clc ~~Hodge~~ w/c
8602

Columbia County New Building Permit Application

For Office Use Only Application # 1908-36 Date Received 8/12/19 By MG Permit # 38529

Zoning Official JC/LH Date 8-19-19 Flood Zone X Land Use AG Zoning A-3

FEMA Map # N/A Elevation N/A MFE 1' Above River N/A Plans Examiner JC Date 8-19-19

Comments Floor 1' Above Rd. Front +- 30' Sides 25' Rear 25' EXISTING

☑ NOC ☑ EH ☑ Deed or PA ☑ Site Plan ☐ State Road Info ☐ Well letter ☐ 911 Sheet ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Auth. from Contractor ☐ F W Comp. letter

☐ Owner Builder Disclosure Statement ☐ Land Owner Affidavit ☐ Ellisville Water ☑ App Fee Paid ☑ Sub VF Form

Septic Permit No. 19-0599 OR City Water ☐ Fax

Applicant (Who will sign/pickup the permit) Gerald Harvey Phone 352-258-9051

Address ~~100 SW Oak Glen~~ PO Box 526 Newberry, FL 32669

Owners Name Carl Michael and Brenda Faith Hartzog Phone 352-215-5666

911 Address 100 SW Oak Glen Ft White 32038 1167 SW Woodland Ave

Contractors Name Gerald Harvey Phone 352-258-9051

Address PO Box 526 Newberry, FL 32669

Contractor Email harveybuilding@gmail.com ***Include to get updates on this job.

Fee Simple Owner Name & Address

Bonding Co. Name & Address

Architect/Engineer Name & Address Jacob Eskeridge Alachua, FL

Mortgage Lenders Name & Address

Circle the correct power company ☐ FL Power & Light ☑ Clay Elec. ☐ Suwannee Valley Elec. ☐ Duke Energy

Property ID Number 31-7S-17-10070-115 Estimated Construction Cost 300,000

Subdivision Name Bluebird Landing Lot 15 Block Unit Phase ~~Fleet~~

Driving Directions from a Major Road 47 S, LCR 138, R Woodland Ave, property on NE corner of woodland and oak gln

Construction of House Commercial OR X Residential

Proposed Use/Occupancy Primary Residence Number of Existing Dwellings on Property 0

Is the Building Fire Sprinkled? If Yes, blueprints included Or Explain

Circle Proposed ☐ Culvert Permit ☐ Culvert Waiver or ☐ D.O.T. Permit or ☑ Have an Existing Drive Private Road

Actual Distance of Structure from Property Lines - Front 240 Side 475 Side 150 Rear 390

Number of Stories 1 Heated Floor Area 2800 Total Floor Area 3909 Acreage 10.06

Zoning Applications applied for (Site & Development Plan, Special Exception, etc.)

8/27 - Spoke w/ Gerald, will call back w/ check #. Has EH + 911 on hand. will provide upon arrival

Columbia County Building Permit Application

CODE: Florida Building Code 2017 and the 2014 National Electrical Code.

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

TIME LIMITATIONS OF APPLICATION : An application for a permit for any proposed work shall be deemed to have been abandoned 180 days after the date of filing, unless pursued in good faith or a permit has been issued.

TIME LIMITATIONS OF PERMITS: Every permit issued shall become invalid unless the work authorized by such permit is commenced within 180 days after its issuance, or if the work authorized by such permit is suspended or abandoned for a period of 180 days after the time work is commenced. A valid permit receives an approved inspection every 180 days. Work shall be considered not suspended, abandoned or invalid when the permit has received an approved inspection within 180 days of the previous approved inspection.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment: According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO CONTRACTOR AND AGENT: ***YOU ARE HEREBY NOTIFIED*** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

OWNERS CERTIFICATION: I CERTIFY THAT ALL THE FOREGOING INFORMATION IS ACCURATE AND THAT ALL WORK WILL BE DONE IN COMPLIANCE WITH ALL APPLICABLE LAWS REGULATING CONSTRUCTION AND ZONING.

NOTICE TO OWNER: There are some properties that may have deed restrictions recorded upon them. These restrictions may limit or prohibit the work applied for in your building permit. You must verify if your property is encumbered by any restrictions or face possible litigation and or fines.

Carl M Hartsog — Print Owners Name

Owners Signature

****Property owners must sign here before any permit will be issued.**

****If this is an Owner Builder Permit Application then, ONLY the owner can sign the building permit when it is issued.**

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit including all application and permit time limitations.

Contractor's Signature

Contractor's License Number CR C058134
Columbia County
Competency Card Number 675

Affirmed under penalty of perjury to by the Contractor and subscribed before me this ____ day of _____ 20___.

Personally known [X] or Produced Identification _____

Kimberly A McGraw — State of Florida Notary Signature (For the Contractor)

SEAL:

KIMBERLY A. MCGRAW
MY COMMISSION # FF 929139
EXPIRES: December 3, 2019
Bonded Thru Notary Public Underwriters

Legend

Parcels

2018Aerials

Addresses

SRWMD Wetlands

2018 Flood Zones

0.2 PCT ANNUAL CHANCE

A

AE

AH

Roads

Roads

others

Dirt

Interstate

Main

Other

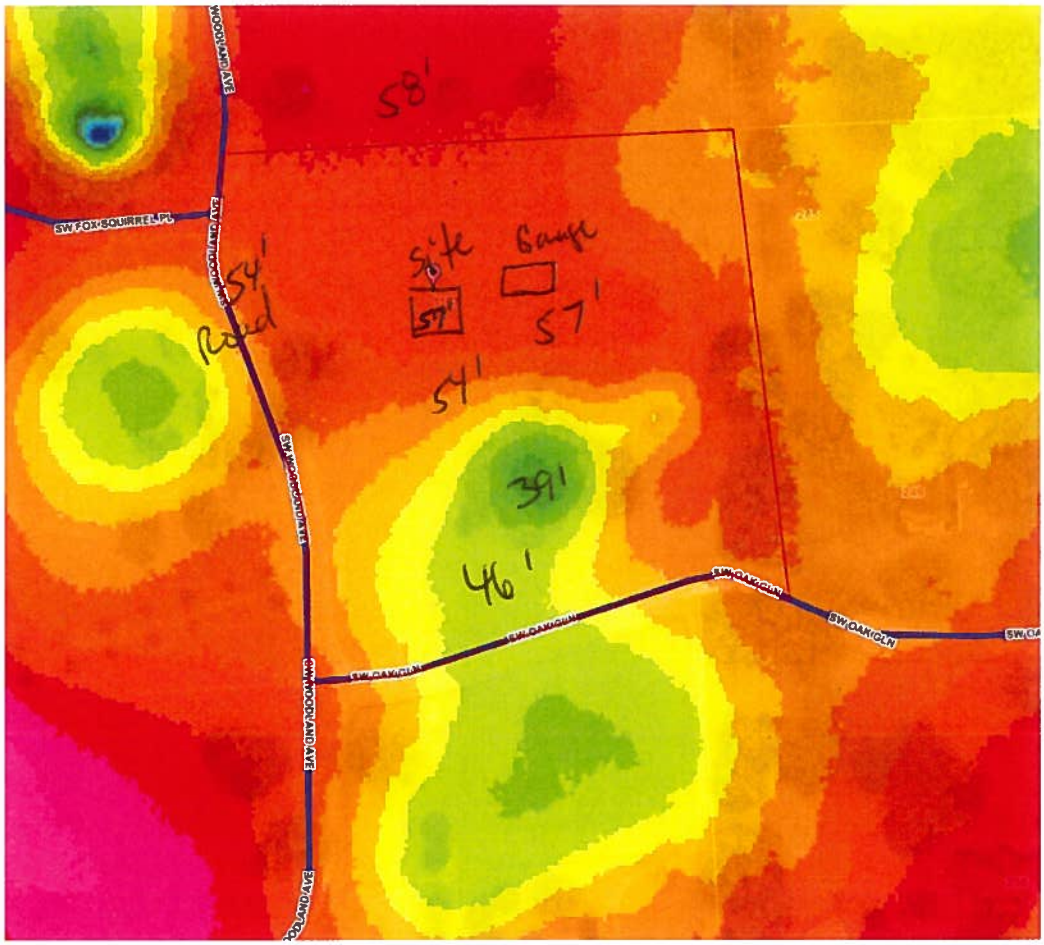
Paved

Private

LidarElevations

Columbia County, FLA - Building & Zoning Property Map

Printed: Mon Aug 26 2019 17:14:54 GMT-0400 (Eastern Daylight Time)



Parcel Information

Parcel No: 31-7S-17-10070-115

Owner: MCADAMS BRIAN L & CATHERINE

Subdivision: BLUEBIRD LANDING UNR

Lot:

Acres: 10.1585732

Deed Acres: 10.06 Ac

District: District 2 Rocky Ford

Future Land Uses: Agriculture - 3

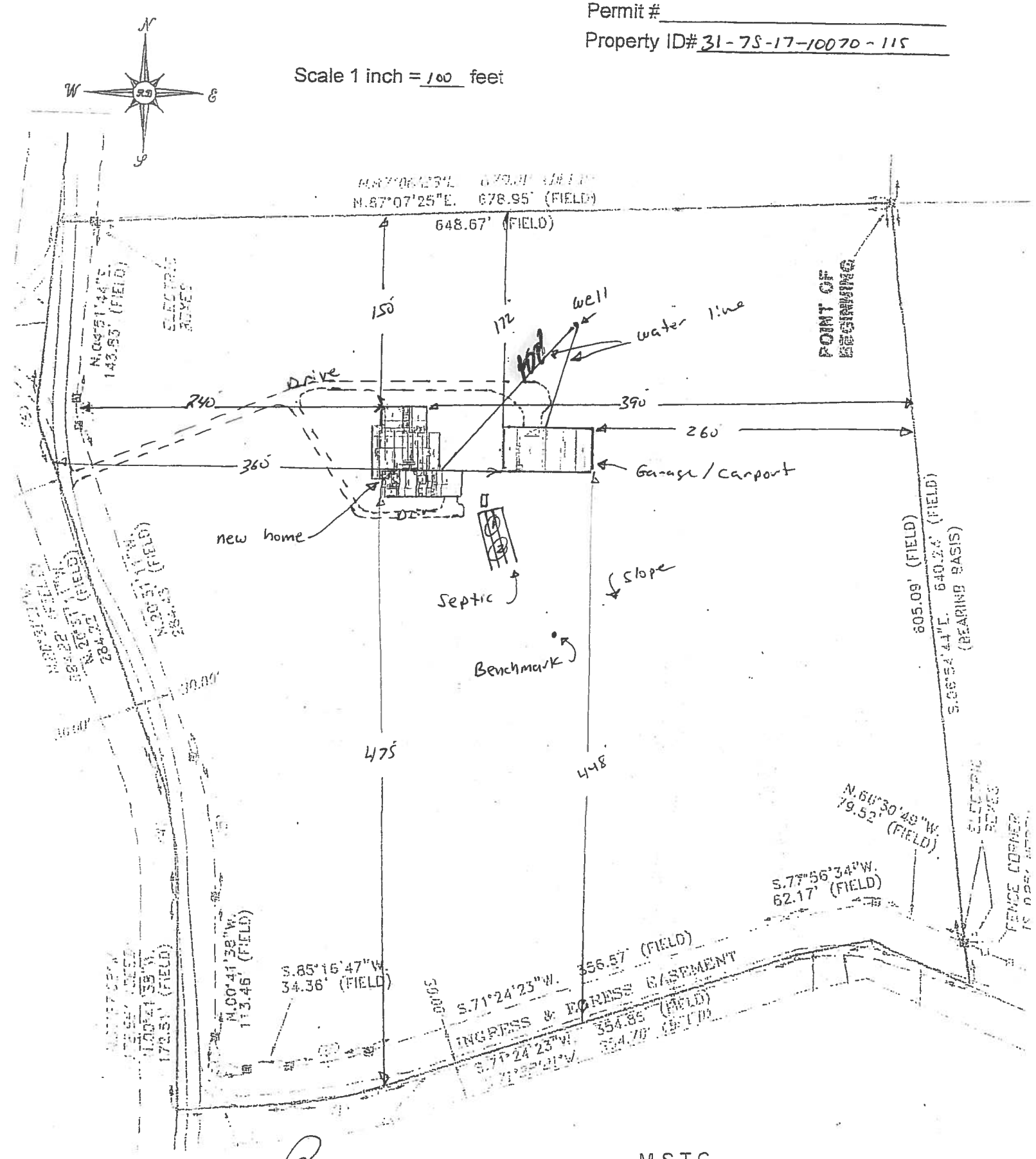
Flood Zones:

Official Zoning Atlas: A-3

Permit #_____

Property ID# 31-7S-17-10070-115

Scale 1 inch = 100 feet



Site Plan submitted by Ron Moore M.S.T.C.

Plan Approved_____ Not Approved_____ Date_____

By _____ County Health Department

Well To Septic 105'

SUBCONTRACTOR VERIFICATION

APPLICATION/PERMIT # _____ JOB NAME Hartzog

THIS FORM MUST BE SUBMITTED BEFORE A PERMIT WILL BE ISSUED

Columbia County issues combination permits. One permit will cover all trades doing work at the permitted site. It is REQUIRED that we have records of the subcontractors who actually did the trade specific work under the general contractors permit.

NOTE: *It shall be the responsibility of the general contractor to make sure that all of the subcontractors are licensed with the Columbia County Building Department.*
Use website to confirm licenses: http://www.columbiacountyfla.com/PermitSearch/ContractorSearch.aspx

NOTE: If this should change prior to completion of the project, it is your responsibility to have a corrected form submitted to our office, before that work has begun.

Violations will result in stop work orders and/or fines.

			Need
ELECTRICAL ☑ CC# 811	Print Name Ryan Beville Signature Ryan Beville Company Name: RBI Electric License #: EC1300-4236 Phone #: 352-514-3382		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
MECHANICAL/ A/C ☑ CC# 768	Print Name Robert Bounds Signature Robert Bounds Company Name: Bounds Heating and Air License #: CA-057642 Phone #: 352-472-2761		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
PLUMBING/ GAS ☑ CC# 578	Print Name Wayne Hodge Signature Wayne Hodge Company Name: Hodge Plumbing License #: CFC 1426382 Phone #: 352-538-9647		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
ROOFING ☑ CC# 1129	Print Name Dana Johnson Signature Company Name: Mac Johnson Roofing License #: CCC1325497 Phone #: 352-472-4943		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
SHEET METAL ☐ CC#	Print Name Signature Company Name: License #: Phone #:		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
FIRE SYSTEM/ SPRINKLER ☐ CC#	Print Name Signature Company Name: License#: Phone #:		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
SOLAR ☐ CC#	Print Name Signature Company Name: License #: Phone #:		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE
STATE SPECIALTY ☐ CC#	Print Name Signature Company Name: License #: Phone #:		☐ Lic ☐ Liab ☐ W/C ☐ EX ☐ DE

Ref: F.S. 440.103; ORD. 2016-30

E-mail to (sjh0330@gmail.com)

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 19-0599
DATE PAID: 8/12/19
FEE PAID: $310.00
RECEIPT #: 142113

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Mike & Brenda Hartzog

AGENT: Ronnie Moore TELEPHONE: 352-246-3997

MAILING ADDRESS: PO Box 158 FT white FL 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 15 BLOCK: _____ SUBDIVISION: Bluebird Landing unrecorded PLATTED: 2002

PROPERTY ID #: 31-7S-17-10070-115 ZONING: SF I/M OR EQUIVALENT: [Y / (N)]

PROPERTY SIZE: 10.06 ACRES WATER SUPPLY: [X] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [Y / (N)] DISTANCE TO SEWER: N/A FT

PROPERTY ADDRESS: 100 Oak GLN FT White FL 32038

DIRECTIONS TO PROPERTY: 47 south to 27 turn left to Mapleton ST turn right to Woodland Ave turn left to 100 Oak Gln on left.

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	single family	4	2800	
2	garage/carport	0	25 sf bath room	
3			2825	
4				

[ ] Floor/Equipment Drains [ ] Other (Specify) _____

SIGNATURE: Ronnie Moore DATE: 08/09/19

DH 4015, 08/09 (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC

Columbia County Property Appraiser

Jeff Hampton

2018 Tax Roll Year
updated: 6/25/2019

Parcel: << **31-7S-17-10070-115** >> See Deed

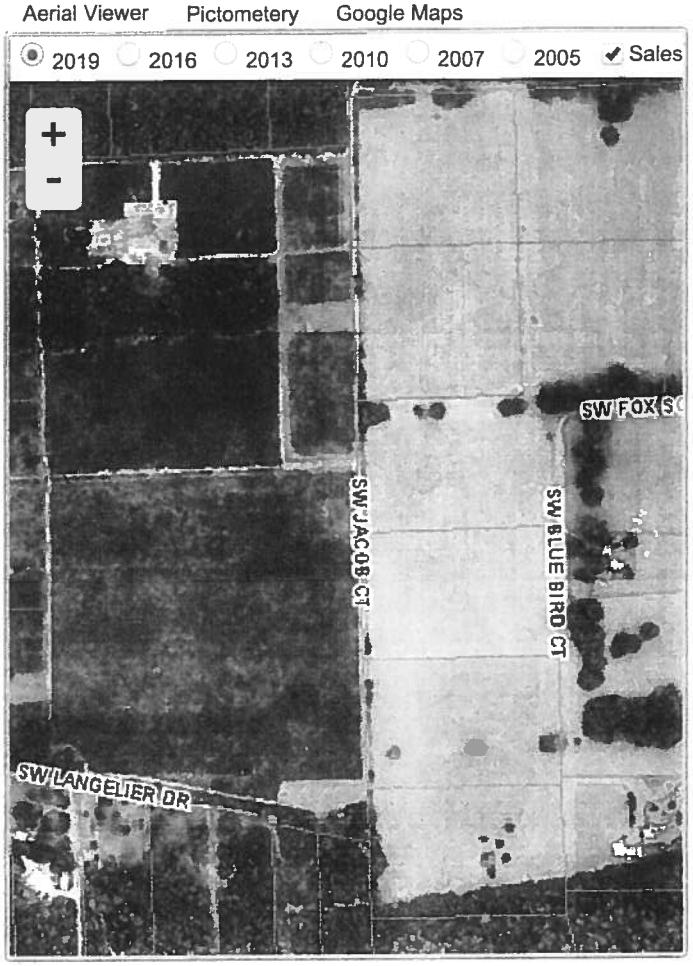
Owner & Property Info

Result: 1 of 4

Owner	**MCADAMS BRIAN L & CATHERINE G** 278 NE LILAC CIRCLE MAYO, FL 32066		
Site	100 OAK GLN,		
Description*	AKA LOT 15 BLUEBIRD LANDING UNREC: COMM NE COR OF NW1/4, RUN E 50.03 FT, S 1392.44 FT FOR POB, CONT S'LY 640.24 FT, N 66 DEG W 87.87 FT, S 77 DEG W 62.17 FT, S 71 DEG W 354.70 FT TO PT OF A CURVE, SW ALONG CURVE 72.57 FT, S 85 DEG W 93.53 FT, N 172.66 FT T ...more>>>		
Area	10.06 AC	S/T/R	31-7S-17E
Use Code**	VACANT (000000)	Tax District	3

*The Description above is not to be used as the Legal Description for this parcel in any legal transaction.
**The Use Code is a FL Dept. of Revenue (DOR) code and is not maintained by the Property Appraiser's office. Please contact your city or county Planning & Zoning office for specific zoning information.

Aerial Viewer Pictometery Google Maps



Property & Assessment Values

2018 Certified Values		2019 Working Values	
Mkt Land (1)	$67,723	Mkt Land (1)	$67,723
Ag Land (0)	$0	Ag Land (0)	$0
Building (0)	$0	Building (0)	$0
XFOB (0)	$0	XFOB (0)	$0
Just	$67,723	Just	$67,723
Class	$0	Class	$0
Appraised	$67,723	Appraised	$67,723
SOH Cap [?]	$0	SOH Cap [?]	$0
Assessed	$67,723	Assessed	$67,723
Exempt	$0	Exempt	$0
Total Taxable	county:$67,723 city:$67,723 other:$67,723 school:$67,723	Total Taxable	county:$67,723 city:$67,723 other:$67,723 school:$67,723

Sales History

Sale Date	Sale Price	Book/Page	Deed	V/I	Quality (Codes)	RCode
8/31/2016	$74,900	1321/2429	WD	V	Q	01
4/23/2009	$252,000	1171/2510	WD	V	Q	01
2/24/2006	$252,000	1075/2222	WD	V	Q	
6/21/2003	$93,500	987/0050	WD	V	Q	
6/12/2002	$100	955/1168	WD	V	U	06

Building Characteristics

Bldg Sketch	Bldg Item	Bldg Desc*	Year Blt	Base SF	Actual SF	Bldg Value
NONE						

Extra Features & Out Buildings (Codes)

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

NOTICE OF COMMENCEMENT

Clerk's Office Stamp

Inst: 201912018609 Date: 08/12/2019 Time: 4:05PM
Page 1 of 1 B: 1391 P: 72, P.DeWitt Cason, Clerk of Court Colum
County, By: PT
Deputy Clerk

Tax Parcel Identification Number:

31-7S-17-10077-115

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this **NOTICE OF COMMENCEMENT.**

1. Description of property *(legal description)*: Parcel #15 of BlueBird Landing: E½ of Section 31, Township 7 South, Range 17 East
   a) Street *(job)* Address: 100 SW Oak Glen, Fort White, FL 32038
2. General description of improvements: New Home Construction
3. Owner Information or Lessee information if the Lessee contracted for the improvements:
   a) Name and address: Mike or Brenda Hartzog P.O.Box 325 High Springs, FL 32655
   b) Name and address of fee simple titleholder (if other than owner) _____
   c) Interest in property _____
4. Contractor Information
   a) Name and address: Gerald Harvey P.O. Box 526 Newberry 32669
   b) Telephone No.: 352-258-9051
5. Surety Information (if applicable, a copy of the payment bond is attached):
   a) Name and address: _____
   b) Amount of Bond: _____
   c) Telephone No.: _____
6. Lender
   a) Name and address: _____
   b) Phone No. _____
7. Person within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes:
   a) Name and address: Gerald Harvey P.O. Box 526 Newberry 32669
   b) Telephone No.: 352-258-9051
8. In addition to himself or herself, Owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
   a) Name: _____ OF _____
   b) Telephone No.: _____
9. Expiration date of Notice of Commencement **(the expiration date will be 1 year from the date of recording unless a different date is specified):** _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. Brenda Hartzog
Signature of Owner or Lessee, or Owner's or Lessee's Authorized Office/Director/Partner/Manager

Brenda Hartzog
Printed Name and Signatory's Title/Office

The foregoing instrument was acknowledged before me, a Florida Notary, this 12 day of August, 2019, by:

Brenda Hartzog (Name of Person) as Owner (Type of Authority) for Brenda Hartzog (name of party on behalf of whom instrument was executed).

Personally Known ____ OR Produced Identification ✓ Type Florida Drivers Lic.

Notary Signature _____ Notary Stamp or Seal:

NICHOLAS LOSI
Commission # GG 317219
Expires March 28, 2023
Bonded Thru Budget Notary Services

This Document Prepared By and Return to:
Darryl J. Tompkins, Esquire
Darryl J. Tompkins, P.A.
14420 NW 151st Blvd.
P.O. Box 519
Alachua, FL 32616

Parcel ID Number: R10070-115

Warranty Deed

This Indenture, Made this 7th day of August, 2019 A.D. **Between** Brian L. McAdams and Catherine G. McAdams, husband and wife

of the County of Lafayette, State of Florida, **grantors,** and Carl Michael Hartzog and Brenda Faith Hartzog, husband and wife

whose address is 21672 NW 142nd Avenue, High Springs, FL 32643

of the County of Alachua, State of Florida, **grantees.**

Witnesseth that the GRANTORS, for and in consideration of the sum of ----------TEN DOLLARS ($10)---------- DOLLARS and other good and valuable consideration to GRANTORS in hand paid by GRANTEES, the receipt whereof is hereby acknowledged, have granted, bargained and sold to the said GRANTEES and GRANTEES' heirs, successors and assigns forever, the following described land, situate, lying and being in the County of Columbia State of Florida to wit

See Exhibit "A" attached hereto.

SUBJECT TO THE FOLLOWING:

A. Zoning restrictions, prohibitions and other requirements imposed by governmental authority;
B. Restrictions and matters appearing on the plat and/or in the public records of Columbia County, Florida; provided, however, the reference herein shall not be deemed to reimpose same;
C. Taxes for the year 2019 and subsequent years.

and the grantors do hereby fully warrant the title to said land, and will defend the same against lawful claims of all persons whomsoever.

In Witness Whereof, the grantors have hereunto set their hands and seals the day and year first above written.

Signed, sealed and delivered in our presence:

Printed Name: Darryl J. Tompkins
Witness

Brian L. McAdams (Seal)
PO Address 278 NE Lilac Circle, Mayo, FL 32066

Printed Name: Sandra E. Howe
Witness

Catherine G. McAdams (Seal)
PO Address 278 NE Lilac Circle, Mayo, FL 32066

STATE OF FLORIDA
COUNTY OF ALACHUA

The foregoing instrument was acknowledged before me this 7th day of August, 2019 by Brian L. McAdams and Catherine G. McAdams, husband and wife

who are personally known to me or who have produced their Florida driver's license as identification.

