Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0609-69 Date Received 9/26 By JW Permit # 25078/1228

Application Approved by - Zoning Official BLK Date 03.10.06 Plans Examiner OK JTH Date 9-30-06

Flood Zone X Pp Dot Development Permit N/A Zoning RSF-2 Land Use Plan Map Category Res. Low Den

Comments ~~ Ex Health ... ~~

Floor Height Letter Est. 1st Floor to be 128.5' Elevation Letter Required

Fax: 487-0396

Applicants Name SUSAN FAIR Phone 752-1711

Address 180 NW AMENITY CT. LAKE CITY FL 32055

Owners Name CORNERSTONE DEVELOPERS Phone 752-1711

911 Address 101 NW ETHELEND CT. LAKE CITY, FL. 32055

Contractors Name BRYAN ZECHER Phone 752-8653

Address PO BOX 815 LAKE CITY FL. 32056

Fee Simple Owner Name & Address NA

Bonding Co. Name & Address NA

Architect/Engineer Name & Address MARK DISOSWAY PO BOX 868 LAKE CITY, FL 32056

Mortgage Lenders Name & Address NA

Circle the correct power company - (FL Power & Light) - Clay Elec. - Suwannee Valley Elec. - Progressive Energy

Property ID Number 28-3S-16-02377-110 Estimated Cost of Construction 100,000

Subdivision Name MAGNOLIA HILLS Lot 10 Block ____ Unit ____ Phase ____

Driving Directions GO DOWN BROWN RD TO NW ETHAN PLACE, TURN RIGHT, GO TO NW KATELYN WAY, TURN RIGHT & THEN LEFT ONTO NW BILLYE, GO TO NW ETHELIND COURT. TURN RIGHT. HOUSE AT END OF CULDESAC

Type of Construction FRAME & HARDI SID Number of Existing Dwellings on Property 0

Total Acreage 1.25 Lot Size 1.25 acre Do you need a - (Culvert Permit) or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 35' L.Side 38' R Side 38' Rear 300

Total Building Height 19' 9½" Number of Stories 1 Heated Floor Area 1799 SQ FT. Roof Pitch 6/12

TOTAL 2431

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Susan Fair

Owner Builder or Agent (Including Contractor)

Contractor Signature

Contractors License Number CBC054575

Competency Card Number

NOTARY STAMP/SEAL

Chris W. Cox
Commission #DD308891
Expires: Apr 17, 2008
Bonded Thru
Atlantic Bonding Co., Inc.

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me

this 26TH day of SEPTEMBER 20 06

Personally known ✓ or Produced Identification ____

Notary Signature

FROM :COLUMBIA CO BUILDING + ZONING FAX NO. :386-758-2160 Jun. 16 2006 09:00AM P2

JW called & left message w/ Sctg: (Summer) 10.3.06

Columbia County Building Department
Culvert Permit

Culvert Permit No.
000001228

DATE 10/04/2006 PARCEL ID # 28-3S-16-02377-110

APPLICANT SUSAN FAIR PHONE 386.752.1711

ADDRESS 180 NW AMENITY COURT LAKE CITY FL 32055

OWNER CORNERSTONE DEVELOPERS PHONE 386.752.1711

ADDRESS 101 NW ETHELIND COURT LAKE CITY FL 32055

CONTRACTOR BRYAN ZECHER PHONE 386.752.8653

LOCATION OF PROPERTY 90-W TO BROWN RD,TR TO ETHAN PL,TR TO KATELYN WAY,TR TO BILLYE,TL TO ETHELIND,TR AND THE LOT IS @ THE VERY END OF THE CUL-DE-SAC.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT MAGNOLIA HILLS 10

SIGNATURE ✓ Susan Fair

INSTALLATION REQUIREMENTS

☐ Culvert size will be 18 inches in diameter with a total lenght of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:
a) a majority of the current and existing driveway turnouts are paved, or;
b) the driveway to be served will be paved or formed with concrete.
Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

☐ Culvert installation shall conform to the approved site plan standards.

☐ Department of Transportation Permit installation approved standards.

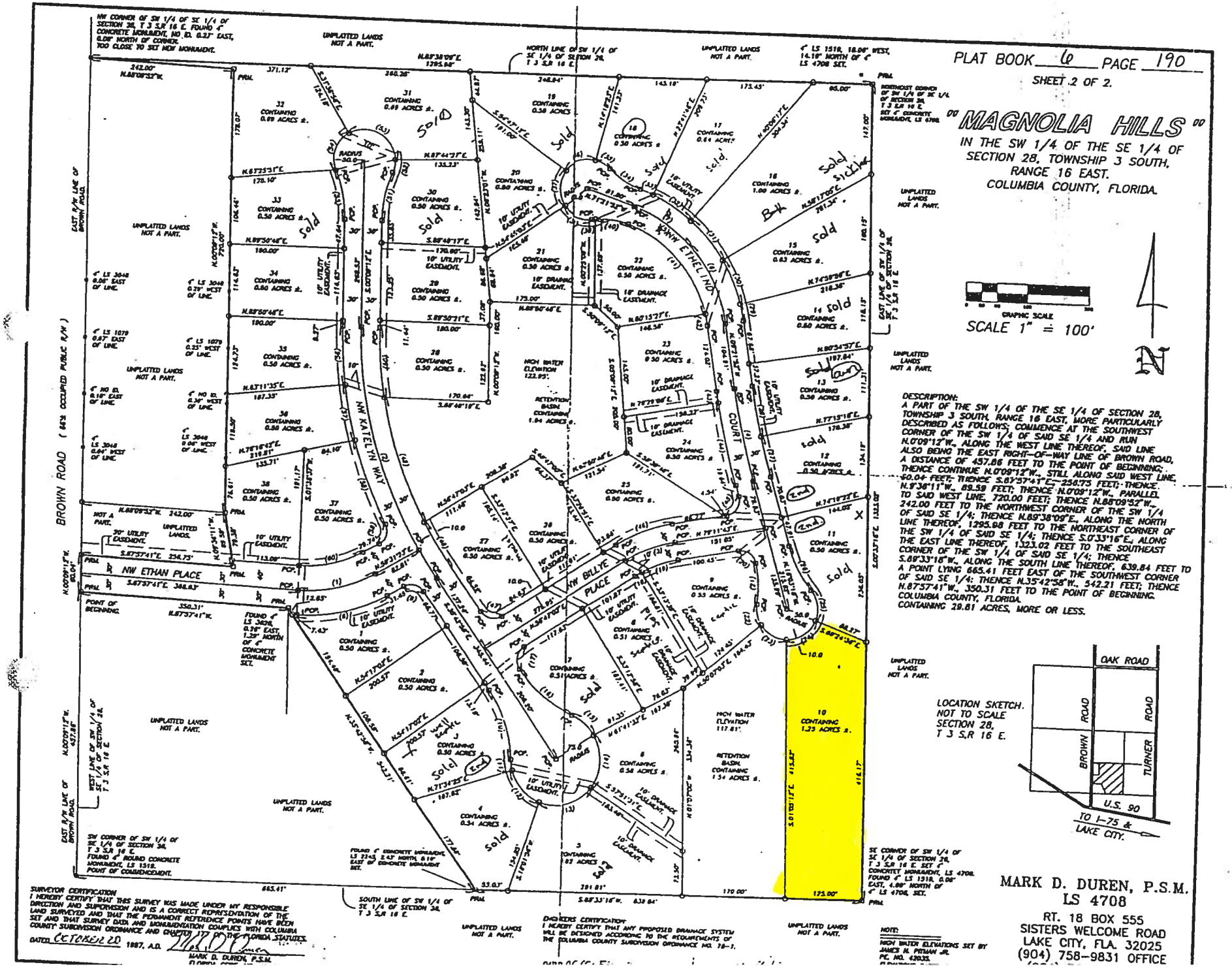
☒ Other WAIVER REQUEST APPROED FOR 18"X24' MITERED CULVERTS IN MAGNOLIA HILLS S.D.

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

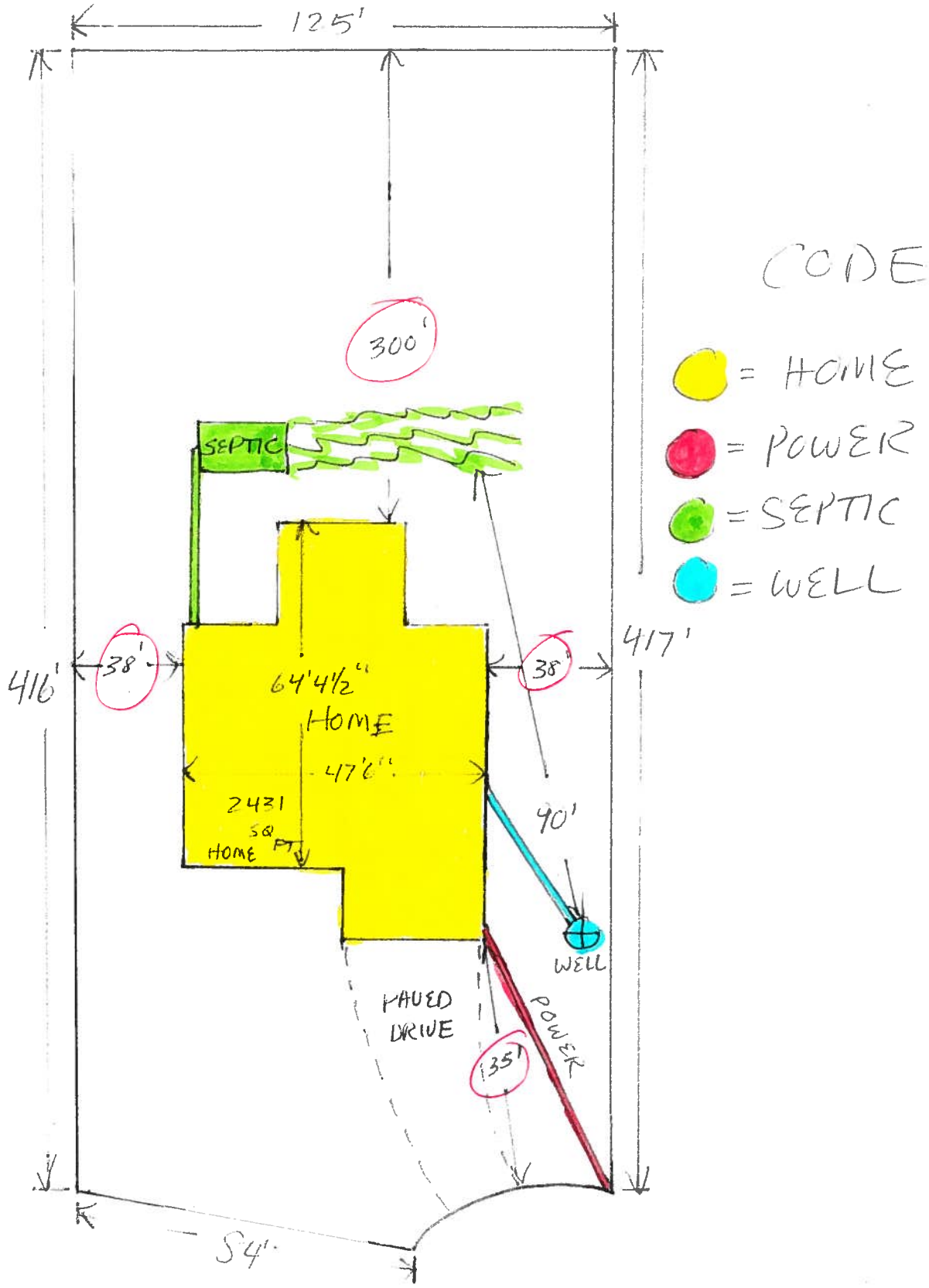
135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00

COLUMBIA COUNTY GATEWAY TO FLORIDA


PLAT BOOK 6 PAGE 190
SHEET 2 OF 2.
MAGNOLIA HILLS
IN THE SW 1/4 OF THE SE 1/4 OF SECTION 28, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA.
SCALE 1" = 100'
DESCRIPTION:
A PART OF THE SW 1/4 OF THE SE 1/4 OF SECTION 28, TOWNSHIP 3 SOUTH, RANGE 16 EAST, MORE PARTICULARLY DESCRIBED AS FOLLOWS; COMMENCE AT THE SOUTHWEST CORNER OF THE SW 1/4 OF SAID SE 1/4 AND RUN N.0°09'12"W., ALONG THE WEST LINE THEREOF, SAID LINE ALSO BEING THE EAST RIGHT-OF-WAY LINE OF BROWN ROAD, A DISTANCE OF 457.86 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE N.0°09'12"W., STILL ALONG SAID WEST LINE, 60.04 FEET; THENCE S.63°57'41"E., 258.75 FEET; THENCE N.9°36'11"W., 89.59 FEET; THENCE N.0°09'12"W., PARALLEL TO SAID WEST LINE, 720.00 FEET; THENCE N.88°09'52"W., 242.00 FEET TO THE NORTHWEST CORNER OF THE SW 1/4 OF SAID SE 1/4; THENCE N.89°38'09"E., ALONG THE NORTH LINE THEREOF, 1295.98 FEET TO THE NORTHEAST CORNER OF THE SW 1/4 OF SAID SE 1/4; THENCE S.0°33'16"E., ALONG THE EAST LINE THEREOF, 1323.02 FEET TO THE SOUTHEAST CORNER OF THE SW 1/4 OF SAID SE 1/4; THENCE S.89°33'16"W., ALONG THE SOUTH LINE THEREOF, 639.84 FEET TO A POINT LYING 665.41 FEET EAST OF THE SOUTHWEST CORNER OF SAID SE 1/4; THENCE N.35°42'58"W., 542.21 FEET; THENCE N.87°57'41"W., 350.31 FEET TO THE POINT OF BEGINNING. COLUMBIA COUNTY, FLORIDA. CONTAINING 29.81 ACRES, MORE OR LESS.
BROWN ROAD (66' OCCUPIED PUBLIC R/W)
NW ETHAN PLACE
NW KAYLEIGH WAY
NW ETHELIND COURT
NW BILLYE PLACE
UNPLATTED LANDS NOT A PART.
LOCATION SKETCH. NOT TO SCALE SECTION 28, T 3 S,R 16 E.
OAK ROAD
BROWN ROAD
TURNER ROAD
U.S. 90
TO I-75 & LAKE CITY.
SURVEYOR CERTIFICATION
I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE DIRECTION AND SUPERVISION AND IS A CORRECT REPRESENTATION OF THE LAND SURVEYED AND THAT THE PERMANENT REFERENCE POINTS HAVE BEEN SET AND THAT SURVEY DATA AND MONUMENTATION COMPLIES WITH COLUMBIA COUNTY SUBDIVISION ORDINANCE AND CHAPTER 177 OF THE FLORIDA STATUTES.
DATED OCTOBER 20 1887, A.D.
MARK D. DUREN, P.S.M.
ENGINEERS CERTIFICATION
I HEREBY CERTIFY THAT ANY PROPOSED DRAINAGE SYSTEM WILL BE DESIGNED ACCORDING TO THE REQUIREMENTS OF THE COLUMBIA COUNTY SUBDIVISION ORDINANCE NO. 78-7.
NOTE:
MARK D. DUREN, P.S.M.
LS 4708
RT. 18 BOX 555
SISTERS WELCOME ROAD
LAKE CITY, FLA. 32025
(904) 758-9831 OFFICE

MAGNOLIA HILLS
COLUMBIA COUNTY
LOT 10 SITE PLAN



Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

July 24, 2006

Cornerstone Developers, LLC
180 NW Amenity Court
Lake City, Florida 32055

Attention: Chris Cox

Reference: Proposed Residence
Lot 10, Magnolia Hills
Columbia County, Florida
Cal-Tech Project No. 06-451

Dear Mr. Cox,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lot 10 of Magnolia Hills in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of a home to be constructed on the lot and to provide recommendations for selecting a finished floor elevation.

Based upon the U. S. Coast and Geodetic Survey marker "BP19" located near the intersection of U. S. 90 and Brown Road, the elevation of the center of the cul-de-sac adjacent the proposed building site is approximately 130.9 feet. The ground surface elevation at the center of the building site is approximately 125.7 feet, and the proposed finished floor elevation is approximately 128.5 feet. This places the finished floor elevation about 2.4 feet below the elevation of the center of the cul-de-sac.

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the USGS topographic quadrangle map, the proposed home site is positioned within an isolated basin of approximately 100 acres for which the basin rim is estimated to be near elevation 153 feet. The FEMA flood map of Columbia County indicates there are no flood zones within this basin. Additionally, only one flood zone located within about one mile of the site has a flood elevation estimated to exceed 153 feet. This zone "A" flood area is located approximately 1/4 mile west of the site, and the flood elevation for this area is estimated to be about 155 feet; however, this flood area is topographically isolated such that flooding within this flood zone would not affect the proposed home site.

Since no flood areas are delineated within the basin by the FEMA flood map, we believe flooding within the basin sufficient to flood the home is highly unlikely. Such flooding would require maximum flood depths on the order of 15 feet within the basin and an average flood depth of about 6 feet over an area of about 20 acres.

Based upon our evaluation, it is our opinion elevating the finished floor to 1 foot above the adjacent roadway will not be required. However, we recommend the finished floor elevation be selected and/or the site be graded such that the finished floor is a minimum of 12 inches above the finished surface grade at the perimeter of the residence. With suitable site grading to provide separation between the finished floor and the surrounding surface grade, the proposed finished floor elevation of approximately 128.5 feet should be sufficient to substantially reduce the likelihood of flooding at the site.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

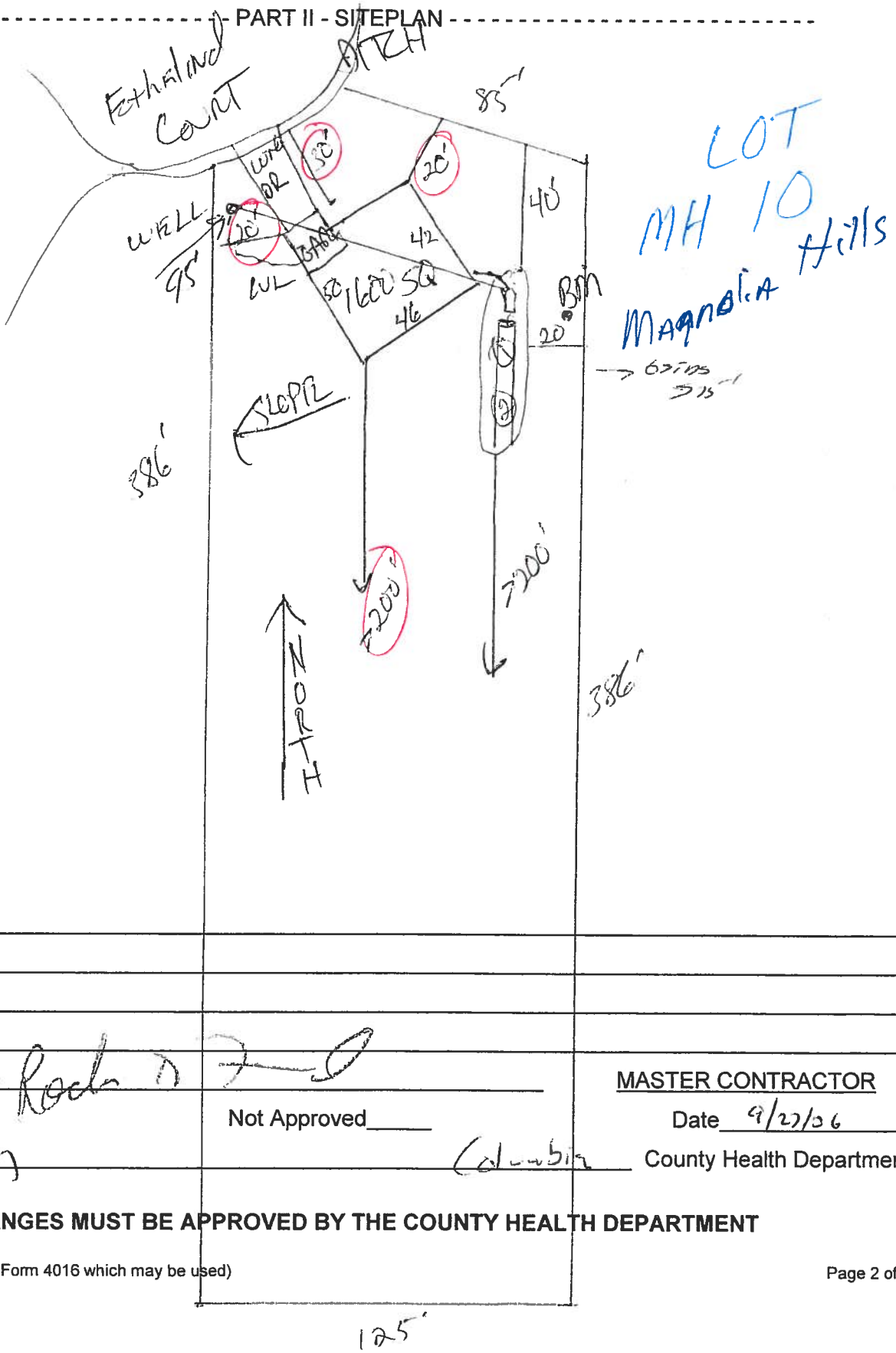
John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer
7/24/06
5/26/12

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 06-0856N

PART II - SITEPLAN

Scale: 1 inch = 50 feet.



Notes: _____

Site Plan submitted by: _____

Plan Approved ✓ Not Approved _____

By _____ Columbia County Health Department

MASTER CONTRACTOR

Date 9/27/06

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

NOTICE OF COMMENCEMENT FORM
COLUMBIA COUNTY, FLORIDA

THE UNDERSIGNED hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Tax Parcel ID Number 28-3S-16-02377-110

1. Description of property: (legal description of the property and street address or 911 address)
   Lot 10 Magnolia Hills
   101 NW Ashleigh Ct.
   Lake City, FL 32055

2. General description of improvement: New Home

3. Owner Name & Address Cornerstone Developers, LLC, 180 N.W. Amenity Ct, Lake City, FL 32055 Interest in Property Fee Simple

4. Name & Address of Fee Simple Owner (if other than owner): _____

5. Contractor Name Bryan Zecher Construction Phone Number 386-752-8653
   Address P.O. Box 815, Lake City, FL 32056

6. Surety Holders Name N/A Phone Number _____
   Address _____
   Amount of Bond N/A

7. Lender Name N/A
   Address _____

Inst:2006014779 Date:06/20/2006 Time:13:04
DC,P.DeWitt Cason,Columbia County B:1087 P:716

8. Persons within the State of Florida designated by the Owner upon whom notices or other documents may be served as provided by section 718.13 (1)(a) 7; Florida Statutes:
   Name N/A Phone Number _____
   Address _____

9. In addition to himself/herself the owner designates N/A of _____ to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) – (a) 7. Phone Number of the designee N/A

10. Expiration date of the Notice of Commencement (the expiration date is 1 (one) year from the date of recording, (Unless a different date is specified) N/A

NOTICE AS PER CHAPTER 713, Florida Statutes:
The owner must sign the notice of commencement and no one else may be permitted to sign in his/her stead.

Signature of Owner

SUSAN ANN FAIR
MY COMMISSION # DD 342845
EXPIRES: August 14, 2008
Bonded Thru Notary Public Underwriters

Sworn to (or affirmed) and subscribed before day of June 19, 2006

NOTARY STAMP/SEAL

Susan Ann Fair

Signature of Notary

Prepared by and return to:

Home Town Title of North Florida
2744 US Highway 90 West
Lake City, FL 32055
386-754-7175
File Number: **2005-2188**

Inst:2006003506 Date:02/13/2006 Time:16:16
Doc Stamp-Deed : 3276.00
DC,P.DeWitt Cason,Columbia County B:1073 P:2551

[Space Above This Line For Recording Data]

Warranty Deed

This Warranty Deed made this **9th** day of **February, 2006** between **James Lambert Hair, II, a married person not residing on subject property** whose post office address is **314 North Marion Avenue, Lake City, FL 32055**, grantor, and **Cornerstone Developers, LLC, a Florida Limited Liability Company** whose post office address is **180 NW Amenity Court, Lake City, FL 32055**, grantee:

(Whenever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives, and assigns of individuals, and the successors and assigns of corporations, trusts and trustees)

Witnesseth, that said grantor, for and in consideration of the sum of TEN AND NO/100 DOLLARS ($10.00) and other good and valuable considerations to said grantor in hand paid by said grantee, the receipt whereof is hereby acknowledged, has granted, bargained, and sold to the said grantee, and grantee's heirs and assigns forever, the following described land, situate, lying and being in **Columbia County, Florida** to-wit:

Lots 1, 2, 6, 9, 10, 20, 22, 23, 24, 25, 26, 28, 29, 33, 35, 36, 37 and 38 of MAGNOLIA HILLS, according to the plat thereof as recorded in Plat Book 6, Page 189, public records of Columbia County, Florida.

Parcel Identification Number: R023777-101,102,106,109,120,110,122

Together with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

To Have and to Hold, the same in fee simple forever.

And the grantor hereby covenants with said grantee that the grantor is lawfully seized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to **December 31, 2005.**

In Witness Whereof, grantor has hereunto set grantor's hand and seal the day and year first above written.

Signed, sealed and delivered in our presence:

Witness Name: BARBARA A. FRADDOSIO

Witness Name: DAWNA W. LANG

James Lambert Hair, II (Seal)

State of Florida
County of Columbia

The foregoing instrument was acknowledged before me this 9th day of February, 2006 by James Lambert Hair, II, who [_] is personally known or [X] has produced a driver's license as identification.

[Notary Seal]

Notary Public

DAWNA W. LANG
MY COMMISSION # DD 154936
EXPIRES: October 3, 2006
Bonded Thru Notary Public Underwriters

Printed Name: DAWNA W. LANG

My Commission Expires:

DAWNA W. LANG
MY COMMISSION # DD 154936
EXPIRES: October 3, 2006
Bonded Thru Notary Public Underwriters

Inst:2006003506 Date:02/13/2006 Time:16:16
Doc Stamp-Deed : 3276.00
DC,P.DeWitt Cason,Columbia County B:1073 P:2552

HALL'S PUMP & WELL SERVICE, INC.

SPECIALIZING IN 4"-6" WELLS

DONALD AND MARY HALL
OWNERS

PHONE (904) 752-1854
FAX (904) 755-7022
~~X724 NORTH FIRST STREET X~~
LAKE CITY, FLORIDA 32055
904 NW Main Blvd.

June 12, 2002

NOTICE TO ALL CONTRACTORS

Please be advised that due to the new building codes we will use a large capacity diaphram tank on all new wells. This will insure a minimum of one (1) minute draw down or one (1) minute refill. If a smaller diaphram tank is used then we will install a cycle stop valve which will produce the same results.

If you have any questions please feel free to call our office anytime.

