

Floor Plan Including:

- a) Rooms labeled and dimensioned
- b) Shear walls
- c) Windows and doors (including garage doors) showing size, mfg., approval listing and attachment specs. (FBC 1707) and safety glazing where needed
- d) Fireplaces (gas appliance) (vented or non-vented) or wood burning with hearth
- e) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails
- f) Must show and identify accessibility requirements (accessible bathroom)

Foundation Plan Including:

- a) Location of all load-bearing wall with required footings indicated as standard
- b) All posts and/or column footings including size and reinforcing
- c) Any special support required by soil analysis such as piling
- d) Location of any vertical steel

Roof System:

- a) Truss package including:
 - 1. Truss layout and truss details signed and sealed by FI. Pro. Eng.
 - 2. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
- b) Conventional Framing Layout including:
 - 1. Rafter size, species and spacing
 - 2. Attachment to wall and uplift
 - 3. Ridge beam sized and valley framing and support details
 - 4. Roof assembly (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)

Wall Sections Including:

- a) Masonry wall
 - 1. All materials making up wall
 - 2. Block size and mortar type with size and spacing of reinforcement
 - 3. Lintel, tie-beam sizes and reinforcement
 - 4. Gable ends with rake beams showing reinforcement or gable truss and wall bracing details
 - 5. All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation
 - 6. Roof assembly shown here or on roof system detail (FBC 104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with resistance rating)
 - 7. Fire resistant construction (if required)
 - 8. Fireproofing requirements
 - 9. Shoe type of termite treatment (termicide or alternative method)
 - 10. Slab on grade
 - a. Vapor retardant (6mil. Polyethylene with joints lapped 6 inches and sealed)
 - b. Must show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and supports
 - 11. Indicate where pressure treated wood will be placed
 - 12. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)

b) Wood frame wall

1. All materials making up wall
2. Size and species of studs
3. Sheathing size, type and nailing schedule
4. Headers sized
5. Gable end showing balloon framing detail or gable truss and wall hinge bracing detail
6. All required fasteners for continuous tie from roof to foundation (truss anchors, straps, anchor bolts and washers)
7. Roof assembly shown here or on roof system detail (FBC104.2.1 Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
8. Fire resistant construction (if applicable)
9. Fireproofing requirements
10. Show type of termite treatment (termicide or alternative method)
11. Slab on grade
- a. Vapor retardant (6Mil. Polyethylene with joints lapped 6 inches and sealed
- b. Must show control joints, synthetic fiber reinforcement or welded wire fabric reinforcement and supports
12. Indicate where pressure treated wood will be placed
13. Provide insulation R value for the following:
 - a. Attic space
 - b. Exterior wall cavity
 - c. Crawl space (if applicable)
- c) Metal frame wall and roof (designed, signed and sealed by Florida Prof. Engineer or Architect)

Floor Framing System:

- a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer
 - b) Floor joist size and spacing
 - c) Girder size and spacing
 - d) Attachment of joist to girder
 - e) Wind load requirements where applicable
- Plumbing Fixture layout**
- Electrical layout including:**
- a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
 - b) Ceiling fans
 - c) Smoke detectors

d) Service panel and sub-panel size and location(s)

- e) Meter location with type of service entrance (overhead or underground)
- f) Appliances and HVAC equipment
- g) Arc Fault Circuits (AFCI) in bedrooms
- HVAC information** *contractor gets*
 - a) Manual J sizing equipment or equivalent computation
 - b) Exhaust fans in bathroom

Energy Calculations (dimensions shall match plans)

Gas System Type (LP or Natural) Location and BTU demand of equipment

Disclosure Statement for Owner Builders

******Notice Of Commencement Required Before Any Inspections Will Be Done**

Private Potable Water

- a) Size of pump motor
- b) Size of pressure tank
- c) Cycle stop valve if used

911 Address ✓

8072 N.U.S Highway 441
Lake City, FL 32055

will get
after bank
closing

WING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

Building Permit Application: A current Building Permit Application form is to be completed and submitted for all residential projects.

2. **Parcel Number:** The parcel number (Tax ID number) from the Property Appraiser (386) 758-1084 is required. A copy of property deed is also requested.

3. **Environmental Health Permit or Sewer Tap Approval:** A copy of the Environmental Health permit, existing septic approval or sewer tap approval is required before a building permit can be issued. (386) 758-1058 (Toilet facilities shall be provided for construction workers)

4. **City Approval:** If the project is to be located within the city limits of the Town of Fort White, prior approval is required. The Town of Fort White approval letter is required to be submitted by the owner or contractor to this office when applying for a Building Permit. (386) 497-2321

5. **Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.8 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.7 of the Columbia County Land Development Regulations. **CERTIFIED FINISHED FLOOR ELEVATIONS WILL BE REQUIRED ON ANY PROJECT WHERE THE BASE FLOOD ELEVATION (100 YEAR FLOOD) HAS BEEN ESTABLISHED.**

A development permit will also be required. Development permit cost is \$50.00

6. **Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (\$25.00) must be made. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (\$50.00). All culvert waivers are sent to the Columbia County Public Works Department for approval or denial.
7. **911 Address:** If the project is located in an area where the 911 address has been issued, then the proper paperwork from the 911 Addressing Department must be submitted. (386) 752-8787

ALL REQUIRED INFORMATION IS TO BE SUBMITTED FOR REVIEW. YOU WILL BE NOTIFIED WHEN YOUR APPLICATION AND PLANS ARE APPROVED AND READY TO PERMIT. PLEASE DO NOT EXPECT OR REQUEST THAT PERMIT APPLICATIONS BE REVIEWED OR APPROVED WHILE YOU ARE HERE - TIME WILL NOT ALLOW THIS - PLEASE DO NOT ASK



January 31, 2002

TO: OUR FLORIDA CUSTOMERS:

Effective February 1, 2002, the following TAMKO shingles, as manufactured at TAMKO's Tusculoosa, Alabama, facility, comply with ASTM D-3161, Type I modified to 110 mph. Testing was conducted using four nails per shingle. These shingles also comply with Florida Building Code TAS 100 for wind driven rain.

- Class-Seal AR
- Elite Class-Seal AR
- ASTM Heritage 30 AR (formerly ASTM Heritage 25 AR)
- Heritage 40 AR (formerly Heritage 30 AR)
- Heritage 50 AR (formerly Heritage 40 AR)

All testing was performed by Florida State certified independent labs.

Please direct all questions to TAMKO's Technical Services Department at 1-800-641-4691.

TAMKO Roofing Products, Inc.

** LAMAR BOOZER PUTNAM STREET 900 EAST CITY, FL 32055
 RESIDENTIAL/LIGHT COMMERCIAL HVAC LOADS
 PROJECT: CLIENT: DATE: DESIGNER: LAMAR BOOZER
 CUSTOM NORTON DURANCE 10 22 05

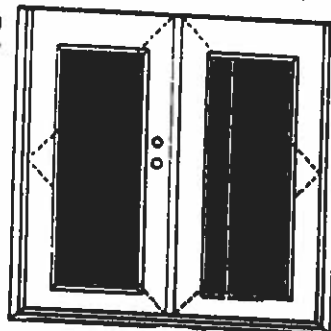
CLIENT INFORMATION:
 NAME: NORTON DURANCE
 ADDRESS: LAKE CITY, STATE: LAKE CITY, FLORIDA
 TOTAL BUILDING LOADS:

BLDG. LOAD DESCRIPTIONS									
AREA	SEN. LOSS	LAT. +	SEN. =	TOTAL	3-C WINDOW DBL PANE CLR GLS METL FR	12-D WALL R-11 + 1/2" ASPHLT BRD(R-1.3)	11-C DOOR METAL POLYSTYRENE CORE	16-G CEILING R-30 INSULATION	22-A SLAB ON GRADE NO EDGE INSUL
85	2,773	0	2,964	2,964	85	1,219	40	1,550	111
10	0	0	3,000	3,000	10	0	0	0	0
0	0	0	2,300	2,300	0	0	0	0	0
0	734	0	1,841	1,841	0	0	0	0	0
0	0	0	1,500	1,500	0	0	0	0	0
0	7,859	0	5,449	5,449	0	0	0	0	0
15,415	8,659	20,243	28,902	28,902	15,415	8,659	20,243	28,902	28,902
SUBTOTALS FOR STRUCTURE:					3,005	14,681	0	8,453	8,453
PEOPLE					10	0	0	0	0
APPLIANCES					0	0	0	0	0
DUCTWORK					0	0	0	0	0
INFILTRATION W.CFM:					0.0	0.0	0.0	0.0	0.0
VENTILATION W.CFM:					235.9	0.0	0.0	0.0	0.0
SENSIBLE GAIN TOTAL					0	0	0	0	0
TEMP. SWING MULTIPLIER					20,243	0	0	0	0
BUILDING LOAD TOTALS					20,243	0	0	0	0

SUPPLY CFM AT 20 DEG DT: 920
 SQUARE FT. OF ROOM AREA: 1,550
 TOTAL HEATING REQUIRED WITH OUTSIDE AIR: 15.415 MBH
 TOTAL COOLING REQUIRED WITH OUTSIDE AIR: 2.409 TONS
 CALCULATIONS ARE BASED ON 7TH EDITION OF ACCA MANUAL J.
 ALL COMPUTED RESULTS ARE ESTIMATES AS BUILDING USE AND WEATHER MAY VARY.
 BE SURE TO SELECT A UNIT THAT MEETS BOTH SENSIBLE AND LATENT LOADS.

WOOD-EDGE STEEL DOORS

APPROVED ARRANGEMENT:



Note:
Units of other sizes are covered by this report as long as the panels used do not exceed 3'0" x 6'8".

Double Door
 Lockdown with 20" x 24"

Design Pressure

+40.5/-40.5

1. 1. The first step in the design process is to define the problem.
 2. 2. The second step is to generate concepts.
 3. 3. The third step is to develop a solution.
 4. 4. The fourth step is to construct a prototype.
 5. 5. The fifth step is to evaluate the prototype.
 6. 6. The sixth step is to refine the design.
 7. 7. The seventh step is to produce the final design.
 8. 8. The eighth step is to test the final design.
 9. 9. The ninth step is to implement the final design.
 10. 10. The tenth step is to maintain the final design.

Large Kinetic Impact Resistance

Hurricane protective system (shutters) is REQUIRED.

Actual design pressure and impact resistance requirements for a specific building design and geographic location is determined by ASCE 7 national code or local building codes specify the edition required.

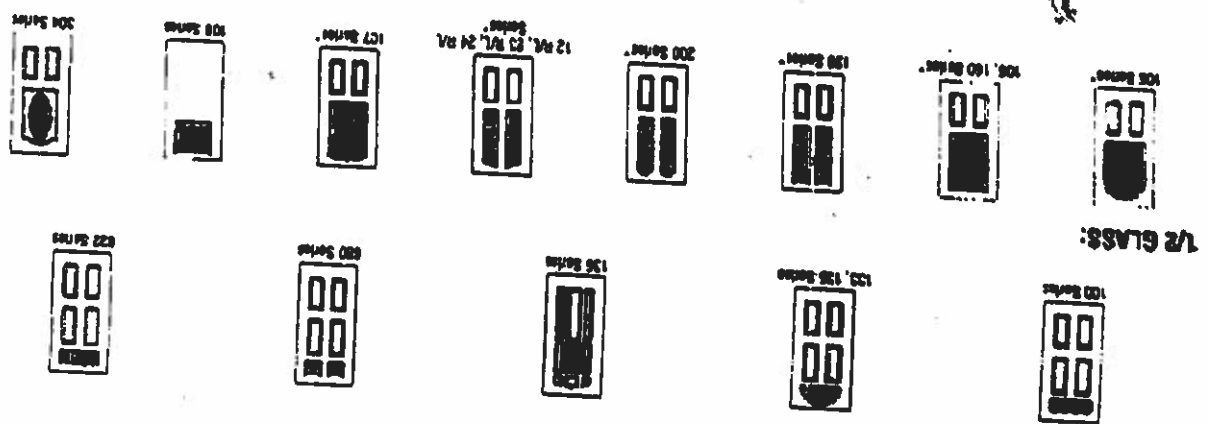
MINIMUM ASSEMBLY DETAIL:

Compliance requires that minimum assembly details have been followed - see MAD-WL-MA0002-02 and MAD-WL-MA0041-02.

MINIMUM INSTALLATION DETAIL:

Compliance requires that minimum installation details have been followed - see MID-WL-MA0002-02.

APPROVED DOOR STYLES:



This chart may also be used in the following color codes: 0-pink; 6-pink; 6-pink with white; 6-pink; 6-pink; 6-pink with white.

Johnson[®] Entersystems

Our continuing program of research and development makes significant contributions to the health and safety of the community.

MASONITE
Masonite International Corporation
800-441-4411
www.masonite.com

XX
Glazed Inswing Unit

COP-WI-JM4142-02

WOOD-EDGE STEEL DOORS

APPROVED DOOR STYLES:



3/4 GLASS:



FULL GLASS:



CERTIFIED TEST REPORTS:

NCTL 210-1897-7, 8, 9, 10, 11, 12; NCTL 210-1861-4, 5, 6, 10, 11, 12; NCTL 210-2185-1, 2, 3

Certifying Engineer and License Number: Barry D. Portney, P.E. / 16258.

Unit Tested in Accordance with Miami-Dade BCCO PA202.

Evaluation report NCTL-210-2794-1

Door panels constructed from 26-gauge 0.017" thick steel sills. Both sills constructed from wood. Top end rails constructed of 0.041" steel. Bottom end rails constructed of 0.021" steel. Interior cavity of slab filled with rigid polyurethane foam core. Slab glazed with insulated glass mounted in a rigid plastic lip like surround.

Frame constructed of wood with an extruded aluminum threshold.

PRODUCT COMPLIANCE LABELING:

TESTED IN
ACCORDANCE WITH
MIAMI-DADE BCCO PA202
COMPANY NAME
CITY, STATE

To the best of my knowledge and ability the above side-hinged exterior door unit conforms to the requirements of the 2001 Florida Building Code, Chapter 17 (Structural Tests and Inspections).

Kurt A. Bartz

State of Florida, Professional Engineer
Kurt Bartz, P.E. - License Number 56533

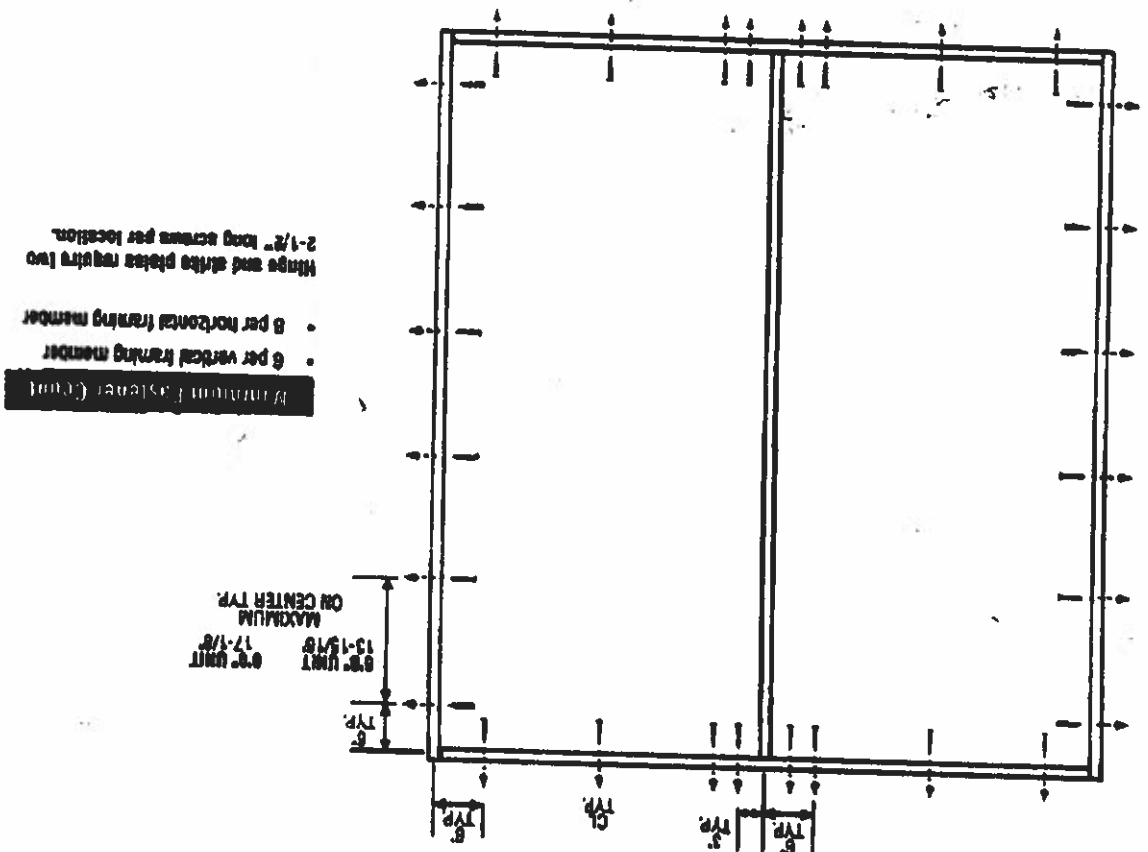
Johnson
Windows & Doors

March 29, 2002
Our company is a member of the International Window & Door Manufacturers Association (IWDA) and is proud to be a part of this organization.

PREMIER
Premium Quality Door

Masonite
Manufactured from
Masonite International Corporation

DOUBLE DOOR



Latching Hardware:

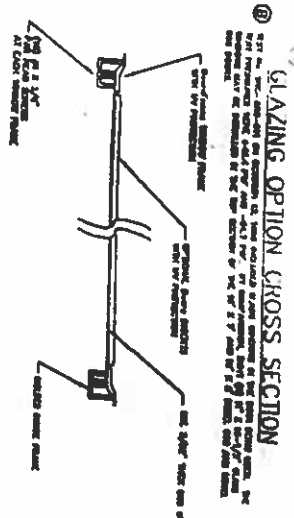
- Compliance requires that GRADE 2 or better (ANSI/BHMA A156.2) cylindrical and deadlock hardware be installed.

Notes:

1. Anchor calculations have been carried out with the lowest (least) fastener rating from the different fasteners being considered for use. Fasteners analyzed for this unit include #8 and #10 wood screws or 3/16" Tapcons.
2. The wood screw single shear design values come from Table 11.3A of ANS/AF & PA NOS for southern pine lumber with a side member thickness of 1-1/4" and penetration of minimum embedment. The 3/16" Tapcon single shear design values come from the ITW and EICO Dads County approvals respectively, each with minimum 1-1/4" embedment.
3. Wood ducts by others, must be anchored properly to transfer loads to the structure.



March 29, 2002
Our ongoing program of product improvement from suggestions, designs and product field testing is always underway.

[illegible]

**AAMA/NWDA 101/S.2-97
TEST REPORT**

Rendered to:

MI HOME PRODUCTS, INC.

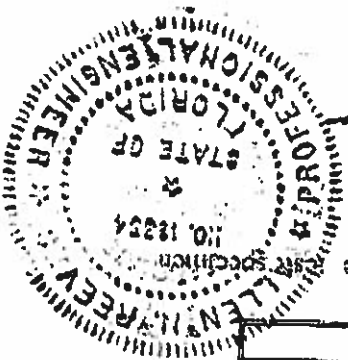
SERIES/MODEL: 650

TYPE: Aluminum Triple Single Hung Window

Title of Test	Summary of Results
AAMA Rating	H-R35 112 x 72
Uniform Load Deflection Test Pressure	+35.3 psf -47.2 psf
Operating Force	25 lb max.
Air Infiltration	0.16 cfm/ft ²
Water Resistance Test Pressure	5.25 psf
Uniform Load Structural Test Pressure	+53.0 psf -52.5 psf
De-glazing	Passed
Forced Entry Resistance	Grade 10

Reference should be made to ATT Report No. 01-41641.01 for complete description and data.

Allen H. R...
7 June 2002



AAMA/NWDA 101/1.S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.
P.O. Box 370
650 West Market Street
Gatz, Pennsylvania 17030-0370

Report No: 01-41641.01
Test Date: 05/13/02
And: 05/16/02
Report Date: 06/05/02
Expiration Date: 05/16/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to witness testing on a Series/Model 650, aluminum triple single hung window at their facility located in Elizabethtown, Pennsylvania. The sample tested successfully met the performance requirements for a H-R35 112 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/NWDA 101/1.S.2-97, *Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors*.

Test Specimen Description:

Series/Model: 650

Type: Aluminum Triple Single Hung Window

Overall Size: 9' 3-1/2" wide by 5' 11-11/16" high

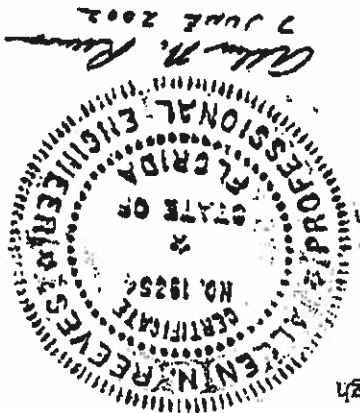
Active Sash Size (3): 3' 0-1/4" wide by 2' 10-3/4" high

Fixed Daylight Opening Size (3): 2' 8-1/4" wide by 2' 9-1/8" high

Screen Size (3): 2' 9-1/8" wide by 2' 11" high

Finish: All aluminum was painted white.

