.18

End verticals not exposed to wind pressure.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Truss must be installed as shown with top chord up.



R=650 PLF U=425 PLF W=2-3-0

R=1203 U=777 W=3.5"

9-7-8 Over 2 Supports

PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.042

QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 22 '08

TC LL	20.0 PSF	REF R8228- 33303
TC DL	10.0 PSF	DATE 01/22/08
BC DL	10.0 PSF	DRW HCUSR8228 08022001
BC LL	0.0 PSF	HC-ENG JB/AP
TOT.LD.	40.0 PSF	SEQN- 27942
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TED8228Z02

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

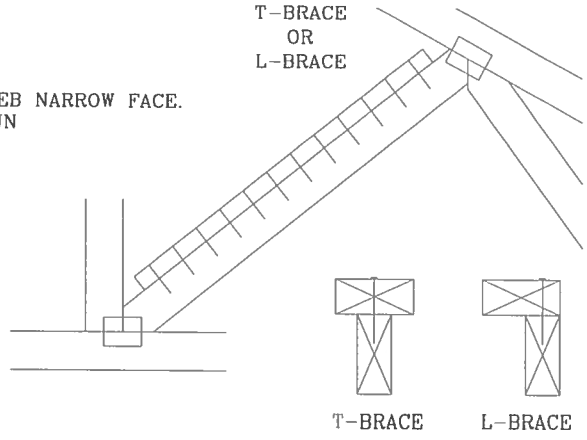
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

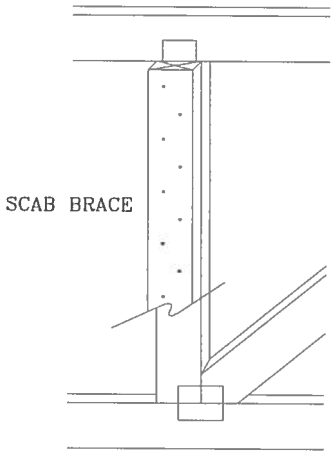
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 22 08

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

26316

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:ITB28228Z0126145201

Truss Fabricator:	Anderson Truss Company
Job Identification:	7-278--GARY JOHNSON GARY JOHNSON / SHOUP -- , **
Truss Count:	28
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Versions 7.36, 7.37.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed

Seal Date: 09/26/2007

-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

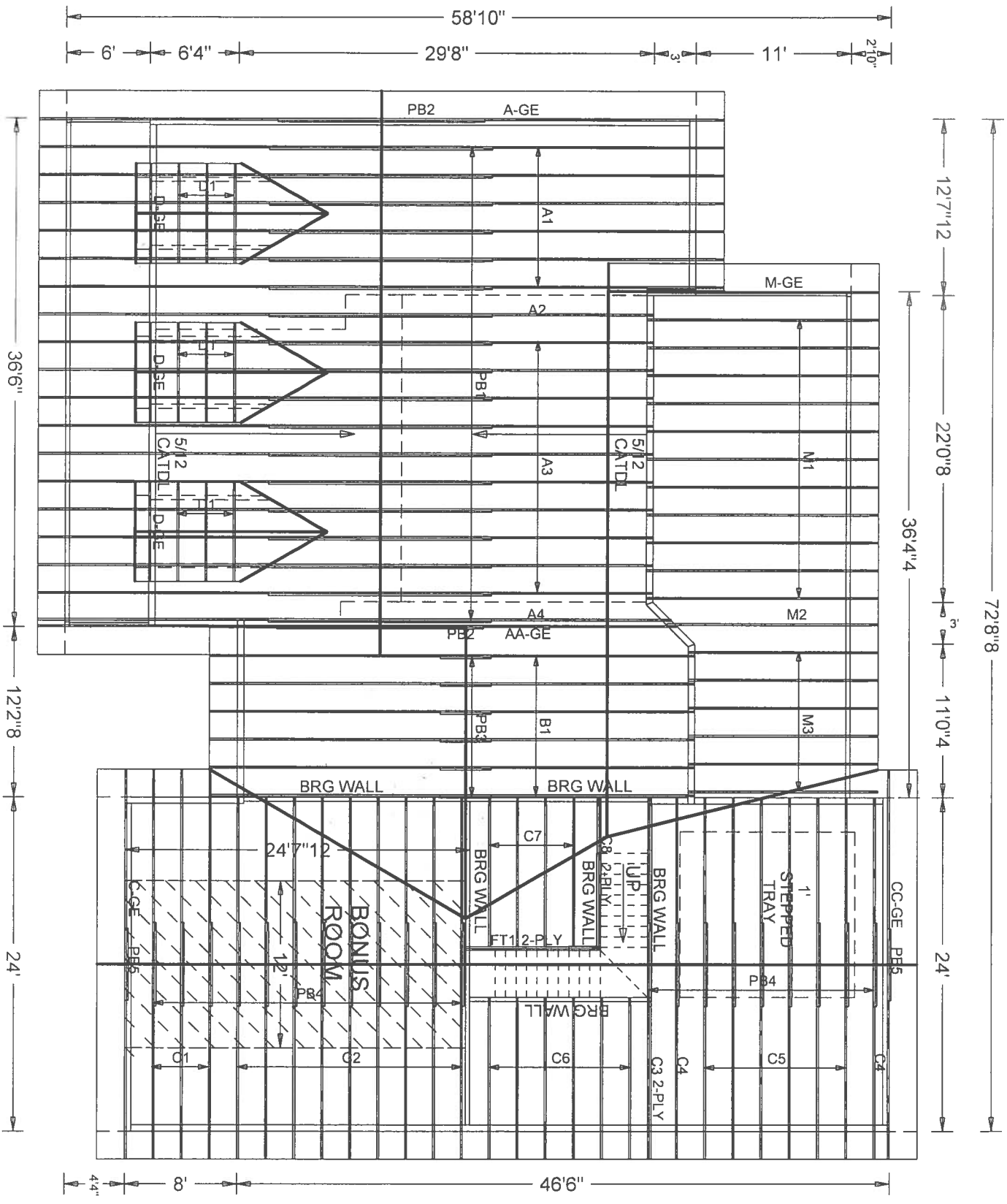
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: BRCLBSUB-140GC-CNBRGBLK-A11015EE-GBLLETIN-A11015EC-140PB-PIGBACKA-PIGBACKB-A11030EE-140GS-A10030EC-GBLBR

#	Ref	Description	Drawing#	Date
1	54791	--A1	07269021	09/26/07
2	54792	--A-GE	07269022	09/26/07
3	54793	--A3	07269023	09/26/07
4	54794	--A2	07269006	09/26/07
5	54795	--A4	07269007	09/26/07
6	54797	--B1	07269009	09/26/07
7	54798	--C5	07269001	09/26/07
8	54799	--C4	07269010	09/26/07
9	54800	--CC-GE	07269028	09/26/07
10	54801	--C3	07269011	09/26/07
11	54802	--C6	07269012	09/26/07
12	54803	--C7	07269013	09/26/07
13	54804	--C8	07269014	09/26/07
14	54805	--C2	07269015	09/26/07
15	54806	--C1	07269016	09/26/07
16	54807	--C-GE	07269029	09/26/07
17	54808	--FT1	07269024	09/26/07
18	54809	--M-GE	07269005	09/26/07
19	54810	--M1	07269002	09/26/07
20	54811	--M2	07269003	09/26/07
21	54812	--M3	07269004	09/26/07
22	54813	--PB4	07269017	09/26/07
23	54814	--PB5	07269025	09/26/07
24	54815	--PB3	07269018	09/26/07
25	54816	--PB1	07269019	09/26/07
26	54817	--PB2	07269026	09/26/07
27	54818	--D1	07269020	09/26/07
28	54819	--D-GE	07269027	09/26/07

#7-278 Gary Johnson / Shoup
9/26/07



JOB DESCRIPTION:: GARY JOHNSON
/: Shoup

JOB NO: 7-278

PAGE NO 1 OF 1

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC	From	63 PLF	at	2.00	to	63 PLF at 6.49
TC	From	63 PLF	at	6.49	to	63 PLF at 14.50
TC	From	63 PLF	at	14.50	to	63 PLF at 30.50
TC	From	63 PLF	at	30.50	to	63 PLF at 32.42
TC	From	219 PLF	at	32.42	to	282 PLF at 38.51
TC	From	282 PLF	at	38.51	to	287 PLF at 39.00
TC	From	63 PLF	at	39.00	to	63 PLF at 47.00
BC	From	5 PLF	at	2.00	to	5 PLF at 0.00
BC	From	20 PLF	at	0.00	to	20 PLF at 16.00
BC	From	20 PLF	at	16.00	to	20 PLF at 29.00
BC	From	20 PLF	at	29.00	to	20 PLF at 45.00
BC	From	5 PLF	at	45.00	to	5 PLF at 47.00
TC	138 LB Conc. Load at 39.00					

Wind reactions based on MWFRS pressures.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

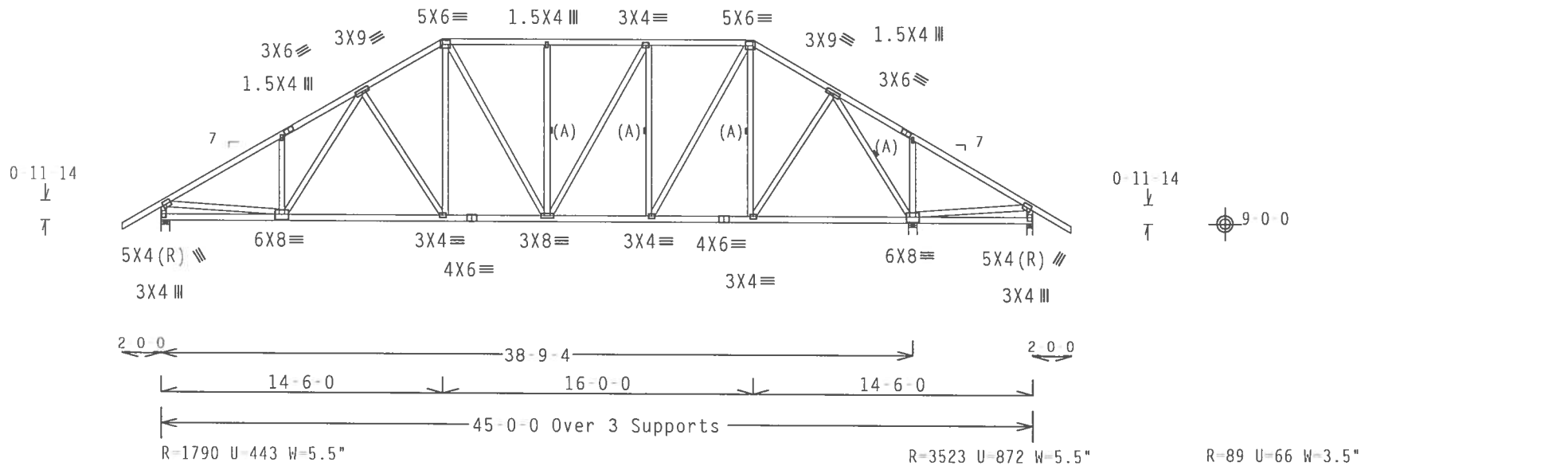
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/ SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave
Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

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ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228-54791
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269021
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51561
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Lt Stubbed Wedge 2x4 SP #3::Rt Stubbed Wedge 2x4 SP #3:

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.

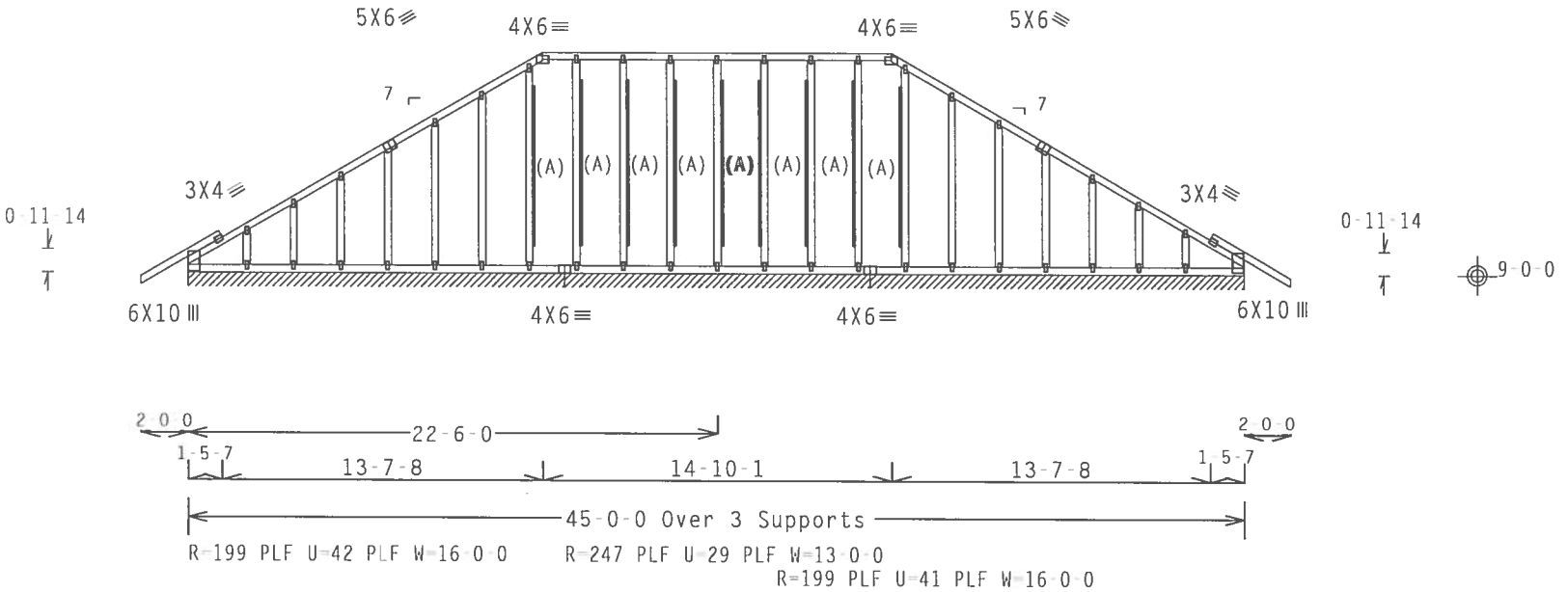
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

(A) 1x4 #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

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Haines City, FL 33844
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep 26 '07

TC LL	20.0 PSF	REF	R8228-54792
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269022
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51520
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP , ** A3)

Top chord 2x4 SP #2 Dense :T4 2x6 SP #1 Dense:
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Rt Slider 2x6 SP #2: BLOCK LENGTH = 1.500'

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	63 PLF	at 0.00	to	63 PLF	at 11.50
TC - From	63 PLF	at 11.50	to	63 PLF	at 27.50
TC - From	63 PLF	at 27.50	to	63 PLF	at 29.42
TC - From	219 PLF	at 29.42	to	265 PLF	at 33.87
TC - From	265 PLF	at 33.87	to	287 PLF	at 36.00
TC - From	63 PLF	at 36.00	to	63 PLF	at 44.00
BC - From	22 PLF	at 0.00	to	22 PLF	at 8.88
BC - From	22 PLF	at 8.88	to	22 PLF	at 18.00
BC - From	22 PLF	at 18.00	to	22 PLF	at 27.12
BC - From	22 PLF	at 27.12	to	22 PLF	at 35.54
BC - From	20 PLF	at 35.54	to	20 PLF	at 42.00
BC - From	5 PLF	at 42.00	to	5 PLF	at 44.00

TC - 138 LB Conc. Load at 36.00

Wind reactions based on MWFRS pressures.

Left end vertical not exposed to wind pressure.

** NEGATIVE REACTION OF 2064 FROM A NON-WIND LOAD CASE REQUIRES UPLIFTCONNECTION. BUILDING DESIGNER MUST APPROVE AND PROVIDE CONNECTION.

Bearing blocks: Nail type: 12d_Common_(0.148"x3.25"_min.)_nails

BRG	X-LOC	#BLOCKS	LENGTH/BLK	#NAILS/BLK	WALL PLATE
2	35.542'	1	12"	5	Rigid Surface

Bearing block to be same size and species as bottom chord.
Refer to drawing CNBRGBLK0207 for additional information.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Calculated horizontal deflection is 0.25" due to live load and 0.45" due to dead load.

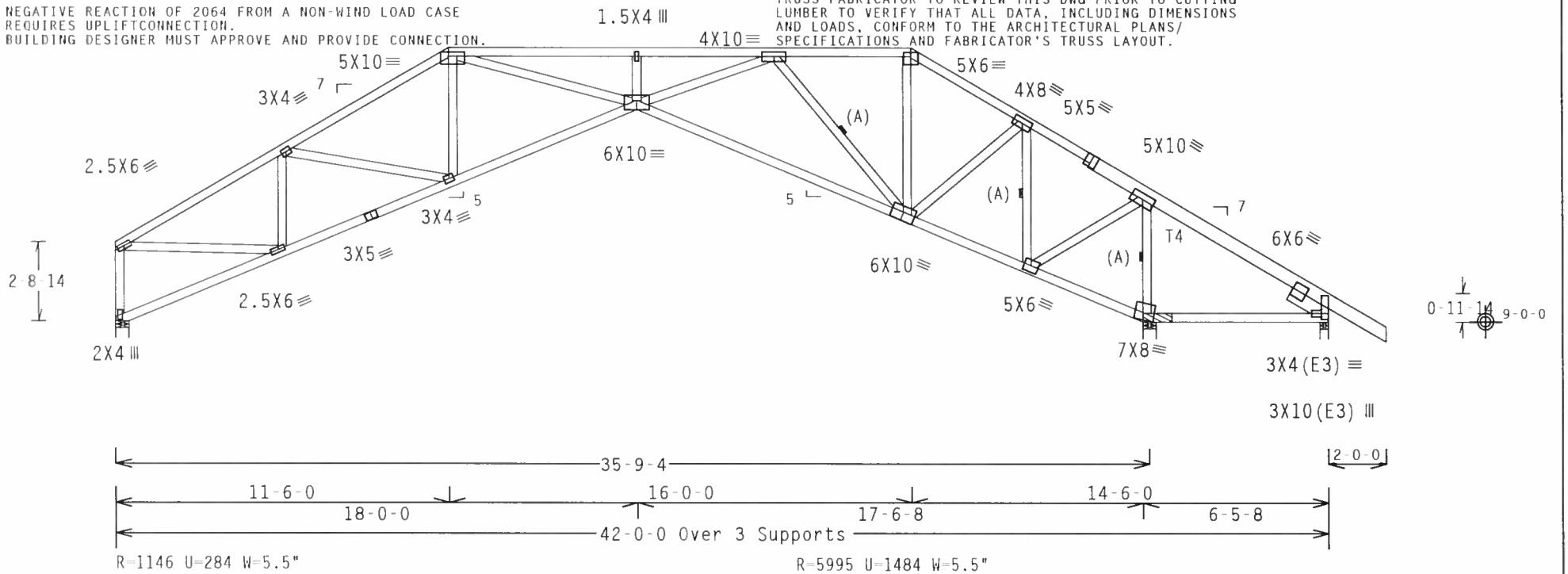
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Shim all supports to solid bearing.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER AND TRUSS FABRICATOR TO REVIEW THIS DWG PRIOR TO CUTTING LUMBER TO VERIFY THAT ALL DATA, INCLUDING DIMENSIONS AND LOADS, CONFORM TO THE ARCHITECTURAL PLANS/ SPECIFICATIONS AND FABRICATOR'S TRUSS LAYOUT.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 667

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 54793
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269023
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51609
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

(7-278 GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - A2)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :W16 2x4 SP #2 Dense:

Left end vertical not exposed to wind pressure.

Calculated horizontal deflection is 0.26" due to live load and 0.43" due to dead load.

(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

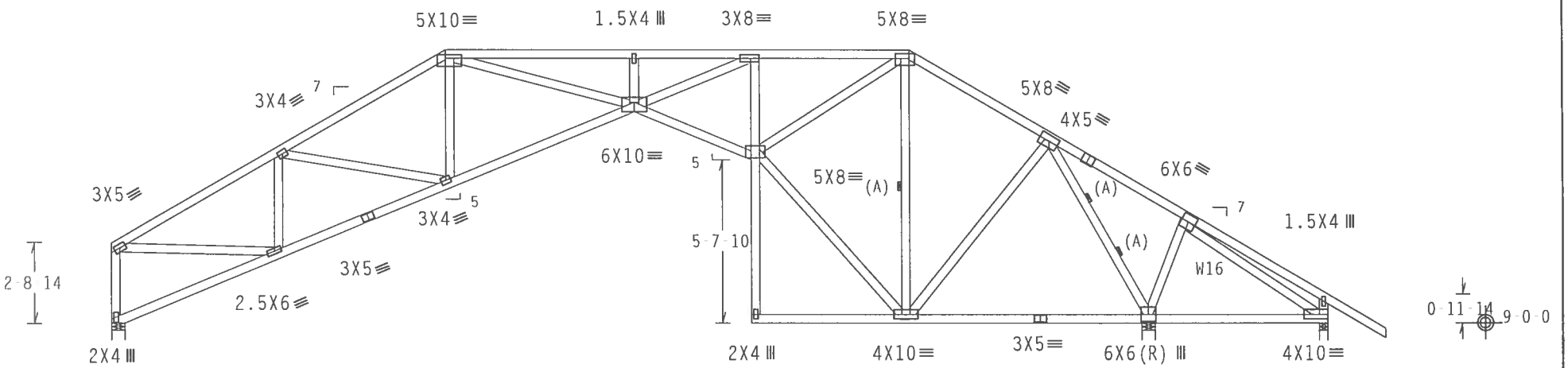
** NEGATIVE REACTION OF 1718 FROM A NON-WIND LOAD CASE REQUIRES UPLIFTCONNECTION.
BUILDING DESIGNER MUST APPROVE AND PROVIDE CONNECTION.

