



To whom it may concern,

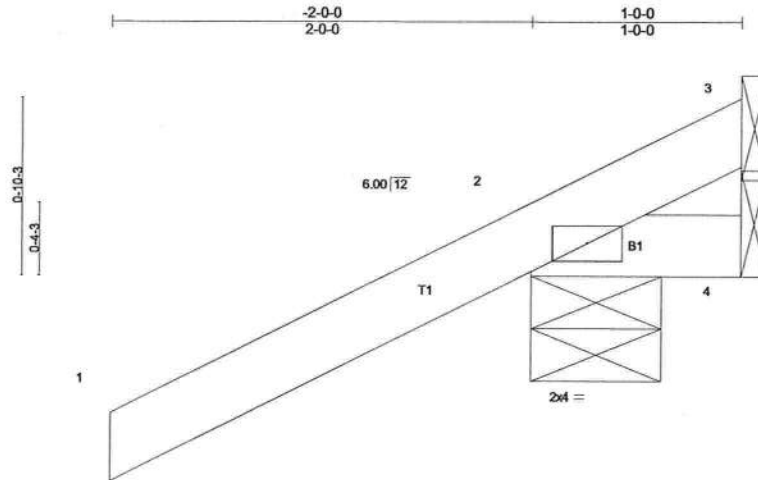
This letter is intended to address the issue of warning notes on 7' jack trusses. I have reviewed the jack truss and it passes without modification for any jack up to 7' with a total loading not to exceed 55# and a maximum overhang of 2'. Below is a copy of note you will see on the jack. This letter will act as an approval for the truss mentioned above.

*****Design Problems*** Review Required/ Max Deflection In Panel Exceeded: A-B**



FL. PE# 34869

Job 300029	Truss CJ1	Truss Type JACK	Qty 14	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029001 Job Reference (optional)
Builders FrstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:53:57 2009 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.35	Vert(LL) -0.00 2 >999 360	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.01	Vert(TL) -0.00 2 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 3 n/a n/a	
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 2 **** 240	Weight: 7 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=265/0-1-8 (input: 0-7-8), 4=5/Mechanical, 3=99/Mechanical
Max Horz 2=106(LC 7)
Max Uplift 2=360(LC 7), 3=99(LC 1)
Max Grav 2=265(LC 1), 4=14(LC 2), 3=172(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-8)
1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (if=lb) 2=360.
6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

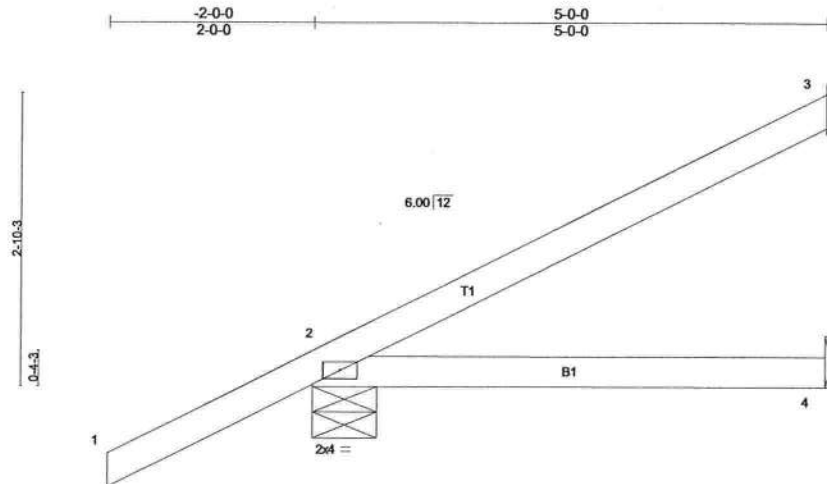
LOAD CASE(S) Standard



Weight: 13 lb

LOAD CASE(S) Standard

Job 300029	Truss CJ5	Truss Type JACK	Qty 8	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029003 Job Reference (optional)
Builders FrstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:53:58 2009 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	L/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.41	Vert(LL) -0.02 2-4	>999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.15	Vert(TL) -0.04 2-4	>999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3	n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 2	**** 240		
					Weight: 19 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=94/Mechanical, 2=304/0-1-8 (input: 0-7-8), 4=23/Mechanical
Max Horz 2=218(LC 7)
Max Uplift 3=101(LC 7), 2=272(LC 7)
Max Grav 3=94(LC 1), 2=304(LC 1), 4=69(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 3=101, 2=272.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