Notary Public State of Florida
Sandra E Howe
My Commission GG 126635
Expires 11/15/2021

Printed Name: Sandra E. Howe
Notary Public
My Commission Expires 11/15/21

EXHIBIT "A"

PARCEL #15 OF BLUEBIRD LANDING, UNRECORDED:

A PART OF THE E ½ OF SECTION 31, TOWNSHIP 7 SOUTH, RANGE 17 EAST, BEING MORE PARTICULARLY DESCRIBED AS FOLLOWS:

COMMENCE AT THE NE CORNER OF THE NW ¼ OF SAID SECTION 31 AND RUN THENCE N.87 Degrees 14'55"E., ALONG THE NORTH LINE THEREOF, A DISTANCE OF 50.03 FEET; THENCE S.00 Degrees 37'02"E., A DISTANCE OF 1392.44 FEET TO THE POINT OF BEGINNING; THENCE S.06 Degrees 54'44"E., A DISTANCE OF 640.24 FEET; THENCE N.66 Degrees 25'47"W., A DISTANCE OF 87.87 FEET; THENCE S.77 Degrees 56'34"W., A DISTANCE OF 62.17 FEET; THENCE S.71 Degrees 22'21"W., A DISTANCE OF 354.70 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT HAVING: A RADIUS OF 300.00 FEET, A CENTRAL ANGLE OF 13 Degrees 51'33", A TANGENT LENGTH OF 36.46 FEET, A CHORD BEARING OF S.78 Degrees 18'07"W., AND A CHORD LENGTH OF 72.39 FEET; THENCE SOUTHWESTERLY, ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 72.57 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE S.85 Degrees 13'54"W., A DISTANCE OF 93.53 FEET; THENCE N.00 Degrees 37'02"W., A DISTANCE OF 172.66 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 300.00 FEET, A CENTRAL ANGLE OF 19 Degrees 54'39", A TANGENT LENGTH OF 52.66 FEET, A CHORD BEARING OF N.10 Degrees 34'21"W., AND A CHORD LENGTH OF 103.73 FEET; THENCE NORTHWESTERLY, ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 104.25 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE N.20 Degrees 31'41"W., A DISTANCE OF 264.45 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 80.00 FEET, A CENTRAL ANGLE OF 25 Degrees 22'17", A TANGENT LENGTH OF 18.01 FEET, A CHORD BEARING OF N.07 Degrees 50'32"W., AND A CHORD LENGTH OF 35.14 FEET; THENCE NORTHWESTERLY, ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 35.43 FEET TO THE POINT OF TANGENCY OF SAID CURVE; THENCE N.04 Degrees 50'37"E., A DISTANCE OF 139.59 FEET; THENCE N.87 Degrees 06'23"E., A DISTANCE OF 679.01 FEET TO THE POINT OF BEGINNING, COLUMBIA COUNTY, FLORIDA.

TOGETHER WITH AN EASEMENT FOR INGRESS AND EGRESS, BEING 60 FEET IN WIDTH AND LYING 30 FEET TO THE LEFT AND 30 FEET TO THE RIGHT, AS MEASURED PERPENDICULAR TO THE FOLLOWING DESCRIBED CENTERLINE:

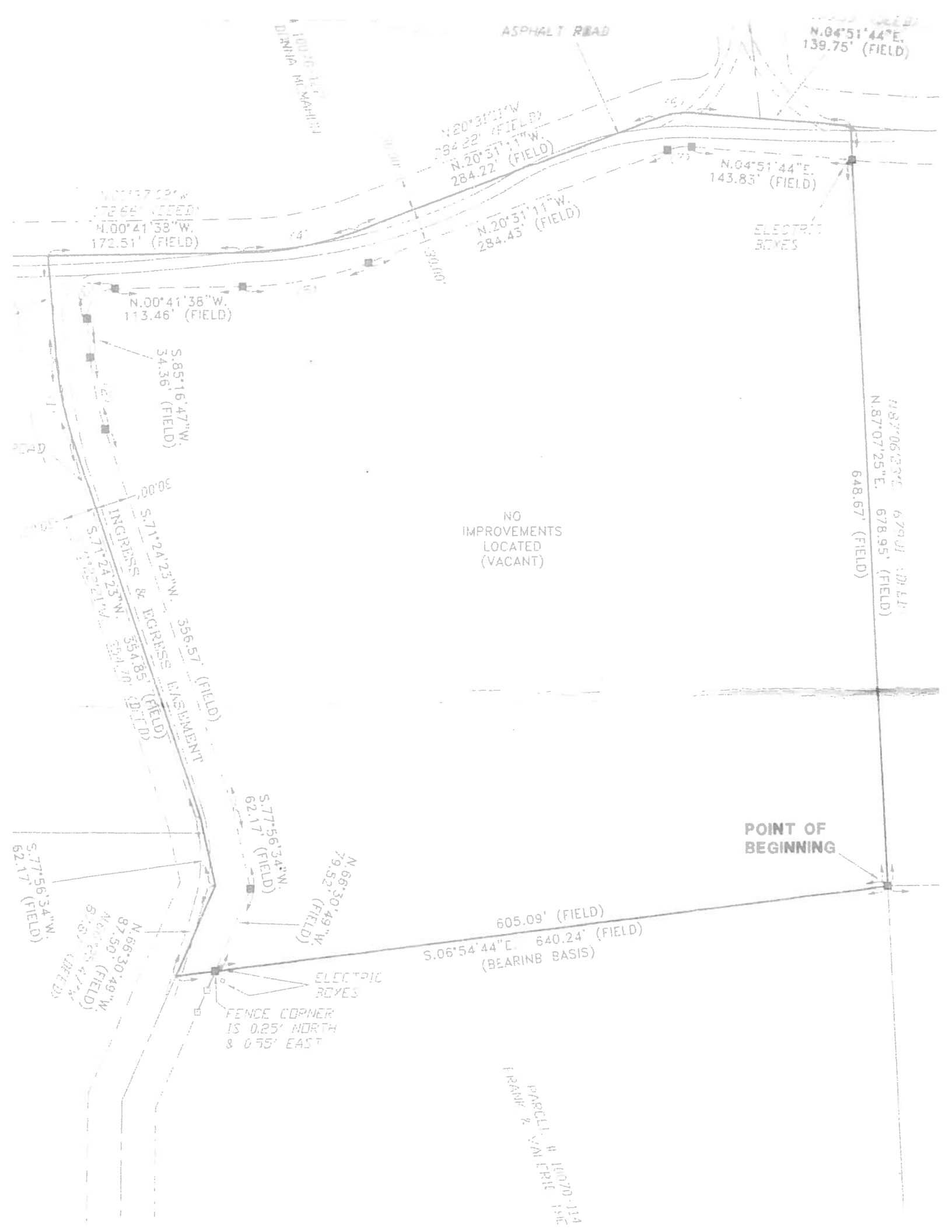
COMMENCE AT THE NE CORNER OF THE NW 1/4 OF SECTION 31, TOWNSHIP 7 SOUTH, RANGE 17 EAST, COLUMBIA COUNTY, FLORIDA AND RUN THENCE S.87 Degrees 14'55"W., ALONG THE NORTH LINE OF SAID SECTION 31, A DISTANCE OF 737.64 FEET TO A POINT ON THE SOUTHERLY RIGHT-OF-WAY LINE OF A COUNTY MAINTAINED GRADE ROAD, SAID POINT BEING THE POINT OF BEGINNING; THENCE S.01 Degrees 47'48"E., A DISTANCE OF 114.27 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT HAVING: A RADIUS OF 230.00 FEET, A CENTRAL ANGLE OF 55 Degrees 09'08", A TANGENT LENGTH OF 120.12 FEET, A CHORD BEARING OF S.29 Degrees 22'22"E., AND A CHORD LENGTH OF 212.95 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 221.39 FEET TO A POINT OF REVERSE CURVE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 170.00 FEET, A CENTRAL ANGLE OF 72 Degrees 58'42", A TANGENT LENGTH OF 125.74 FEET, A CHORD BEARING OF S.20 Degrees 27'35"E., AND A CHORD LENGTH OF 202.19 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 216.53 FEET; THENCE S.16 Degrees 01'48"W., A DISTANCE OF 282.20 FEET TO A POINT HEREINAFTER REFERRED TO AS POINT "A", SAID POINT ALSO BEING THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 300.00 FEET, A CENTRAL ANGLE OF 16 Degrees 57'20", A TANGENT LENGTH OF 44.72 FEET, A CHORD BEARING OF S.07 Degrees 33'06"W., AND A CHORD LENGTH OF 88.46 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 88.78 FEET; THENCE S.00 Degrees 55'34"E., A DISTANCE OF 223.29 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT HAVING: RADIUS OF 270.00 FEET, A CENTRAL ANGLE OF 09 Degrees 29'10", A TANGENT LENGTH OF 22.40 FEET, A CHORD BEARING OF S.05 Degrees 40'09"E., AND A CHORD LENGTH OF 44.65 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 44.70 FEET; THENCE S.10 Degrees 24'44"E., A DISTANCE OF 143.54 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 330.00 FEET, A CENTRAL ANGLE OF 15 Degrees 15'21", A TANGENT LENGTH OF 44.19 FEET, A CHORD BEARING OF S.02 Degrees 47'03"E., AND A CHORD LENGTH OF 87.61 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 87.87 FEET; THENCE S.04 Degrees 50'37"W., A DISTANCE OF 125.00 FEET TO A POINT HEREINAFTER REFERRED TO AS POINT "B"; THENCE CONTINUE S.04 Degrees 50'37"W., A DISTANCE OF 58.27 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 80.00 FEET, A CENTRAL ANGLE OF 25 Degrees 22'17", A TANGENT LENGTH OF 18.01 FEET, A CHORD BEARING OF S.07 Degrees 50'32"E., AND A CHORD LENGTH OF 35.14 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 35.43 FEET; THENCE S.20 Degrees 31'41"E., A DISTANCE OF 284.45 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 300.00 FEET, A CENTRAL ANGLE OF 19 Degrees 54'39", A TANGENT LENGTH OF 52.66 FEET, A CHORD BEARING OF S.10 Degrees 34'21"E., AND A CHORD LENGTH OF 103.73 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 104.25 FEET; THENCE S.00 Degrees 37'02"E., A DISTANCE OF 172.66 FEET TO A POINT HEREINAFTER REFERRED TO AS POINT "C"; THENCE CONTINUE S.00 Degrees 37'02"E., A DISTANCE OF 266.23 FEET TO THE POINT OF CURVATURE OF A CURVE, HAVING: A RADIUS OF 150.00 FEET, A CENTRAL ANGLE OF 47 Degrees 58'58", A TANGENT LENGTH OF 66.78 FEET, A CHORD BEARING OF S.23 Degrees 22'27"W., AND A CHORD LENGTH OF 121.98 FEET; THENCE ALONG THE ARC OF SAID CURVE AN ARC DISTANCE OF 125.62 FEET; THENCE S.47 Degrees 21'56"W., A DISTANCE OF 70.00 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 150.00 FEET, A CENTRAL

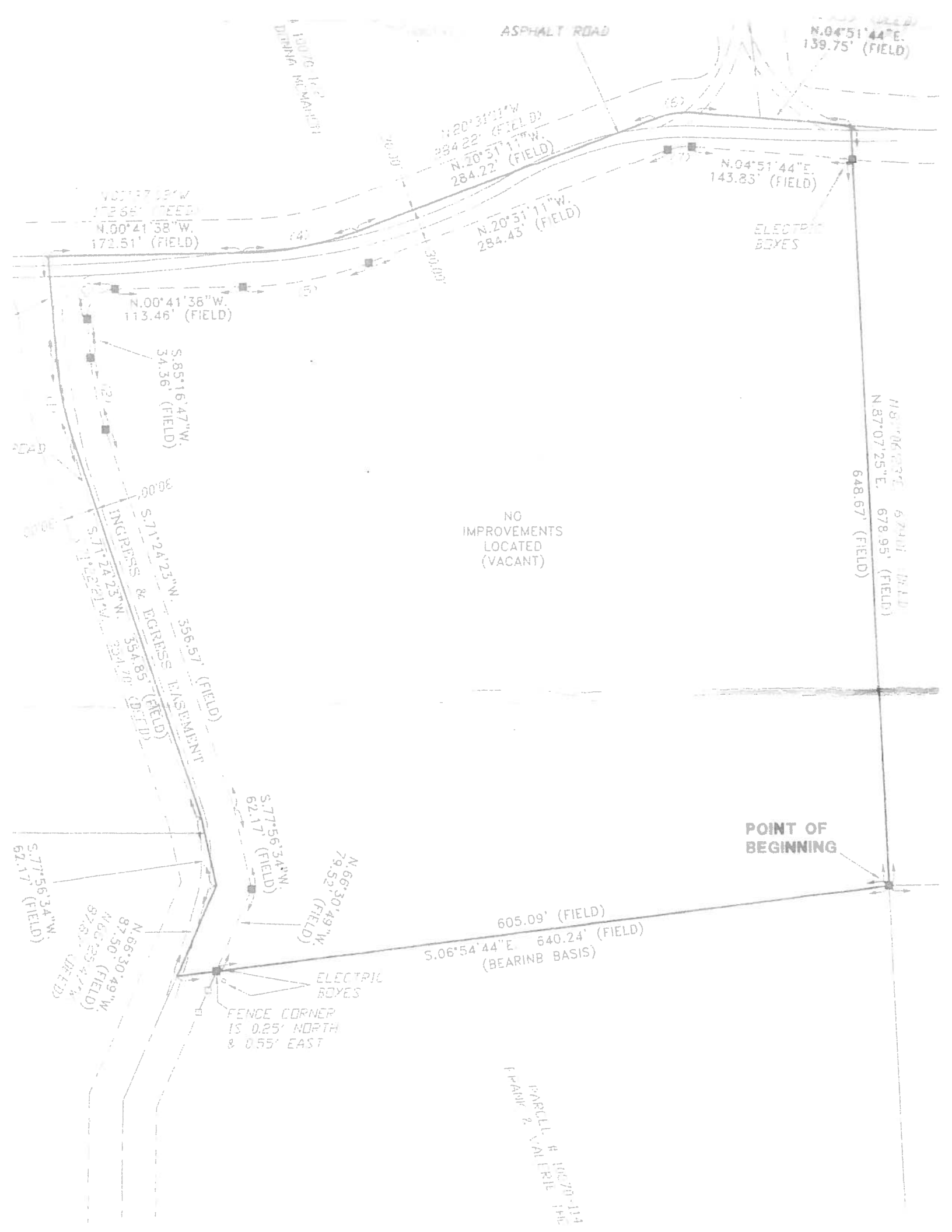
ANGLE OF 58 Degrees 35'13", A TANGENT LENGTH OF 84.15 FEET, A CHORD BEARING OF S.18 Degrees 04'19"W., AND A CHORD LENGTH OF 146.78 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 153.38 FEET; THENCE S.11 Degrees 13'17"E., A DISTANCE OF 205.89 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 150.00 FEET, A CENTRAL ANGLE OF 43 Degrees 09'40", A TANGENT LENGTH OF 59.33 FEET, A CHORD BEARING OF S.10 Degrees 21'33"W., AND A CHORD LENGTH OF 110.34 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 113.00 FEET; THENCE S.31 Degrees 56'23"W., A DISTANCE OF 183.67 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 330.00 FEET, A CENTRAL ANGLE OF 12 Degrees 59'20", A TANGENT LENGTH OF 37.57 FEET, A CHORD BEARING OF S.25 Degrees 26'43"W., AND A CHORD LENGTH OF 74.65 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 74.81 FEET TO A POINT HEREINAFTER REFERRED TO AS POINT "D", SAID POINT BEING A POINT OF TERMINATION OF SAID CENTERLINE. THENCE BEGIN AT THE AFOREMENTIONED POINT "A", AND RUN THENCE N.81 Degrees 32'45"E., A DISTANCE OF 701.09 FEET; THENCE N.87 Degrees 14'29"E., A DISTANCE OF 627.20 FEET TO THE RADIUS POINT OF A 25 FOOT CUL-DE-SAC, SAID POINT BEING A POINT OF TERMINATION OF SAID CENTERLINE. THENCE BEGIN AT A AFOREMENTIONED POINT "B", AND RUN THENCE S.84 Degrees 04'22"W., A DISTANCE OF 140.14 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 300.00 FEET, A CENTRAL ANGLE OF 29 Degrees 28'54", A TANGENT LENGTH OF 78.93 FEET, A CHORD BEARING OF N.81 Degrees 11'11"W., AND A CHORD LENGTH OF 152.67 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 154.37 FEET; THENCE N.66 Degrees 26'45"W., A DISTANCE OF 156.72 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 330.00 FEET, A CENTRAL ANGLE OF 26 Degrees 53'38", A TANGENT LENGTH OF 78.90 FEET, A CHORD BEARING OF N.79 Degrees 53'33"W., AND A CHORD LENGTH OF 153.48 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 154.90 FEET; THENCE S.86 Degrees 39'38"W., A DISTANCE OF 616.14 FEET TO A POINT HEREINAFTER REFERRED TO AS POINT "E"; THENCE N.44 Degrees 18'47"W., A DISTANCE OF 17.12 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 200.00 FEET, A CENTRAL ANGLE OF 42 Degrees 37'37", A TANGENT LENGTH OF 78.03 FEET, A CHORD BEARING OF N.22 Degrees 59'59"W., AND A CHORD LENGTH OF 145.39 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 148.80 FEET; THENCE N.01 Degrees 41'10"W., A DISTANCE OF 565.91 FEET TO THE RADIUS POINT OF A 25 FOOT CUL-DE-SAC, SAID POINT BEING A POINT OF TERMINATION OF SAID CENTERLINE; THENCE RETURNING TO THE AFOREMENTIONED POINT "E", RUN THENCE S.44 Degrees 18'47"E., A DISTANCE OF 46.97 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE RIGHT, HAVING: A RADIUS OF 105.00 FEET, A CENTRAL ANGLE OF 42 Degrees 33'27", A TANGENT LENGTH OF 40.89 FEET, A CHORD BEARING OF S.23 Degrees 02'04"E., AND A CHORD LENGTH OF 76.21 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 77.99 FEET; THENCE S.01 Degrees 45'20"E., A DISTANCE OF 1970.74 FEET TO THE RADIUS POINT OF A 25 FOOT CUL-DE-SAC, SAID POINT BEING A POINT OF TERMINATION OF SAID CENTERLINE. THENCE BEGIN AT THE AFOREMENTIONED POINT "C", AND RUN THENCE N.85 Degrees 13'54"E., A DISTANCE OF 93.53 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 300.00 FEET, A CENTRAL ANGLE OF 13 Degrees 51'33", A TANGENT LENGTH OF 36.46 FEET, A CHORD BEARING OF N.78 Degrees 18'07"E., AND A CHORD LENGTH OF 72.39 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC LENGTH OF 72.57 FEET; THENCE N.71 Degrees 22'21"E., A DISTANCE OF 354.70 FEET; THENCE N.77 Degrees 56'34"E., 62.17 FEET; THENCE S.66 Degrees 25'47"E., A DISTANCE OF 207.86 FEET; THENCE S.89 Degrees 03'47"E., 142.06 FEET; THENCE N.87 Degrees 13'37"E., A DISTANCE OF 301.36 FEET TO THE RADIUS POINT OF A 25 FOOT CUL-DE-SAC, SAID POINT BEING A POINT OF TERMINATION OF SAID CENTERLINE. THENCE BEGIN AT THE AFOREMENTIONED POINT "D" AND RUN THENCE N.71 Degrees 02'57"W., A DISTANCE OF 6.24 FEET; THENCE S.19 Degrees 16'58"W., A DISTANCE OF 21.70 FEET; THENCE S.72 Degrees 18'51"E., A DISTANCE OF 780.47 FEET TO THE POINT OF CURVATURE OF A CURVE TO THE LEFT, HAVING: A RADIUS OF 500.00 FEET, A CENTRAL ANGLE OF 20 Degrees 27'26", AND A TANGENT LENGTH OF 90.22 FEET, A CHORD BEARING OF S.82 Degrees 32'34"E., AND A CHORD LENGTH OF 177.58 FEET; THENCE ALONG THE ARC OF SAID CURVE, AN ARC DISTANCE OF 178.52 FEET; THENCE N.87 Degrees 13'43"E., A DISTANCE OF 541.28 FEET TO THE RADIUS POINT OF A 25 FOOT CUL-DE-SAC, SAID POINT BEING THE POINT OF TERMINATION OF SAID CENTERLINE.

ALSO:

AN EASEMENT OVER AND ACROSS THE FOLLOWING DESCRIBED PARCEL:

BEGIN AT THE AFOREMENTIONED POINT "D" AND RUN THENCE N.71 Degrees 02'57"W., A DISTANCE OF 26.24 FEET; THENCE S.19 Degrees 16'56"W., A DISTANCE OF 1823.87 FEET; THENCE S.70 Degrees 43'02"E., A DISTANCE OF 64.05 FEET; THENCE N.27 Degrees 25'42"E., A DISTANCE OF 40.56 FEET; THENCE N.18 Degrees 54'40"E., A DISTANCE OF 631.89 FEET; THENCE N.70 Degrees 43'02"W., A DISTANCE OF 25.70 FEET; THENCE N.19 Degrees 16'58"E., A DISTANCE OF 1152.07 FEET; THENCE N.71 Degrees 02'57"W., A DISTANCE OF 13.76 FEET TO THE POINT OF BEGINNING.

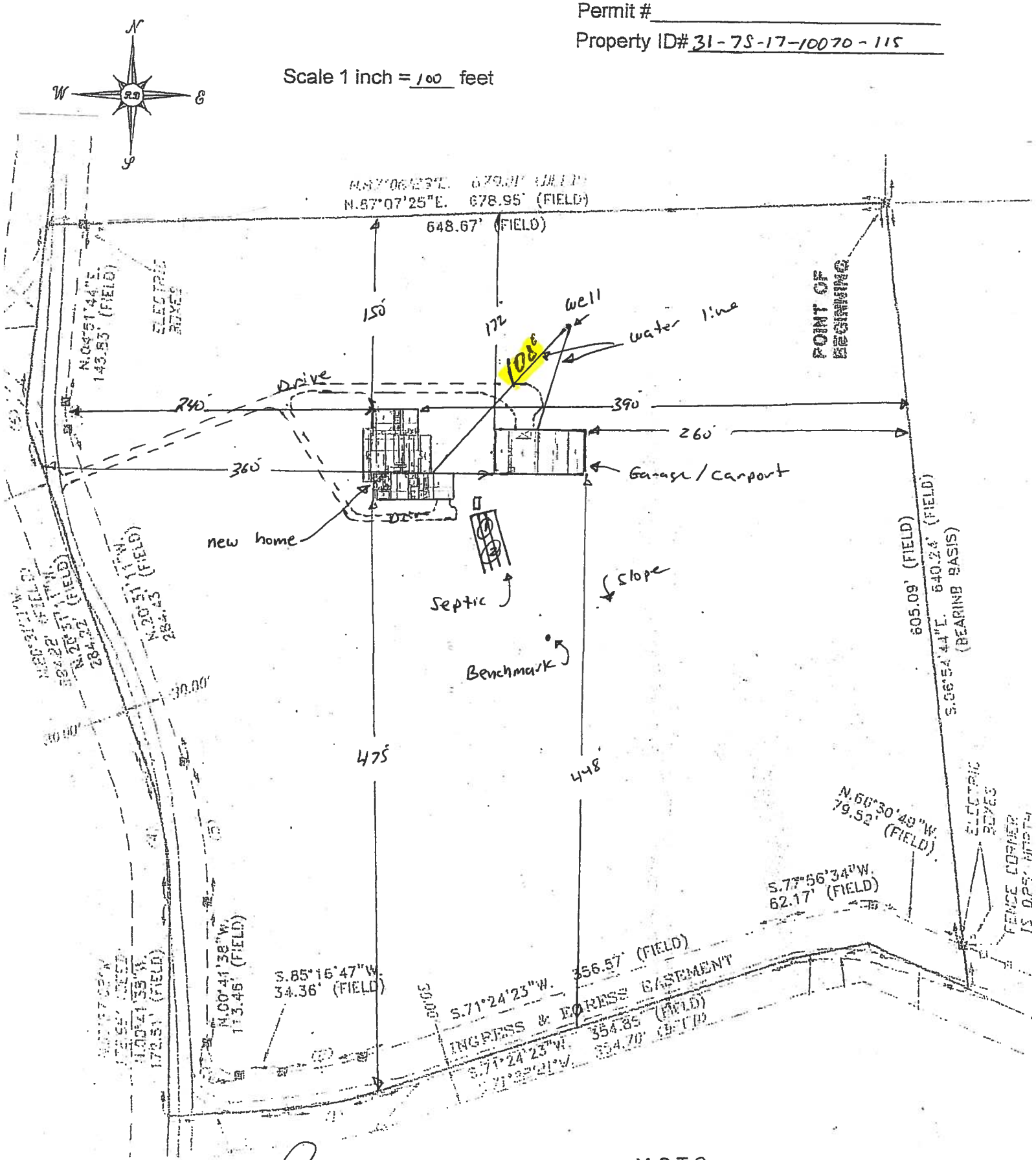

ASPHALT ROAD
N.04°51'44"E.
139.75' (FIELD)
N.20°31'11"W.
284.22' (FIELD)
N.20°31'11"W.
284.22' (FIELD)
N.04°51'44"E.
143.85' (FIELD)
N.00°41'38"W.
172.51' (FIELD)
N.20°31'11"W.
284.45' (FIELD)
ELECTRIC BOXES
N.00°41'38"W.
113.46' (FIELD)
S.85°16'47"W.
34.36' (FIELD)
N.87°07'25"E. 678.95' (FIELD)
648.67' (FIELD)
NO
IMPROVEMENTS
LOCATED
(VACANT)
S.71°24'23"W. 356.57' (FIELD)
INGRESS & EGRESS EASEMENT
S.77°56'34"W.
62.17' (FIELD)
N.69°30'49"W.
79.52' (FIELD)
POINT OF
BEGINNING
605.09' (FIELD)
S.06°54'44"E. 640.24' (FIELD)
(BEARINB BASIS)
ELECTRIC BOXES
FENCE CORNER
IS 0.25' NORTH
& 0.55' EAST


ASPHALT ROAD
N.04°51'44"E.
139.75' (FIELD)
N.20°31'11"W.
284.22' (FIELD)
N.20°31'11"W.
284.22' (FIELD)
N.04°51'44"E.
143.83' (FIELD)
N.20°31'11"W.
284.43' (FIELD)
ELECTRIC
BOXES
N.00°41'38"W.
172.51' (FIELD)
N.00°41'38"W.
113.46' (FIELD)
S.85°16'47"W.
34.36' (FIELD)
30.00'
S.71°24'23"W. 356.57' (FIELD)
S.71°24'23"W. 354.85' (FIELD)
INGRESS & EGRESS EASEMENT
N.87°07'25"E. 678.95' (FIELD)
648.67' (FIELD)
NO
IMPROVEMENTS
LOCATED
(VACANT)
S.77°56'34"W.
62.17' (FIELD)
S.77°56'34"W.
62.17' (FIELD)
N.69°30'49"W.
87.52' (FIELD)
N.69°30'49"W.
87.52' (FIELD)
POINT OF
BEGINNING
605.09' (FIELD)
S.06°54'44"E. 640.24' (FIELD)
(BEARINB BASIS)
ELECTRIC
BOXES
FENCE CORNER
IS 0.25' NORTH
& 0.55' EAST

Permit #_____

Property ID# 31-7S-17-10070-115

Scale 1 inch = 100 feet



Site Plan submitted by Ron Moore M.S.T.C.