Thank you,

Donald D. Hall
DDH/jk

FORM 600A-2004 EnergyGauge® 4.1

FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name:	**608232CornerStoneTheGriffin**	Builder:	
Address:	**Lot: 10, Sub: Magnolia Hills, Plat:**	Permitting Office:	Columbia
City, State:	**Lake City, FL**	Permit Number:	
Owner:		Jurisdiction Number:	22100C
Climate Zone:	**North**		

1. New construction or existing	New	12. Cooling systems	
2. Single family or multi-family	Single family	a. Central Unit	Cap: 32.0 kBtu/hr SEER: 10.00
3. Number of units, if multi-family	1	b. N/A	
4. Number of Bedrooms	3	c. N/A	
5. Is this a worst case?	Yes	13. Heating systems	
6. Conditioned floor area (ft²)	1799 ft²	a. Electric Heat Pump	Cap: 32.0 kBtu/hr HSPF: 7.00
7. Glass type[1] and area: (Label reqd. by 13-104.4.5 if not default)		b. N/A	
a. U-factor: (or Single or Double DEFAULT)	Description Area 7a. (Dble Default) 156.0 ft²	c. N/A	
b. SHGC: (or Clear or Tint DEFAULT)	7b. (Clear) 156.0 ft²	14. Hot water systems	
8. Floor types		a. Electric Resistance	Cap: 40.0 gallons EF: 0.93
a. Slab-On-Grade Edge Insulation	R=0.0, 184.0(p) ft	b. N/A	
b. N/A		c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)	
c. N/A		15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)	
9. Wall types			
a. Frame, Wood, Exterior	R=13.0, 1002.0 ft²		
b. Frame, Wood, Exterior	R=13.0, 233.0 ft²		
c. N/A			
d. N/A			
e. N/A			
10. Ceiling types			
a. Under Attic	R=30.0, 1859.0 ft²		
b. N/A			
c. N/A			
11. Ducts			
a. Sup: Unc. Ret: Unc. AH: Garage	Sup. R=6.0, 170.0 ft		
b. N/A			

Glass/Floor Area: 0.09 Total as-built points: 25543 Total base points: 26353 PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [signature]

DATE: 9-7-06

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: [signature]

DATE: 9-26-06

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL: _____

DATE: _____

SEAL OF THE STATE OF FLORIDA IN GOD WE TRUST

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 10, Sub: Magnolia Hills, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
GLASS TYPES .18 X Conditioned Floor Area	X BSPM	= Points		Type/SC	Overhang Ornt	Len	Hgt	Area X	SPM X	SOF	= Points
.18	1799.0	20.04	6489.4	Double, Clear	E	1.5	5.5	45.0	42.06	0.90	1696.4
				Double, Clear	E	1.5	6.5	20.0	42.06	0.93	779.5
				Double, Clear	N	1.5	5.5	15.0	19.20	0.93	267.3
				Double, Clear	W	1.5	5.5	60.0	38.52	0.90	2073.1
				Double, Clear	W	6.0	7.2	13.3	38.52	0.57	291.9
				Double, Clear	S	1.5	1.2	2.7	35.87	0.49	47.2
				As-Built Total:				**156.0**			**5155.4**

WALL TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Adjacent	0.0	0.00	0.0	Frame, Wood, Exterior	13.0	1002.0	1.50	1503.0
Exterior	1235.0	1.70	2099.5	Frame, Wood, Exterior	13.0	233.0	1.50	349.5
Base Total:	1235.0		2099.5	As-Built Total:		1235.0		1852.5

DOOR TYPES	Area X	BSPM	= Points	Type	Area X	SPM	= Points
Adjacent	20.0	1.60	32.0	Exterior Insulated	20.0	4.10	82.0
Exterior	40.0	4.10	164.0	Exterior Insulated	20.0	4.10	82.0
				Adjacent Insulated	20.0	1.60	32.0
Base Total:	60.0		196.0	As Built Total:	60.0		196.0

CEILING TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM X	SOF	= Points
Under Attic	1799.0	1.73	3112.3	Under Attic	30.0	1859.0	1.73	1	3216.1
Base Total:	1799.0		3112.3	As-Built Total:		1859.0			3216.1

FLOOR TYPES	Area X	BSPM	= Points	Type	R-Value	Area X	SPM	= Points
Slab	184.0(p)	-37.0	-6808.0	Slab-On-Grade Edge Insulation	0.0	184.0(p	-41.20	-7580.8
Raised	0.0	0.00	0.0					
Base Total:			-6808.0	As-Built Total:		184.0		-7580.8

INFILTRATION	Area X	BSPM	= Points		Area X	SPM	= Points
	1799.0	10.21	18373.6		1799.0	10.21	18373.6

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 10, Sub: Magnolia Hills, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Summer Base Points: 23456.9			**Summer As-Built Points:**					**21207.0**
Total Summer Points	X System Multiplier	= Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Cooling Points
			(sys 1: Central Unit 32000 btuh ,SEER/EFF(10.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0(INS)					
			21207	1.00	(1.09 x 1.147 x 1.00)	0.341	1.000	9049.1
23456.9	**0.4266**	**10006.7**	**21207.0**	**1.00**	**1.250**	**0.341**	**1.000**	**9049.1**

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 10, Sub: Magnolia Hills, Plat: , Lake City, FL,** PERMIT #:

BASE

GLASS TYPES

.18 X	Conditioned Floor Area	X BWPM	= Points
.18	1799.0	12.74	4125.5

WALL TYPES

	Area X	BWPM	= Points
Adjacent	0.0	0.00	0.0
Exterior	1235.0	3.70	4569.5
Base Total:	1235.0		4569.5

DOOR TYPES

	Area X	BWPM	= Points
Adjacent	20.0	8.00	160.0
Exterior	40.0	8.40	336.0
Base Total:	60.0		496.0

CEILING TYPES

	Area X	BWPM	= Points
Under Attic	1799.0	2.05	3687.9
Base Total:	1799.0		3687.9

FLOOR TYPES

	Area X	BWPM	= Points
Slab	184.0(p)	8.9	1637.6
Raised	0.0	0.00	0.0
Base Total:			1637.6

INFILTRATION

Area X	BWPM	= Points
1799.0	0.59	-1061.4

AS-BUILT

Type/SC	Overhang Ornt	Len	Hgt	Area X	WPM X	WOF =	Points
Double, Clear	E	1.5	5.5	45.0	18.79	1.04	880.6
Double, Clear	E	1.5	6.5	20.0	18.79	1.03	387.4
Double, Clear	N	1.5	5.5	15.0	24.58	1.00	369.8
Double, Clear	W	1.5	5.5	60.0	20.73	1.03	1278.6
Double, Clear	W	6.0	7.2	13.3	20.73	1.15	316.7
Double, Clear	S	1.5	1.2	2.7	13.30	3.11	111.8
As-Built Total:				**156.0**			**3344.9**

Type	R-Value	Area X	WPM	= Points
Frame, Wood, Exterior	13.0	1002.0	3.40	3406.8
Frame, Wood, Exterior	13.0	233.0	3.40	792.2
As-Built Total:		1235.0		41 .

Type	Area X	WPM	= Points
Exterior Insulated	20.0	8.40	168
Exterior Insulated	20.0	8.40	168.0
Adjacent Insulated	20.0	8.00	160.0
As Built Total:	60.0		496.0

Type	R-Value	Area X	WPM X	WCM =	Points
Under Attic	30.0	1859.0	2.05 X	1.00	3810.9
As-Built Total:		1859.0			3810.9

Type	R-Value	Area X	WPM	= Points
Slab-On-Grade Edge Insulation	0.0	184.0(p	18.80	3459.2
As-Built Total:		184.0		3459.2

Area X	WPM	= Points
1799.0	0.59	-1061.4

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 10, Sub: Magnolia Hills, Plat: , Lake City, FL,** PERMIT #:

BASE			AS-BUILT					
Winter Base Points:		**13455.1**	**Winter As-Built Points:**					**14248.7**
Total Winter Points	X System Multiplier	= Heating Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	= Heating Points
			(sys 1: Electric Heat Pump 32000 btuh ,EFF(7.0) Ducts:Unc(S),Unc(R),Gar(AH),R6.0					
			14248.7	1.000	(1.069 x 1.169 x 1.00)	0.487	1.000	8674.1
13455.1	**0.6274**	**8441.7**	**14248.7**	**1.00**	**1.250**	**0.487**	**1.000**	**8674.1**

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 10, Sub: Magnolia Hills, Plat: , Lake City, FL,** PERMIT #:

BASE				AS-BUILT							
WATER HEATING											
Number of Bedrooms	X	Multiplier	= Total	Tank Volume	EF	Number of Bedrooms	X	Tank Ratio	X Multiplier	X Credit Multiplier	= Total
3		2635.00	7905.0	40.0	0.93	3		1.00	2606.67	1.00	7820.0
				As-Built Total:							**7820.0**

CODE COMPLIANCE STATUS

BASE				AS-BUILT			
Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points	Cooling Points	+ Heating Points	+ Hot Water Points	= Total Points
10007	8442	7905	26353	9049	8674	7820	25543

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: **Lot: 10, Sub: Magnolia Hills, Plat: , Lake City, FL,** PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers, combustion space heaters have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences)

COMPONENTS	SECTION	REQUIREMENTS
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1 A C.3.2. Switch or clearly mar breaker (electric) or cutoff (gas) must be provided. External or built-in he t trap re
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-comm ial po l must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 83.3

The higher the score, the more efficient the home.

, Lot: 10, Sub: Magnolia Hills, Plat: , Lake City, FL,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family	Single family		a. Central Unit	Cap: 32.0 kBtu/hr
3.	Number of units, if multi-family	1			SEER: 10.00
4.	Number of Bedrooms	3		b. N/A	
5.	Is this a worst case?	Yes			
6.	Conditioned floor area (ft²)	1799 ft²		c. N/A	
7.	Glass type [1] and area: (Label reqd. by 13-104.4.5 if not default)				
	a. U-factor:	Description Area	13.	Heating systems	
	(or Single or Double DEFAULT) 7a.	(Dble Default) 156.0 ft²		a. Electric Heat Pump	Cap: 32.0 kBtu/hr
	b. SHGC:				HSPF: 7.00
	(or Clear or Tint DEFAULT) 7b.	(Clear) 156.0 ft²		b. N/A	
8.	Floor types				
	a. Slab-On-Grade Edge Insulation	R=0.0, 184.0(p) ft		c. N/A	
	b. N/A				
	c. N/A		14.	Hot water systems	
9.	Wall types			a. Electric Resistance	Cap: 40.0 gallons
	a. Frame, Wood, Exterior	R=13.0, 1002.0 ft²			EF: 0.93
	b. Frame, Wood, Exterior	R=13.0, 233.0 ft²		b. N/A	
	c. N/A				
	d. N/A			c. Conservation credits	
	e. N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
	a. Under Attic	R=30.0, 1859.0 ft²	15.	HVAC credits	
	b. N/A			(CF-Ceiling fan, CV-Cross ventilation,	
	c. N/A			HF-Whole house fan,	
11.	Ducts			PT-Programmable Thermostat,	
	a. Sup: Unc, Ret: Unc, AH: Garage	Sup: R=6.0, 170.0 ft		MZ-C-Multizone cooling,	
	b. N/A			MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Chris A. Cr Date: 9-26-06

Address of New Home: 101 NW ETHELEND CT. City/FL Zip: LAKE CITY, FL 32055

GREAT SEAL OF THE STATE OF FLORIDA * IN GOD WE TRUST *

NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a DEP Participating Builder), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.floridaenergygauge.com for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

25078

Cal-Tech Testing, Inc.
• Engineering
• Geotechnical
• Environmental
LABORATORIES

P.O. Box 1625 • Lake City, FL 32056-1625
6919 Distribution Avenue S., Unit #5 • Jacksonville, FL 32257

Tel. (386) 755-3633 • Fax (386) 752-5456
Tel. (904) 262-4046 • Fax (904) 262-4047

NOTE: THE SLAB ELEVATION IS 11" BELOW CUL-DE-SAC

July 24, 2006

Cornerstone Developers, LLC
180 NW Amenity Court
Lake City, Florida 32055

Attention: Chris Cox

Reference: Proposed Residence
Lot 10, Magnolia Hills
Columbia County, Florida
Cal-Tech Project No. 06-451

Dear Mr. Cox,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lot 10 of Magnolia Hills in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of a home to be constructed on the lot and to provide recommendations for selecting a finished floor elevation.

Based upon the U. S. Coast and Geodetic Survey marker "BP19" located near the intersection of U. S. 90 and Brown Road, the elevation of the center of the cul-de-sac adjacent the proposed building site is approximately 130.9 feet. The ground surface elevation at the center of the building site is approximately 125.7 feet, and the proposed finished floor elevation is approximately 128.5 feet. This places the finished floor elevation about 2.4 feet below the elevation of the center of the cul-de-sac.

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the USGS topographic quadrangle map, the proposed home site is positioned within an isolated basin of approximately 100 acres for which the basin rim is estimated to be near elevation 153 feet. The FEMA flood map of Columbia County indicates there are no flood zones within this basin. Additionally, only one flood zone located within about one mile of the site has a flood elevation estimated to exceed 153 feet. This zone "A" flood area is located approximately 1/4 mile west of the site, and the flood elevation for this area is estimated to be about 155 feet; however, this flood area is topographically isolated such that flooding within this flood zone would not affect the proposed home site.

Since no flood areas are delineated within the basin by the FEMA flood map, we believe flooding within the basin sufficient to flood the home is highly unlikely. Such flooding would require maximum flood depths on the order of 15 feet within the basin and an average flood depth of about 6 feet over an area of about 20 acres.

Based upon our evaluation, it is our opinion elevating the finished floor to 1 foot above the adjacent roadway will not be required. However, we recommend the finished floor elevation be selected and/or the site be graded such that the finished floor is a minimum of 12 inches above the finished surface grade at the perimeter of the residence. With suitable site grading to provide separation between the finished floor and the surrounding surface grade, the proposed finished floor elevation of approximately 128.5 feet should be sufficient to substantially reduce the likelihood of flooding at the site.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer 7/24/06
52612

UNIVERSAL
ENGINEERING SCIENCES
Consultant In: Geotechnical Engineering,
Environmental Sciences, Construction Materials Testing
4475 SW 35th Terrace, Gainesville, Florida 32608 (352) 372-3392

Project No.: 27915-008-01
Report No.: 7719
Date: October 31, 2006

REPORT ON IN-PLACE DENSITY TESTS

Permit 25078

Client: Cornerstone Development
P.O. Box 1867
Lake City, FL 32056

Project: Magnolia Hills, Lot No. 10, 180 NW Amenity Court, Permit No. 000025078, Lake City, Columbia County, FL

Area Tested: Fill Beneath Proposed Building Pad

Course: Final Grade **Depth of Test:** 0-1'

Type of Test: ASTM D-2922 **Date Tested:** 10-27-06

Remarks: The tests below meet the minimum 95 percent relative soil compaction requirement of Laboratory Modified Proctor maximum dry density. (ASTM D-1557)
RETEST

TEST LOCATION	LABORATORY RESULTS		FIELD TEST RESULTS		
Description of Test Location	Maximum Density (pcf)	Optimum Moisture (%)	Dry Density (pcf)	Field Moisture (%)	Soil Compaction (%)
1. Approximately 10' Northwest of Southeast Corner of Pad	115.5	10.0	110.9	5.1	96.0
2. Approximately 10' Northeast of Southwest Corner Pad	115.5	10.0	110.6	4.7	95.8
3. Approximately 15' Southwest of Northeast Corner of Pad	115.5	10.0	111.2	5.4	96.3

Technician: TG/ts

Keith L. Butts, P.E.
FL Professional Engineer No. 53986

UNIVERSAL
ENGINEERING SCIENCES
Consultant In: Geotechnical Engineering,
Environmental Sciences, Construction Materials Testing
4475 SW 35th Terrace, Gainesville, Florida 32608 (352) 372-3392

Project No.: 27915-008-01
Report No.: 7330
Date: October 25, 2006

REPORT ON
IN-PLACE DENSITY TESTS

PERMIT #25078

Client: Cornerstone Development
P.O. Box 1867
Lake City, FL 32056

Project: Magnolia Hills, Lot No. 10, Lake City, Columbia County, FL

Area Tested: Fill Beneath Proposed Building Pad

Course: Final Grade **Depth of Test:** 0-1'

Type of Test: ASTM D-2922 **Date Tested:** 10-11-06

Remarks: The tests below meet the minimum 95 percent relative soil compaction requirement of Laboratory Modified Proct maximum dry density (ASTM D-1557).

TEST LOCATION	LABORATORY RESULTS		FIELD TEST RESULTS		
Description of Test Location	Maximum Density (pcf)	Optimum Moisture (%)	Dry Density (pcf)	Field Moisture (%)	Soil Compaction (%)
1. Approximately 10' Southeast of Northwest Corner of Pad	115.5	10.0	114.0	6.0	98.7
2. Approximate Center of Pad	115.5	10.0	111.0	4.8	96.1
3. Approximately 10' Northwest of Southeast Corner of Pad	115.5	10.0	111.3	6.1	96.4

Technician: CS/lm

10/31/06
Keith L. Butts, P.E.
FL Professional Engineer No. 53986

UNIVERSAL
ENGINEERING SCIENCES
Consultant In: Geotechnical Engineering,
Environmental Sciences, Construction Materials Testing
4475 SW 35th Terrace, Gainesville, Florida 32608 (352) 372-3392

Project No.: 27915-008-01
Report No.: 7331
Date: October 25, 2006

REPORT ON
IN-PLACE DENSITY TESTS

PERMIT # 25078

Client: Cornerstone Development
P.O. Box 1867
Lake City, FL 32056

Project: Magnolia Hills, Lot No. 10, Permit No. 025078, Lake City, Columbia County, FL

Area Tested: Fill Beneath Proposed Building Foundation

Course: Final Grade **Depth of Test:** 0-1'

Type of Test: ASTM D-2922 **Date Tested:** 10-17-06

Remarks: The tests below meet the minimum 95 percent relative soil compaction requirement of Laboratory Modified Proct maximum dry density (ASTM D-1557).

TEST LOCATION	LABORATORY RESULTS		FIELD TEST RESULTS		
Description of Test Location	Maximum Density (pcf)	Optimum Moisture (%)	Dry Density (pcf)	Field Moisture (%)	Soil Compaction (%)
1. Approximately 5' South of Northeast Corner of Foundation	115.5	10.0	113.1	6.0	97.9
2. Approximate Center of West Side Foundation	115.5	10.0	111.6	5.2	96.6
3. Approximate Center of Center Interior Foundation	115.5	10.0	110.5	5.4	95.7

Technician: DH/lm

Keith L. Davis, P.E. 10/31/06
FL Professional Engineer No. 53986

25078

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest .com)
Address: Bava Ave
City Lake City Phone 752 1703

Site Location: Subdivision Magnolia Hills
Lot # 10 Block# _____ Permit # 25078
Address 101 NW ETHELWND COURT

Product used	Active Ingredient	% Concentration
☐ Premise	Imidacloprid	0.1%
☐ Termidor	Fipronil	0.12%
☑ Bora-Care	Disodium Octaborate Tetrahydrate	23.0%

Type treatment: ☐ Soil ☑ Wood

Area Treated	Square feet	Linear feet	Gallons Applied
Dwelling	2431	622	4.5

As per Florida Building Code 104.2.6 – If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____.

11/30/06 (Date) 0915 (Time) F254 / F299 (Print Technician's Name)

Remarks: _____

Applicator - White Permit File - Canary Permit Holder - Pink

10/05 ©

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4475 S.W. 35th Terrace • Gainesville, Florida 32608 • (352) 372-3392

REPORT ON IN-PLACE DENSITY TESTS

Permit # 000025078

CLIENT: Corner Stone

PROJECT: Magnolia Hills
780 N.W. Amenity Court Lake City, FL Columbia County

AREA TESTED: Fill ↓ prop. bldg. pad.

COURSE: F/G

DEPTH OF TEST: 0-1'

TYPE OF TEST: ASTMD-2922

DATE TESTED: 10-27-06

NOTE: The below tests DO/~~DO NOT~~ meet the minimum 95 % compaction requirements of maximum density.

REMARKS: * Retest

LOCATION OF TESTS	DRY DEN.	MAX. DEN.	% MAX. DEN.	% MOIST.	OPT. MOIST.
		115.5			10.0
App. 10 N.W. of S.E. corner of pad.	110.9	↓	96.0	5.1	↓
App. 10 N.E. of S.W. corner of pad	110.6	↓	95.8	4.7	↓
App. 15 S.W. of N.E. corner of pad	111.2	↓	96.3	5.4	↓

4404

TECH. T.G.

CERTIFICATE OF OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 28-3S-16-02377-110 **Building permit No.** 000025078

Use Classification SFD/UTILITY **Fire:** 50.22

Permit Holder BRYAN ZECHER **Waste:** 150.75

Owner of Building CORNERSTONE DEVELOPERS **Total:** 200.97

Location: 101 NW ETHELIND CT(MAGNOLIA HILLS,LOT 10)

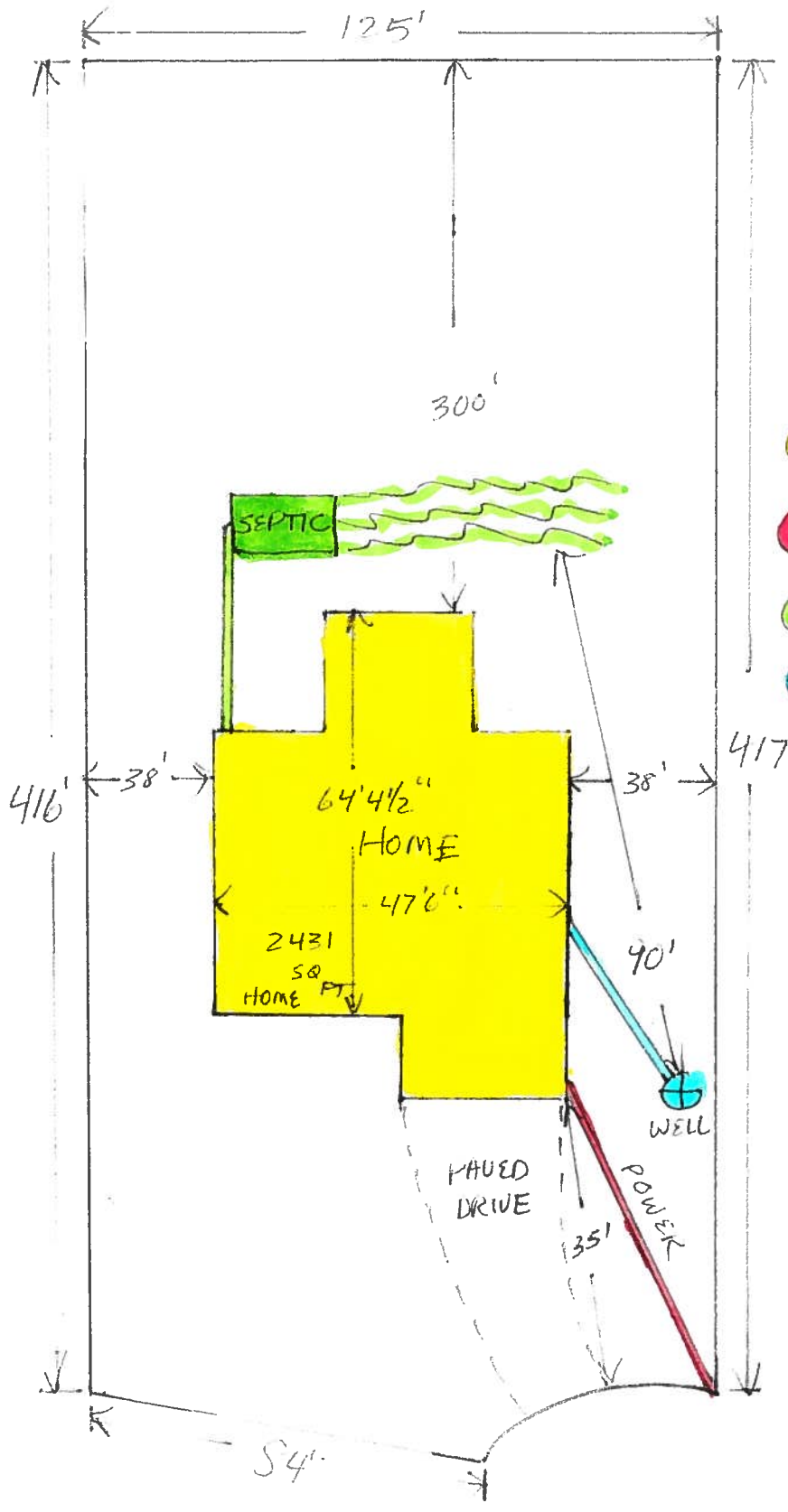
Date: 01/24/2007

Harry Dicks

Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)

MAGNOLIA HILLS
COLUMBIA COUNTY
LOT 10 SITE PLAN



CODE

= HOME

= POWER

= SEPTIC

= WELL

Columbia County Property Appraiser

J. Doyle Crews, CFA - Lake City, Florida - 386-758-1083

PARCEL: 28-3S-16-02377-110 - VACANT (000000)

Name:	CORNERSTONE DEVELOPERS LLC	LandVal	$16,000.00
Site:	MAGNOLIA HILLS S/D	BldgVal	$0.00
Mail:	180 NW AMENITY COURT LAKE CITY, FL 32055	ApprVal	$16,000.00
		JustVal	$16,000.00
Sales Info	2/9/2006 $468,000.00V / Q	Assd	$16,000.00
	2/8/2006 $468,000.00V / U	Exmpt	$0.00
	8/11/2005 $420,000.00V / Q	Taxable	$16,000.00

0 0.05 0.1 0.15 mi

N W E S

This information, GIS Map Updated: 9/1/2006, was derived from data which was compiled by the Columbia County Property Appraiser Office solely for the governmental purpose of property assessment. This information should not be relied upon by anyone as a determination of the ownership of property or market value. No warranties, expressed or implied, are provided for the accuracy of the data herein, it's use, or it's interpretation. Although it is periodically updated, this information may not reflect the data currently on file in the Property Appraiser's office. The assessed values are NOT certified values and therefore are subject to change before being finalized for ad valorem assessment purposes.

Project Information for:	L209566		
Builder:	CORNERSTONE	Date:	9/5/2006
Lot:	LOT 10 MAGNOLIA HILLS	Start Number:	**1184**
Subdivision:	N/A	SEI Ref:	L209566
County or City:	COLUMBIA COUNTY		
Truss Page Count:	36		

Truss Design Load Information (UNO) Design Program: MiTek

Gravity		Wind		Building Code:	FBC2004
Roof (psf):	**42**	Wind Standard:	**ASCE 7-02**		
Floor (psf):	55	Wind Speed (mph):	**110**		

Note: See individual truss drawings for special loading conditions

Building Designer, responsible for Structural Engineering: (See attached)

ZECHER, BRYAN C. CBC 054575
Address: PO BOX 815
LAKE CITY,FLORIDA 32056 Designer: **179**

Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987
Company: Structural Engineering and Inspections, Inc. EB 9196
Address 16105 N. Florida Ave, Ste B, Lutz, FL 33549 Phone: 813-849-5769

Notes:

1. Truss Design Engineer is responsible for the individual trusses as components only.
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSI/TPI
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.
4. Trusses designed for veritcal loads only, unless noted otherwise.
5. Where hangers are shown, Carried Member hanger capacity per Simpson C-2006 (SYP/Full Nailing Value) as an individual component. Building Designer shall verify the suitablity and use of Carrying Member hanger capacity.