130 Darry Court
York, PA 17402-9405
Phone: 717.764.7700
Fax: 717.764.4129
www.archtest.com



Allen H. Rouse
7 June 2002

Test Specimen Description: (Continued)

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl spacer system. The active sash was channel glazed utilizing a flexible vinyl wrap-around gasket. The fixed lite was interior glazed against double-sided adhesive foam tape and secured with PVC snap-in glazing beads.

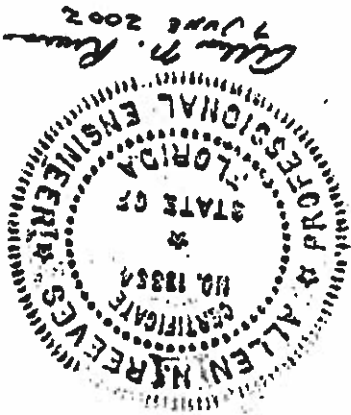
Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypropylene with center fin	Row	Fixed meeting rail
0.250" high by 0.187" backed polypropylene with center fin	2 Rows	Active sash stiles
1/2" by 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam filled vinyl bulb seal	1 Row	Active sash, bottom rail

Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. The mullions were secured utilizing four #8 x 1-1/4" screws through the head and sill into the mullion screw boss.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each stile screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.



Test Specimen Description: (Continued)

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	1	Midspace of each active meeting rail with adjacent keepers
Plastic tilt latch	2	Each active sash meeting rail ends
Metal tilt pin	2	Each active sash bottom rail ends
Balance assembly	2	Each active sash contained one in each jamb
Screen plunger	2	Each screen contained two 4" from rail ends on top rail
Drainage: Sloped sill		

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	25 lbs	30 lbs max.
	Air Infiltration (ASTM E 283-91) @ 1.57 psf (25 mph)	0.16 cfm/ft ²	0.3 cfm/ft ² max
	Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/I.S. 2-97 for air infiltration.		

Water Resistance (ASTM E 547-00) (with and without screen) WTP = 2.86 psf

No leakage

Allen, R. 2 June 2002



Test Results: (Continued)

Paragraph Title of Test - Test Method Results Allowed

2.1.4.1 Uniform Load Deflection (ASTM E 330-97)

(Measurements reported were taken on the mullion)

(Loads were held for 52 seconds)

@ 15.0 psf (positive)

@ 15.0 psf (negative)

0.15"

0.29"

0.41" max.

0.41" max.

Uniform Load Structural (ASTM E 330-97)

(Measurements reported were taken on the mullion)

(Loads were held for 10 seconds)

@ 22.5 psf (positive)

@ 22.5 psf (negative)

0.01"

0.01"

0.29" max.

0.29" max.

2.2.6.2 Deglazing Test (ASTM E 987-88)

In operating direction at 70 lbs

Right sash, meeting rail

Right sash, bottom rail

Middle sash, meeting rail

Middle sash, bottom rail

Left sash, meeting rail

Left sash, bottom rail

0.12"/25%

0.12"/25%

0.12"/25%

0.12"/25%

0.12"/25%

0.12"/25%

0.50"/100%

0.50"/100%

0.50"/100%

0.50"/100%

0.50"/100%

0.50"/100%

In remaining direction at 50 lbs

Right sash, right stile

Right sash, left stile

Middle sash, right stile

Middle sash, left stile

Left sash, right stile

Left sash, left stile

0.06"/12%

0.06"/12%

0.06"/12%

0.06"/12%

0.06"/12%

0.06"/12%

0.50"/100%

0.50"/100%

0.50"/100%

0.50"/100%

0.50"/100%

0.50"/100%

Forced Entry Resistance (ASTM F 588-97)

Type: A

Grade: 10

Lock Manipulation Test

Test A1 through A5

Test A7

Lock Manipulation Test

No entry

No entry

No entry

No entry

No entry

No entry

No entry



Test Results: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
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4.3	Water Resistance (ASTM E 547-00) (with and without screen) WTP = 5.25 psf	No leakage	No leakage
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	Uniform Load Deflection (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 52 seconds) @ 35.3 psf (positive) @ 47.2 psf (negative)	0.46"* 0.67"*	0.41" max 0.41" max
--	--	------------------	------------------------

*Exceeds L/175 for deflection, but meets all other test requirements

	Uniform Load Structural (ASTM E 330-97) (Measurements reported were taken on the mullion) (Loads were held for 10 seconds) @ 53.0 psf (positive) @ 52.5 psf (negative)	0.03" 0.02"	0.29" max 0.29" max
--	--	----------------	------------------------

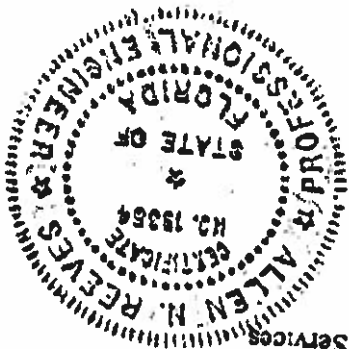
Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess
Technician

MAH:mb
01-41641.01

Allen N. Reeves, P.E.
Director - Engineering Services
7 June 2002





ACCEPTANCE NO.: 01-0313.06
APPROVED: APR 19 2001
EXPIRES: February 19, 2006

NOTICE OF ACCEPTANCE: SPECIFIC CONDITIONS

1. DESCRIPTION OF UNIT
1.1 This renews the Notice of Acceptance No. 97-0910.11 which was issued on February 19, 1998. It approves a residential insulated steel door, as described in Section 2 of this Notice of Acceptance, designed to comply with the South Florida Building Code (SFBC), 1994 Edition for Miami-Dade County, for the locations where the pressure requirements, as determined by SFBC Chapter 23, do not exceed the Design Pressure Rating values indicated in the approved drawings.
2. PRODUCT DESCRIPTION
2.1 The Series Entry 6' 8" S-W/E Inswing Opaque Residential Insulated Steel Door in a Hollow Metal Frame-Impact and its components shall be constructed in strict compliance with the following documents: Drawing No 31-1032-EV-1, Sheets 1 through 5 of 5, titled "Premdor (Entry Brand Wood Edge) 3'0" x 6'8" Steel door in a Hollow Metal Frame (Inswing)" dated 6/25/97 with revision C, dated 3/20/01, bearing the Miami-Dade County Product Approval stamp with the Notice of Acceptance number and approval date by the Miami-Dade County Product Control Division. These documents shall hereinafter be referred to as the approved drawings.

3. INSTALLATIONS
3.1 This approval applies to single unit applications of single door only, as shown in approved drawings.
3.2 Unit shall be installed only at locations protected by a canopy or overhang such that the angle between the edge of canopy or overhang to sill is less than 45 degrees. Unless unit is installed in non-habitable areas where the unit and the area are designed to accept water infiltration.
4. INSTALLATION
4.1 The residential insulated steel door and its components shall be installed in strict compliance with the approved drawings.
4.2 Hurricane protection system (shutters): the installation of this unit will not require a hurricane protection system.

5. LABELING
5.1 Each unit shall bear a permanent label with the manufacturer's name or logo, city, state and following statement: "Miami-Dade County Product Control Approved".
6. BUILDING PERMIT REQUIREMENTS
6.1 Application for building permit shall be accompanied by copies of the following:
6.1.1 This Notice of Acceptance
6.1.2 Duplicate copies of the approved drawings, as identified in Section 2 of this Notice of Acceptance, clearly marked to show the components selected for the proposed installation.
6.1.3 Any other documents required by the Building Official or the South Florida Building Code (SFBC) in order to properly evaluate the installation of this system.

Manuel Perez, P.E. Product Control Division
Product Control Examiner

**AAMA/NWDA 101/1.S.2-97
TEST REPORT SUMMARY**

Rendered to:

MI HOME PRODUCTS, INC.

**SERIES/MODEL: 650 Fin
TYPE: Aluminum Single Hung Window**

Title of Test	Results
Overall Design Pressure	H-R40 52 x 72 +45.0 psf
Operating Force	-47.2 psf
Air Infiltration	11 lb max.
Water Resistance	0.13 cfm/ft ²
Structural Test Pressure	6.00 psf +67.5 psf
De-glazing	-70.8 psf
Forced Entry Resistance	Passed
Grade 10	

Reference should be made to Report No. 01-41134.01 dated 03/26/02 for complete test specimen description and data.

For ARCHITECTURAL TESTING, INC.

Mark A. Hess

Mark A. Hess, Technician

MAR:nlb



Architectural Testing

AAMA/WWDA 101/S.2-97 TEST REPORT

Rendered to

MI HOME PRODUCTS, INC.

650 West Market Street

P.O. Box 370

Graft, Pennsylvania 17030-0370

Report No: 01-41134.01
Test Date: 03/07/02
Report Date: 03/26/02
Expiration Date: 03/07/06

Project Summary: Architectural Testing, Inc. (ATI) was contracted by MI Home Products, Inc. to perform tests on Series/Model 650 Fin, aluminum single hung window at their facility located in Elizabethville, Pennsylvania. The samples tested successfully met the performance requirements for a H-R40 52 x 72 rating.

Test Specification: The test specimen was evaluated in accordance with AAMA/WWDA 101/S.2-97, Voluntary Specifications for Aluminum, Vinyl (PVC) and Wood Windows and Glass Doors.

Test Specimen Description

Series/Model: 650 Fin

Type: Aluminum Single Hung Window

Overall Size: 4'-4-1/4" wide by 6'-0-3/8" high

Active Sash Size: 4'-1-3/4" wide by 3'-0-5/8" high

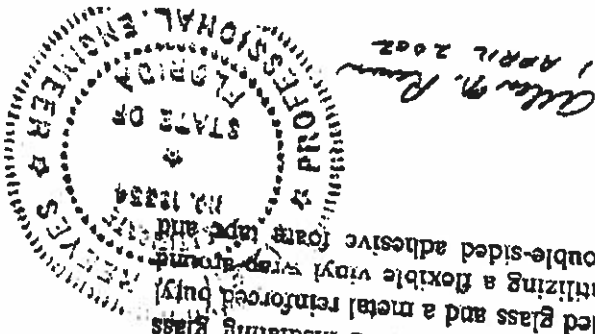
Daylight Opening Size: 3'-1-3/8" wide by 2'-9-1/2" high

Screen Size: 4'-0-1/4" wide by 2'-11-1/8" high

Finish: All aluminum was white.

Glazing Details: The active and fixed lites utilized 5/8" thick, sealed insulating glass constructed from two sheets of 1/8" thick, clear annealed glass and a metal reinforced butyl gasket. The fixed lite was interior glazed utilizing a flexible vinyl wrap around adhesive secured with PVC snap-in glazing beads.

130 Perry Court
York, PA 17402-9405
phone: 717.764.7700
fax: 717.764.4129
www.archtest.com



Test Specimen Description: (Continued)

Weatherstripping:

Description	Quantity	Location
0.230" high by 0.270" backed polypile with center fin	1 Row	Fixed meeting rail
0.250" high by 0.187" backed polypile with center fin	2 Rows	Active sash stiles
1/2" x 1/2" dust plug	4 Pieces	Active sash, top and bottom of stiles
1/4" foam-filled vinyl bulb seal	1 Row	Active sash, bottom rail

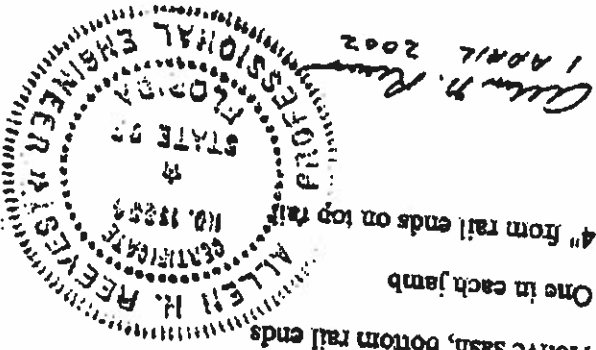
Frame Construction: The frame was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1" screws through the head and sill into each jamb screw boss. End caps were utilized on the ends of the fixed meeting rail and secured with two 1-1/4" screws per cap. Meeting rail was secured to the frame utilizing two 1-1/4" screws.

Sash Construction: The sash was constructed of extruded aluminum with coped, butted, and sealed corners fastened with two #8 x 1-1/2" screws through the rails into each jamb screw boss.

Screen Construction: The screen was constructed from roll-formed aluminum with keyed corners. The fiberglass mesh was secured with a flexible spline.

Hardware:

Description	Quantity	Location
Metal cam lock with keeper	2	Midspan, active meeting rail with keeper adjacent on fixed meeting rail
Plastic tilt latch	2	Active sash, meeting rail ends
Metal tilt pin	2	Active sash, bottom rail ends
Balance assembly	2	One in each jamb
Screen plunger	2	4" from rail ends on top rail



Test Specimen Description: (Continued)

Drainage: Sloped sill

Reinforcement: No reinforcement was utilized.

Installation: The test specimen was installed into a 2 x 8 #2 Spruce-Pine-Fir wood test buck with #8 x 1-5/8" drywall screws every 8" on center around the nail fin. Polyurethane was used as a sealant under the nail fin and around the exterior perimeter.

Test Results:

The results are tabulated as follows:

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.1	Operating Force	11 lbs	30 lbs max

Air Infiltration (ASTM E 283-91)
@ 1.57 psf (25 mph)

0.13 cfm/ft²
0.3 cfm/ft² max

Note #1: The tested specimen meets the performance levels specified in AAMA/NWDA 101/I.S. 2-97 for air infiltration.

Water Resistance (ASTM E 547-00)
(with and without screen)
WTP = 2.86 psf

No leakage

No leakage

2.1.4.1

Uniform Load Deflection (ASTM E 330-97)
(Measurements reported were taken on the meeting rail)

0.42"

0.43"

0.26" max.
0.26" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

2.1.4.2

Uniform Load Structural (ASTM E 330-97)
(Measurements reported were taken on the meeting rail)

0.02"

0.02"

0.18" max.
0.18" max.

@ 38.9 psf (positive)
@ 52.1 psf (negative)

(Loads were held for 10 seconds)

Allen H. Reeves
1 APRIL 2002



Test Specimen Description: (Continued)

Paragraph	Title of Test - Test Method	Results	Allowed
2.2.1.6.2	De-glazing Test (ASTM E 987) In operating direction at 70 lbs	Meeting rail Bottom rail	0.12"/25% 0.12"/25%
	In remaining direction at 50 lbs	Left stile Right stile	0.06"/12% 0.06"/12%
	Forced Entry Resistance (ASTM F 588-97)		0.50"/100% 0.50"/100%
	Type: A Grade: 10		
	Lock Manipulation Test	No entry	No entry
	Tests A1 through A5 Test A7	No entry No entry	No entry No entry
	Lock Manipulation Test	No entry	No entry

4.3

Water Resistance (ASTM E 547-00)
(with and without screen)
WTP = 6.00 psf

No leakage

No leakage

Uniform Load Deflection (ASTM E 330-97)
(Measurements reported were taken on the meeting rail)

0.47"

0.46"

0.26" max.
0.26" max.

*Exceeds L/175 for deflection, but passes all other test requirements.

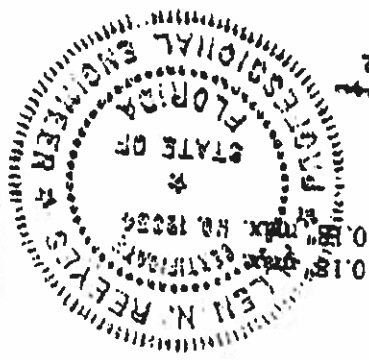
Uniform Load Structural (ASTM E 330-97)
(Measurements reported were taken on the meeting rail)

0.05"

0.05"

@ 67.5 psf (positive)
@ 70.8 psf (negative)

ALL N. REYES
STATE OF FLORIDA
1 APRIL 2002



Detailed drawings, representative samples of the test specimen, and a copy of this report will be retained by ATI for a period of four years. The above results were secured by using the above referenced specification. This report does not constitute certification of this product, which may only be granted by the certification program administrator.

For ARCHITECTURAL TESTING, INC:

Mark A. Hess
Mark A. Hess
Technician

MAH:mb
01-41134.01

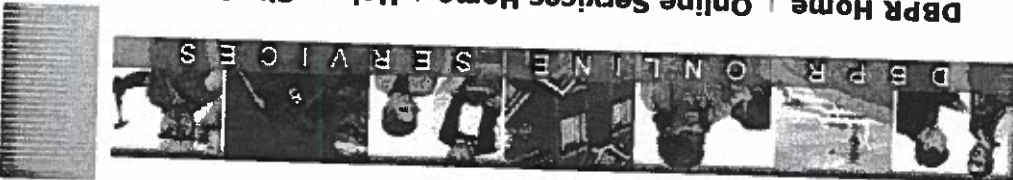
Allen N. Reeves
Allen N. Reeves, P.E.
Director - Engineering Services
1 APRIL 2002



Project Information for:		L134531		Builder:		Norton Homes		Date:		11/4/2005		Lot:		N/A		Subdivision:		8072 N. US Hwy 441		County or City:		Columbia County		Truss Page Count:		15	
Truss Design Load Information (UNO)		Design Program: Mittek 5.2 / 6.2		Building Code:		FBC2004		Roof (psf):		42		Wind Standard:		ASCE 7-98		Wind Speed (mph):		110		Note: See individual truss drawings for special loading conditions							
Building Designer, responsible for Structural Engineering: (See attached)		NORTON, JAMES H. RB 0031780		Address:		RT 28 BOX 388A		HIGH SPRINGS, FL. 32643		Designer:		84															
Truss Design Engineer: Thomas, E. Miller, P.E., 56877 - Byron K. Anderson, PE FL 60987		Company:		Structural Engineering and Inspections, Inc. EB 9196		Address:		16105 N. Florida Ave, Ste B, Lutz, FL 33549		Notes:																	
1. Truss Design Engineer is responsible for the individual trusses as components only.																											
2. Determination as to the suitability and use of these truss components for the structure is the responsibility of the Building Designer of Record, as defined in ANSIT/PI																											
3. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.																											
4. Trusses designed for vertical loads only, unless noted otherwise.																											