Plan Approved_____ Not Approved_____ Date_____

By _____ County Health Department

WATER TO SEPTIC 105

E-mail to (sjh0330@gmail.com)

STATE OF FLORIDA
DEPARTMENT OF HEALTH
ONSITE SEWAGE TREATMENT AND DISPOSAL SYSTEM
APPLICATION FOR CONSTRUCTION PERMIT

PERMIT NO. 19-0599
DATE PAID: 8/12/19
FEE PAID: $310.00
RECEIPT #: 1423113

APPLICATION FOR:
[X] New System [ ] Existing System [ ] Holding Tank [ ] Innovative
[ ] Repair [ ] Abandonment [ ] Temporary [ ] _____

APPLICANT: Mike & Brenda Hartzog

AGENT: Ronnie Moore TELEPHONE: 352-246-3997

MAILING ADDRESS: PO Box 158 FT white FL 32038

TO BE COMPLETED BY APPLICANT OR APPLICANT'S AUTHORIZED AGENT. SYSTEMS MUST BE CONSTRUCTED BY A PERSON LICENSED PURSUANT TO 489.105(3)(m) OR 489.552, FLORIDA STATUTES. IT IS THE APPLICANT'S RESPONSIBILITY TO PROVIDE DOCUMENTATION OF THE DATE THE LOT WAS CREATED OR PLATTED (MM/DD/YY) IF REQUESTING CONSIDERATION OF STATUTORY GRANDFATHER PROVISIONS.

PROPERTY INFORMATION

LOT: 15 BLOCK: _____ SUBDIVISION: Bluebird Landing unrecorded PLATTED: 2002

PROPERTY ID #: 31-7S-17-10070-115 ZONING: SF I/M OR EQUIVALENT: [Y / (N)]

PROPERTY SIZE: 10.06 ACRES WATER SUPPLY: [X] PRIVATE PUBLIC [ ]<=2000GPD [ ]>2000GPD

IS SEWER AVAILABLE AS PER 381.0065, FS? [Y / (N)] DISTANCE TO SEWER: N/A FT

PROPERTY ADDRESS: 100 Oak GLN FT White FL 32038

DIRECTIONS TO PROPERTY: 47 south to 27 turn left to Mapleton ST turn right to Woodland Ave turn left to 100 Oak Gln on left.

BUILDING INFORMATION [X] RESIDENTIAL [ ] COMMERCIAL

Unit No	Type of Establishment	No. of Bedrooms	Building Area Sqft	Commercial/Institutional System Design Table 1, Chapter 64E-6, FAC
1	single family	4	2800	
2	garage/carport	0	25 sf bath room	
3			2825	
4				

[ ] Floor/Equipment Drains [ ] Other (Specify) _____

SIGNATURE: Ron Moore DATE: 08/09/19

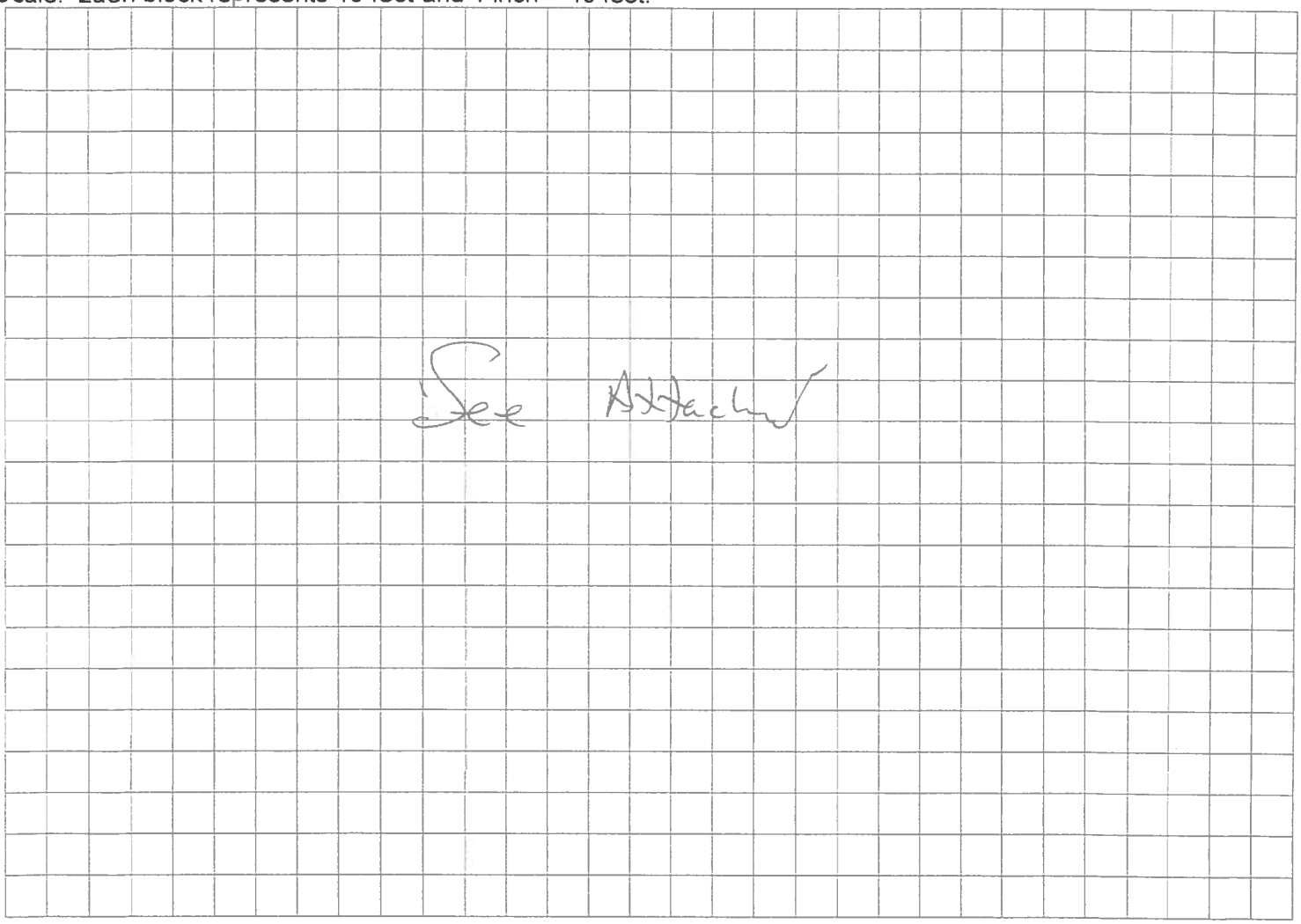
DH 4015, 08/09. (Obsoletes previous editions which may not be used)
Incorporated 64E-6.001, FAC

STATE OF FLORIDA
DEPARTMENT OF HEALTH
APPLICATION FOR CONSTRUCTION PERMIT

Permit Application Number 19-0999

---------- PART II - SITEPLAN ----------

Scale: Each block represents 10 feet and 1 inch = 40 feet.



Notes: _____

Site Plan submitted by: _____ M.S.T.C.

Plan Approved X Not Approved _____ Date 8/14/19

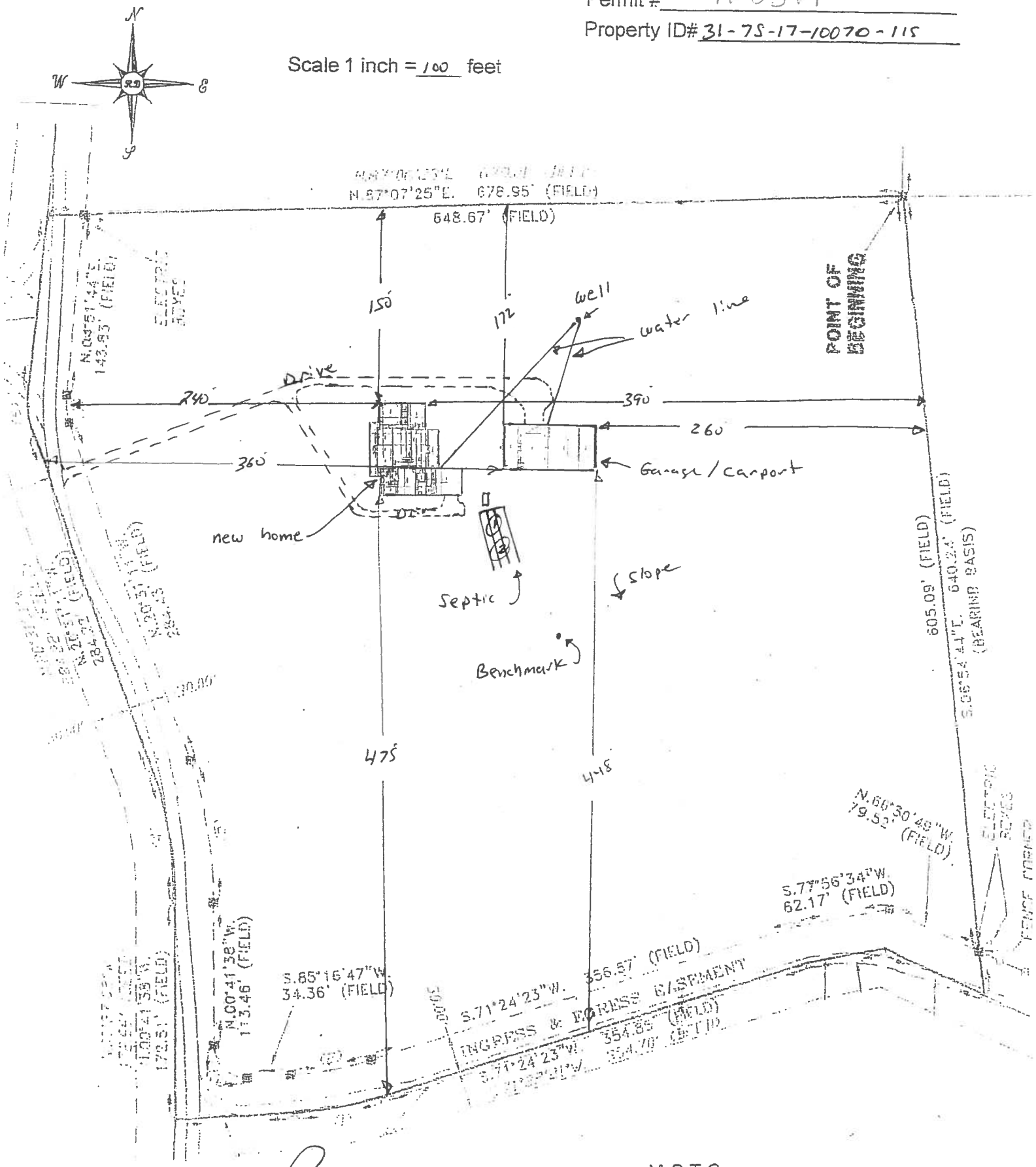
By _____ Columbia CHD County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

Permit # 19-0599

Property ID# 31-7S-17-10070-115

Scale 1 inch = 100 feet



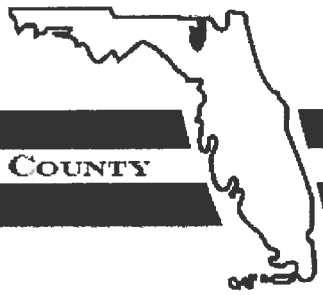
Site Plan submitted by Ron Moore M.S.T.C.

Plan Approved _____ Not Approved _____ Date _____

By _____ County Health Department

District No. 1 - Ronald Williams
District No. 2 - Rocky Ford
District No. 3 - Bucky Nash
District No. 4 - Toby Witt
District No. 5 - Tim Murphy

BOARD OF COUNTY COMMISSIONERS • COLUMBIA COUNTY



Address Assignment and Maintenance Document

To maintain the county wide Addressing Policy you must make application for a 9-1-1 Address at the time you apply for a building permit. The established standards for addressing and posting numbers to all principal buildings, dwellings, businesses and industries are contained in Columbia County Ordinance 2001-9. The addressing system is to enable Emergency Services Agencies to locate you in an emergency, and to assist the United States Postal Service and the public in the timely and efficient provision of services to residents and businesses of Columbia County

Date/Time Issued:	8/14/2019 7:25:44 PM
Address:	1167 SW WOODLAND Ave
City:	FORT WHITE
State:	FL
Zip Code	32038
Parcel ID	10070-115

REMARKS: Address for proposed structure on parcel.

NOTICE: THIS ADDRESS WAS ISSUED BASED ON LOCATION AND ACCESS INFORMATION RECEIVED FROM THE REQUESTER. SHOULD, AT A LATER DATE, THE LOCATION AND/OR ACCESS INFORMATION BE FOUND TO BE IN ERROR OR CHANGED, THIS ADDRESS IS SUBJECT TO CHANGE.

Address Issued By: **Signed:/ Matt Crews**

Columbia County GIS/911 Addressing Coordinator

COLUMBIA COUNTY
911 ADDRESSING / GIS DEPARTMENT
263 NW Lake City Ave., Lake City, FL 32055 Telephone: (386) 758-1125
Email: gis@columbiacountyfla.com

BRIAN MC ADAMS

COLUMBIA COUNTY BUILDING DEPARTMENT
RESIDENTIAL CHECK LIST

MINIMUM PLAN REQUIREMENTS: FLORIDA BUILDING CODE RESIDENTIAL 2014 EFFECTIVE 1 JULY 2015 AND THE NATIONAL ELECTRICAL CODE 2011 EFFECTIVE 1 JULY 2015

ALL REQUIREMENTS ARE SUBJECT TO CHANGE

ALL BUILDING PLANS MUST INDICATE COMPLIANCE WITH THE CURRENT 2014 FLORIDA BUILDING CODES RESIDENTIAL, EFFECTIVE 1 JULY 2015. NATIONAL ELECTRICAL CODE 2011 EFFECTIVE 1 JULY 2015. ALL PLANS OR DRAWINGS SHALL PROVIDE CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS.

FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEEDS ARE PER FLORIDA BUILDING CODE FIGURE 1609-A THROUGH 1609-C ULTIMATE DESIGN WIND SPEEDS FOR RISK CATEGORY AND BUILDINGS AND OTHER STRUCTURES
Revised 12/2016

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Marked as Applicable
		Select From the Dropbox
1	Two (2) complete sets of plans containing the following:	- YES
2	All drawings must be clear, concise, drawn to scale, details that are not used shall be marked void	- YES
3	Condition space (Sq. Ft.) 2800 Total (Sq. Ft.) under roof 3969	YES / NO / N/A

Designers name and signature shall be on all documents and a licensed architect or engineer, signature and official embossed seal shall be affixed to the plans and documents as per the FLORIDA BUILDING CODES RESIDENTIAL R101.2.1

Site Plan information including:

4	Dimensions of lot or parcel of land	- YES
5	Dimensions of all building set backs	- YES
6	Location of all other structures (include square footage of structures) on parcel, existing or proposed well and septic tank and all utility easements.	- YES
7	Provide a full legal description of property.	- YES

Wind-load Engineering Summary, calculations and any details are required.

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Marked as Applicable
8	**Plans or specifications must show compliance with FBCR Chapter 3**	YES / NO / N/A
		Select From the Dropbox
9	Basic wind speed (3-second gust), miles per hour	-
10	(Wind exposure – if more than one wind exposure is used, the wind exposure and applicable wind direction shall be indicated)	-
11	Wind importance factor and nature of occupancy	-
12	The applicable internal pressure coefficient, Components and Cladding	-
13	The design wind pressure in terms of psf (kN/m²), to be used for the design of exterior component, cladding materials not specifally designed by the registered design professional.	-

Elevations Drawing including:

14	All side views of the structure	- ✓
15	Roof pitch	- ✓
16	Overhang dimensions and detail with attic ventilation	- ✓
17	Location, size and height above roof of chimneys	- ✓
18	Location and size of skylights with Florida Product Approval	- ✓
18	Number of stories	- ✓
20A	Building height from the established grade to the roofs highest peak	- ✓

Floor Plan including:

20	Dimensioned area plan showing rooms, attached garage, breeze ways, covered porches, deck, balconies	- ✓
21	Raised floor surfaces located more than 30 inches above the floor or grade	- N/A
22	All exterior and interior shear walls indicated	- ✓
23	Shear wall opening shown (Windows, Doors and Garage doors)	-
24	Show compliance with Section FBCR 310 Emergency escape and rescue opening shown in each bedroom (net clear opening shown) and Show compliance with Section FBC 1405.13.2 where the opening of an operable window is located more than 72 inches above the finished grade or surface below, the lowest part of the clear opening of the window shall be a minimum of 24 inches above the finished floor of the room in which the window is located. Glazing between the floor and 24 inches shall be fixed or have openings through which a 4-inch-diameter sphere cannot pass.	- ✓
25	Safety glazing of glass where needed	- ✓
26	Fireplaces types (gas appliance) (vented or non-vented) or wood burning with Hearth (see chapter 10 and chapter 24 of FBCR)	-
27	Show stairs with dimensions (width, tread and riser and total run) details of guardrails, Handrails	- ✓
28	Identify accessibility of bathroom (see FBCR SECTION 320)	- ✓

All materials placed within opening or onto/into exterior walls, soffits or roofs shall have Florida product approval number and mfg. installation information submitted with the plans (see Florida product approval form)

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Marked as Applicable YES / NO N A Select From the Dropbox

FBCR 403: Foundation Plans

29	Location of all load-bearing walls footings indicated as standard, monolithic, dimensions, size and type of reinforcing.	- ✓
30	All posts and/or column footing including size and reinforcing	- ✓
31	Any special support required by soil analysis such as piling.	- ✓
32	Assumed load-bearing valve of soil _____ Pound Per Square Foot	-
33	Location of horizontal and vertical steel, for foundation or walls (include # size and type) **For** structures with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an grounding electrode system. Per the National Electrical Code article 250.52.3	- ✓

FBCR 506: CONCRETE SLAB ON GRADE

34	Show Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)	- ✓
35	Show control joints, synthetic fiber reinforcement or welded fire fabric reinforcement and Supports	- ✓

FBCR 318: PROTECTION AGAINST TERMITES

36	Indicate on the foundation plan if soil treatment is used for subterranean termite prevention or Submit other approved termite protection methods. **Protection shall be provided by registered termiticides**	- ✓

FBCR 606: Masonry Walls and Stem walls (load bearing & shear Walls)

37	Show all materials making up walls, wall height, and Block size, mortar type	-
38	Show all Lintel sizes, type, spans and tie-beam sizes and spacing of reinforcement	-

Metal frame shear wall and roof systems shall be designed, signed and sealed by Florida Prof. Engineer or Architect

Floor Framing System: First and/or second story

39	Floor truss package shall including layout and details, signed and sealed by Florida Registered Professional Engineer	- ✓

40	Show conventional floor joist type, size, span, spacing and attachment to load bearing walls, stem walls and/or priers	-
41	Girder type, size and spacing to load bearing walls, stem wall and/or priers	-
42	Attachment of joist to girder	-
43	Wind load requirements where applicable	-
44	Show required under-floor crawl space	-
45	Show required amount of ventilation opening for under-floor spaces	-
46	Show required covering of ventilation opening	-
47	Show the required access opening to access to under-floor spaces	-
48	Show the sub-floor structural panel sheathing type, thickness and fastener schedule on the edges & intermediate of the areas structural panel sheathing	-
49	Show Draftstopping, Fire caulking and Fire blocking	-
50	Show fireproofing requirements for garages attached to living spaces, per FBCR section 302.6	-
51	Provide live and dead load rating of floor framing systems (psf).	-

FBCR CHAPTER 6 WOOD WALL FRAMING CONSTRUCTION

YES / NO / N/A

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL		Items to Include- Each Box shall be Marked as Applicable Select From the Dropbox
52	Stud type, grade, size, wall height and oc spacing for all load bearing or shear walls	-
53	Fastener schedule for structural members per table IRC 602.3 are to be shown	-
54	Show Wood structural panel's sheathing attachment to studs, joist, trusses, rafters and structural members, showing fastener schedule attachment on the edges & intermediate of the areas structural panel sheathing	-
55	Show all required connectors with a max uplift rating and required number of connectors and oc spacing for continuous connection of structural walls to foundation and roof trusses or rafter systems	-
56	Show sizes, type, span lengths and required number of support jack studs, king studs for shear wall opening and girder or header per IRC Table 502.5 (1)	-
57	Indicate where pressure treated wood will be placed	-
58	Show all wall structural panel sheathing, grade, thickness and show fastener schedule for structural panel sheathing edges & intermediate areas	-
59	A detail showing gable truss bracing, wall balloon framing details or/ and wall hinge bracing detail	-

FBCR :ROOF SYSTEMS:

60	Truss design drawing shall meet section FBCR 802.1.6.1 Wood trusses	-
61	Include a layout and truss details, signed and sealed by Florida Professional Engineer	-
62	Show types of connector's assemblies' and resistance uplift rating for all trusses and rafters	-
63	Show gable ends with rake beams showing reinforcement or gable truss and wall bracing details	-
64	Provide dead load rating of trusses	-

FBCR 802:Conventional Roof Framing Layout

65	Rafter and ridge beams sizes, span, species and spacing	-
66	Connectors to wall assemblies' include assemblies' resistance to uplift rating	-
67	Valley framing and support details	-
68	Provide dead load rating of rafter system	-

FBCR 803 ROOF SHEATHING

69	Include all materials which will make up the roof decking, identification of structural panel sheathing, grade, thickness	-
70	Show fastener Size and schedule for structural panel sheathing on the edges & intermediate areas	-

ROOF ASSEMBLIES FRC Chapter 9

71	Include all materials which will make up the roof assembles covering	-
72	Submit Florida Product Approval numbers for each component of the roof assembles covering	-

FBCR Chapter 11 Energy Efficiency Code for residential building

Residential construction shall comply with this code by using the following compliance methods in the FBCR chapter11 Residential buildings compliance methods. **Two of the required forms are to be submitted**, *N1100.1.1.1 As an alternative to the computerized Compliance Method A, the Alternate Residential Point System Method hand calculation, Alternate Form 600A, may be used. All requirements specific to this calculation are located in Sub appendix C to Appendix G. Buildings complying by this alternative shall meet all mandatory requirements of this chapter. Computerized versions of the Alternate Residential Point System Method shall not be acceptable for code compliance.*

YES / NO / N/A

	GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Marked as Applicable Select From the Dropbox
73	Show the insulation R value for the following areas of the structure	-
74	Attic space	-
75	Exterior wall cavity	-
76	Crawl space	-
	HVAC information	
77	Submit two copies of a Manual J sizing equipment or equivalent computation study	-
78	Exhaust fans shown in bathrooms **Mechanical exhaust capacity of 50 cfm intermittent or 20 cfm continuous required**	-
79	Show clothes dryer route and total run of exhaust duct	-
	Plumbing Fixture layout shown	
80	All fixtures waste water lines shall be shown on the foundation plan	-
81	Show the location of water heater	-
	Private Potable Water	
82	Pump motor horse power	-
83	Reservoir pressure tank gallon capacity	-
84	Rating of cycle stop valve if used	-
	Electrical layout shown including	
85	Show Switches, receptacles outlets, lighting fixtures and Ceiling fans	-
86	Show all 120-volt, single phase, 15- and 20-ampere branch circuits outlets required to be protected by **Ground-Fault Circuit Interrupter (GFCI) Article 210.8 A**	-
87	Show the location of smoke detectors & Carbon monoxide detectors	-
88	Show service panel, sub-panel, location(s) and total ampere ratings	-
89	On the electrical plans identify the electrical service overcurrent protection device for the main electrical service. This device shall be installed on the exterior of structures to serve as a disconnecting means for the utility company electrical service. Conductors used from the exterior disconnecting means to a panel or sub panel shall have four-wire conductors, of which one conductor shall be used as an equipment ground. Indicate if the utility company service entrance cable will be of the overhead or underground type. **For structures** with foundation which establish new electrical utility companies service connection a Concrete Encased Electrode will be required within the foundation to serve as an Grounding electrode system. Per the National Electrical Code article 250.52.3	-
90	Appliances and HVAC equipment and disconnects	-
91	Show all 120-volt, single phase, 15- and 20-ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, or similar rooms or areas shall be protected by a listed **Combination arc-fault circuit interrupter,** Protection device.	-

GENERAL REQUIREMENTS: APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL	Items to Include- Each Box shall be Circled as Applicable

THE FOLLOWING ITEMS MUST BE SUBMITTED WITH BUILDING PLANS

		YES	NO	N/A
92	**Building Permit Application** A current Building Permit Application is to be completed, by following the Checklist all supporting documents must be submitted. There is a **$15.00** application fee. The completed application with attached documents and application fee can be mailed.	NO		
93	**Parcel Number** The parcel number (Tax ID number) from the Property Appraisers Office (386) 758-1083 is required. A copy of property deed is also required. www.columbiacountyfla.com	NO		
94	**Town of Fort White** (386) 497-2321 If the parcel in the application for building permit is within the Corporate city limits of Fort White, an approval land use development letter issued by the Town of Fort is required to be submitted with the application for a building permit.	NO		
***	**BELOW ITEMS ONLY NEEDED AFTER ZONING APPROVAL HAS GIVEN.**	****	***	***
95	**Environmental Health Permit or Sewer Tap Approval** A copy of a approved Columbia County Environmental Health (386) 758-1058	NO		
96	**City of Lake City** A City Water and/or Sewer letter. Call 386-752-2031	NO		
97	**Flood Information:** All projects within the Floodway of the Suwannee or Santa Fe Rivers shall require permitting through the Suwannee River Water Management District, before submitting a application to this office. Any project located within a flood zone where the base flood elevation (100 year flood) has been established shall meet the requirements of Section 8.5.2 of the Columbia County Land Development Regulations. Any project located within a flood zone where the base flood elevation has not been established (Zone A) shall meet the requirements of Section 8.5.3 of the Columbia County Land Development Regulations	NO		
98	**CERTIFIED FINISHED FLOOR ELEVATIONS** will be required on any project where the approved FIRM Flood Maps show the property is in a AE, Floodway, and AH flood zones. Additionally One Foot Rise letters are required for AE and AH zones. In the Floodway Flood zones a Zero Rise letter is required.			
99	A Flood development permit is also required for AE, Floodway & AH. Development permit cost is **$50.00**			
100	**Driveway Connection:** If the property does not have an existing access to a public road, then an application for a culvert permit (**$25.00**) must be made. County Public Works Dept. determines the size and length of every culvert before instillation and completes a final inspection before permanent power is granted. If the applicant feels that a culvert is not needed, they may apply for a culvert waiver (**$50.00**) Separate Check when issued. If the project is to be located on an F.D.O.T. maintained road, then an F.D.O.T. access permit is required.	NO		
101	**911 Address:** An application for a 911address must be applied for and **received** through the Columbia County Emergency Management Office of 911 Addressing Department (386) 758-1125.	NO		

TOILET FACILITIES SHALL BE PROVIDED FOR ALL CONSTRUCTION SITES. NO

Disclosure Statement for Owner Builders *If you as the applicant will be acting as an owner/builder under section 489.103(7) of the Florida Statutes, submit the required owner builder disclosure statement form.*

Notice Of Commencement

A notice of commencement form **recorded** in the Columbia County Clerk Office is required to be filed with the building department Before Any Inspections can be preformed.