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	0905061184	9/5/2006				
2	CJ3	0905061185	9/5/2006				
3	CJ3T	0905061186	9/5/2006				
4	CJ5	0905061187	9/5/2006				
5	CJ5T	0905061188	9/5/2006				
6	EJ3	0905061189	9/5/2006				
7	EJ5	0905061190	9/5/2006				
8	EJ7	0905061191	9/5/2006				
9	EJ7T	0905061192	9/5/2006				
10	HJ4	0905061193	9/5/2006				
11	HJ7	0905061194	9/5/2006				
12	HJ9	0905061195	9/5/2006				
13	HJ9T	0905061196	9/5/2006				
14	T01	0905061197	9/5/2006				
15	T02	0905061198	9/5/2006				
16	T03	0905061199	9/5/2006				
17	T04	0905061200	9/5/2006				
18	T05	0905061201	9/5/2006				
19	T06	0905061202	9/5/2006				
20	T07	0905061203	9/5/2006				
21	T08	0905061204	9/5/2006				
22	T09	0905061205	9/5/2006				
23	T10	0905061206	9/5/2006				
24	T11	0905061207	9/5/2006				
25	T12	0905061208	9/5/2006				
26	T13	0905061209	9/5/2006				
27	T14	0905061210	9/5/2006				
28	T15	0905061211	9/5/2006				
29	T16	0905061212	9/5/2006				
30	T17	0905061213	9/5/2006				
31	T18	0905061214	9/5/2006				
32	T19	0905061215	9/5/2006				
33	T20	0905061216	9/5/2006				
34	T21	0905061217	9/5/2006				
35	T22	0905061218	9/5/2006				
36	T23	0905061219	9/5/2006				

SEP 0 5 2006

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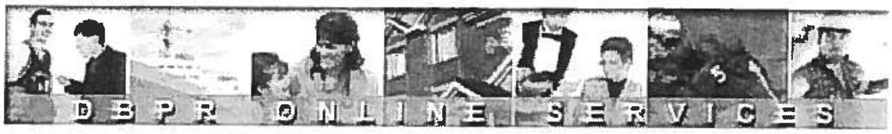

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10:00:13 AM

Licensee Details

Licensee Information

Name:	**ZECHER, BRYAN CHRISTIAN** (Primary Name) **BRYAN ZECHER CONSTRUCTION INC** (DBA
Main Address:	**P O BOX 815** **LAKE CITY, Florida 32056**
Lic. Location:	**465 NW ORANGE ST** **LAKE CITY, FL 32055 United States** **Columbia**

License Information

License Type:	**Certified Building Contractor**
Rank:	**Cert Building**
License Number:	**CBC054575**
Status:	**Current, Active**
Licensure Date:	**12/05/1991**
Expires:	**08/31/2006**

Special Qualifications	Effective Date
Bldg Code Core Course Credit	
Qualified Business License Required	04/13/2004

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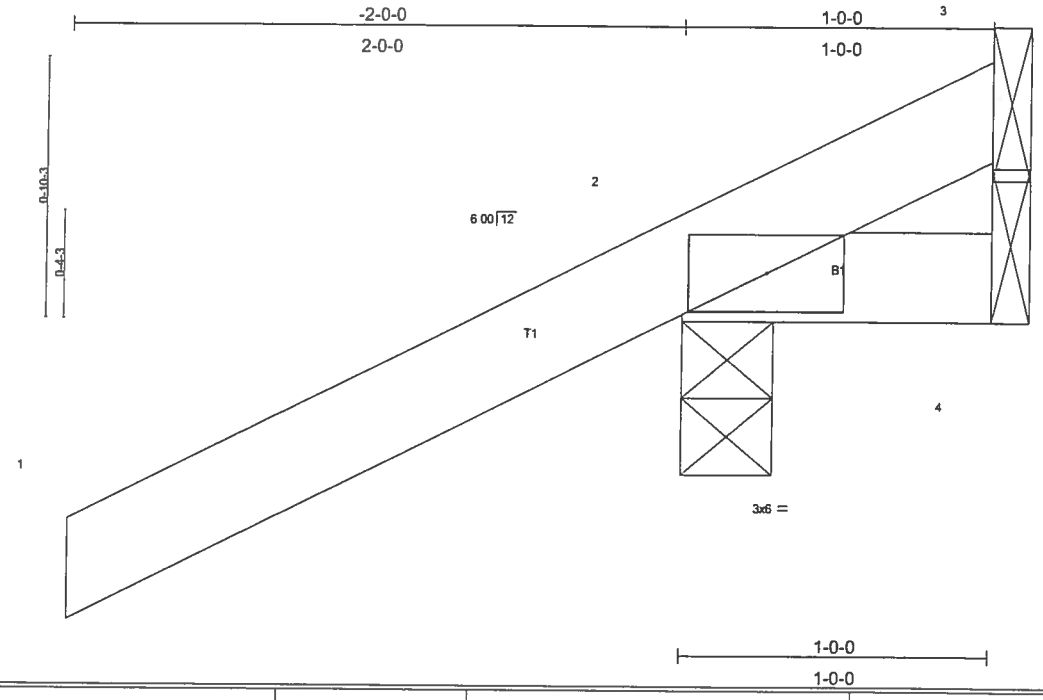
SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061184

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	CJ1	JACK	18	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:27 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.28	Vert(LL)	-0.00	2	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.01	Vert(TL)	-0.00	2	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 7 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=266/0-3-8, 4=14/Mechanical, 3=-90/Mechanical
Max Horz 2=87(load case 5)
Max Uplift2=-286(load case 5), 4=-9(load case 3), 3=-90(load case 1)
Max Grav 2=266(load case 1), 4=14(load case 1), 3=127(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-69/75
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.14

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 286 lb uplift at joint 2, 9 lb uplift at joint 4 and 90 lb uplift at joint 3.

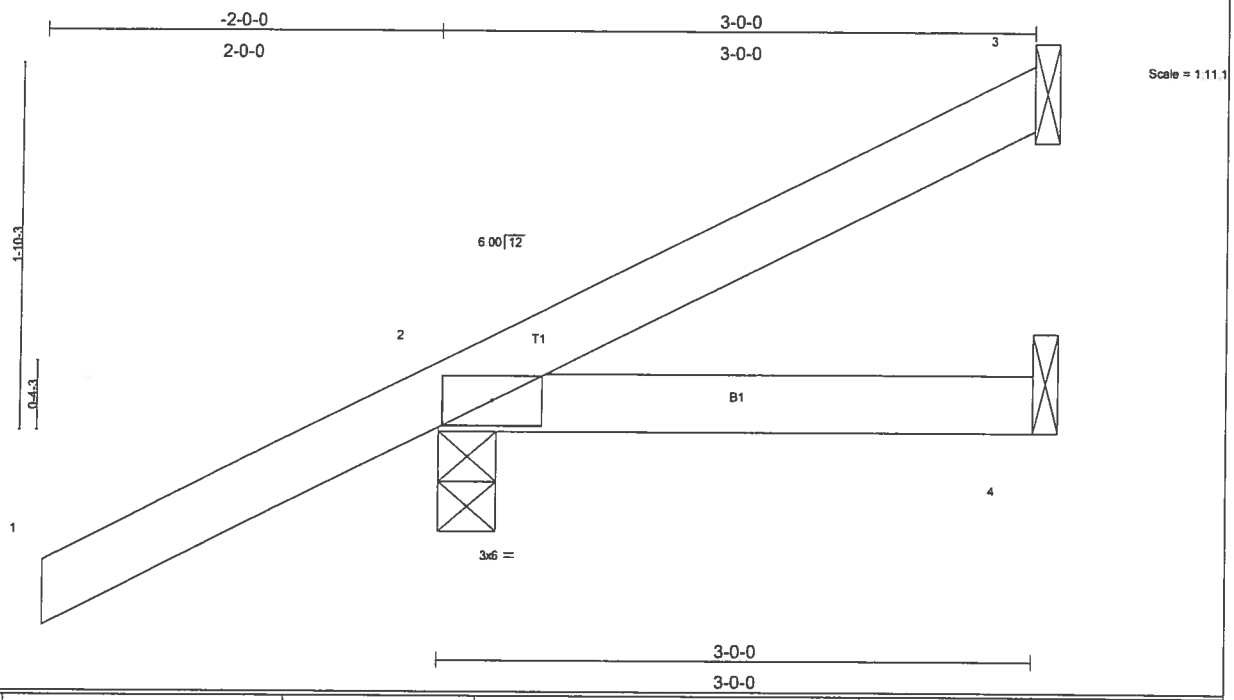
LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061185

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	CJ3	JACK	12	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:27 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.00	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.06	Vert(TL)	-0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3 and 203 lb uplift at joint 2.

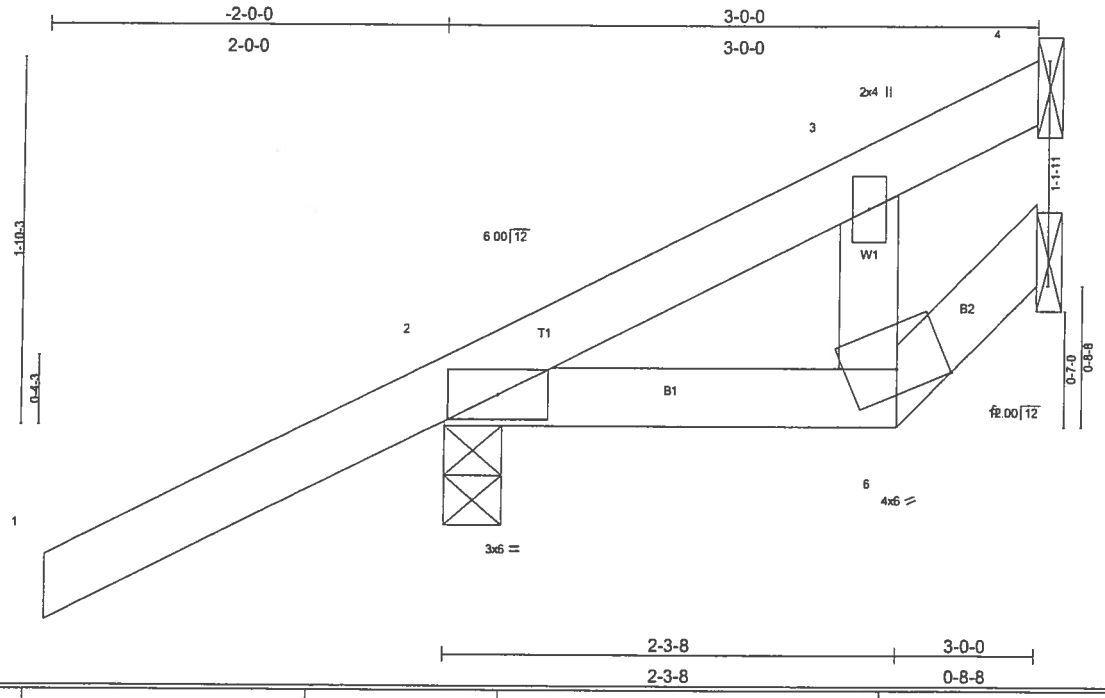
LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061186

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	CJ3T	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:28 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.01	6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.03	Vert(TL)	-0.01	6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(TL)	-0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 15 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 4=63/Mechanical, 2=278/0-3-8, 5=10/Mechanical
Max Horz 2=132(load case 5)
Max Uplift4=-21(load case 6), 2=-203(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-58/2, 3-4=-18/26
BOT CHORD 2-6=-6/0, 5-6=-6/6
WEBS 3-6=0/42

JOINT STRESS INDEX
2 = 0.13, 3 = 0.03 and 6 = 0.02

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 4 and 203 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061187

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	CJ5	JACK	8	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:29 2006 Page 1

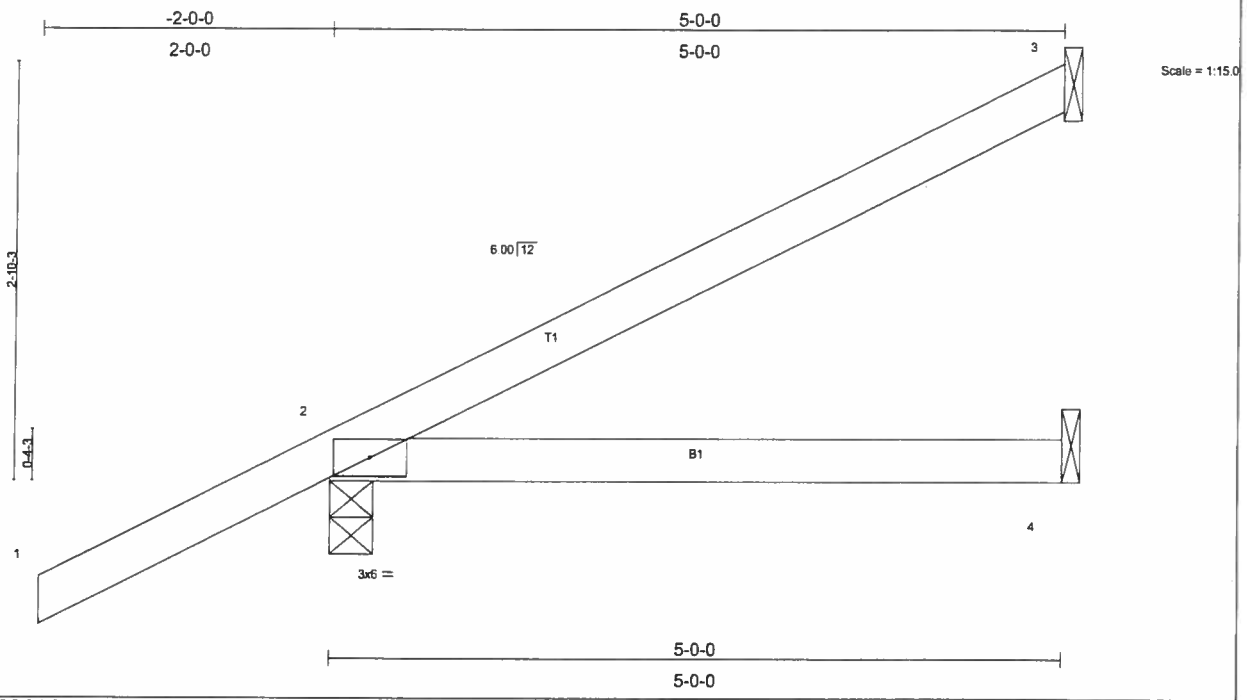


Plate Offsets (X,Y): [2:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.16	Vert(TL)	-0.05	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3 and 199 lb uplift at joint 2.

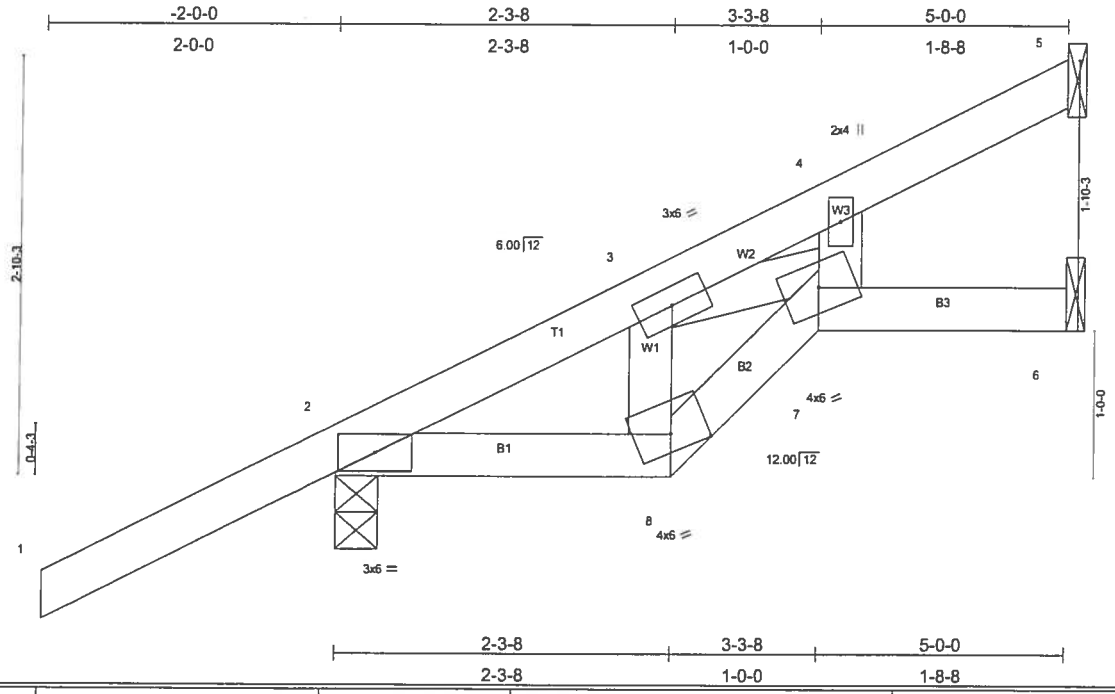
LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061188

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	CJ5T	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22.29 2006 Page 1



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.31	Vert(LL) 0.04 7 >999 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.06	Vert(TL) -0.04 7 >999 180	
BCLL 10.0	Rep Stress Incr YES	WB 0.06	Horz(TL) 0.02 6 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 24 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=151/Mechanical, 2=343/0-3-8, 6=25/Mechanical
Max Horz 2=178(load case 5)
Max Uplift5=-78(load case 5), 2=-199(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-229/0, 3-4=-114/0, 4-5=-68/64
BOT CHORD 2-8=-78/152, 7-8=-97/206, 6-7=-0/0
WEBS 3-8=-91/76, 3-7=-103/55, 4-7=-64/190

JOINT STRESS INDEX
2 = 0.16, 3 = 0.04, 4 = 0.13, 7 = 0.07 and 8 = 0.06

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 78 lb uplift at joint 5 and 199 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061189

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	EJ3	JACK	3	1	Job Reference (optional)

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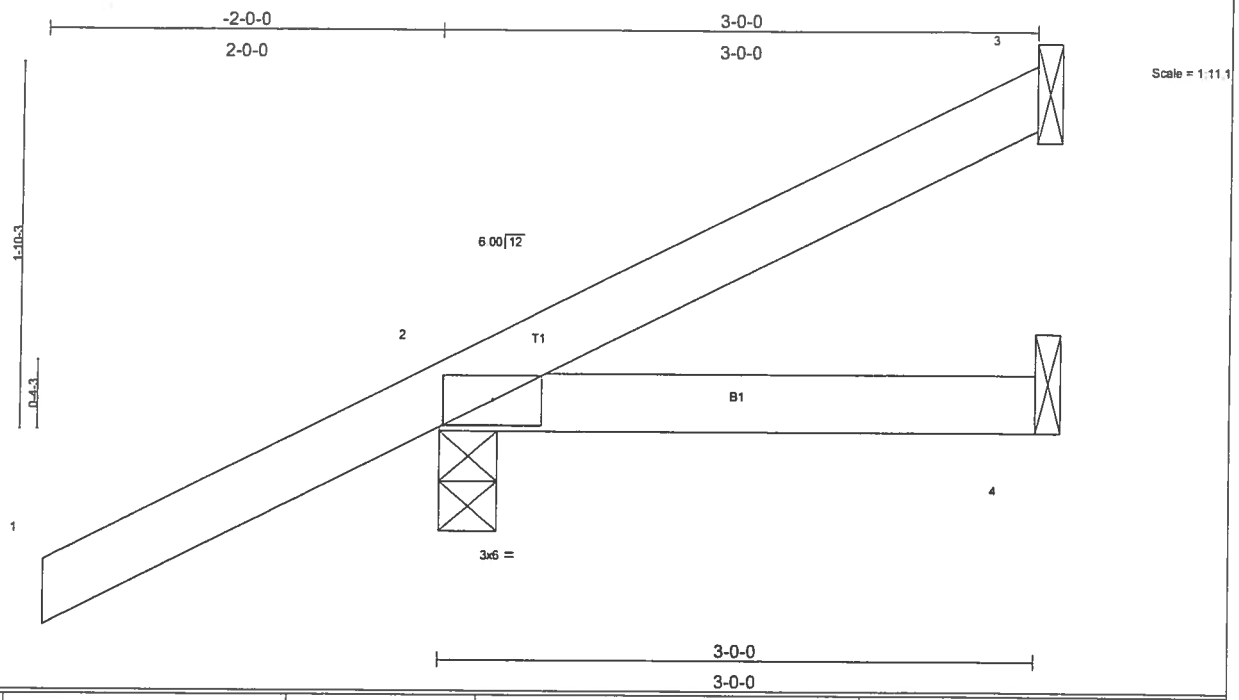


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.01	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 13 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=31/Mechanical, 2=278/0-3-8, 4=42/Mechanical
Max Horz 2=132(load case 5)
Max Uplift3=-28(load case 6), 2=-238(load case 5), 4=-27(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-57/7
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 3, 238 lb uplift at joint 2 and 27 lb uplift at joint 4.

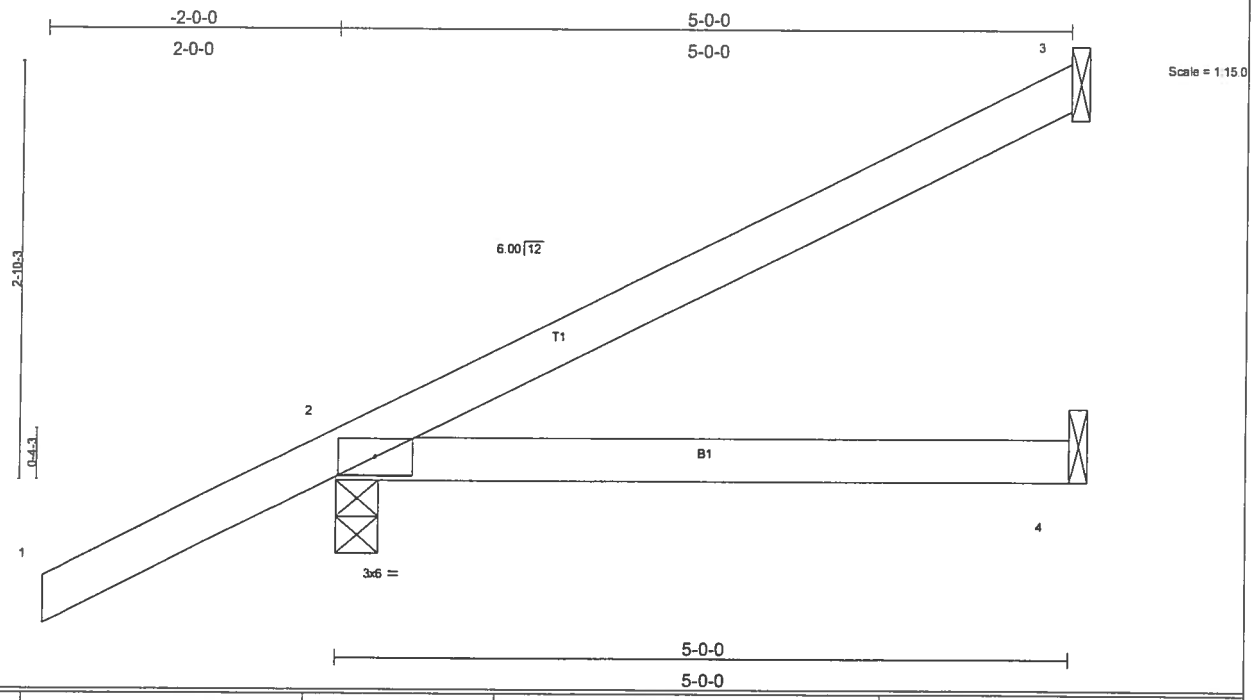
LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061190

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	EJ5	JACK	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22.31 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.09	2-4	>663	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.24	Vert(TL)	0.07	2-4	>774	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 19 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=103/Mechanical, 2=343/0-3-8, 4=72/Mechanical
Max Horz 2=178(load case 5)
Max Uplift3=-87(load case 5), 2=-260(load case 5), 4=-46(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-88/36
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.15

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 87 lb uplift at joint 3, 260 lb uplift at joint 2 and 46 lb uplift at joint 4.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061191

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	EJ7	MONO TRUSS	30	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:31 2006 Page 1

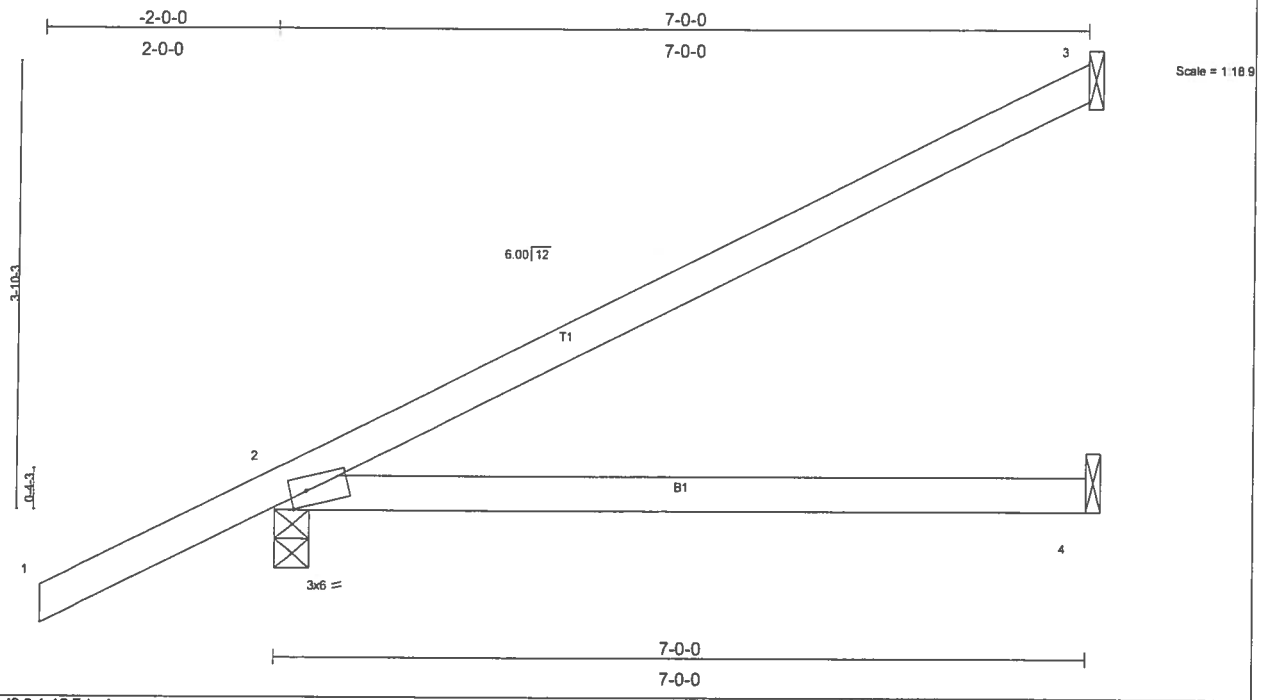


Plate Offsets (X,Y): [2:0-1-12,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.48	Vert(LL)	-0.12	2-4	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.35	Vert(TL)	-0.21	2-4	>397	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=162/Mechanical, 2=419/0-3-8, 4=104/Mechanical
Max Horz 2=224(load case 5)
Max Uplift3=-134(load case 5), 2=-210(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-119/58
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.82

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 3 and 210 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061192

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	EJ7T	SPECIAL	3	1	Job Reference (optional)