#	Truss ID	Dwg. #	Seal Date	#	Truss ID	Dwg. #	Seal Date
1	CJ1	1104051334	11/4/2005				
2	CJ3	1104051335	11/4/2005				
3	CJ5	1104051336	11/4/2005				
4	EJ7	1104051337	11/4/2005				
5	HJ9	1104051338	11/4/2005				
6	T01	1104051339	11/4/2005				
7	T01G	1104051340	11/4/2005				
8	T01G	1104051341	11/4/2005				
9	T01GA	1104051342	11/4/2005				
10	T02	1104051343	11/4/2005				
11	T02A	1104051344	11/4/2005				
12	T02G	1104051345	11/4/2005				
13	T03	1104051346	11/4/2005				
14	T04	1104051347	11/4/2005				
15	T05	1104051348	11/4/2005				

NOV 04 2005



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01:57:03 PM 10/6/2004

Licensee Details

Licensee Information

Name: **NORTON, JAMES H (Primary Name)**
Main Address: **NORTON HOME IMPROVEMENT COMPANY
INC (DBA Name)
RT 28 BOX 388A
LAKE CITY, Florida 32025
RT 28 BOX 388A
LAKE CITY, FL 32025
Columbia**
Lic. Location:
License Information
License Type: **Registered Building Contractor**
Rank: **RB0031780**
License Number: **Current, Active**
Status: **02/16/1978**
Expires: **08/31/2005**

Special Qualifications
Effective Date

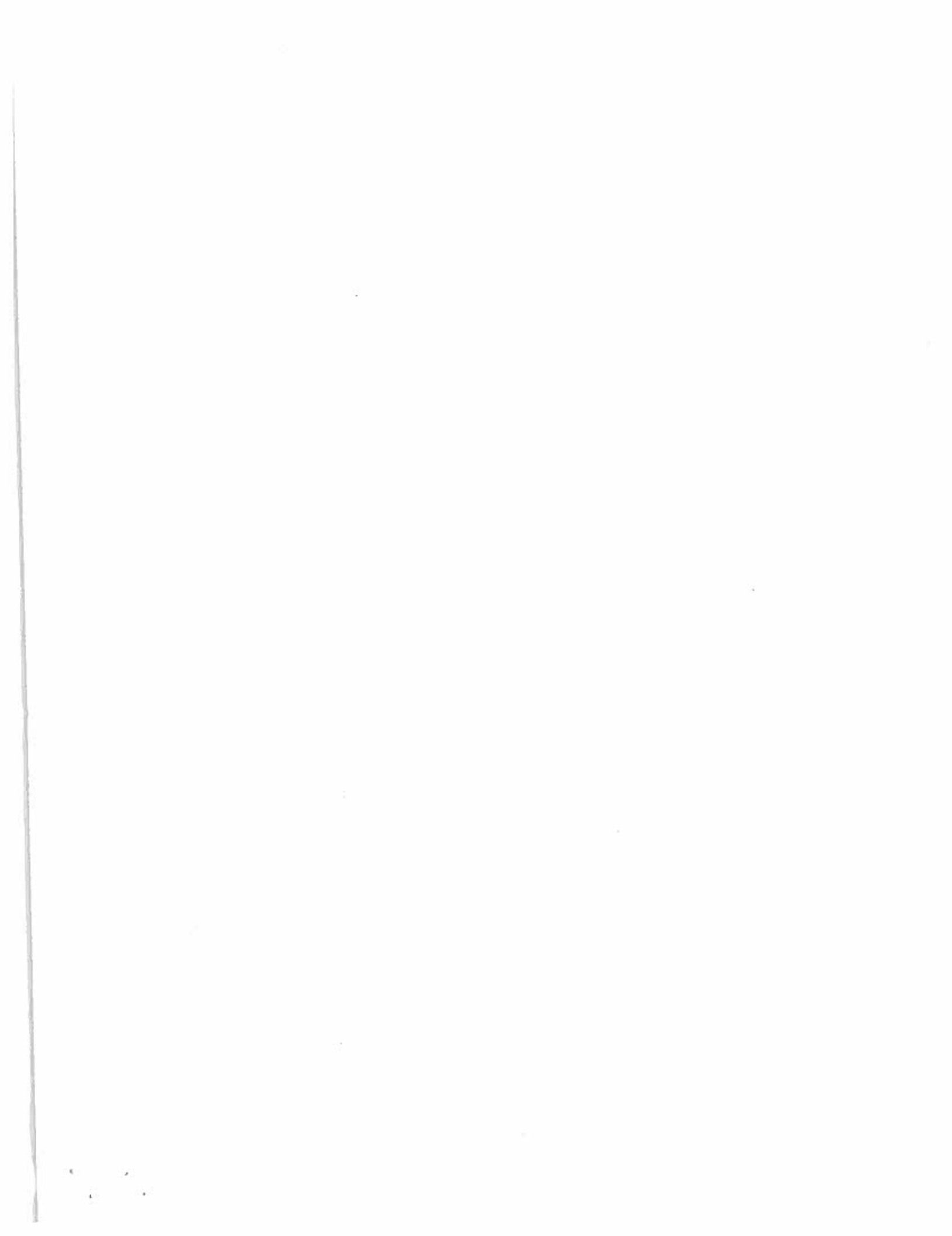
Bldg Code Core Course Credit

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LOAD CASE(S) Standard

1) Wind: ASCE 7-96: 110mph (3-second gust); $h=20ft$; $TCDL=4.2psf$; $BCDL=3.0psf$; Category II; Exp B: enclosed; MWFRS gable and zone and C-C Exterior(2) zone; porch left and right exposed; Lumber $DOL=1.60$ plate girth $DOL=1.60$. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 274 lb uplift at joint 2, 9 lb uplift at joint 4 and 85 lb uplift at joint 3.

NOTES

 $2 = 0.13$

JOINT STRESS INDEX

BOT CHORD 2-4=0/0

TOP CHORD 1-2=0/40, 2-3=58/54

REACTIONS (lb/size) 2=263/0-3-8, 4=15/Mechanical, 3=85/Mechanical
Max Horiz 2=73(load case 5)
Max Up/In 2=274(load case 5), 4=9(load case 3), 3=85(load case 1)
Max Grav 2=263(load case 1), 4=15(load case 1), 3=113(load case 5)

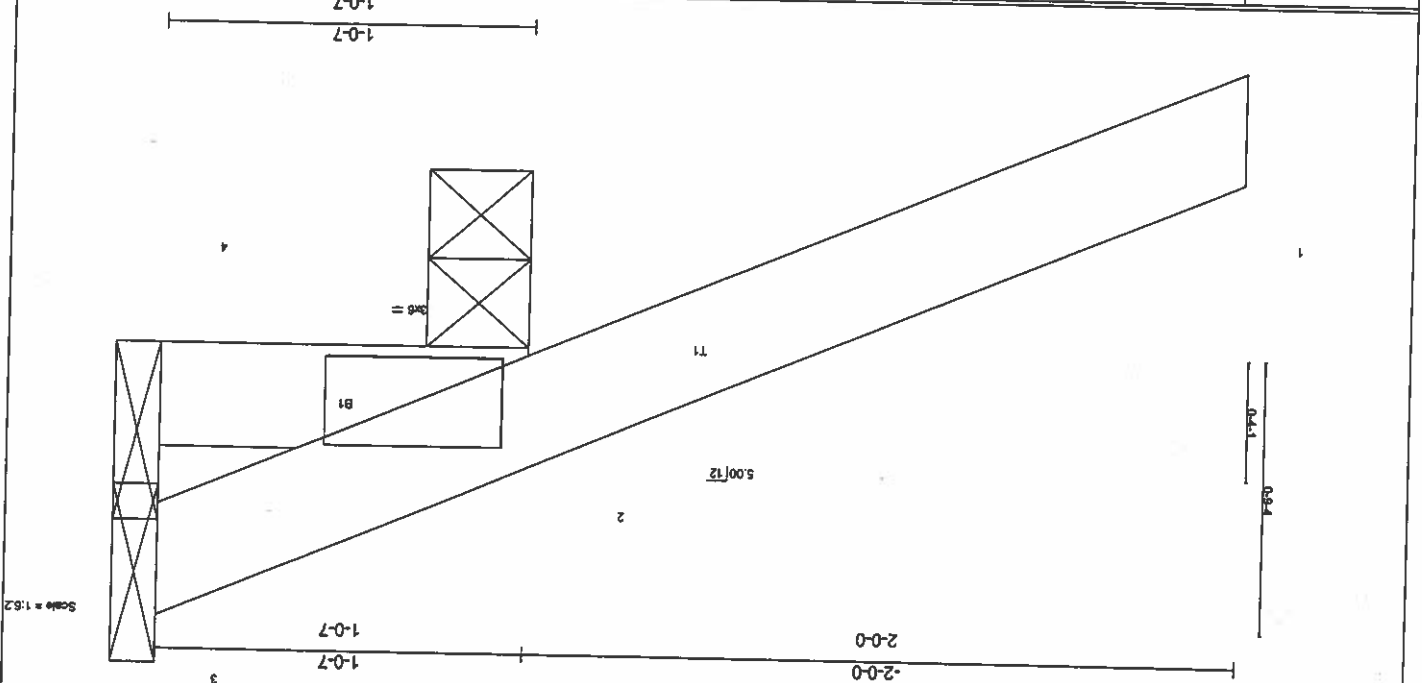
REACTIONS (lb/size) 2=263/0-3-6, 4=15/Mechanical, 3=85/Mechanical

BOT CHORD 2 X 4 SYP No.2

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 1-0-7 cc putlins.
Right ceiling directly applied or 10-0-0 cc bracing.

LOADING (psf)	20.0	SPACING	2-0-0	CS1	0.26	DEFL	in	3	0.00	Weight: 7 lb
TCL	20.0	Plates Increase	1.25	TC	0.26	in (loc)	2	2	>.998	PLATES
TCOL	7.0	Lumber Increase	1.25	BC	0.01	Vert(T/L)	2	2	>.999	MT20
SCOL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(T/L)	2	2	n/a	GRP
BCOL	5.0	Code FBC2004/TP12002								244/190





LOAD CASE(S) Standard

NOTES

(1) Wind: ASCE 7-98; 110 mph (-3-second gust); $h=20ft$; $TCDL=4.2psf$; $BCL=3.0psf$; Category II; Exp. B. enclosed; MWFRS gable end zone and C-C Extension(2) zone; porch left and right exposed; Lumber DOL=1.60 plate gfp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

(2) All bearings are assumed to be SYP No.2 crushing capacity of \$65.00 psi

(3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 3, 24 lb to uplift at joint 2 and 27 lb uplift at joint 4.

NOTES

 $2 = 0.12$

JOINT STRESS INDEX

1000

2-4=0/0 BOT CHORD

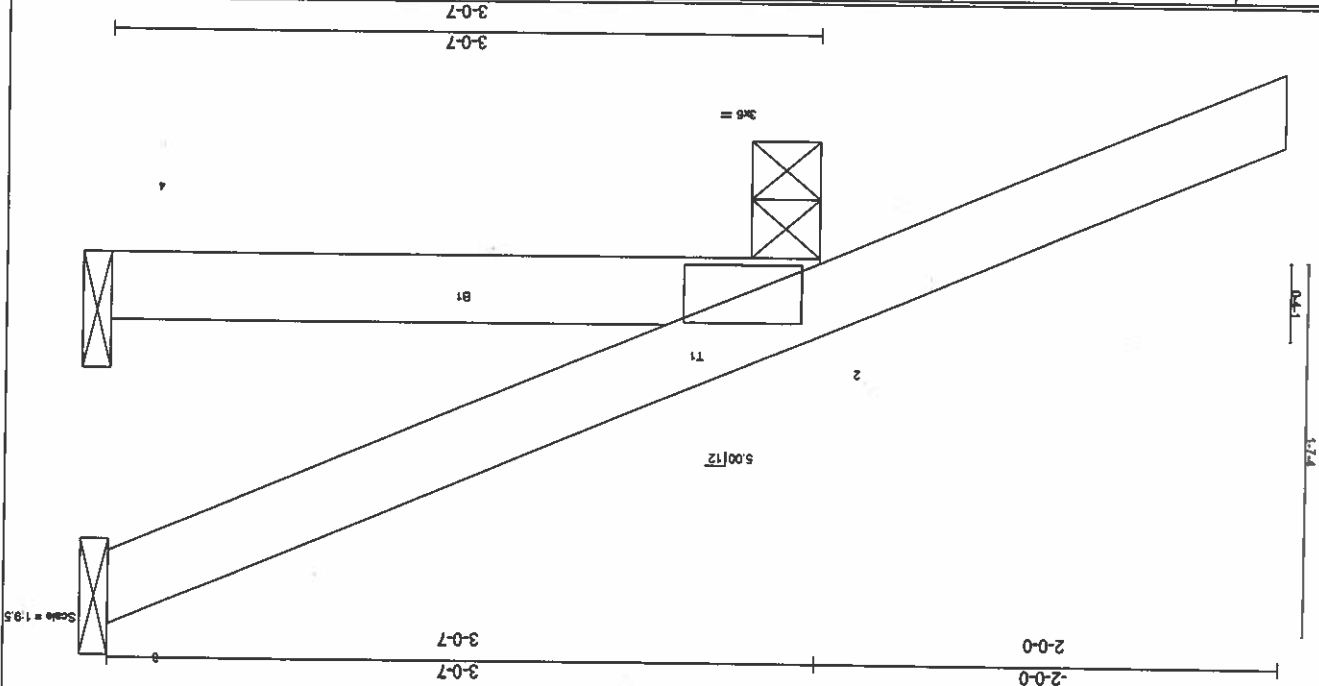
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=-49/7

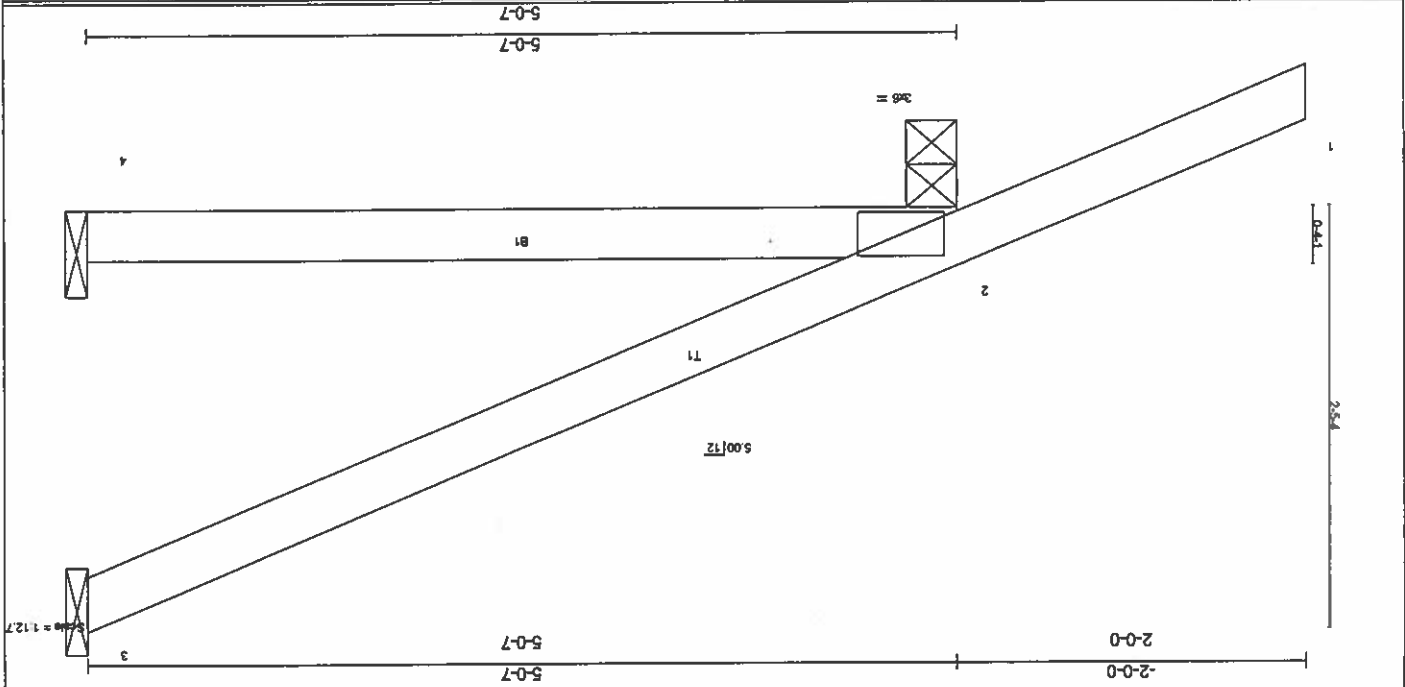
Max Horiz 2=110(load case 5)
Max Uplift3=25(load case 4), 2=-241(load case 5), 4=-27(load case 3)

REACTIONS (lb/size) 3=32/Mechanical, 2=279/0-3-8, 4=42/Mechanical

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 3-0-7 cc purlins.
Rigid ceiling directly applied or 10-0-0 cc bracing.

[illegible]

[illegible]

Job	Truss	Truss Type	MONO TRUSS	Qty	4	1	Dwg.#1104051336
L134531	CJS						Job Reference (optional)

6,200 s Jul 13 2005 Milltek Industries, Inc. Thu Nov 03 14:02:25 2005 Page 1

LOAD CASE(S) Standard

- NOTES
 (1) Wind: ASCE 7-98: 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C
 (2) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
 (3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 3 and 221 lb uplift at joint 2

JOINT STRESS INDEX

2 = 0.76

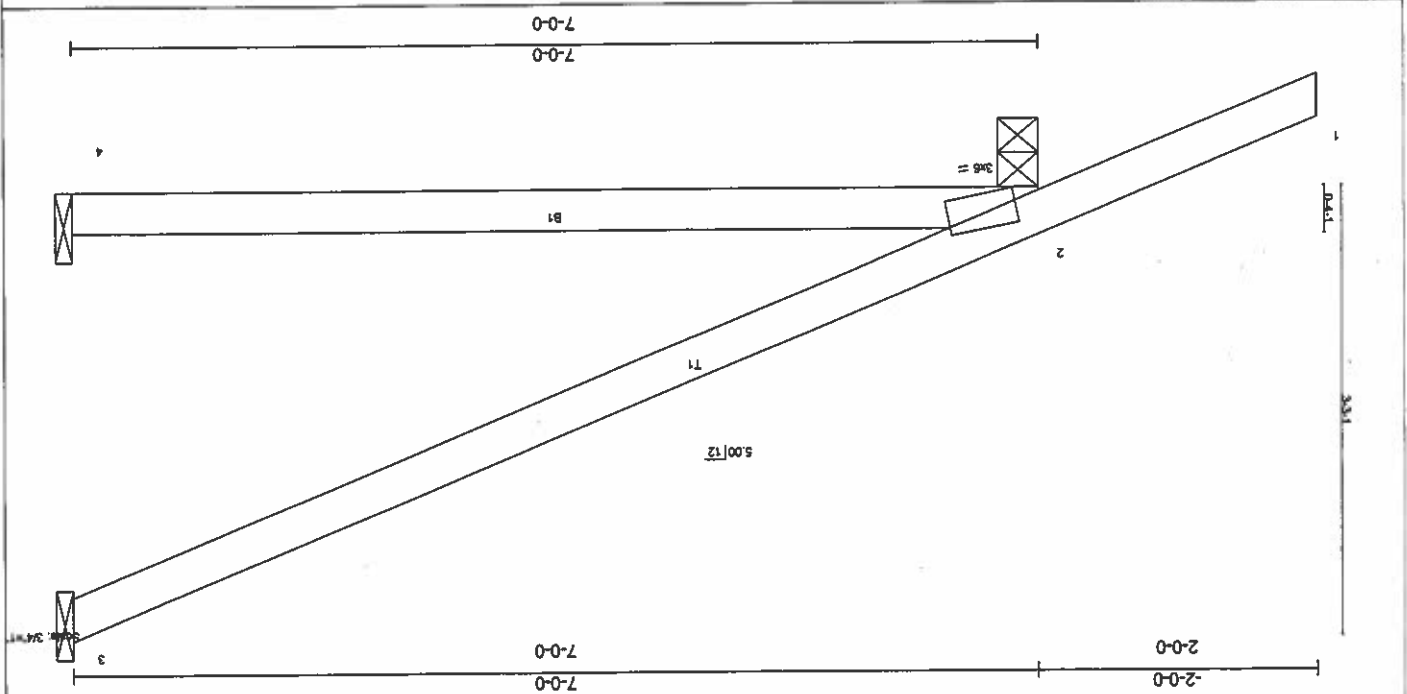
FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/40, 2-3=-115/50
 BOT CHORD 2-4=0/0

REACTIONS (lb/size) 3=163/Mechanical, 2=419/0-3-8, 4=104/Mechanical
 Max Horiz 2=166(load case 5)
 Max Uplift 3=-124(load case 5), 2=-221(load case 5)

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2
 BRACING TOP CHORD BOT CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

LOADING (psf)	20.0	SPACING	2-0-0	CSI	DEFL	in (loc)	Udfl	L/d	PLATES	GRP	Weight 25 lb
TCDL	7.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.12	2-4	MT20	244/190	
BCDL	5.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.20	2-4			
BCLL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3			
		Code FBC2004/TP12002		(Matrix)							

Plate Offsets (X Y): 2-0-2-0-Edge



6.2008 JUL 13 2005 MITTAK Industries, Inc. Thu Nov 03 14:02:26 2005 Page 1 Builders FirstSource, Lake City, FL 32055

LOAD CASE(S) Standard

at joint 10.

4) Beaming at joints: 10 Considered portion of girder moment at joint 2, 790 lb uplift at joint 14 and 350 lb uplift surface.