Section R101.2.1 of the Florida Building Code Residential:

The provisions of Chapter 1, Florida Building Code shall govern the administration and enforcement of the Florida Building Code, Residential.

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Plastpro	Fiberglass door	FL-14347.9
B. SLIDING			
C. SECTIONAL/ROLL UP	Overhead Door	Sectional	FL14170
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MT	vinyl window	FL 17676
B. HORIZONTAL SLIDER			
C. CASEMENT			
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	Cemplank	Primed Hardi siding board	FL 13192.1
B. SOFFITS	ACM	Aluminum Soffit	FL 2416.1
C. STOREFRONTS			
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS			
A. ASPHALT SHINGLES	~~Tamko~~	~~Heritage 30 AR~~	~~FL 18355-R-4~~
B. NON-STRUCT METAL	Tri County Metal	26 ga Rib Metal	FL 4595.16 R4
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER	Trinity ERD	Rhino Roof underlayment	FL 15216
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS			
B. WOOD ANCHORS			
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR ENVELOPE PRODUCTS			

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements.
Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

NOTES: _____

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

RE: S1073 - HARTZOG RES. / SKIP HARVEY

MiTek USA, Inc.
6904 Parke East Blvd.
Tampa, FL 33610-4115

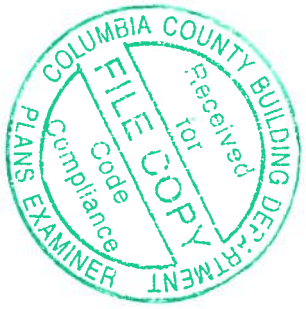
Site Information:
Customer Info: 84 LUMBER / HARTZOG RES. Project Name: - Model: -
Lot/Block: - Subdivision: -
Address: -, -
City: - State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building.
Name: License #:
Address:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):
Design Code: FRC2017/TPI2014 Design Program: MiTek 20/20 8.2
Wind Code: ASCE 7-10 Wind Speed: 140 mph
Roof Load: 37.0 psf Floor Load: N/A psf

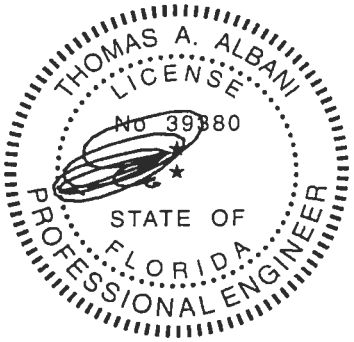
This package includes 25 individual, Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T17808564	CJ1	8/8/19	23	T17808586	V3	8/8/19
2	T17808565	EJ5	8/8/19	24	T17808587	V4	8/8/19
3	T17808566	SJ1	8/8/19	25	T17808588	V5	8/8/19
4	T17808567	SJ3	8/8/19				
5	T17808568	T1	8/8/19				
6	T17808569	T2	8/8/19				
7	T17808570	T3	8/8/19				
8	T17808571	T4	8/8/19				
9	T17808572	T5	8/8/19				
10	T17808573	T6	8/8/19				
11	T17808574	T7	8/8/19				
12	T17808575	T8	8/8/19				
13	T17808576	T9	8/8/19				
14	T17808577	T10	8/8/19				
15	T17808578	T11	8/8/19				
16	T17808579	T12	8/8/19				
17	T17808580	T13	8/8/19				
18	T17808581	T14	8/8/19				
19	T17808582	T15	8/8/19				
20	T17808583	T16	8/8/19				
21	T17808584	V1	8/8/19				
22	T17808585	V2	8/8/19				



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Duley Truss.

Truss Design Engineer's Name: Albani, Thomas
My license renewal date for the state of Florida is February 28, 2021.



Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:
August 8,2019

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808564
S1073	CJ1	Diagonal Hip Girder	2	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:01 2019 Page 1
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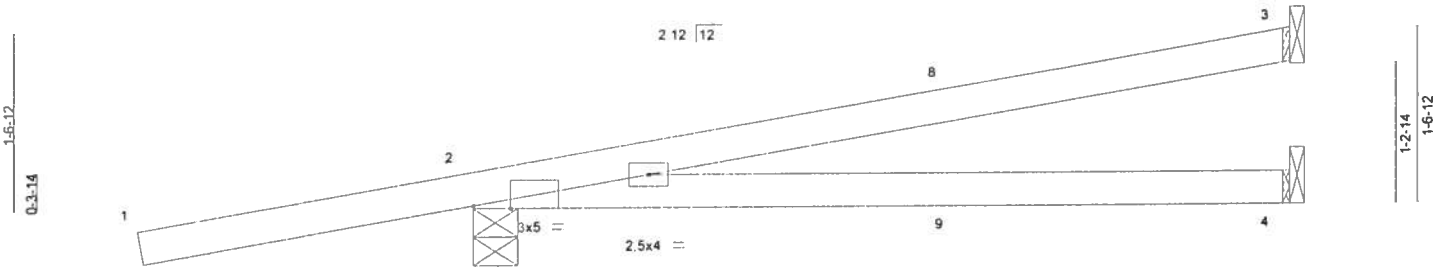

7-0-2 / 7-0-2

Plate Offsets (X,Y)-- [2:0-3-13,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.68	Vert(LL)	-0.21	4-7	>401	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.52	Vert(CT)	-0.26	4-7	>326	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MP							Weight: 25 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=135/Mechanical, 2=348/0-4-9, 4=65/Mechanical
Max Horz 2=120(LC 20)
Max Uplift 3=-130(LC 4), 2=-459(LC 4)
Max Grav 3=155(LC 28), 2=542(LC 28), 4=115(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=130, 2=459.
6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 28 lb down and 35 lb up at 4-1-14, and 28 lb down and 35 lb up at 4-1-14 on top chord, and 176 lb down and 156 lb up at 1-3-15, 176 lb down and 156 lb up at 1-3-15, and 21 lb down at 4-1-14, and 21 lb down at 4-1-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 4-5=-20
Concentrated Loads (lb)
Vert: 7=121(F=61, B=61) 9=-1(F=-0, B=-0)

Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date: August 8,2019

Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808565
S1073	EJ5	Jack-Open	4	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:02 2019 Page 1
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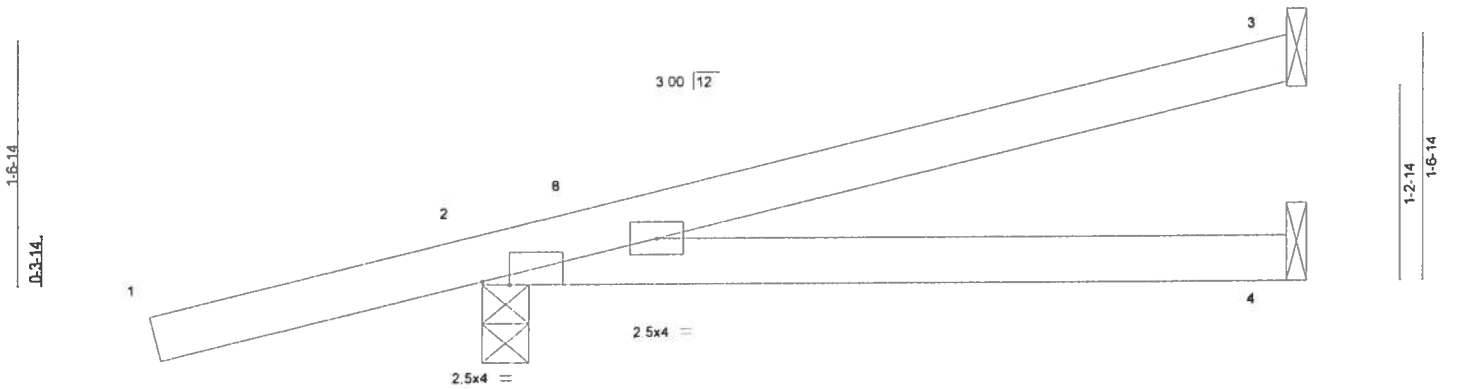

5-0-0 / 5-0-0

Plate Offsets (X,Y)-- [2:0-2-0,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.32	Vert(LL)	0.04	4-7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.23	Vert(CT)	-0.04	4-7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MP							Weight: 18 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=105/Mechanical, 2=315/0-3-8, 4=55/Mechanical
Max Horz 2=103(LC 8)
Max Uplift 3=-72(LC 8), 2=-239(LC 8)
Max Grav 3=105(LC 1), 2=315(LC 1), 4=83(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -2-0-7 to 1-1-0, Interior(1) 1-1-0 to 4-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=239.

Thomas A. Albani PE No.39380
MiTek USA, Inc. FL Cert 6634
6904 Parke East Blvd. Tampa FL 33610
Date:

August 8,2019

Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808566
S1073	SJ1	Jack-Open	4	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8 240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:03 2019 Page 1
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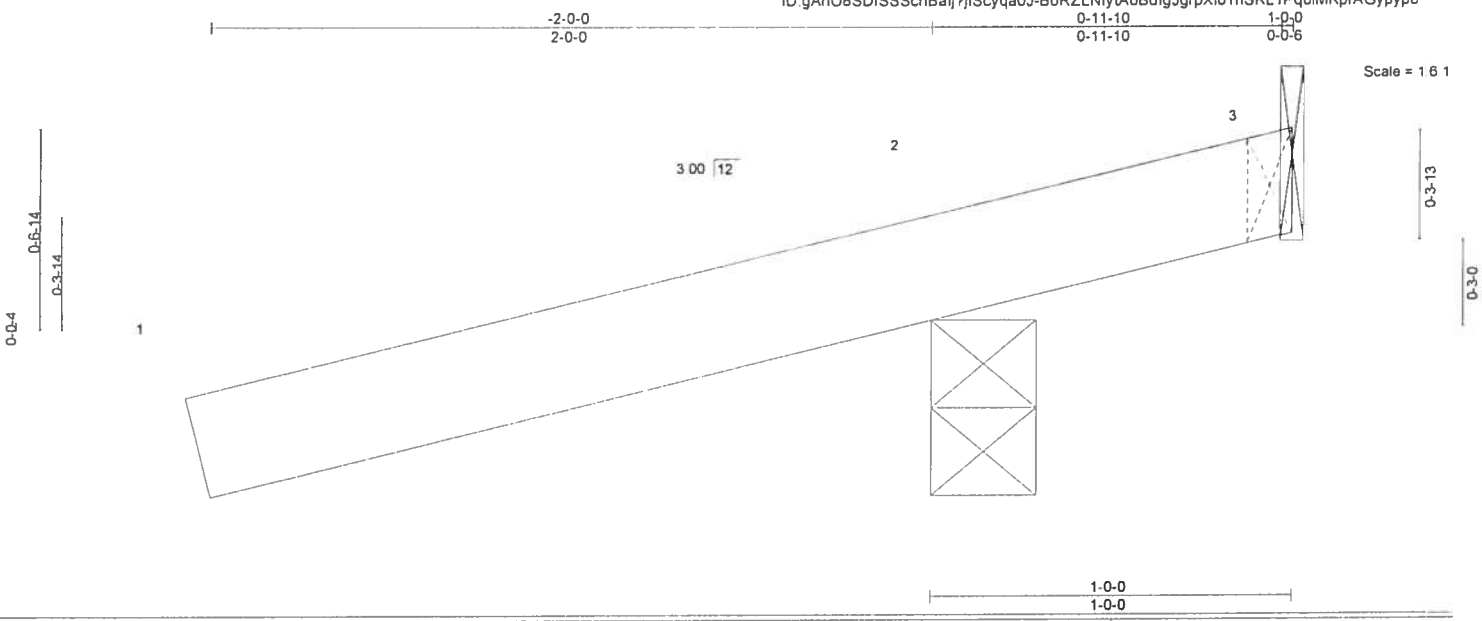

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.37	Vert(LL) 0.00 2 >999 240	
TCDL 7.0	Lumber DOL 1.25	BC 0.00	Vert(CT) 0.00 2-3 >999 180	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 3 n/a n/a	
BCDL 10.0	Code FRC2017/TPI2014	Matrix-MP		Weight: 5 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D

BRACING-
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=-129/Mechanical, 2=292/0-3-8
Max Horz 2=53(LC 8)
Max Uplift 3=-129(LC 1), 2=-370(LC 8)
Max Grav 3=184(LC 8), 2=292(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
3) Refer to girder(s) for truss to truss connections.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=129, 2=370.
5) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

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Date:
August 8,2019

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808567
S1073	SJ3	Corner Jack	4	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:04 2019 Page 1
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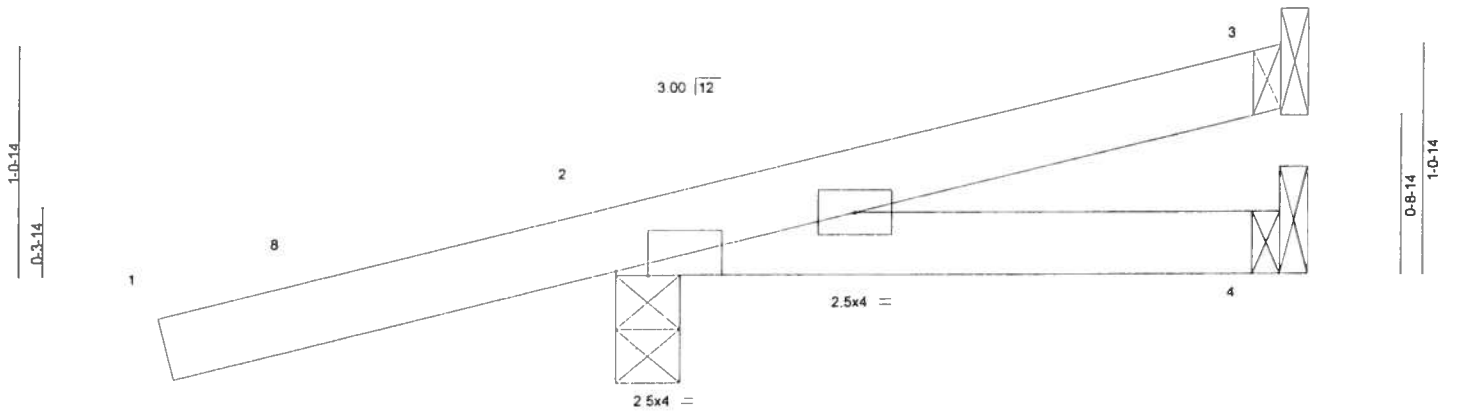
-2-0-0 / 2-0-0 | 3-0-0 / 3-0-0

Scale = 1:10.0



3-0-0 / 3-0-0

Plate Offsets (X,Y)-- [2:0-1-12,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.32	Vert(LL)	-0.00	7	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.07	Vert(CT)	-0.01	7	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MP							Weight: 12 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=48/Mechanical, 2=257/0-3-8, 4=23/Mechanical
Max Horz 2=77(LC 8)
Max Uplift 3=-23(LC 8), 2=-231(LC 8)
Max Grav 3=48(LC 1), 2=257(LC 1), 4=44(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -2-0-7 to 1-1-0, Interior(1) 1-1-0 to 2-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=231.

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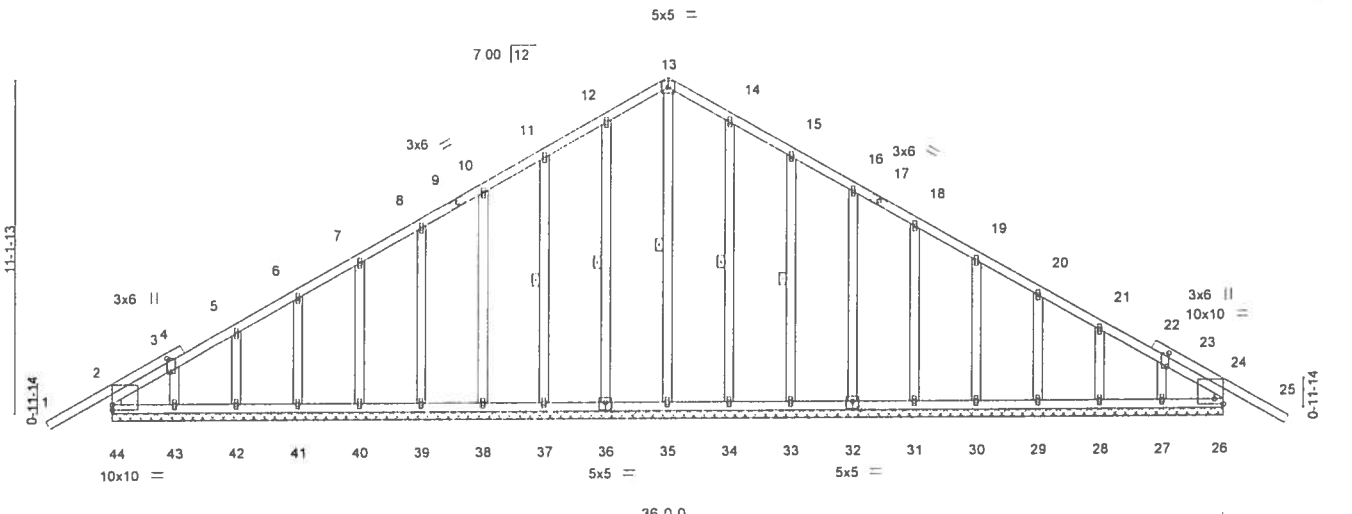
Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808568
S1073	T1	Common Supported Gable	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:06 2019 Page 1
ID:gAhO8SDfSSSchBalj?jlScyqa0J-bb7hzPhqA5BlUmPtL_MEJRfHcYMOc8bC3H1Vmbypypl

-2-0-0 2-0-0 | 18-0-0 18-0-0 | 36-0-0 18-0-0 | 38-0-0 2-0-0

Scale = 1:71.8



36-0-0
36-0-0

Plate Offsets (X,Y)-- [2:0-1-12,0-1-0], [3:0-5-4,0-1-0], [23:0-5-4,0-1-0], [24:Edge,0-2-2], [24:0-1-12,0-1-0], [26:0-1-12,0-0-0], [32:0-2-8,0-3-0], [36:0-2-8,0-3-0], [44:0-1-12,0-0-0], [44:0-0-0,0-2-2]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.38	Vert(LL)	-0.02	25	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.09	Vert(CT)	-0.04	25	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.20	Horz(CT)	0.01	26	n/a	n/a		
BCDL	10.0	Code	FRC2017/TPI2014	Matrix-R							Weight: 275 lb	FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.2D
BOT CHORD	2x4 SP No.2D
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

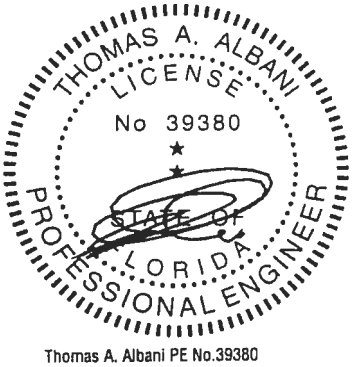
BRACING-

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 13-35, 12-36, 11-37, 14-34, 15-33

REACTIONS. All bearings 36-0-0.
(lb) - Max Horz 44=-407(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 36, 37, 38, 39, 40, 41, 42, 43, 34, 33, 32, 31, 30, 29, 28, 27 except 44=-155(LC 8), 26=-149(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 26, 36, 37, 38, 39, 40, 41, 42, 43, 34, 33, 32, 31, 30, 29, 28, 27 except 44=274(LC 18), 35=319(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-264/260, 10-11=-272/312, 11-12=-341/395, 12-13=-395/460, 13-14=-395/460, 14-15=-341/395, 15-16=-272/312
WEBS 13-35=-340/253

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=36ft; eave=2ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3) -2-0-14 to 1-6-5, Exterior(2) 1-6-5 to 18-0-0, Corner(3) 18-0-0 to 21-7-3, Exterior(2) 21-7-3 to 38-0-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 1.5x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 36, 37, 38, 39, 40, 41, 42, 43, 34, 33, 32, 31, 30, 29, 28, 27 except (jt=lb) 44=155, 26=149.