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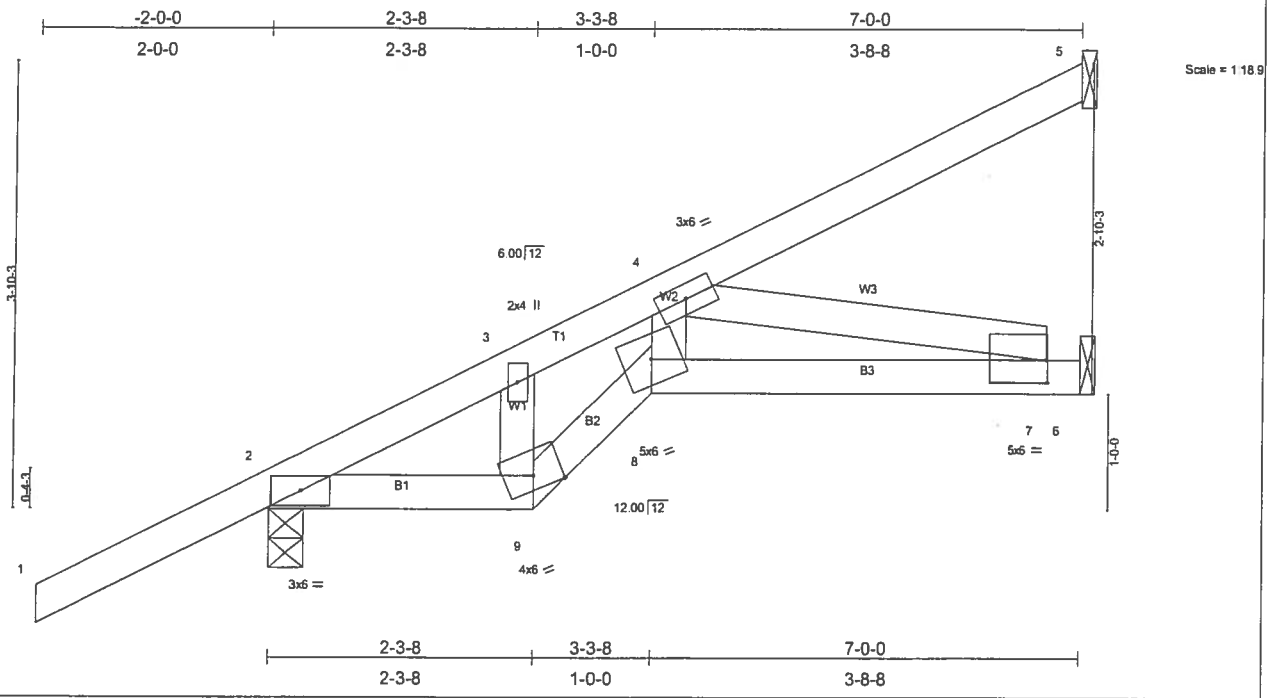

Plate Offsets (X,Y): [7:0-0-3,0-2-5]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	0.06	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.08	7-8	>994	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 5=109/Mechanical, 2=419/0-3-8, 6=158/Mechanical
Max Horz 2=224(load case 5)
Max Uplift5=-85(load case 5), 2=-210(load case 5), 6=-41(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-509/116, 3-4=-497/149, 4-5=-69/40
BOT CHORD 2-9=-270/417, 8-9=-238/432, 7-8=-251/351, 6-7=0/0
WEBS 3-9=-123/88, 4-8=-36/249, 4-7=-360/257

JOINT STRESS INDEX
2 = 0.47, 3 = 0.05, 4 = 0.18, 7 = 0.14, 8 = 0.65 and 9 = 0.66

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 85 lb uplift at joint 5, 210 lb uplift at joint 2 and 41 lb uplift at joint 6.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061193

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	HJ4	JACK	2	1	Job Reference (optional)

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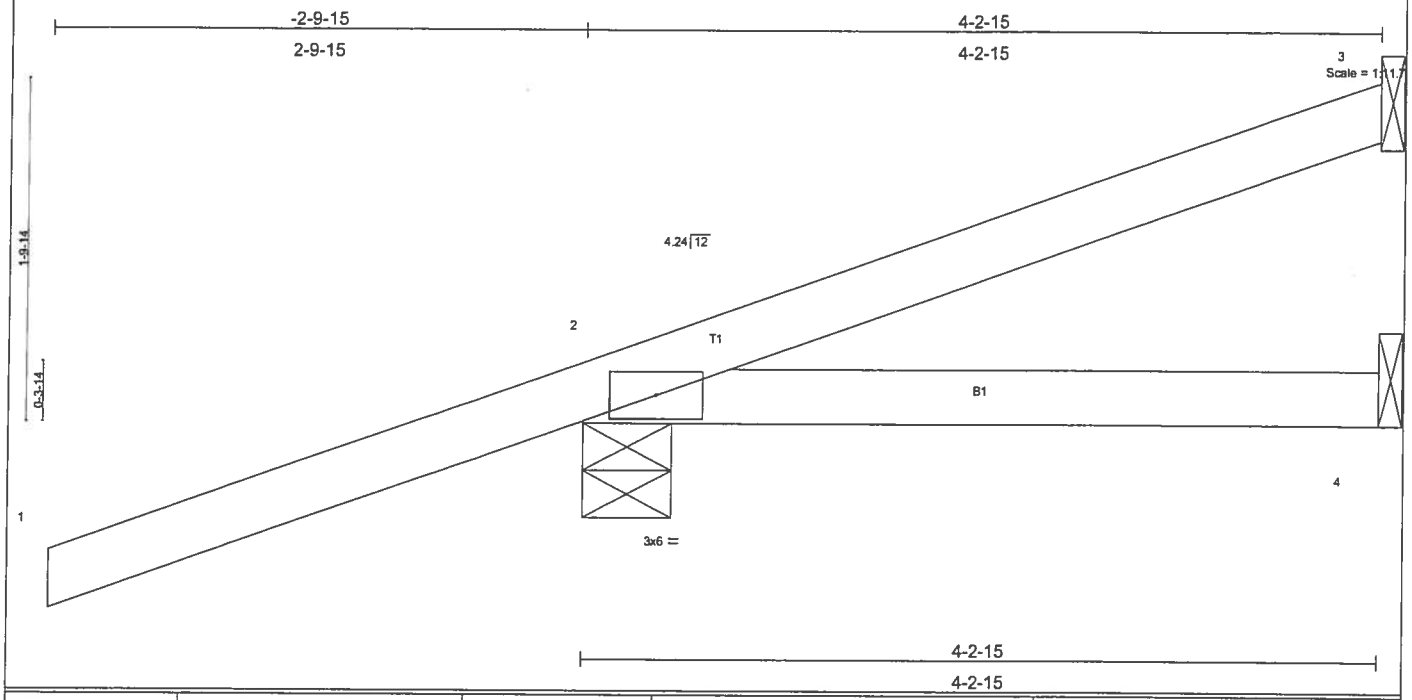

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	0.02	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.07	Vert(TL)	0.01	2-4	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 18 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=13/Mechanical, 2=293/0-5-11, 4=42/Mechanical
Max Horz 2=98(load case 2)
Max Uplift3=-5(load case 4), 2=-305(load case 2), 4=-41(load case 2)
Max Grav 3=33(load case 5), 2=293(load case 1), 4=42(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-38/11
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.12

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 3, 305 lb uplift at joint 2 and 41 lb uplift at joint 4.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-57(F=-2, B=-2), 2=-0(F=15, B=15)-to-4=-32(F=-1, B=-1)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061194

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	HJ7	MONO TRUSS	2	1	Job Reference (optional)

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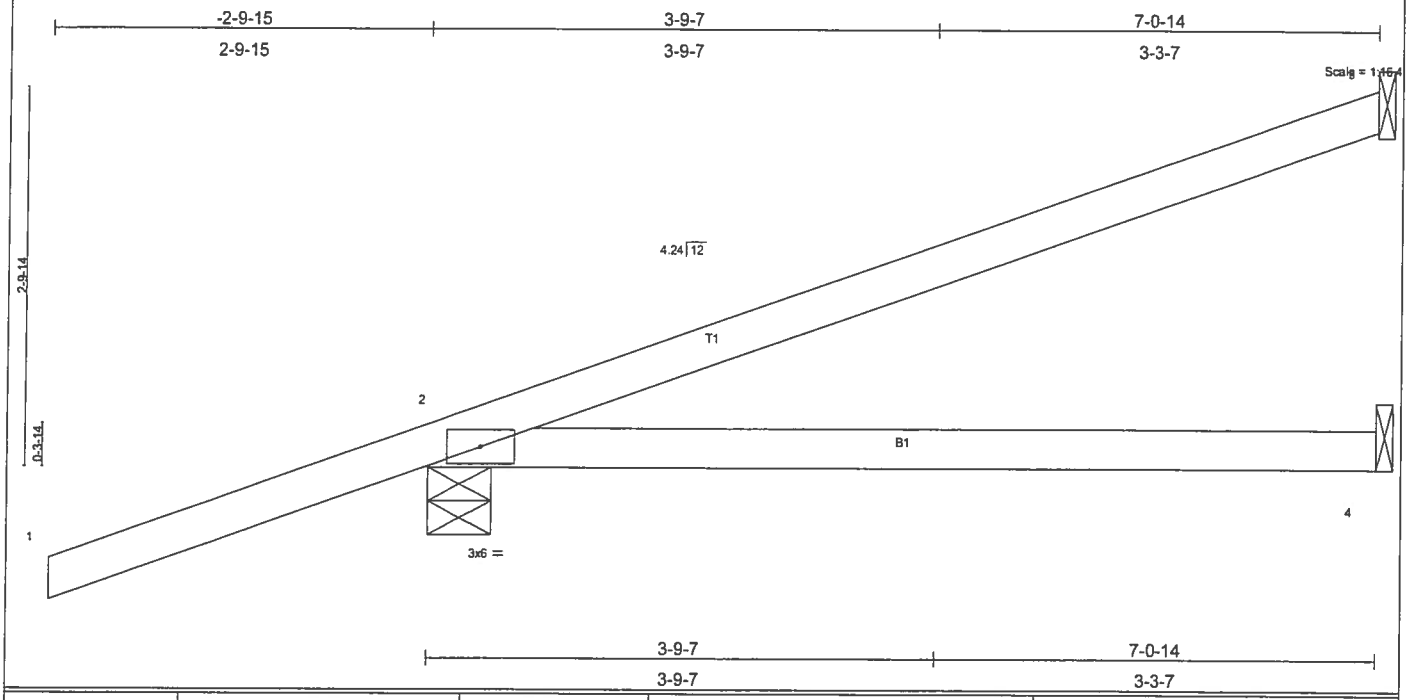

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.54	Vert(LL)	-0.07	2-4	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.27	Vert(TL)	-0.13	2-4	>626	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 26 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 7-0-14 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=191/Mechanical, 2=378/0-5-11, 4=110/Mechanical
Max Horz 2=167(load case 2)
Max Uplift3=-144(load case 2), 2=-251(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-68/45
BOT CHORD 2-4=0/0

JOINT STRESS INDEX
2 = 0.44

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3 and 251 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-3=-95(F=-21, B=-21), 2=-0(F=15, B=15)-to-4=-53(F=-12, B=-12)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061195

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	HJ9	MONO TRUSS	4	1	Job Reference (optional)

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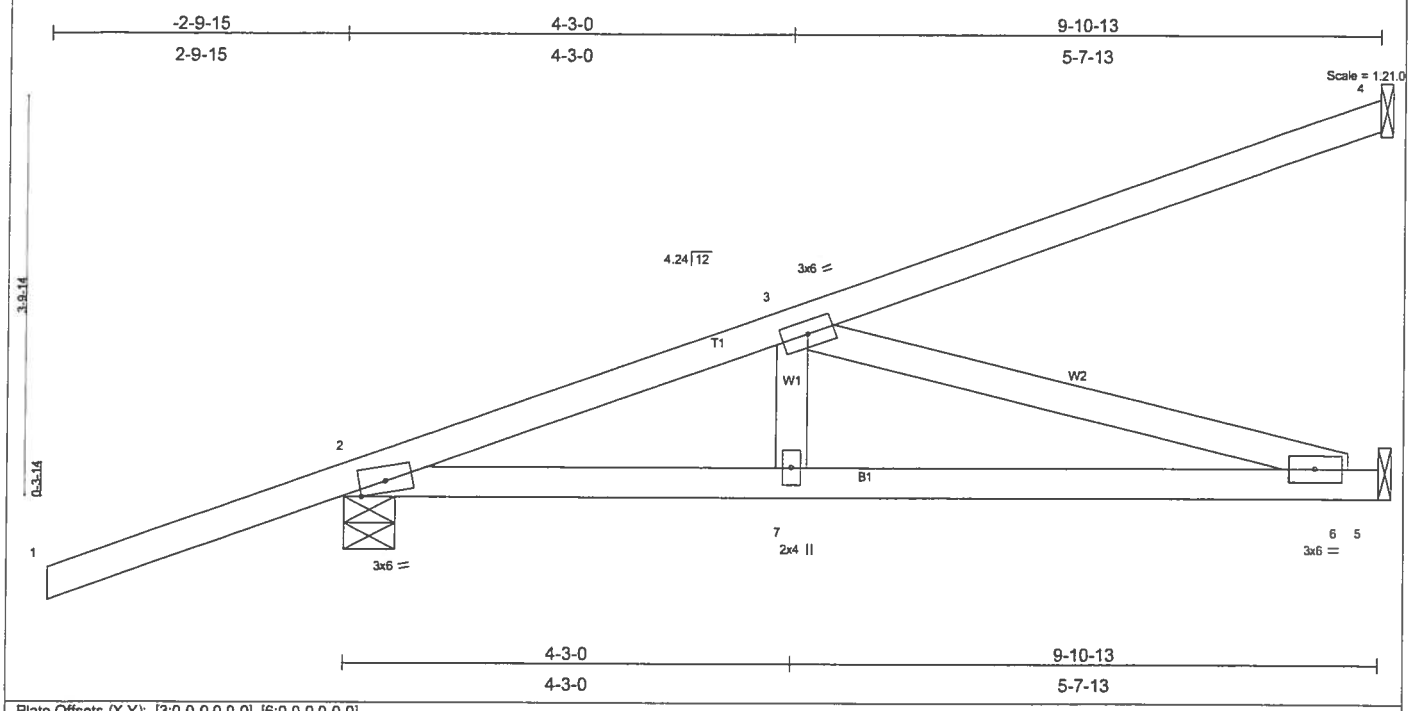

Plate Offsets (X,Y): [3:0-0-0,0-0-0], [6:0-0-0,0-0-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.61	Vert(LL)	-0.10	6-7	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.17	6-7	>685	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.49	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 45 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 4=269/Mechanical, 2=535/0-5-11, 5=375/Mechanical
Max Horz 2=269(load case 2)
Max Uplift4=-231(load case 2), 2=-281(load case 2), 5=-62(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-882/118, 3-4=-105/66
BOT CHORD 2-7=-306/817, 6-7=-306/817, 5-6=0/0
WEBS 3-7=0/178, 3-6=-849/318

JOINT STRESS INDEX
2 = 0.78, 3 = 0.23, 6 = 0.24 and 7 = 0.13

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 231 lb uplift at joint 4, 281 lb uplift at joint 2 and 62 lb uplift at joint 5.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-5=-74(F=-22, B=-22)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061196

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	HJ9T	SPECIAL	1	1	Job Reference (optional)

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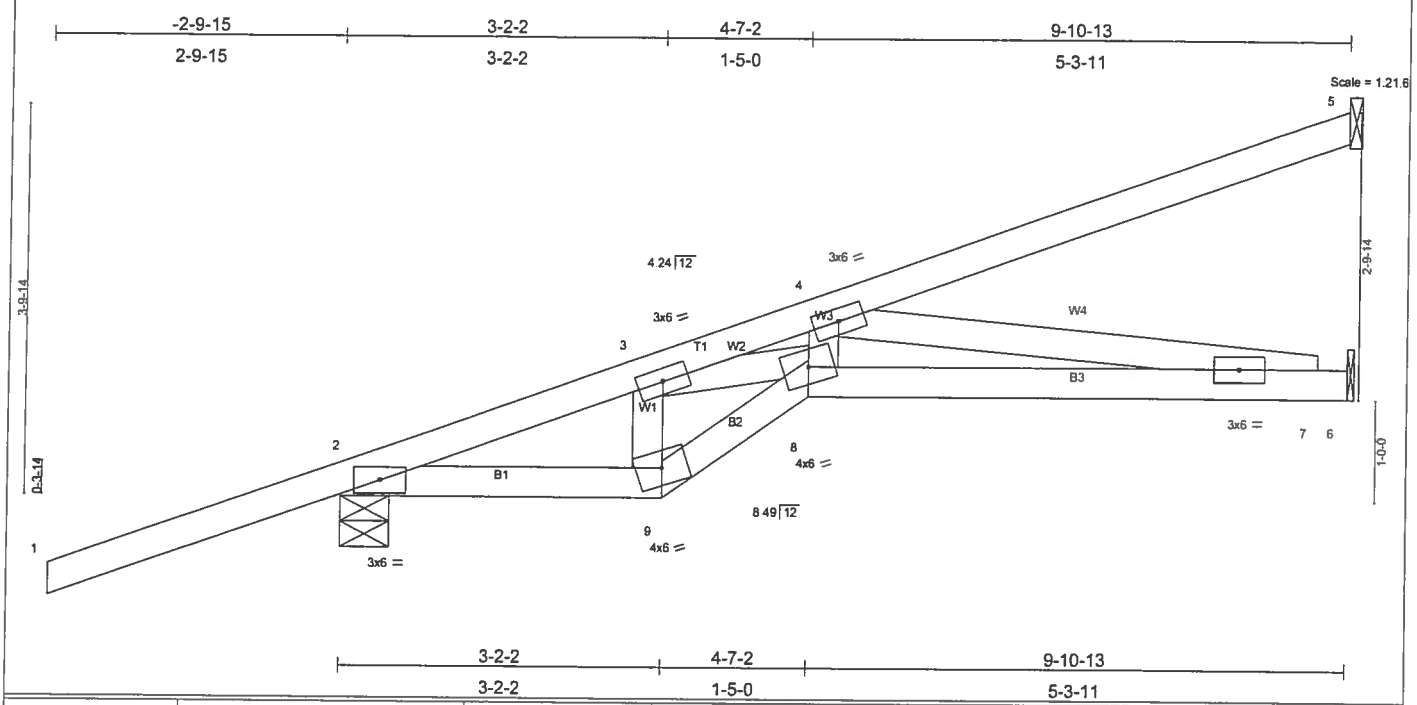

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.55	Vert(LL) -0.14 7-8 >853 240	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.76	Vert(TL) -0.22 7-8 >518 180	
BCLL 10.0	Rep Stress Incr NO	WB 0.83	Horz(TL) 0.06 6 n/a n/a	
BCDL 5.0	Code FBC2004/TPI2002	(Matrix)		Weight: 47 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-11-8 oc bracing: 7-8.

REACTIONS (lb/size) 5=256/Mechanical, 2=536/0-5-11, 6=388/Mechanical
Max Horz 2=273(load case 2)
Max Uplift5=-226(load case 2), 2=-275(load case 2), 6=-87(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/50, 2-3=-621/0, 3-4=-1971/606, 4-5=-102/63
BOT CHORD 2-9=-148/546, 8-9=-147/647, 7-8=-805/1774, 6-7=0/0
WEBS 3-9=-420/108, 3-8=-700/1393, 4-8=0/518, 4-7=-1791/813

JOINT STRESS INDEX
2 = 0.77, 3 = 0.69, 4 = 0.54, 7 = 0.52, 8 = 0.86 and 9 = 0.30

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
2) Refer to girder(s) for truss to truss connections.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 226 lb uplift at joint 5, 275 lb uplift at joint 2 and 87 lb uplift at joint 6.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-54
Trapezoidal Loads (plf)
Vert: 2=-3(F=25, B=25)-to-5=-134(F=-40, B=-40), 2=-0(F=15, B=15)-to-9=-23(F=4, B=4), 9=-23(F=4, B=4)-to-8=-34(F=-2, B=-2), 8=-34(F=-2, B=-2)-to-6=-74(F=-22, B=-22)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061197

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T01	HIP	1	1	Job Reference (optional)

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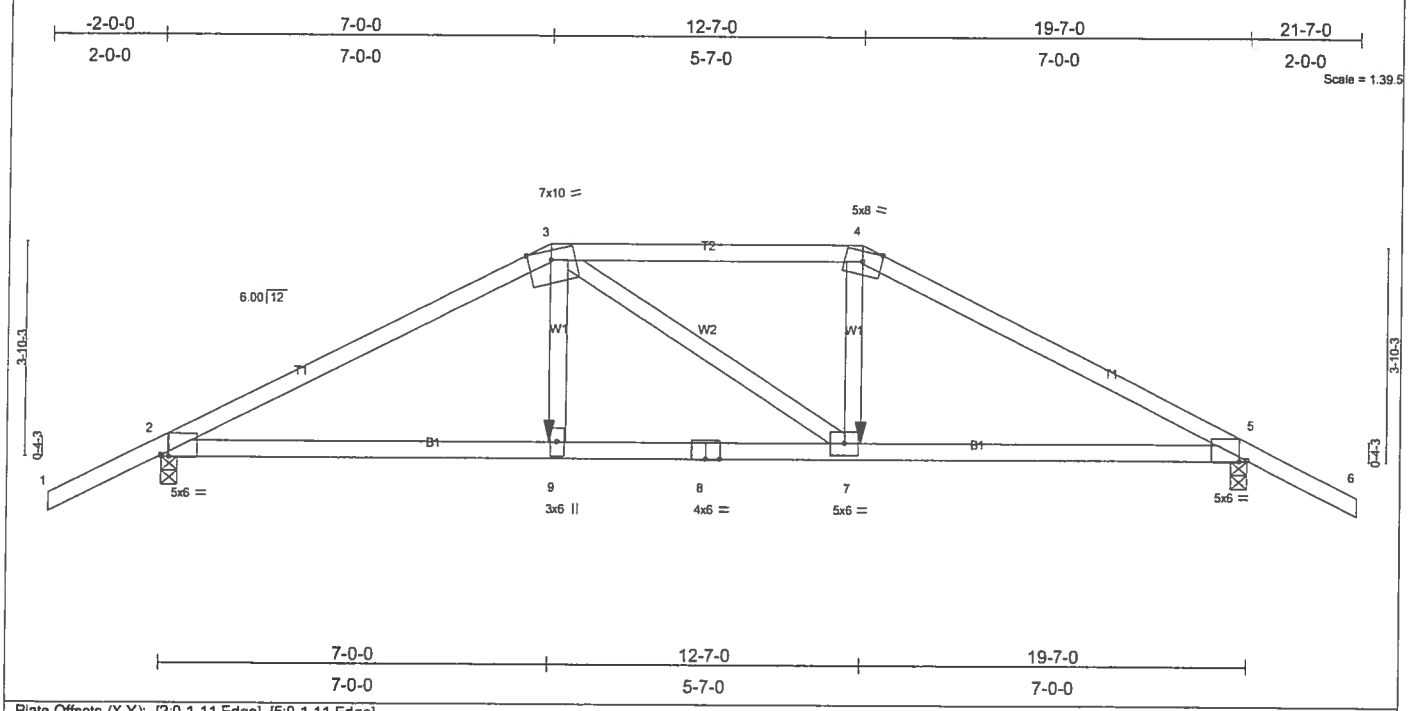

Plate Offsets (X,Y): [2:0-1-11,Edge], [5:0-1-11,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.15	7-9	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.70	Vert(TL)	-0.24	7-9	>979	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.29	Horz(TL)	0.08	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 86 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-1-14 oc bracing.

REACTIONS (lb/size) 2=1736/0-3-8, 5=1736/0-3-8
Max Horz 2=-87(load case 5)
Max Uplift2=-792(load case 4), 5=-792(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3055/1240, 3-4=-2684/1181, 4-5=-3057/1240, 5-6=0/47
BOT CHORD 2-9=-1035/2648, 8-9=-1044/2682, 7-8=-1044/2682, 5-7=-999/2650
WEBS 3-9=-224/835, 3-7=-123/126, 4-7=-246/889

JOINT STRESS INDEX
2 = 0.77, 3 = 0.90, 4 = 0.76, 5 = 0.77, 7 = 0.31, 8 = 0.92 and 9 = 0.27

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 792 lb uplift at joint 2 and 792 lb uplift at joint 5.
6) Girder carries hip end with 7-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 12-7-0, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-118(F=-64), 4-6=-54, 2-9=-30, 7-9=-65(F=-35), 5-7=-30
Concentrated Loads (lb)
Vert: 9=-539(F) 7=-539(F)

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Dwg.#0905061198

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T02	HIP	1	1	Job Reference (optional)

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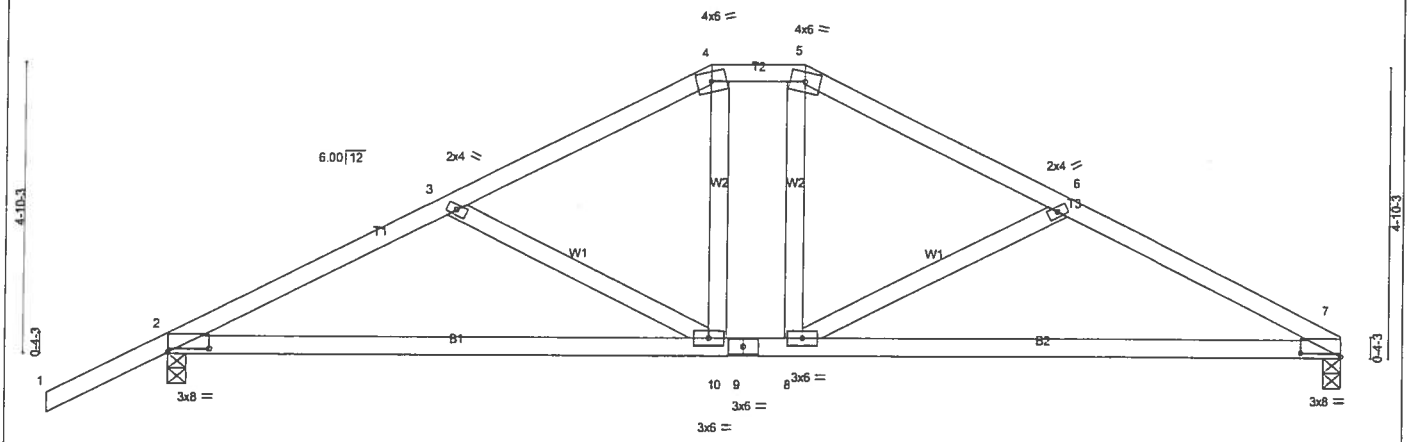
-2-0-0	4-9-4	9-0-0	10-7-0	14-9-12	19-7-0
2-0-0	4-9-4	4-2-12	1-7-0	4-2-12	4-9-4

Scale = 1:36.7



9-0-0	10-7-0	19-7-0
9-0-0	1-7-0	9-0-0

Plate Offsets (X,Y): [2:0-8-0,0-0-10], [7:0-8-0,0-0-10]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.19	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.31	7-8	>753	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.04	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 92 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 9-0-4 oc bracing.