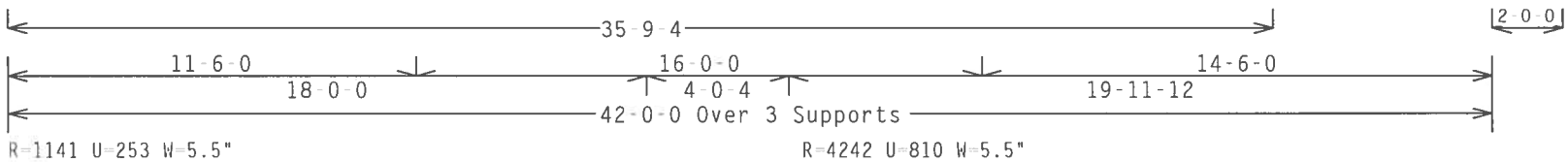
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Shim all supports to solid bearing.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 54794
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269006
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51551
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Calculated horizontal deflection is 0.11" due to live load and 0.17" due to dead load.

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

(C) 2x6 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

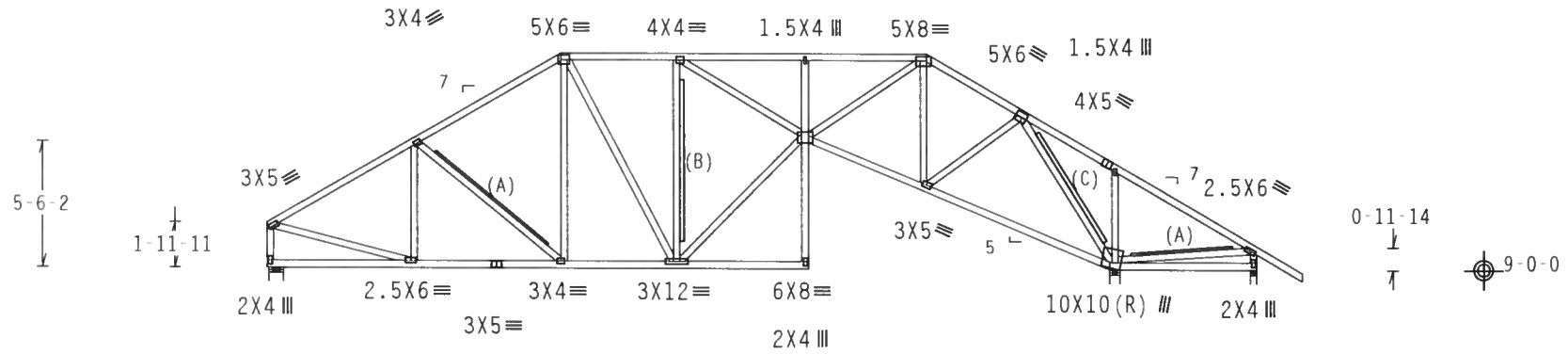
Negative reaction(s) of -792# MAX. (See below) from a non-wind load case requires uplift connection.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

(B) 2x4 #3 or better "T" brace. 80% length of web member. Attach with 16d Box or Gun (0.135"x3.5",min.)nails @ 6" OC.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.04

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 667

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF R8228- 54795
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269007
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 51636
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TB28228Z01

(7-278 GARY JOHNSON GARY JOHNSON / SHOUP - - , ** - B1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

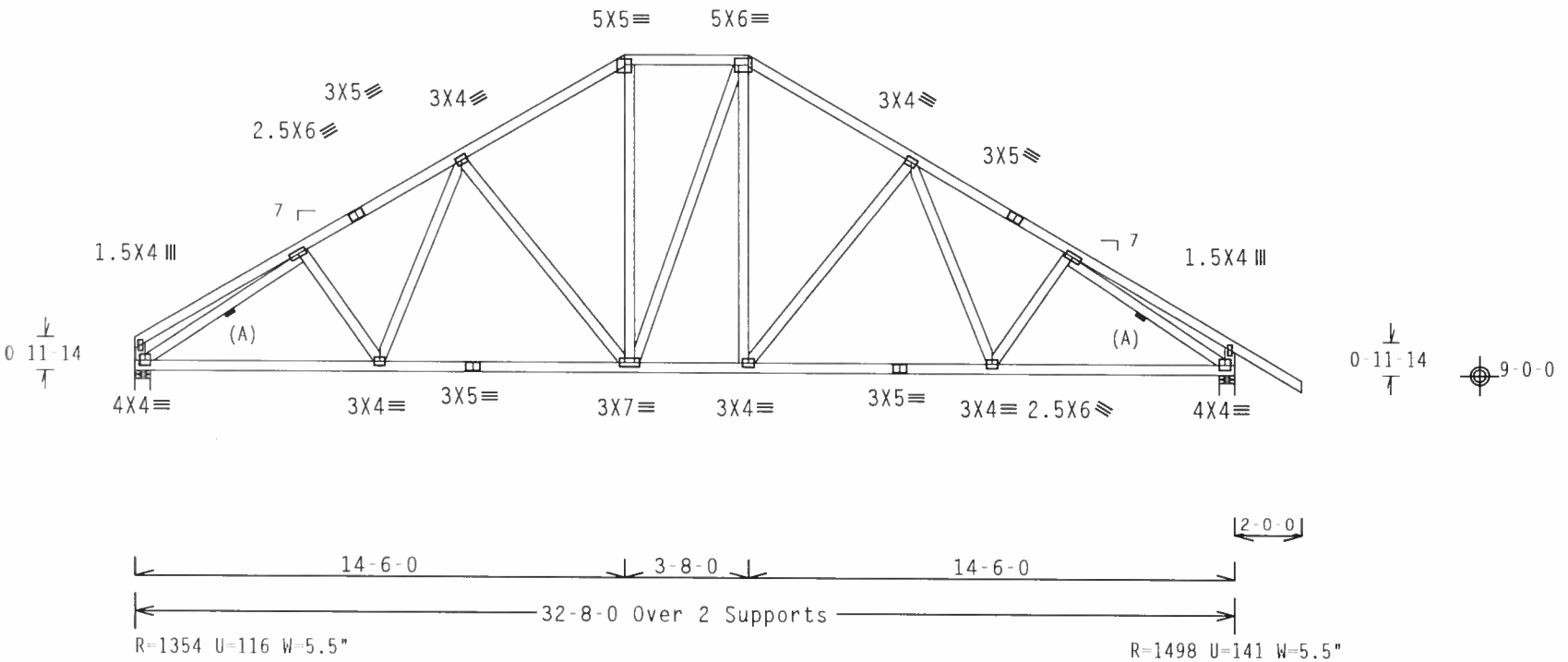
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.042

QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

WARNING TRU SES REQUIRE EXTREME CA IN FABRI AT ON, HANDLING, SH PPING, INSTALLING AND BRACING E B (B DING COMPONENT AFE Y INFORMATION), PUBLISHED BY TPI (IR NSTITUTE, 218 N TH FE ST EET, TE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS UNCIL OF AMERICA, 6300 ENTERPRI E L NE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS UNLESS OTHERWI E INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROP RL ATTACHED RIGID CEILING

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRA TOR. TH BCG, N , SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SH PPING, INSTALLING & BRACING OF TRUSSES.
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ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 54797
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269009
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51623
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB2822RZ01

(7-278 GARY JOHNSON GARY JOHNSON / SHOUP -- . ** - C5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

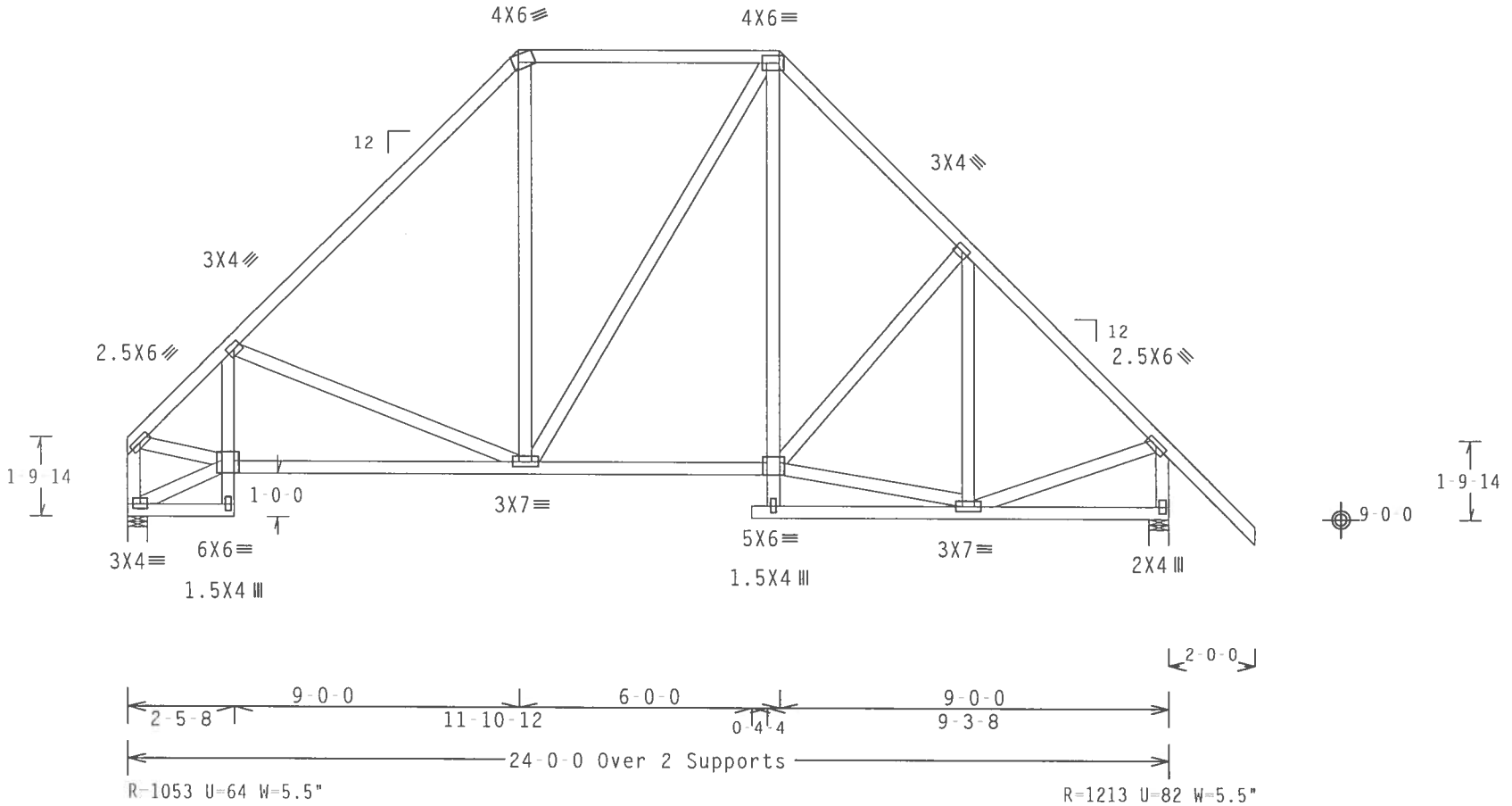
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.04

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Sep 26 '07

TC LL	20.0 PSF	REF R8228- 54798
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269001
BC LL	0.0 PSF	HC-ENG JB/DF *
TOT.LD.	40.0 PSF	SEQN- 51327
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JRFF- 1TB28228Z01

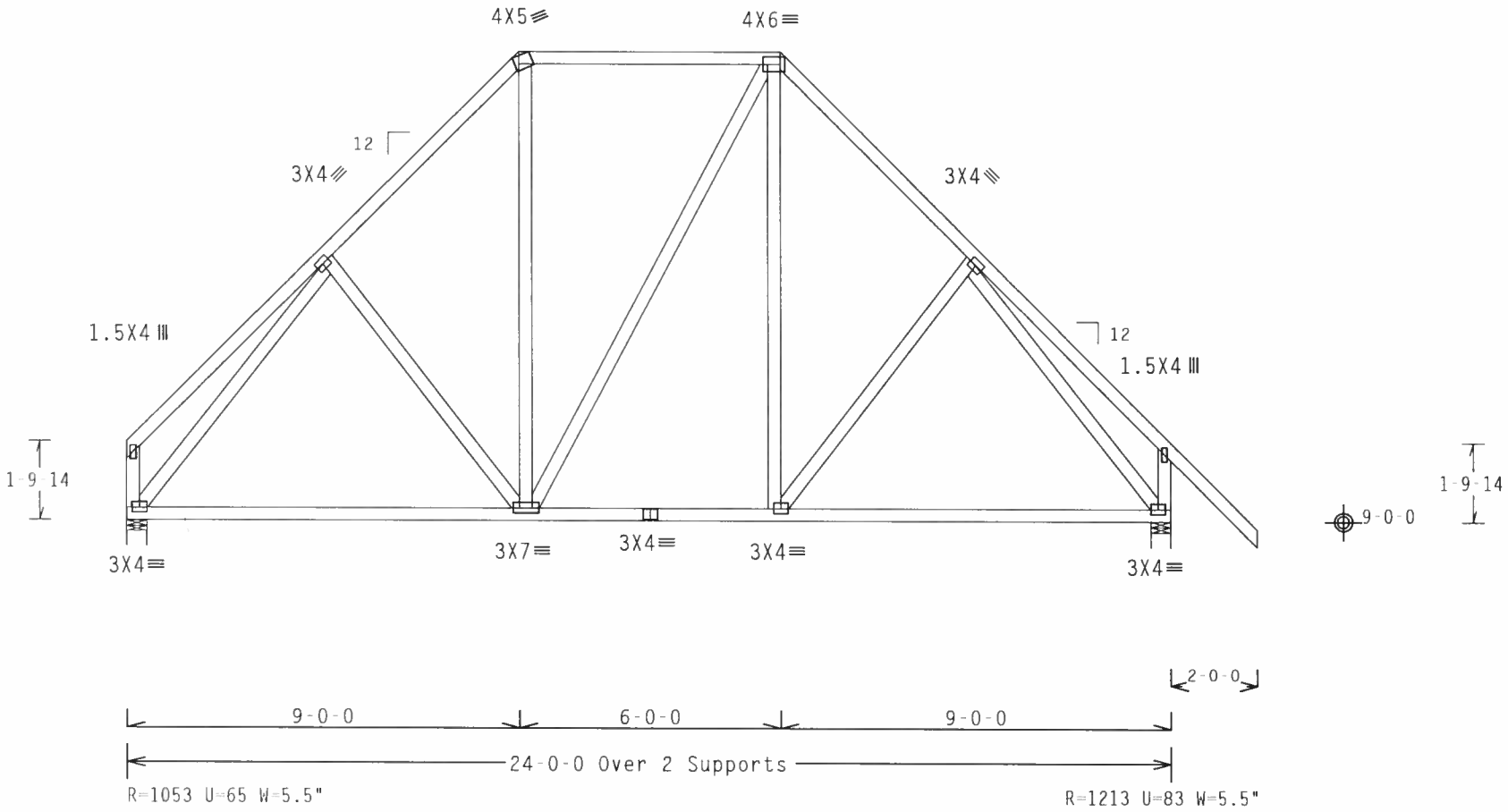
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF R8228- 54799
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269010
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 51339
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TB28228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3 :M1, M13 2x4 SP #2 Dense:

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

See DWGS A11015EE0207 & GBLLETIN0207 for more requirements.

(A) 1x4 #3 or better "L" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

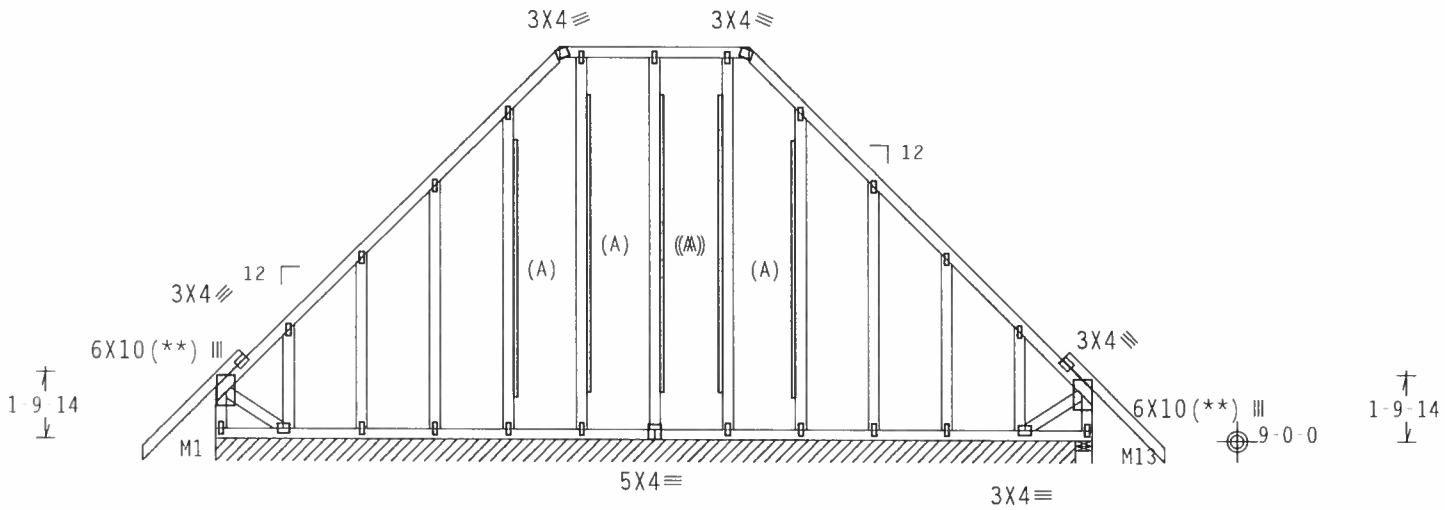
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

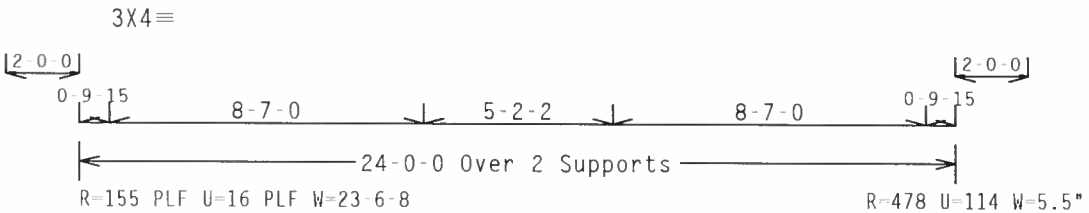
Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.





Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.37.05 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep 26 '07

TC LL	20.0 PSF	REF	R8228- 54800
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269028
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	2467 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.25 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

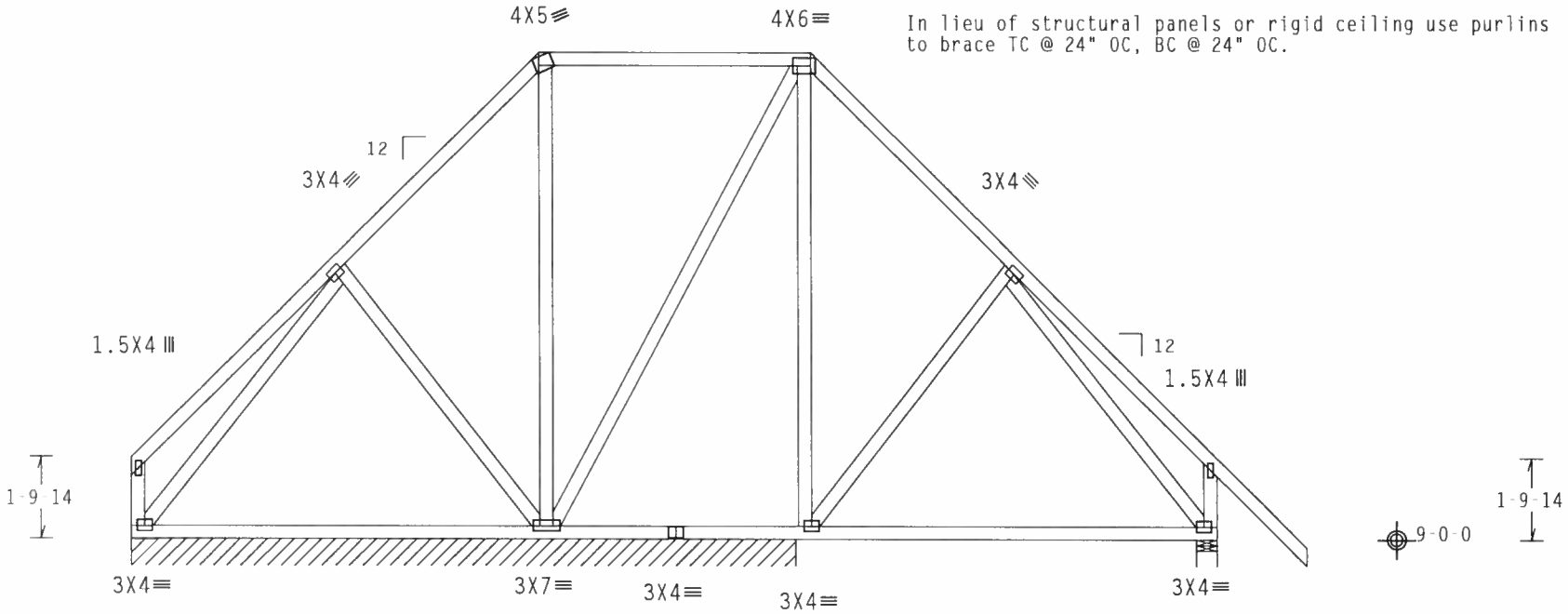
Trusses to be spaced at 34.5" OC maximum.