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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808569
S1073	T2	Common	6	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:15 2019 Page 1
ID:gAhO8SDfSSSchBalj?lScyqa0J-qK95sUoT2sJU38bcMN0MBKXjeAFiD8DW7BjTbaypypc

-2-0-0	6-2-5	12-1-3	18-0-0	23-10-13	29-9-11	36-0-0	38-0-0
2-0-0	6-2-5	5-10-13	5-10-13	5-10-13	5-10-13	6-2-5	2-0-0

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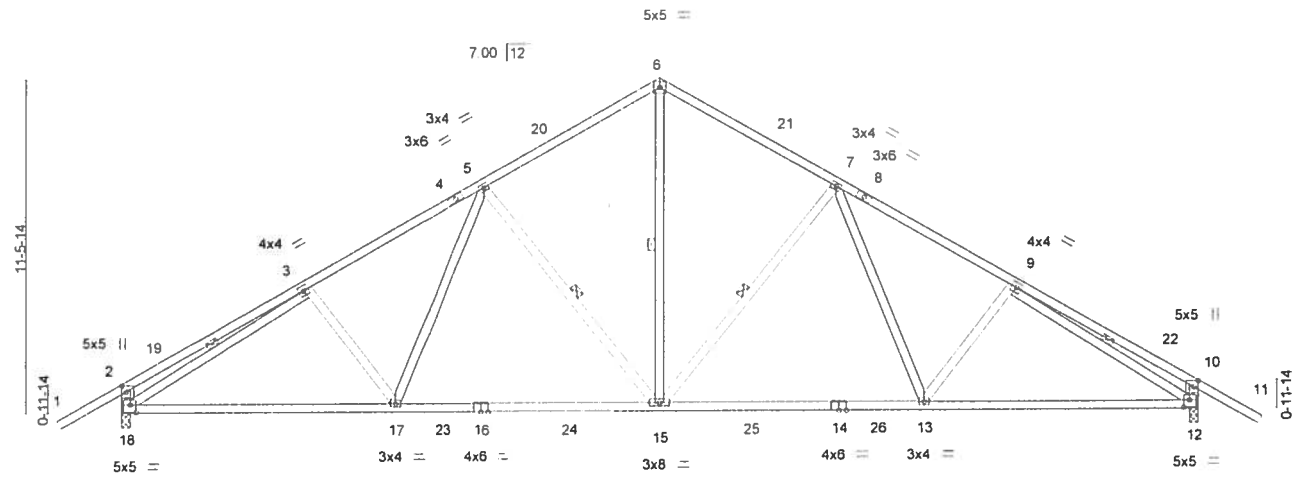

9-1-12	18-0-0	26-10-4	36-0-0
9-1-12	8-10-4	8-10-4	9-1-12

Plate Offsets (X,Y)-- [2:0-2-8,0-1-12], [10:0-2-8,0-1-12], [12:0-2-4,0-3-0], [18:0-2-4,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.81	Vert(LL)	-0.20	15-17	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.78	Vert(CT)	-0.33	15-17	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.07	12	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 228 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-1-8 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-0-6 oc bracing.
WEBS 1 Row at midpt 6-15, 7-15, 5-15, 3-18, 9-12

REACTIONS. (lb/size) 18=1441/0-3-8, 12=1441/0-3-8
Max Horz 18=427(LC 11)
Max Uplift 18=-625(LC 12), 12=-625(LC 12)
Max Grav 18=1458(LC 17), 12=1458(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-651/232, 3-5=-2067/769, 5-6=-1610/707, 6-7=-1610/707, 7-9=-2067/769, 9-10=-651/232, 2-18=-556/362, 10-12=-556/362
BOT CHORD 17-18=-462/1845, 15-17=-302/1611, 13-15=-297/1445, 12-13=-457/1751
WEBS 6-15=-461/1270, 7-15=-693/365, 7-13=-71/423, 9-13=-263/228, 5-15=-693/365, 5-17=-71/423, 3-17=-263/228, 3-18=-1609/653, 9-12=-1610/653

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=36ft; eave=5ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -2-0-14 to 1-6-5, Interior(1) 1-6-5 to 18-0-0, Exterior(2) 18-0-0 to 21-7-3, Interior(1) 21-7-3 to 38-0-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 18=625, 12=625.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808570
S1073	T3	Common	10	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8 240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:16 2019 Page 1
ID:gAhO8SDfSSSchBaIj?jISCyqa0J-IWjT3qp6pARLhIAow4YbjY4uOabwybSgMrT170ypypb

-2-0-0	6-2-5	12-1-3	18-0-0	23-10-13	29-9-11	36-0-0
2-0-0	6-2-5	5-10-13	5-10-13	5-10-13	5-10-13	6-2-5

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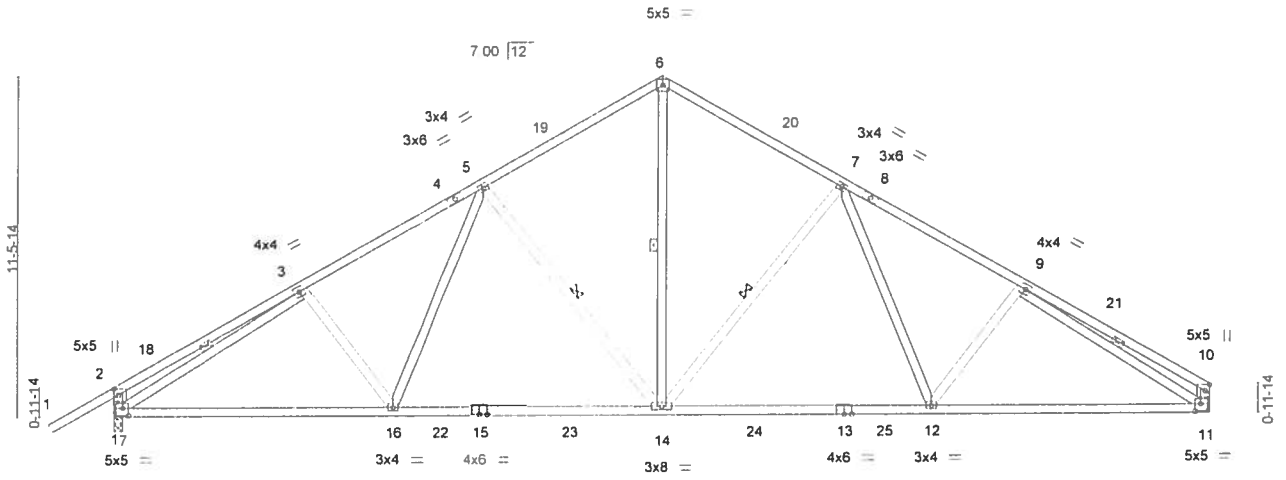

9-1-12	18-0-0	26-10-4	36-0-0
9-1-12	8-10-4	8-10-4	9-1-12

Plate Offsets (X,Y)-- [2:0-2-8,0-1-12], [11:0-2-4,0-3-0], [17:0-2-4,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.81	Vert(LL)	-0.20	12-14	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.78	Vert(CT)	-0.33	12-14	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.07	11	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 224 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No 2D
BOT CHORD 2x4 SP No 2D
WEBS 2x4 SP No 3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-1-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-11-7 oc bracing.
WEBS 1 Row at midpt 6-14, 7-14, 5-14, 3-17, 9-11

REACTIONS. (lb/size) 17=1445/0-3-8, 11=1317/Mechanical
Max Horz 17=416(LC 11)
Max Uplift 17=-629(LC 12), 11=-481(LC 12)
Max Grav 17=1460(LC 17), 11=1343(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-652/234, 3-5=-2073/780, 5-6=-1616/711, 6-7=-1615/720, 7-9=-2073/815, 9-10=-492/220, 2-17=-556/363, 10-11=-409/234
BOT CHORD 16-17=-573/1823, 14-16=-414/1589, 12-14=-409/1418, 11-12=-596/1694
WEBS 6-14=-472/1275, 7-14=-689/368, 7-12=-97/428, 9-12=-263/259, 5-14=-693/364, 5-16=-70/423, 3-16=-262/227, 3-17=-1612/656, 9-11=-1674/598

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=36ft; eave=5ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -2-0-14 to 1-6-5, Interior(1) 1-6-5 to 18-0-0, Exterior(2) 18-0-0 to 21-7-3, Interior(1) 21-7-3 to 35-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 17=629, 11=481.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808571
S1073	T4	Common Structural Gable	2	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:19 2019 Page 1
ID:gAhO8SDfSSSchBalj?jIScyqa0J-j5Pchrr_65pvYluNbC5ILAiUhnbT9zJ62phhkLypypY

6-6-5	12-9-3	19-0-0	25-2-13	31-5-11	38-0-0
6-6-5	6-2-13	6-2-13	6-2-13	6-2-13	6-6-5

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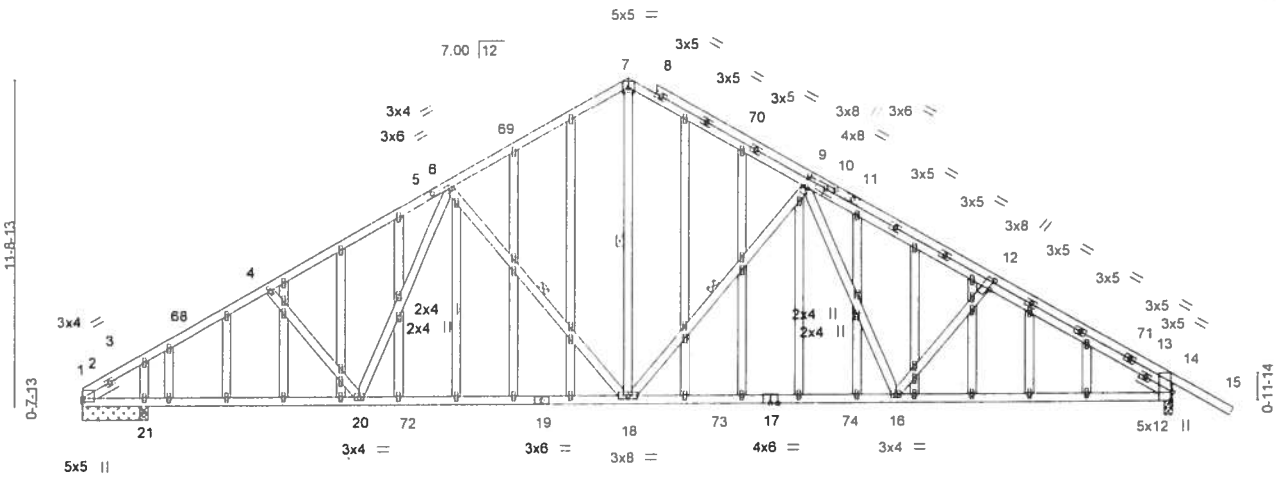

2-0-0	9-7-12	19-0-0	28-4-4	38-0-0
2-0-0	7-7-12	9-4-4	9-4-4	9-7-12

Plate Offsets (X,Y)-- [9:0-5-0,0-1-4], [10:0-4-0,0-1-12], [10:0-0-0,0-1-12], [12:0-4-8,0-0-12], [14:0-3-12,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.48	Vert(LL)	-0.26	18-20	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.92	Vert(CT)	-0.45	18-20	>960	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.43	Horz(CT)	0.08	14	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 385 lb	FT = 20%

LUMBER-

TOP CHORD	2x4 SP No.2D
BOT CHORD	2x4 SP No.2D
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 1-4-0, Right 2x4 SP No.3 1-4-0

BRACING-

TOP CHORD	Structural wood sheathing directly applied or 3-6-11 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	1 Row at midpt 7-18, 9-18, 6-18

REACTIONS. All bearings 2-3-8 except (jt=length) 14=0-3-8.
(lb) - Max Horz 1=-388(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 21 except 1=-474(LC 12), 14=-656(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 21, 21 except 1=1373(LC 17), 14=1550(LC 18), 1=1334(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-3=-2305/775, 3-4=-2346/881, 4-6=-2269/873, 6-7=-1745/763, 7-9=-1723/749, 9-12=-2267/841, 12-14=-2359/846
BOT CHORD	1-21=-588/2130, 20-21=-588/2130, 18-20=-393/1784, 16-18=-381/1554, 14-16=-585/1987
WEBS	7-18=-498/1353, 9-18=-715/381, 9-16=-87/506, 12-16=-457/277, 6-18=-765/411, 6-20=-99/495, 4-20=-343/256

NOTES-

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=38ft; eave=5ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-0-0 to 3-9-10, Interior(1) 3-9-10 to 19-0-0, Exterior(2) 19-0-0 to 22-9-10, Interior(1) 22-9-10 to 40-0-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 1.5x4 MT20 unless otherwise indicated.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21 except (jt=lb) 1=474, 14=656, 1=474.

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August 8,2019

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808572
S1073	T5	Common	15	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:21 2019 Page 1
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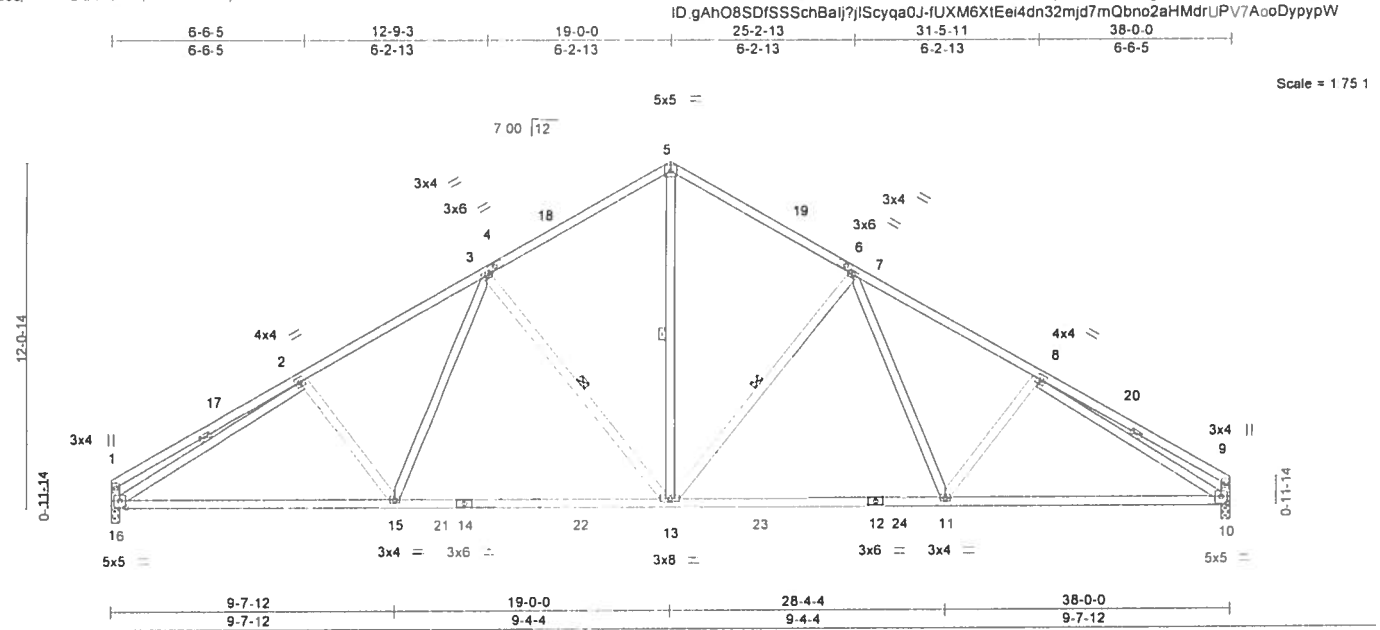

Plate Offsets (X,Y)-- [4:0-2-11,0-1-8], [6:0-2-11,0-1-8], [10:0-2-8,0-2-12], [16:0-2-8,0-2-12]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	-0.25	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.89	Vert(CT)	-0.40	13-15	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.58	Horz(CT)	0.08	10	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 233 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 3-11-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-7-14 oc bracing.
WEBS 1 Row at midpt 5-13, 7-13, 3-13, 2-16, 8-10

REACTIONS. (lb/size) 16=1395/0-3-8, 10=1395/0-3-8
Max Horz 16=404(LC 11)
Max Uplift 16=-512(LC 12), 10=-512(LC 12)
Max Grav 16=1428(LC 17), 10=1428(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-535/239, 2-3=-2200/868, 3-5=-1707/765, 5-7=-1707/765, 7-8=-2200/868, 8-9=-535/239, 1-16=-439/250, 9-10=-439/250
BOT CHORD 15-16=-629/1986, 13-15=-443/1707, 11-13=-436/1521, 10-11=-637/1810
WEBS 5-13=-502/1347, 7-13=-733/389, 7-11=-105/462, 8-11=-287/277, 3-13=-732/389, 3-15=-105/461, 2-15=-287/277, 2-16=-1772/633, 8-10=-1772/633

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=38ft; eave=5ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-12 to 3-11-6, Interior(1) 3-11-6 to 19-0-0, Exterior(2) 19-0-0 to 22-9-10, Interior(1) 22-9-10 to 37-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 16=512, 10=512.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808573
S1073	T6	Common	4	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:22 2019 Page 1
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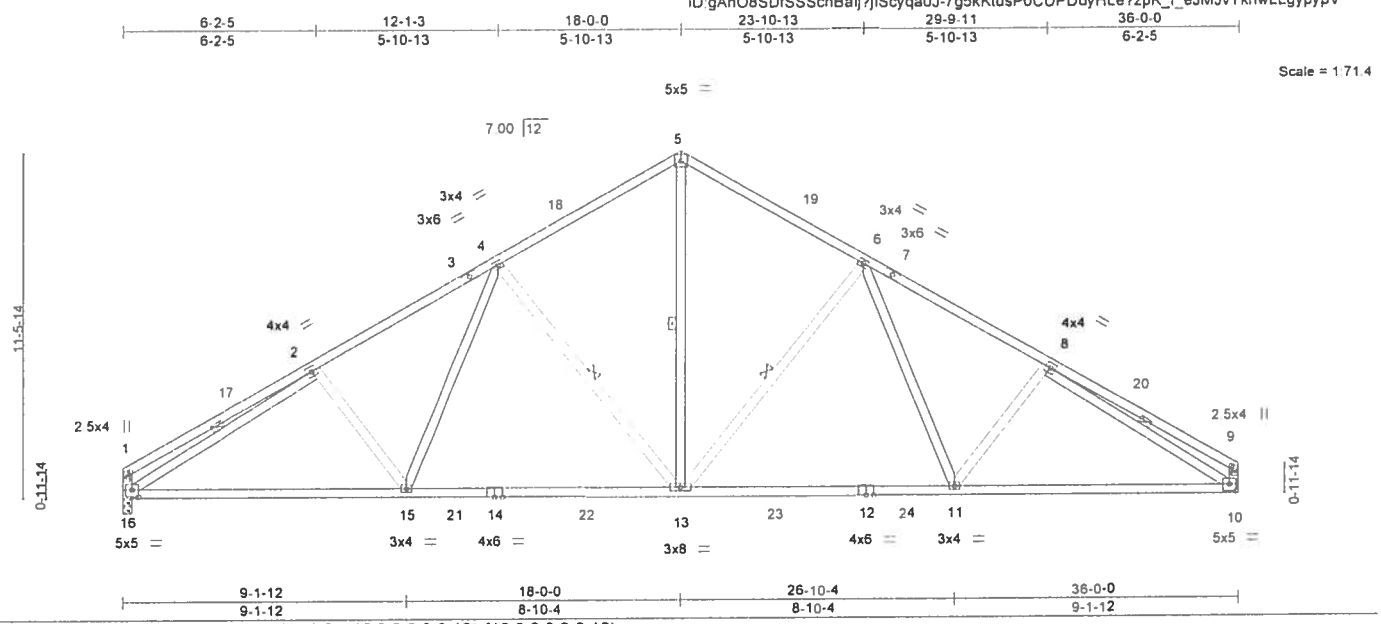

Plate Offsets (X,Y)-- [1:0-2-0,0-1-0], [9:0-2-0,0-1-0], [10:0-2-8,0-2-12], [16:0-2-8,0-2-12]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.50	Vert(LL)	-0.20	13-15	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.78	Vert(CT)	-0.33	13-15	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.08	10	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 221 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-1-10 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-11-4 oc bracing.
WEBS 1 Row at midpt 5-13, 6-13, 4-13, 2-16, 8-10

REACTIONS. (lb/size) 16=1321/0-3-8, 10=1321/Mechanical
Max Horz 16=-384(LC 10)
Max Uplift 16=-485(LC 12), 10=-485(LC 12)
Max Grav 16=1345(LC 17), 10=1345(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-492/221, 2-4=-2070/818, 4-5=-1613/724, 5-6=-1613/724, 6-8=-2070/818, 8-9=-492/221, 1-16=-409/234, 9-10=-409/234
BOT CHORD 15-16=-591/1854, 13-15=-419/1599, 11-13=-412/1423, 10-11=-599/1699
WEBS 5-13=-475/1271, 6-13=-689/367, 6-11=-97/428, 8-11=-263/259, 4-13=-689/367, 4-15=-97/427, 2-15=-263/259, 2-16=-1680/601, 8-10=-1680/601

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=36ft; eave=5ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-12 to 3-8-15, Interior(1) 3-8-15 to 18-0-0, Exterior(2) 18-0-0 to 21-7-3, Interior(1) 21-7-3 to 35-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 16=485, 10=485.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808574
S1073	T7	Common Supported Gable	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:24 2019 Page 1
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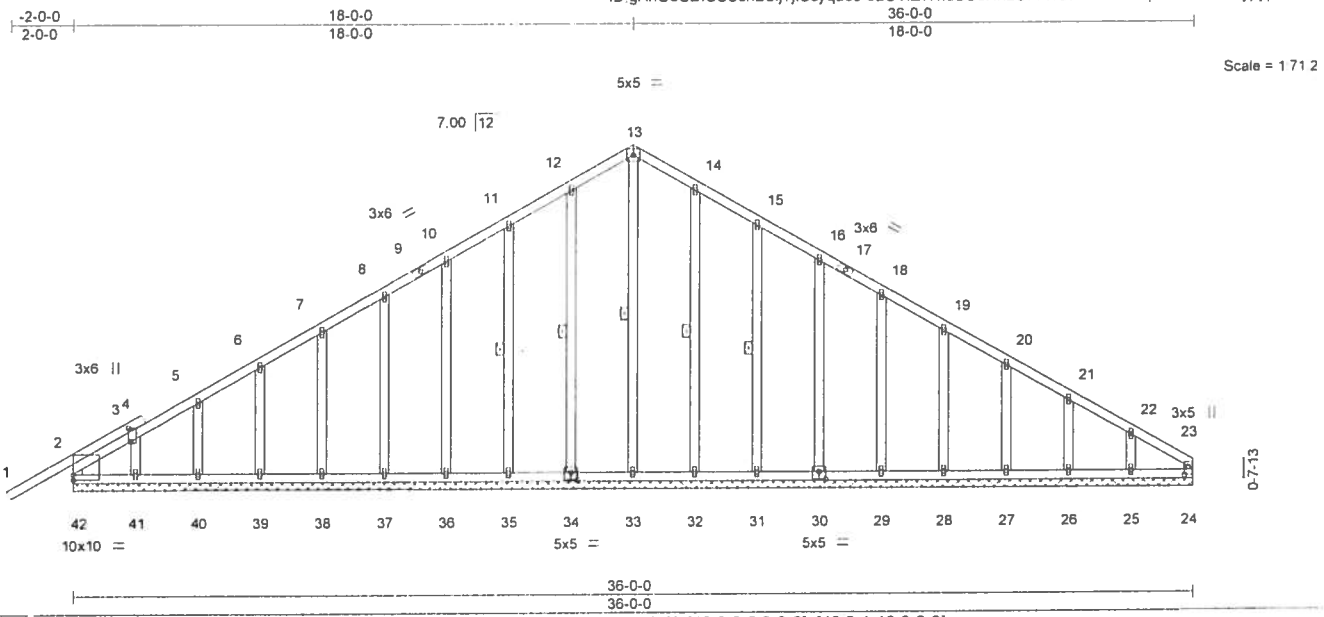

Plate Offsets (X,Y)-- [2:0-1-12,0-1-0], [3:0-5-4,0-1-0], [23:0-3-4,0-1-8], [30:0-2-8,0-3-0], [34:0-2-8,0-3-0], [42:0-0-0,0-2-2], [42:0-1-12,0-0-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.38	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.10	Vert(CT)	-0.01	1	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.20	Horz(CT)	0.01	24	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-R							Weight: 267 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 13-33, 12-34, 11-35, 14-32, 15-31

REACTIONS. All bearings 36-0-0.
(lb) - Max Horz 42=394(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 24, 34, 35, 36, 37, 38, 39, 40, 32, 31, 30, 29, 28, 27, 26 except 42=-149(LC 8), 41=-100(LC 9), 25=-145(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 24, 34, 35, 36, 37, 38, 39, 40, 41, 32, 31, 30, 29, 28, 27, 26, 25 except 42=280(LC 18), 33=303(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-264/250, 10-11=-268/301, 11-12=-337/384, 12-13=-392/449, 13-14=-392/449, 14-15=-337/384, 15-16=-268/301
WEBS 13-33=-330/249

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=36ft; eave=2ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3) -2-0-14 to 1-6-5, Exterior(2) 1-6-5 to 18-0-0, Corner(3) 18-0-0 to 21-7-3, Exterior(2) 21-7-3 to 35-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 1.5x4 MT20 unless otherwise indicated.
5) Gable requires continuous bottom chord bearing.
6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 34, 35, 36, 37, 38, 39, 40, 32, 31, 30, 29, 28, 27, 26 except (jt=lb) 42=149, 41=100, 25=145.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808575
S1073	T8	COMMON GIRDER	1	3	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:27 2019 Page 1
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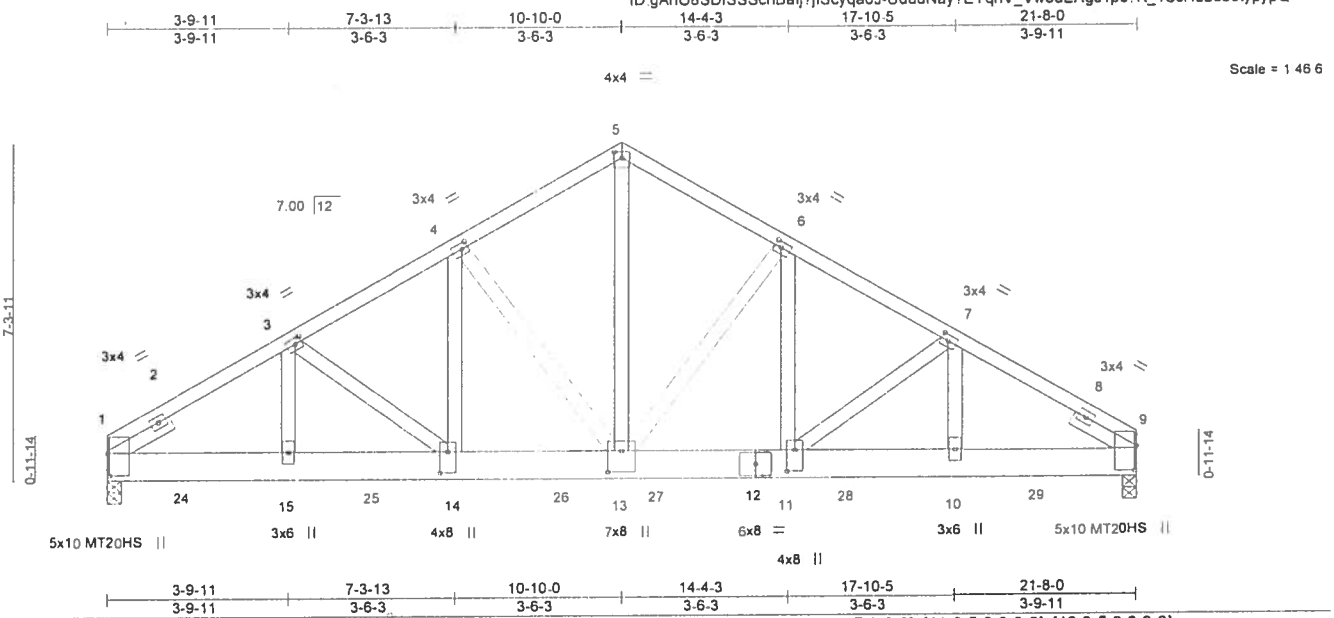