REACTIONS (lb/size) 7=804/0-3-8, 2=933/0-3-8
Max Horz 2=126(load case 5)
Max Uplift7=-256(load case 6), 2=-388(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1309/571, 3-4=-1047/459, 4-5=-894/463, 5-6=-1051/465, 6-7=-1334/612
BOT CHORD 2-10=-425/1131, 9-10=-232/894, 8-9=-232/894, 7-8=-478/1163
WEBS 3-10=-305/220, 4-10=-50/278, 5-8=-115/317, 6-8=-339/282

JOINT STRESS INDEX
2 = 0.78, 3 = 0.34, 4 = 0.44, 5 = 0.44, 6 = 0.34, 7 = 0.75, 8 = 0.35, 9 = 0.73 and 10 = 0.35

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 256 lb uplift at joint 7 and 388 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
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16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

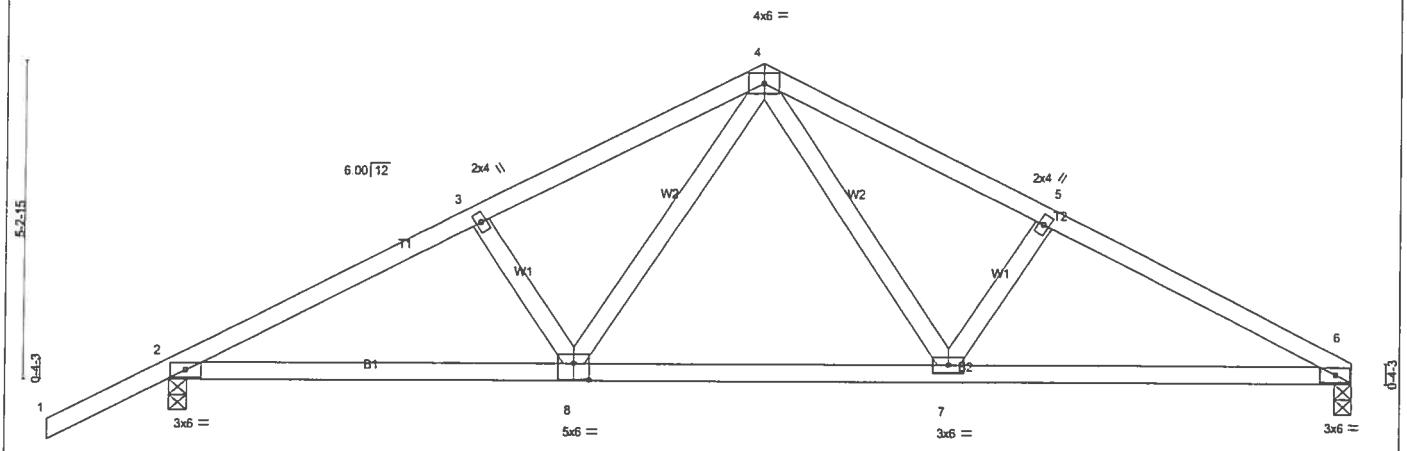
Dwg.#0905061199

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T03	COMMON	6	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:37 2006 Page 1

-2-0-0 | 5-2-0 | 9-9-8 | 14-5-0 | 19-7-0
2-0-0 | 5-2-0 | 4-7-8 | 4-7-8 | 5-2-0

Scale = 1:36.3



6-8-8 | 12-10-8 | 19-7-0
6-8-8 | 6-2-0 | 6-8-8

Plate Offsets (X,Y): [8:0-3-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	-0.15	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.71	Vert(TL)	-0.23	7-8	>992	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.04	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 90 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-0-2 oc bracing.

REACTIONS (lb/size) 6=958/0-3-8, 2=1087/0-3-8
Max Horz 2=131(load case 5)
Max Uplift6=-319(load case 6), 2=-450(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1709/711, 3-4=-1567/710, 4-5=-1592/751, 5-6=-1722/754
BOT CHORD 2-8=-546/1460, 7-8=-318/1017, 6-7=-596/1489
WEBS 3-8=-217/202, 4-8=-227/649, 4-7=-291/685, 5-7=-235/231

JOINT STRESS INDEX
2 = 0.76, 3 = 0.34, 4 = 0.55, 5 = 0.34, 6 = 0.76, 7 = 0.52 and 8 = 0.76

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 319 lb uplift at joint 6 and 450 lb uplift at joint 2.
5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-54, 4-6=-54, 2-8=-30, 7-8=-80(F=-50), 6-7=-30

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061200

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T04	SPECIAL	1	2	Job Reference (optional)

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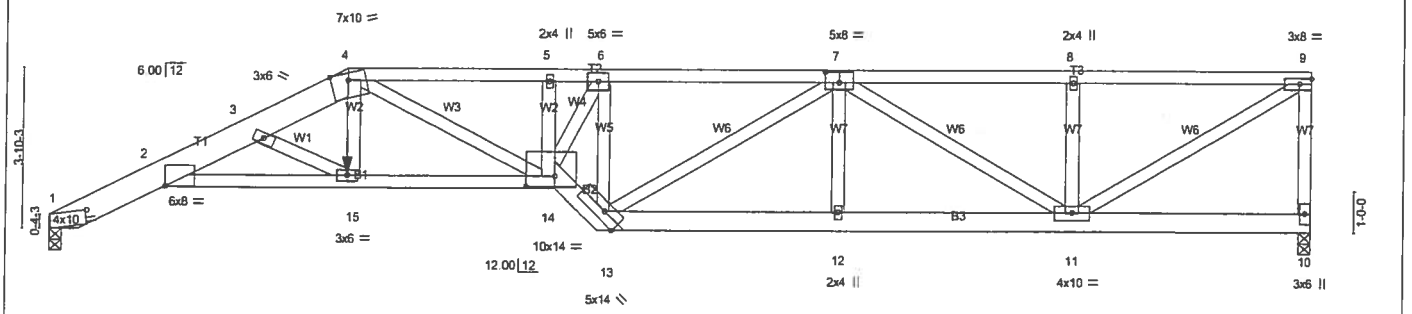
5-0-0	7-0-0	11-11-8	12-11-8	18-8-12	24-4-4	30-1-8
5-0-0	2-0-0	4-11-8	1-0-0	5-9-4	5-7-8	5-9-4

Scale = 1:52.6



2-8-6	7-0-0	11-11-8	12-11-8	18-8-12	24-4-4	30-1-8
2-8-6	4-3-10	4-11-8	1-0-0	5-9-4	5-7-8	5-9-4

Plate Offsets (X,Y): [2:Edge,0-0-2], [7:0-4-0,0-3-0], [13:0-5-4,Edge], [14:0-7-15,0-2-14], [14:11-1-5,0-6-2]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.92	Vert(LL)	-0.39	14-15	>910	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.63	14-15	>567	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.68	Horz(TL)	0.27	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 369 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 6 SYP No.1D *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 8-6-6 oc bracing.

REACTIONS (lb/size) 1=2458/0-3-8, 10=2759/0-3-8
Max Horz 1=129(load case 4)
Max Uplift1=-830(load case 3), 10=-1034(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-939/286, 2-3=-8036/2842, 3-4=-7009/2525, 4-5=-7749/2859, 5-6=-7742/2861, 6-7=-5792/2152, 7-8=-3733/1395, 8-9=-3733/1395, 9-10=-2547/993
BOT CHORD 2-15=-2828/7953, 14-15=-2302/6405, 13-14=-2693/7326, 12-13=-2131/5708, 11-12=-2130/5703, 10-11=-57/132
WEBS 4-15=-623/1881, 4-14=-625/1501, 5-14=-41/176, 6-14=-1463/4039, 6-13=-4185/1593, 7-13=-111/106, 7-12=-83/476, 7-11=-2314/863, 8-11=-663/353, 9-11=-1573/4232, 3-15=-1851/708

JOINT STRESS INDEX
2 = 0.77, 3 = 0.35, 4 = 0.69, 5 = 0.34, 6 = 0.86, 7 = 0.45, 8 = 0.34, 9 = 0.75, 10 = 0.35, 11 = 0.99, 12 = 0.34, 13 = 0.77, 14 = 0.89, 14 = 0.00 and 15 = 0.66

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 8 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc, 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
4) Provide adequate drainage to prevent water ponding.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 830 lb uplift at joint 1 and 1034 lb uplift at joint 10.
8) Girder carries tie-in span(s): 3-7-3 from 7-0-0 to 12-11-8; 5-4-1 from 7-0-0 to 12-11-8
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 204 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-65, 2-4=-54, 4-6=-81(F=-27), 6-9=-118(F=-64), 2-15=-30, 14-15=-94(F=-64), 13-14=-94(F=-64), 10-13=-65(F=-35)
Concentrated Loads (lb)
Vert: 15=-539(F)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

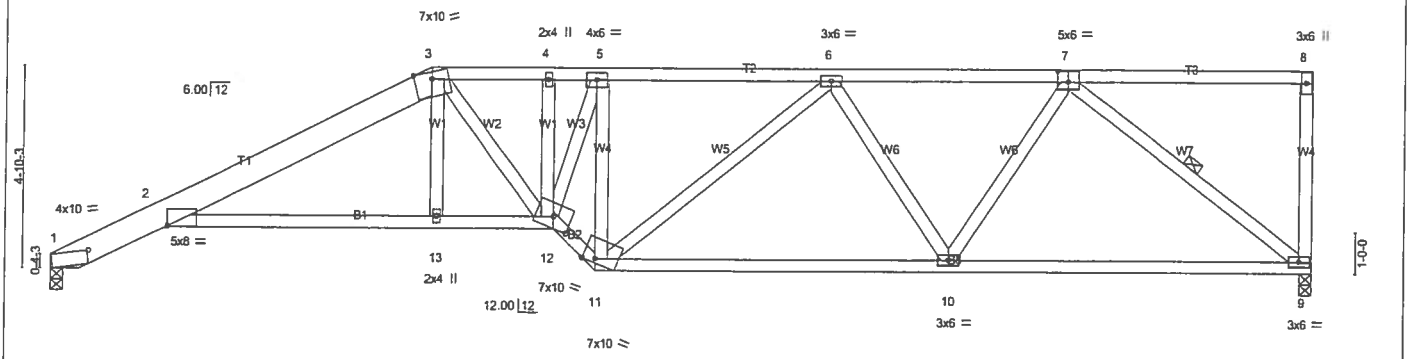
Dwg.#0905061201

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T05	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:39 2006 Page 1

3-3-8 | 9-0-0 | 11-11-8 | 12-11-8 | 18-7-0 | 24-2-8 | 30-1-8
3-3-8 | 5-8-8 | 2-11-8 | 1-0-0 | 5-7-8 | 5-7-8 | 5-11-0

Scale = 1:52.6



2-8-6 | 9-0-0 | 11-11-8 | 12-11-8 | 21-4-12 | 30-1-8
2-8-6 | 6-3-10 | 2-11-8 | 1-0-0 | 8-5-4 | 8-8-12

Plate Offsets (X,Y): [2:1-10-10,0-4-5], [2:0-0-0,0-0-2], [7:0-3-0,0-3-0], [11:0-3-11,Edge], [12:0-5-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.44	2-13	>817	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.85	Vert(TL)	-0.71	2-13	>504	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.54	Horz(TL)	0.33	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 174 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-7-9 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 1=1206/0-3-8, 9=1251/0-3-8
Max Horz 1=212(load case 5)
Max Uplift1=-352(load case 5), 9=-460(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-458/39, 2-3=-2580/1043, 3-4=-2427/1065, 4-5=-2417/1063, 5-6=-2083/895, 6-7=-1702/678, 7-8=-57/11, 8-9=-152/102
BOT CHORD 2-13=-1089/2390, 12-13=-1091/2404, 11-12=-1119/2500, 10-11=-839/1954, 9-10=-557/1293
WEBS 3-13=-44/330, 3-12=-136/88, 4-12=-160/311, 5-12=-563/1112, 5-11=-1400/768, 6-11=-72/166, 6-10=-479/307, 7-10=-231/778, 7-9=-1574/695

JOINT STRESS INDEX
2 = 0.60, 2 = 0.00, 3 = 0.40, 4 = 0.34, 5 = 0.74, 6 = 0.43, 7 = 0.49, 8 = 0.35, 9 = 0.69, 10 = 0.60, 11 = 0.72, 12 = 0.50 and 13 = 0.34

NOTES
1) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
2) Provide adequate drainage to prevent water ponding.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 352 lb uplift at joint 1 and 460 lb uplift at joint 9.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

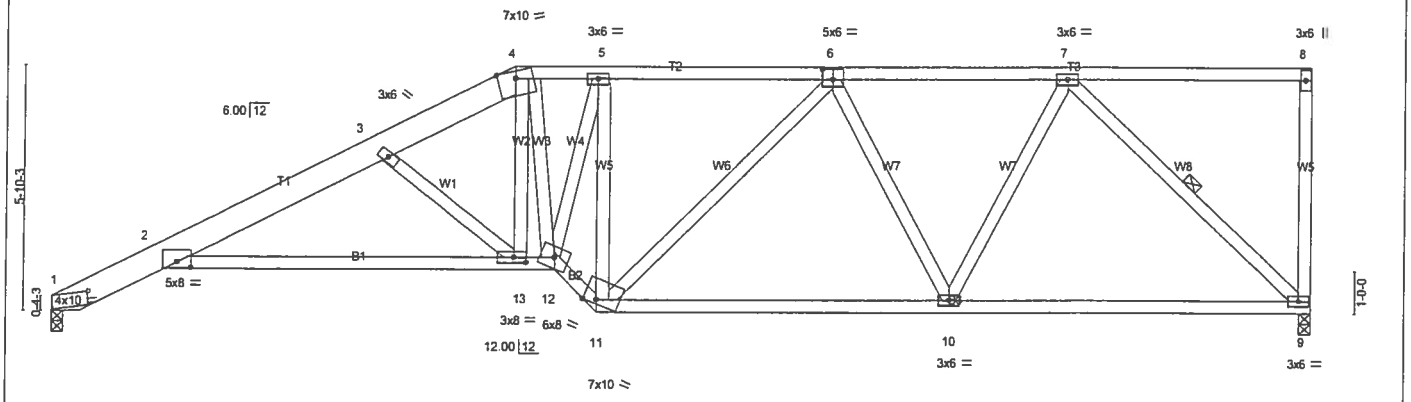
Dwg.#0905061202

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T06	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:40 2006 Page 1

3-3-8 | 11-0-0 | 12-11-8 | 18-7-0 | 24-2-8 | 30-1-8
3-3-8 | 7-8-8 | 1-11-8 | 5-7-8 | 5-7-8 | 5-11-0

Scale = 1:52.6



2-8-6 | 11-11-8 | 12-11-8 | 21-4-12 | 30-1-8
2-8-6 | 9-3-2 | 1-0-0 | 8-5-4 | 8-8-12

Plate Offsets (X,Y): [2:0-4-0,0-1-10], [6:0-3-0,0-3-0], [11:0-3-11,Edge], [13:0-3-8,0-1-8], [13:10-1-13,0-4-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.45	2-13	>802	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.73	2-13	>490	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.63	Horz(TL)	0.30	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 190 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except* T1 2 X 8 SYP 2400F 2.0E
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-1-9 oc bracing.
WEBS 1 Row at midpt 7-9

REACTIONS (lb/size) 1=1206/0-3-8, 9=1251/0-3-8
Max Horz 1=259(load case 5)
Max Uplift1=-363(load case 5), 9=-462(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-458/12, 2-3=-2869/1240, 3-4=-2318/1000, 4-5=-1918/889, 5-6=-1704/767, 6-7=-1397/574, 7-8=-44/8, 8-9=-149/100
BOT CHORD 2-13=-1378/2754, 12-13=-880/1940, 11-12=-971/2056, 10-11=-712/1603, 9-10=-469/1061
WEBS 5-11=-1143/691, 6-11=-86/143, 6-10=-454/305, 7-10=-232/743, 7-9=-1413/639, 4-12=-172/0, 5-12=-519/900, 4-13=-458/1324, 3-13=-1097/658

JOINT STRESS INDEX
2 = 0.67, 3 = 0.27, 4 = 0.55, 5 = 0.89, 6 = 0.37, 7 = 0.61, 8 = 0.32, 9 = 0.65, 10 = 0.61, 11 = 0.63, 12 = 0.54, 13 = 0.86 and 13 = 0.00

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 363 lb uplift at joint 1 and 462 lb uplift at joint 9.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061203

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T07	SPECIAL	1	1	Job Reference (optional)

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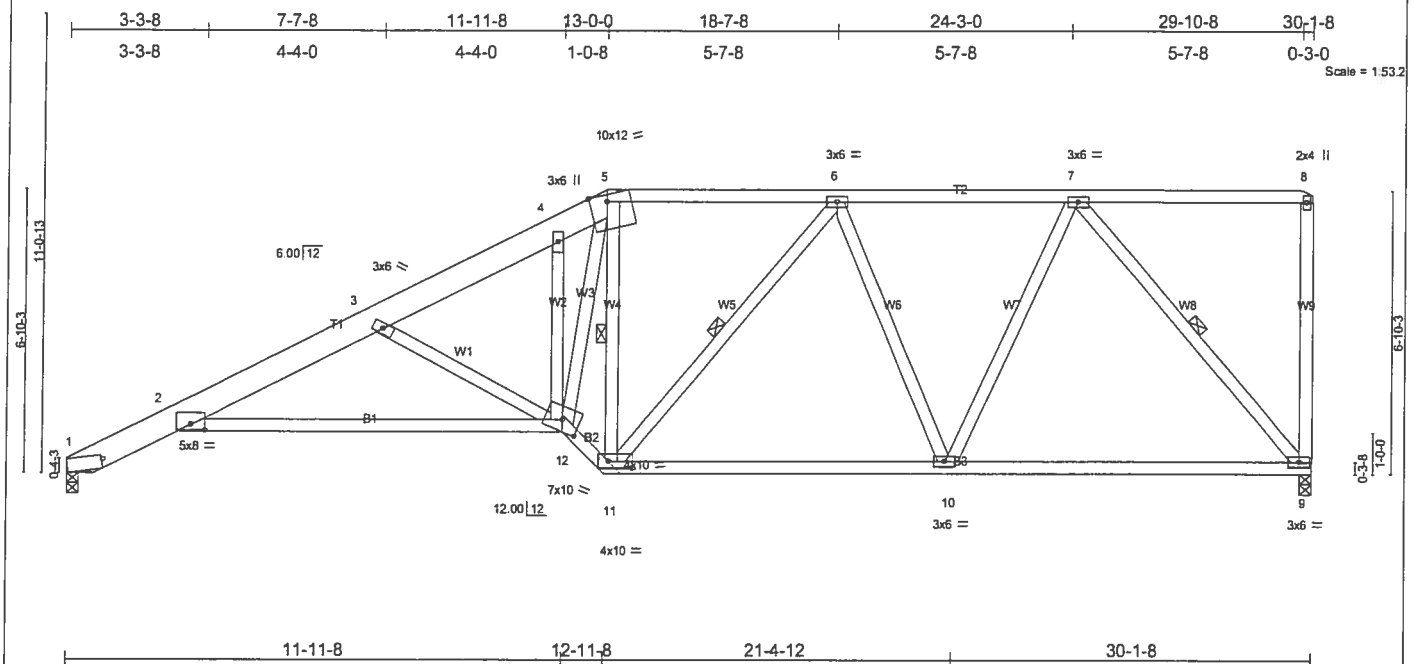


Plate Offsets (X,Y): [2:0-4-0,0-1-10], [5:0-5-3,Edge], [11:12-1-5,1-6-3], [11:0-7-1,0-2-0], [12:0-5-0,0-3-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.79	Vert(LL)	-0.50	2-12	>715	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.75	Vert(TL)	-0.82	2-12	>435	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.59	Horz(TL)	0.31	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 197 lb	

LUMBER
TOP CHORD 2 X 8 SYP 2400F 2.0E *Except*
T2 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-11-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-11-6 oc bracing.
WEBS 1 Row at midpt 6-11, 7-9, 5-11

REACTIONS (lb/size) 1=1206/0-3-8, 9=1251/0-3-8
Max Horz 1=305(load case 5)
Max Uplift1=-370(load case 5), 9=-458(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-458/0, 2-3=-2943/1290, 3-4=-2144/901, 4-5=-1853/854, 5-6=-1416/674, 6-7=-1196/506, 7-8=0/0
BOT CHORD 2-12=-1494/2855, 11-12=-930/2019, 10-11=-620/1351, 9-10=-406/887
WEBS 3-12=-1237/737, 4-12=-87/347, 6-11=-104/105, 6-10=-402/298, 7-10=-242/746, 7-9=-1356/621, 5-11=-1365/719, 5-12=-885/1842, 8-9=-121/87

JOINT STRESS INDEX
2 = 0.70, 3 = 0.41, 4 = 0.18, 5 = 0.45, 6 = 0.48, 7 = 0.55, 8 = 0.34, 9 = 0.46, 10 = 0.55, 11 = 0.54, 11 = 0.00 and 12 = 0.59

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 370 lb uplift at joint 1 and 458 lb uplift at joint 9.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061204

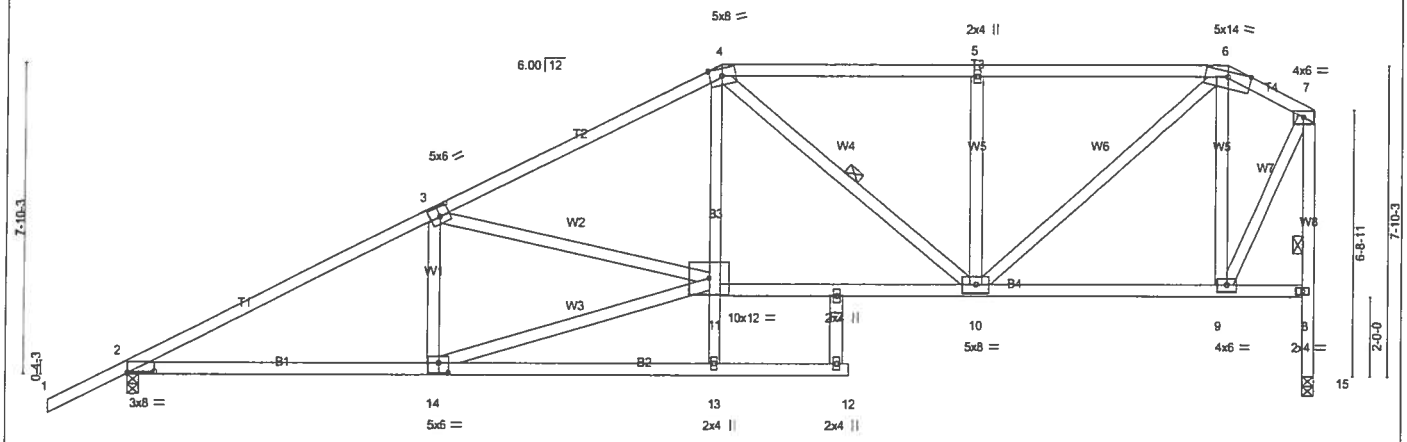
Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T08	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:41 2006 Page 1

-2-0-0 | 7-9-4 | 15-0-0 | 21-5-4 | 27-10-8 | 30-1-8

2-0-0 | 7-9-4 | 7-2-12 | 6-5-4 | 6-5-4 | 2-3-0

Scale = 1:55.7



7-9-4 | 15-0-0 | 18-3-0 | 21-5-4 | 27-10-8 | 30-1-8

7-9-4 | 7-2-12 | 3-3-0 | 3-2-4 | 6-5-4 | 2-3-0

Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-3-0,0-3-0], [14:0-3-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.27	12	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.44	12	>817	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.91	Horz(TL)	0.13	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 197 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B3 2 X 4 SYP No.3
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-2 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-10, 7-15
JOINTS 1 Brace at Jt(s): 11

REACTIONS (lb/size) 2=1419/0-3-8, 15=1305/0-3-8
Max Horz 2=376(load case 5)
Max Uplift2=-501(load case 5), 15=-352(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-2316/791, 3-4=-2271/874, 4-5=-1510/655, 5-6=-1510/655, 6-7=-580/245, 8-15=-1305/529, 7-8=-1292/523
BOT CHORD 2-14=-920/1983, 13-14=-24/0, 12-13=0/0, 11-13=0/208, 4-11=-272/982, 10-11=-789/1934, 9-10=-196/500, 8-9=-3/6
WEBS 3-14=-378/323, 11-14=-945/2098, 3-11=-83/152, 4-10=-559/247, 5-10=-365/257, 6-10=-535/1339, 6-9=-892/446, 7-9=-456/1154

JOINT STRESS INDEX
2 = 0.74, 3 = 0.84, 4 = 0.50, 5 = 0.34, 6 = 0.66, 7 = 0.59, 8 = 0.34, 9 = 0.69, 10 = 0.63, 11 = 0.62, 13 = 0.43, 14 = 0.81, 16 = 0.34 and 17 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 15 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 501 lb uplift at joint 2 and 352 lb uplift at joint 15.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

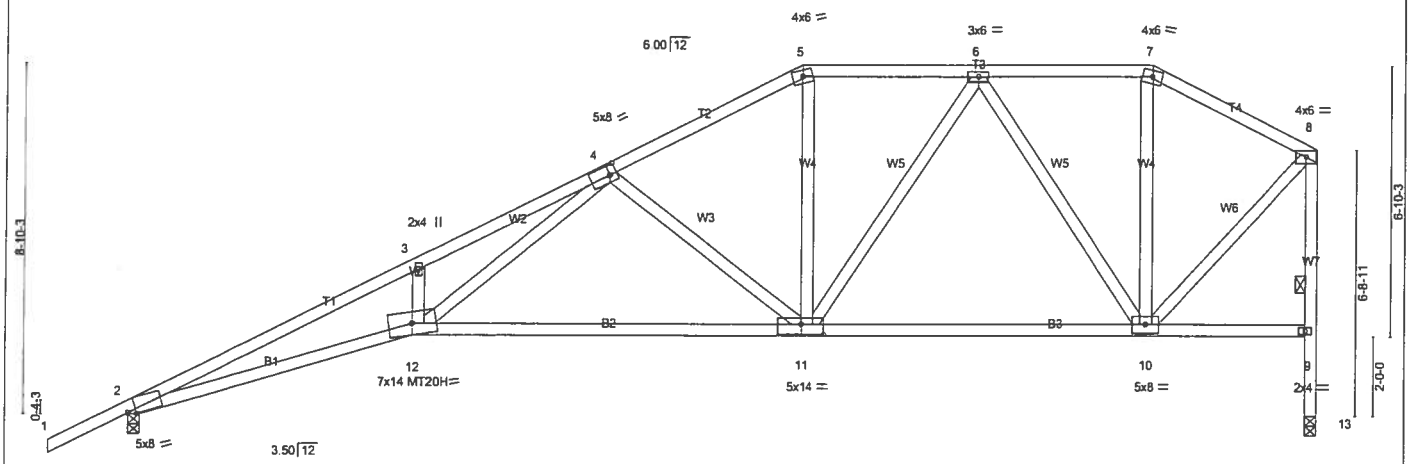
Dwg.#0905061205

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T09	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:42 2006 Page 1

-2-0-0	7-1-13	12-0-14	17-0-0	21-5-4	25-10-8	30-1-8
2-0-0	7-1-13	4-11-2	4-11-2	4-5-4	4-5-4	4-3-0

Scale = 1:55.7



7-1-13	17-0-0	25-10-8	30-1-8
7-1-13	9-10-3	8-10-8	4-3-0

Plate Offsets (X,Y): [2:0-2-8,Edge], [4:0-2-0,0-3-0], [11:0-7-0,0-3-0], [12:0-0-0,0-0-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	-0.57	11-12	>628	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.93	11-12	>384	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.38	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 177 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-6-0 oc bracing.
WEBS 1 Row at midpt 8-13

REACTIONS (lb/size) 2=1373/0-3-8, 13=1249/0-3-8
Max Horz 2=389(load case 5)
Max Uplift2=-519(load case 5), 13=-366(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4854/2088, 3-4=-4793/2260, 4-5=-1685/769, 5-6=-1451/742, 6-7=-724/401, 7-8=-852/393, 9-13=-1249/551, 8-9=-1207/551
BOT CHORD 2-12=-2159/4432, 11-12=-1095/2183, 10-11=-514/1157, 9-10=-13/15
WEBS 3-12=-222/290, 4-12=-1300/2674, 4-11=-942/589, 5-11=-138/468, 6-11=-220/535, 6-10=-815/398, 7-10=-4/181, 8-10=-412/1028

JOINT STRESS INDEX
2 = 0.96, 3 = 0.34, 4 = 0.71, 5 = 0.54, 6 = 0.43, 7 = 0.44, 8 = 0.86, 9 = 0.34, 10 = 0.49, 11 = 0.57 and 12 = 0.72

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 519 lb uplift at joint 2 and 366 lb uplift at joint 13.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

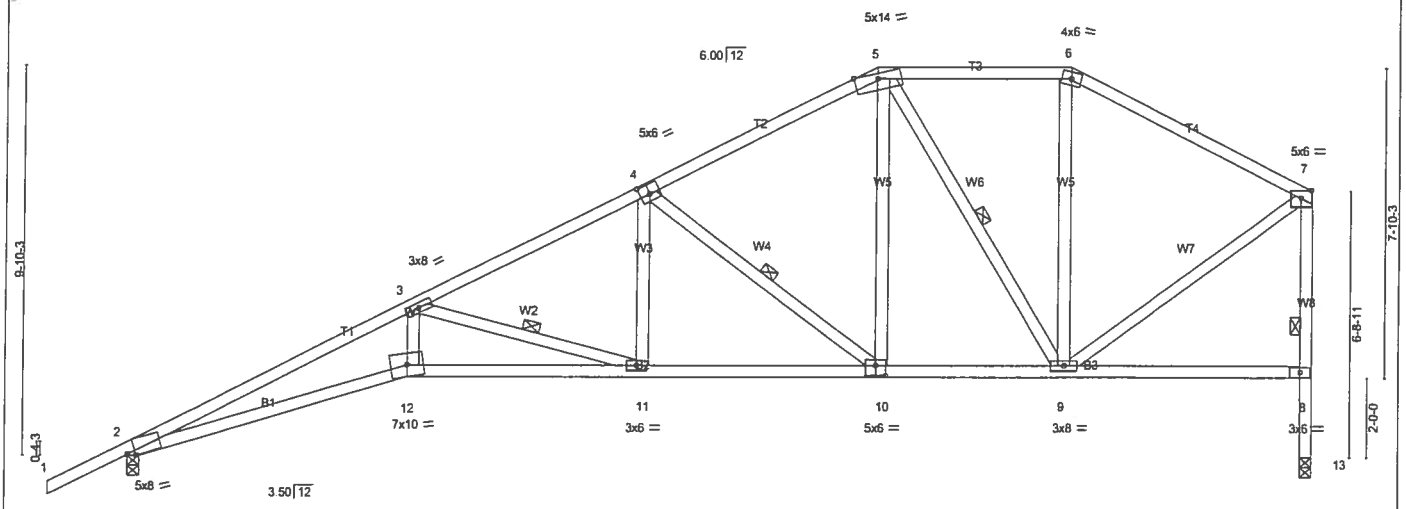
Dwg.#0905061206

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T10	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:43 2006 Page 1

-2-0-0	7-1-13	13-0-14	19-0-0	23-10-8	30-1-8
2-0-0	7-1-13	5-11-2	5-11-2	4-10-8	6-3-0

Scale = 1:55.7



7-1-13	13-0-14	19-0-0	23-10-8	30-1-8
7-1-13	5-11-2	5-11-2	4-10-8	6-3-0

Plate Offsets (X,Y): [2:0-2-8,Edge], [4:0-3-0,0-3-0], [10:0-3-0,0-3-0], [12:0-0-0,0-0-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.84	Vert(LL)	-0.42	11-12	>843	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.83	Vert(TL)	-0.68	11-12	>526	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.47	Horz(TL)	0.41	13	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 180 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2 *Except*
B1 2 X 4 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 4-3-13 oc bracing.
WEBS 1 Row at midpt 3-11, 4-10, 5-9, 7-13

REACTIONS (lb/size) 2=1373/0-3-8, 13=1249/0-3-8
Max Horz 2=403(load case 5)
Max Uplift2=-526(load case 5), 13=-387(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/46, 2-3=-4814/2134, 3-4=-2355/1050, 4-5=-1366/670, 5-6=-845/508, 6-7=-1014/491, 8-13=-1249/561, 7-8=-1152/573
BOT CHORD 2-12=-2204/4389, 11-12=-2064/4069, 10-11=-993/2032, 9-10=-511/1171, 8-9=-29/48
WEBS 3-12=-563/1409, 3-11=-2108/1111, 4-11=-287/791, 4-10=-1107/618, 5-10=-346/824, 5-9=-668/298, 6-9=-26/214, 7-9=-398/977

JOINT STRESS INDEX
2 = 0.96, 3 = 0.83, 4 = 0.58, 5 = 0.68, 6 = 0.72, 7 = 0.71, 8 = 0.35, 9 = 0.94, 10 = 0.50, 11 = 0.59 and 12 = 0.94

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2, 13 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 526 lb uplift at joint 2 and 387 lb uplift at joint 13.