PLATES **GRIP**
MT20 244/190

Weight: 13 lb

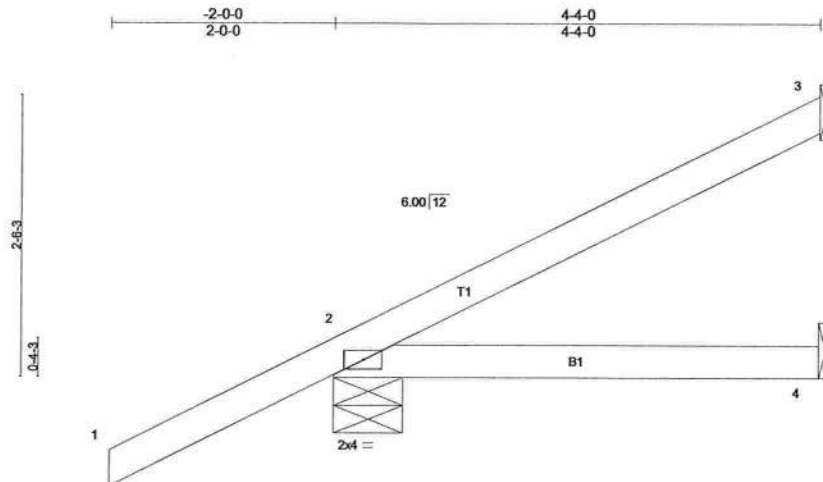
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (7-8)

LOAD CASE(S) Standard

Job 300029	Truss EJ4	Truss Type JACK	Qty 2	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029005 Job Reference (optional)
Builders FrstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:00 2009 Page 1		



Scale = 1:19.8

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.41	Vert(LL) -0.01 2-4 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.11	Vert(TL) -0.02 2-4 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00 3 n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 2 **** 240		Weight: 17 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-4-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