3) All bearings are assumed to be SYP No.2 crushing capacity of 565,00 psi

Extension(2) zone; porch left exposed; Lumber DOL=1.60 plate gmp DOL=1.00; THIS TRUSS IS DESIGNED FOR 0.90 MAXIMUM
reactions specified.
2) All bearings are assumed to be SYP No.2; crushing capacity of 565.00 psi

1) Unpublished root files have been considered for this category.
2) Wind: ASCE 7-98, 110 mph (3-second gust).
Extension(2) zone; porch left exposed; lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for C-C for members and forces, and for MWFRS for

NOTES

JOINT STRESS INDEX
2 = 0.89, 3 = 0.34, 4 = 0.72, 5 = 0.71, 6 = 0.63, 7 = 0.45, 8 = 0.70, 9 = 0.34, 10 = 0.78, 11 = 0.82, 12 = 0.74, 13 = 0.55 and 14 = 0.96

JOINT STRESS INDEX

BOT CHORD
 2-14=1724/62, 13-14=66/302, 12-13=146/66, 11-12=65/121, 10-11=74/93, 9-10=73/93, 8-9=73/93, 7-8=62/85, 6-7=62/85, 5-6=62/85, 4-5=62/85, 3-4=62/85, 2-3=62/85, 1-2=62/85, 0-1=62/85, 1016
 WEBS
 3-14=3322/34, 4-14=268/286, 5-14=2534/1217, 6-13=1957/66, 7-13=1957/66, 8-13=64/318, 9-12=672/1476, 10-12=672/1476, 11-12=672/1476, 12-12=672/1476, 13-12=672/1476, 14-12=672/1476, 15-12=672/1476, 16-12=672/1476, 17-12=672/1476, 18-12=672/1476, 19-12=672/1476, 20-12=672/1476, 21-12=672/1476, 22-12=672/1476, 23-12=672/1476, 24-12=672/1476, 25-12=672/1476, 26-12=672/1476, 27-12=672/1476, 28-12=672/1476, 29-12=672/1476, 30-12=672/1476, 31-12=672/1476, 32-12=672/1476, 33-12=672/1476, 34-12=672/1476, 35-12=672/1476, 36-12=672/1476, 37-12=672/1476, 38-12=672/1476, 39-12=672/1476, 40-12=672/1476, 41-12=672/1476, 42-12=672/1476, 43-12=672/1476, 44-12=672/1476, 45-12=672/1476, 46-12=672/1476, 47-12=672/1476, 48-12=672/1476, 49-12=672/1476, 50-12=672/1476, 51-12=672/1476, 52-12=672/1476, 53-12=672/1476, 54-12=672/1476, 55-12=672/1476, 56-12=672/1476, 57-12=672/1476, 58-12=672/1476, 59-12=672/1476, 60-12=672/1476, 61-12=672/1476, 62-12=672/1476, 63-12=672/1476, 64-12=672/1476, 65-12=672/1476, 66-12=672/1476, 67-12=672/1476, 68-12=672/1476, 69-12=672/1476, 70-12=672/1476, 71-12=672/1476, 72-12=672/1476, 73-12=672/1476, 74-12=672/1476, 75-12=672/1476, 76-12=672/1476, 77-12=672/1476, 78-12=672/1476, 79-12=672/1476, 80-12=672/1476, 81-12=672/1476, 82-12=672/1476, 83-12=672/1476, 84-12=672/1476, 85-12=672/1476, 86-12=672/1476, 87-12=672/1476, 88-12=672/1476, 89-12=672/1476, 90-12=672/1476, 91-12=672/1476, 92-12=672/1476, 93-12=672/1476, 94-12=672/1476, 95-12=672/1476, 96-12=672/1476, 97-12=672/1476, 98-12=672/1476, 99-12=672/1476, 100-12=672/1476, 101-12=672/1476, 102-12=672/1476, 103-12=672/1476, 104-12=672/1476, 105-12=672/1476, 106-12=672/1476, 107-12=672/1476, 108-12=672/1476, 109-12=672/1476, 110-12=672/1476, 111-12=672/1476, 112-12=672/1476, 113-12=672/1476, 114-12=672/1476, 115-12=672/1476, 116-12=672/1476, 117-12=672/1476, 118-12=672/1476, 119-12=672/1476, 120-12=672/1476, 121-12=672/1476, 122-12=672/1476, 123-12=672/1476, 124-12=672/1476, 125-12=672/1476, 126-12=672/1476, 127-12=672/1476, 128-12=672/1476, 129-12=672/1476, 130-12=672/1476, 131-12=672/1476, 132-12=672/1476, 133-12=672/1476, 134-12=672/1476, 135-12=672/1476, 136-12=672/1476, 137-12=672/1476, 138-12=672/1476, 139-12=672/1476, 140-12=672/1476, 141-12=672/1476, 142-12=672/1476, 143-12=672/1476, 144-12=672/1476, 145-12=672/1476, 146-12=672/1476, 147-12=672/1476, 148-12=672/1476, 149-12=672/1476, 150-12=672/1476, 151-12=672/1476, 152-12=672/1476, 153-12=672/1476, 154-12=672/1476, 155-12=672/1476, 156-12=672/1476, 157-12=672/1476, 158-12=672/1476, 159-12=672/1476, 160-12=672/1476, 161-12=672/1476, 162-12=672/1476, 163-12=672/1476, 164-12=672/1476, 165-12=672/1476, 166-12=672/1476, 167-12=672/1476, 168-12=672/1476, 169-12=672/1476, 170-12=672/1476, 171-12=672/1476, 172-12=672/1476, 173-12=672/1476, 174-12=672/1476, 175-12=672/1476, 176-12=672/1476, 177-12=672/1476, 178-12=672/1476, 179-12=672/1476, 180-12=672/1476, 181-12=672/1476, 182-12=672/1476, 183-12=672/1476, 184-12=672/1476, 185-12=672/1476, 186-12=672/1476, 187-12=672/1476, 188-12=672/1476, 189-12=672/1476, 190-12=672/1476, 191-12=672/1476, 192-12=672/1476, 193-12=672/1476, 194-12=672/1476, 195-12=672/1476, 196-12=672/1476, 197-12=672/1476, 198-12=672/1476, 199-12=672/1476, 200-12=672/1476, 201-12=672/1476, 202-12=672/1476, 203-12=672/1476, 204-12=672/1476, 205-12=672/1476, 206-12=672/1476, 207-12=672/1476, 208-12=672/1476, 209-12=672/1476, 210-12=672/1476, 211-12=672/1476, 212-12=672/1476, 213-12=672/1476, 214-12=672/1476, 215-12=672/1476, 216-12=672/1476, 217-12=672/1476, 218-12=672/1476, 219-12=672/1476, 220-12=672/1476, 221-12=672/1476, 222-12=672/1476, 223-12=672/1476, 224-12=672/1476, 225-12=672/1476, 226-12=672/1476, 227-12=672/1476, 228-12=672/1476, 229-12=672/1476, 230-12=672/1476, 231-12=672/1476, 232-12=672/1476, 233-12=672/1476, 234-12=672/1476, 235-12=672/1476, 236-12=672/1476, 237-12=672/1476, 238

TOP CHORD
2-14=-12/26/1695
9-10=-13/32/81
TOP FORCE
(b) - 1-2/0/40, 2-3=-63/71/410, 3-4=-85/0/17305, 4-5=-72/11/708, 5-6=-65/14/38, 6-7=-1908/1013, 7-8=-3074/1645, 8-9=-3738/1628, 9-10=-12/26/1695
11-12=-85/12/108, 10-11=-149/0/3072
12-13=-149/0/3072
13-14=1476, 12-13=-628/514, 7-11=-55

Max Horiz Z=209(road case 5), 14=-790(road case 5), 10=-360(road case 5)
Max Vert Z=-395(road case 5), 14=2452(road case 1), 10=1060(road case 1)
Max Gray Z=13(road case 5), 14=2452(road case 1), 10=1060(road case 1)

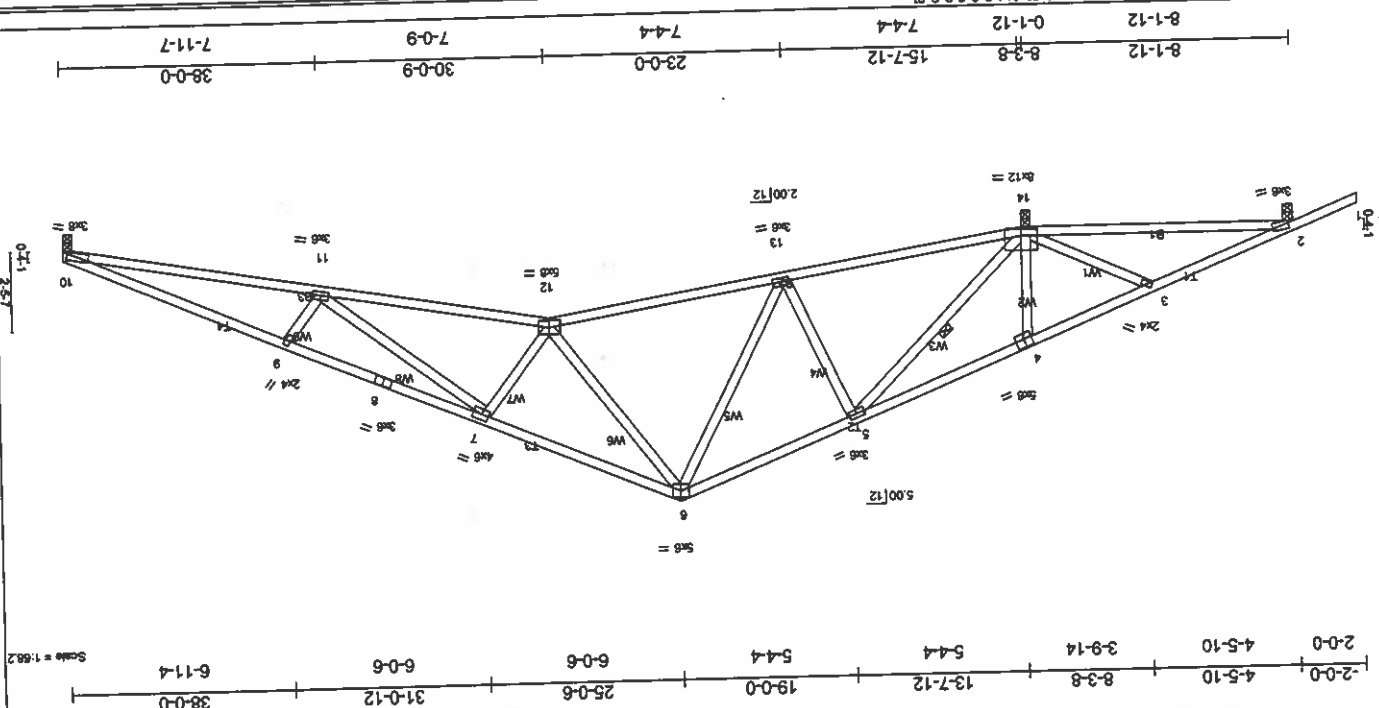
REACTIONS (lb/size) 2=229/0-3-8, 14=2452/0-3-8, 10=1060/0-3-8

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

Structural wood sheathing directly applied or 2-1/8-oc purlins.
Rigid ceiling directly applied or 4-11-5-oc bracing.
5-14
1 Row at midpt
WEBS
TOP CHORD
BOT CHORD

[illegible]

Plate Offsets (X,Y): [2:0-1-10:0-0-7],[4:0-3-0-0-3-0],[14:0-6-0-0-2-8]



Builders FirstSource, Lake City, FL 32055

L134531

SSRU

SPECIAL
Truss type

11

4

Dwg.#1104051339

6200 s Jul 13 2005 Mittek Industries, Inc. Thu Nov 03 14:02:27 2005 Page 1

NOTES

1) Unbraced roof live loads have been considered for this design.

2) Wind: ASCE 7-98: 11 mph (3-second gust); $h=20'$; $TCL=2$; $g=0.5$; $BCL=3$; $g=0.5$; $DOL=1.50$. This truss is designed for C-C for members and forces, and for MWFRS for reactions (2) zone.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mtak "Standard Gable End Detail".

4) All plates are 2x4 MT20 unless otherwise indicated.

5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 pc

6) Beams at joints(6) 15 considers parallel to grain value using NDS/TP 1 angle to grain formula. Building designer should verify capacity of bearing

7) Bearing at joint(6) 15 considers parallel to grain value using NDS/TP 1 angle to grain formula.

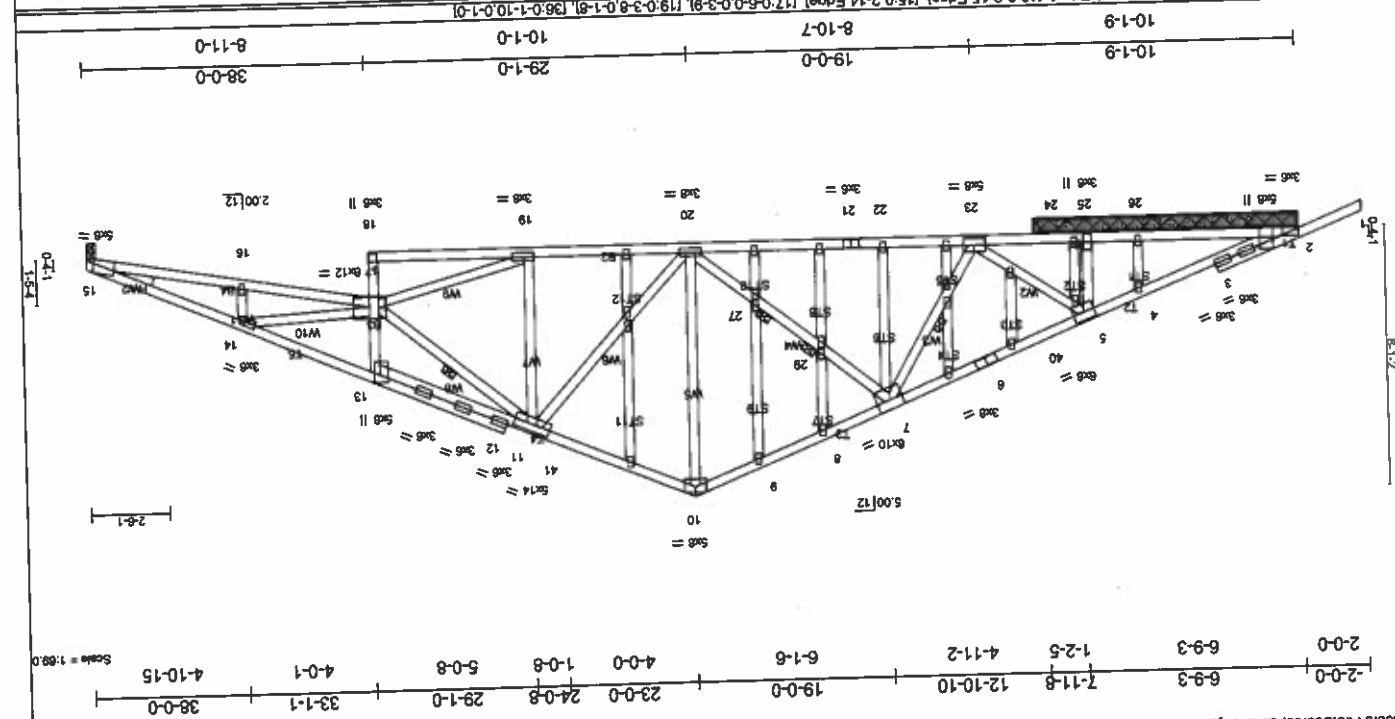
8) Provide mechanical connection (D) by others of truss to bearing plate capable of withstanding 256 to uplift at joint 25, 689 to uplift at joint 15, 387 to uplift at joint 24, 4 to uplift at joint 26 and 1454 to uplift at joint 25.

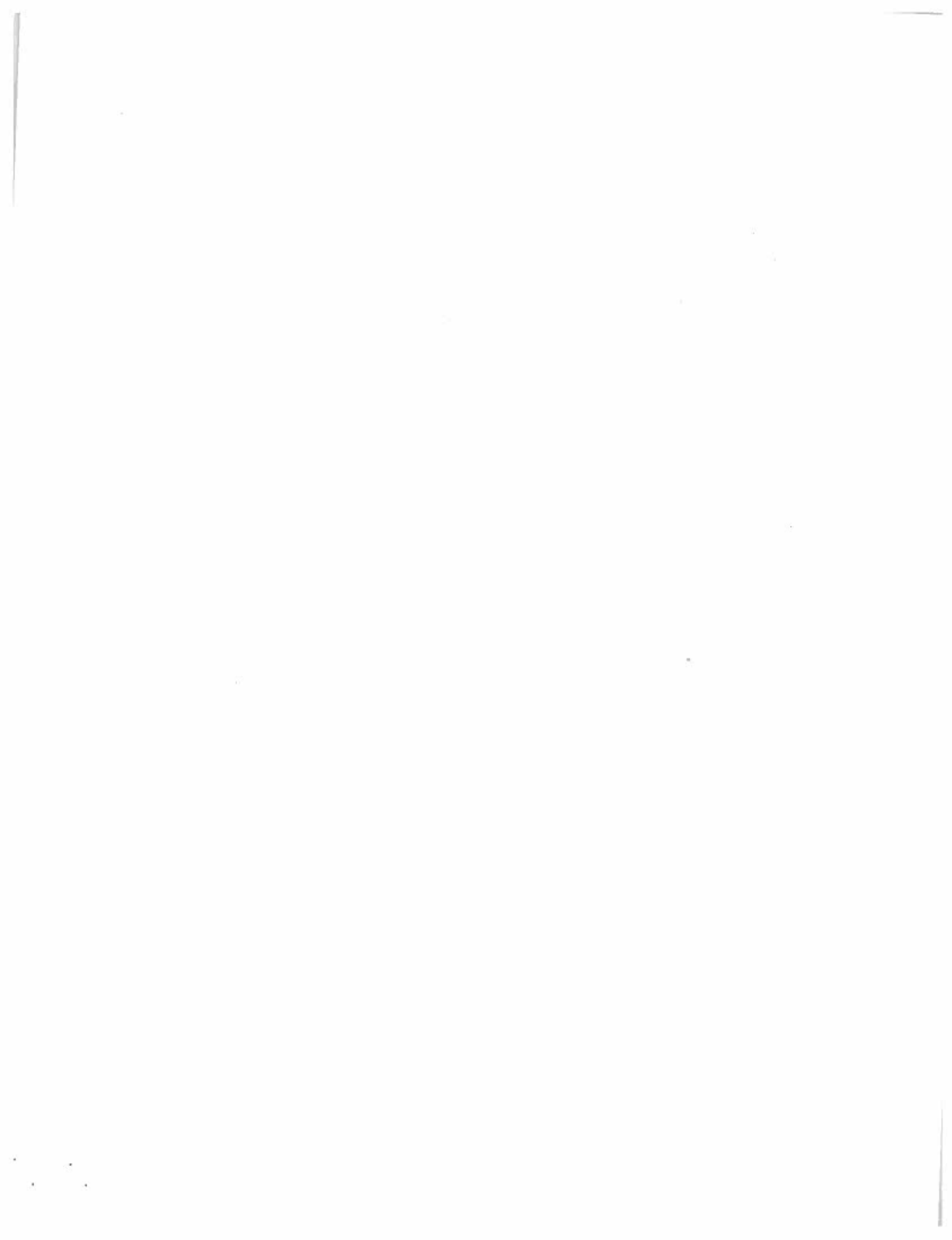
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

REACTIONS (lb/size)	Max Horiz 2=39 (load case 5)	Max Uprt2=258 (load case 8)	Max Grav 2=89 (load case 3), 15=689 (load case 5), 24=387 (load case 5), 25=4 (load case 4), 25=1454 (load case 1)
2=-139/8-3-8, 15=1498/0-3-8, 24=831/8-3-8, 25=2858/8-3-8			
FORCES (lb) - Maximum Compression/Maximum Tension			
1-2=-17/85, 2-3=-1953/2153, 3-4=-1981/2306, 4-5=-2023/2401, 5-6=-564/393, 6-7=-576/392, 7-8=-2010/1684			
8-9=-1897/664, 9-10=-1810/1644, 10-11=-1735/1629, 11-12=-1946/1677, 11-13=-4610/4171, 12-13=-4693/4168, 13-14=-4870/4221, 14-15=-5177/4577			
BOT CHORD	2-26=-2099/1818, 25-26=-2099/1818, 27-28=-2099/1818, 28-29=-2099/1818, 29-30=-2099/1818, 30-31=-2099/1818, 31-32=-2099/1818, 32-33=-2099/1818, 33-34=-2099/1818, 34-35=-2099/1818, 35-36=-2099/1818, 36-37=-2099/1818, 37-38=-2099/1818, 38-39=-2099/1818, 39-40=-2099/1818, 40-41=-2099/1818, 41-42=-2099/1818, 42-43=-2099/1818, 43-44=-2099/1818, 44-45=-2099/1818, 45-46=-2099/1818, 46-47=-2099/1818, 47-48=-2099/1818, 48-49=-2099/1818, 49-50=-2099/1818, 50-51=-2099/1818, 51-52=-2099/1818, 52-53=-2099/1818, 53-54=-2099/1818, 54-55=-2099/1818, 55-56=-2099/1818, 56-57=-2099/1818, 57-58=-2099/1818, 58-59=-2099/1818, 59-60=-2099/1818, 60-61=-2099/1818, 61-62=-2099/1818, 62-63=-2099/1818, 63-64=-2099/1818, 64-65=-2099/1818, 65-66=-2099/1818, 66-67=-2099/1818, 67-68=-2099/1818, 68-69=-2099/1818, 69-70=-2099/1818, 70-71=-2099/1818, 71-72=-2099/1818, 72-73=-2099/1818, 73-74=-2099/1818, 74-75=-2099/1818, 75-76=-2099/1818, 76-77=-2099/1818, 77-78=-2099/1818, 78-79=-2099/1818, 79-80=-2099/1818, 80-81=-2099/1818, 81-82=-2099/1818, 82-83=-2099/1818, 83-84=-2099/1818, 84-85=-2099/1818, 85-86=-2099/1818, 86-87=-2099/1818, 87-88=-2099/1818, 88-89=-2099/1818, 89-90=-2099/1818, 90-91=-2099/1818, 91-92=-2099/1818, 92-93=-2099/1818, 93-94=-2099/1818, 94-95=-2099/1818, 95-96=-2099/1818, 96-97=-2099/1818, 97-98=-2099/1818, 98-99=-2099/1818, 99-100=-2099/1818		
WEBS	1-17=-2402/22648, 14-17=-2093/48, 17-18=-2093/48, 18-19=-2093/48, 19-20=-2093/48, 20-21=-2093/48, 21-22=-2093/48, 22-23=-2093/48, 23-24=-2093/48, 24-25=-2093/48, 25-26=-2093/48, 26-27=-2093/48, 27-28=-2093/48, 28-29=-2093/48, 29-30=-2093/48, 30-31=-2093/48, 31-32=-2093/48, 32-33=-2093/48, 33-34=-2093/48, 34-35=-2093/48, 35-36=-2093/48, 36-37=-2093/48, 37-38=-2093/48, 38-39=-2093/48, 39-40=-2093/48, 40-41=-2093/48, 41-42=-2093/48, 42-43=-2093/48, 43-44=-2093/48, 44-45=-2093/48, 45-46=-2093/48, 46-47=-2093/48, 47-48=-2093/48, 48-49=-2093/48, 49-50=-2093/48, 50-51=-2093/48, 51-52=-2093/48, 52-53=-2093/48, 53-54=-2093/48, 54-55=-2093/48, 55-56=-2093/48, 56-57=-2093/48, 57-58=-2093/48, 58-59=-2093/48, 59-60=-2093/48, 60-61=-2093/48, 61-62=-2093/48, 62-63=-2093/48, 63-64=-2093/48, 64-65=-2093/48, 65-66=-2093/48, 66-67=-2093/48, 67-68=-2093/48, 68-69=-2093/48, 69-70=-2093/48, 70-71=-2093/48, 71-72=-2093/48, 72-73=-2093/48, 73-74=-2093/48, 74-75=-2093/48, 75-76=-2093/48, 76-77=-2093/48, 77-78=-2093/48, 78-79=-2093/48, 79-80=-2093/48, 80-81=-2093/48, 81-82=-2093/48, 82-83=-2093/48, 83-84=-2093/48, 84-85=-2093/48, 85-86=-2093/48, 86-87=-2093/48, 87-88=-2093/48, 88-89=-2093/48, 89-90=-2093/48, 90-91=-2093/48, 91-92=-2093/48, 92-93=-2093/48, 93-94=-2093/48, 94-95=-2093/48, 95-96=-2093/48, 96-97=-2093/48, 97-98=-2093/48, 98-99=-2093/48, 99-100=-2093/48		