Plate Offsets (X,Y)-- [1:0-5-14,0-0-8], [3:0-1-12,0-1-8], [4:0-1-8,0-1-8], [5:0-2-0,0-1-8], [6:0-1-8,0-1-8], [7:0-1-12,0-1-8], [9:0-6-7,0-0-8], [11:0-5-8,0-2-0], [13:0-5-8,0-3-8], [14:0-5-8,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.62	Vert(LL)	0.12	11-13	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.88	Vert(CT)	-0.18	11-13	>999	180	MT20HS	187/143
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.90	Horz(CT)	0.05	9	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 500 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x8 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 1-6-0, Right 2x4 SP No.3 1-6-0

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=8931/0-3-8, 9=7169/0-3-8
Max Horz 1=-208(LC 6)
Max Uplift 1=-3480(LC 8), 9=-2714(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-3=-10062/3844, 3-4=-9301/3593, 4-5=-7300/2889, 5-6=-7300/2889, 6-7=-9203/3552, 7-9=-9891/3772
BOT CHORD 1-15=-3205/8528, 14-15=-3205/8528, 13-14=-2958/8034, 11-13=-2923/7949, 10-11=-3143/8383, 9-10=-3143/8383
WEBS 5-13=-2768/7104, 6-13=-2854/1160, 6-11=-1104/3011, 7-11=-585/280, 7-10=-351/857, 4-13=-2997/1219, 4-14=-1172/3173, 3-14=-662/312, 3-15=-388/947

NOTES-
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-5-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed , end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) All plates are MT20 plates unless otherwise indicated.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=3480, 9=2714.
9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1507 lb down and 669 lb up at 0-0-0, 1301 lb down and 505 lb up at 1-7-4, 1301 lb down and 505 lb up at 3-7-4, 1301 lb down and 505 lb up at 5-7-4, 1301 lb down and 505 lb up at 7-7-4, 1297 lb down and 501 lb up at 9-7-4, 1297 lb down and 501 lb up at 11-7-4, 1297 lb down and 501 lb up at 13-7-4, 1297 lb down and 501 lb up at 15-7-4, and 1297 lb down and 501 lb up at 17-7-4, and 1297 lb down and 501 lb up at 19-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

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Continued on page 2

LOAD CASE(S) Standard

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
   Uniform Loads (plf)
   Vert: 1-5=-54, 5-9=-54, 16-20=-20
   Concentrated Loads (lb)
   Vert: 12=-1297(F) 10=-1297(F) 14=-1301(F) 15=-1301(F) 16=-1507(F) 24=-1301(F) 25=-1301(F) 26=-1297(F) 27=-1297(F) 28=-1297(F) 29=-1297(F)

Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808576
S1073	T9	Common	5	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:29 2019 Page 1
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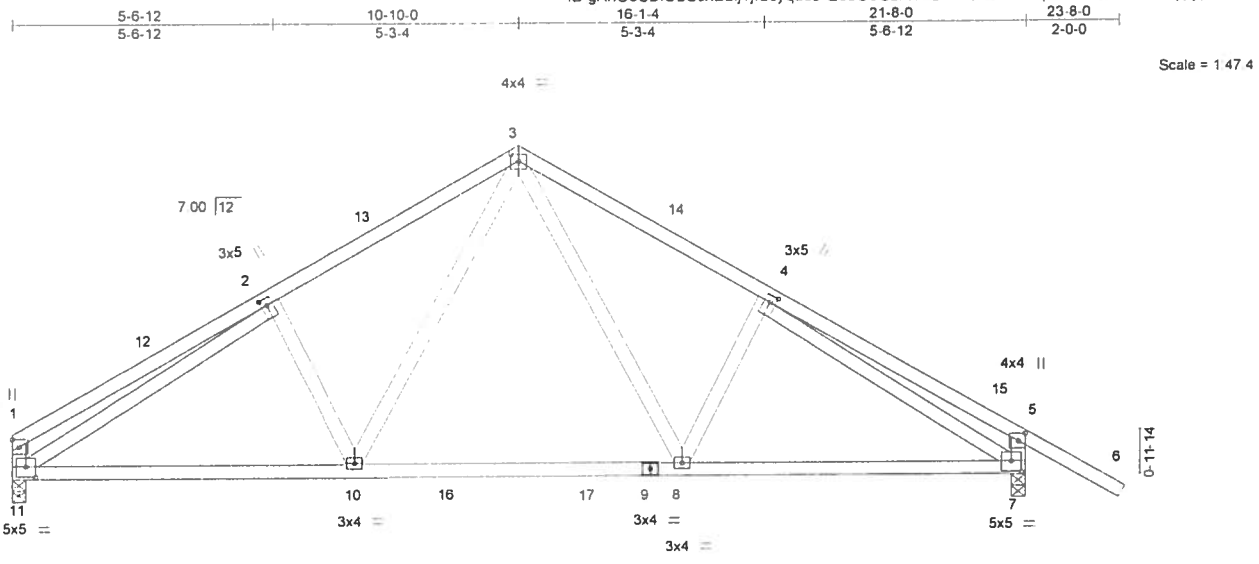

Plate Offsets (X,Y)-- [2:0-1-8,0-1-8], [3:0-2-0,0-1-12], [4:0-1-8,0-1-8], [5:0-2-0,0-1-12], [7:0-2-8,0-2-12], [11:0-2-8,0-2-12]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.59	Vert(LL)	-0.09	8-10	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.45	Vert(CT)	-0.13	8-10	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(CT)	0.02	7	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 128 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-6-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 11=785/0-3-8, 7=917/0-3-8
Max Horz 11=-278(LC 10)
Max Uplift 11=-283(LC 12), 7=-437(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-346/169, 2-3=-1165/510, 3-4=-1166/490, 4-5=-451/157, 1-11=-311/190, 5-7=-432/306
BOT CHORD 10-11=-217/995, 8-10=-23/684, 7-8=-204/976
WEBS 3-8=-154/479, 4-8=-314/254, 3-10=-169/476, 2-10=-312/269, 2-11=-855/301, 4-7=-877/390

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 10-10-0, Exterior(2) 10-10-0 to 13-10-0, Interior(1) 13-10-0 to 23-8-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=283, 7=437.

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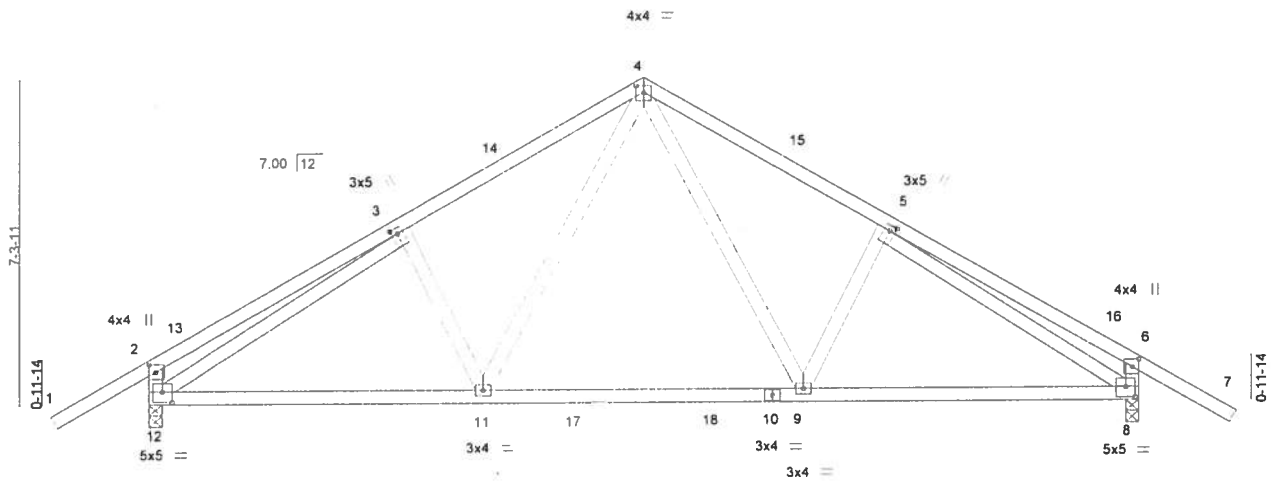
Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808577
S1073	T10	Common	8	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430,

8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:07 2019 Page 1
ID:gAhO8SDfSSSchBalj?jlScyqa0J-3nh3AliSxPJc6vz4uhtTseCO3xczLTrLHxn2J1ypypk

-2-0-0	5-6-12	10-10-0	16-1-4	21-8-0	23-8-0
2-0-0	5-6-12	5-3-4	5-3-4	5-6-12	2-0-0

Scale 1/4"=1'



7-3-13	14-4-3	21-8-0
7-3-13	7-0-5	7-3-13

Plate Offsets (X,Y)-- [2:0-2-0,0-1-12], [3:0-1-8,0-1-8], [4:0-2-0,0-1-12], [5:0-1-8,0-1-8], [6:0-2-0,0-1-12], [8:0-2-8,0-2-12], [12:0-2-8,0-2-12]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.59	Vert(LL)	-0.09	9-11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.45	Vert(CT)	-0.13	9-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.71	Horz(CT)	0.02	8	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 131 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 5-6-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 12=911/0-3-8, 8=911/0-3-8
Max Horz 12=-288(LC 10)
Max Uplift 12=-431(LC 12), 8=-431(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-450/156, 3-4=-1156/470, 4-5=-1156/470, 5-6=-451/156, 2-12=-433/306, 6-8=-433/306
BOT CHORD 11-12=-199/965, 9-11=-18/676, 8-9=-188/968
WEBS 4-9=-154/479, 5-9=-315/254, 4-11=-154/479, 3-11=-315/254, 3-12=-866/385, 5-8=-866/385

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -2-0-14 to 0-11-2, Interior(1) 0-11-2 to 10-10-0, Exterior(2) 10-10-0 to 13-10-0, Interior(1) 13-10-0 to 23-8-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=431, 8=431.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808578
S1073	T11	Common Supported Gable	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430,

8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:08 2019 Page 1
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Scale = 1:46.9

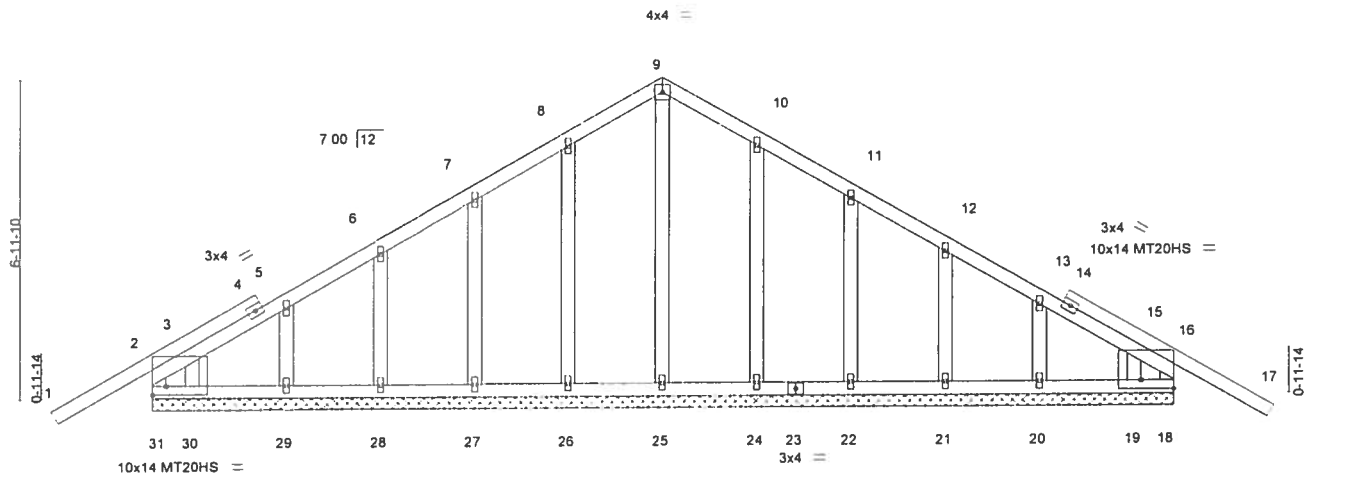

Plate Offsets (X,Y)-- [2:0-1-12,0-1-0], [3:0-1-12,0-1-0], [15:0-1-12,0-1-0], [16:Edge,0-2-6], [16:0-1-12,0-1-0], [18:0-1-12,0-0-0], [19:0-1-12,0-0-0], [30:0-1-12,0-0-0], [31:0-1-12,0-0-0], [31:Edge,0-2-6]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.38	Vert(LL)	-0.02	17	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.06	Vert(CT)	-0.03	17	n/r	120	MT20HS	187/143
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(CT)	0.00	18	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-R							Weight: 138 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 21-8-0.
(lb) - Max Horz 31=-268(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 26, 27, 28, 29, 30, 24, 22, 21, 20, 19 except 31=-271(LC 12), 18=-271(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 25, 26, 27, 28, 29, 30, 24, 22, 21, 20, 19 except 31=297(LC 21), 18=297(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-31=-288/384, 8-9=-242/285, 9-10=-242/285, 16-18=-288/383

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3) -2-0-14 to 0-10-0, Exterior(2) 0-10-0 to 10-10-0, Corner(3) 10-10-0 to 13-10-0, Exterior(2) 13-10-0 to 23-8-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
8) Gable studs spaced at 2-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 26, 27, 28, 29, 30, 24, 22, 21, 20, 19 except (jt=lb) 31=271, 18=271.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808579
S1073	T12	Monopitch Supported Gable	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:09 2019 Page 1
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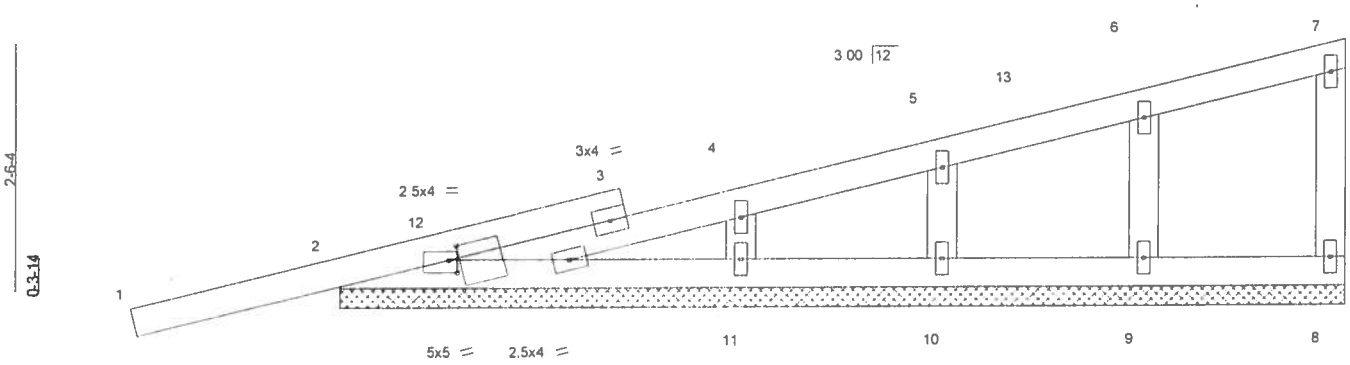
Scale = 1:22.0



10-0-0
10-0-0

Plate Offsets (X,Y)-- [2:0-1-4,0-1-8], [2:0-1-0,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.45	Vert(LL)	0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.09	Vert(CT)	-0.00	1	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	8	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-S							Weight: 43 lb	FT = 20%

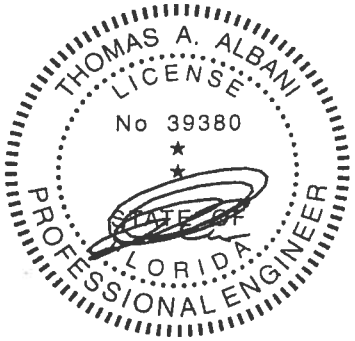
LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 10-0-0.
(lb) - Max Horz 2=140(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 8, 9, 10, 11 except 2=-229(LC 8)
Max Grav All reactions 250 lb or less at joint(s) 8, 9, 10, 11 except 2=271(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 6-9=-117/252

NOTES-
1) Wind: ASCE 7-10, Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Corner(3) -2-0-7 to 0-11-9, Exterior(2) 0-11-9 to 9-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) All plates are 1.5x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 2-0-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 9, 10, 11 except (jt=lb) 2=229.
9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808580
S1073	T13	Monopitch	14	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:10 2019 Page 1
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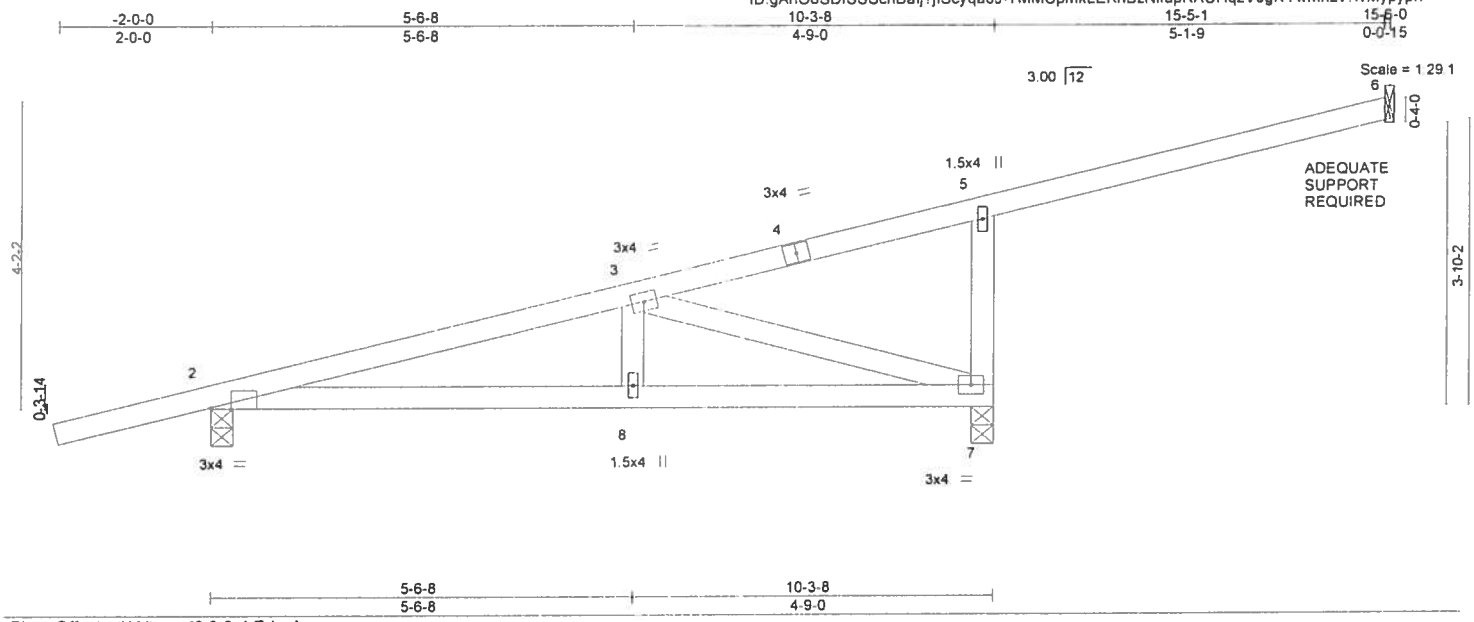

Plate Offsets (X,Y)-- [2:0-3-4,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.32	Vert(LL)	0.03	8-11	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.27	Vert(CT)	-0.06	8-11	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.32	Horz(CT)	0.01	7	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 54 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 6=111/Mechanical, 2=480/0-3-8, 7=556/0-3-8
Max Horz 2=207(LC 9)
Max Uplift 6=-98(LC 8), 2=-276(LC 8), 7=-319(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-703/132, 5-7=-313/267
BOT CHORD 2-8=-298/664, 7-8=-298/664
WEBS 3-7=-702/280

NOTES-
1) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -2-0-7 to 1-1-0, Interior(1) 1-1-0 to 15-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 2=276, 7=319.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808581
S1073	T14	Monopitch	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8 240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08 50 11 2019 Page 1
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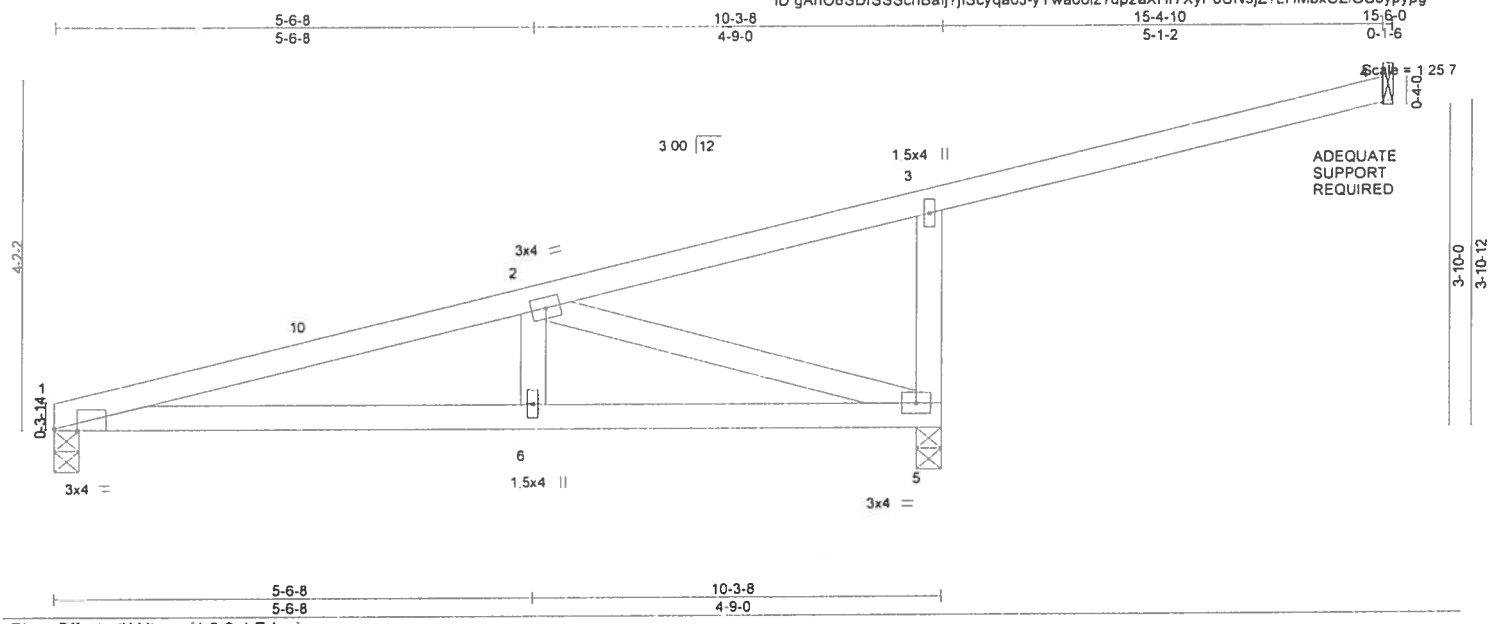

Plate Offsets (X,Y)-- [1:0-3-4,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.29	Vert(LL)	0.05	6-9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.36	Vert(CT)	-0.07	6-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.35	Horz(CT)	0.01	5	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MS							Weight: 51 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=359/0-3-8, 4=109/Mechanical, 5=565/0-3-8
Max Horz 1=193(LC 9)
Max Uplift 1=-123(LC 8), 4=-97(LC 8), 5=-331(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-758/287, 3-5=-307/263
BOT CHORD 1-6=-334/720, 5-6=-334/720
WEBS 2-5=-762/387

NOTES-
1) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 15-4-6 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 1=123, 5=331.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808582
S1073	T15	Half Hip Girder	2	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:12 2019 Page 1
ID:gAhO8SDfSSSchBalj?jIScyqa0J-QlUyDSmbmxxvChs1hETeZivK?yMA0ry4RDVp_Fypypf

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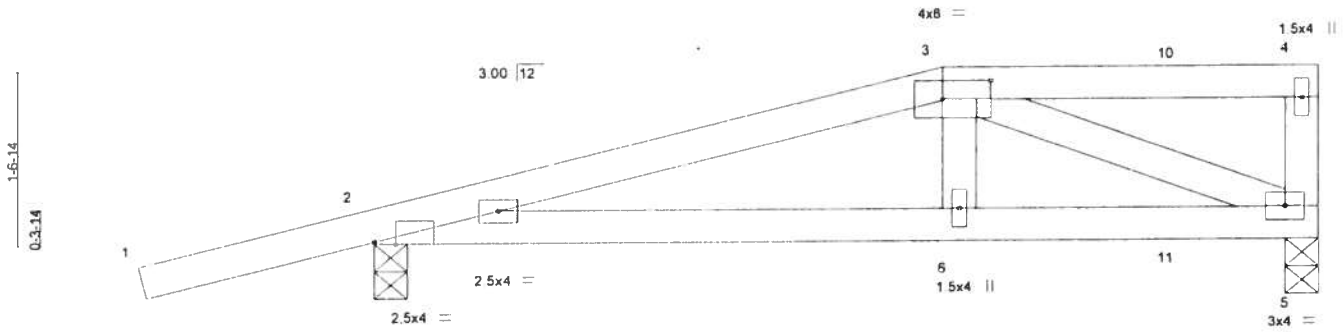
Scale = 1:19.4



5-0-0 / 5-0-0 | 8-3-8 / 3-3-8

Plate Offsets (X,Y)-- [2:0-2-4,Edge], [3:0-5-0,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.26	Vert(LL)	0.03	6-9	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.32	Vert(CT)	-0.05	6-9	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.21	Horz(CT)	0.01	5	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MP							Weight: 35 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