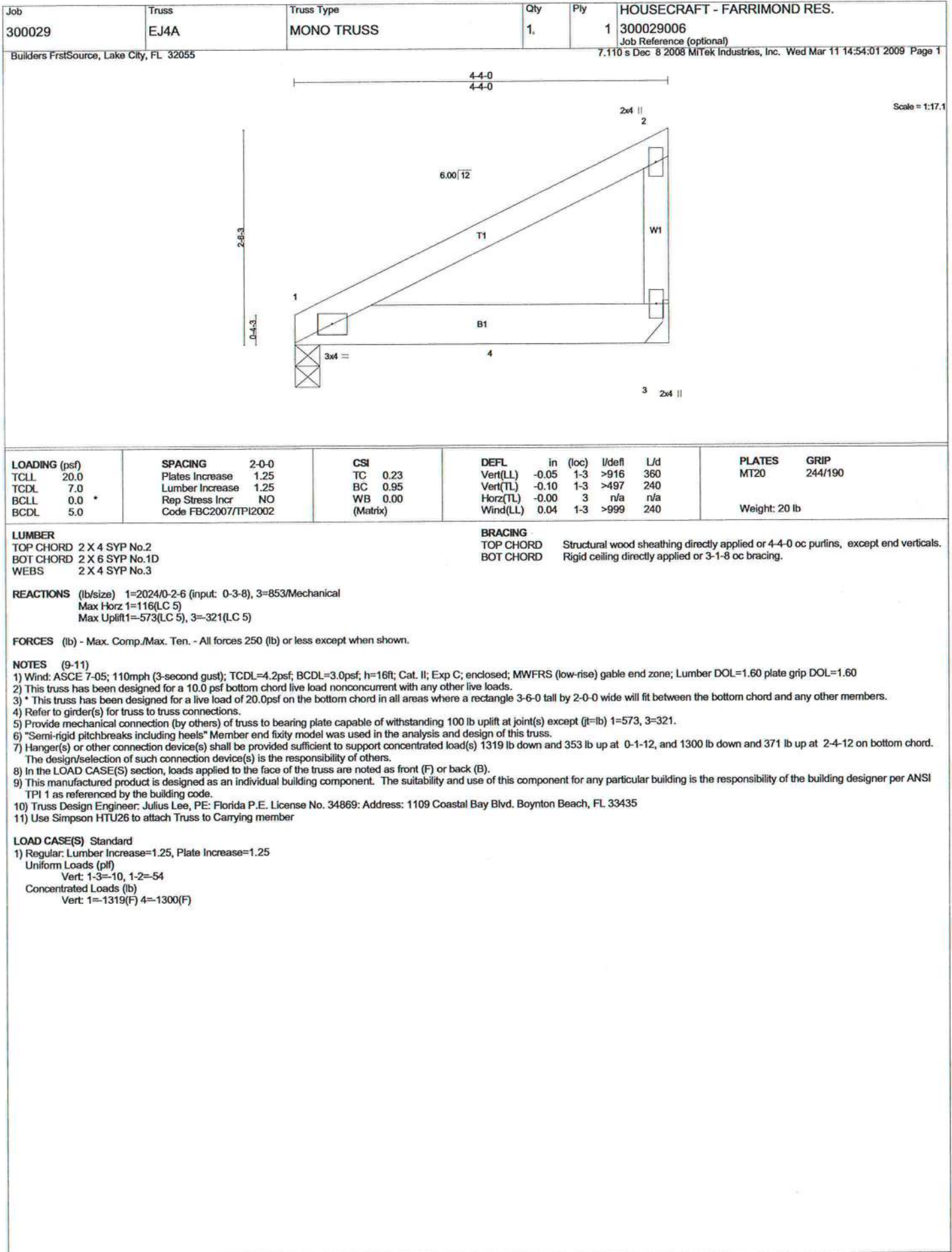
REACTIONS (lb/size) 3=70/Mechanical, 2=288/0-1-8 (input: 0-7-8), 4=20/Mechanical
Max Horz 2=199(LC 7)
Max Uplift 3=72(LC 7), 2=271(LC 7)
Max Grav 3=70(LC 1), 2=288(LC 1), 4=59(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

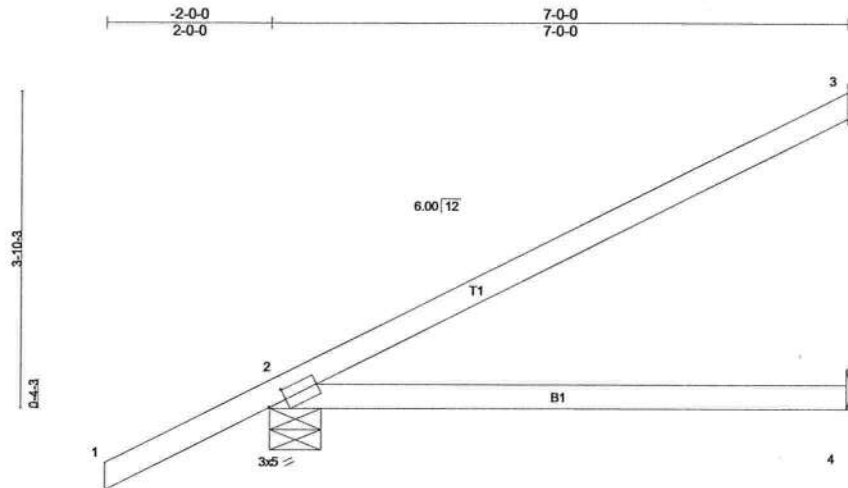
NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (if=lb) 2=271.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job 300029	Truss EJ7	Truss Type MONO TRUSS	Qty 31	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029007 Job Reference (optional)
Builders FrstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:01 2009 Page 1		



Scale = 1/26.9
Camber = 1/16 in

***** Design Problems *****
REVIEW REQUIRED
Max Deflection In Panel Exceeded: 2-3

Plate Offsets (X,Y): [2-0-2-10,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.56	Vert(LL)	-0.08	2-4	>992	360	MT20	244/190
TCDL 7.0	Lumber Increase	1.25	BC 0.27	Vert(TL)	-0.15	2-4	>520	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.08	2-4	>999	240		Weight: 26 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