ITEM	DESCRIPTION	QUANTITY	UNIT	PRICE	TOTAL
1	BRACING				
2	TOP CHORD				
3	BOT CHORD				
4	WEBS				
5	JOINTS				
6	1 Brace at midpt	1		27.29	27.29
7	1 Row at midpt	1		7.23	7.23
8	Rigid ceiling directly applied or 3-1-14 oc brading				
9	Structural wood sheathing directly applied or 2-4-0 oc purlins				
10	TOP CHORD 2 X 4 SYP No.2 "Except"	12	2 X 4 SYP No.1D		
11	BOT CHORD 2 X 4 SYP No.2 "Except"	12	2 X 4 SYP No.1D		
12	WEBS 2 X 4 SYP No.3	12	2 X 4 SYP No.3		
13	JOINTS 2 X 4 SYP No.3	12	2 X 4 SYP No.3		
14	RIGHT: 2 X 4 SYP No.3	12	2 X 4 SYP No.3		

LOADING (psf)	SPACING	2'-0"	CS	CS	Defl	In (loc)	Udefl	L/d	Weight: 269 lb
CL	Plating increase	1.25	TC	0.98	Ver(TL)	0.60	16-17	240	PLATES
CL	Lumber increase	1.25	BC	0.90	Ver(TL)	0.65	16-17	180	MT20
CL	Rep Stress Incr	NO	WB	0.99	Horz(TL)	0.23	15	180	GRP
CL	Code FBC2004/TP12002								244/190





10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
uplift at joint 12.

- Unbraced roof live loads have been considered for this design.
- Wind: ASCE 7-98: 11 mph (3-second gust); $w=20\text{ft}$; TCDF=4.2psf; BDCL=1.60 plate gfp DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for End Zone (2) zone; for wind exposed; Lumber DOL=1.60 plate gfp
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MITek "Standard Gable End Detail"
- All plates are MT20 unless otherwise indicated.
- Gable studs spaced at 2'-0" oc.
- All bearings are assumed to be SYP No.2 crushing capacity of 565.0 psi
- Bearing at joints: 12 considers parallel to grain value using ANS/PT 1 angle to grain formula. Building designer should verify capacity of bearing
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 169 lb uplift at joint 2, 1764 lb uplift at joint 16 and 611 lb

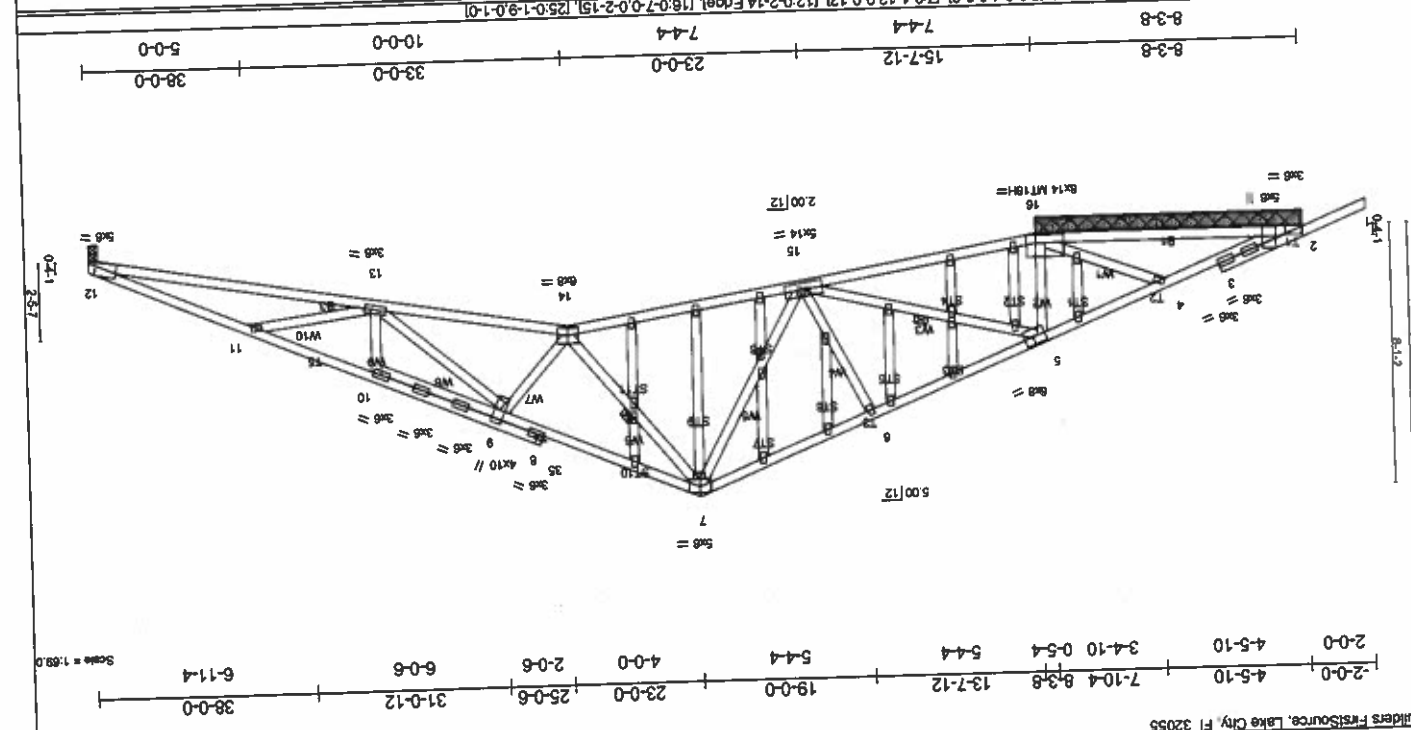
POINT STRESS INDEX

RNCES (b) - Maximum Compression/Maximum Tension
 1.2-1.1/2185, 2.3-1.555/1410, 3.4-1.582/1445, 4.5-1.774/2318, 5.6-1.860/1460, 6.7-1.509/1341, 7.35-1.315/82771, 8.35-3.206/27782.
 CP CHORD
 8.9-1.0/247732, 9.10-0.987/38755, 10.11-4.016/3590, 11.12-1.17-1.383/3932
 2.16-1.652/1537, 3.15-1.6-1.228/1905, 4.15-1.004/1685, 5.15-3.000/3878, 6.15-1.5-709/507, 7.15-837/520, 7.14-1.1836/2077, 8.14-4-808/625, 10.13-801/82.
 OT CHORD
 4.16-4.69/216, 5.16-3.11/21413, 6.15-336/418
 EBS
 9.13-801/376, 11.13-336/418

ACTIONS (b/size) 2=4/8-3-8, 16=3863/8-3-8, 12=1322/0-3-8
Max Horiz 2=203/load case 5)
Max Uplift=2-169/load case 5), 16=1784/load case 5), 12=611/load case 5)
Max Grav 2=201/load case 7), 16=3863/load case 1), 12=1322/load case 1)

CHORD 2 X SYP No.2 "Except"
T2 2 X A SYP No.1D
CHORD 2 X A SYP No.2 "Except"
T2 2 X A SYP No.1D
CHORD 2 X A SYP No.2 "Except"
B3 2 X A SYP No.1D
EBS 2 X A SYP No.3 "Except"
W3 2 X A SYP No.2
HERS 2 X A SYP No.3

BRACING
TOP CHORD
BOT CHORD
WEBS
1 Row at midpt
5-15, 7-14
Structural wood sheathing directly applied or 2-6-7 oc purlins.
Rigid ceiling directly applied or 3-2-10 oc bracing

[illegible]



LOAD CASE(S) Standard

NOTES

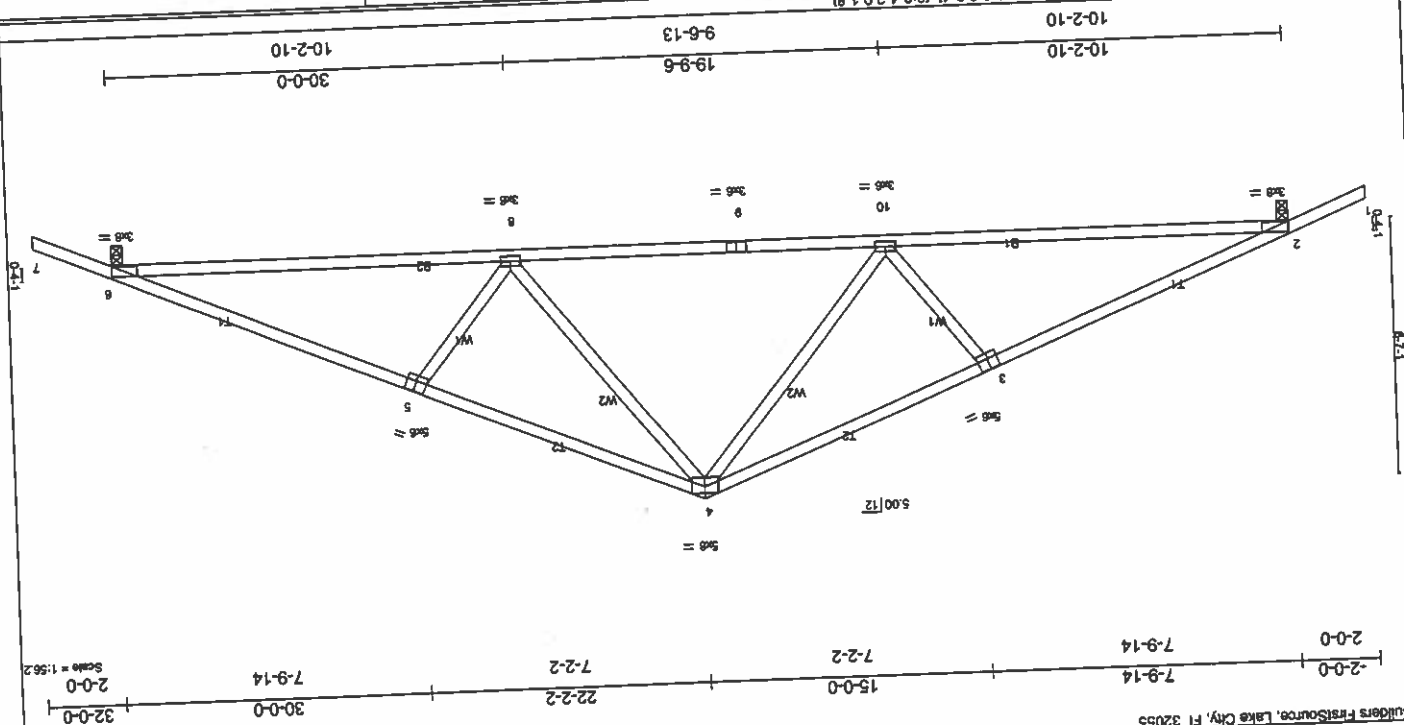
1) Unabridged roof live loads have been considered for this design.
2) Wind: ASCE 7-98: 11 mph (3-second gust); n=2; TCDF=1.2psf; BCLD=3.0psf; Category II; Exp B: enclosed; MWFRS gable and zone and C-L
Exterior(2) zone; Lumber DCL=1.60 plf gnp DCL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All beams are assumed to be SYP No.2 crushing capacity of 565.00 psi
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 508 lb uplift at joint 2 and 508 lb uplift at joint 6.

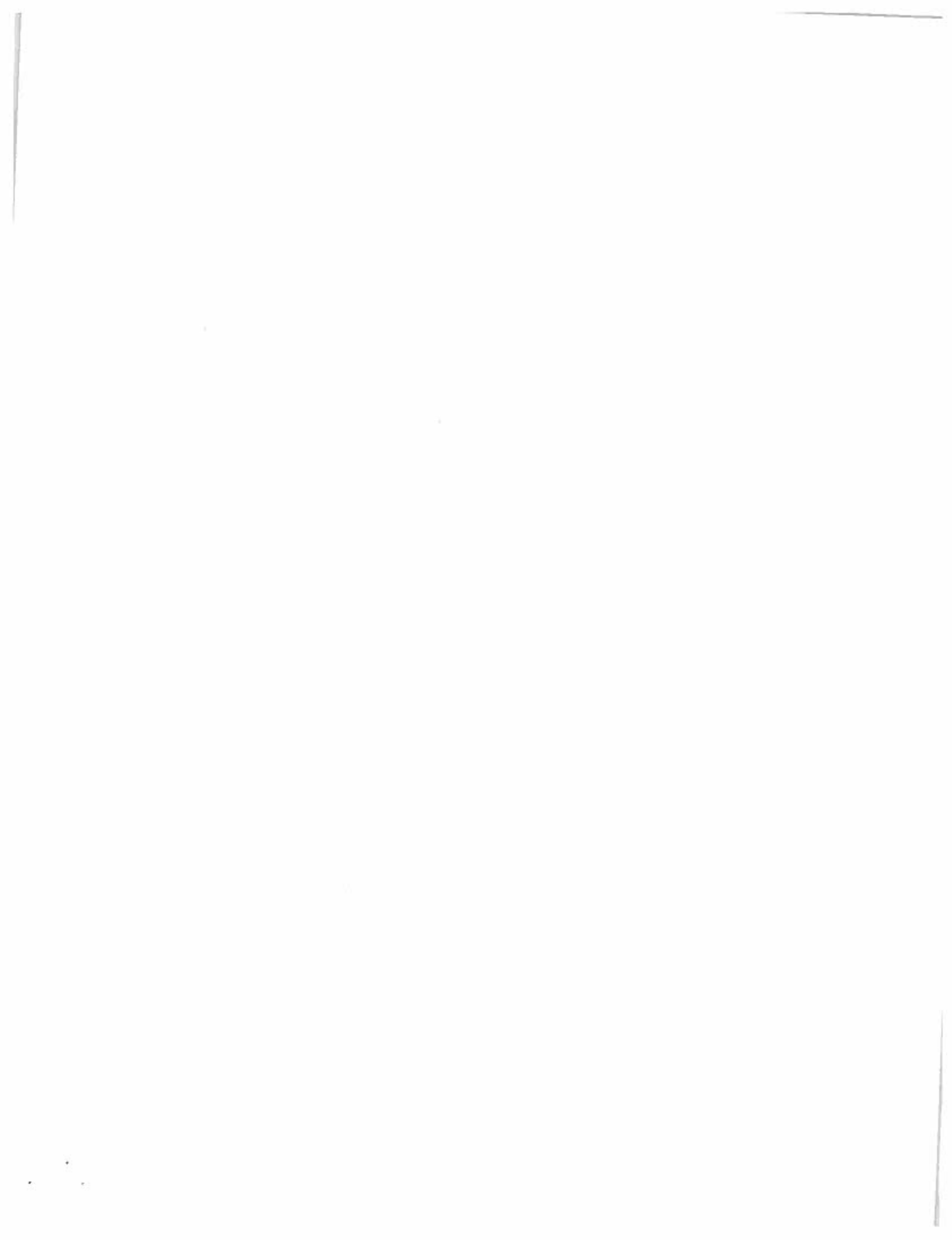
JOINT STRESS INDEX
2 = 0.71, 3 = 0.77, 4 = 0.56, 5 = 0.77, 6 = 0.71, 8 = 0.57, 9 = 0.63 and 10 = 0.57

FORCES (b) - Maximum compression/extension
1-2-0/40, 2-3-2=2443/1273, 3-4-2=1944/1187, 4-5-2=1944/1187, 5-6-2=2443/1273, 6-7-0=0/40
OPT CHORD
2-10=998/2198, 9-10=5371/477, 8-9=5371/477, 7-8=999/2198
VEBS
3-10=420/415, 4-10=339/808, 4-8=339/808, 5-8=420/415

EACIONS (b/size) 2=1364/0-3-8, 6=1364/0-3-8
Max Horiz 2=161(load case 6)
Max Uplift=-508(load case 5), 6=-508(load case 6)

NUMBER 2 X 4 SYP No.2
OP CHORD 2 X 4 SYP No.2
OT CHORD 2 X 4 SYP No.2
EBS 2 X 4 SYP No.3

[illegible]



LOAD CASE(S) Standard

NOTES

- (1) Unbalanced roof live loads have been considered for this design.
- (2) Wind: ASCE 7 zone: 1 (10mph [3-second gust])
Exposure: B
Risk Category II; Exp. B; enclosed MWFRS gable end zone and C-C members and forces, and for MWFRS for reactions specified.
- (3) All beams are assumed to be SYP No. 2, crushing capacity of 565.00 psi.
Bearing at joints: 2 compressors parallel to grain values using ANSI/VPI-1 angle to grain formula. Building designer should verify capacity of bearing surface.
- (4) Provide mechanical connection (by others) or truss to bearing plate capable of withstanding 508 lb uplift at joint 2 and 508 lb uplift at joint 6.

