

Wind Load Analysis and Certification

Cannon Residence (Bay V) by Fla Homes

2023 Florida Building Code section 1609 according to ASCE 7-22

Ultimate Design Wind Speed (Vult) = 130 MPH (3 second gust)

Nominal Design Wind Speed (Vasd)) = 101 MPH

Risk Category = II

Exposure Category = B, Enclosed Building

Applicable Internal Pressure Coefficient = .18

Design Wind Pressure for use of External Components (Components and Cladding)= +32.1psf, -43.3psf



Roof Decking

7/16" or 5/8" OSB or 1/2", 5/8" or 3/4" CDX Decking; 48"x96" Sheets, Perpendicular to Roof Framing Members

8d common (.131" dia) or 8d ring-shank (.113" dia.) nails at 4" O.C. on Ends, 8" O.C. in Interior

Trusses or Rafters at 2' O.C. (horizontal distance), No Intermediate Blocking Required

Rafters: 2x6 SYP #2 up to 10' horizontal span, 2x8 SYP #2 up to 14' horizontal span

Shear Wall Segments

7/16" OSB or 1/2" CDX plywood, 48" Wide Sheets - Sheathing Continuous from Top Plate down to Pressure Treated Sole Plate Bearing on Foundation.

8d common (.131" dia) nails at 3" O.C. on Edges and Ends, 8" O.C. in Interior

Transverse Shearwall = 30', Longitudinal Shearwall = 30'

2x4 SPF (No. 1&2) Studs at 16" O.C., up to 12'

2x6 SPF (No. 1&2) Studs at 16" O.C., up to 17'

See attached detail for stud and jack requirements for wall openings

Nail Together Double Top Plate with two (2) 12-d comm nails every 16" O.C. (SYP top plates)

Other Wall Segments - Same as Shear Walls

Gabled End Wall Framing

N/A

Special Notes: All headers to be double 2x8 SP#2 and all beams to be double 2x12 SP#2.

All girders and beams to have a min of two (2) 2x4 SP#2 or two (2) 2x6 SP#2 studs under each bearing point.

This structural and windload analysis is based on the attached truss layout. Any deviation from the attached layout invalidates this structural and windload analysis.

Footings and Foundations (Based on Truss Engineering)

20" deep x 12" wide monolithic with 2-#5's, Continuous, 3000 psi Concrete

or: 20" Wide x 10" Deep 3000 psi Concrete Strip Footing with 2-#5's, Continuous

8"x8"x16" Concrete Masonry Stemwall, Minimum 2 Courses, Maximum 4 Courses, Fully Grouted, except sections over 3 courses need only cells with rebar to be grouted. 1-#5 Vertical Dowel at Corners and 6'-0" O.C. (10" hook top and bottom) (min 25" lap all #5 rebar)

(1) #5 continuous top course. All 4" slabs requires 6x6 WWM

Porch Footers: 16" deep x 12" wide monolithic with 2-#5's, Continuous or see above or: 8" wide by 8" deep bell footing with 1-#5,

Continuous with minimum of 30"x30" x 15" pad under each post (w/ 3- #5 each way)

Note: footer design based on continuous bearing of 2000 psf. Footers for any concentrated loads greater than 10,000 lbs must be reviewed with windload engineer. Movement – The information presented in this document is not calculated or intended for the use or purpose of mitigating or addressing unsuitable soils or subsurface conditions in any way or manner, whatsoever.

Hurricane-Resistance Hardware (Based on Truss Engineering)

Truss Clips/Headers/Girders/Posts/Beams /Top and Bottom of Wall Unit - See Table

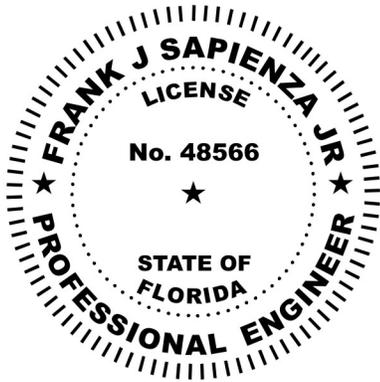
Note: regarding strapping of headers and beams, 50% of the strap shall be on the header/beam and 50% of the strap is on the supporting member below it (jacks and posts).