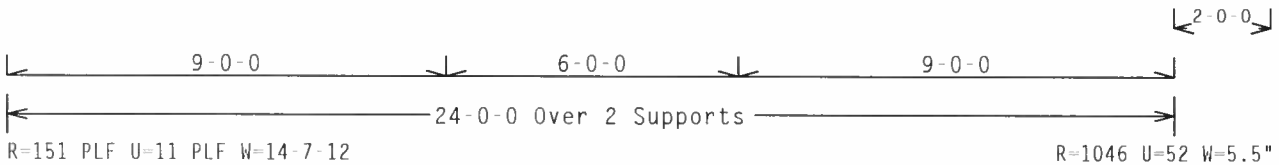
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.





Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #667

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF R8228- 54801
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269011
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 51367
DUR.FAC.	1.25	FROM AH
SPACING	34.5"	JREF- 1TB28228Z01

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP , ** C6)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

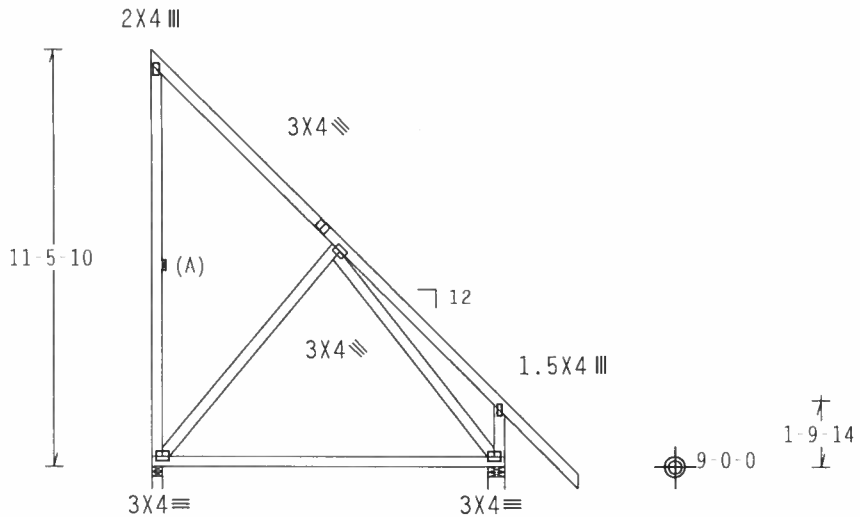
Left end vertical not exposed to wind pressure.

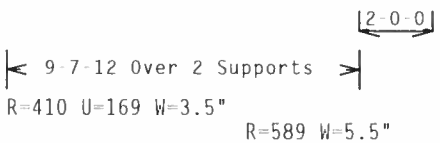
(A) Continuous lateral bracing equally spaced on member.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.04[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

PLT TYP. Wave

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #5[illegible]

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
[illegible] 26 '07

TC LL	20.0 PSF	REF	R8228- 54802
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269012
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51373
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

(7-278 GARY JOHNSON GARY JOHNSON / SHOUP , ** C7)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

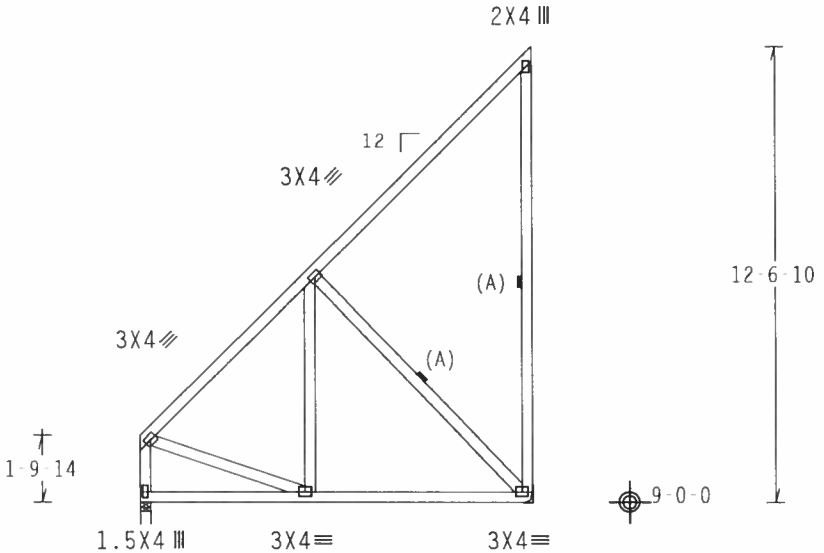
(A) Continuous lateral bracing equally spaced on member.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 16.19 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



10-8-12 Over 2 Supports

R=474 W=3.5" R=474 U=199

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

WARNING TRUSSES REQU RE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTAL ING AND BRACING REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY PI (TRUSS PLATE NSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 EN ERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 54803
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269013
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51393
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB28228Z01

(7-278--GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - C8)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 16.19 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 6.25 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

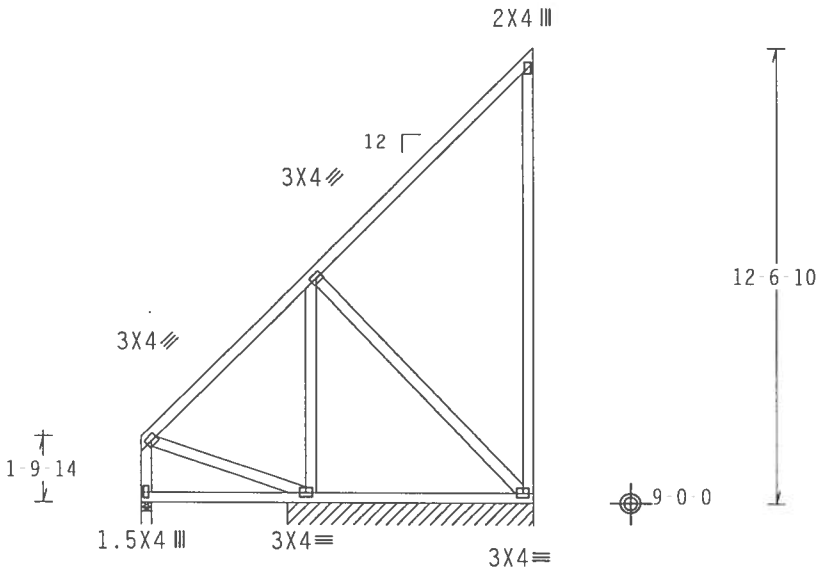
In lieu of structural panels or rigid ceiling use purlins to brace TC @ 24" OC, BC @ 24" OC.

2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d Common (0.148"x3.25", min.) nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Right end vertical not exposed to wind pressure.

Trusses to be spaced at 34.5" OC maximum.



10-8-12 Over 2 Supports

R=352 W=3.5"
R=150 PLF U=46 PLF W=6-8-12

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
9/26 '07

TC LL	20.0 PSF	REF	R8228-54804
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269014
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51399
DUR.FAC.	1.25	FROM	AH
SPACING	34.5"	JREF-	1TB28228701

(7-278 GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - C2)

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

Calculated horizontal deflection is 0.11" due to live load and 0.21" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 18-0-0.

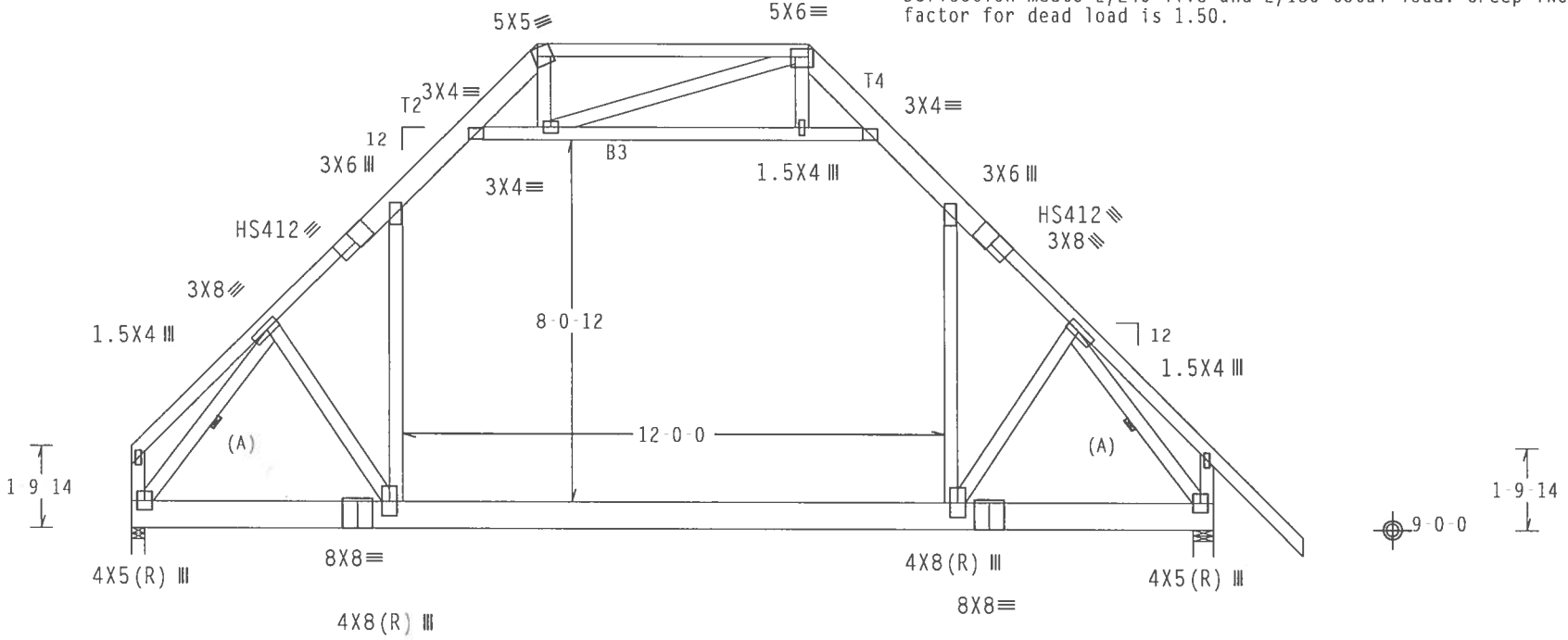
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

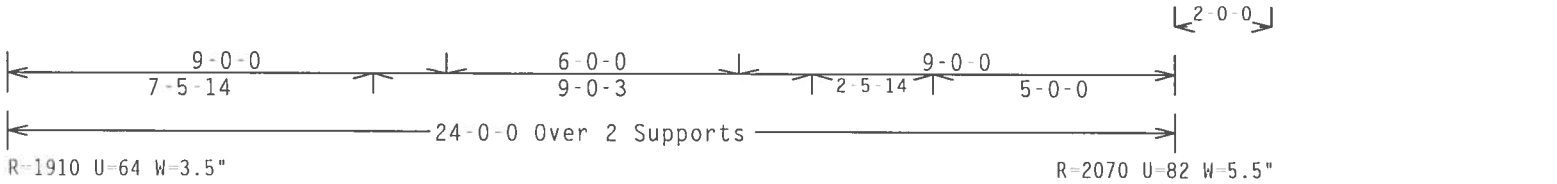
Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF R8228- 54805
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269015
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 51423
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JRFF- 1TB28228Z01

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - C1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY

Top chord 2x4 SP #2 Dense :T2, T4 2x6 SP #1 Dense:
Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

Calculated horizontal deflection is 0.11" due to live load and 0.20" due to dead load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 18-0-0.

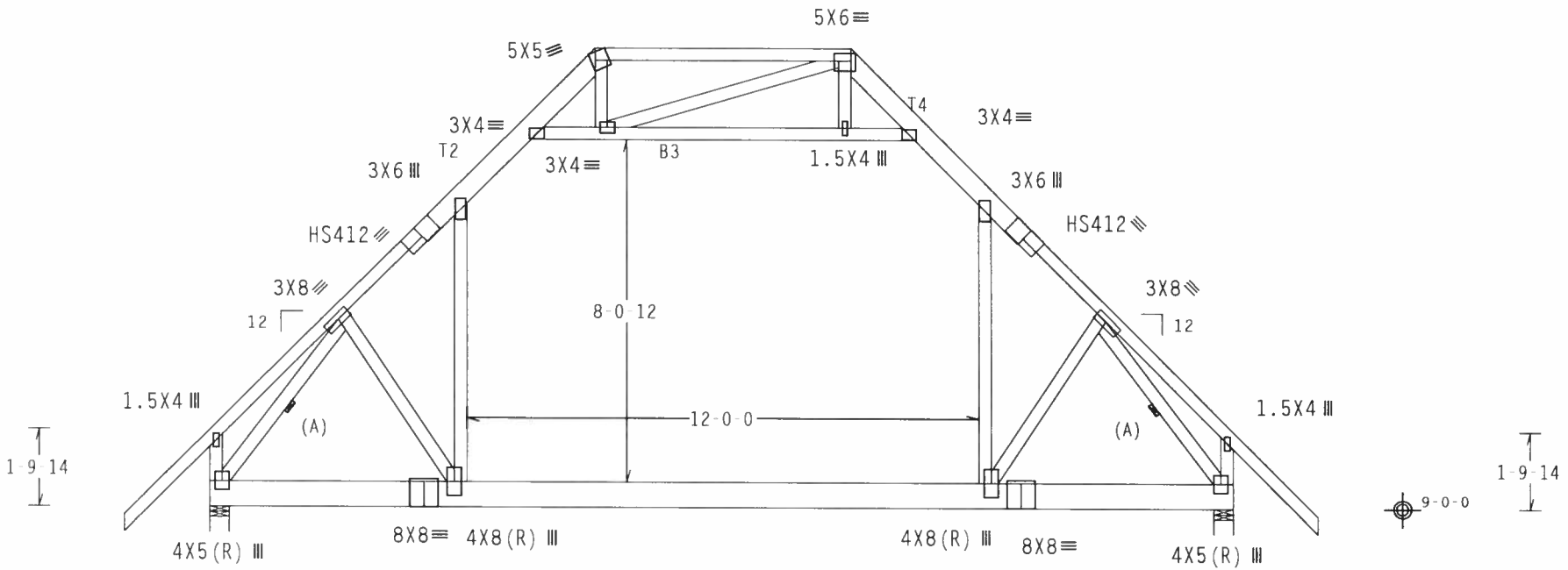
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

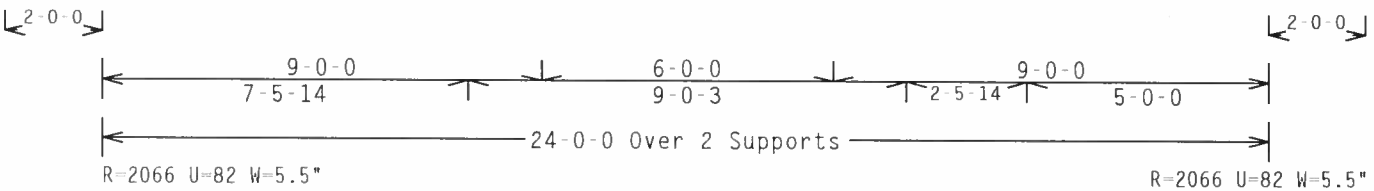
Wind reactions based on MWFRS pressures.

(A) Continuous lateral bracing equally spaced on member.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.





Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.043 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
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WARNING TRUSSES REQU REME IN F BRI A ION. HAND NG. SHIPPING. INSTALLING AND BRACING. REFER TO BCSI (BUILDING MP N N I Y INF RM N. P B I Y TPI .. P A N. . 8 NOR H TEE STREET, SUITE 312, A XANDR A, VA, 14 N HT W R N ME . 6 ENTERPRISE LANE, MADISON, WI 19 F R A Y 'RA R CR O M NG I E FINC ION UNLE OTHERWISE INDICATED TOP CHORD SH L H VE P P R Y A H I A PANE N BO JM H H HAV A PROPERLY ATTACHED RIGID CEILING

IMPORTANT FURNISH A COPY OF TH S DE IGN THE INSTALLATION CONT TOR ITH BCG, INC HA N I RE PONSIBLE FOR ANY DEVIATION FROM TH DE IGN, ANY FAILURE TO BUILD THE RUS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISION. OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H,H/SS/K) ASTM A653 GRADE 40/60 (H, K/H,SS) GALV. STEE . APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1 2002 SEC 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF R8228- 54806
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269016
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 51430
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TB28228Z01

Top chord 2x4 SP #2 Dense :T3, T5 2x6 SP #1 Dense:
Bot chord 2x8 SP #1 Dense :B3 2x4 SP #2 Dense:
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.

See DWGS A11015EC0207 & GBLLETIN0207 for more requirements.

(**) 2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

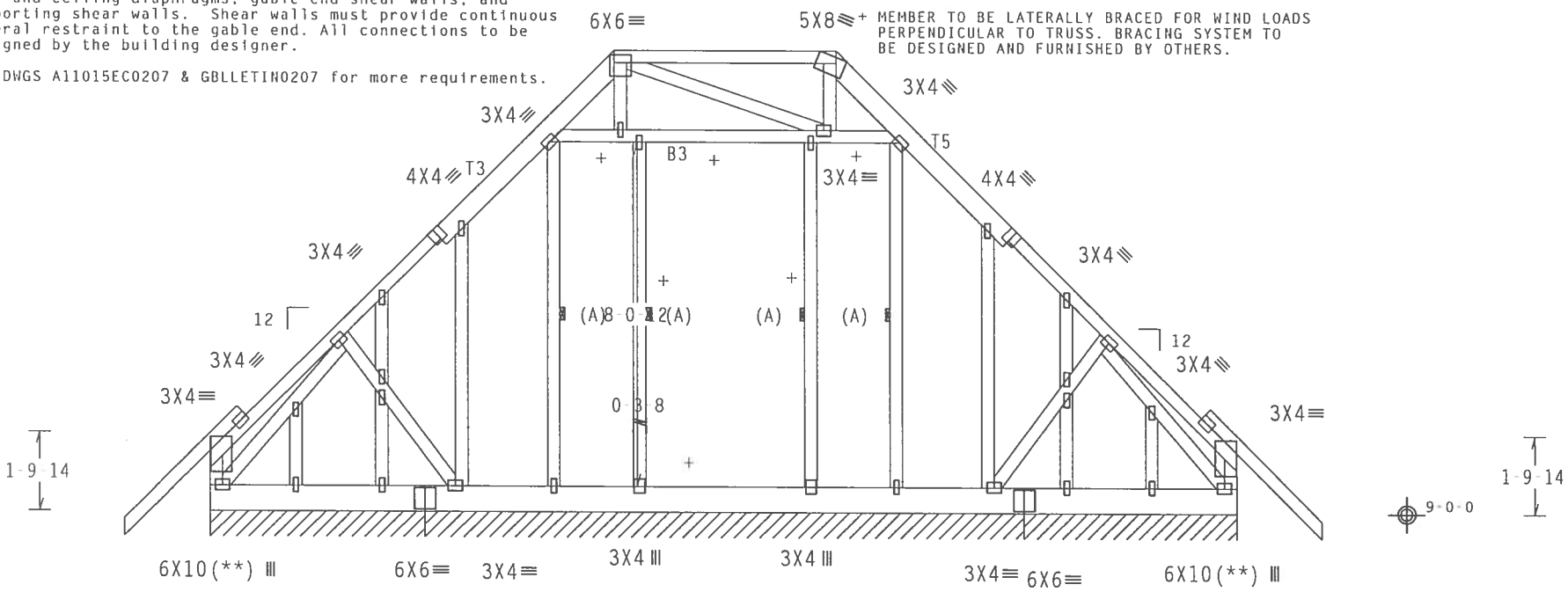
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

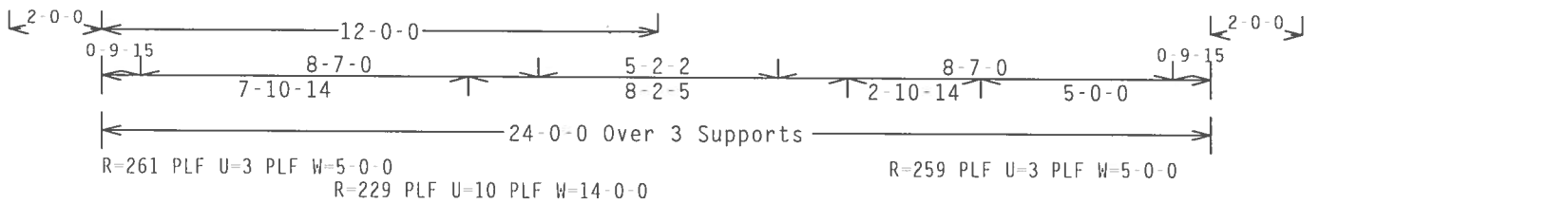
(A) Continuous lateral bracing equally spaced on member.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

BC attic room floor loading: LL = 40.00 psf; DL = 10.00 psf; from 6-0-0 to 18-0-0.

+ MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.





Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.37.052[illegible] QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #[illegible]

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H,H/SS/K) ASTM A653 GRADE 40/60 (H, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPII-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF	R8228- 54807
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269029
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	2471 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB28228Z01

(7-278--GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - FT1)

Top chord 2x4 SP #2 Dense
Bot chord 2x6 SP #2
Webs 2x4 SP #3

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 60 PLF at 0.00 to 60 PLF at 9.63
BC - From 20 PLF at 0.00 to 20 PLF at 9.63
BC - 474 LB Conc. Load at 1.92, 3.92, 5.92, 7.92

Wind reactions based on MWFRS pressures.

End verticals not exposed to wind pressure.

Truss must be installed as shown with top chord up.

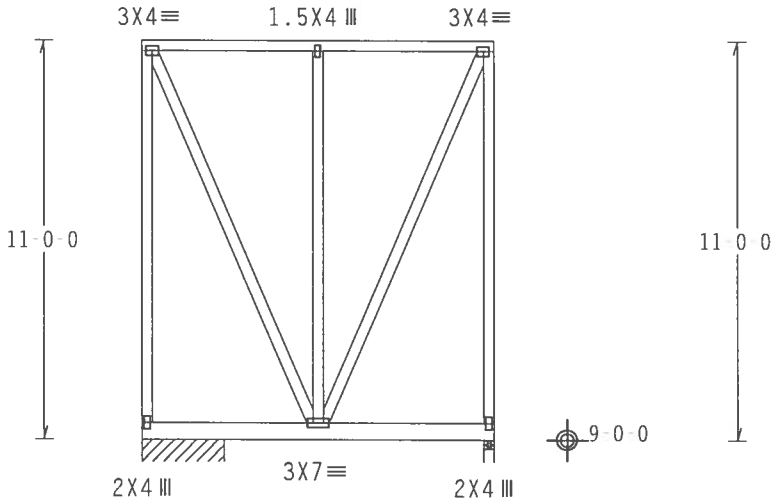
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @ 9.25" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails
in each row to avoid splitting.

110 mph wind, 20.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not
located within 6.25 ft from roof edge, CAT II, EXP B, wind TC
DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Deflection meets L/240 live and L/180 total load. Creep increase
factor for dead load is 1.50.

The TC of this truss shall be braced with attached spans at 24"
OC in lieu of structural sheathing.



9-7-8 Over 2 Supports
R=650 PLF U=425 PLF W=2-3-0
R=1203 U=778 W=3.5"

Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Sep 26 '07

TC LL	20.0 PSF	REF R8228- 54808
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269024
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 51410
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JRFF- 1TB28228Z01

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP -- , ** M GE)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3
:Stack Chord SC1 2x4 SP #2 Dense:

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

Stacked top chord must NOT be notched or cut in area (NNL). Dropped top chord braced at 24" o.c. intervals. Attach stacked top chord (SC) to dropped top chord in notchable area using 3x4 tie plates 24" o.c. Center plate on stacked/dropped chord interface, plate length perpendicular to chord length. Splice top chord in notchable area using 3x6.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

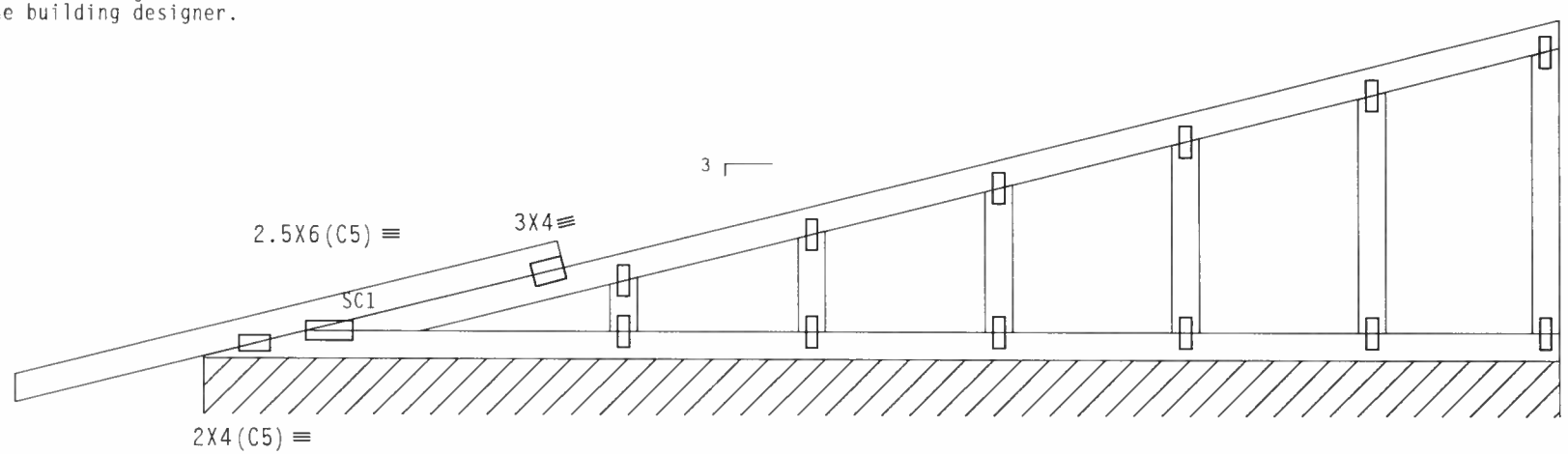
Wind reactions based on MWFRS pressures.

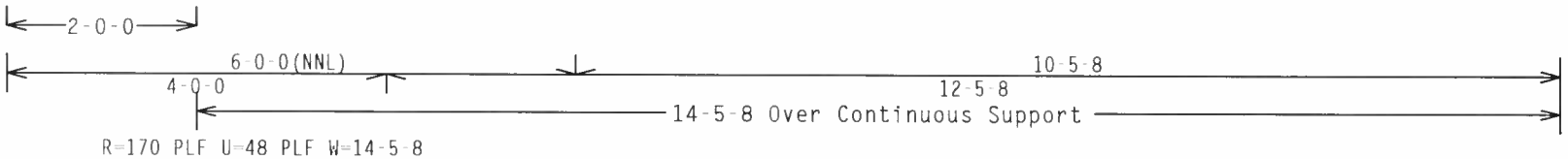
Right end vertical not exposed to wind pressure.

In lieu of structural panels use purlins to brace TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SEE DRW HCUSR001 02086015 FOR GABLE DETAILS.





Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.043 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF	R8228- 54809
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269005
BC LL	0.0 PSF	HC-ENG	JB/DF *
TOT.LD.	40.0 PSF	SEQN-	51454
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB2822RZ01

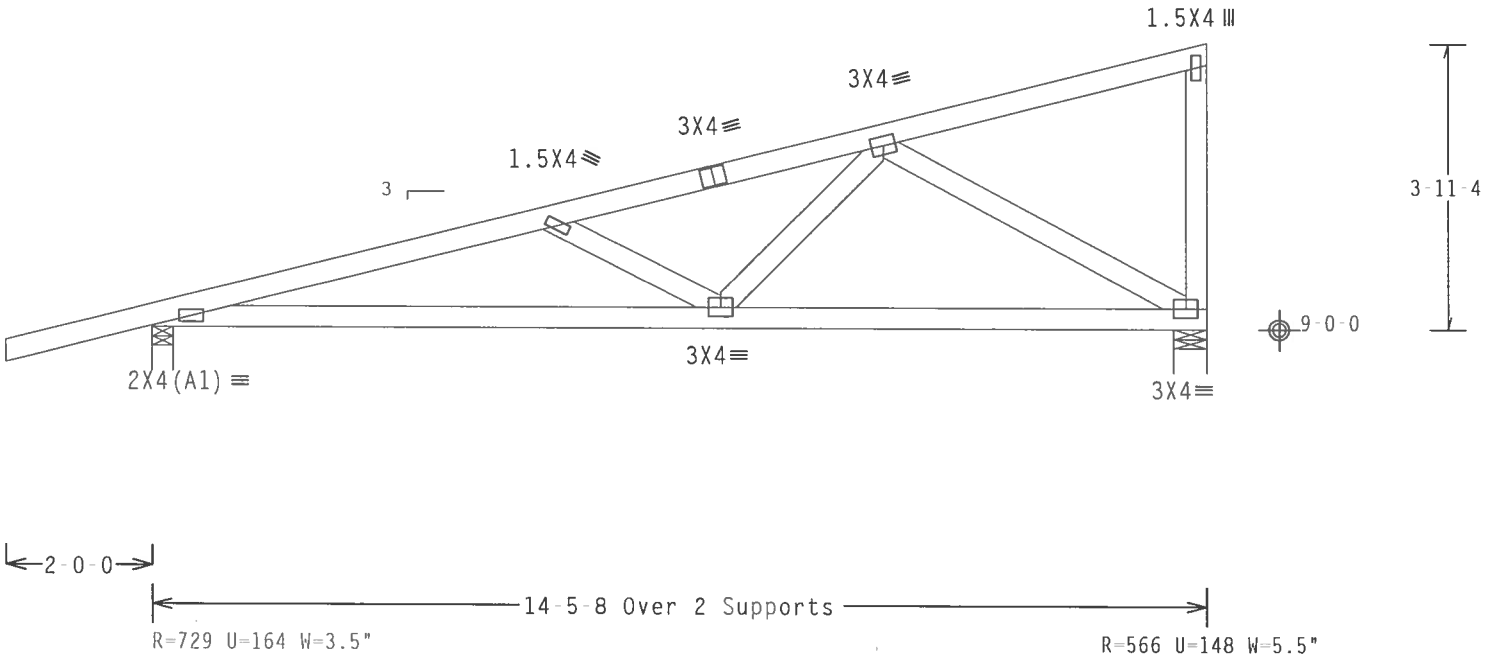
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424

QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

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ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorizatio- # 667

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
9/26 '07

TC LL	20.0 PSF	REF R8228- 54810
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269002
BC LL	0.0 PSF	HC-ENG JB/DF *
TOT.LD.	40.0 PSF	SEQN- 51459
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TB28228Z01

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP , ** M2)

THIS DRG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

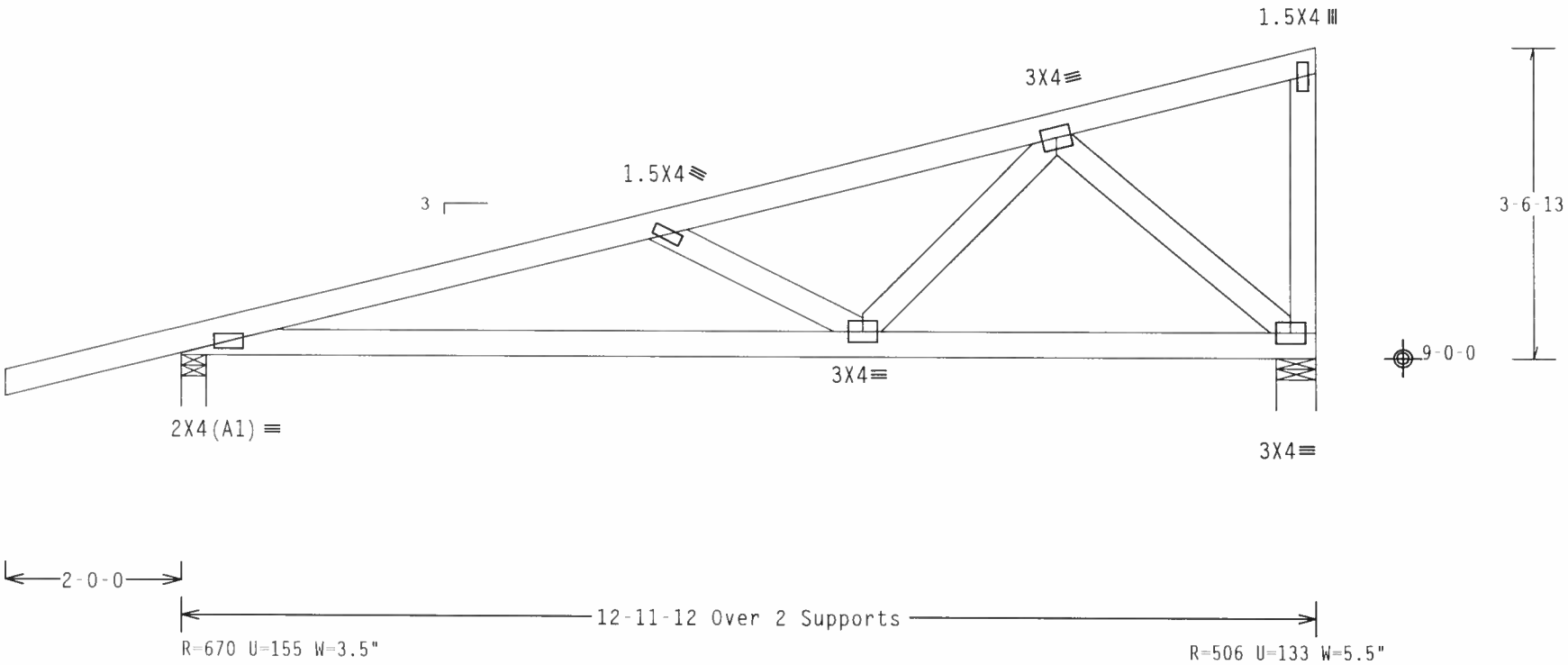
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF R8228- 54811
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269003
BC LL	0.0 PSF	HC-ENG JB/DF *
TOT.LD.	40.0 PSF	SEQN- 51479
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JRFF- 1TB28728Z01

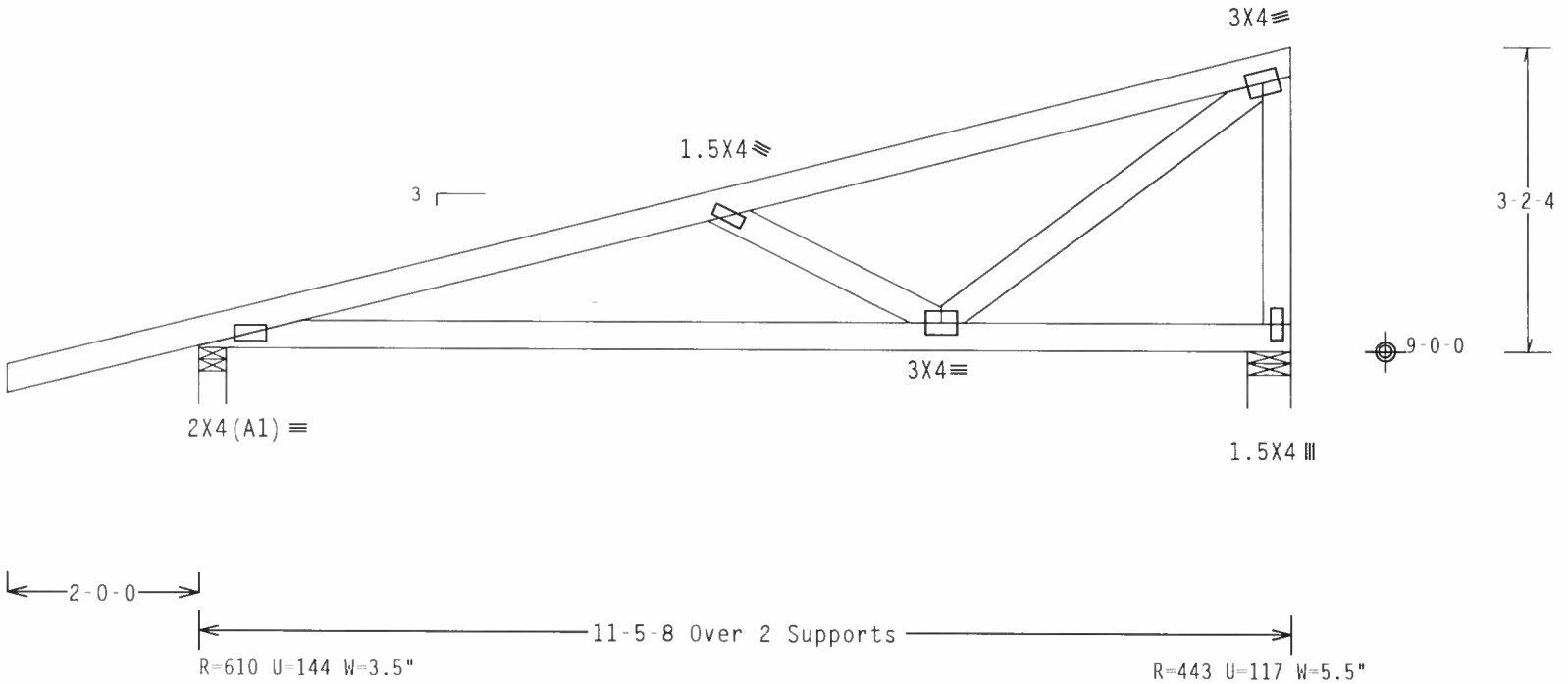
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Right end vertical not exposed to wind pressure.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.043 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

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ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 5627

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF R8228- 54812
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269004
BC LL	0.0 PSF	HC-ENG JB/DF *
TOT.LD.	40.0 PSF	SEQN- 51474
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TB28228Z01

(7 278 GARY JOHNSON GARY JOHNSON / SHOUP , ** PB4)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.32 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

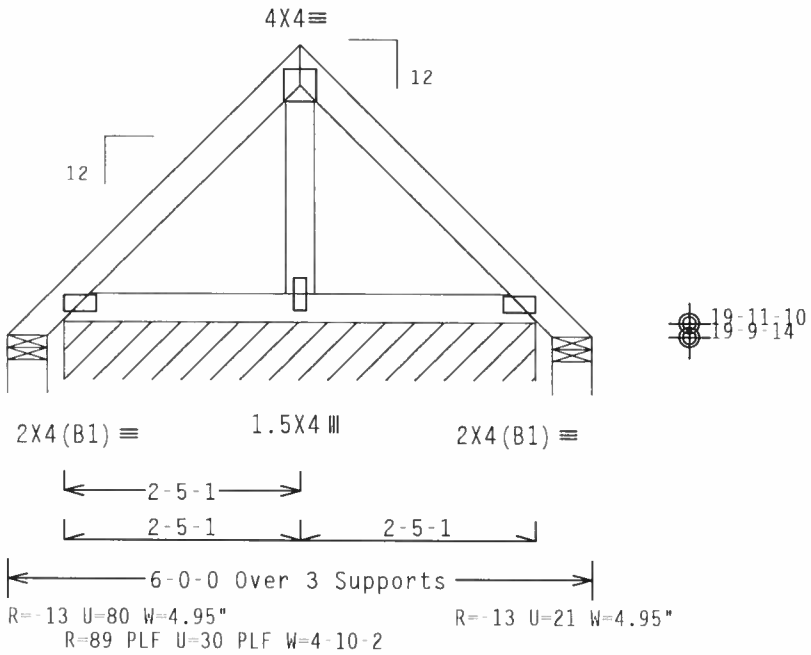
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	68 PLF	at	0.00	to	68 PLF	at	3.00
TC - From	68 PLF	at	3.00	to	68 PLF	at	6.00
BC - From	4 PLF	at	0.00	to	4 PLF	at	6.00

Wind reactions based on MWFRS pressures.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.



Design Crit: TPI-2002(STD)
Cq/RT=1.00(1.25)/0(0) 7.37.05 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 607

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IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI, OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (H,H/SS/K) ASTM A653 GRADE 40/60 (H, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 54813
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269017
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	2442 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 21.12 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

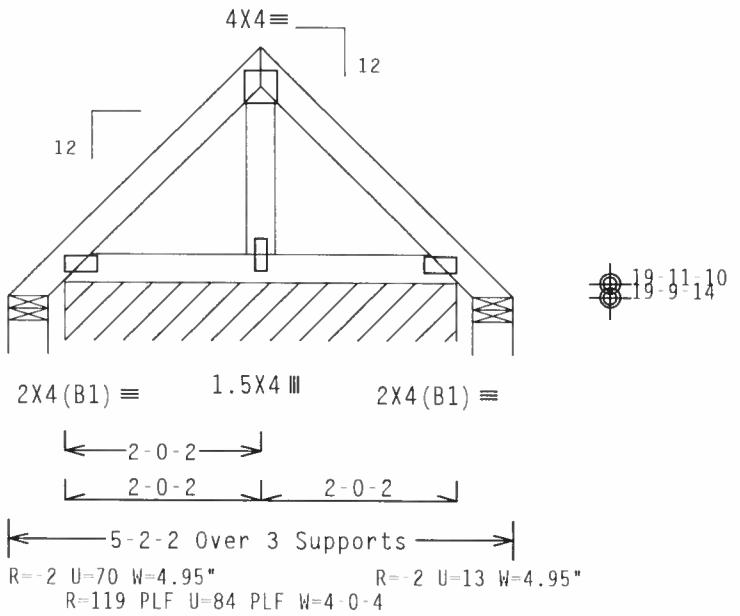
TC - From	68 PLF	at	0.00	to	68 PLF	at 2.59
TC - From	68 PLF	at	2.59	to	68 PLF	at 5.18
BC - From	4 PLF	at	0.00	to	4 PLF	at 5.18

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.