JOINT STRESS INDEX
 $2 = 0.76, 3 = 0.34, 4$

[illegible]

REACTIONS

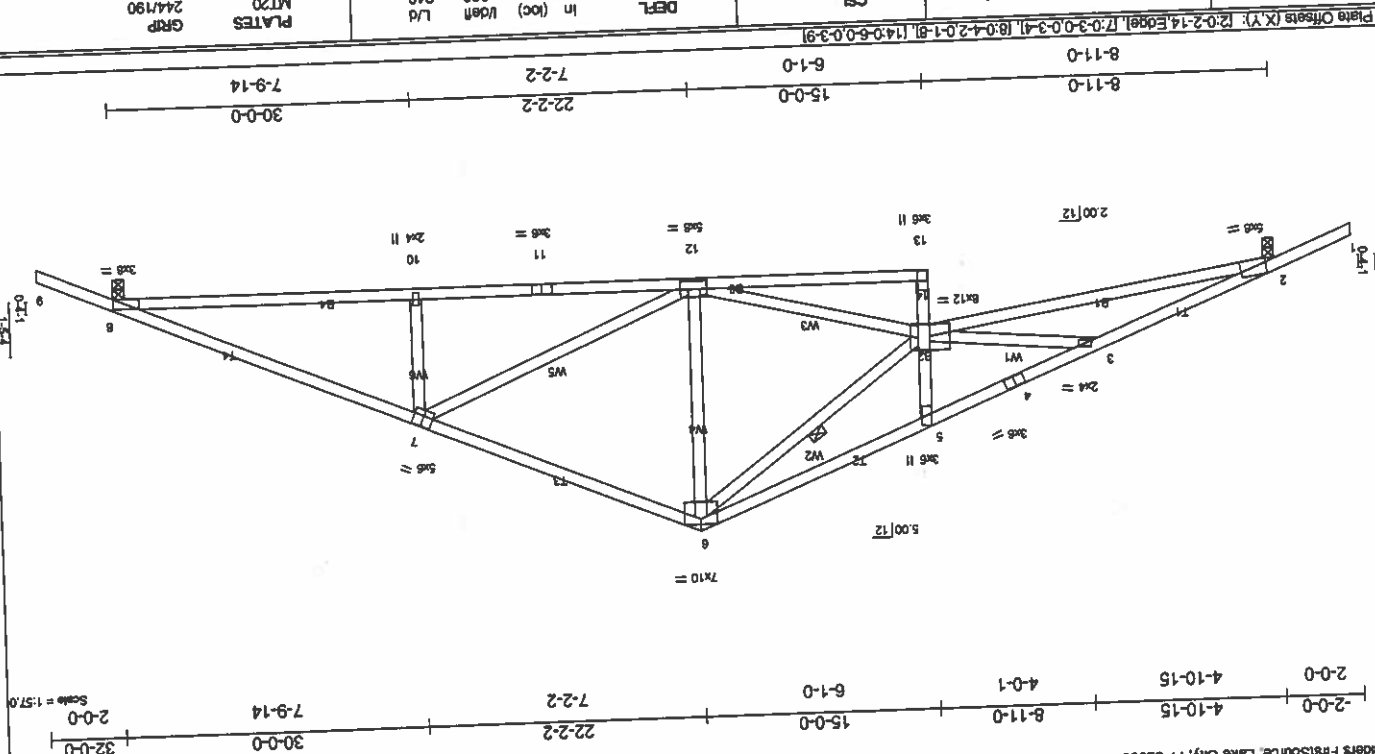
(lb/size) 2=1364/0-3-8, 8=1364/0-3-8
Max Horiz 2=161(load case 6)
Max Uplift=508(load case 5), 8=508(load case 6)

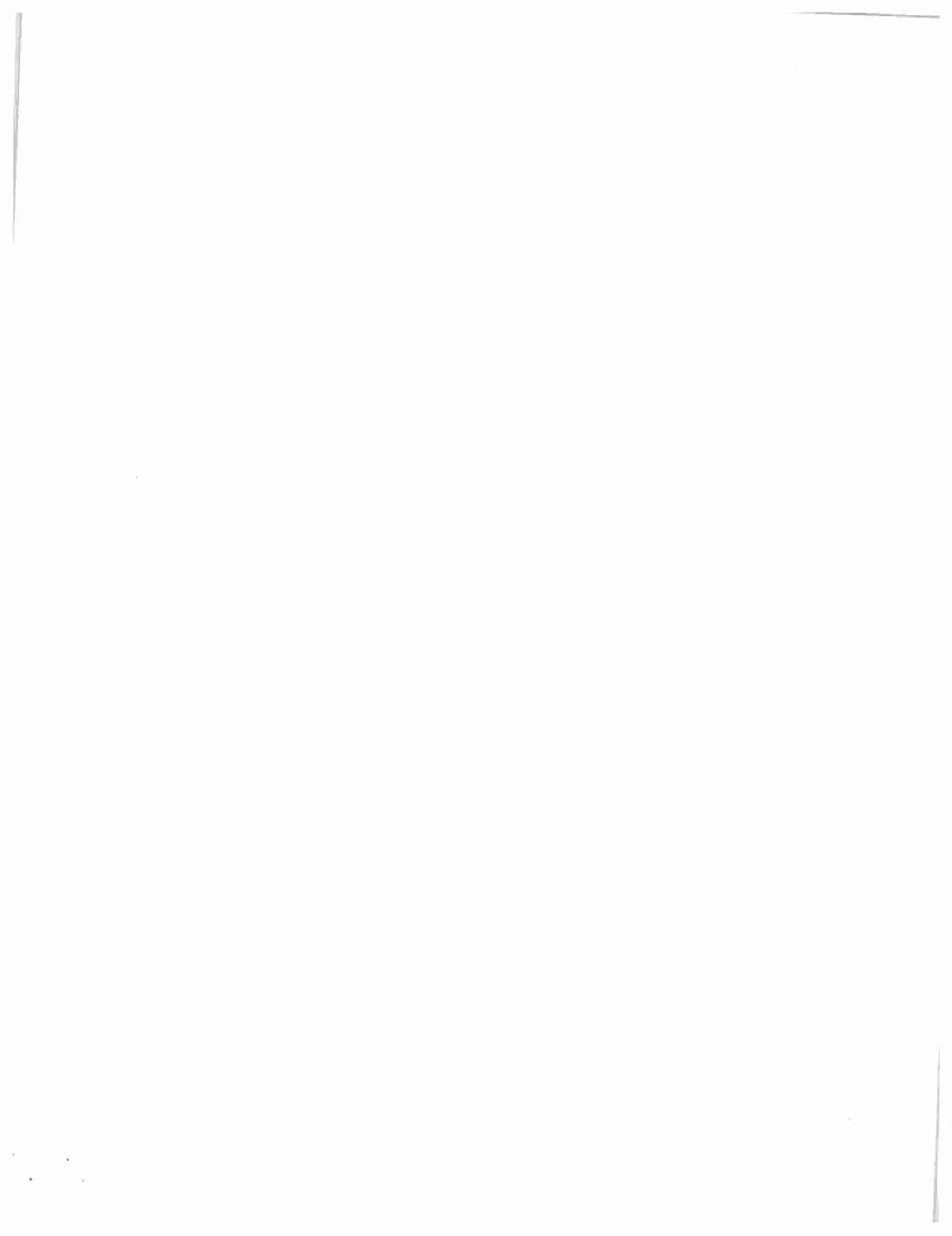
2 X 4 SYP NO.2
2 X 4 SYP NO.2 Except
B22 X 4 SYP NO.3
2 X 4 SYP NO.3
EBS

BRACING
TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 2-10-2 oc punting.
Rigid ceiling directly applied or 4-8-14 oc bracing.
1 Row at midpt
6-14

Plate Offsets (X, Y): 12.0-2.2-1.4, Edge, 17.0-3.0-0.3-1.1, 18.0-2.2-0.1-1.8, 19.0-0.0-0.0-0.0									
LOADING (psi)	20.0	Plates Increase	1.25	TC	0.48	DEPL	In	I/den	L/d
TC/L	7.0	Lumber Increase	1.25	BC	0.85	Vert(L)	-0.40	-2.14	>553
BC/L	10.0	Rep Stress Incr	YES	WB	0.90	Horz(L)	0.24	8	n/a
BC/L	5.0	Code FBC2004/TP12002		(Matrx)					
BRACING									
Structural wood sheathing directly applied or 2-10-2-oc pultrins									





LOAD CASES) Standard
1) Regular: Lumber increase=1.25, Plate increase=1.25
Uniform Loads (plf)
Vert 1.-9=-114(F=-80), 9.-17=-114(F=-60), 2.-16=-30

NOTES

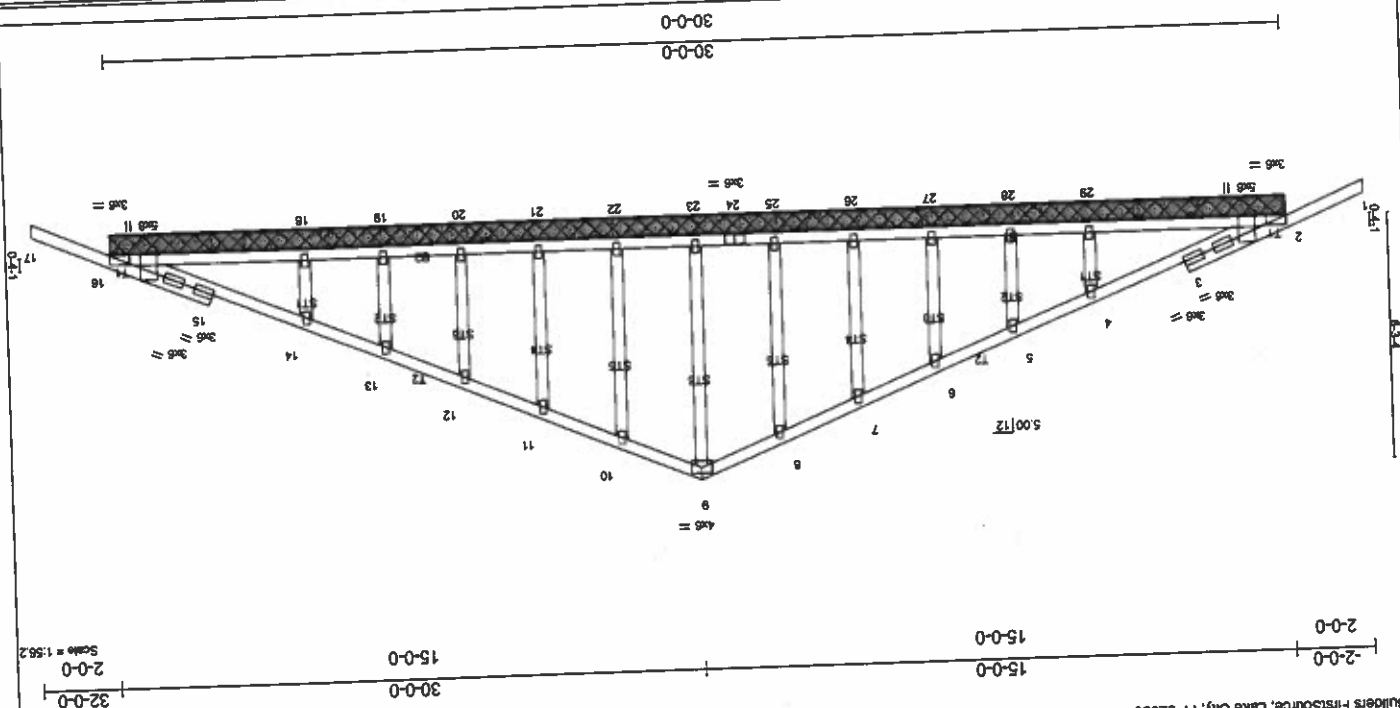
1) Unbraced roof line loads have been considered for this design.
2) Wind: ASCE 7-Lube: 11 mph (1-second gust); $z=20'$; TCDL = 4.2psf; BCDL = 3.0psf; Category II; Exp B; anisotropic; MWFRS and zone and C-C Exterior(2) zone; Lube: DOL = 1.6; flat gip DOL = 1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
3) All plates are 2x4 (M1720 unless otherwise indicated.
4) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MITrak Standard Cable End Detail
5) Cable requires continuous bottom chord bearing.
6) Cable studs spaced at 2'-0" oc.
7) All bearings are assumed to be SYP No. 2 crushing capacity of 565.00 psi
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 2, 292 lb uplift at joint 16, 21 lb uplift at joint 22, 125 lb uplift at joint 23, 123 lb uplift at joint 25, 124 lb uplift at joint 26, 133 lb uplift at joint 27, 90 lb uplift at joint 28, 214 lb uplift at joint 29, 120 lb uplift at joint 22, 125 lb uplift at joint 21, 133 lb uplift at the face of the truss and are noted as front (F) or back (B).
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

[illegible]

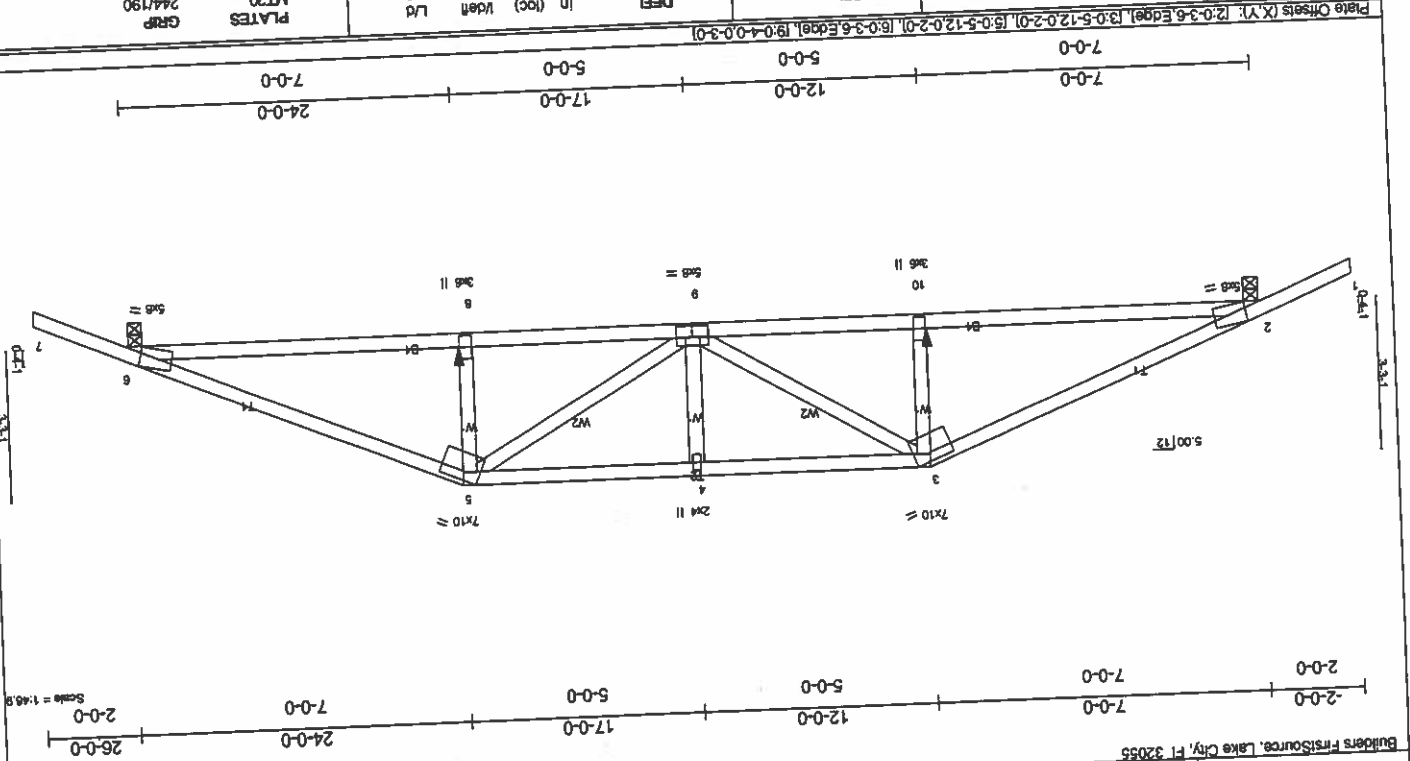
ITEM NO.	DESCRIPTION	QTY	UNIT	PRICE	TOTAL
1	LUMBER				
2	TOP CHORD 2 X 4 SYP No.2				
3	BOT CHORD 2 X 4 SYP No.2				
4	OTHERS 2 X 4 SYP No.3				
5	BRACING				
6	TOP CHORD				
7	BOT CHORD				
8	Structural wood sheathing directly applied or 10-0-oc purlins.				
9	Rigid ceiling directly applied or 6-0-oc bracing.				

Plate Offsets (X,Y) : [2;0;-3-8 Edge], [2;0;-1-1-1-1-3 Edge], [16;0;-3-8 Edge], [16;0;-1-1-1-3 Edge]					
LOADING (psi)	TCLL 20.0	TCDL 7.0	BCDL 5.0		
SPACING	2-0-0				
Plates Increase	1.25				
Lumber Increase	1.25				
Rep Stress Incr	NO				
Code FBC2004/TP12002					
CSJ	0.49				
TC	0.14				
BC	0.16				
(Matrix)					
DEFL	In (loc)				
Vert(LL)	-0.02	n/r	L/d		
Vert(TL)	-0.02	n/r			
Horz(TL)	0.01	n/a			
Weight: 162 lb					
PLATES	MT20				
GRP	244/190				

Plate Offsets (X,Y): [2:0-3:8,Edge], [2:0-1:13,Edge], [16:0-3:8,Edge], [16:0-1:13,Edge]



34531	102G	COMMON	Job Reference (optional)	6,200 S. J. 13 2005 Mitek Industries, Inc. Thu Nov 03 14:02:32 2005 Page 1
D	Trust	Trust type	1	
			2	
			Qty	
			Ply	
			10.0	
				DWG.#1104051345



LOADING (psf)	SPACING	Code FBC2004/TP12002	Rep Stress Incr	Lumber Increase	Plates Increase	CSF	TC	BC	WB	(Mathx)	BRACING	Structural wood sheathing directly applied or 2-5-9 oc purlins.
5.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	TOP CHORD	3-5 oc bracing
7.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
10.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
12.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
15.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
20.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
25.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
30.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
35.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
40.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
45.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
50.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
55.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
60.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
65.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
70.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
75.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
80.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
85.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
90.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
95.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
100.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
105.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
110.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
115.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
120.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
125.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
130.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
135.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
140.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
145.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
150.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
155.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
160.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
165.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
170.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
175.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
180.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
185.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
190.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
195.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
200.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
205.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
210.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
215.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
220.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
225.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
230.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
235.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
240.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
245.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
250.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
255.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
260.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
265.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
270.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
275.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
280.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
285.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
290.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
295.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
300.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
305.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
310.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
315.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
320.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
325.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
330.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
335.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
340.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
345.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
350.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
355.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
360.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
365.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
370.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
375.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
380.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
385.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
390.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
395.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
400.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
405.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
410.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
415.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
420.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
425.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
430.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
435.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
440.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
445.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
450.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
455.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
460.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
465.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
470.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
475.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
480.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
485.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
490.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
495.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
500.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
505.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
510.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
515.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
520.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32	(Mathx)	BRACING	3-5 oc bracing
525.0	BCDL	10.0	NO	1.25	0.32	0.70	0.32	0.32	0.32			

TOP CHORD 2 X 4 SYP No.2	2 X 4 SYP No.3	WEBS
BOT CHORD 2 X 4 SYP No.2		
BOT CHORD 2 X 4 SYP No.3		

REACTIONS (lb/size) 2=2140/0-3-8, 6=2140/0-3-8
Max Horiz 2=95/(load case 4)
Max Uplift 2=-1167/(load case 2), 6=-1167/(load case 3)

FORCES (b) - Maximum Compression/Maximum Tension
1-2=0/40, 2-3=-45/22/2370, 3-4=-48/76/2657, 4-5=-48/76/2657, 5-6=-45/22/2370, 6-7=0/40
TOP CHORD
1-2=0/40, 2-3=-45/22/2370, 3-4=-48/76/2657, 4-5=-48/76/2657, 5-6=-45/22/2370, 6-7=0/40
BOT CHORD
1-2=2105/4082, 2-10=-2123/44132, 3-9=-4933/990, 4-8=-602/4068, 5-9=-492/990, 5-8=-371/821
WEBS
3-10=-371/821, 3-9=-4933/990, 4-8=-602/4068, 5-9=-492/990, 5-8=-371/821

JOINT STRESS INDEX
2 = 0.89, 3 = 0.97, 4 = 0.34, 5 = 0.97, 6 = 0.89, 8 = 0.27, 9 = 0.91 and 10 = 0.27

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-98: 110 mph (3-second gust); h=20ft; TCFL=4.2psf; BCLD=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; porch left and right exposed; Lumber DOL=1.60 plate gfp DOL=1.60.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2, crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1167 lb uplift at joint 2 and 1167 lb uplift at joint 6.
- 6) Girders carries hip end with 7'-0" end seaboard.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 539 lb down and 255 lb up at 17'-0", and 539 lb down and 255 lb up at 7'-0" on bottom chord. The design/sufficiency of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) Standard, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
Plate Increase=1.25
1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-5=-54, 5-7=-54, 7-10=-30, 8-10=-30, 9-10=-30, 10-12=-30
Concentrated Loads (lb)
Vert: 10=-539(F) 8=-539(F)

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-98, 110mph (3-second gust); h=20ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 700 lb uplift at joint 2 and 700 lb uplift at joint 7.