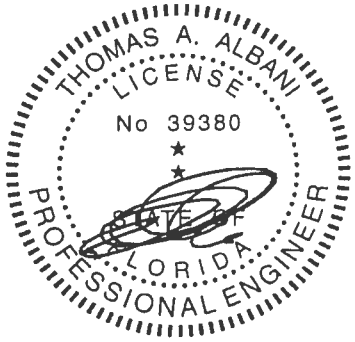
BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 2=505/0-3-8, 5=475/0-3-8
Max Horz 2=87(LC 7)
Max Uplift 2=-315(LC 4), 5=-182(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-802/268
BOT CHORD 2-6=-283/778, 5-6=-275/804
WEBS 3-6=0/294, 3-5=-858/280

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=315, 5=182.
7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 217 lb down and 237 lb up at 5-0-0, and 53 lb down and 87 lb up at 7-0-12 on top chord, and 139 lb down at 5-0-0, and 43 lb down at 7-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 5-7=-20
Concentrated Loads (lb)
Vert: 6=-72(F) 3=-109(F) 10=-51(F) 11=-35(F)



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August 8,2019

Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808583
S1073	T16	Monopitch	16	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:13 2019 Page 1
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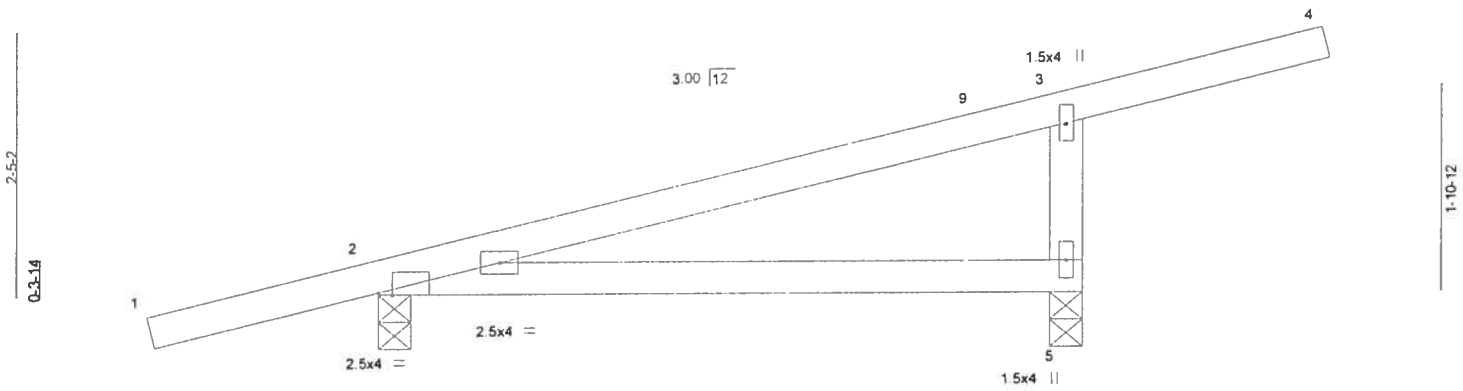
-2-0-0 2-0-0 | 6-3-8 6-3-8 | 8-6-0 2-2-8

Scale = 1:19.8



6-3-8
6-3-8

Plate Offsets (X,Y)-- [2:0-1-8,Edge]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.42	Vert(LL)	-0.05	5-8	>999	240	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.31	Vert(CT)	-0.10	5-8	>707	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-MP							Weight: 27 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 5=358/0-3-8, 2=332/0-3-8
Max Horz 2=137(LC 9)
Max Uplift 5=-250(LC 12), 2=-233(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-5=-295/389

NOTES-
1) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -2-0-7 to 1-1-0, Interior(1) 1-1-0 to 8-5-9 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=250, 2=233.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES / SKIP HARVEY	T17808584
S1073	V1	Valley	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:30 2019 Page 1
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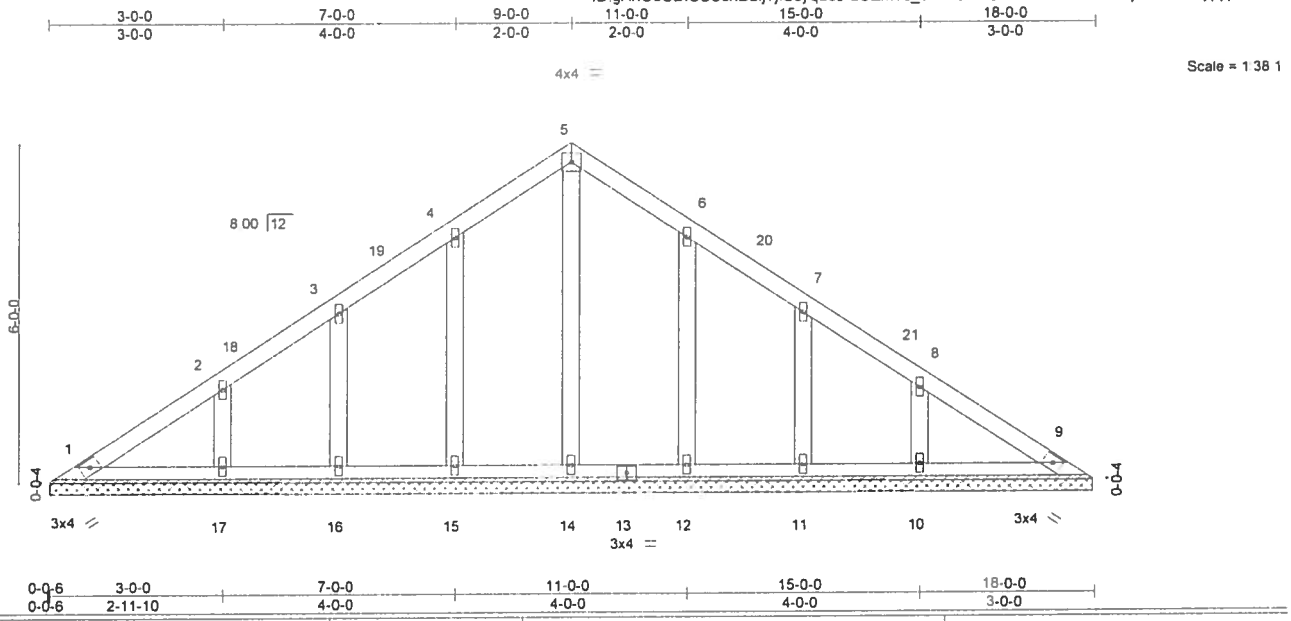

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.04	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	9	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-S							Weight: 90 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 17-11-4.
(lb) - Max Horz 1=-187(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 16, 11, 15, 12 except 17=-128(LC 12), 10=-128(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 1, 9, 14, 16, 11, 17, 15, 12, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 9-0-0, Exterior(2) 9-0-0 to 12-0-0, Interior(1) 12-0-0 to 17-6-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are 1.5x4 MT20 unless otherwise indicated.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 16, 11, 15, 12 except (jt=lb) 17=128, 10=128.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808585
S1073	V2	Valley	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:31 2019 Page 1
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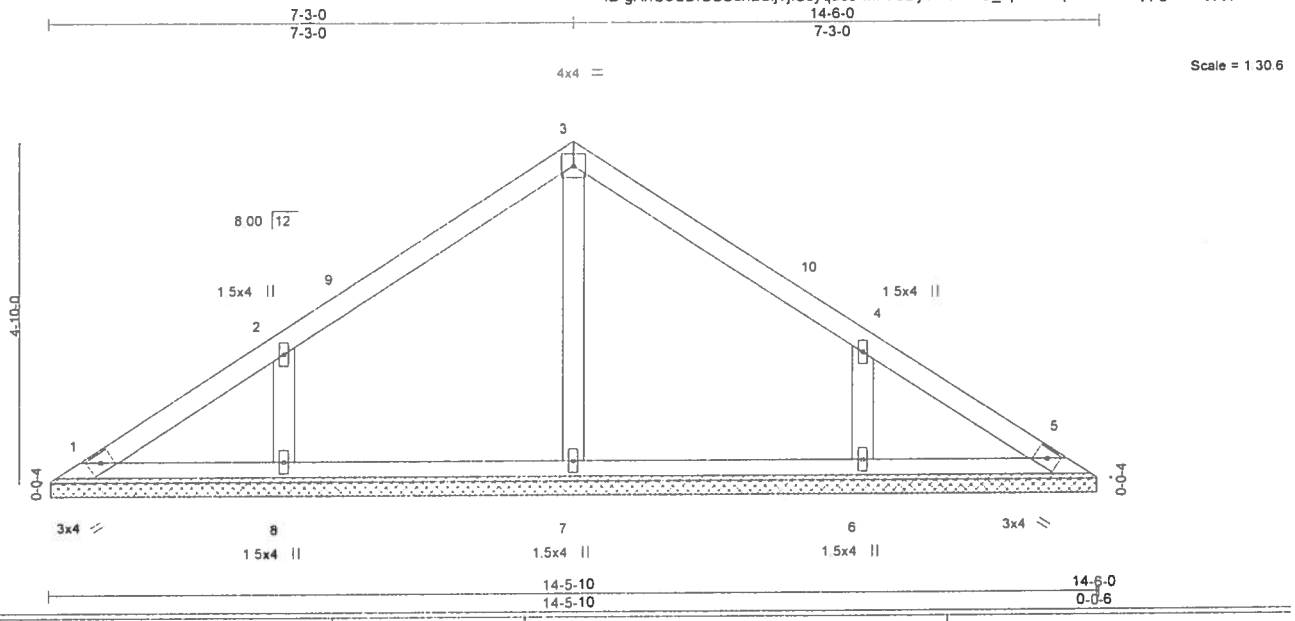

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.16	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.11	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-S							Weight: 57 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-5-4.
(lb) - Max Horz 1=-148(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=-197(LC 12), 6=-197(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=331(LC 17), 6=331(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-332/268, 4-6=-332/268

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-5-12 to 3-3-0, Interior(1) 3-3-0 to 7-3-0, Exterior(2) 7-3-0 to 10-3-0, Interior(1) 10-3-0 to 14-0-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=197, 6=197.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808586
S1073	V3	Valley	1	1	Job Reference (optional)	

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8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:32 2019 Page 1
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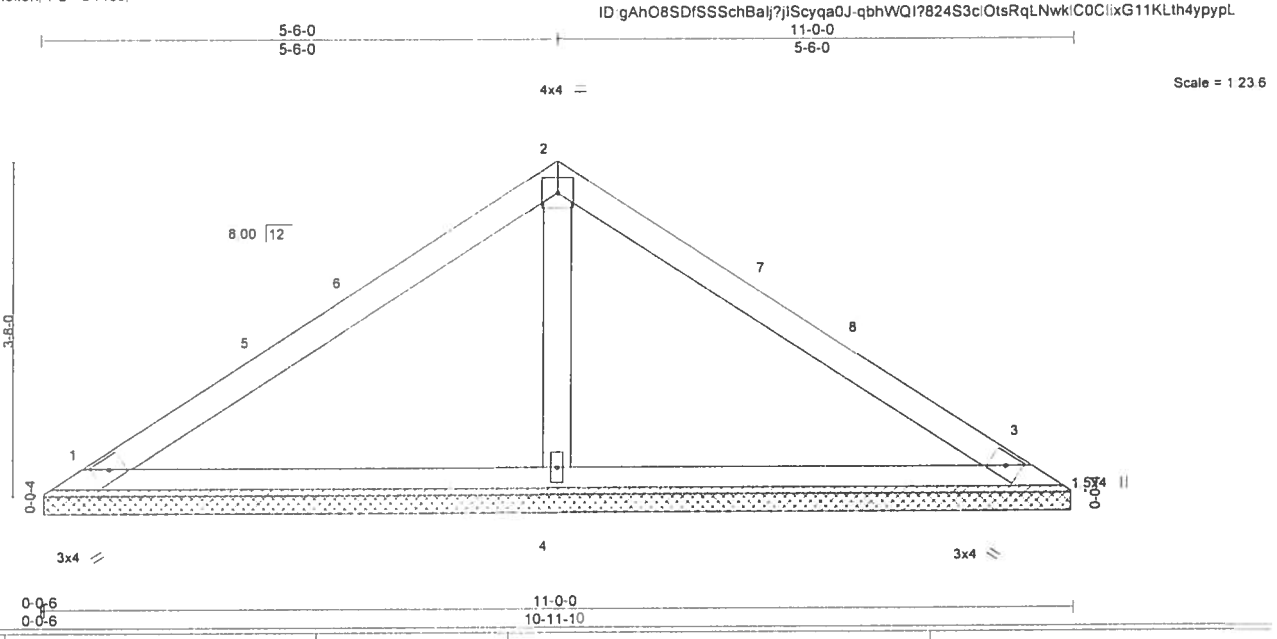

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.30	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.23	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-S							Weight: 39 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=185/10-11-4, 3=185/10-11-4, 4=373/10-11-4
Max Horz 1=-110(LC 10)
Max Uplift 1=-91(LC 12), 3=-91(LC 12), 4=-91(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-5-12 to 3-5-12, Interior(1) 3-5-12 to 5-6-0, Exterior(2) 5-6-0 to 8-6-0, Interior(1) 8-6-0 to 10-6-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808587
S1073	V4	Valley	1	1	Job Reference (optional)	

Duley Truss, Dunnellon, FL - 34430, 8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:33 2019 Page 1
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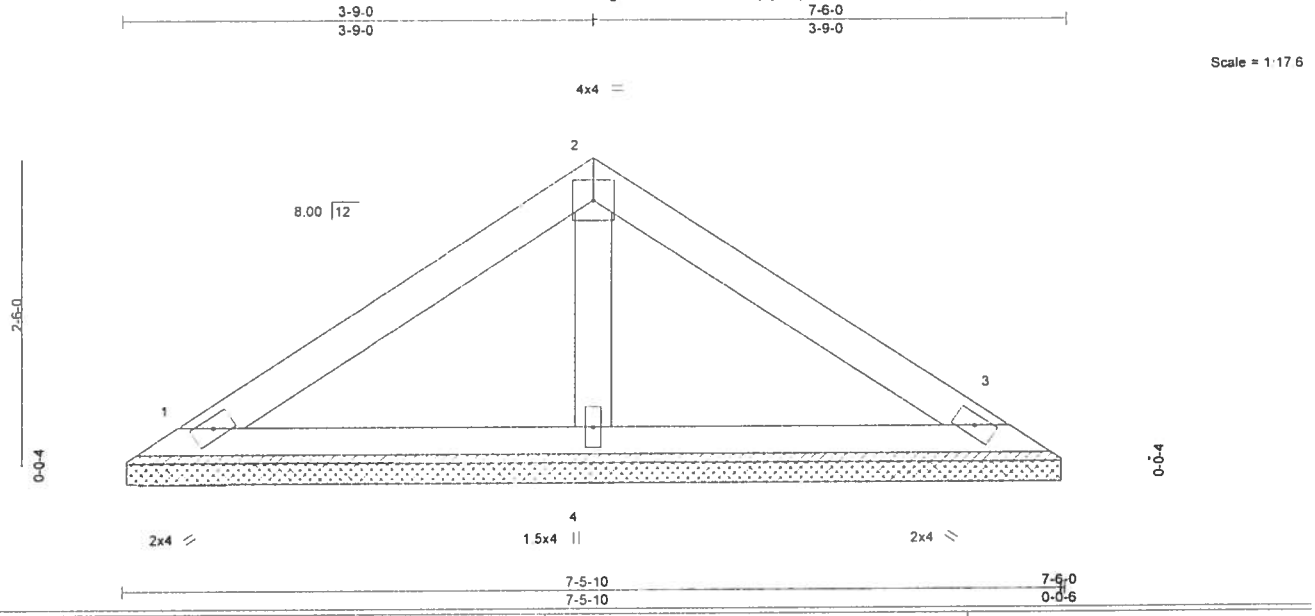

LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.25	TC 0.19	Vert(LL) n/a - n/a 999	MT20 244/190
TCDL 7.0	Lumber DOL 1.25	BC 0.10	Vert(CT) n/a - n/a 999	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.03	Horz(CT) 0.00 3 n/a n/a	
BCDL 10.0	Code FRC2017/TPI2014	Matrix-P		Weight: 26 lb FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
OTHERS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=131/7-5-4, 3=131/7-5-4, 4=221/7-5-4
Max Horz 1=72(LC 11)
Max Uplift 1=-73(LC 12), 3=-73(LC 12), 4=-32(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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Job	Truss	Truss Type	Qty	Ply	HARTZOG RES. / SKIP HARVEY	T17808588
S1073	V5	Valley	1	1	Job Reference (optional)	

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8.240 s Jul 14 2019 MiTek Industries, Inc. Thu Aug 8 08:50:35 2019 Page 1
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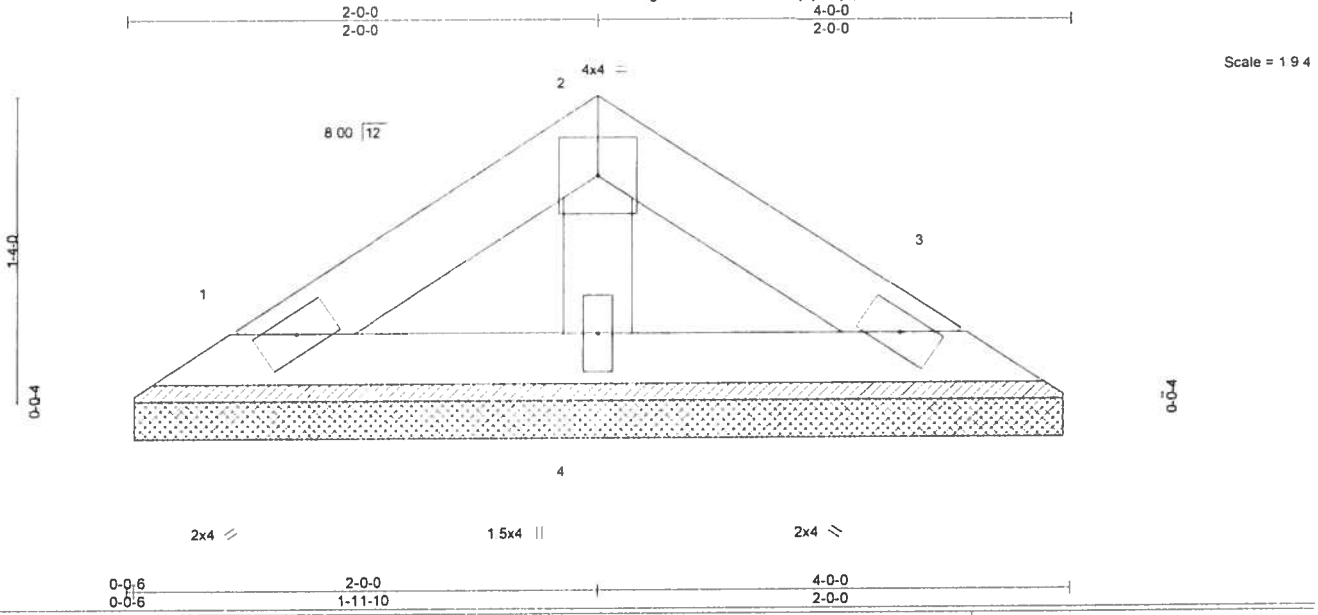

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.02	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code FRC2017/TPI2014		Matrix-P							Weight: 12 lb	FT = 20%

LUMBER-
TOP CHORD 2x4 SP No.2D
BOT CHORD 2x4 SP No.2D
WEBS 2x4 SP No.3

BRACING-
TOP CHORD Structural wood sheathing directly applied or 4-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=61/3-11-4, 3=61/3-11-4, 4=103/3-11-4
Max Horz 1=-33(LC 10)
Max Uplift 1=-34(LC 12), 3=-34(LC 12), 4=-15(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=140mph (3-second gust) Vasd=108mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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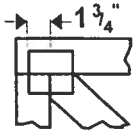
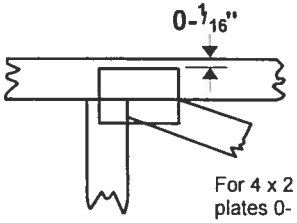
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

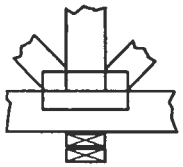
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

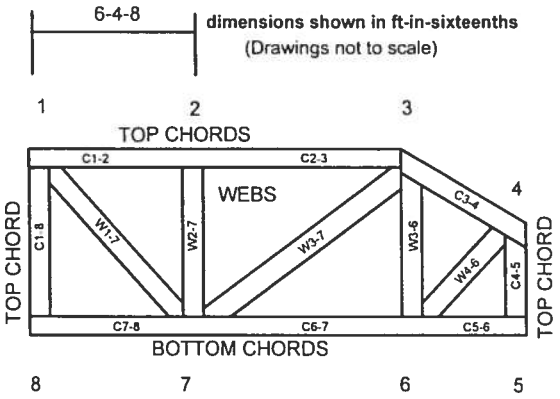


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

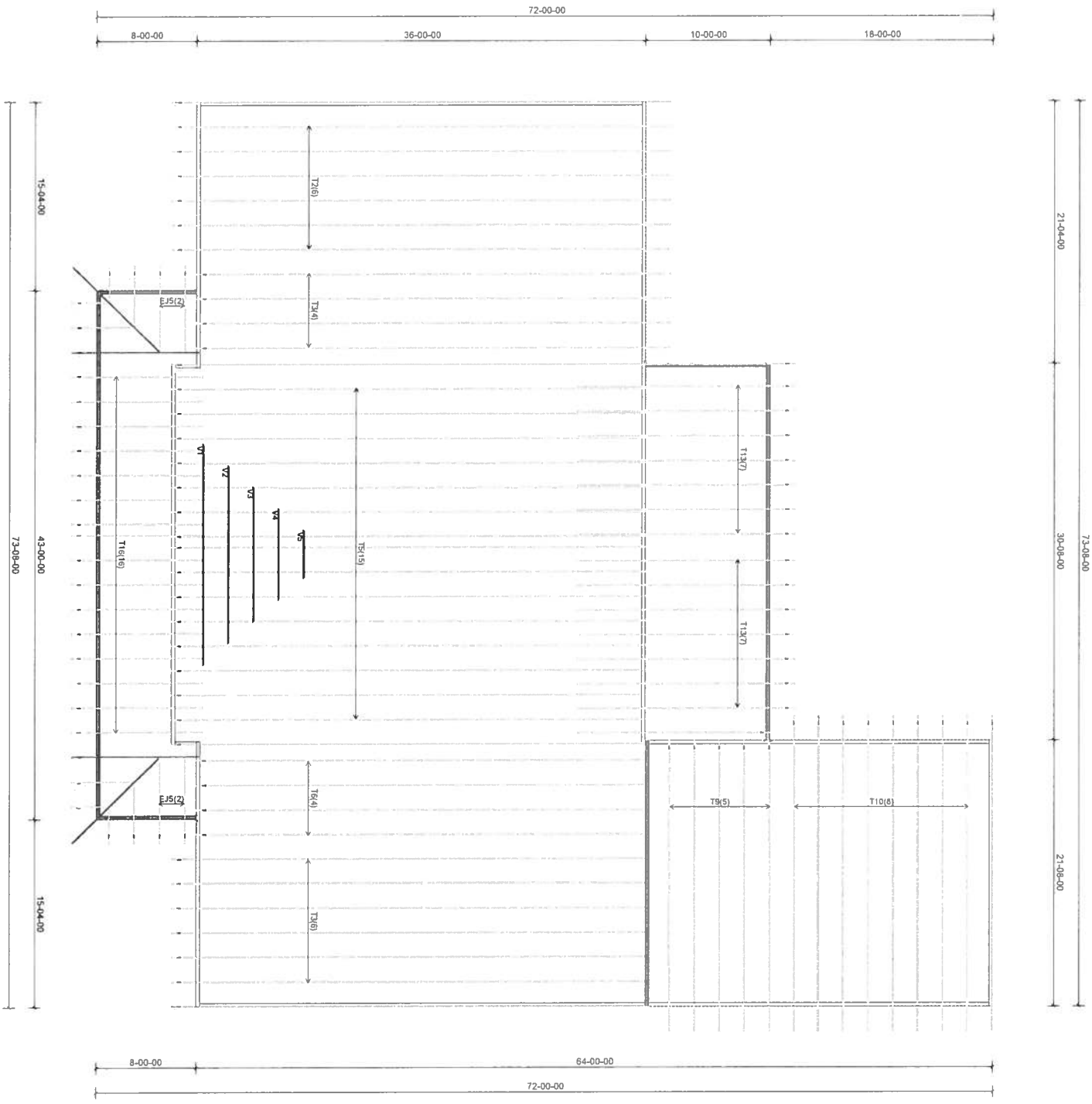
Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

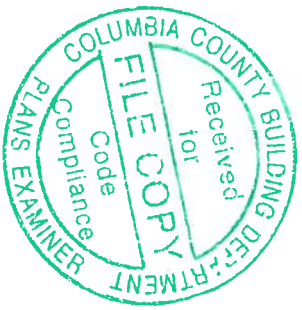
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



JOB NO. **S1073**	Customer: 84 LUMBER Description: HARTZOG RES. Designer: Jack Duley	Pitch: --- Overhang: ---	PRODUCT APPROVAL NUMBER FL 2197.4 MT20 PLATES MITEK INDUSTRIES, INC.	DULEY TRUSS INC. (352) 465-0964

Schafer Engineering LLC

14705 Main St. Alachua FL 32615



Prepared for:

HARTZOG RESIDNECE
100 SW OAK GLEN
FORT WHITE, FLORIDA 32038

By:

Schafer Engineering, LLC CA9312

386-462-1340

NO COPIES ARE TO BE PERMITTED

SCHAFER ENGINEERING, LLC
7104 NW 42ND LANE \ GAINESVILLE FL. 32606
PHONE: 386-462-1340

Trusses: Pre-engineered, pre-fabricated with the manufacturer's required bracing system installed.