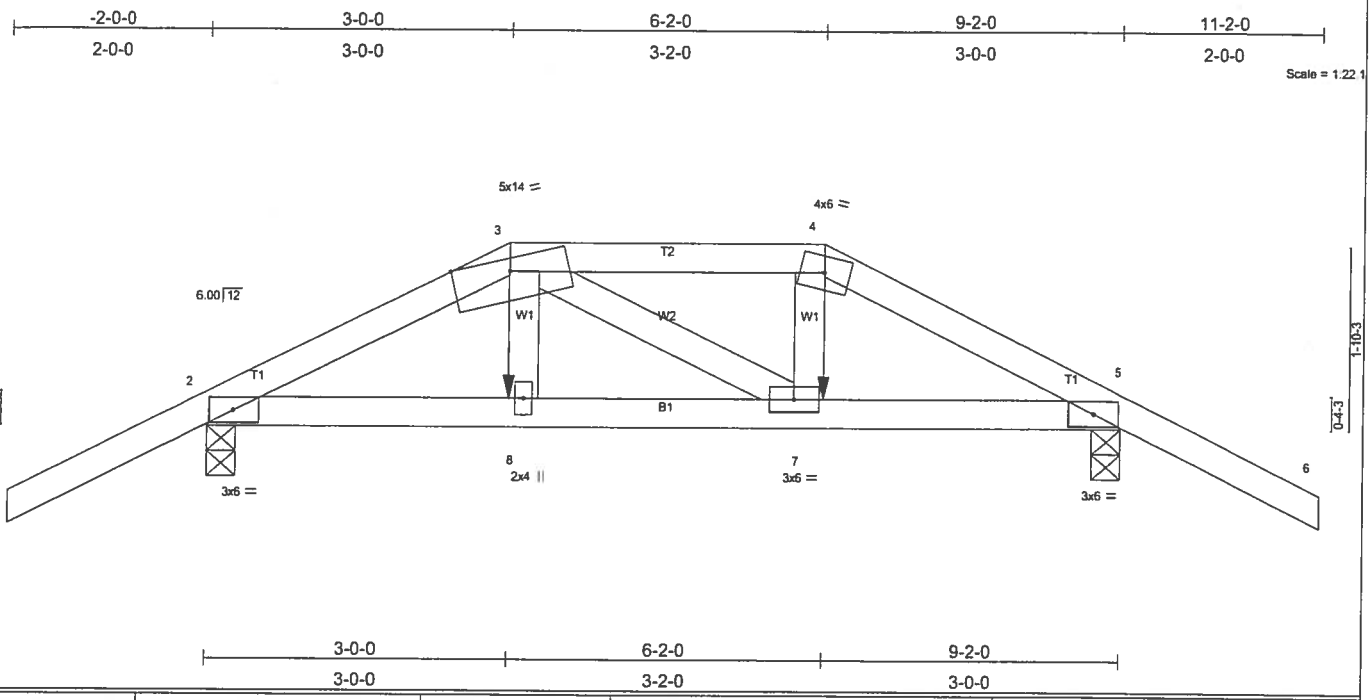
LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061207

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T11	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.02	7-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.02	7-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.05	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 43 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=574/0-3-8, 5=574/0-3-8
Max Horz 2=-59(load case 5)
Max Uplift2=-399(load case 4), 5=-399(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-685/371, 3-4=-579/351, 4-5=-686/371, 5-6=0/47
BOT CHORD 2-8=-293/565, 7-8=-301/578, 5-7=-280/566
WEBS 3-8=-92/157, 3-7=-39/40, 4-7=-88/164

JOINT STRESS INDEX
2 = 0.55, 3 = 0.16, 4 = 0.18, 5 = 0.55, 7 = 0.11 and 8 = 0.11

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 399 lb uplift at joint 2 and 399 lb uplift at joint 5.
6) Girder carries hip end with 3-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 32 lb up at 6-2-0, and 63 lb down and 32 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

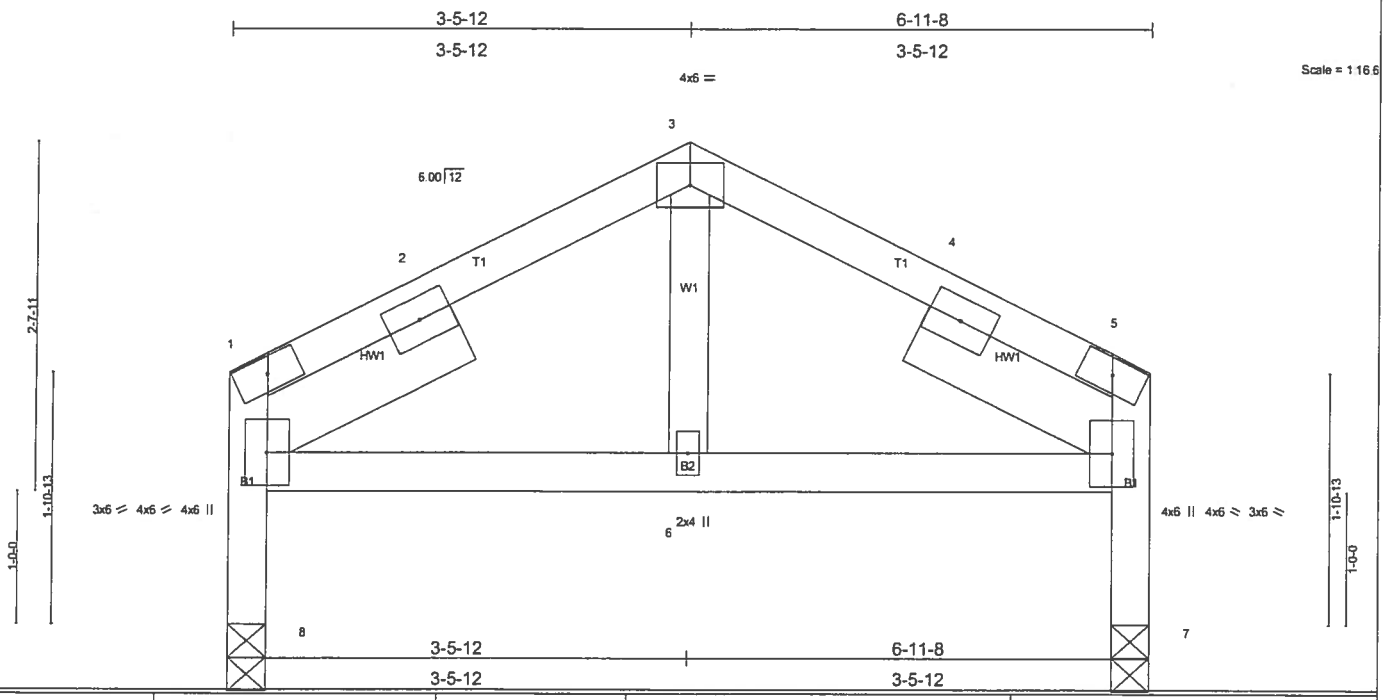
LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-64(F=-10), 4-6=-54, 2-8=-30, 7-8=-35(F=-5), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-63(F) 7=-63(F)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061208

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T12	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:44 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.10	Vert(LL)	-0.00	1-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.08	Vert(TL)	-0.01	1-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.02	Horz(TL)	-0.02	7	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 38 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
SLIDER Left 2 X 6 SYP No.1D 1-8-12, Right 2 X 6 SYP No.1D 1-8-12

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 7=280/0-3-8, 8=280/0-3-8
Max Horz 8=28(load case 4)
Max Uplift7=-84(load case 6), 8=-84(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-301/189, 2-3=-261/198, 3-4=-261/198, 4-5=-301/189
BOT CHORD 1-6=-100/240, 5-6=-100/240, 1-8=-280/170, 5-7=-280/170
WEBS 3-6=0/66

JOINT STRESS INDEX
1 = 0.24, 1 = 0.11, 1 = 0.13, 2 = 0.00, 3 = 0.29, 4 = 0.00, 5 = 0.24, 5 = 0.11, 5 = 0.13 and 6 = 0.08

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Bearing at joint(s) 7, 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 84 lb uplift at joint 7 and 84 lb uplift at joint 8.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

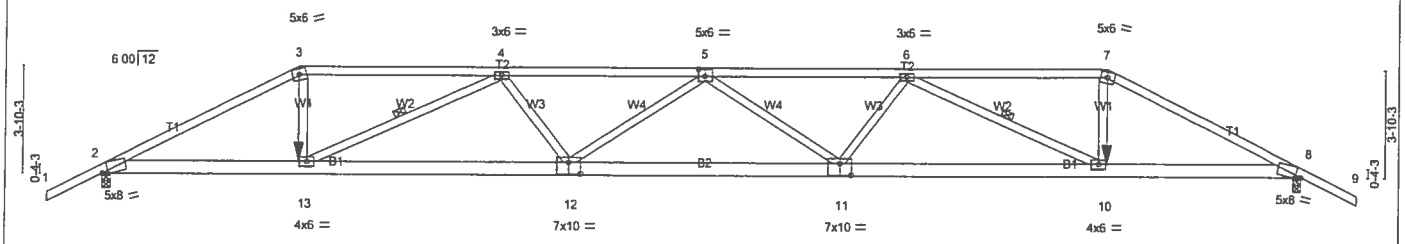
Dwg.#0905061209

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T13	HIP	1	2	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:45 2006 Page 1

-2-0-0	7-0-0	14-2-10	21-5-4	28-7-14	35-10-8	42-10-8	44-10-8
2-0-0	7-0-0	7-2-10	7-2-10	7-2-10	7-2-10	7-0-0	2-0-0

Scale = 1:78.5



7-0-0	16-7-8	26-3-0	35-10-8	42-10-8
7-0-0	9-7-8	9-7-8	9-7-8	7-0-0

Plate Offsets (X,Y): [2:0-2-7,Edge], [5:0-3-0,0-3-0], [8:0-2-7,Edge], [11:0-5-0,0-4-12], [12:0-5-0,0-4-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.76	Vert(LL)	-0.64	11-12	>803	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.74	Vert(TL)	-1.02	11-12	>500	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.57	Horz(TL)	0.20	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 468 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T2 2 X 4 SYP No.1D, T2 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-7-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 7-9-10 oc bracing.
WEBS 1 Row at midpt 4-13, 6-10

REACTIONS (lb/size) 2=3866/0-3-8, 8=3866/0-3-8
Max Horz 2=-89(load case 5)
Max Uplift2=-1600(load case 4), 8=-1600(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/51, 2-3=-7874/3368, 3-4=-7089/3103, 4-5=-11743/5107, 5-6=-11743/5107, 6-7=-7089/3103, 7-8=-7874/3368, 8-9=0/51
BOT CHORD 2-13=-2975/6967, 12-13=-4889/11009, 11-12=-5478/12324, 10-11=-4850/11009, 8-10=-2936/6967
WEBS 3-13=-1108/2914, 4-13=-4450/2152, 4-12=-266/1309, 5-12=-743/591, 5-11=-743/591, 6-11=-266/1309, 6-10=-4450/2152, 7-10=-1108/2914

JOINT STRESS INDEX
2 = 0.75, 3 = 0.75, 4 = 0.69, 5 = 0.81, 6 = 0.69, 7 = 0.75, 8 = 0.75, 10 = 0.67, 11 = 0.97, 12 = 0.97 and 13 = 0.67

NOTES
1) 2-ply truss to be connected together with 0.131"x3" Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
5) Provide adequate drainage to prevent water ponding.
6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1600 lb uplift at joint 2 and 1600 lb uplift at joint 8.
8) Girder carries hip end with 7-0-0 end setback.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 277 lb up at 35-10-8, and 539 lb down and 277 lb up at 7-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-7=-118(F=-64), 7-9=-54, 2-13=-30, 10-13=-65(F=-35), 8-10=-30
Concentrated Loads (lb)
Vert: 13=-539(F) 10=-539(F)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061210

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T14	HIP	1	1	Job Reference (optional)

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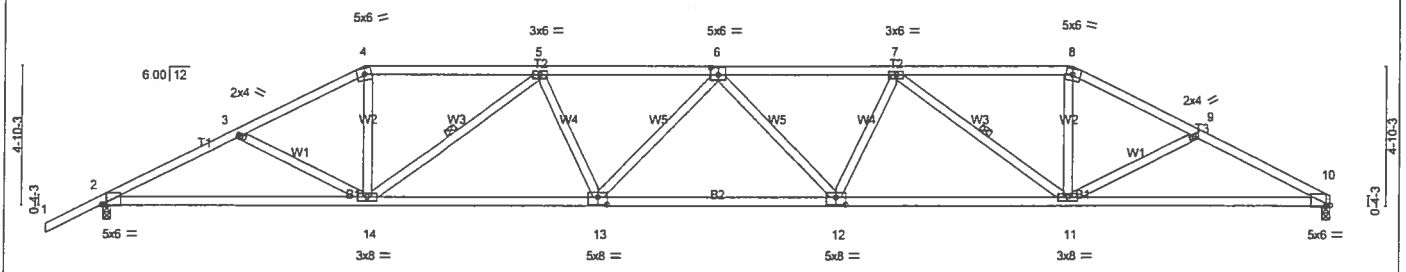
-2-0-0	4-9-4	9-0-0	15-2-10	21-5-4	27-7-14	33-10-8	38-1-4	42-10-8
2-0-0	4-9-4	4-2-12	6-2-10	6-2-10	6-2-10	6-2-10	4-2-12	4-9-4

Scale = 1:76.8



9-0-0	17-3-8	25-7-0	33-10-8	42-10-8
9-0-0	8-3-8	8-3-8	8-3-8	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-3-0,0-3-0], [10:0-1-11,Edge], [12:0-4-0,0-3-0], [13:0-4-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.42	Vert(LL)	-0.53	12-13	>956	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.86	12-13	>591	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.25	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 215 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-11-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-14, 7-11

REACTIONS (lb/size) 10=1786/0-3-8, 2=1907/0-3-8
Max Horz 2=126(load case 5)
Max Uplift10=-498(load case 3), 2=-609(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3385/1371, 3-4=-3180/1282, 4-5=-2838/1213, 5-6=-4022/1674, 6-7=-4027/1681, 7-8=-2856/1242, 8-9=-3202/1318, 9-10=-3424/1435
BOT CHORD 2-14=-1132/2963, 13-14=-1406/3835, 12-13=-1556/4176, 11-12=-1420/3843, 10-11=-1204/3007
WEBS 3-14=-183/185, 4-14=-341/1099, 5-14=-1314/547, 5-13=-94/488, 6-13=-274/180, 6-12=-270/177, 7-12=-90/483, 7-11=-1307/542, 8-11=-366/1115, 9-11=-213/230

JOINT STRESS INDEX
2 = 0.85, 3 = 0.34, 4 = 0.61, 5 = 0.47, 6 = 0.56, 7 = 0.47, 8 = 0.61, 9 = 0.34, 10 = 0.85, 11 = 0.63, 12 = 0.91, 13 = 0.91 and 14 = 0.63

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 498 lb uplift at joint 10 and 609 lb uplift at joint 2.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

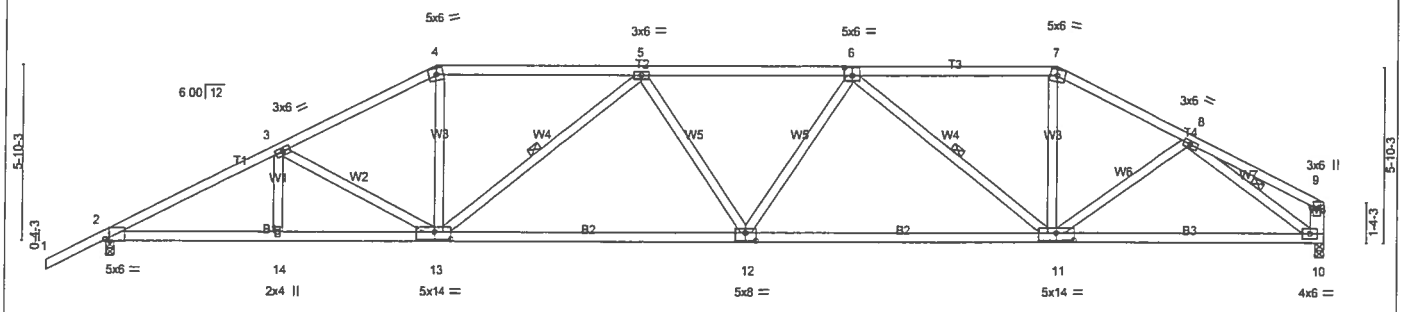
Dwg.#0905061211

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T15	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:47 2006 Page 1

-2-0-0	5-9-4	11-0-0	17-11-8	24-11-0	31-10-8	36-2-12	40-10-8
2-0-0	5-9-4	5-2-12	6-11-8	6-11-8	6-11-8	4-4-4	4-7-12

Scale = 1:73.6



5-9-4	11-0-0	21-5-4	31-10-8	40-10-8
5-9-4	5-2-12	10-5-4	10-5-4	9-0-0

Plate Offsets (X,Y): [2:0-1-11,Edge], [6:0-3-0,0-3-0], [11:0-7-0,0-3-0], [12:0-4-0,0-3-4], [13:0-7-0,0-3-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.44	12-13	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.91	Vert(TL)	-0.73	12-13	>663	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(TL)	0.18	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 219 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W8 2 X 6 SYP No.1D

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-3-4 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-10-10 oc bracing.
WEBS 1 Row at midpt 5-13, 6-11, 8-10

REACTIONS (lb/size) 2=1820/0-3-8, 10=1698/0-3-8
Max Horz 2=177(load case 5)
Max Uplift2=-609(load case 6), 10=-462(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-3249/1278, 3-4=-2840/1184, 4-5=-2488/1120, 5-6=-3113/1335, 6-7=-2175/996, 7-8=-2471/1050, 8-9=-414/163, 9-10=-304/170
BOT CHORD 2-14=-1103/2821, 13-14=-1103/2821, 12-13=-1151/3062, 11-12=-1111/2958, 10-11=-790/1954
WEBS 3-14=0/127, 3-13=-383/237, 4-13=-274/905, 5-13=-837/386, 5-12=0/159, 6-12=-11/335, 6-11=-1072/449, 7-11=-225/749, 8-11=-126/390, 8-10=-2074/897

JOINT STRESS INDEX
2 = 0.81, 3 = 0.41, 4 = 0.67, 5 = 0.43, 6 = 0.60, 7 = 0.64, 8 = 0.63, 9 = 0.54, 10 = 0.69, 11 = 0.64, 12 = 0.85, 13 = 0.67 and 14 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 609 lb uplift at joint 2 and 462 lb uplift at joint 10.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

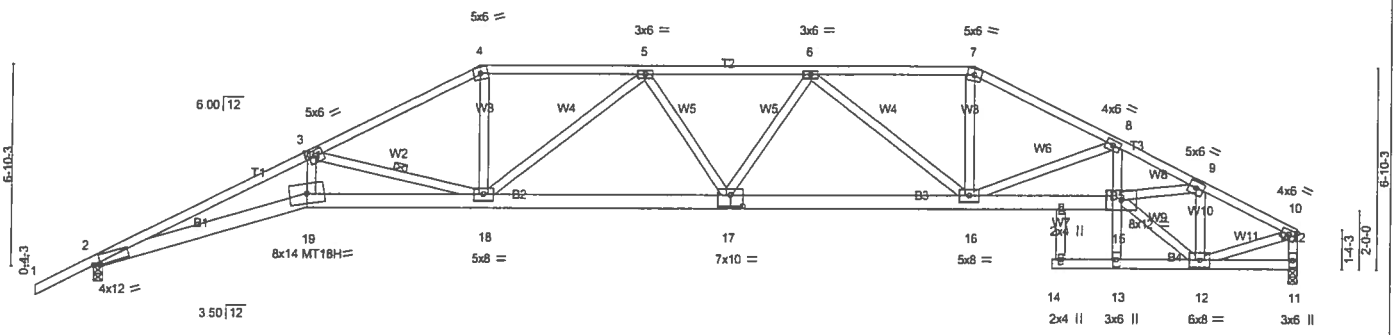
Dwg.#0905061212

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T16	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:48 2006 Page 1

-2-0-0	7-1-13	13-0-0	18-7-8	24-3-0	29-10-8	34-7-0	37-7-0	40-10-8
2-0-0	7-1-13	5-10-3	5-7-8	5-7-8	5-7-8	4-8-8	3-0-0	3-3-8

Scale = 1:74.7



7-1-13	13-0-0	21-5-4	29-10-8	32-7-0	34-7-0	37-7-0	40-10-8
7-1-13	5-10-3	8-5-4	8-5-4	2-8-8	2-0-0	3-0-0	3-3-8

Plate Offsets (X,Y): [2:0-2-12,0-0-10], [17:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	-0.65	18-19	>751	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.95	Vert(TL)	-1.04	18-19	>469	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.66	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 254 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.3, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-0-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-18

REACTIONS (lb/size) 2=1835/0-3-8, 11=1755/0-3-8
Max Horz 2=190(load case 5)
Max Uplift2=-625(load case 5), 11=-472(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-7056/2739, 3-4=-3895/1574, 4-5=-3472/1484, 5-6=-3898/1613, 6-7=-3197/1353, 7-8=-3564/1433, 8-9=-5468/2097, 9-10=-2161/860, 10-11=-1666/685
BOT CHORD 2-19=-2480/6471, 18-19=-2332/6037, 17-18=-1379/3874, 16-17=-1336/3783, 15-16=-1779/4878, 13-15=0/170, 8-15=-430/1375, 13-14=0/0, 12-13=-65/135, 11-12=-73/161
WEBS 3-19=-584/1845, 3-18=-2707/1182, 4-18=-464/1383, 5-18=-670/318, 5-17=-19/140, 6-17=-24/283, 6-16=-867/377, 7-16=-412/1262, 8-16=-1854/772, 12-15=-842/2249, 9-15=-1045/3002, 9-12=-2033/812, 10-12=-686/1831

JOINT STRESS INDEX
2 = 0.98, 3 = 0.87, 4 = 0.72, 5 = 0.42, 6 = 0.42, 7 = 0.66, 8 = 0.75, 9 = 0.98, 10 = 0.67, 11 = 0.35, 12 = 0.84, 13 = 0.28, 15 = 0.95, 16 = 0.42, 17 = 0.70, 18 = 0.67, 19 = 0.92, 20 = 0.34 and 21 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 625 lb uplift at joint 2 and 472 lb uplift at joint 11.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

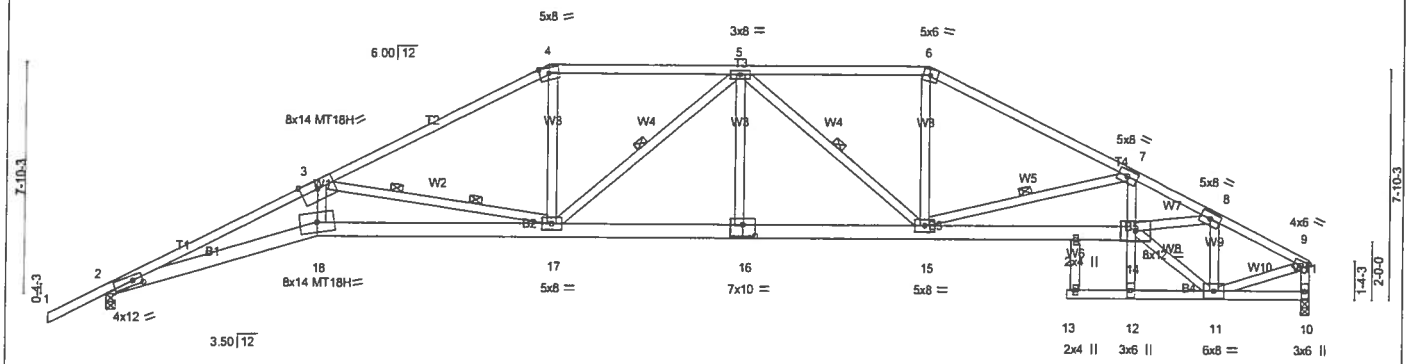
Dwg.#0905061213

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T17	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, FI 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:49 2006 Page 1

-2-0-0	7-1-13	15-0-0	21-5-4	27-10-8	32-7-0	37-7-0	40-10-8
2-0-0	7-1-13	7-10-3	6-5-4	6-5-4	4-8-8	5-0-0	3-3-8

Scale = 1:74.7



7-1-13	15-0-0	27-10-8	32-7-0	34-7-0	37-7-0	40-10-8
7-1-13	7-10-3	12-10-8	4-8-8	2-0-0	3-0-0	3-3-8

Plate Offsets (X,Y): [2:0-4-0,0-2-0], [16:0-5-0,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	1.00	Vert(LL)	-0.75	17-18	>652	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.94	Vert(TL)	-1.20	17-18	>406	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(TL)	0.72	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 260 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.3, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-15, 7-15, 5-17
2 Rows at 1/3 pts 3-17

REACTIONS (lb/size) 2=1835/0-3-8, 10=1755/0-3-8
Max Horz 2=204(load case 5)
Max Uplift2=-641(load case 5), 10=-490(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-7090/2803, 3-4=-3492/1433, 4-5=-3074/1379, 5-6=-2880/1284, 6-7=-3266/1341, 7-8=-5487/2132, 8-9=-2159/868, 9-10=-1666/692
BOT CHORD 2-18=-2547/6507, 17-18=-2478/6325, 16-17=-1085/3205, 15-16=-1084/3207, 14-15=-1859/4964, 12-14=-1/169, 7-14=-400/1372, 12-13=0/0, 11-12=-75/132, 10-11=-78/164
WEBS 3-18=-581/1881, 3-17=-3337/1460, 4-17=-311/1090, 5-15=-592/252, 6-15=-301/1033, 7-15=-2195/947, 11-14=-833/2250, 8-14=-1079/3030, 8-11=-2031/810, 9-11=-686/1825, 5-16=0/114, 5-17=-394/206

JOINT STRESS INDEX
2 = 0.98, 3 = 0.88, 4 = 0.77, 5 = 0.57, 6 = 0.84, 7 = 0.68, 8 = 0.74, 9 = 0.68, 10 = 0.35, 11 = 0.84, 12 = 0.28, 14 = 0.93, 15 = 0.62, 16 = 0.59, 17 = 0.84, 18 = 0.95, 19 = 0.34 and 20 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 641 lb uplift at joint 2 and 490 lb uplift at joint 10.