NOTES

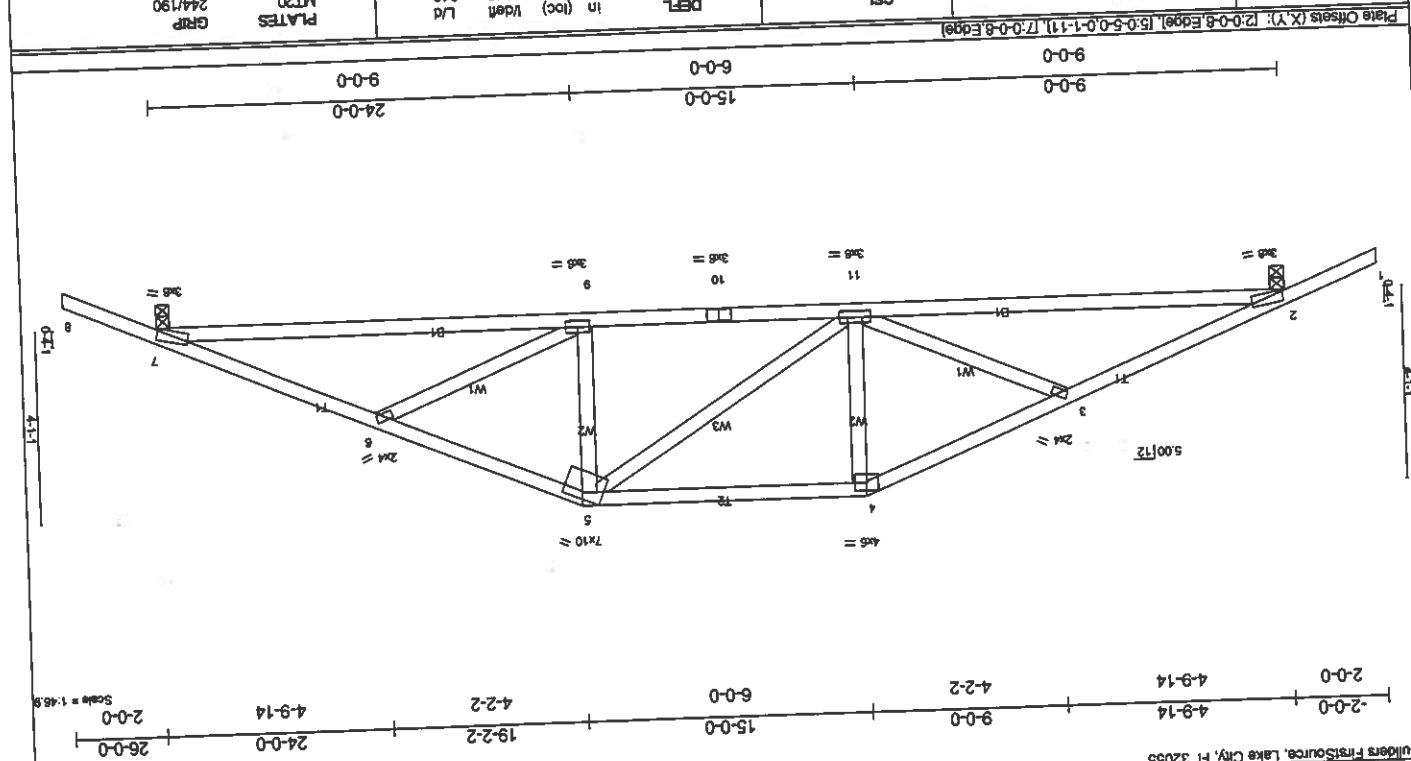
JOINT STRESS INDEX
2 = 0.83, 3 = 0.34, 4 = 0.79, 5 = 0.68, 6 = 0.34, 7 = 0.83, 9 = 0.35, 10 = 0.54 and 11 = 0.57

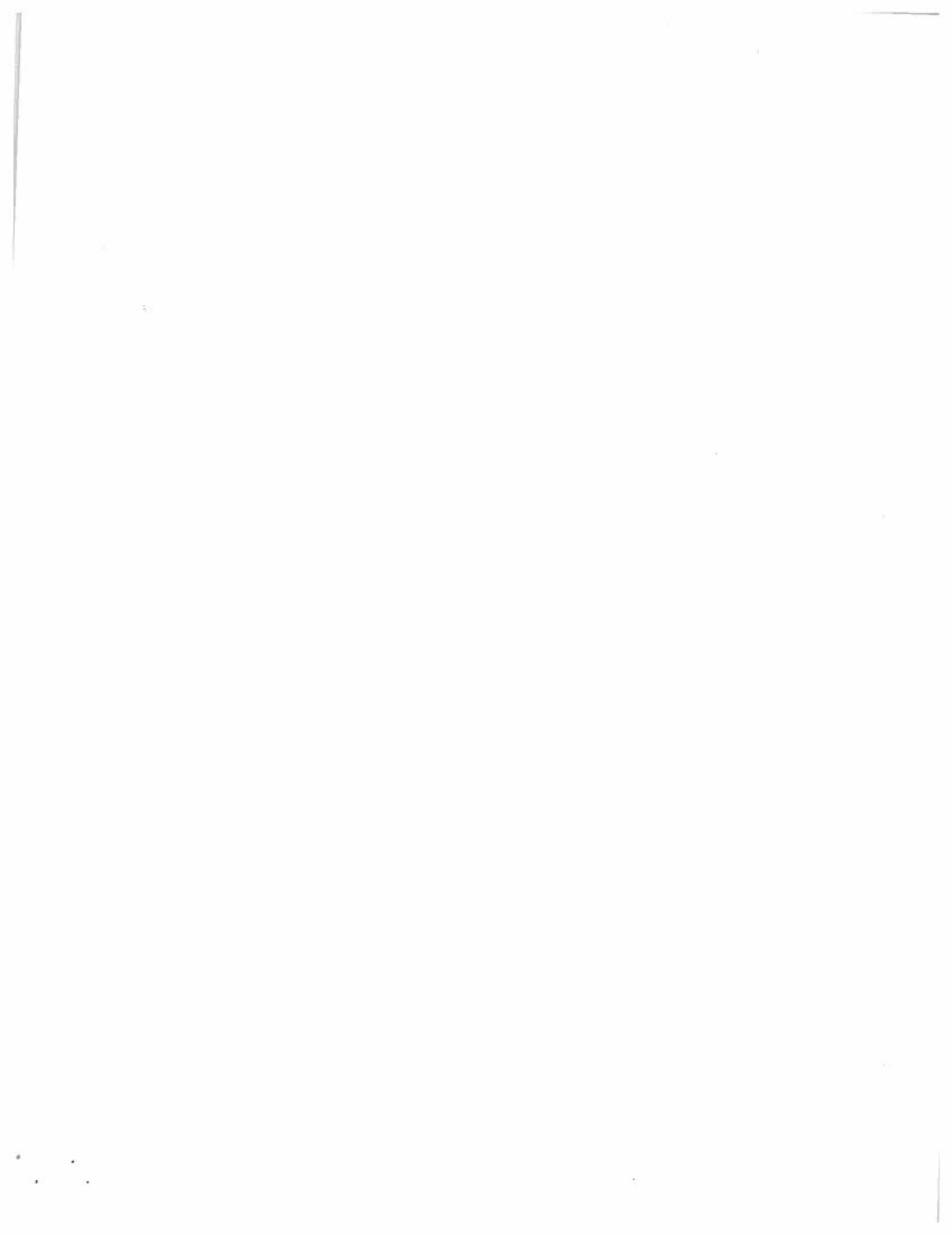
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-11=1738/1740, 10-11=1443/1501, 9-10=1443/1501, 7-8=1739/1740
BOT CHORD 2-11=276/342, 4-11=468/329, 5-11=123/125, 5-9=468/329, 6-9=276/342

REACTIONS (lb/size) 2=1112/0-3-8, 7=1112/0-3-8
Max Horiz 2=-112/0load case 6)
Max Uplift 2=700/0load case 5), 7=700/0load case 6)

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3
BRACING
Structural wood sheathing directly applied or 4-2-15 oc purlins.
Rigid ceiling directly applied or 4-1 oc bracing.

LOADING (psf)	SPACING	CS	DEFL	in (loc)	l/defl	L/d
TCDL 20.0	2-0-0	TC 0.46	Vert(L) 0.44	7-9 > 643	240	
BCDL 10.0	2-0-0	BC 0.53	Horz(TL) 0.36	7-9 > 794	180	
BCLL 5.0	2-0-0	WB 0.12	Vert(TL) 0.07	7 n/a	n/a	
Code FBC2004/TP12002						
Rep Stress Incr YES						
Lumber Increase 1.25						
Plates Increase 1.25						
Weight: 114 lb						
PLATES MT20						
GRIP 244/190						





LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-98; 110mph (3-second gust); h=20ft; TCFL=4.2psf; BCFL=3.0psf; ECFL=1.80. This truss is designed for C-C for members and zone and C-C for reactions.
- 3) Provide adequate drainage to prevent water ponding.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 720 lb uplift at joint 2 and 720 lb uplift at joint 7.

NOTES

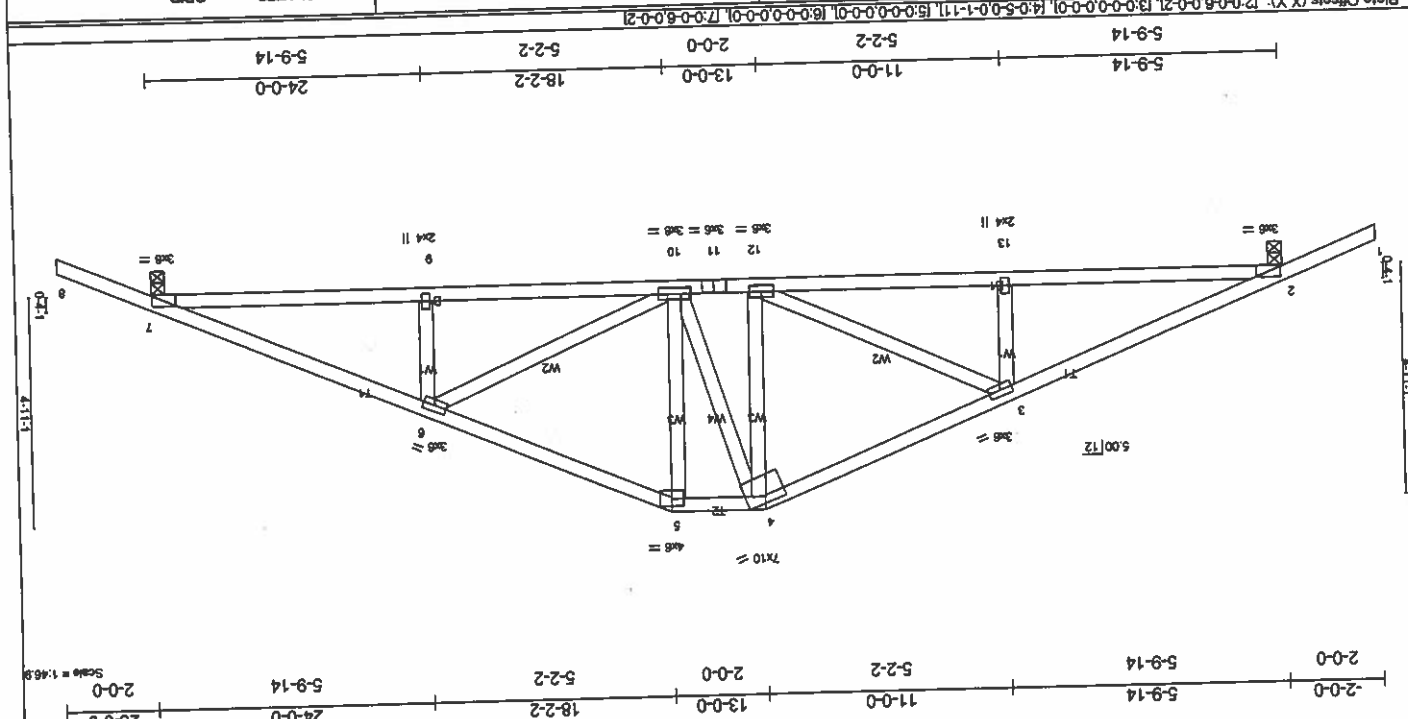
JOINT STRESS INDEX
2 = 0.76, 3 = 0.39, 4 = 0.60, 5 = 0.71, 6 = 0.39, 7 = 0.76, 9 = 0.34, 10 = 0.49, 12 = 0.35 and 13 = 0.34

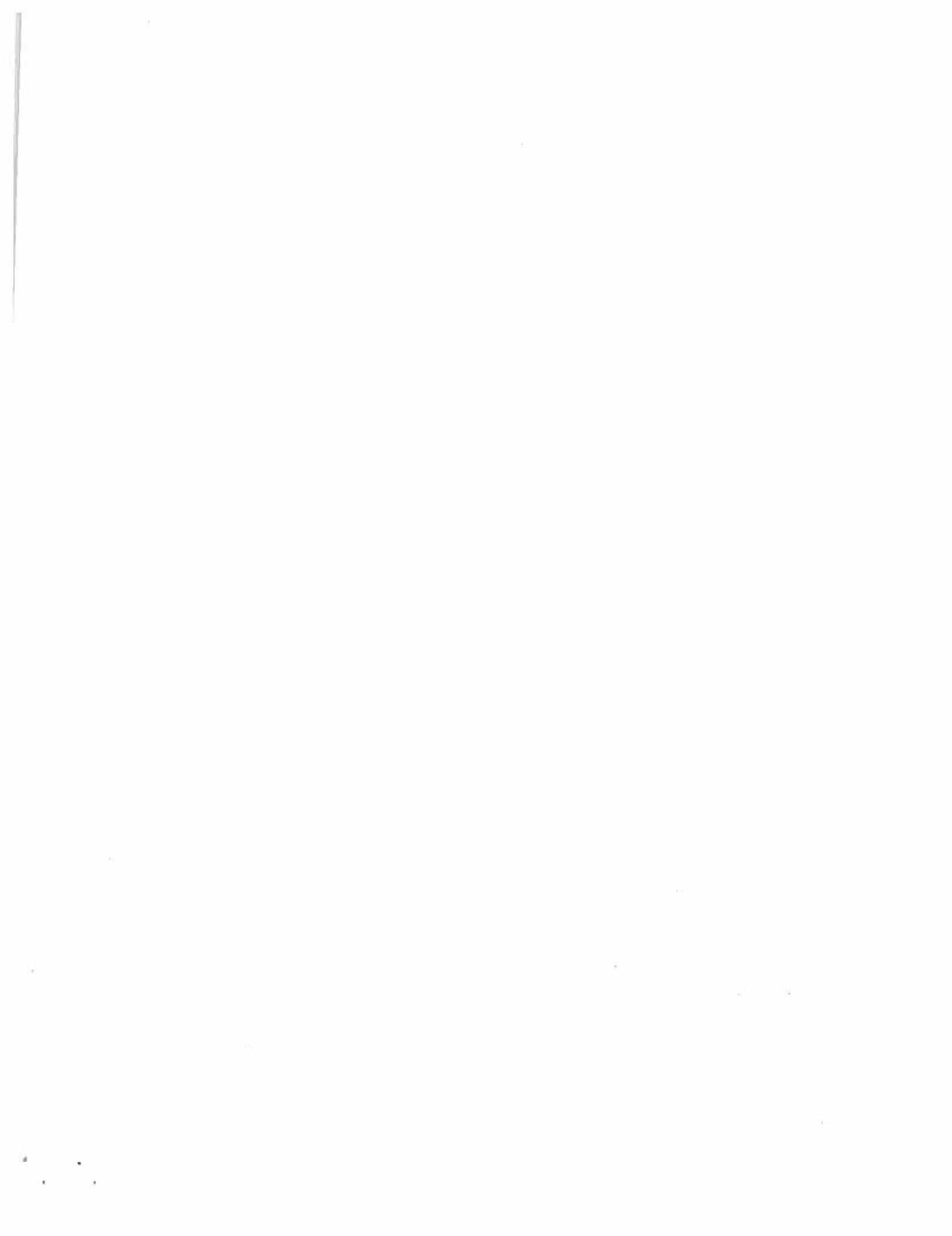
FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/40, 2-3=1994/2168, 3-4=1420/1600, 4-5=1270/1545, 5-6=1423/1603, 6-7=1993/2167, 7-8=0/40
BOT CHORD 2-13=1846/1776, 12-13=1846/1776, 11-12=1207/1267, 10-11=1207/1267, 9-10=1844/1775, 7-9=1844/1775
WEBS 3-13=263/178, 3-12=577/720, 4-12=450/338, 4-10=132/145, 5-10=452/354, 6-10=573/714, 6-9=260/176

REACTIONS (lb/size) 2=1112/0-3-8, 7=1112/0-3-8
Max Uplift=720(load case 5), 7=720(load case 6)
Max Horiz 2=128(load case 5), 7=128(load case 6)

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.3 WEBS 2 X 4 SYP No.3
BRACING Structural wood sheathing directly applied or 4-3-7 oc purlins. Rigid ceiling directly applied or 4-6-12 oc bracing.

LOADING (psf)	SPACING	Plates Increase	Lumber Increase	Rep Stress Incr	Code FBC2004/TP12002	CS	TC	BC	WB	(Metric)
20.0	2.0-0	1.25	1.25	YES		0.43	0.40	0.35	0.35	
7.0										
10.0										
5.0										



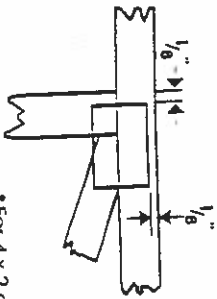


Symbols

PLATE LOCATION AND ORIENTATION



• Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply plates to both sides of truss and securely seal.



• For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



• This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

4 X 4

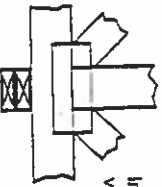
The first dimension is the width, perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



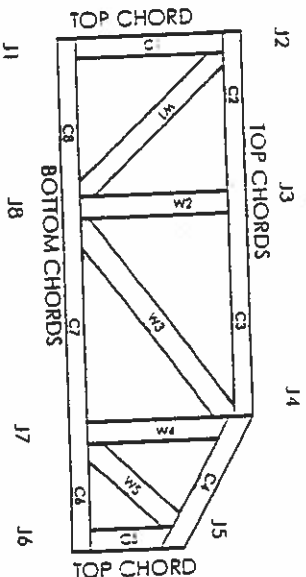
Indicates location of required continuous lateral bracing.

BEARING



Indicates location of joints at which bearings (supports) occur.

Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 96-67
ICBO	3907, 4922
SBCCI	9667, 9432A
WISC/DILLIR	960022-W, 970036-N
NER	561



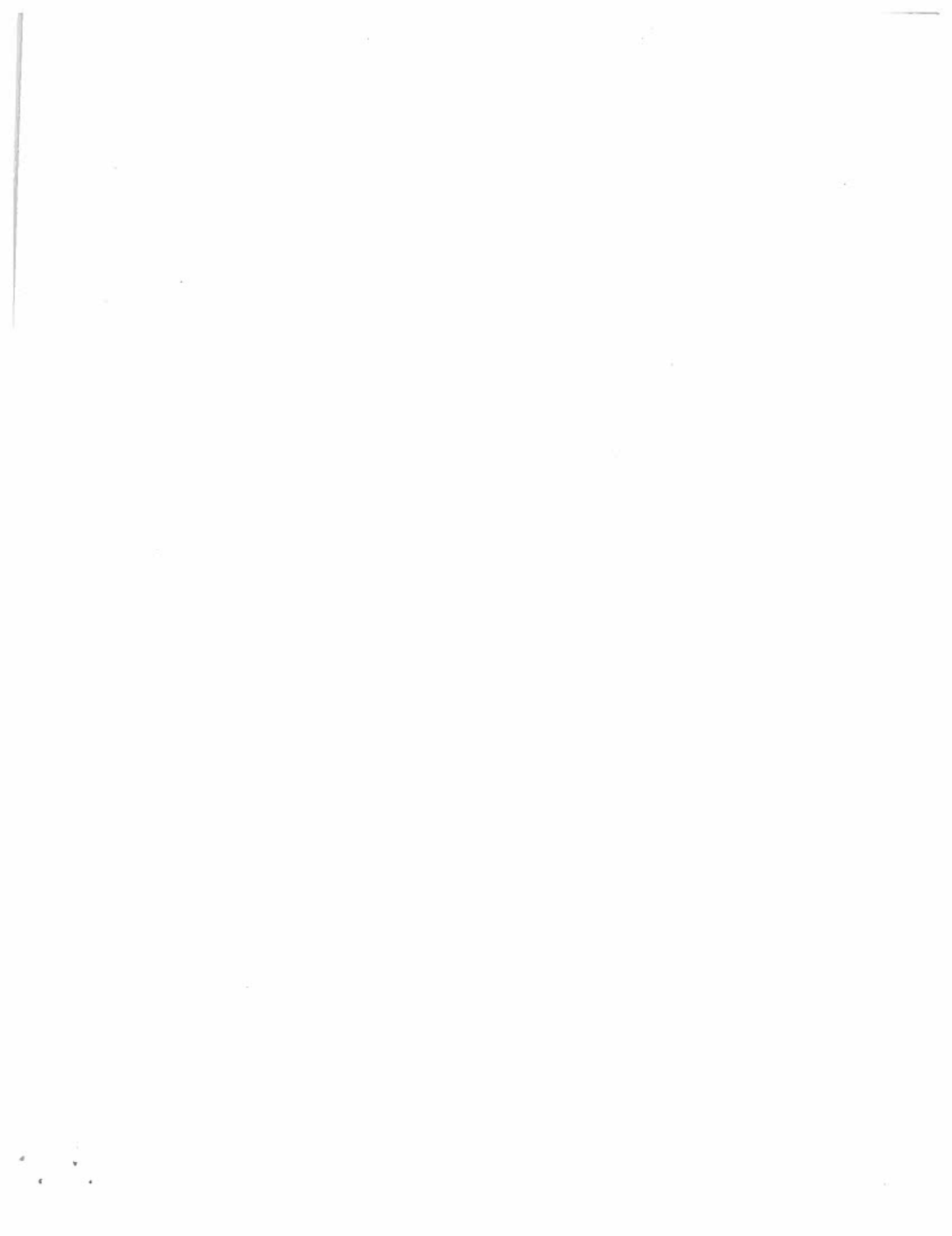
MITAK Engineering Reference Sheet: MIT-7473

General Safety Notes

Failure to Follow Could Cause Properly Damage or Personal Injury

1. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
2. Cut members to bear tightly against each other.
3. Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
4. Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint).
5. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
6. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
7. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
8. Plate type, size and location dimensions shown indicate minimum plating requirements.
9. Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
10. Top chords must be sheathed or pultruded provided at spacing shown on design.
11. Bottom chords require lateral bracing at 10 ft spacing, or less, if no ceiling is installed, unless otherwise noted.
12. Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
13. Do not overload roof or floor trusses with stacks of construction materials.
14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
15. Care should be exercised in handling, erection and installation of trusses.

© 1993 MITAK Holdings, Inc.