The building designer is responsible for the design of the roof and ceiling diaphragms, gable end shear walls, and supporting shear walls. Shear walls must provide continuous lateral restraint to the gable end. All connections to be designed by the building designer.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF	R8228- 54814
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269025
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN	51352
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF	1TB28228Z01

(7-278--GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - PB3)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

110 mph wind, 18.98 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

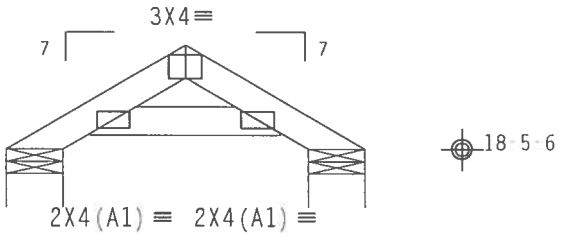
Wind reactions based on MWFRS pressures.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.

SPECIAL LOADS
------(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)
TC - From 63 PLF at 0.00 to 63 PLF at 1.83
TC - From 63 PLF at 1.83 to 63 PLF at 3.67
BC - From 4 PLF at 0.00 to 4 PLF at 3.67

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 567

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
26 '07

TC LL	20.0 PSF	REF	R8228- 54815
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269018
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51490
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JRFF-	1TB28228Z01

(7-278 GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - PB1)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 20.78 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=1.2 psf.

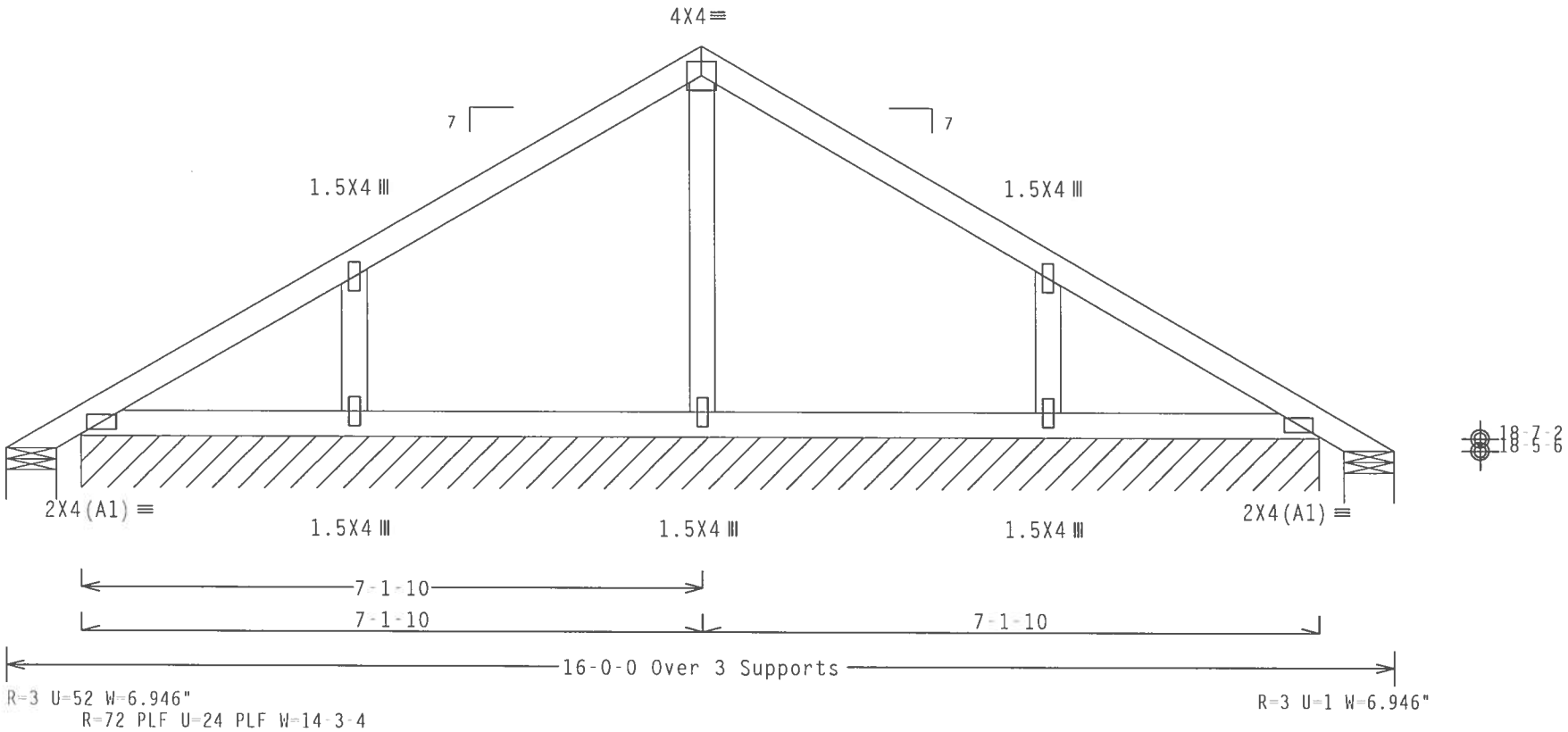
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.

SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC - From	63 PLF at	0.00 to	63 PLF at	8.00
TC - From	63 PLF at	8.00 to	63 PLF at	16.00
BC - From	4 PLF at	0.00 to	4 PLF at	16.00

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)
Cq/RT=1.00(1.25)/0(0) 7.37.052

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
SEP 26 '07

TC LL	20.0 PSF	REF	R8228- 54816
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269019
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	2447 REV
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB2822RZ01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

110 mph wind, 20.61 ft mean hgt, ASCE 7-02, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=2.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWGS A11030EE0207 & GBLLETIN0207 for more requirements.

Refer to DWG PIGBACKA0207 or PIGBACKB0207 for piggyback details.

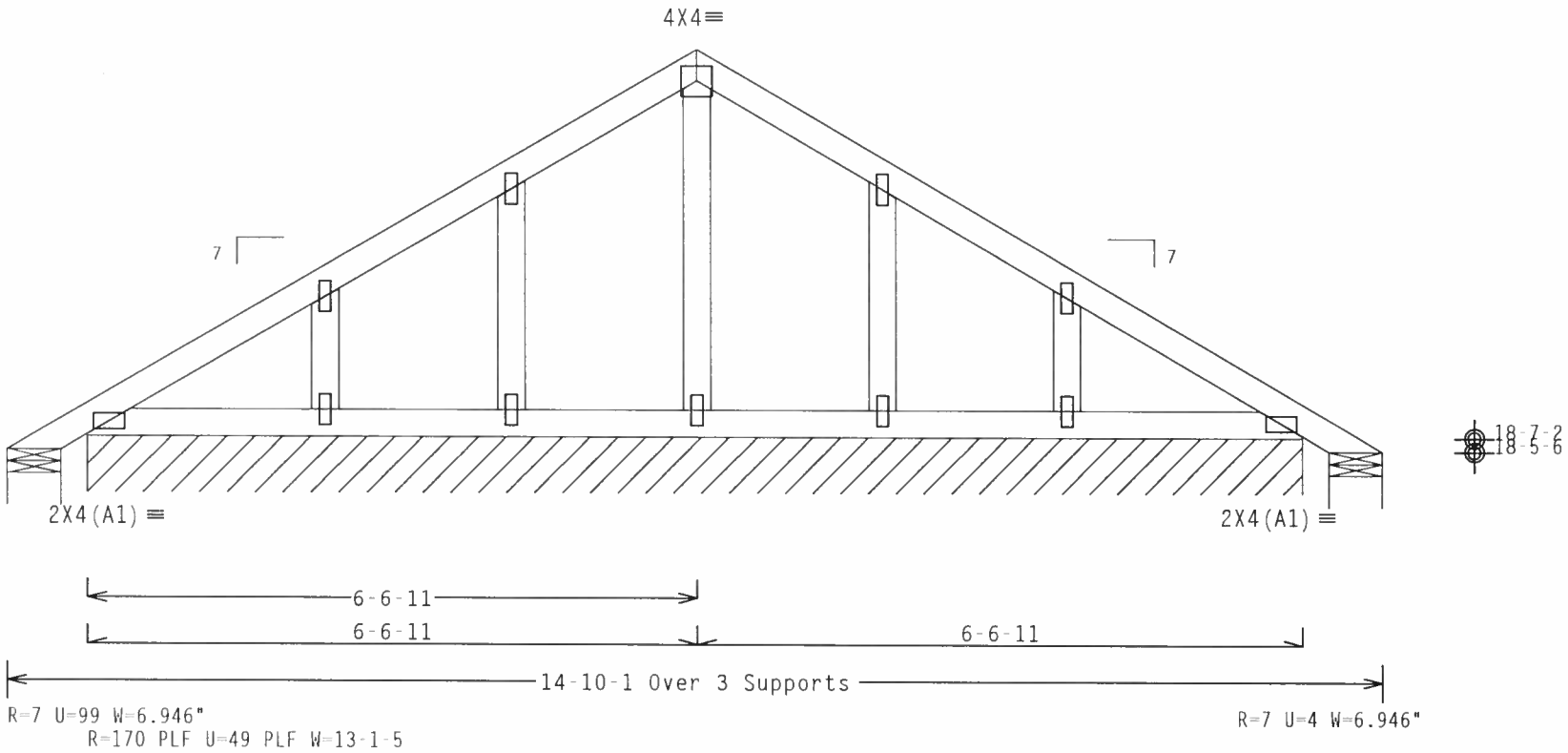
SPECIAL LOADS
(LUMBER DUR.FAC.=1.25 / PLATE DUR.FAC.=1.25)

TC	From	63 PLF	at	0.00	to	63 PLF at 7.42
TC	From	63 PLF	at	7.42	to	63 PLF at 14.84
BC	From	4 PLF	at	0.00	to	4 PLF at 14.84

Truss spaced at 24.0" OC designed to support 2-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
Certificate [illegible] Authorization [illegible]

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

Sep 26 '07

TC LL	20.0 PSF	REF	R8228- 54817
TC DL	10.0 PSF	DATE	09/26/07
BC DL	10.0 PSF	DRW	HCUSR8228 07269026
BC LL	0.0 PSF	HC-ENG	JB/DF
TOT.LD.	40.0 PSF	SEQN-	51507
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TB28228Z01

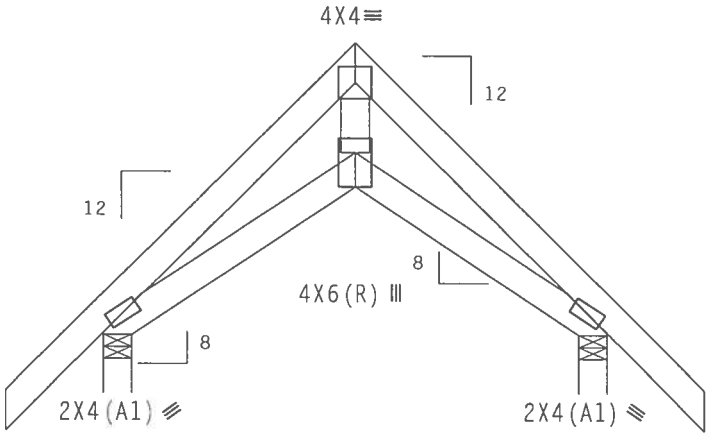
(7-278 GARY JOHNSON GARY JOHNSON / SHOUP -- , ** - D1)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

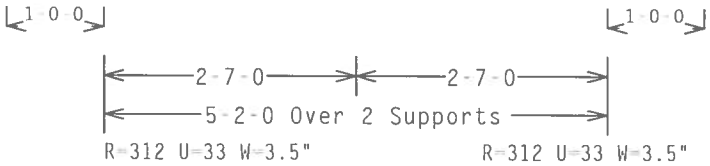
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 19.08 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.



17-10-5



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

26 '07

TC LL	20.0 PSF	REF R8228-54818
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269020
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 51304
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TB28228Z01

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Wind reactions based on MWFRS pressures.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 10.00 PSF. Top chord must not be cut or notched.

| MEMBER TO BE LATERALLY BRACED FOR WIND LOADS PERPENDICULAR TO TRUSS. BRACING SYSTEM TO BE DESIGNED AND FURNISHED BY OTHERS.

See DWGS A10030EC0207, GBLLETIN0207, & GBLBRSTC0207 for more requirements.

(**) 3 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 18.88 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

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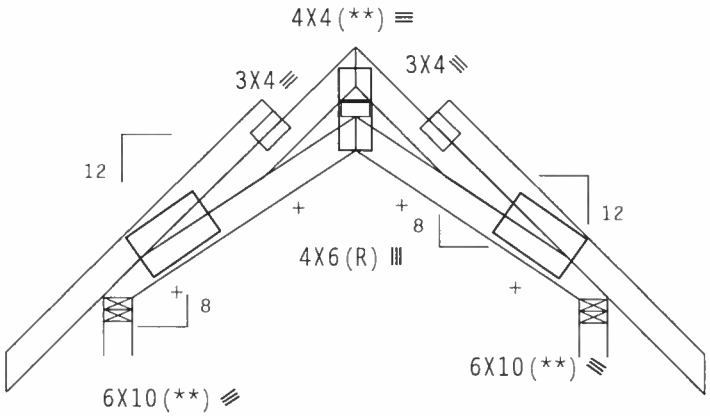

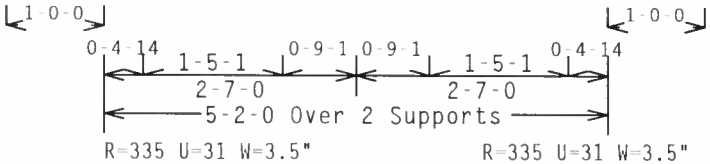

Design Crit: TPI-2002(STD)

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.37.05 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization #6673

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DOUGLAS FLEMING LICENSE No.66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

[illegible] 26 '07

TC LL	20.0 PSF	REF R8228- 54819
TC DL	10.0 PSF	DATE 09/26/07
BC DL	10.0 PSF	DRW HCUSR8228 07269027
BC LL	0.0 PSF	HC-ENG JB/DF
TOT.LD.	40.0 PSF	SEQN- 2463 REV
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TB28228Z01

CLB WEB BRACE SUBSTITUTION

THIS DETAIL IS TO BE USED WHEN CONTINUOUS LATERAL BRACING (CLB) IS SPECIFIED ON AN ALPINE TRUSS DESIGN BUT AN ALTERNATIVE WEB BRACING METHOD IS DESIRED.

NOTES:

THIS DETAIL IS ONLY APPLICABLE FOR CHANGING THE SPECIFIED CLB SHOWN ON SINGLE PLY SEALED DESIGNS TO T-BRACING OR SCAB BRACING.

ALTERNATIVE BRACING SPECIFIED IN CHART BELOW MAY BE CONSERVATIVE. FOR MINIMUM ALTERNATIVE BRACING, RE-RUN DESIGN WITH APPROPRIATE BRACING.

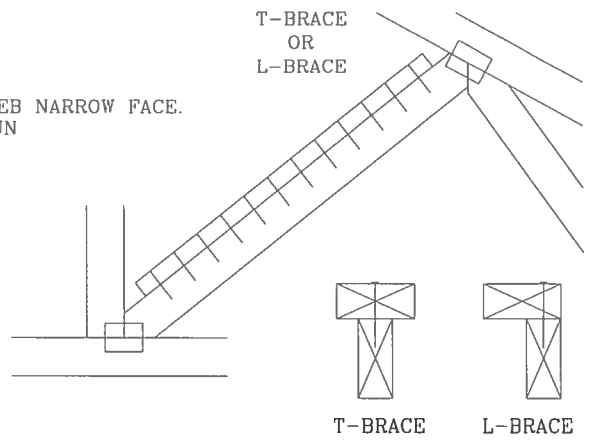
WEB MEMBER SIZE	SPECIFIED CLB BRACING	ALTERNATIVE BRACING T OR L-BRACE	ALTERNATIVE BRACING SCAB BRACE
2X3 OR 2X4	1 ROW	2X4	1-2X4
2X3 OR 2X4	2 ROWS	2X6	2-2X4
2X6	1 ROW	2X4	1-2X6
2X6	2 ROWS	2X6	2-2X4(*)
2X8	1 ROW	2X6	1-2X8
2X8	2 ROWS	2X6	2-2X6(*)

T-BRACE, L-BRACE AND SCAB BRACE TO BE SAME SPECIES AND GRADE OR BETTER THAN WEB MEMBER UNLESS SPECIFIED OTHERWISE ON ENGINEER'S SEALED DESIGN.

(*) CENTER SCAB ON WIDE FACE OF WEB. APPLY (1) SCAB TO EACH FACE OF WEB.

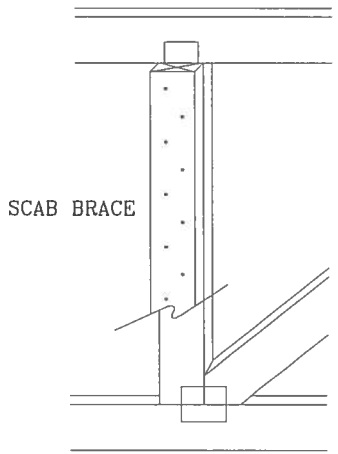
T-BRACING OR L-BRACING:

APPLY TO EITHER SIDE OF WEB NARROW FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



SCAB BRACING:

APPLY SCAB(S) TO WIDE FACE OF WEB.
NO MORE THAN (1) SCAB PER FACE.
ATTACH WITH 10d BOX OR GUN (0.128"x 3.",MIN) NAILS.
AT 6" O.C. BRACE IS A MINIMUM 80% OF WEB MEMBER LENGTH



THIS DRAWING REPLACES DRAWING 579,640

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 MAR 26 '07 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	PSF	REF	CLB SUBST.
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	BRCLBSUB0207
BC LL	PSF	-ENG	MLH/KAR
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

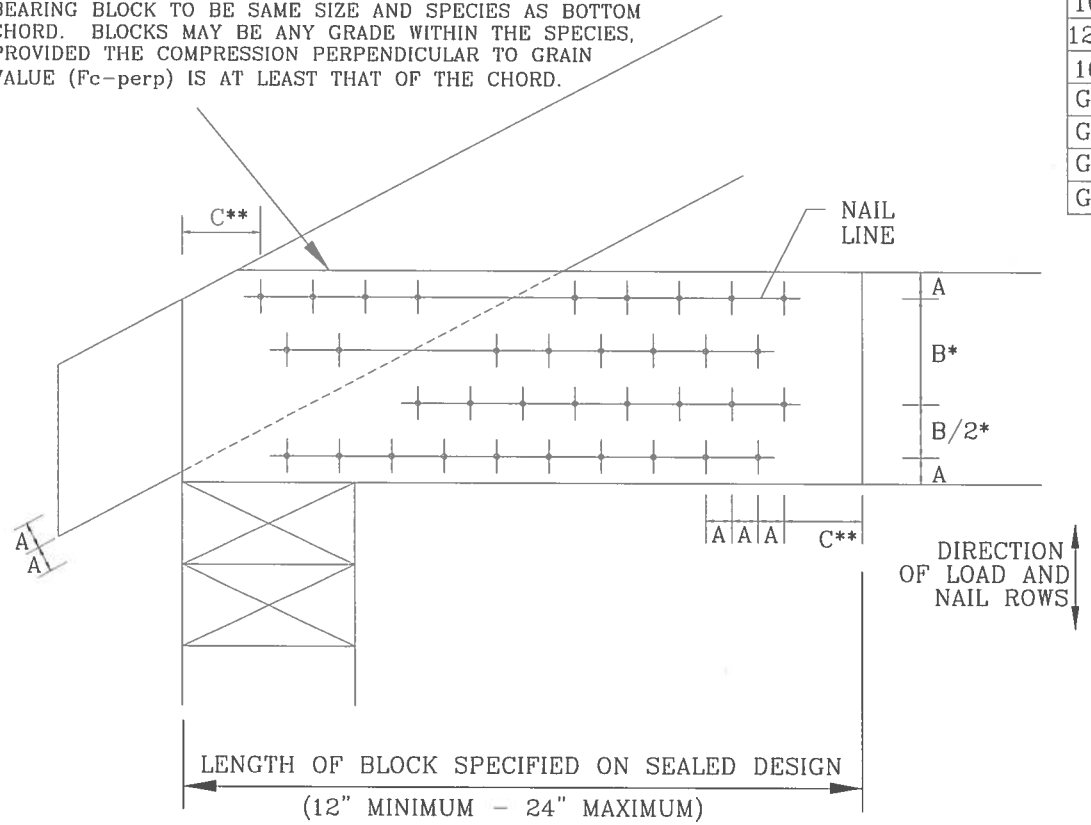
BEARING BLOCK NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

A – EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
B – SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
C – END DISTANCE (15 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:
- * SPACING MAY BE REDUCED BY 50%
- ** SPACING MAY BE REDUCED BY 33%

BEARING BLOCK TO BE SAME SIZE AND SPECIES AS BOTTOM CHORD. BLOCKS MAY BE ANY GRADE WITHIN THE SPECIES, PROVIDED THE COMPRESSION PERPENDICULAR TO GRAIN VALUE (Fc-perp) IS AT LEAST THAT OF THE CHORD.