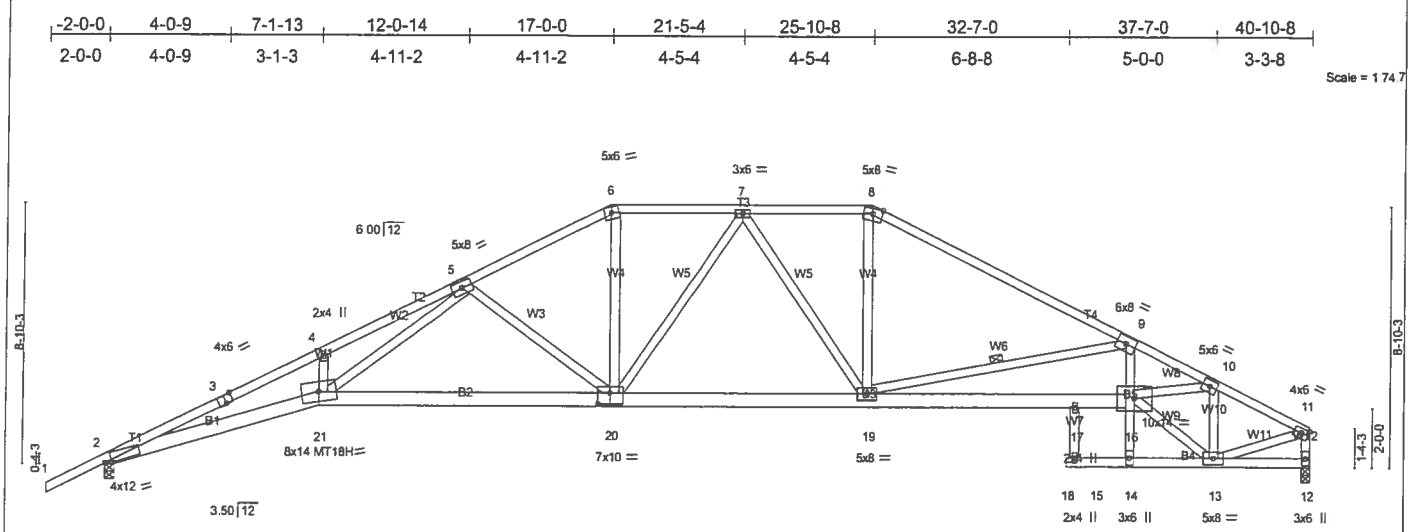
LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061214

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T18	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:50 2006 Page 1



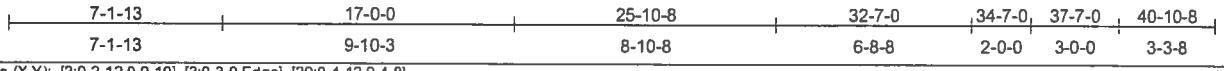


Plate Offsets (X,Y): [2:0-2-12,0-0-10], [3:0-3-0,Edge], [20:0-4-12,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.85	Vert(LL)	-0.73	20-21	>664	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.89	Vert(TL)	-1.18	20-21	>413	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.86	Horz(TL)	0.65	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 264 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.1D *Except*
T3 2 X 4 SYP No.2, T4 2 X 4 SYP No.2
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.2, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W2 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied or 5-1-15 oc bracing.
WEBS 1 Row at midpt 9-19

REACTIONS (lb/size) 2=1825/0-3-8, 12=1708/0-3-8
Max Horz 2=219(load case 5)
Max Uplift2=-658(load case 5), 12=-515(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-7007/2753, 3-4=-6882/2766, 4-5=-6928/2937, 5-6=-3046/1338, 6-7=-2666/1252, 7-8=-2557/1224, 8-9=-2944/1256, 9-10=-5012/2055, 10-11=-2110/891, 11-12=-1636/715
BOT CHORD 2-21=-2500/6422, 20-21=-1359/3564, 19-20=-884/2658, 17-19=-1850/4583, 16-17=-1847/4586, 14-16=-90/111, 9-16=-298/1073, 15-18=0/0, 14-15=0/0, 13-14=-168/309, 12-13=-79/154
WEBS 4-21=-214/291, 5-21=-1386/3372, 5-20=-1137/615, 6-20=-373/1059, 7-20=-178/153, 7-19=-378/186, 8-19=-215/839, 9-19=-2097/1035, 13-16=-740/1963, 10-16=-1001/2660, 10-13=-1761/728, 11-13=-706/1786, 15-17=-88/198

JOINT STRESS INDEX
2 = 0.98, 3 = 0.98, 4 = 0.34, 5 = 0.90, 6 = 0.54, 7 = 0.42, 8 = 0.80, 9 = 0.85, 10 = 0.87, 11 = 0.68, 12 = 0.32, 13 = 0.92, 14 = 0.91, 15 = 0.34, 16 = 0.88, 17 = 0.34, 19 = 0.55, 20 = 0.59 and 21 = 0.84

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 658 lb uplift at joint 2 and 515 lb uplift at joint 12.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

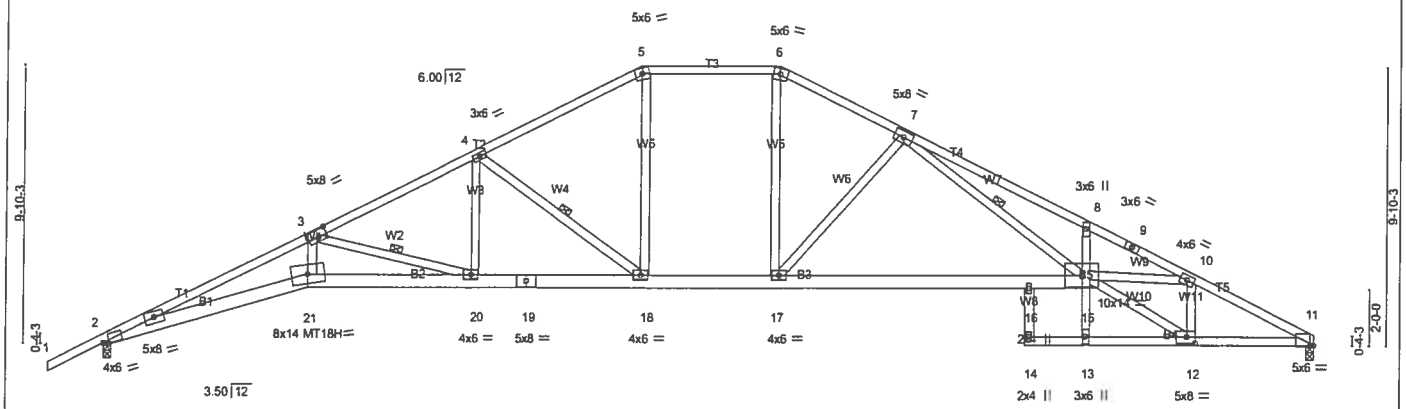
Dwg.#0905061215

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T19	SPECIAL	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industnes, Inc. Tue Sep 05 11:22:51 2006 Page 1

-2-0-0	7-1-13	13-0-14	19-0-0	23-10-8	28-2-12	38-5-8	42-10-8
2-0-0	7-1-13	5-11-2	5-11-2	4-10-8	4-4-4	10-2-12	4-5-0

Scale = 1:77.8



7-1-13	13-0-14	19-0-0	23-10-8	32-7-0	34-7-0	38-5-8	42-10-8
7-1-13	5-11-2	5-11-2	4-10-8	8-8-8	2-0-0	3-10-8	4-5-0

Plate Offsets (X,Y): [2:0-2-8,Edge], [3:0-3-12,0-3-0], [11:0-1-11,Edge], [12:0-3-8,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	-0.81	16-17	>631	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-1.31	16-17	>391	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.87	Horz(TL)	0.72	11	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 263 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T4 2 X 4 SYP No.1D, T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.3, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W7 2 X 4 SYP No.2, W10 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-20, 4-18, 7-15
JOINTS 1 Brace at Jt(s): 15

REACTIONS (lb/size) 2=1907/0-3-8, 11=1786/0-3-8
Max Horz 2=197(load case 5)
Max Uplift2=-692(load case 5), 11=-567(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-7424/2948, 3-4=-4123/1749, 4-5=-2903/1312, 5-6=-2552/1244, 6-7=-2903/1341, 7-8=-6398/2742, 8-9=-6202/2549, 9-10=-6258/2542, 10-11=-3527/1506
BOT CHORD 2-21=-2615/6813, 20-21=-2458/6357, 19-20=-1295/3631, 18-19=-1295/3631, 17-18=-758/2552, 16-17=-1142/3207, 15-16=-1142/3207, 13-15=0/81, 8-15=-289/293, 13-14=0/0, 12-13=-134/178, 11-12=-1270/3088
WEBS 3-21=-618/1950, 3-20=-2829/1208, 4-20=-340/1043, 4-18=-1394/676, 5-18=-349/940, 6-17=-453/1098, 7-17=-1042/605, 7-15=-1302/3163, 12-15=-1301/3334, 10-15=-842/2461, 10-12=-1510/650, 14-16=-58/86

JOINT STRESS INDEX
2 = 1.00, 2 = 0.80, 3 = 0.93, 4 = 0.77, 5 = 0.68, 6 = 0.54, 7 = 0.91, 8 = 0.27, 9 = 0.89, 10 = 0.87, 11 = 0.86, 12 = 0.77, 13 = 0.62, 14 = 0.34, 15 = 0.86, 16 = 0.34, 17 = 0.50, 18 = 0.43, 19 = 0.73, 20 = 0.79 and 21 = 0.96

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) Provide adequate drainage to prevent water ponding.
4) All plates are MT20 plates unless otherwise indicated.
5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 692 lb uplift at joint 2 and 567 lb uplift at joint 11.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061216

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T20	SPECIAL	2	1	Job Reference (optional)

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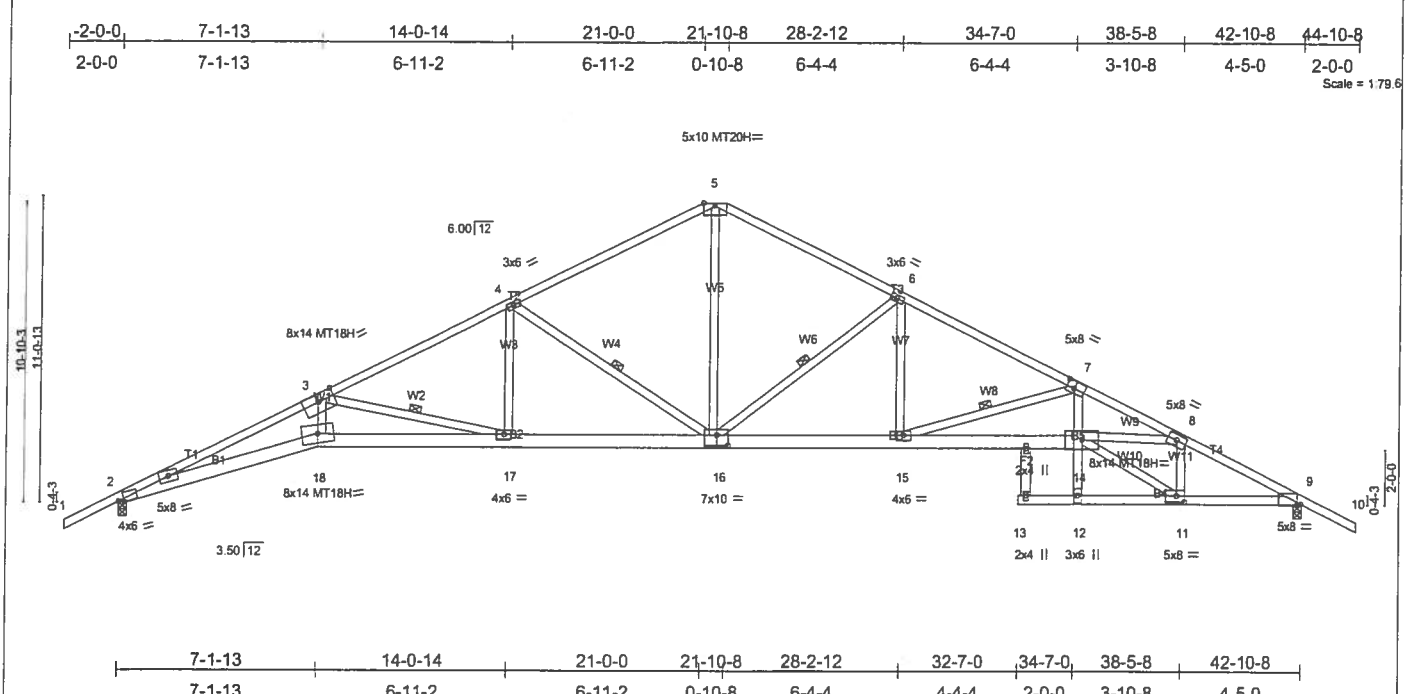


Plate Offsets (X,Y): [2:0-2-8,Edge], [7:0-3-8,0-3-0], [9:0-1-11,Edge], [11:0-3-8,0-2-8], [16:0-4-12,0-4-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.81	Vert(LL)	-0.82	13	>624	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-1.32	13	>388	180	MT20H	187/143
BCLL	10.0	Rep Stress Incr	YES	WB	0.96	Horz(TL)	0.81	9	n/a	n/a	MT18H	244/190
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 270 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.2, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W10 2 X 4 SYP No.2, W9 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-11-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-17, 4-16, 6-16, 7-15

REACTIONS (lb/size) 2=1918/0-3-8, 9=1955/0-3-8
Max Horz 2=189(load case 5)
Max Uplift2=-702(load case 5), 9=-695(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-7480/2868, 3-4=-3953/1626, 4-5=-2589/1172, 5-6=-2614/1190, 6-7=-3826/1565, 7-8=-7277/2704, 8-9=-3565/1392, 9-10=0/47
BOT CHORD 2-18=-2463/6866, 17-18=-2387/6645, 16-17=-1098/3470, 15-16=-1022/3362, 14-15=-2192/6435, 12-14=-2/174, 7-14=-673/2298, 12-13=0/0, 11-12=-95/203, 9-11=-1079/3105
WEBS 3-18=-571/1961, 3-17=-3248/1319, 4-17=-284/1025, 4-16=-1483/714, 6-16=-1399/647, 6-15=-285/1043, 7-15=-3225/1227, 11-14=-1159/3418, 8-14=-1095/3358, 8-11=-1759/689, 5-16=-741/1868

JOINT STRESS INDEX
2 = 1.00, 2 = 0.81, 3 = 0.90, 4 = 0.76, 5 = 0.81, 6 = 0.77, 7 = 0.95, 8 = 0.83, 9 = 0.81, 11 = 0.79, 12 = 0.40, 14 = 0.79, 15 = 0.89, 16 = 0.59, 17 = 0.92, 18 = 0.99, 19 = 0.34 and 20 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 702 lb uplift at joint 2 and 695 lb uplift at joint 9.

LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

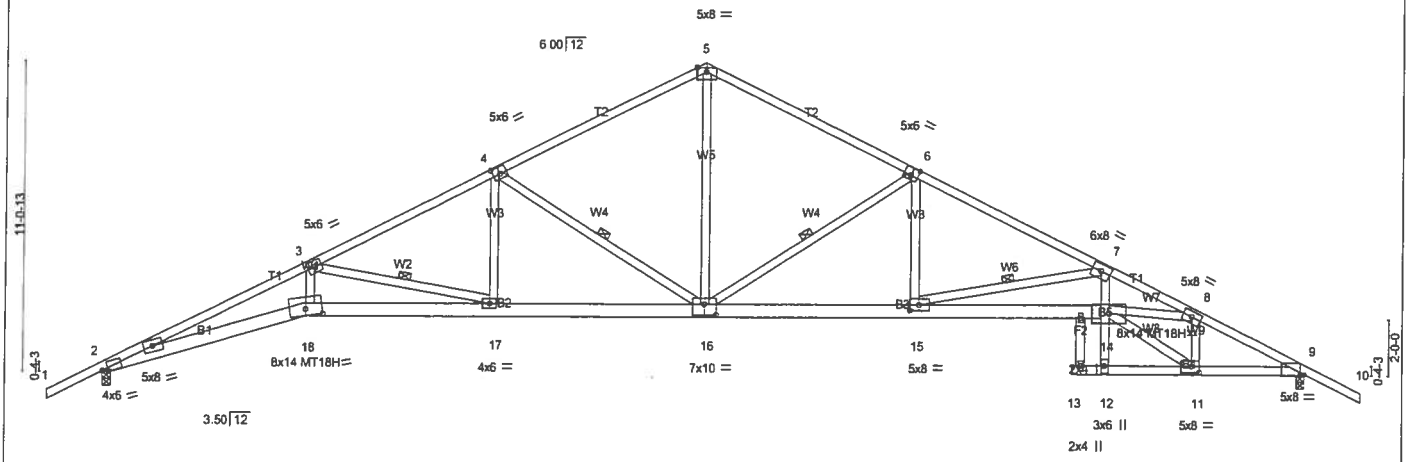
Dwg.#0905061217

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T21	SPECIAL	2	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:52 2006 Page 1

-2-0-0	7-1-13	13-11-0	21-5-4	28-11-8	35-7-0	38-11-8	42-10-8	44-10-8
2-0-0	7-1-13	6-9-4	7-6-4	7-6-4	6-7-8	3-4-8	3-11-0	2-0-0

Scale = 1:78.5



7-1-13	13-11-0	21-5-4	28-7-0	34-7-0	35-7-0	38-11-8	42-10-8
7-1-13	6-9-3	7-6-4	7-1-12	6-0-1	1-0-0	3-4-8	3-11-0

Plate Offsets (X,Y): [2:0-2-8,Edge], [4:0-3-0,0-3-0], [6:0-3-0,0-3-4], [9:0-1-11,Edge], [11:0-3-8,0-2-8], [15:0-3-8,0-2-8], [16:0-5-0,0-4-8], [18:0-7-0,0-2-12]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.87	Vert(LL)	-0.77	17-18	>666	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.97	Vert(TL)	-1.23	17-18	>416	180	MT18H	244/190
BCLL	10.0	Rep Stress Incr	YES	WB	0.95	Horz(TL)	0.84	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 267 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2 *Except*
T1 2 X 4 SYP No.1D, T1 2 X 4 SYP No.1D
BOT CHORD 2 X 6 SYP No.1D *Except*
B5 2 X 4 SYP No.1D, B4 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3 *Except*
W8 2 X 4 SYP No.2, W7 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 3-17, 4-16, 6-16, 7-15

REACTIONS (lb/size) 2=1910/0-3-8, 9=1933/0-3-8
Max Horz 2=189(load case 5)
Max Uplift2=-704(load case 5), 9=-699(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/49, 2-3=-7445/2862, 3-4=-3956/1645, 4-5=-2590/1182, 5-6=-2591/1183, 6-7=-3966/1637, 7-8=-7844/2991, 8-9=-3513/1388, 9-10=0/47
BOT CHORD 2-18=-2455/6832, 17-18=-2308/6371, 16-17=-1114/3476, 15-16=-1112/3502, 14-15=-2515/7061, 12-14=-20/151, 7-14=-723/2331, 12-13=0/0, 11-12=-126/299, 9-11=-1078/3058
WEBS 3-18=-578/1968, 3-17=-2978/1228, 4-17=-293/1028, 4-16=-1481/714, 5-16=-719/1827, 6-16=-1512/712, 6-15=-277/1041, 7-15=-3648/1438, 11-14=-1149/3329, 8-14=-1375/3917, 8-11=-1854/732

JOINT STRESS INDEX
2 = 1.00, 2 = 0.80, 3 = 0.96, 4 = 0.83, 5 = 0.76, 6 = 0.75, 7 = 0.82, 8 = 0.96, 9 = 0.79, 11 = 0.79, 12 = 0.60, 14 = 0.97, 15 = 0.88, 16 = 0.57, 17 = 0.84, 18 = 0.78, 19 = 0.34 and 20 = 0.34

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are MT20 plates unless otherwise indicated.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 704 lb uplift at joint 2 and 699 lb uplift at joint 9.

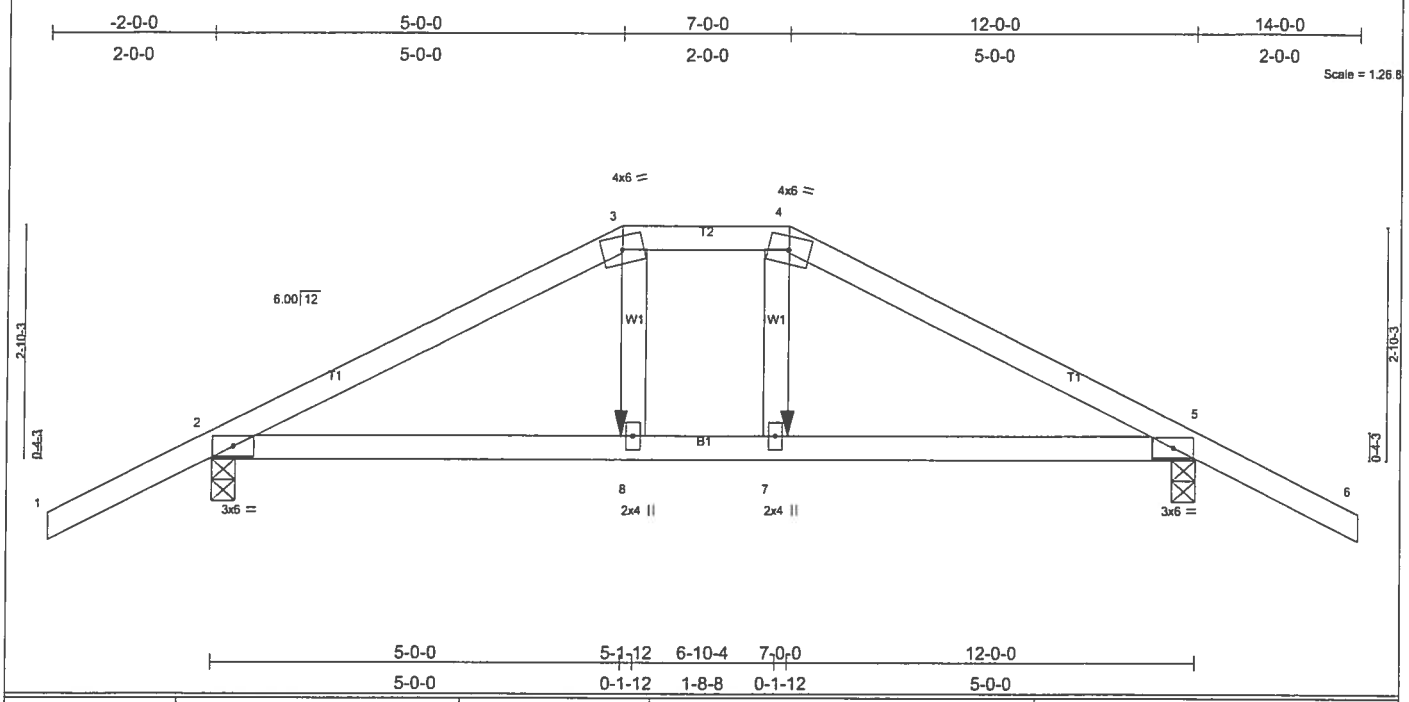
LOAD CASE(S) Standard

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061218

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T22	HIP	1	1	Job Reference (optional)

Builders FirstSource, Lake City, Fl 32055 — 6.200 s Jul 13 2005 MiTek Industries, Inc. Tue Sep 05 11:22:53 2006 Page 1



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.05	2-8	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.07	2-8	>999	180		
BCLL	10.0	Rep Stress Incr	NO	WB	0.13	Horz(TL)	0.02	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 51 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-4-9 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=907/0-3-8, 5=907/0-3-8
Max Horz 2=73(load case 4)
Max Uplift2=-439(load case 4), 5=-439(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-1314/491, 3-4=-1129/472, 4-5=-1314/491, 5-6=0/47
BOT CHORD 2-8=-361/1108, 7-8=-365/1129, 5-7=-358/1108
WEBS 3-8=-121/409, 4-7=-121/409

JOINT STRESS INDEX
2 = 0.60, 3 = 0.36, 4 = 0.36, 5 = 0.60, 7 = 0.30 and 8 = 0.30

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
3) Provide adequate drainage to prevent water ponding.
4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 439 lb uplift at joint 2 and 439 lb uplift at joint 5.
6) Girder carries hip end with 5-0-0 end setback.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 245 lb down and 126 lb up at 7-0-0, and 245 lb down and 126 lb up at 5-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-91(F=-37), 4-6=-54, 2-8=-30, 7-8=-50(F=-20), 5-7=-30
Concentrated Loads (lb)
Vert: 8=-245(F) 7=-245(F)

SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
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STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Dwg.#0905061219

Job	Truss	Truss Type	Qty	Ply	CORNERSTONE LOT 10 MAGNOLIA HILLS
L209566	T23	KINGPOST	3	1	Job Reference (optional)

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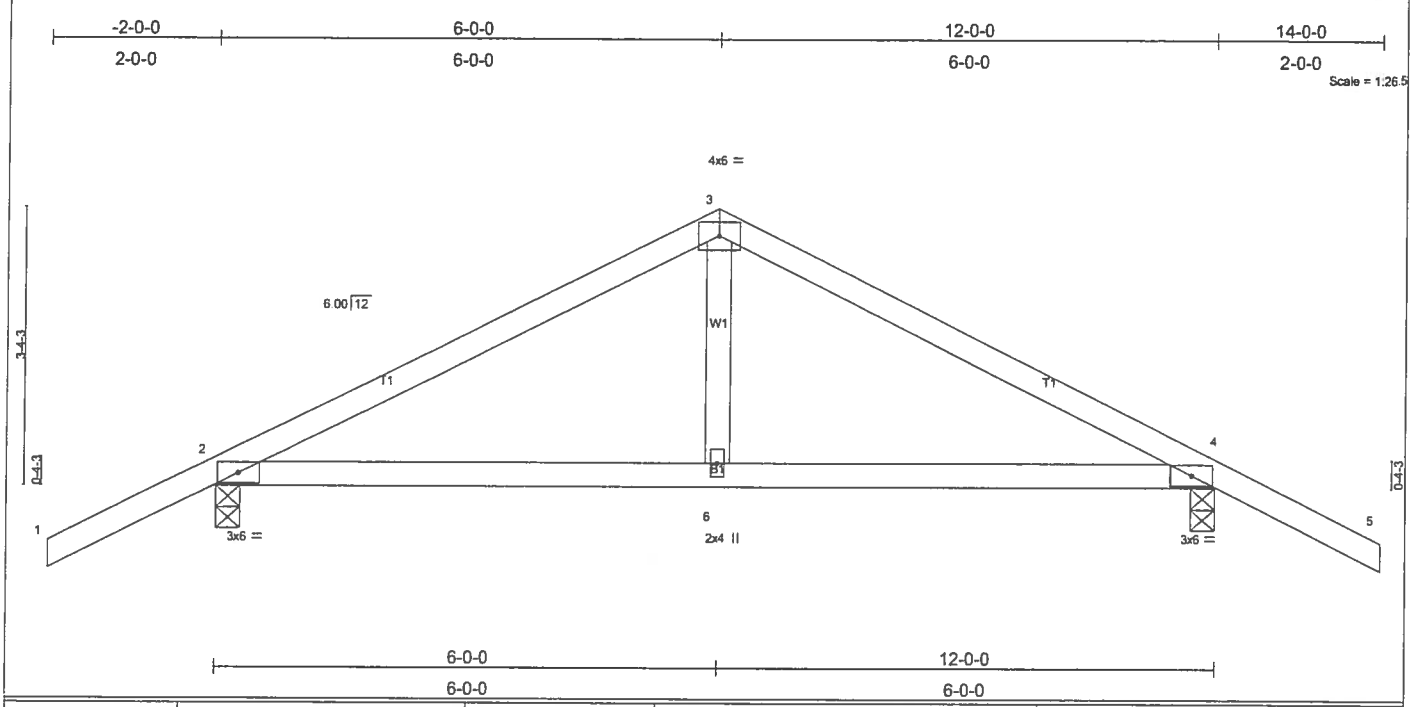


LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.04	4-6	>999	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.06	2-6	>999	180		
BCLL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.01	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Matrix)							Weight: 49 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=608/0-3-8, 4=608/0-3-8
Max Horz 2=80(load case 5)
Max Uplift2=-289(load case 5), 4=-289(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/47, 2-3=-659/259, 3-4=-659/259, 4-5=0/47
BOT CHORD 2-6=-73/526, 4-6=-73/526
WEBS 3-6=0/203

JOINT STRESS INDEX
2 = 0.42, 3 = 0.68, 4 = 0.42 and 6 = 0.15

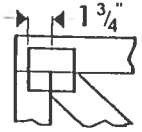
NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 289 lb uplift at joint 2 and 289 lb uplift at joint 4.