THIS INSTRUMENT WAS PREPARED BY:
TERRY MCDAVID
05-903
POST OFFICE BOX 1328
LAKE CITY, FL 32056-1328

PERMIT NO. _____

TAX FOLIO NO.: _____

NOTICE OF COMMENCEMENT

STATE OF FLORIDA
COUNTY OF COLUMBIA

The undersigned hereby gives notice that improvement will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

1. Description of property:
SEE EXHIBIT "A" ATTACHED HERETO AND MADE A PART HEREOF.

2. General description of improvement: Construction of Dwelling

3. Owner information:
a. Name and address: WILLIAM D. DURRANCE and KATHLEEN DURRANCE
8072 N US Highway 441, Lake City, FL 32055

- b. Interest in property: Fee Simple
- c. Name and address of fee simple title holder (if other than Owner): None

4. Contractor: MORTON CONSTRUCTION
3367 S US Highway 441, Suite 101
Lake City, FL 32025

5. Surety n/a
a. Name and address:
b. Amount of bond:

6. Lender: ATLANTIC COAST FEDERAL
10151 Deerwood Park Blvd., Ste 501, Bldg 100
Jacksonville, FL 32256

7. Persons within the state of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes: NONE

8. In addition to himself, Owner designates Atlantic Coast Federal, 10151 Deerwood Park Blvd., Suite 501, Bldg 100, Jacksonville, FL 32256, to receive a copy of the Lender's Notice as provided in Section 713.13(1)(b), Florida Statutes.

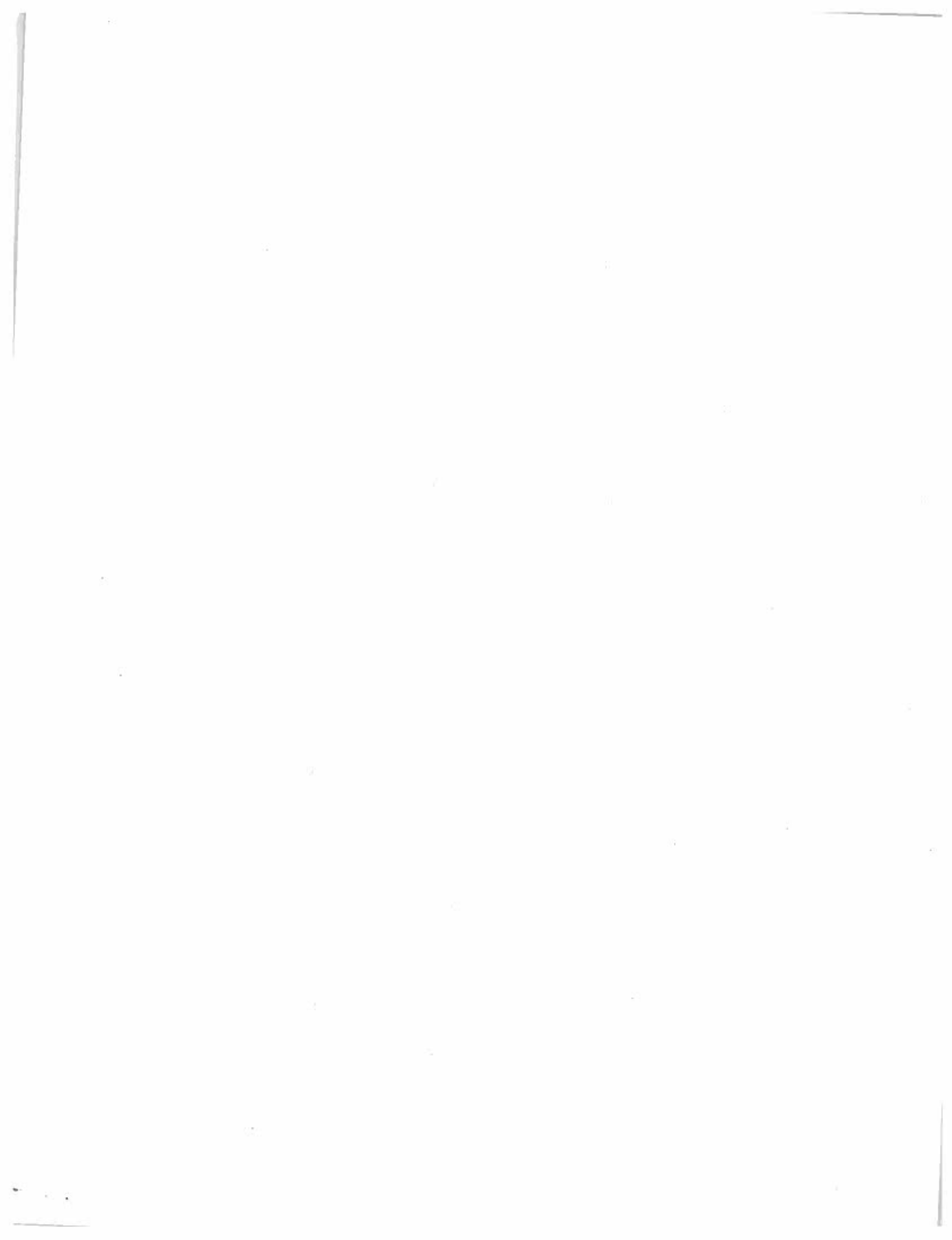
9. Expiration date of notice of commencement (the expiration date is 1 year from the date of recording unless a different date is specified).
November 23, 2006.

The foregoing instrument was acknowledged before me this 23rd day of November, 2005, by WILLIAM D. DURRANCE and KATHLEEN DURRANCE, Husband and wife, who is personally known to me and who did not take an oath.

Notary Public
My commission expires: _____




perm. + 23880



COMMENCE AT THE NORTHWEST CORNER OF NW 1/4 OF SE 1/4 OF SECTION 20, TOWNSHIP 2 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA; RUN THENCE N 1°09'45" E ALONG THE WEST LINE OF THE E 1/2 OF SAID SECTION 20, 1248.51 FEET; RUN THENCE N 89°59'15" E, 100.00 FEET; RUN THENCE N 1°09'45" E, 358.60 FEET; RUN THENCE N 89°59'15" E, 654.41 FEET; RUN THENCE S 2°15'15" W, 254.38 FEET; RUN THENCE S 0°0'15" E, 209.93 FEET; RUN THENCE N 89°13'55" E, 419.72 FEET TO WEST RIGHT-OF-WAY LINE OF U.S. 441; RUN THENCE DOWN SAID RIGHT-OF-WAY LINE S 0°25'32" E, 246.11 FEET TO THE POINT OF BEGINNING; RUN THENCE S 80°52'28" W, 285.00 FEET; RUN THENCE S 0°25'32" E, 135 FEET; RUN THENCE N 80°52'28" E, 285.00 FEET TO THE WEST RIGHT-OF-WAY LINE OF U.S. 441; THENCE N 0°25'32" W ALONG SAID WEST RIGHT-OF-WAY LINE OF U.S. 441, 135 FEET TO THE POINT OF BEGINNING.

EXHIBIT "A"



COLUMBIA COUNTY
OFFICE

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 20-2S-17-04741-000

Building permit No. 000023880

Use Classification SFD, UTILITY

Fire: 0.00

Permit Holder JAMES NORTON

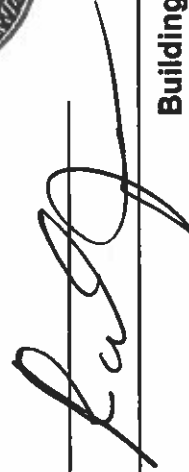
Waste: _____

Owner of Building WILLIAM & KATHLEEN DURRANCE

Total: 0.00

Location: 8072 N US HIGHWAY 441, LAKE CITY, FL

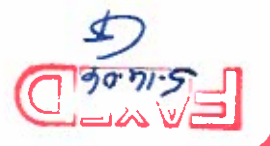
Date: 05/16/2006



Building Inspector



POST IN A CONSPICUOUS PLACE
(Business Places Only)



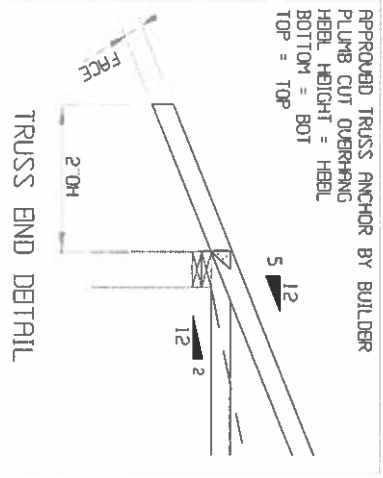
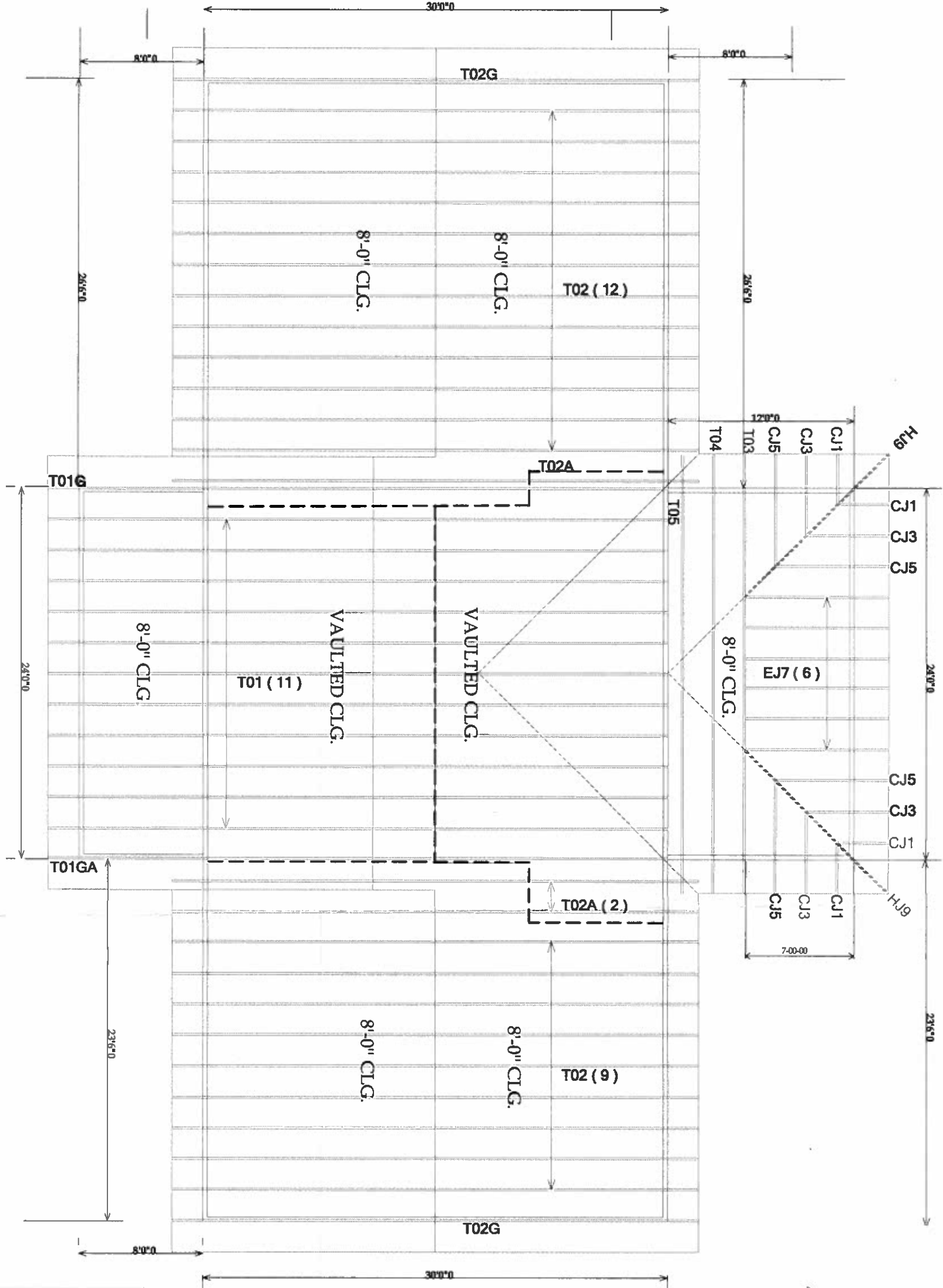
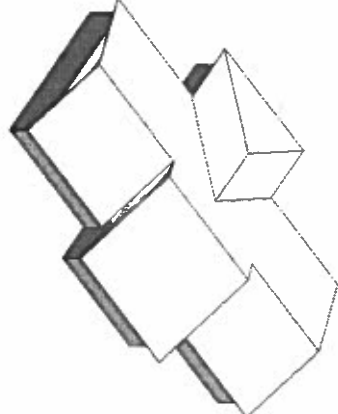
752-6427

8'-1"

OVERHAN
2'-0"

ROOF PITCH
5/12

EXTERIOR WALL SIZE 8"



NOTES:

- 1) REFER TO HUB 9 (RECOMMENDATIONS FOR BEARING INSTALLATION AND TYPICAL 8 REFER TO ENGINEERED DRAWINGS FOR FLOOR DRAGGING REQUIRED).
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE CORRECTLY DECIDED OR REFER TO DETAIL V09 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING. UNLESS OTHERWISE NOTE.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING. UNLESS OTHERWISE NOTED.
- 6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL E007 TRUSSES HANGERS TO BE SIMPSON HUS28 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSSES HANGERS TO BE SIMPSON TH4422 UNLESS OTHERWISE NOTED.
- 8) DEAMHEAD/EQUINTEL (Q02) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROV

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATING TRUSSES AND V09S. ALL PREVIOUS ADAPTATIONS OF TRUSSES LAYOUTS, REVISION AND APPROVAL OF THIS LAYOUT BE RETURNED BEFORE ANY TRUSSES WILL BE BUILT. V09 CONDITIONS TO MAKE AGAINST CHANGES THAT WILL INCREASE CHARGES TO YOU.

Approved By: _____

Site: _____



PHONE: 904-437-3349 FAX: 904-437-3349

PHONE: 904-772-6100 FAX: 904-772-6100

PHONE: 904-725-6894 FAX: 904-725-6894

PHONE: 407-322-0059 FAX: 407-322-0059

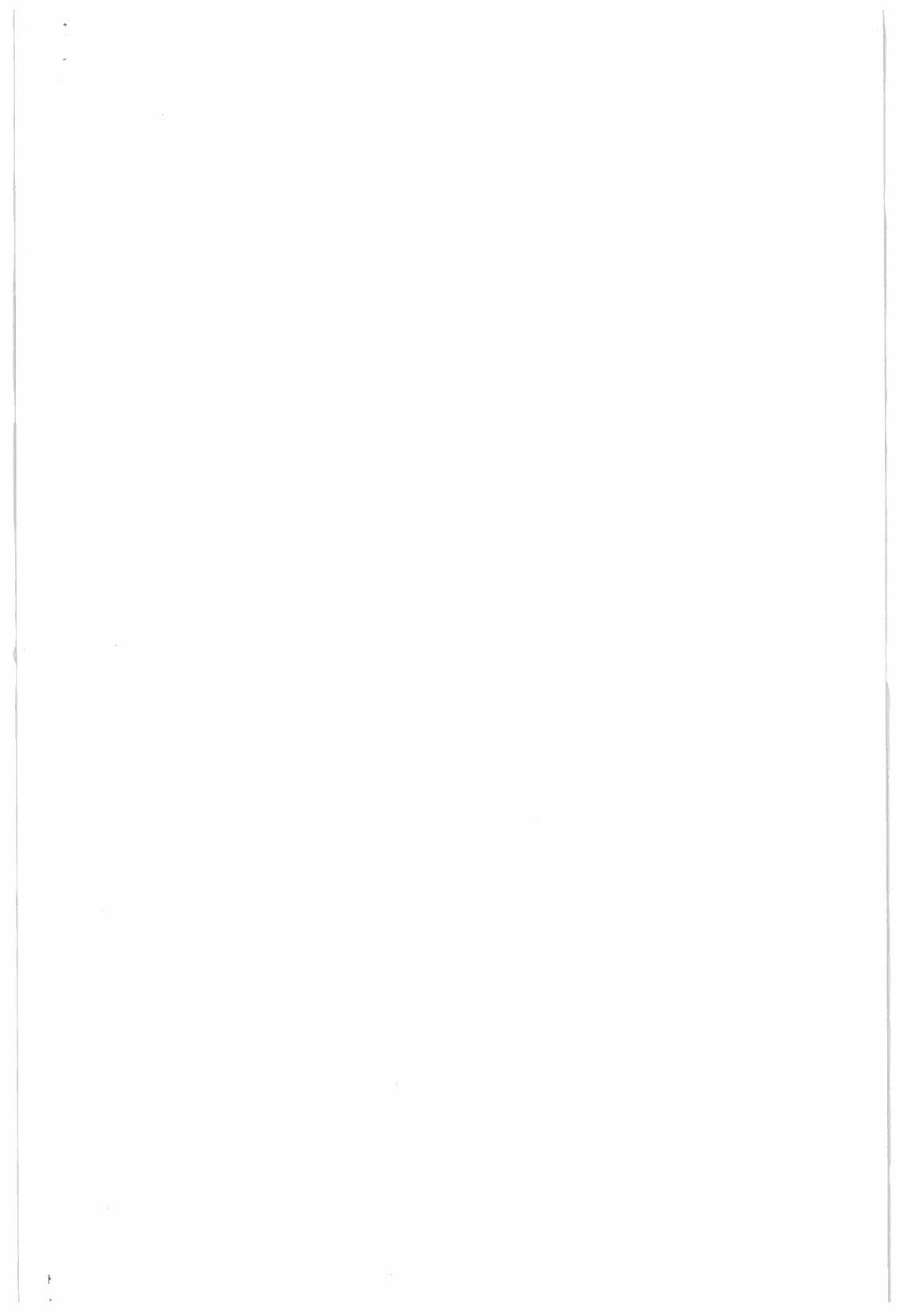
BUILDER: NORTON HOMI

LEGAL ADDRESS: LAKE CYTI

MODEL: CUSTOM

DATE: 10/28/05

AM L134



IN THE E 1/2 OF
SECTION 20, TOWNSHIP 2 SOUTH, RANGE 17 EAST
COLUMBIA COUNTY, FLORIDA

DEED:

[illegible]

TOGETHER WITH

COMMENCE AT THE NORTHWEST CORNER OF THE NW 1/4 OF THE SE 1/4 OF SECTION 20, TOWNSHIP 2 SOUTH, RANGE 17 EAST AND RUN THENCE N 01°09'45" E, ALONG THE WEST LINE OF THE E 1/2 OF SAID SECTION 20, 1248.51 FEET; THENCE N 89°59'15" E, 100.00 FEET; THENCE N 01°09'45" E, 358.66 FEET; THENCE N 89°59'15" E, 654.41 FEET; THENCE S 02°15' E, 254.38 FEET; THENCE N 89°59'15" E, 209.93 FEET; THENCE N 89°13'41" E, 419.12 FEET TO THE WEST RIGHT OF WAY OF U. S. HIGHWAY NO. 441; THENCE S 00°25'32" E, ALONG SAID WEST RIGHT OF WAY, 228.73 FEET TO THE POINT OF BEGINNING; THENCE CONTINUE S 00°25'32" E, ALONG SAID WEST RIGHT OF WAY, 17.20 FEET; THENCE S 80°47'44" W, 284.86 FEET; THENCE N 00°25'32" W, 17.20 FEET; THENCE N 80°47'44" E, 284.86 FEET TO THE POINT OF BEGINNING, CONTAINING 0.11 ACRES, MORE OR LESS.



SITE PLAN FOR
WILLIAM D. and KATHLEEN
DURRANCE RESIDENCE



BAILEY BISHOP & LANE, INC.
484 SW COMMERCE DRIVE, SUITE 130
P. O. BOX 3717
LAKE CITY, FL 32056-3717
PH. (386) 752-5840 FAX (386) 755-7771
Eng. Lic. 7362 Survey Lic. LB-0006885

DRAWN BY:

PA

JOB NUMBER
03111ADIB

FIELD BOOK

EFB

1 OF 1

Notice of Treatment

Applicator: Florida Pest Control & Chemical Co. (www.flapest.com)

Address: Boca Raton

City: West Palm Beach

Phone: 762-1203

Site Location: Subdivision 11A

Lot # 11A Block # 23880

Address 8072 N US Hwy 991

Permit # 14

Product used

☐ Premise

☐ Termitor

☒ Bora Care

Active Ingredient

Imidacloprid

Fipronil

Disodium Octaborate Tetrahydrate

% Concentration

0.1%

0.12%

23.0%

Type treatment:

☐ Soil

☒ Wood

Area Treated

Square feet

Linear feet

Gallons Applied

Drilling

2655

630

4.5

As per Florida Building Code 104.2.6 - If soil chemical barrier method for termite prevention is used, final exterior treatment shall be completed prior to final building approval.

If this notice is for the final exterior treatment, initial this line _____

Date 1/13/06

Time

1000

Print Technician's Name

F254 Gundy

Remarks: _____

Applicator - White

Permit File - Canary

Permit Holder - Pink

10/05 ©