MAXIMUM NUMBER OF NAIL LINES PARALLEL TO GRAIN

	CHORD SIZE				
NAIL TYPE	2X4	2X6	2X8	2X10	2X12
8d BOX (0.113"X 2.5",MIN)	3	6	9	12	15
10d BOX (0.128"X 3.",MIN)	3	5	7	10	12
12d BOX (0.128"X 3.25",MIN)	3	5	7	10	12
16d BOX (0.135"X 3.5",MIN)	3	5	7	10	12
20d BOX (0.148"X 4.",MIN)	2	4	5	6	8
8d COMMON (0.131"X 2.5",MIN)	3	5	7	10	12
10d COMMON (0.148"X 3.",MIN)	2	4	6	8	10
12d COMMON (0.148"X 3.25",MIN)	2	4	6	8	10
16d COMMON (0.162"X 3.5",MIN)	2	4	6	8	10
GUN (0.120"X 2.5",MIN)	3	6	8	11	14
GUN (0.131"X 2.5",MIN)	3	5	7	10	12
GUN (0.120"X 3.",MIN)	3	6	8	11	14
GUN (0.131"X 3.",MIN)	3	5	7	10	12

MINIMUM NAIL SPACING DISTANCES

	DISTANCES		
NAIL TYPE	A	B*	C**
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"
16d COMMON (0.162"X 3.5",MIN)	1'	2"	2 1/2"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"

THIS DRAWING REPLACES DRAWING B139 AND CNBRGBLK0699

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 '07 STATE OF FLORIDA PROFESSIONAL ENGINEER

REF	BEARING BLOCK
DATE	2/23/07
DRWG	CNBRGBLK0207
-ENG	SJP/KAR

ASCE 7-02: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

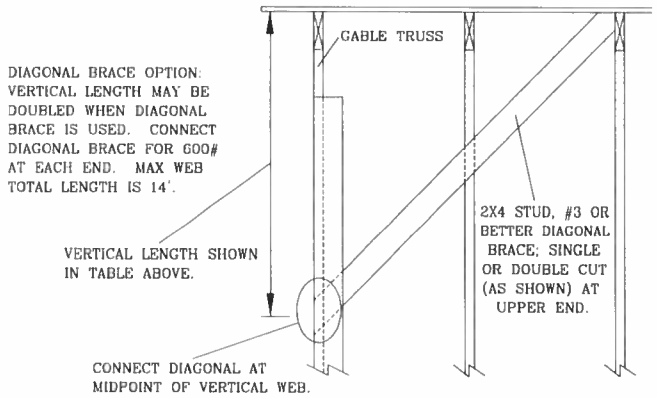
SOUTHERN PINE
#3
STUD
STANDARD

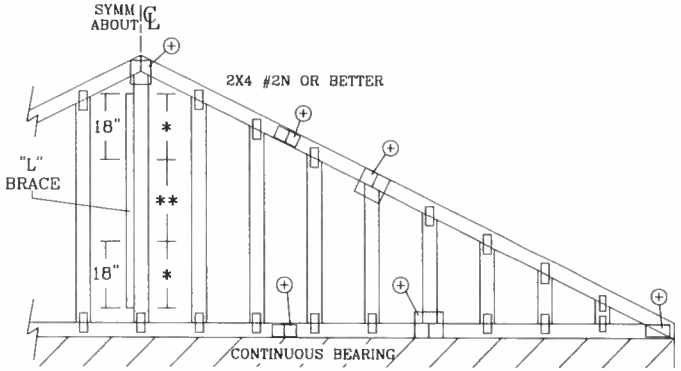
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

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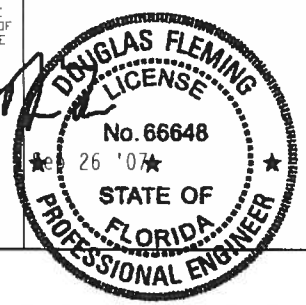
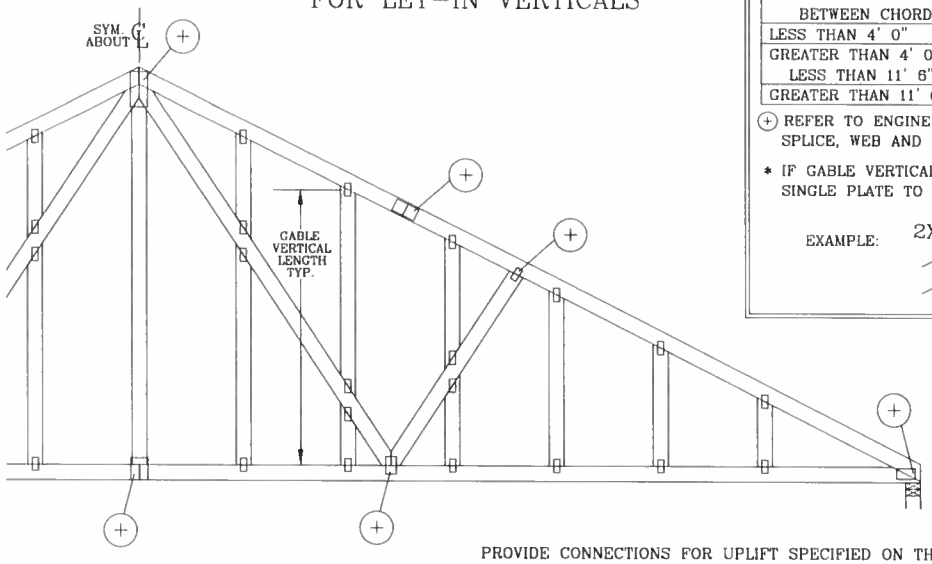


MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11015
DATE	2/23/07
DRWG	A11015EE0207
-ENG	

GABLE DETAIL
FOR LET-IN VERTICALS

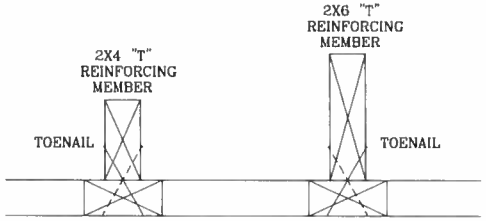


GABLE VERTICAL PLATE SIZES		
VERTICAL LENGTH BETWEEN CHORDS	PLATE SIZE	IF PLATES OVERLAP*
LESS THAN 4' 0"	1X4 OR 2X3	2X8
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4	2X8
GREATER THAN 11' 6"	2.5X4	2.5X8

⊕ REFER TO ENGINEERED TRUSS DESIGN FOR PEAK, SPLICE, WEB AND HEEL PLATES.

* IF GABLE VERTICAL PLATES OVERLAP, USE A SINGLE PLATE TO SPAN THE WEB.

EXAMPLE: 2X4 / 2X4 — 2X8



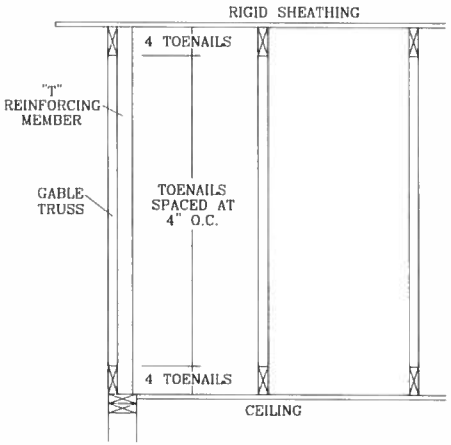
TO CONVERT FROM "L" TO "T" REINFORCING MEMBERS, MULTIPLY "T" FACTOR BY LENGTH (BASED ON GABLE VERTICAL SPECIES, GRADE AND SPACING) FOR (1) 2X4 "L" BRACE, GROUP A, OBTAINED FROM THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

MAXIMUM ALLOWABLE "T" REINFORCED GABLE VERTICAL LENGTH IS 14' FROM TOP TO BOTTOM CHORD.

WEB LENGTH INCREASE W/ "T" BRACE

WIND SPEED AND MRH	"T" REINF. MBR. SIZE	SBCCI	ASCE
110 MPH	2x4	10 %	10 %
15 FT	2x6	40 %	50 %
110 MPH	2x4	10 %	10 %
30 FT	2x6	50 %	50 %
100 MPH	2x4	10 %	10 %
15 FT	2x6	30 %	50 %
100 MPH	2x4	10 %	10 %
30 FT	2x6	40 %	40 %
90 MPH	2x4	20 %	10 %
15 FT	2x6	20 %	40 %
90 MPH	2x4	10 %	10 %
30 FT	2x6	30 %	50 %
80 MPH	2x4	10 %	20 %
15 FT	2x6	10 %	30 %
80 MPH	2x4	20 %	10 %
30 FT	2x6	20 %	40 %
70 MPH	2x4	0 %	20 %
15 FT	2x6	0 %	20 %
70 MPH	2x4	10 %	20 %
30 FT	2x6	10 %	30 %

EXAMPLE:
ASCE WIND SPEED = 100 MPH
MEAN ROOF HEIGHT = 30 FT
GABLE VERTICAL = 24" O.C. SP #3
"T" REINFORCING MEMBER SIZE = 2X4
"T" BRACE INCREASE (FROM ABOVE) = 10% = 1.10
(1) 2X4 "L" BRACE LENGTH = 6' 7"
MAXIMUM "T" REINFORCED GABLE VERTICAL LENGTH
1.10 x 6' 7" = 7' 3"



PROVIDE CONNECTIONS FOR UPLIFT SPECIFIED ON THE ENGINEERED TRUSS DESIGN.

ATTACH EACH "T" REINFORCING MEMBER WITH
HAND DRIVEN NAILS:
10d COMMON (0.148"X 3.",MIN) TOENAILS AT 4" O.C. PLUS
(4) 16d COMMON (0.162" X 3.5",MIN) TOENAILS IN TOP AND BOTTOM CHORD.

GUN DRIVEN NAILS:
8d COMMON (0.131"X 2.5",MIN) TOENAILS AT 4" O.C. PLUS
(4) TOENAILS IN TOP AND BOTTOM CHORD.

THIS DETAIL TO BE USED WITH THE APPROPRIATE ALPINE GABLE DETAIL FOR ASCE OR SBCCI WIND LOAD.

ASCE 7-93 GABLE DETAIL DRAWINGS
A11015EN0207, A10015EN0207, A09015EN0207, A08015EN0207, A07015EN0207, A11030EN0207, A10030EN0207, A09030EN0207, A08030EN0207, A07030EN0207

ASCE 7-98 GABLE DETAIL DRAWINGS
A13015EC0207, A12015EC0207, A11015EC0207, A10015EC0207, A08515EC0207, A13030EC0207, A12030EC0207, A11030EC0207, A10030EC0207, A08530EC0207

ASCE 7-02 GABLE DETAIL DRAWINGS
A13015EE0207, A12015EE0207, A11015EE0207, A10015EE0207, A08515EE0207, A13030EE0207, A12030EE0207, A11030EE0207, A10030EE0207, A08530EE0207

ASCE 7-05 GABLE DETAIL DRAWINGS
A13015E50207, A12015E50207, A11015E50207, A10015E50207, A08515E50207, A13030E50207, A12030E50207, A11030E50207, A10030E50207, A08530E50207

SEE APPROPRIATE ALPINE GABLE DETAIL (ASCE OR SBCCI WIND LOAD) FOR MAXIMUM UNREINFORCED GABLE VERTICAL LENGTH.

THIS DRAWING REPLACES DRAWINGS GAB98117 876,719 & HC26294035

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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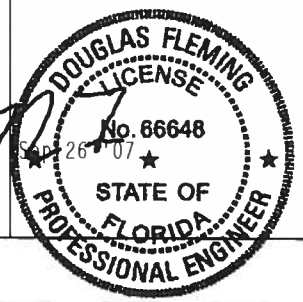


MAX TOT. LD.	60 PSF
DUR. FAC.	ANY
MAX SPACING	24.0"

REF	LET-IN VERT
DATE	2/23/07
DRWG	GBLLETIN0207
-ENG	DLJ/KAR

ASCE 7-98: 110 MPH WIND SPEED, 15' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE * GROUP A	(1) 1X4 "L" BRACE * GROUP B	(1) 2X4 "L" BRACE * GROUP A	(1) 2X4 "L" BRACE * GROUP B	(2) 2X4 "L" BRACE ** GROUP A	(2) 2X4 "L" BRACE ** GROUP B	(1) 2X6 "L" BRACE * GROUP A	(1) 2X6 "L" BRACE * GROUP B	(2) 2X6 "L" BRACE ** GROUP A	(2) 2X6 "L" BRACE ** GROUP B
24" O.C.	SPF HF	#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 5"	9' 8"	12' 5"	12' 9"	14' 0"	14' 0"
		#3	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"
		STUD	3' 9"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 3"	12' 3"	14' 0"	14' 0"
		STANDARD	3' 9"	5' 2"	5' 2"	6' 9"	6' 9"	9' 1"	9' 1"	10' 7"	10' 7"	14' 0"	14' 0"
	SP DFL	#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#2	4' 2"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"	14' 0"
		#3	4' 0"	6' 2"	6' 2"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 8"	14' 0"	14' 0"
		STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 0"	9' 5"	9' 11"	12' 5"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 5"	7' 8"	7' 10"	9' 1"	9' 4"	10' 10"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	7' 4"	7' 4"	9' 1"	9' 1"	10' 10"	10' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 4"	6' 4"	6' 4"	8' 4"	8' 4"	10' 10"	10' 10"	12' 11"	12' 11"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 9"	7' 8"	8' 3"	9' 1"	9' 9"	10' 10"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 7"	7' 7"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 6"	7' 6"	9' 1"	9' 6"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 6"	8' 6"	10' 10"	11' 1"	13' 3"	13' 3"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 11"	8' 5"	8' 8"	10' 0"	10' 3"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	8' 5"	8' 5"	10' 0"	10' 0"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 9"	7' 3"	7' 3"	9' 7"	9' 7"	11' 11"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 3"	8' 5"	9' 1"	10' 0"	10' 9"	11' 11"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	8' 5"	8' 5"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 0"	8' 5"	8' 7"	10' 0"	10' 6"	11' 11"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 11"	7' 5"	7' 5"	9' 10"	9' 10"	11' 11"	12' 3"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR		HEM-FIR	
#1 / #2	STANDARD	#2	STUD
#3	STUD	#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

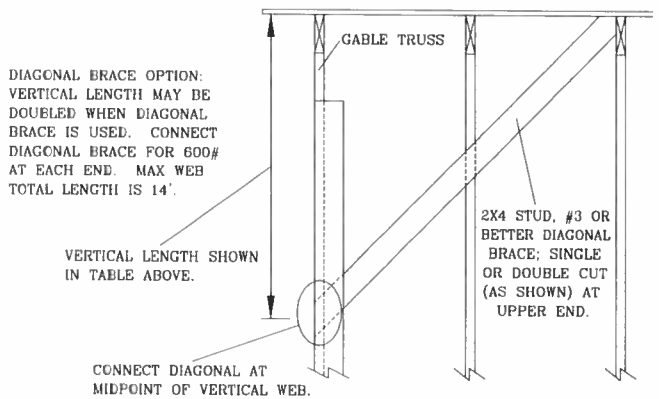
SOUTHERN PINE
#3
STUD
STANDARD

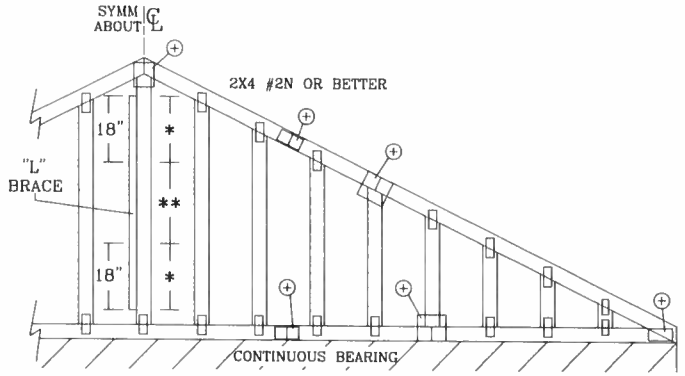
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 80 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

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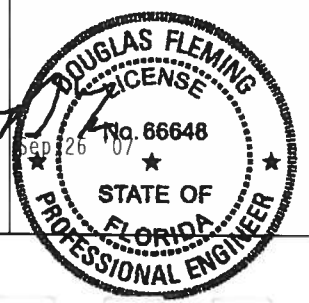


MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-98-GAB11015
DATE	2/23/07
DRWG	A11015EC0207
-ENG	

PIGGYBACK DETAIL

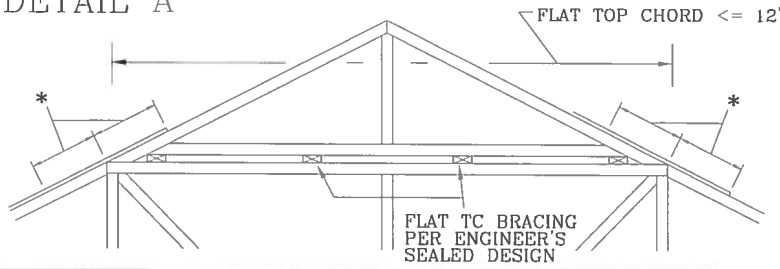
100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

80 MPH WIND, 30.00 FT MEAN HGT, SBC, ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

100 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP. C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

NOTE: TOP CHORDS OF TRUSSES SUPPORTING PIGGYBACK CAP TRUSSES MUST BE ADEQUATLY BRACED BY SHEATHING OR PURLINS. PROVIDE DIAGONAL BRACING OR OTHER SUITABLE ANCHORAGE TO PERMANENTLY RESTRAIN PURLINS.

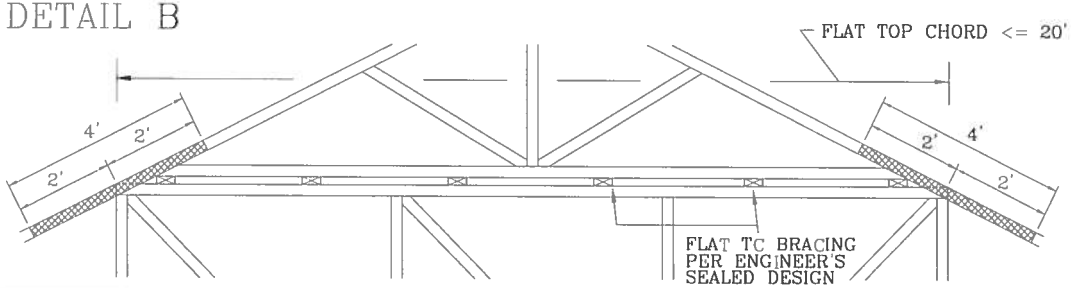
DETAIL A



PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"x3") NAILS.

* 12" MIN RIGID SHEATHING OVERLAP WITH 8d COMMON (0.131"x2.5") OR GUN NAILS IN OVERLAP ZONE SPACED AT 4" O.C.

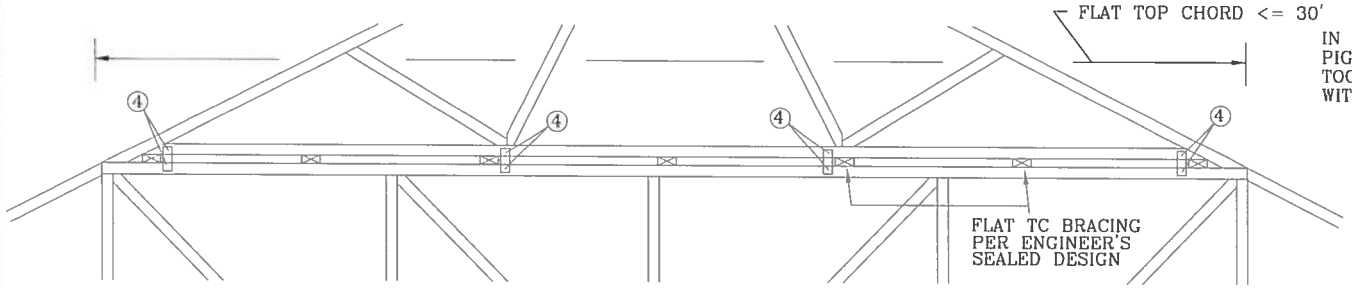
DETAIL B



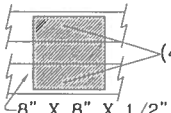
PIGGYBACK CAP TRUSS TOENAILED TO ALL TOP CHORD BRACING WITH (2) 10d COMMON (0.148"X3") NAILS AND SECURED WITH 2X4 #3 GRADE SCAB (1 SIDE ONLY) ATTACHED WITH 10d COMMON NAILS AT 4" O.C.

DETAIL C

CAP TRUSS TOENAILED TO TOP CHORD BRACING AND SECURED WITH 3X8 TRULOX PLATES (EACH FACE) AT EACH END AND AT 1/3 POINTS. CIRCLED NUMBER INDICATES REQUIRED NUMBER OF 0.120" X 1.375" NAILS PER FACE. SEE DRAWING 160TL FOR TRULOX INFORMATION.



IN LIEU OF TRULOX CONNECTORS, ALPINE 62PB SPECIAL PIGGYBACK CONNECTORS MAY BE USED. SHOP APPLY TOOTHED PORTION, FIELD ATTACH TO MATING TRUSS WITH (4) 0.120" X 0.375" NAILS MINIMUM EACH FACE.