Roof Sheathing: Type: OSB Size: 7/16" Fastener type nails: 8d / .113 Ring Shank

Interior zone spacing: Interior: 6" Periphery: 4"

Edge and end zone spacing: Interior: 6" Periphery: 4"

Double Top Plate: Type: Spruce Grade: #2 Size: 2 x 4 Nail Spacing: 8" o.c.

Stud Type: Spruce Grade: #2 Size: 2 x 4

Interior stud spacing: 16" End stud spacing: 16"

Required Shear Wall Siding: Type: OSB Thickness: 7/16"

76 ft Trans: Fastener 8d/131 Spacing: Int: 8 Edge: 4"

54 ft Long: Fastener 8d/131 Spacing: Int: 8 Edge: 4"

Allowable Unit Shear on Shear Walls: 314 pounds per linear foot
Allowable Unit Shear Transferred from Diaphragm: Trans: 152 Long: 97

Wall Tension Transferred by: Siding Nails: 8d/131 @ 4" O.C. Edges

Foundation Anchor Bolts: Concrete Strength: 3000 psi Size: 1/2"

Washer: 2" Embedment: 7" Location of first anchor bolt from corner: 8"

Anchor Bolts @ 48" o.c. Model: A307 Loc. from corner: 8"

Type of Foundation: (1) - #5 rebar continuous required in bond beam.
Floor Slab: 4" Cmu size: 8" x 16" Height: 32" Rein.: #5 at 72" o.c.
Monolithic Footing: Depth: 20" Bottom Width: 12 Rein.: 2 #5 rebars

Stemwall Footing: Width: 20 Depth: 10 Rein.: 2 #5 rebar

Interior Footings 20" Wide X 12" Deep with 2-#5 rebar continuous

Porch Columns: 6 X 6 X 10' syp #2 pt @ 11'-0" o.c. max. spacing Column Fasteners: Simpson PC66 \ PBS66 or equal

Special Comments: Install 2 ply 2 x 12 syp #2 with 7/16" osb flitch beam over all doors, windows and covered porches.

Notes:

1. Balloon frame all gable ends unless accompanied by gable end detail
2. All walls to be nailed with same nailing pattern as the shear walls.
3. This wind load is not valid without a raised, embossed seal. (NO COPIES).
4. 1500 psf soil bearing pressure minimum.
5. Fiber mesh or WWM may be used in concrete slab. All steel must be grade 40 min. Install standard 10" ACI hook top and bottom.
6. Trusses must be installed and anchored in accordance to the truss engineering.
7. All headers spanning 12' and over must be pre-engineered.
8. This is a windload only. Not a structural analysis. Schafer Engineering strongly recommends always having a structural analysis.
9. The foundation is for minimum design use, and may be increased.
10. Wind load is for one use only \ FBC-2017 \ No copies permitted
11. Install anchor bolts a 48" o.c., & Simpson SP1 at bottom plate and Simpson SP2 at top plate or equal @ 32" O.C. for all interior bearing walls.
12. Truss company to use all exterior porch walls for bearing when possible.

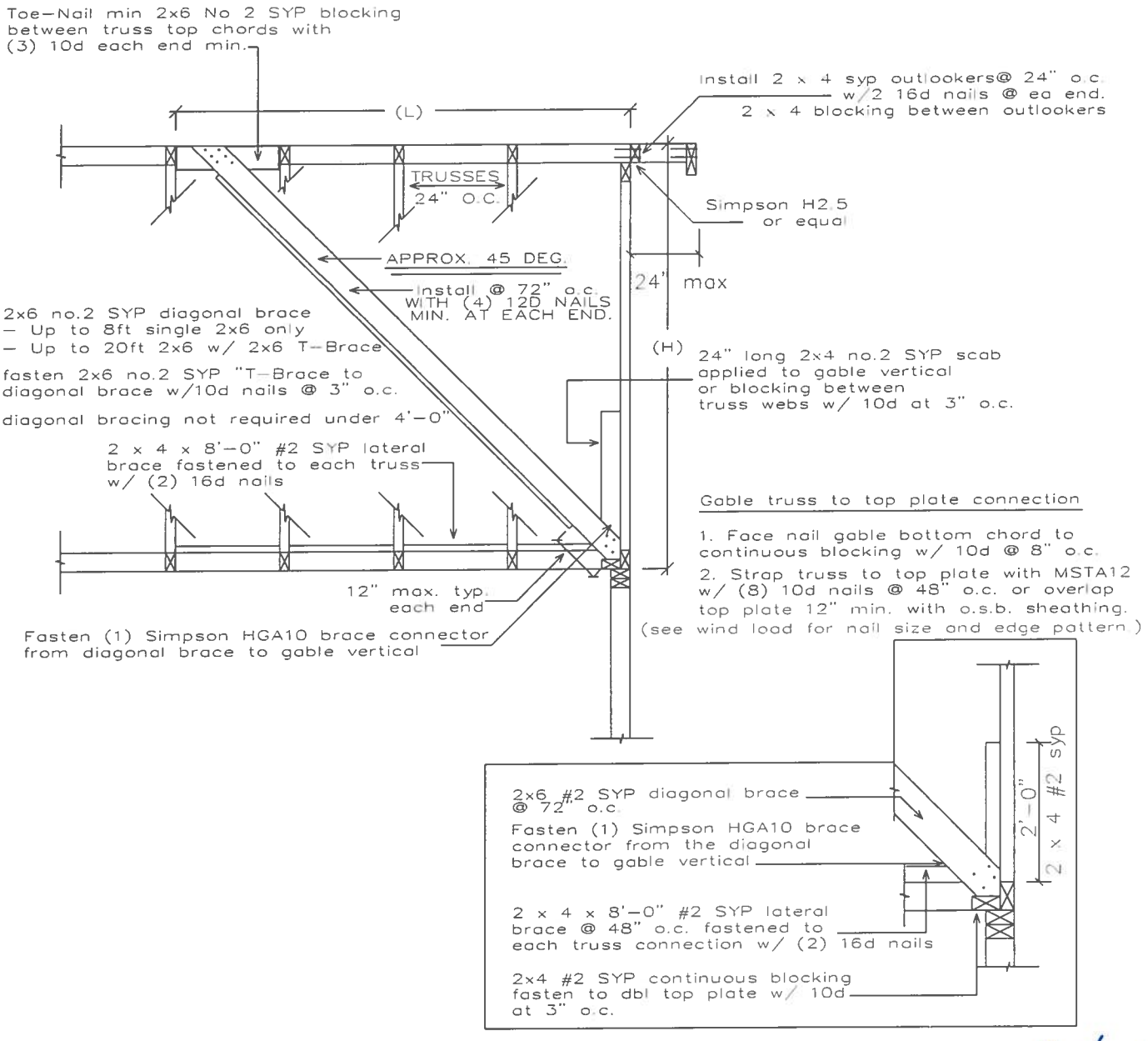
7-26-19

Bruce Schafer, P. E. #48984 ca 9312
7104 NW 42ND LN
GAINESVILLE, FL. 32606

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606

PHONE: 386-462-1340



TYPICAL GABLE END BRACING

7-26-19

Bruce Schafer, P. E. #48984 CA #9312
7104 NW 42ND LN
GAINESVILLE, FL. 32606

SCHAFER ENGINEERING, LLC

7104 NW 42ND LANE \ GAINESVILLE FL. 32606

PHONE: 386-462-1340

TIE-DOWN TABLES

HEADER STRAPPING				
Uplift Lbs	Top Connector	Rating Lbs	Bottom Connector	Rating Lbs
to 455	LSTA9	635	H3	320
to 910	LSTA12	795	2-H3	640
to 1265	LSTA18	1110	LTT19	1305
to 1750	2-LSTA12	1810	LTT20	1750
to 2530	2-LSTA18	2530	HD2A-2.5	2165
to 2865	3-LSTA18	3255	HD2A-3.5	2565
to 3700	3-LSTA24	3880	HD5A-3	3130

Total the uplift for each truss sitting on the header and divide by 2 to determine the uplift on the header. Use proper bolt anchors sufficient to support required uplift loads.

TRUSSES \ GIRDERS			
Uplift Lbs	Top Connector	Bottom Connector	Rating Lbs
to 535	H2.5A	NA	
to 1015	H10A	NA	
to 1215	TS22	LTT19	1305
to 1750	2-TS22	LTT20	1750
to 2570	2-TS22	HD2A	2565
to 3665	3-TS22	HD5A	3645
to 5420	2-MST37	HTT22	5250
to 9660	2-MST60	HD10A	8160

Two 12d common toenials are required per truss for each bearing point into top plate.
It is the contractors responsibility to provide a continuous load path from truss to foundation.

	TOP CONNECTOR	RATING LBS	BOTTOM CONNECTOR	RATING LBS
BEAM SEATS	LSTA18	1110	LTT19	1305
POSTS	2-LSTA18	2220	ABU44	2200

1. Simpson or equivlent hardware may be used. For nailing into spruce members, multiply table values by .86
2. See truss engineering for anchor uplift values.
3. This schedule is not meant to be a replacement to the specified values of any manufactures values.

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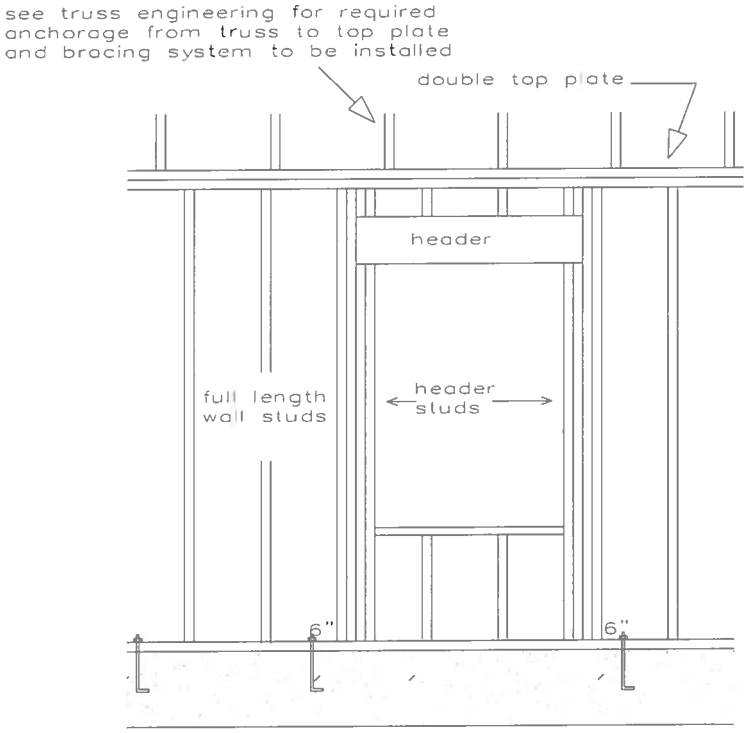


total each truss uplift on the header and divide by two for header and header stud anchorages

		Maximum Header Span (ft)					
		3'	6'	9'	12'	15'	18'
		Number of Header Studs Supporting End of Header					
		1	1	2	2	2	2
Unsupported Wall Height	Stud Spacing	Number of Full Length Studs at Each End of Header					
10'-0" or less	12"	2	2	3	3	3	3
	16"	2	2	3	3	3	3
	24"	1	2	2	2	2	2
Greater than 10'-0"	12"	2	2	3	4	5	5
	16"	2	2	3	3	4	4
	24"	1	2	2	2	3	3

User Input Data		
Structure Type	Building	
Basic Wind Speed (V)	135	mph
Structural Category	II	
Exposure	B	
Struc Nat Frequency (n1)	1	Hz
Slope of Roof (Theta)	30.3	Deg
Type of Roof	Gabled	
Eave Height (Eht)	10.00	ft
Ridge Height (RHt)	21.83	ft
Mean Roof Height (Ht)	15.92	ft
Width Perp. to Wind (B)	72.00	ft
Width Parallel to Wind (L)	73.67	ft
Damping Ratio (beta)	0.01	

Red values should be changed only through "Main Menu"

Calculated Parameters		
Type of Structure		
Height/Least Horizontal Dim	0.22	
Flexible Structure	No	

Calculated Parameters		
Importance Factor	1	
Non-Hurricane, Hurricane (v=85-100 mph) & Alaska		
Table C6-4 Values		
Alpha =	7.000	
zg =	1200.000	
At =	0.143	
Bt =	0.840	
Am =	0.250	
Bm =	0.450	
Cc =	0.300	
l =	320.00	ft
Epsilon =	0.333	
Zmin =	30.00	ft

Gust Factor Category I: Rigid Structures - Simplified Method			
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85	
Gust Factor Category II: Rigid Structures - Complete Analysis			
Zm	Zmin	30.00	ft
lzm	Cc * (33/z)^0.167	0.3048	
Lzm	l*(zm/33)^Epsilon	309.99	ft
Q	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	0.8822	
Gust2	0.925*((1+1.7*Izm*3.4*Q)/(1+1.7*3.4*Izm))	0.8555	
Gust Factor Category III: Flexible or Dynamically Sensitive Structures			
Vhref	V*(5280/3600)	198.00	ft/s
Vzm	bm*(zm/33)^Am*Vhref	87.00	ft/s
NF1	NatFreq*Lzm/Vzm	3.56	Hz
Rn	(7.47*NF1)/(1+10.302*NF1)^1.667	0.0627	
Nh	4.6*NatFreq*Ht/Vzm	0.84	
Nb	4.6*NatFreq*B/Vzm	3.81	
Nd	15.4*NatFreq*Depth/Vzm	13.04	
Rh	1/Nh-(1/(2*Nh^2)*(1-Exp(-2*Nh)))	0.6134	
Rb	1/Nb-(1/(2*Nb^2)*(1-Exp(-2*Nb)))	0.2282	
Rd	1/Nd-(1/(2*Nd^2)*(1-Exp(-2*Nd)))	0.0737	
RR	((1/Beta)*Rn*Rh*Rb*(0.53+0.47*Rd))^0.5	0.7042	
gg	+(2*LN(3600*n1))^0.5+0.577/(2*LN(3600*n1))^0.5	4.19	
Gust3	0.925*((1+1.7*Izm*(3.4^2*Q^2+GG^2*RR^2)^0.5)/(1+1.7*3.4*Izm))	1.07	

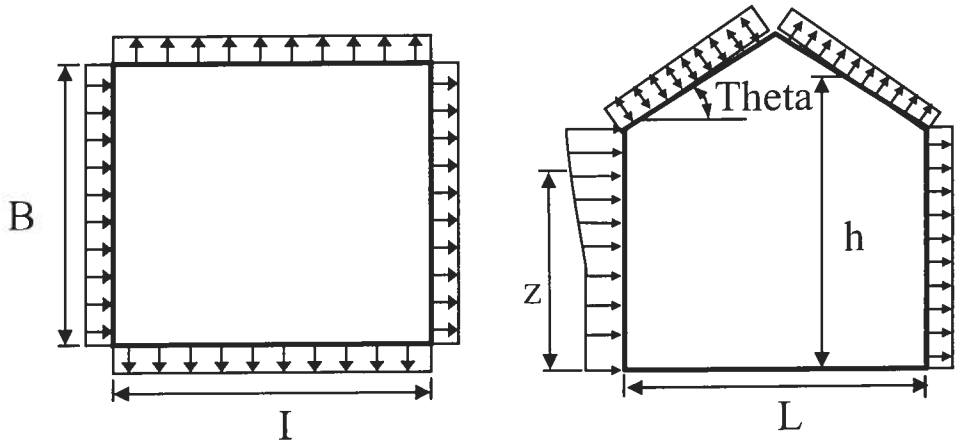
Gust Factor Summary			
Main Wind-force resisting system:		**Components and Cladding:**	
Gust Factor Category:	I	Gust Factor Category:	I
Gust Factor (G)	0.86	Gust Factor (G)	0.86

6.5.12.2.1 Design Wind Pressure - Buildings of All Heights (Non-flexible)

Elev.	Kz	Kzt	Kd	qz	Pressure (lb/ft^2) Windward Wall*	
ft			1.00	lb/ft^2	+GCpi	-GCpi
21.83	0.70	1.00	1.00	32.69	17.46	27.28
20	0.70	1.00	1.00	32.69	17.46	27.28
15.92	0.70	1.00	1.00	32.69	17.46	27.28
15	0.70	1.00	1.00	32.69	17.46	27.28

Figure 6-3 - External Pressure Coefficients, Cp

Loads on Main Wind-Force Resisting Systems



Variable	Formula	Value	Units
Kh	2.01*(Ht/zg)^(2/Alpha)	0.58	
Kht	Topographic factor (Fig 6-2)	1.00	
Qh	.00256*(V)^2*ImpFac*Kh*Kht*Kd	27.27	psf

Wall Pressure Coefficients, Cp	
Surface	Cp
Windward Wall (See Figure 6.5.12.2.1 for Pressures)	0.80

Roof Pressure Coefficients, Cp	
Roof Area (sq. ft.)	-
Reduction Factor	1.00

Description	Cp	Pressure (psf) +GCpi	-GCpi
Leeward Walls (Wind Dir Parallel to 72 ft wall)	-0.50	-16.47	-6.65
Leeward Walls (Wind Dir Parallel to 73.67 ft wall)	-0.50	-16.58	-6.76
Side Walls	-0.70	-21.24	-11.42
Roof - Normal to Ridge (Theta>=10)			
Windward - Max Negative	-0.19	-9.30	0.52
Windward - Max Positive	0.31	2.23	12.05
Leeward Normal to Ridge	-0.60	-18.91	-9.09
Overhang Top	-0.19	-4.39	-4.39
Overhang Bottom	0.80	0.68	0.68
Roof - Parallel to Ridge (All Theta)			
Dist from Windward Edge: 0 ft to 7.96 ft	-0.90	-25.91	-16.09
Dist from Windward Edge: 7.96 ft to 15.92 ft	-0.90	-25.91	-16.09
Dist from Windward Edge: 15.92 ft to 31.84 ft	-0.50	-16.58	-6.76
Dist from Windward Edge: > 31.84 ft	-0.30	-11.91	-2.09

* Horizontal distance from windward edge

Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.58
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	27.27

Case A						
Surface	**GCpf**	**+GCpi**	**-GCpi**	**qh (psf)**	**Min P (psf)**	**Max P (psf)**
1	0.56	0.18	-0.18	32.69	12.42	24.19
2	0.21	0.18	-0.18	32.69	0.98	12.75
3	-0.43	0.18	-0.18	32.69	-19.94	-8.17
4	-0.37	0.18	-0.18	32.69	-17.98	-6.21
5	0.00	0.18	-0.18	32.69	-5.88	5.88
6	0.00	0.18	-0.18	32.69	-5.88	5.88
1E	0.69	0.18	-0.18	32.69	16.67	28.44
2E	0.27	0.18	-0.18	32.69	2.94	14.71
3E	-0.53	0.18	-0.18	32.69	-23.21	-11.44
4E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
5E	0.00	0.18	-0.18	32.69	-5.88	5.88
6E	0.00	0.18	-0.18	32.69	-5.88	5.88

* p = qh * (GCpf - GCpi)

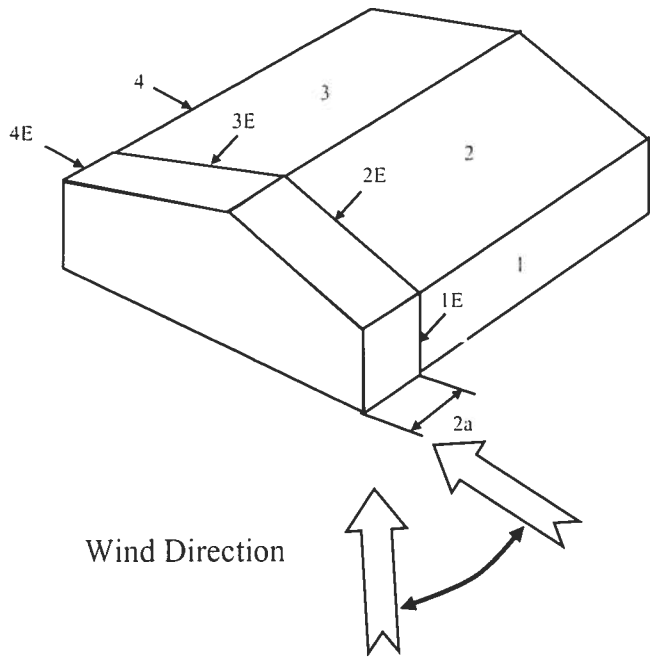


Figure 6-4 - External Pressure Coefficients, GCpf

Loads on Main Wind-Force Resisting Systems w/ Ht <= 60 ft

Kh =	2.01*(Ht/zg)^(2/Alpha)	=	0.58
Kht =	Topographic factor (Fig 6-2)	=	1.00
Qh =	0.00256*(V)^2*ImpFac*Kh*Kht*Kd	=	27.27

Case B						
Surface	GCpf	+GCpi	-GCpi	qh (psf)	Min P (psf)	Max P (psf)
1	-0.45	0.18	-0.18	32.69	-20.59	-8.83
2	-0.69	0.18	-0.18	32.69	-28.44	-16.67
3	-0.37	0.18	-0.18	32.69	-17.98	-6.21
4	-0.45	0.18	-0.18	32.69	-20.59	-8.83
5	0.40	0.18	-0.18	32.69	7.19	18.96
6	-0.29	0.18	-0.18	32.69	-15.36	-3.60
1E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
2E	-1.07	0.18	-0.18	32.69	-40.86	-29.09
3E	-0.53	0.18	-0.18	32.69	-23.21	-11.44
4E	-0.48	0.18	-0.18	32.69	-21.57	-9.81
5E	0.61	0.18	-0.18	32.69	14.06	25.82
6E	-0.43	0.18	-0.18	32.69	-19.94	-8.17

* p = qh * (GCpf - GCpi)

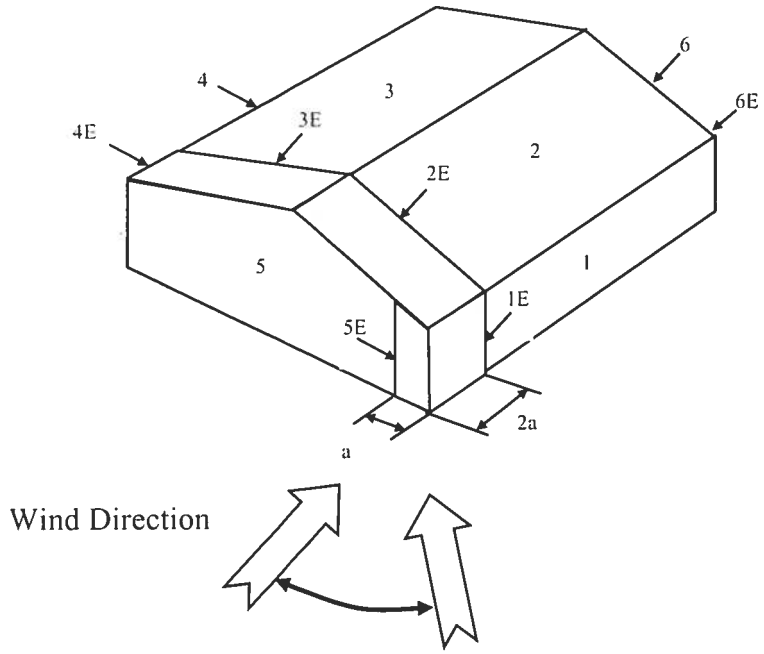
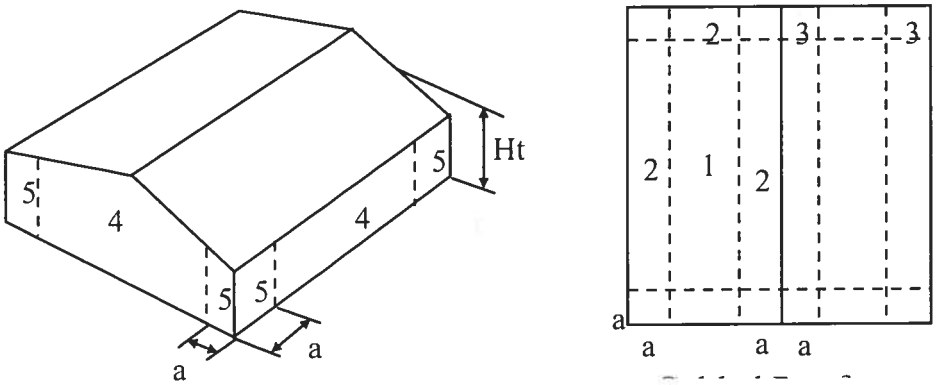


Figure 6-5 - External Pressure Coefficients, GCp

Loads on Components and Cladding for Buildings w/ Ht <= 60 ft



a

Gabled Roof
10 < Theta <= 45

a = 6.368 ==> 6.37 ft

Component	Width (ft)	Length (ft)	Area (ft^2)	Zone	GCp Max	GCp Min	Wind Press (lb/ft^2) Max	Wind Press (lb/ft^2) Min
	16	7	112.00	5	0.81	-1.03	27.13	-32.99
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
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	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					
	0	0	0.00					

Note: * Enter Zone 1 through 5, or 1H through 3H for overhangs.

Table 6-7 Internal Pressure Coefficients for Buildings, Gcpi

Condition	Gcpi Max +	Gcpi Max -
Open Buildings	0.00	0.00
Partially Enclosed Buildings	0.55	-0.55
Enclosed Buildings	0.18	-0.18
Enclosed Buildings	**0.18**	**-0.18**