Anchor Bolts- 1/2"Dia. x 10" J Bolts (with min 7.5" embedment) at 48"O.C. (First bolt at 9" from Corner, then 48" O.C.) and at each end of Each Opening (2" round or square washers).

I hereby certify that the accompanying Wind Load Analysis for the **Bay V**, demonstrates compliance with the 2023 FBC section 1609 according to ASCE 7-22, to the best of my knowledge.

Frank J. Sapienza Jr.
License Professional Engineer
Florida License Number 48566

This item has been digitally signed and sealed by FRANK J SAPIENZA JR PE using Digital Signature.
Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.



HOLD-DOWN TABLE

| Wood Sections | Uplift Force Lbs | Top Connector Simpson ** | Rating Lbs | Bottom Connector Simpson ** | Rating Lbs |
|--|------------------|--------------------------|------------|-----------------------------|------------|
| HEADERS | | | | | |
| | up to 455 lbs | LSTA9 | 775 | H3 | 455 |
| | up to 910 lbs | LSTA12 | 970 | 2-H3 | 910 |
| | up to 1235 lbs | LSTA18 | 1235 | LTTP2 | 1680 |
| | up to 1940 lbs | 2-LSTA12 | 1940 | LTTP2 | 2275 |
| | up to 2470 lbs | 2-LSTA18 | 2470 | HTT4 | 3000 |
| | up to 3000 lbs | 3-LSTA18 | 3705 | HTT4 | 3000 |
| | up to 3705 lbs | 3-LSTA18 | 3705 | HTT5 | 4350 |
| To determine uplift force on header at each end, total the uplifts for each truss resting on the header and divide by 2 (assumes uniform load) Note: must use proper bolt anchors sufficient to support required load | | | | | |

Trusses/Girders - Uplift

| |
|--|
| up to 600 lbs - use H2.5A top, no special device required at bottom |
| over 600 lbs but under 990 lbs use H10 top, no special device required at bottom |
| up to 1230 lbs use H6 or equivalent at top and LTTP2 at bottom |
| up to 1680 lbs use 2-H6 or equivalent at top and LTTP2 at bottom |
| up to 2460 lbs use 2-H6 or equivalent at top and HTT4 bottom |
| up to 3690 lbs use 3-H6 or equivalent at top and HTT5 bottom |
| Must Use proper bolt anchors |

Note: it is the contractors responsibility to provide a continuous load path from truss/rafter/ridge beam to foundation

Strap rafters to truss or at each end with min uplift resistance of 450 lbs each end

Strap ridge beam at each end with min uplift resistance of 1800 lbs

Note: Three (3) 12d comm toenails (2 on one side, one on the other) required per truss/rafter per bearing point into plate to resist both lateral loads (wall to truss) and transverse loads

Horizontal Resistance (from truss loads) - Note: these devices are in addition to required toe-nails

| | |
|---------------------------------|---|
| up to 110 lbs - use H2.5A | Note: hardware to be used must satisfy both |
| up to 525 lbs use H10 | uplift and horizontal resistance, combination |
| up to 1090 lbs use H10 plus A23 | of devices is acceptable |

| | top | | bottom | |
|-------------------|----------------------------|------|------------------------------|------|
| BEAM SEATS | LSTA18* | 1235 | LTTP2* | 1680 |
| POSTS | 2-LSTA18 | 2400 | ABU44 or ABU66 | 2200 |
| | * or per truss engineering | | Must Use proper bolt anchors | |

STUDS

Wall Sheathing Nailing Adequate Exterior Walls bottom (8d nails at 3" O.C.), must cover sill plate

Wall Sheathing Nailing Adequate Exterior Walls Top (8d nails at 3" O.C.), as long as sheathing covers top plate, otherwise use SP2 @32" O.C. in addition to sheathing nailing,