LOAD CASE(S) Standard

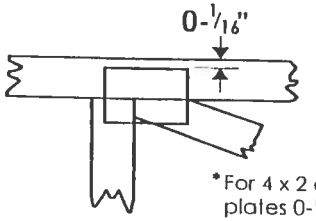
SEPTEMBER 05, 2006 TRUSS DESIGN ENGINEER:
THOMAS E. MILLER PE 56877, BYRON K. ANDERSON PE 60987
STRUCTURAL ENGINEERING AND INSPECTIONS, INC. EB 9196
16105 N. FLORIDA AVE. STE B, LUTZ, FL 33549

Symbols

PLATE LOCATION AND ORIENTATION



*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seal.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

***Plate location details available in MiTek 20/20 software or upon request.**

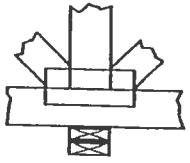
PLATE SIZE

4 X 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

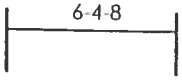


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

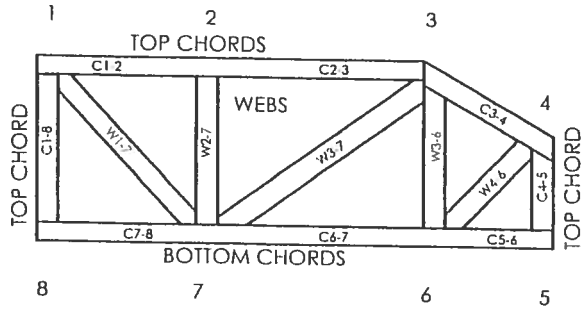
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK®

MiTek Engineering Reference Sheet: MII-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

Residential System Sizing Calculation

Summary

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

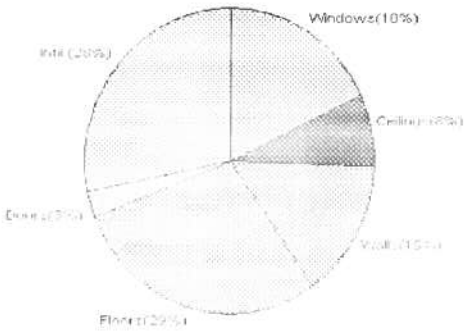
9/7/2006

Location for weather data: Gainesville - Defaults: Latitude(29) Altitude(152 ft.) Temp Range(M) Humidity data: Interior RH (50%) Outdoor wet bulb (77F) Humidity difference(54gr.)					
Winter design temperature	33	F	Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	F
Winter temperature difference	37	F	Summer temperature difference	17	F
Total heating load calculation	**27851**	**Btuh**	**Total cooling load calculation**	**22500**	**Btuh**
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	Btuh
Total (Electric Heat Pump)	114.9	32000	Sensible (SHR = 0.50)	90.8	16000
Heat Pump + Auxiliary(0.0kW)	114.9	32000	Latent	327.8	16000
			Total (Electric Heat Pump)	142.2	32000

WINTER CALCULATIONS

Winter Heating Load (for 1799 sqft)

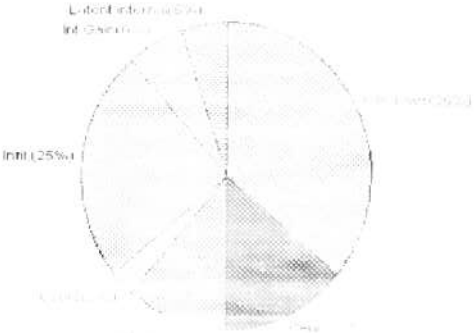
Load component			Load	
Window total	156	sqft	5022	Btuh
Wall total	1235	sqft	4056	Btuh
Door total	60	sqft	777	Btuh
Ceiling total	1859	sqft	2191	Btuh
Floor total	184	sqft	8033	Btuh
Infiltration	192	cfm	7773	Btuh
Duct loss			0	Btuh
Subtotal			27851	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			27851	Btuh



SUMMER CALCULATIONS

Summer Cooling Load (for 1799 sqft)

Load component			Load	
Window total	156	sqft	8121	Btuh
Wall total	1235	sqft	2576	Btuh
Door total	60	sqft	588	Btuh
Ceiling total	1859	sqft	3079	Btuh
Floor total			0	Btuh
Infiltration	101	cfm	1875	Btuh
Internal gain			1380	Btuh
Duct gain			0	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			17618	Btuh
Latent gain(ducts)			0	Btuh
Latent gain(infiltration)			1252	Btuh
Latent gain(ventilation)			3	Btuh
Latent gain(internal/occupants/others)			1200	Btuh
Total latent gain			4882	Btuh
TOTAL HEAT GAIN			22500	Btuh



For Florida residences only

EnergyGauge® System Sizing
PREPARED BY:
DATE: 9-7-06

System Sizing Calculations - Winter

Residential Load - Whole House Component Details

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

9/7/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	SE	60.0	32.2	1931 Btuh
5	2, Clear, Metal, 0.87	SE	13.3	32.2	428 Btuh
6	2, Clear, Metal, 0.87	NE	2.7	32.2	87 Btuh
	Window Total		156(sqft)		5022 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1002	3.3	3291 Btuh
2	Frame - Wood - Ext(0.09)	13.0	233	3.3	765 Btuh
	Wall Total		1235		4056 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1859	1.2	2191 Btuh
	Ceiling Total		1859		2191Btuh
Floors	Type	R-Value	Size X	HTM=	Load
1	Slab On Grade	0	184.0 ft(p)	43.7	8033 Btuh
	Floor Total		184		8033 Btuh
			Zone Envelope Subtotal:		19818 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.80	14392	191.9	7773 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.00)	0 Btuh
Zone #1			Sensible Zone Subtotal		27851 Btuh

WHOLE HOUSE TOTALS

	Subtotal Sensible	27851 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	27851 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title:
608232CornerStoneTheGriffin

Lake City, FL

Class 3 Rating
Registration No. 0
Climate: North

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Winter

Residential Load - Room by Room Component Details

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

9/7/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	NW	45.0	32.2	1449 Btuh
2	2, Clear, Metal, 0.87	NW	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	SW	15.0	32.2	483 Btuh
4	2, Clear, Metal, 0.87	SE	60.0	32.2	1931 Btuh
5	2, Clear, Metal, 0.87	SE	13.3	32.2	428 Btuh
6	2, Clear, Metal, 0.87	NE	2.7	32.2	87 Btuh
	Window Total		156(sqft)		5022 Btuh
Walls	Type	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1002	3.3	3291 Btuh
2	Frame - Wood - Ext(0.09)	13.0	233	3.3	765 Btuh
	Wall Total		1235		4056 Btuh
Doors	Type		Area X	HTM=	Load
1	Insulated - Adjacent		20	12.9	259 Btuh
2	Insulated - Exterior		20	12.9	259 Btuh
3	Insulated - Exterior		20	12.9	259 Btuh
	Door Total		60		777Btuh
Ceilings	Type/Color/Surfac	R-Val e	, a X	HTM=	Load
1	Vented Attic/D/Shin)	30.0	1859	1.2	2191 Btuh
	Ceiling Total		1859		2191Btuh
Floors	Type	R-Value	Size X	HTM=	Loa l
1	Slab On Grade	0	184.0 ft(p)	43.7	8033 Btuh
	Floor Total		184		8033 Btuh
			Zone Envelope Subtotal:		20078 Btuh
Infiltration	Type	ACH X	Zone Volume	CFM=	
	Natural	0.30	14392	191.9	7773 Btuh
Ductload	Average sealed, R6.0, Supply(Attic), Return(Attic)			(DLM of 0.30)	0 Btuh
Zone #1			Sensible Zone Subtotal		27851 Btuh

WHOLE HOUSE TOTALS

	Subtotal Sensible	27851 Btuh
	Ventilation Sensible	0 Btuh
	Total Btuh Loss	27851 Btuh

Manual J Winter Calculations

Residential Load - Component Details (continued)

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Whole House Component Details

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F 9/7/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Whole House

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Overhang Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	HTM Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	60.0	24.3	35.7	29	63	2937 Btuh
5	2, Clear, 0.87, None,N,N	SE	6ft.	7.16	13.3	13.3	0.0	29	63	385 Btuh
6	2, Clear, 0.87, None,N,N	NE	1.5ft.	1.16	2.7	0.0	2.7	29	60	162 Btuh
	Window Total				156 (sqft)					8121 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1002.0	2.1	2090 Btuh
2	Frame - Wood - Ext	13.0/0.09	233.0	2.1	486 Btuh
	Wall Total		1235 (sqft)		2576 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceilings	Type/Color/Surface	R-Value	Area(sqft)	HTM	Load
1	Vented Attic/DarkShingle	30.0	1859.0	1.7	307 Btuh
	Ceiling Total		1859 (sqft)		3179 Btuh

Floors	Type	R-Value	Size	HTM	Load
1	Slab On Grade	0.0	184 (ft(p))	0.0	0 Btuh
	Floor Total		184.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		14363 Btuh
Infiltration	Type	ACH	Volume(cuft)	CFM=	Load
	SensibleNatural	0.42	14392	100.7	1875 Btuh
Internal gain		Occupants	Btuh/occupant	Appliance	Load
		6	X 230 +	0	1380 Btuh
Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)			DGM = 0.00	0.0 Btuh
			Sensible Zone Load		17618 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

9/7/2006

WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	**17618 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**17618 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
Whole House	**Total sensible gain**	**17618 Btuh**
Totals for Cooling	Latent infiltration gain (for 54 gr. humidity difference)	3682 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	4882 Btuh
	TOTAL GAIN	22500 Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

System Sizing Calculations - Summer

Residential Load - Room by Room Component Details

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

Reference City: Gainesville (Defaults) Summer Temperature Difference: 17.0 F

9/7/2006

This calculation is for Worst Case. The house has been rotated 315 degrees.

Component Loads for Zone #1: Main

Window	Type* Pn/SHGC/U/InSh/ExSh/IS	Ornt	Overhang Len	Hgt	Window Area(sqft) Gross	Shaded	Unshaded	HTM Shaded	Unshaded	Load
1	2, Clear, 0.87, None,N,N	NW	1.5ft.	5.5ft.	45.0	0.0	45.0	29	60	2702 Btuh
2	2, Clear, 0.87, None,N,N	NW	1.5ft.	6.5ft.	20.0	0.0	20.0	29	60	1201 Btuh
3	2, Clear, 0.87, None,N,N	SW	1.5ft.	5.5ft.	15.0	6.1	8.9	29	63	734 Btuh
4	2, Clear, 0.87, None,N,N	SE	1.5ft.	5.5ft.	60.0	24.3	35.7	29	63	2937 Btuh
5	2, Clear, 0.87, None,N,N	SE	6ft.	7.16	13.3	13.3	0.0	29	63	385 Btuh
6	2, Clear, 0.87, None,N,N	NE	1.5ft.	1.16	2.7	0.0	2.7	29	60	162 Btuh
	Window Total				156 (sqft)					8121 Btuh

Walls	Type	R-Value/U-Value	Area(sqft)	HTM	Load
1	Frame - Wood - Ext	13.0/0.09	1002.0	2.1	2090 Btuh
2	Frame - Wood - Ext	13.0/0.09	233.0	2.1	486 Btuh
	Wall Total		1235 (sqft)		2576 Btuh

Doors	Type	Area (sqft)	HTM	Load
1	Insulated - Adjacent	20.0	9.8	196 Btuh
2	Insulated - Exterior	20.0	9.8	196 Btuh
3	Insulated - Exterior	20.0	9.8	196 Btuh
	Door Total	60 (sqft)		588 Btuh

Ceili	Ty[illegible]e/Color/Surfac	R-Value	Area(sqft)	HTM	oad
1	Vented Attic/DarkShingle	30.0	1859.0	1.7	3079 Btuh
	Ceiling Total		1859 (sqft)		3079 Btuh

Fl	Type	R-Value	Si[illegible]	HTM	Load
1	Slab On Grade	0.0	184 (ft(p))	0.0	0 Btuh
	Floor Total		184.0 (sqft)		0 Btuh

			Zone Envelope Subtotal:		14363

Infiltration	Type	ACH	Volume(cuft)	CFM =	Load
	SensibleNatural	0.42	14392	100.7	1875 Btuh

Internal gain	Occupants	Btuh/occupant	Appliance	Load
	6	X 230 +	0	1380 Btuh

Duct load	Average sealed, R6.0, Supply(Attic), Return(Attic)	DGM = 0.00	0.0 Btuh
		Sensible Zone Load	17618 Btuh

Manual J Summer Calculations

Residential Load - Component Details (continued)

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

Lake City, FL

9/7/2006

WHOLE HOUSE TOTALS

Whole House Totals for Cooling	**Sensible Envelope Load All Zones**	**17618 Btuh**
	Sensible Duct Load	0 Btuh
	Total Sensible Zone Loads	**17618 Btuh**
	Sensible ventilation	0 Btuh
	Blower	0 Btuh
	Total sensible gain	**17618 Btuh**
	Latent infiltration gain (for 54 gr. humidity difference)	3682 Btuh
	Latent ventilation gain	0 Btuh
	Latent duct gain	0 Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200 Btuh
	Latent other gain	0 Btuh
	Latent total gain	4882 Btuh
	TOTAL GAIN	22500 Btuh

*Key: Window types (Pn - Number of panes of glass)
(SHGC - Shading coefficient of glass or SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Drapes(D) or Roller Shade(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))
(Ornt - compass orientation)

For Florida residences only

Residential Window Diversity

MidSummer

Project Title:
608232CornerStoneTheGriffin

Class 3 Rating
Registration No. 0
Climate: North

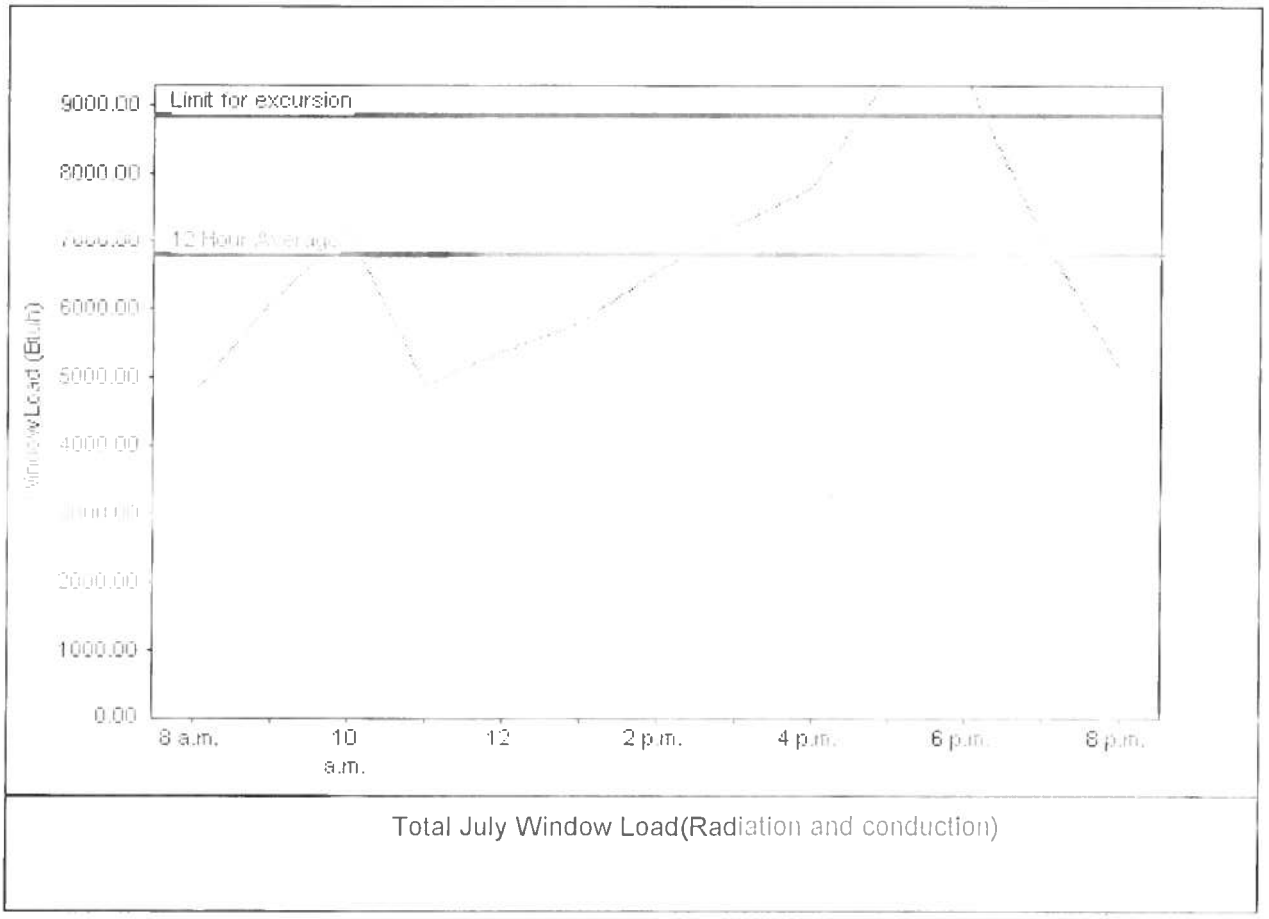
Lake City, FL

9/7/2006

Weather data for: Gainesville - Defaults

Summer design temperature	92 F	Average window load for July	6803 Btuh
Summer setpoint	75 F	Peak window load for July	9489 Btuh
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	8844 Btuh
Latitude	29 North	Window excursion (July)	645 Btuh

WINDOW Average and Peak Loads



Total July Window Load(Radiation and conduction)

Warning: This application has glass areas that produce relatively large heat gains for part of the day. Variable air volume devices may be required to overcome spikes in solar gain for one or more rooms. A cooling system may be required or some rooms may require zone control.

EnergyGauge® System Sizing for Florida residences only
PREPARED BY:
DATE: 9-7-06

From: The Columbia County Building & Zoning Department
Plan Review
135 NE Hernando Av.
P.O. Box 1529
Lake City Florida 32056-1529

Reference to a building permit application Number: 0609-69

Bryan Zecher Construction Owner Cornerstone Development Property ID# 28-3s-16-02377-110

On the date of September 26, 2006 application 0609-69 and plans for construction of a single family dwelling were reviewed and the following information or alteration to the plans will be required to continue processing this application. If you should have any question please contact the above address, or contact phone number (386) 758-1163 or fax any information to (386) 754-7088.

Please include application number 0609-69 and when making reference to this application.

This is a plan review for compliance with the Florida Residential Code 2004 only and doesn't make any consideration toward the land use and zoning requirements.

1. Please provide a copy of a signed released site plan from the Columbia County Environmental Health Department which confirms approval of the waste water disposal system.

2. Please verify compliance with the FRC-2004 section R308.4 Hazardous locations :(Master Bathroom) Glazing in doors and enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers. Glazing in any part of a building wall enclosing these compartments where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) measured vertically above any standing or walking surface. Each pane of glazing installed in hazardous locations as defined in Section R308.4 shall be provided with a manufacturer's or installer's label, designating the type and thickness of glass and the safety glazing standard with which it complies, which is visible in the final installation. The label shall be acid etched, sandblasted, ceramic-fired, embossed mark, or shall be of a type which once applied cannot be removed without being destroyed.

3. Within the garage area is a room which two appliances are located, one of these appliances being a HVAC unit to which the FRC-2004 sections R309.1.1 requires any duct penetration: Ducts in the garage and ducts penetrating the walls or ceilings separating the dwelling from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other approved material and shall have no openings into the garage. Please indicate the method to be used to comply with the FRC-2004 sections R309.1.1section

4. The attic access opening (pull down ladder type attic egress door) in the garage ceiling shall have the same protection requirements of FRC-2004 C: R309.2 Separation required. The garage shall be separated from the residence and its

attic area by not less than ½-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch (15.9 mm) Type X gypsum board or equivalent. Where the separation is a floor-ceiling assembly, the structure supporting the separation shall also be protected by not less than ½-inch (12.7 mm) gypsum board or equivalent. Other openings between the garage and residence shall be equipped with solid wood doors not less than 13/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 13/8 inches (35 mm) thick, or 20-minute fire-rated doors.

Thank You:

Joe Haltiwanger
Plan Examiner
Columbia County Building
Department

25078

WILLIAM N. KITCHEN

PROFESSIONAL SURVEYOR AND MAPPER
152 N. MARION AVENUE
LAKE CITY, FLORIDA 32055
PHONE (386) 755-7786 FAX (386) 755-5506
E-MAIL BSSK@BELLSOUTH.NET

DATE : JAN. 24, 2007

CORNERSTONE DEVOLOPERS, LLC
180 NW AMENITY COURT
LAKE CITY, FLORIDA. 32055

To Whom It May Concern:

RE: FINISH FLOOR ELEVATION

SUBJECT PARCEL # 28-3S-16-02377-110 , LOT 10 MAGNOLIA HILLS,101 NW ETHELIND COURT, LAKE CITY, FL. THE AS-BUILT OF THE FINISH FLOOR IS AT ELEVATION 130.2 FEET. WHICH IS 1.7 FEET HIGHER THAN THE PROPOSED FINISH FLOOR ELEVATION OF 128.5 FEET. SEE ATTACH LETTER FROM CAL TEC TESTING.

Thank you,

WILLIAM N. KITCHEN PSM # 5490
William N. Kitchen
1-24-2007

Cal-Tech Testing, Inc.
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LABORATORIES

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July 24, 2006

Cornerstone Developers, LLC
180 NW Amenity Court
Lake City, Florida 32055

Attention: Chris Cox

Reference: Proposed Residence
Lot 10, Magnolia Hills
Columbia County, Florida
Cal-Tech Project No. 06-451

Dear Mr. Cox,

Cal-Tech Testing, Inc. has completed an investigation and evaluation of lot 10 of Magnolia Hills in Columbia County, Florida. The purposes of our work were to evaluate the potential for flooding of a home to be constructed on the lot and to provide recommendations for selecting a finished floor elevation.

Based upon the U. S. Coast and Geodetic Survey marker "BP19" located near the intersection of U. S. 90 and Brown Road, the elevation of the center of the cul-de-sac adjacent the proposed building site is approximately 130.9 feet. The ground surface elevation at the center of the building site is approximately 125.7 feet, and the proposed finished floor elevation is approximately 128.5 feet. This places the finished floor elevation about 2.4 feet below the elevation of the center of the cul-de-sac.

Columbia County regulations require the finished floor of a new residence to be at least 12 inches above the elevation of the adjacent roadway unless it can be shown such an elevation is not required to substantially reduce the likelihood of flooding.

Based upon the USGS topographic quadrangle map, the proposed home site is positioned within an isolated basin of approximately 100 acres for which the basin rim is estimated to be near elevation 153 feet. The FEMA flood map of Columbia County indicates there are no flood zones within this basin. Additionally, only one flood zone located within about one mile of the site has a flood elevation estimated to exceed 153 feet. This zone "A" flood area is located approximately 1/4 mile west of the site, and the flood elevation for this area is estimated to be about 155 feet; however, this flood area is topographically isolated such that flooding within this flood zone would not affect the proposed home site.

Since no flood areas are delineated within the basin by the FEMA flood map, we believe flooding within the basin sufficient to flood the home is highly unlikely. Such flooding would require maximum flood depths on the order of 15 feet within the basin and an average flood depth of about 6 feet over an area of about 20 acres.

Based upon our evaluation, it is our opinion elevating the finished floor to 1 foot above the adjacent roadway will not be required. However, we recommend the finished floor elevation be selected and/or the site be graded such that the finished floor is a minimum of 12 inches above the finished surface grade at the perimeter of the residence. With suitable site grading to provide separation between the finished floor and the surrounding surface grade, the proposed finished floor elevation of approximately 128.5 feet should be sufficient to substantially reduce the likelihood of flooding at the site.

We appreciate the opportunity to be of service on this project and look forward to a continued association. Please do not hesitate to contact us should you have questions concerning this report or if we may be of further assistance.

Respectfully submitted,
Cal-Tech Testing, Inc.

Linda Creamer
President / CEO

John C. Dorman, Jr., Ph.D., P.E.
Geotechnical Engineer