8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 8d COMMON NAILS PER GUSSET, (4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

THIS DRAWING REPLACES DRAWINGS 581,670 & 961,860

ALPINE

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER Sep 26 07

TC LL	PSF	REF	PIGGYBACK
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	PIGBACKA0207
BC LL	PSF	-ENG	DLJ/KAR
TOT. LD.	MAX 60 PSF		
DUR. FAC.	1.15		
SPACING	24.0"		

PIGGYBACK DETAIL

TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

PIGGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

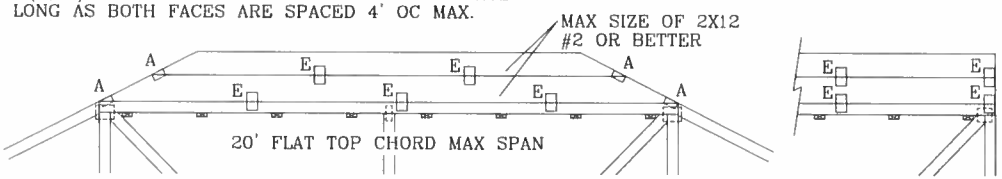
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

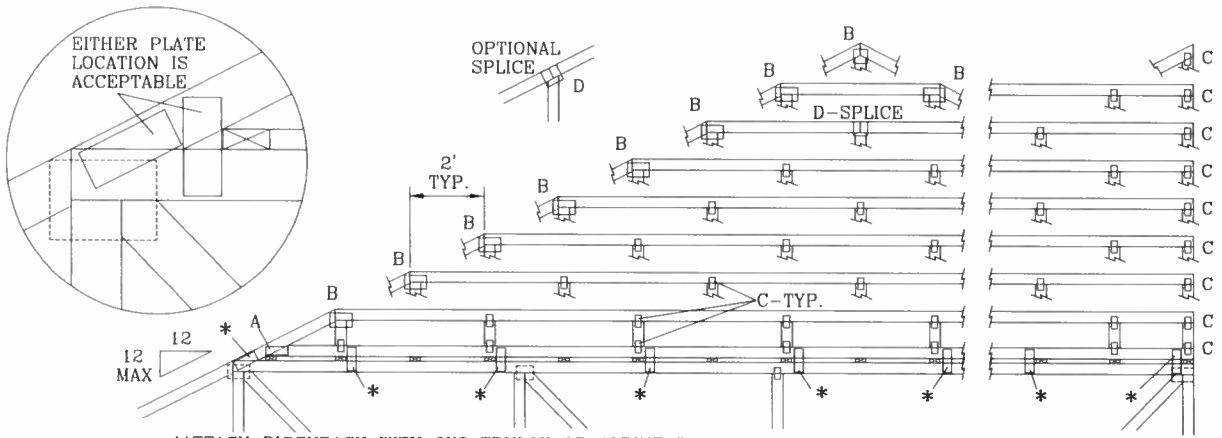
THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

130 MPH WIND, 30' MEAN HGT, ASCE 7-98, ASCE 7-02 OR ASCE 7-05, CLOSED BLGD, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF

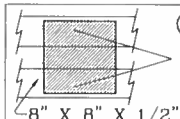
110 MPH WIND, 30' MEAN HGT, SBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL=5 PSF, WIND BC DL=5 PSF

FRONT FACE (E,*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.





*ATTACH PIGGYBACK WITH 3X8 TRULOX OR ALPINE PIGGYBACK SPECIAL PLATE.



(4) 6d BOX (0.099"X 2.",MIN) NAILS.

8" X 8" X 1/2" RATED SHEATHING GUSSETS (EACH FACE) MAY BE USED IN LIEU OF TRULOX PLATES, ATTACH WITH (8) 6d BOX (0.099"X 2.",MIN) NAILS PER GUSSET.

(4) IN CAP BC AND (4) IN BASE TRUSS FLAT TC.

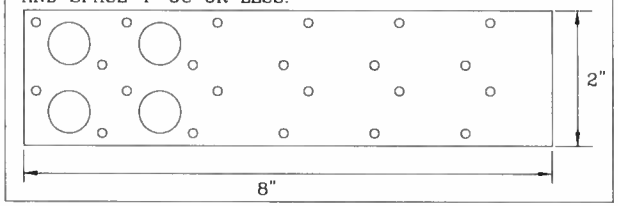
JOINT TYPE	SPANS UP TO			
	30'	34'	38'	52'
A	2X4	2.5X4	2.5X4	3X5
B	4X6	5X6	5X6	5X6
C	1.5X3	1.5X4	1.5X4	1.5X4
D	5X4	5X5	5X5	5X6
E	4X6 OR 3X6 TRULOX AT 4' OC, ROTATED VERTICALLY			

ATTACH TRULOX PLATES WITH (8) 0.120" X 1.375" NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

WEB BRACING CHART	
WEB LENGTH	REQUIRED BRACING
0' TO 7'9"	NO BRACING
7'9" TO 10'	1x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 8d BOX (0.113"X 2.5",MIN) NAILS AT 4" OC.
10' TO 14'	2x4 "T" BRACE. SAME GRADE, SPECIES AS WEB MEMBER, OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d BOX (0.135"X 3.5",MIN) NAILS AT 4" OC

* PIGGYBACK SPECIAL PLATE

ATTACH TEETH TO THE PIGGYBACK AT THE TIME OF FABRICATION. ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PIGGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



THIS DRAWING REPLACES DRAWINGS 634,016 634,017 & 847,045

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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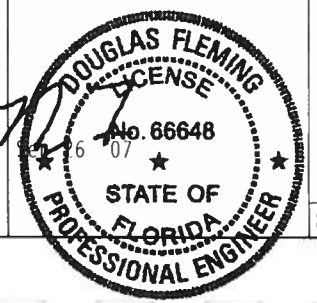
IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW, BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W,K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.



MAX LOADING
55 PSF AT 1.33 DUR. FAC.
50 PSF AT 1.25 DUR. FAC.
47 PSF AT 1.15 DUR. FAC.
SPACING 24.0"

REF	PIGGYBACK
DATE	2/23/07
DRWG	PIGBACKB0207
ENG	DLJ/KAR

ASCE 7-02: 110 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 8"	6' 4"	6' 6"	7' 6"	7' 8"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"	14' 0"
		#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"	14' 0"
		STUD	3' 7"	5' 5"	5' 5"	7' 1"	7' 1"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"	14' 0"
		STANDARD	3' 7"	4' 8"	4' 8"	6' 1"	6' 1"	8' 3"	8' 3"	9' 6"	9' 6"	12' 11"	12' 11"
	SP DFL	#1	4' 0"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#2	3' 11"	6' 4"	6' 10"	7' 6"	8' 1"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"	14' 0"
		#3	3' 9"	5' 7"	5' 7"	7' 4"	7' 4"	8' 11"	9' 5"	11' 5"	11' 5"	14' 0"	14' 0"
		STUD	3' 9"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	9' 5"	11' 4"	11' 4"	14' 0"	14' 0"
		STANDARD	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 2"	7' 3"	7' 5"	8' 7"	8' 10"	10' 3"	10' 6"	13' 5"	13' 10"	14' 0"	14' 0"
		#3	4' 1"	6' 8"	6' 8"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STUD	4' 1"	8' 0"	8' 0"	8' 7"	8' 7"	10' 3"	10' 3"	13' 5"	13' 5"	14' 0"	14' 0"
		STANDARD	4' 1"	5' 8"	5' 8"	7' 6"	7' 6"	10' 1"	10' 1"	11' 8"	11' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 7"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#2	4' 6"	7' 3"	7' 9"	8' 7"	9' 3"	10' 3"	11' 0"	13' 5"	14' 0"	14' 0"	14' 0"
		#3	4' 4"	6' 10"	6' 10"	8' 7"	9' 0"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STUD	4' 4"	6' 9"	6' 9"	8' 7"	8' 11"	10' 3"	10' 9"	13' 5"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 2"	5' 10"	5' 10"	7' 8"	7' 8"	10' 3"	10' 4"	11' 11"	11' 11"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	4' 7"	8' 0"	8' 2"	9' 5"	9' 8"	11' 3"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 6"	7' 8"	7' 8"	9' 5"	9' 5"	11' 3"	11' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 3"	11' 3"	13' 6"	13' 6"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 0"	8' 7"	9' 5"	10' 2"	11' 3"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 11"	7' 11"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 9"	7' 9"	7' 9"	9' 5"	9' 11"	11' 3"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 7"	6' 9"	6' 9"	8' 10"	8' 10"	11' 3"	11' 7"	13' 10"	13' 10"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

SOUTHERN PINE
#3
STUD
STANDARD

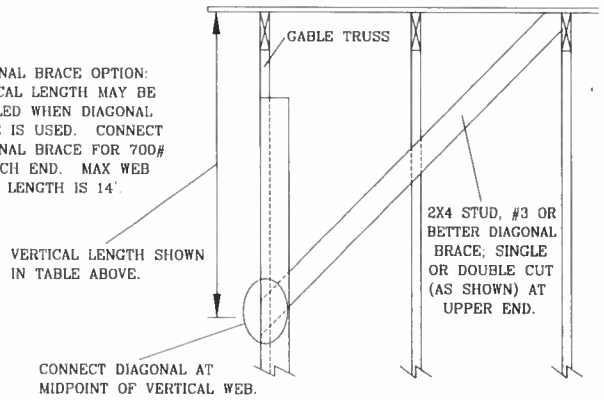
GROUP B:

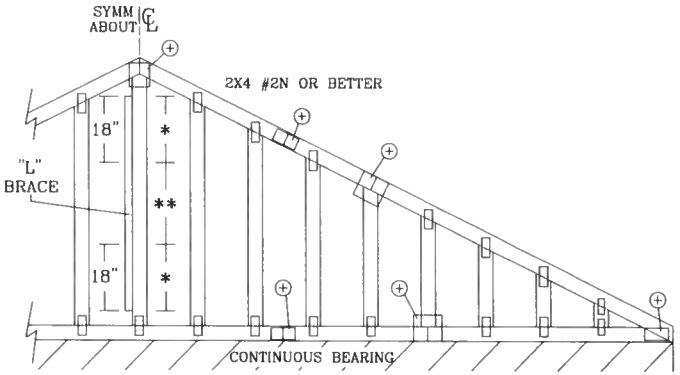
HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2

DIAGONAL BRACE OPTION: VERTICAL LENGTH MAY BE DOUBLED WHEN DIAGONAL BRACE IS USED. CONNECT DIAGONAL BRACE FOR 700# AT EACH END. MAX WEB TOTAL LENGTH IS 14'.





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 100 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 FEB 26 '07 STATE OF FLORIDA PROFESSIONAL ENGINEER

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-02-GAB11030
DATE	2/23/07
DRWG	A11030EE0207
-ENG	

ASCE 7-98: 100 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH

2X4 GABLE VERTICAL SPACING	SPECIES	BRACE GRADE	NO BRACES	(1) 1X4 "L" BRACE *		(1) 2X4 "L" BRACE *		(2) 2X4 "L" BRACE **		(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE **	
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B
24" O.C.	SPF HF	#1 / #2	3' 11"	6' 10"	7' 0"	8' 1"	8' 4"	9' 8"	9' 11"	12' 8"	13' 1"	14' 0"	14' 0"
		#3	3' 10"	6' 1"	6' 1"	8' 1"	8' 1"	9' 8"	9' 8"	12' 7"	12' 7"	14' 0"	14' 0"
		STUD	3' 10"	6' 1"	6' 1"	8' 0"	8' 0"	9' 8"	9' 8"	12' 6"	12' 6"	14' 0"	14' 0"
		STANDARD	3' 10"	5' 3"	5' 3"	6' 11"	6' 11"	9' 3"	9' 3"	10' 9"	10' 9"	14' 0"	14' 0"
	SP DFL	#1	4' 4"	6' 10"	7' 4"	8' 1"	8' 9"	9' 8"	10' 5"	12' 8"	13' 8"	14' 0"	14' 0"
		#2	4' 3"	6' 10"	7' 4"	8' 1"	8' 9"	9' 8"	10' 5"	12' 8"	13' 8"	14' 0"	14' 0"
		#3	4' 1"	6' 3"	6' 3"	8' 1"	8' 3"	9' 8"	10' 2"	12' 8"	12' 10"	14' 0"	14' 0"
		STUD	4' 1"	6' 2"	6' 2"	8' 1"	8' 2"	9' 8"	10' 2"	12' 8"	12' 9"	14' 0"	14' 0"
		STANDARD	3' 11"	5' 4"	5' 4"	7' 1"	7' 1"	9' 6"	9' 6"	11' 0"	11' 0"	14' 0"	14' 0"
16" O.C.	SPF HF	#1 / #2	4' 6"	7' 10"	8' 0"	9' 3"	9' 6"	11' 1"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 6"	7' 6"	9' 3"	9' 3"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 5"	7' 5"	7' 5"	9' 3"	9' 3"	11' 1"	11' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 5"	6' 5"	6' 5"	8' 5"	8' 5"	11' 1"	11' 1"	13' 2"	13' 2"	14' 0"	14' 0"
	SP DFL	#1	5' 0"	7' 10"	8' 5"	9' 3"	10' 0"	11' 1"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 10"	7' 10"	8' 5"	9' 3"	10' 0"	11' 1"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	7' 8"	7' 8"	9' 3"	9' 9"	11' 1"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 8"	7' 7"	7' 7"	9' 3"	9' 9"	11' 1"	11' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 6"	6' 7"	6' 7"	8' 8"	8' 8"	11' 1"	11' 4"	13' 5"	13' 5"	14' 0"	14' 0"
12" O.C.	SPF HF	#1 / #2	5' 0"	8' 7"	8' 10"	10' 2"	10' 6"	12' 2"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 10"	8' 7"	8' 7"	10' 2"	10' 2"	12' 2"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	4' 10"	8' 7"	8' 7"	10' 2"	10' 2"	12' 2"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	4' 10"	7' 5"	7' 5"	9' 9"	9' 9"	12' 2"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 6"	8' 7"	9' 3"	10' 2"	11' 0"	12' 2"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 4"	8' 7"	9' 3"	10' 2"	11' 0"	12' 2"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	8' 7"	8' 10"	10' 2"	10' 9"	12' 2"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STUD	5' 1"	8' 7"	8' 9"	10' 2"	10' 9"	12' 2"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"
		STANDARD	5' 0"	7' 7"	7' 7"	10' 0"	10' 0"	12' 2"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"

BRACING GROUP SPECIES AND GRADES:

GROUP A:

SPRUCE-PINE-FIR	
#1 / #2	STANDARD
#3	STUD

HEM-FIR	
#2	STUD
#3	STANDARD

DOUGLAS FIR-LARCH
#3
STUD
STANDARD

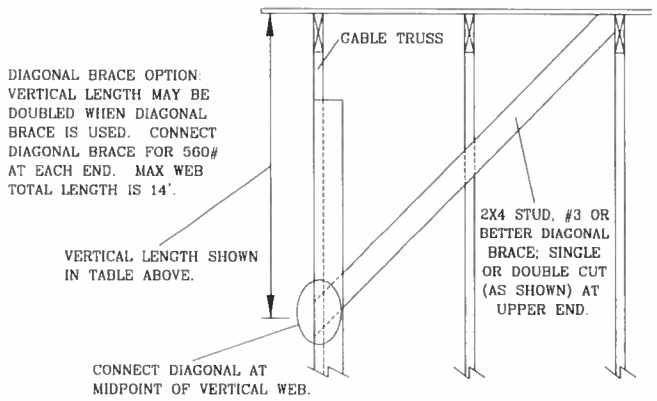
SOUTHERN PINE
#3
STUD
STANDARD

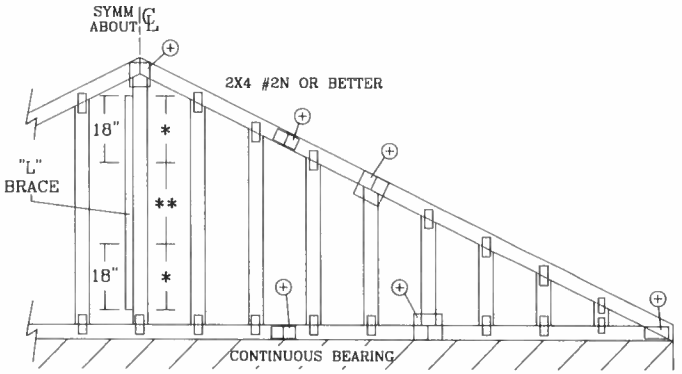
GROUP B:

HEM-FIR
#1 & BTR
#1

SOUTHERN PINE
#1
#2

DOUGLAS FIR-LARCH
#1
#2





REFER TO CHART ABOVE FOR MAX GABLE VERTICAL LENGTH.

GABLE TRUSS DETAIL NOTES:

LIVE LOAD DEFLECTION CRITERIA IS L/240.

PROVIDE UPLIFT CONNECTIONS FOR 75 PLF OVER CONTINUOUS BEARING (5 PSF TC DEAD LOAD).

GABLE END SUPPORTS LOAD FROM 4' 0" OUTLOOKERS WITH 2' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.

ATTACH EACH "L" BRACE WITH 10d NAILS.
* FOR (1) "L" BRACE: SPACE NAILS AT 2" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.
** FOR (2) "L" BRACES: SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.

"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.

GABLE VERTICAL PLATE SIZES

VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 6"	2X4
GREATER THAN 11' 6"	2.5X4

+ REFER TO COMMON TRUSS DESIGN FOR PEAK, SPLICE, AND HEEL PLATES.

ALPINE

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POMPANO BEACH, FLORIDA

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IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW, BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W,K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Se 26 07

MAX. TOT. LD. 60 PSF

MAX. SPACING 24.0"

REF	ASCE7-98-GAB10030
DATE	2/23/07
DRWG	A10030EC0207
-ENG	

ASCE 7-98: EXPOSURE C
COMMON RESIDENTIAL GABLE END WIND BRACING REQUIREMENTS - STIFFENERS

100 MPH 3 SECOND GUST WIND, 30 FT MEAN HGT, ASCE 7-98, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, I=1.00, CAT II, EXP C, WIND TC DL=5.0 PSF, WIND BC DL=5.0 PSF.

LATERAL CHORD BRACING REQUIREMENTS
TOP: CONTINUOUS ROOF SHEATHING
BOT: CONTINUOUS CEILING DIAPHRAGM

SEE ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN ON THIS DETAIL.

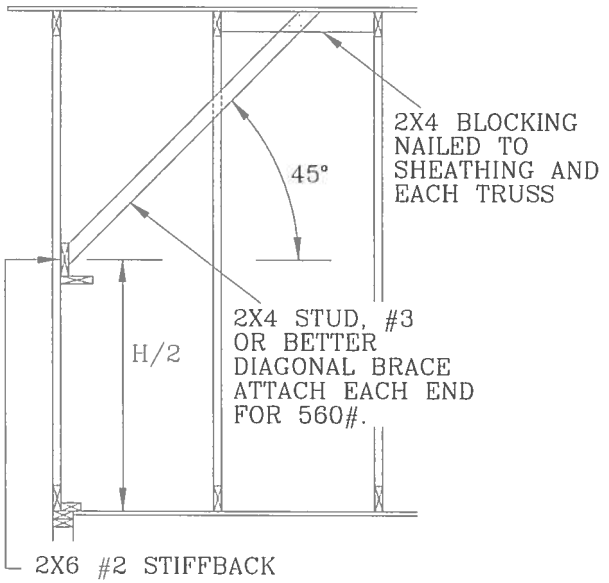
NAILS: 10d COMMON OR BOX (0.148"X 3.",MIN) NAILS OR GUN (0.125"X 3.",min) NAILS.

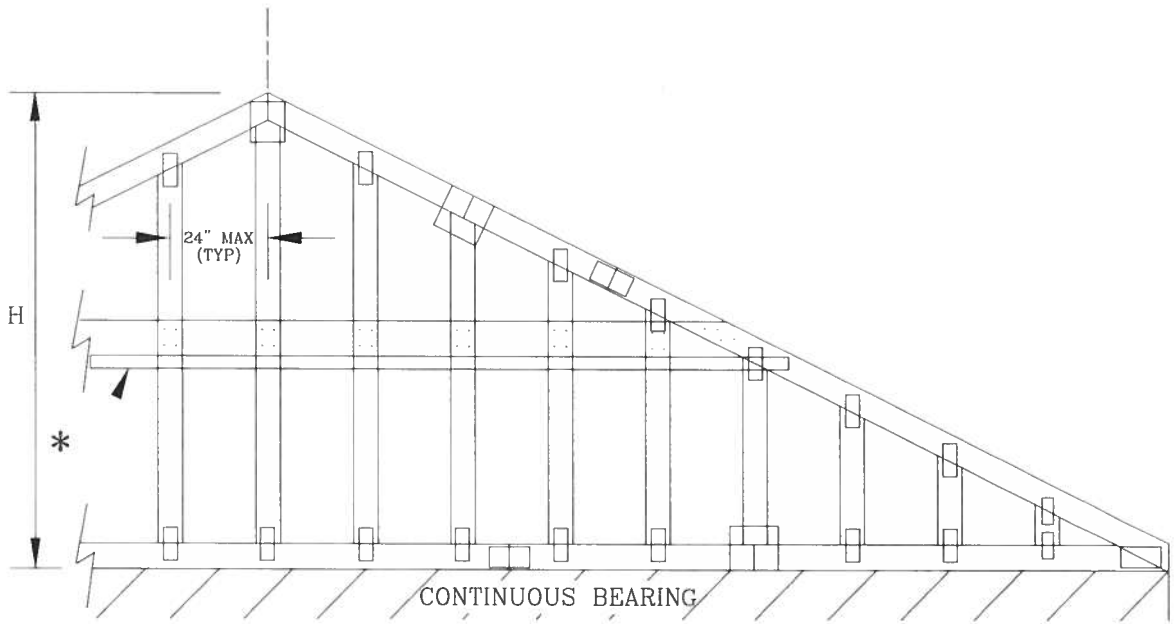
H LESS THAN 4'6" - NO STUD BRACING REQUIRED

H GREATER THAN 4'6" TO 7'6" IN LENGTH
PROVIDE A 2X6 STIFFBACK AT MID-HEIGHT AND BRACE STIFFBACK TO ROOF DIAPHRAGM EVERY 6'0" (SEE DETAIL BELOW OR REFER TO DRAWING A10030EC1103).