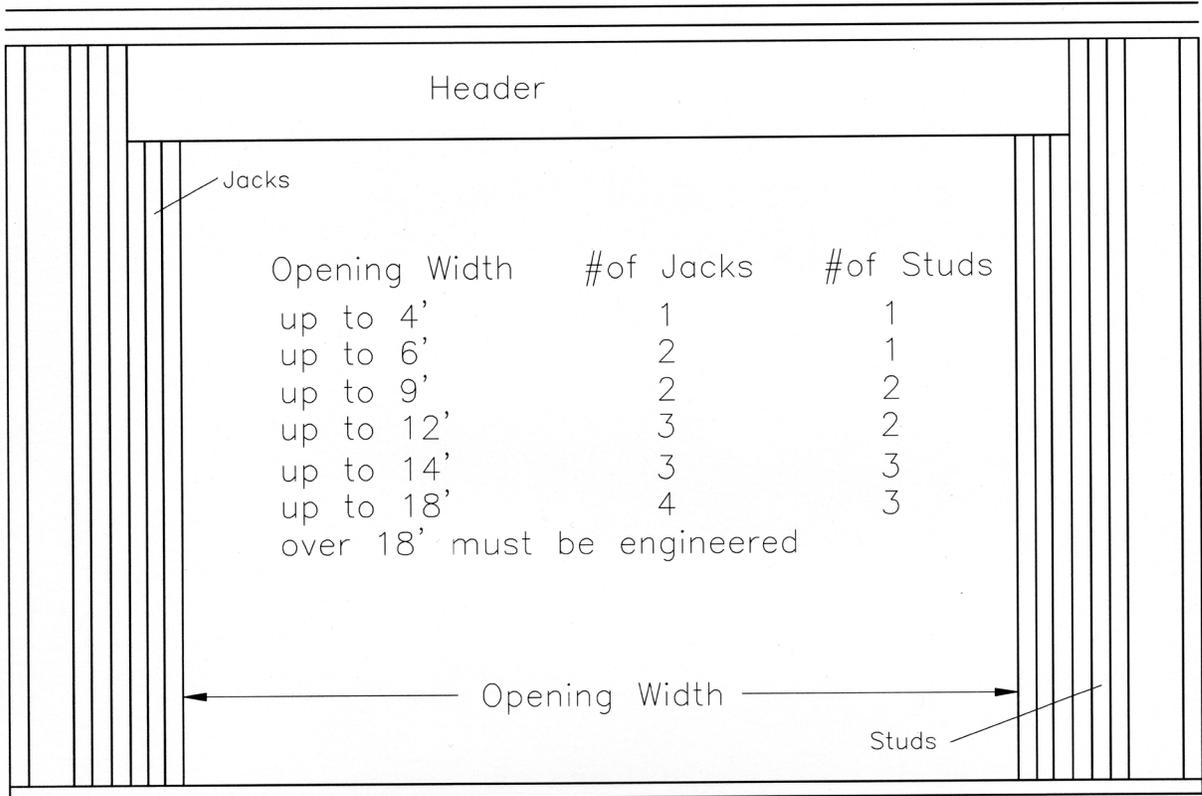
Use SP2 top and SP1 bottom each stud an anchor bolts @ 32" O.C. for all interior load bearing walls that have uplift. Interior anchor bolts to be 1/2" x 10" A307 or 1/2" x 6" Titan HD with 2" washers

Please Note: All Beams must be sheathed or strapped to double top plate (if applicable)

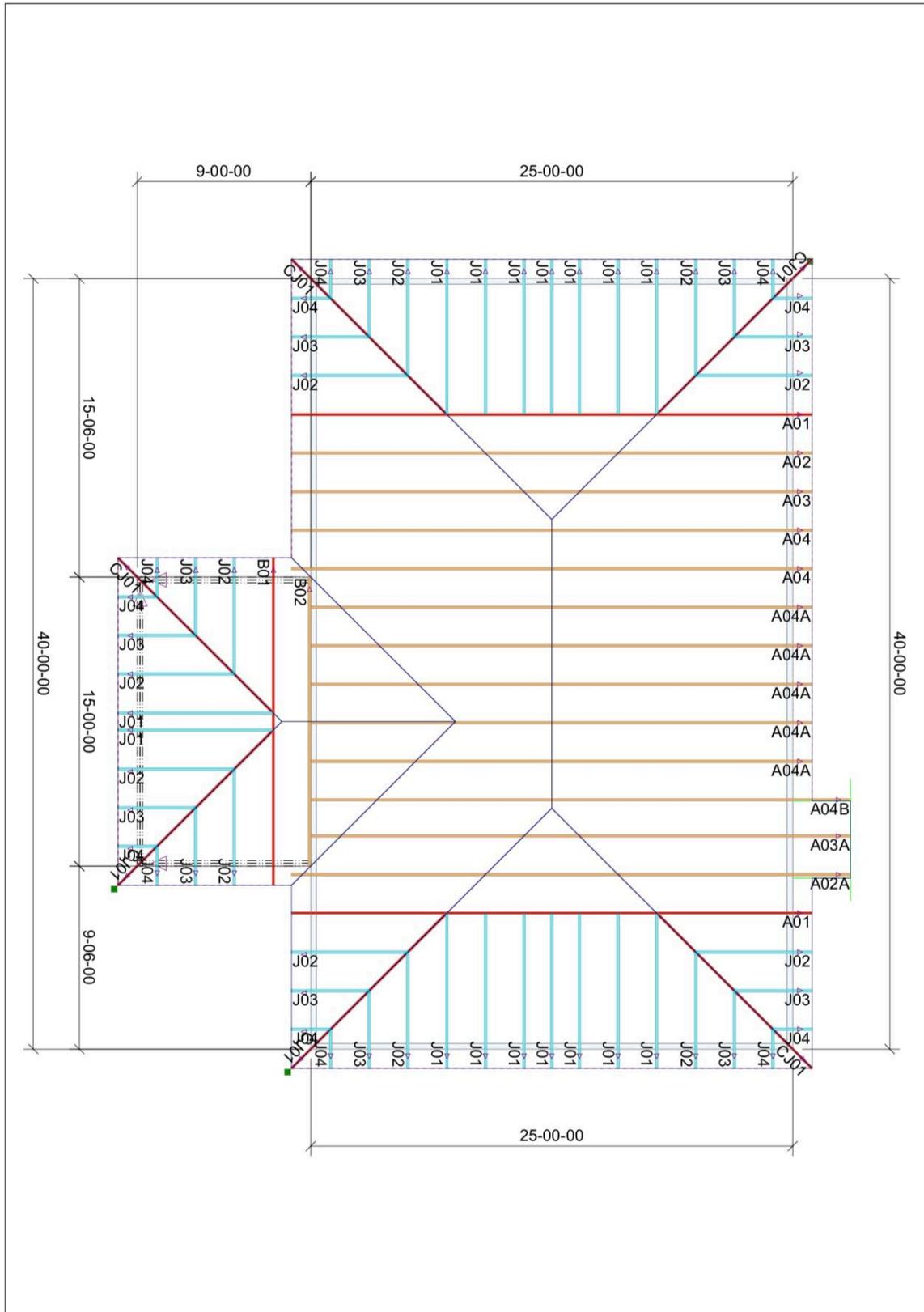
An equivalent device of same or other manufactures can be substituted for any of the devices specified on this page as long as it meets the required load capacities

Note: For nailing into SPF members, multiply table values by .86

Number of Jack and Stud Requirements per Opening Width
2x4 or 2x6 SPF #1&2 Construction – max Wall Height=12'
(based on 16" O.C. Stud Spacing)



Note – Based on uniform loads. Heavy concentrated loads require engineering review



Bay V

Client: FLA HOMES
 Date: 6/27/2025
 Quote Date: / /
 Seal Date: / /
 Designer: Jason DeGroff
 Job Number: 0625-057


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