H GREATER THAN 7'6" TO 12'0" MAX:
PROVIDE A 2X6 STIFFBACK AT MID-HEIGHT AND BRACE TO ROOF DIAPHRAGM EVERY 4'0" (SEE DETAIL BELOW OR REFER TO DRWG A10030EC1103).

* OPTIONAL 2X L-REINFORCEMENT ATTACHED TO STIFFBACK WITH 10D BOX OR GUN (0.128" X 3", MIN.) NAILS @ 6" O.C.





THIS DRAWING REPLACES DRAWING 59469/GE

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 Sep 26 '07 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	PSF	REF	GE WHALER
TC DL	PSF	DATE	2/23/07
BC DL	PSF	DRWG	GBLBRSTC0207
BC LL	PSF	-ENG	SJP/KAR
TOT. LD.	PSF		
DUR. FAC.			
MAX SPACING	24"		

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC. BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-02, PART. ENC. BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TCDL=5.0 PSF, WIND BCDL=5.0 PSF.

+ FOR VERTICAL WEBS LESS THAN 4'0": W1X4
FOR VERTICAL WEBS GREATER THAN 4'0" BUT NO MORE THAN 11'6": W2X4.

* SPLICE, PEAK, AND HEEL PLATES TO MATCH COMMON TRUSS.

** 2X4 OR GREATER CHORDS.

DROP GABLE WILL SUPPORT 4'0" OUTLOOKERS WITH 2'0" OVERHANG (DROP HEEL GABLE) SPACED 24" O.C., OR THE LOAD FROM 12" PLYWOOD OVERHANG (NOMINAL HEEL GABLE).

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO DESIGN THE ROOF AND CEILING DIAPHRAGMS AND SPECIFY CONNECTIONS TO TRANSFER ALL OUT-OF-PLANE LOADS INTO THE ROOF AND CEILING DIAPHRAGMS.

THE BUILDING DESIGNER IS RESPONSIBLE FOR THE GABLE SHEAR WALL DESIGN, CEILING AND ROOF SHEATHING DIAPHRAGM CONNECTIONS, AND ALL TRUSS TO WALL CONNECTIONS.

++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

R1 NOTE: NAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2 16D NAILS AT EACH END.

R1 NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

BRACING DEFINITIONS:
NOTE: "END ZONE" EXISTS 18" AT BOTH ENDS OF VERTICAL WEB.

(A) (1) 2X4 SP #3 "L" BRACE. ATTACH WITH 0.128"X3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.

(B) (2) 2X4 SP #3 "L" BRACES. ATTACH EACH WITH 0.128"X3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.

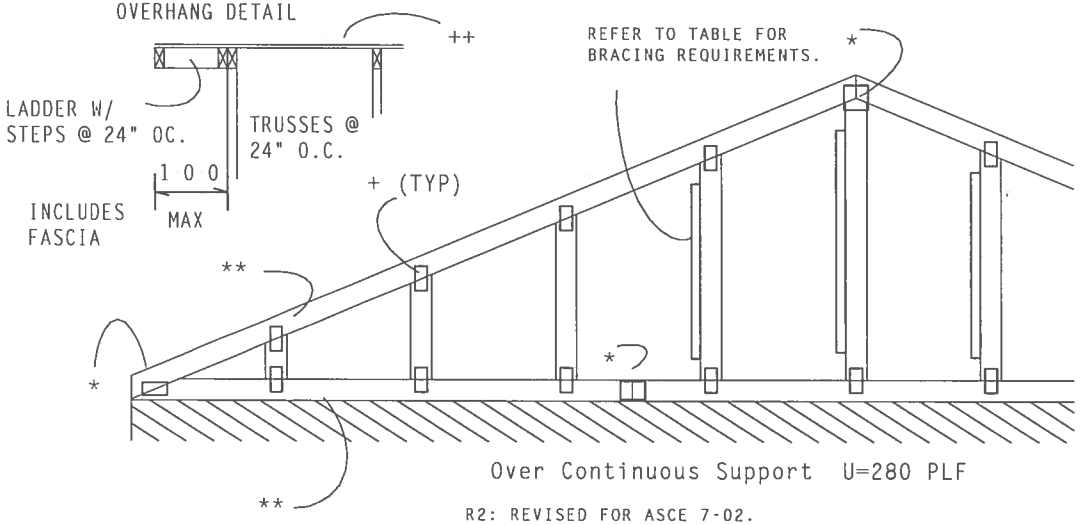
(C) (1) 2X6 SP #2 N "L" BRACE. ATTACH WITH 0.128"X3" NAILS @ 2" OC. IN END ZONES; 4" OC. BETWEEN ZONES.

(D) (2) 2X6 SP #2 N "L" BRACES. ATTACH EACH WITH 0.128"X3" NAILS @ 3" OC. IN END ZONES; 6" OC. BETWEEN ZONES.

STUD SPACING / BRACING TABLE:

2X4 SP #3 STUD SPACING	DEFLECTION CRITERIA	NO BRACE	(1) 2X4 "L" BRACE TYPE (A)	(2) 2X4 "L" BRACE TYPE (B)	(1) 2X6 "L" BRACE TYPE (C)	(2) 2X6 "L" BRACE TYPE (D)
24"	L/360	-----	3' 1"	4' 2"	6' 3"	8' 0"
24"	L/180	-----	3' 4"	5' 7"	6' 3"	11' 0"
16"	L/360	-----	3'11"	5' 3"	7'10"	9'11"
16"	L/180	-----	4' 9"	7' 4"	9' 6"	11' 0"
12"	L/360	-----	4' 7"	6' 1"	8'11"	11' 0"
12"	L/180	-----	5'11"	8' 5"	11' 0"	11' 0"

OVERHANG DETAIL



ALT. GABLE SHAPES:

Note: All Plates Are 2X4 Except As Shown.

PLT TYP. Wave TPI-95 Design Crit: TPI-1995(STD)

R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/2006

R2: REVISED FOR ASCE 7-02.
DLJ 09/30/2005
R1 REV 2 5 02 JWC
HI/-/1/-/-/R/-

DETAIL: 140GC
Scale =.375"/Ft.

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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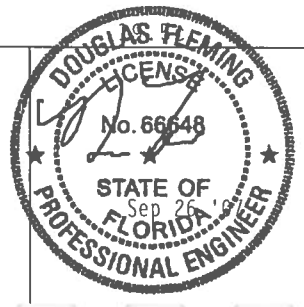


TC LL	30.0 PSF	REF R001-- 0
TC DL	7.0 PSF	DATE 03/27/02
BC DL	10.0 PSF	DRW HCUSR001 02086015
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	47.0 PSF	SEQN- 24860
DUR.FAC.	1.33	
SPACING	24.0"	JREF- 1SV3001 R03

(PIGGYBACK)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

TOP CHORD 2x4 SP SP #2N
BOT CHORD 2x4 SP #3 OR #2N
WEBS 2x4 SP #3

*,E (4) 0.131"X1.375" SCOTCH NAILS OR EQUAL IN EACH MEMBER. TRULOX PLATE TO BE APPLIED TO EACH FACE AT 2'0" O.C. MAXIMUM SPACING, REFER TO DRAWING 142 FOR TRULOX INFORMATION. PLATES ON THE FRONT FACE OF TRUSS MAY BE OFFSET FROM THE PLATES ON THE BACK FACE AS LONG AS PLATES ARE SPACED 4'0" OC. MAX.

NOTE: PIGGYBACK VERTICALS TO BE SPACED AT 4'0" O.C. MAXIMUM.

** MAXIMUM SIZE OF 2X12, #2 HEM-FIR OR BETTER.

E - 4X6 ALPINE, 3X6 TRULOX AT 2'0" OC. MAX.

++ PIGGYBACY BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH W1.5X3 ALPINE.

* - 3X8 TRULOX PLATE OR ALPINE PIGGYBACK SPECIAL PLATE (SEE DRWG. 847,847)

TRUSSES BUILT PER THIS DETAIL DESIGNED TO BE USED FOR THE FOLLOWING:
140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-98, PART. ENC.BLDG, CAT II, EXP C.

140 MPH WIND, 30.0 FT MEAN HGT, ASCE 7-02, PART. ENC.BLDG, CAT II, EXP C.

NOTE: THIS DETAIL MAY ALSO BE USED FOR A MONO OR HIP-MONO PIGGYBACK USING A TYPE-C PLATE AT THE HIGH END. AND END VERTICAL WHICH IS GREATER THAN 6-0-0 IN LENGTH AND EXPOSED TO WIND MUST BE VERIFIED BY ALPINE ENGINEERED PRODUCTS.

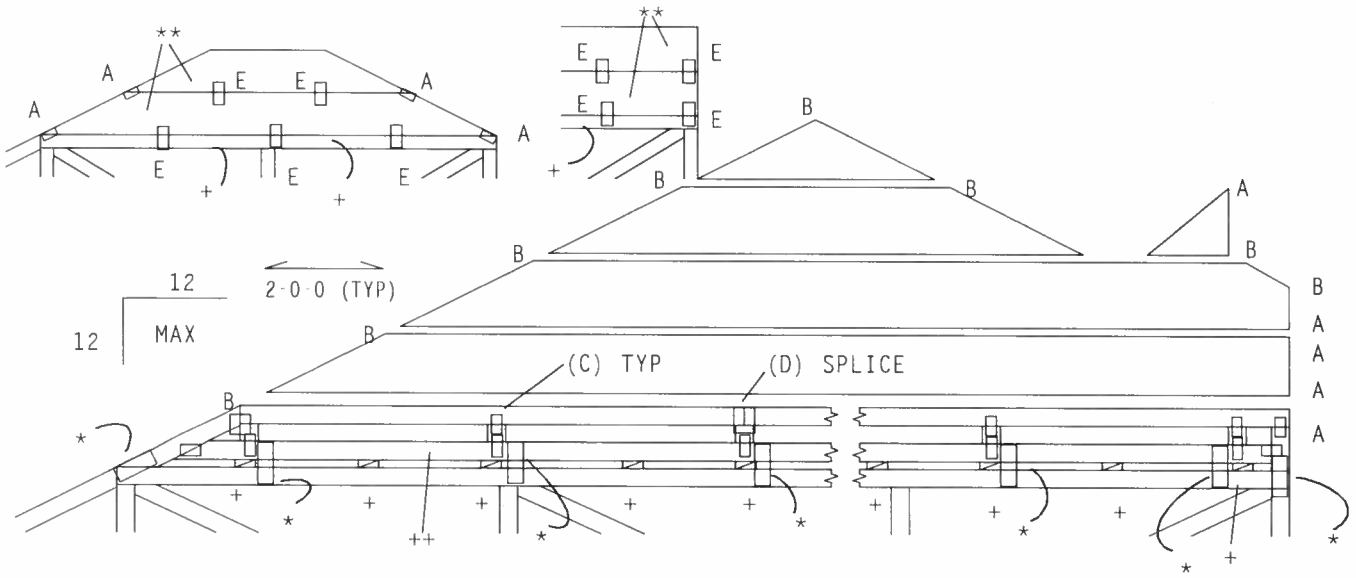
NOTE: TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED SO THAT ONE SPLICE IS NOT DIRECTLY OVER ANOTHER.

+ 2X4 CONTINUOUS LATERAL BRACING AT 24" OC. MAX SPACING. ATTACH TO TOP SIDE OF SUPPORTED TRUSS TOP CHORD WITH 2-16D NAILS IN EACH TRUSS.
OR
1X4 CONTINUOUS LATERAL BRACING AT 24" OC. MAX. SPACING. ATTACH TO BOTTOM SIDE OF SUPPORTED TRUSS TOP CHORD WITH 2-16D NAILS IN EACH TRUSS. BOTTOM CHORD OF PIGGYBACK SHOULD REST DIRECTLY ON THE TOP CHORD OF THE SUPPORTED TRUSS.

NOTE: BRACING MATERIAL IS TO BE ATTACHED TO A SUITABLE SUPPORT AT EACH END, AND MUST BE #3 HEM-FIR OR BETTER.

JOINT TYPE	SPANS UP TO 30'0"	34'0"	38'0"	42'0"
A	W2X4	W2X5	W3X4	W3X5
B	W5X4	W5X5	W5X5	W5X5
C	W1X3	W1X3	W1.5X3	W1.5X3
D	W5X4	W5X4	W5X5	W5X5

WEB BRACING
UP TO 7'9" NO BRACING
7'9" TO 12'3"-1X4 "T" BRACE, SAME GRADE AND LENGTH AS WEB, ATTACH WITH 8D NAILS AT 6" OC.
12'3" TO 14'0"-2X4 "T" BRACE, SAME GRADE AND LENGTH AS WEB, ATTACH WITH 16D NAILS AT 4" OC.



42-0-0 MAXIMUM PIGGYBACK SPAN

ALTERNATE LOADING:

TCLL 20	OR	30 PSF
TCDL 20		15 PSF
BCDL 10		10 PSF
TOTL 50		55 PSF
1.25		1.33

R1: REVISED FOR ASCE 7-02.
DLJ 09/30/2005

DETAIL: 140PB

PLT TYP. High Strength,Wave TPI-95 Design Criteria: TPI(STD) QTY:1 HI/-/1/-/-/R/-

ALPINE
Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA Sep 26, PROFESSIONAL ENGINEER

TC LL	30.0 PSF
TC DL	7.0 PSF
BC DL	10.0 PSF
BC LL	0.0 PSF
TOT.LD.	47.0 PSF
DUR.FAC.	1.33
SPACING	24.0"

REF	R001-- 0
DATE	03/27/02
DRW	HCUSR001 02086006
HC-ENG	DLJ/DLJ
SEQN	- 24938
JREF-	1SQV001 R38

(ASCE 7 98 GABLE END DETAILS ,140 MPH,EXP C, PART. ENC)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

(**) 2X4 SO. PINE #3 GABLE STUDS. ATTACH TO TOP CHORD, DIAGONAL WEBS, AND BOTTOM CHORD WITH W2X4 ALPINE PLATES. ALL (**) GABLE STUDS REQUIRED REINFORCING MEMBER. REINFORCING MEMBER MUST BE TOENAILED TO GABLE STUD WITH 0.131"X3" GUN NAILS AT 4" O.C. PLUS A CLUSTER OF 0.131"X3" TOENAILS AT THE TOP AND BOTTOM CHORD.SEE DETAIL FOR NAILING. SEE CHART FOR STUD BRACING AND SPACING OF VERTICALS.

NOTE:TRUSS ERECTOR IS RESPONSIBLE FOR PERMANENT WEB BRACING. WHEN BRACING IS REQUIRED, FURNISH A COPY OF THIS DRAWING TO TRUSS ERECTOR.

+PLATE AS REQUIRED ON APPROPRIATE DRAWING.

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NOTE: NAIL STEPS OF LADDER TRUSS ONTO THE OUTSIDE PIECES WITH 2-16D NAILS AT EACH END.

NOTE: ATTACH LADDER TRUSS TO TOP CHORD OF GABLE TRUSS WITH TWO ROWS OF 16D NAILS @ 8" O.C. STAGGERED 4"

+++ 7/16 MINIMUM APA RATED SHEATHING PROPERLY ATTACHED WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS.

140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-98, PART. ENCLOSED BLDG, CAT II, EXP. C.

140 MPH WIND, 30.00 FT MEAN HGT, ASCE 7-02, PART. ENCLOSED BLDG, CAT II, EXP. C.

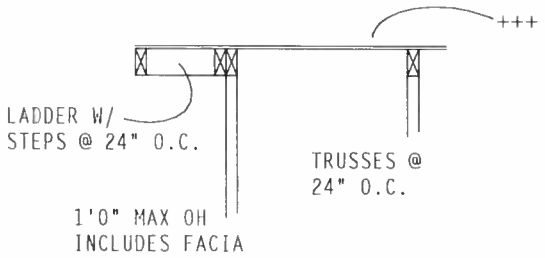
SEE APPROPRIATE ALPINE DRAWING FOR LUMBER,PLATES AND OTHER DATA NOT SHOWN HERE.

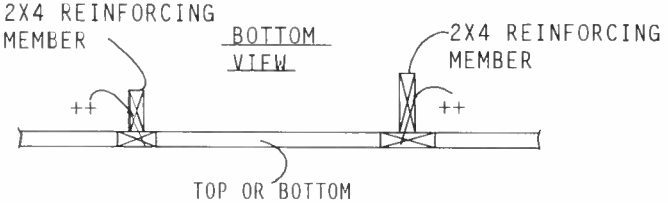
** STUD MUST BE ATTACHED TO CHORDS AND DIAGONAL

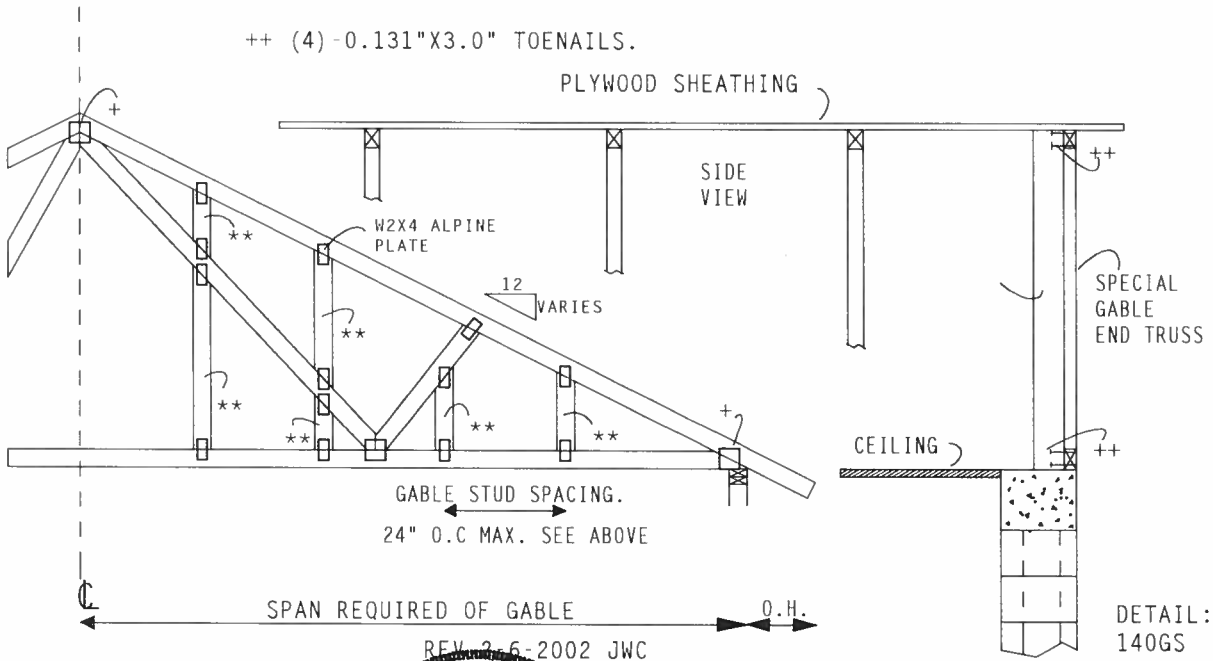
REINFORCING MEMBER REQUIRED	SPACING	MAX.LENGTH
2X4 SO. PINE #3	24"O.C.	2'- 1"
2X4 SO. PINE #3	16"O.C.	2'-10"
2X4 SO. PINE #3	12"O.C.	3'- 5"
2X6 SO. PINE #2 N	24"O.C.	5'- 0"
2X6 SO. PINE #2 N	16"O.C.	6'- 2"
2X6 SO. PINE #2 N	12"O.C.	7'- 1"
2X8 SO. PINE #2 N	24"O.C.	7'- 6"
2X8 SO. PINE #2 N	16"O.C.	9'- 1"
2X8 SO. PINE #2 N	12"O.C.	10'- 4"

R2: REVISED FOR ASCE 7-02
DLJ 09/30/2005

R3: REVISED DIAPHRAGM NOTE.
DLJ 02/27/2006







Note: All Plates Are 2X4 Except As Shown.

DETAIL: 140GS

REV 2-6-2002 JWC

PLT TYP. Wave TPI-95 Design Criteria: TPI(STD) QTY:1 HI/-/1/-/-/R/- Scale =.3125"/Ft.

ALPINE

Alpine Engineered Products, Inc.
1950 Marley Drive
Haines City, FL 33844
Certificate of Authorization # 567

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA Sep 26, '06 PROFESSIONAL ENGINEER

TC LL	30.0 PSF	REF R001-- 0
TC DL	15.0 PSF	DATE 03/27/02
BC DL	10.0 PSF	DRW HCUSR001 02086012
BC LL	0.0 PSF	HC-ENG DLJ/DLJ
TOT.LD.	55.0 PSF	SEQN - 24104
DUR.FAC.	1.33	FROM HC
		JRFF- 1SV3001 R03