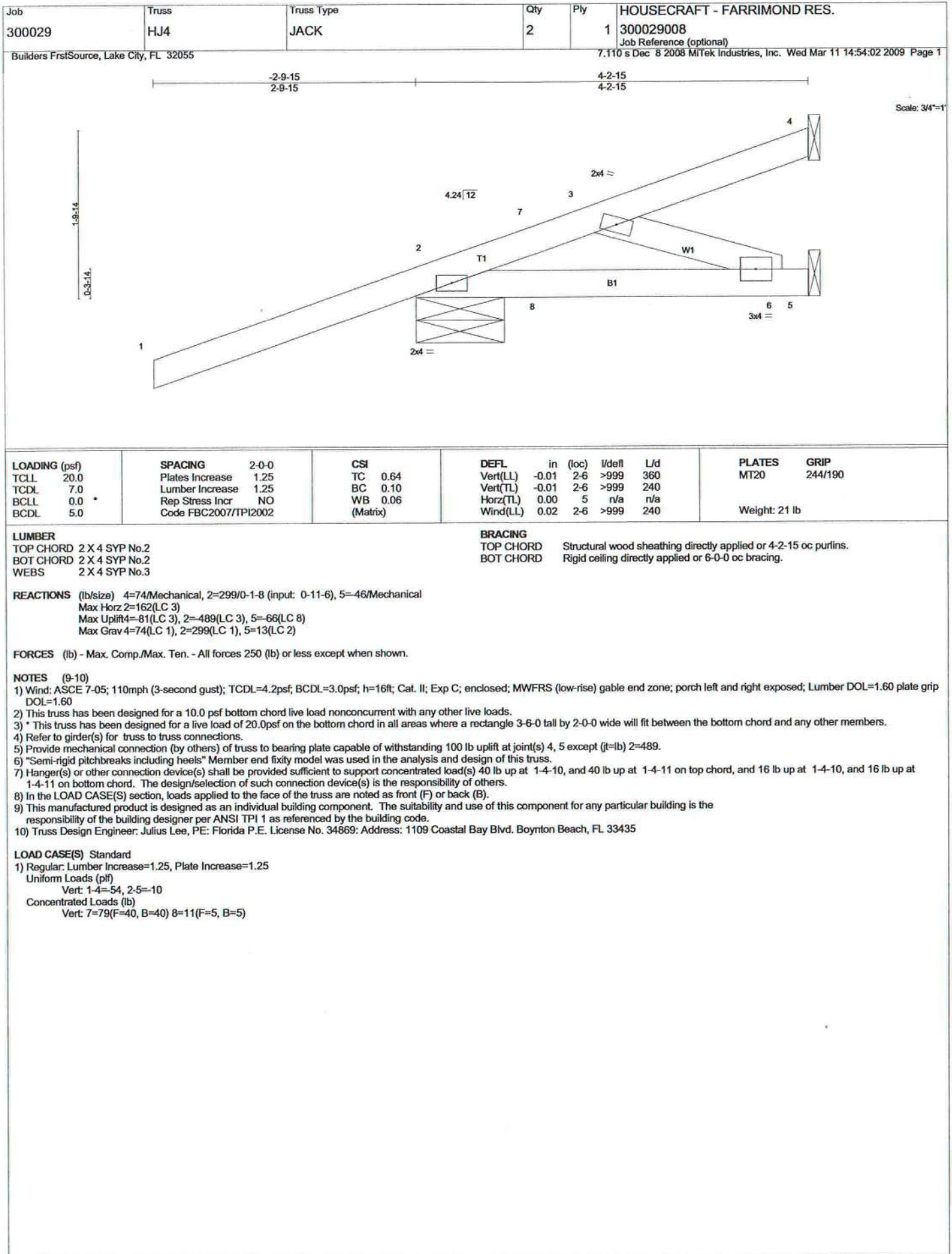
REACTIONS (lb/size) 3=151/Mechanical, 2=359/0-1-8 (input: 0-7-8), 4=39/Mechanical
Max Horz 2=198(LC 7)
Max Uplift 3=108(LC 7), 2=196(LC 7)
Max Grav 3=151(LC 1), 2=359(LC 1), 4=93(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

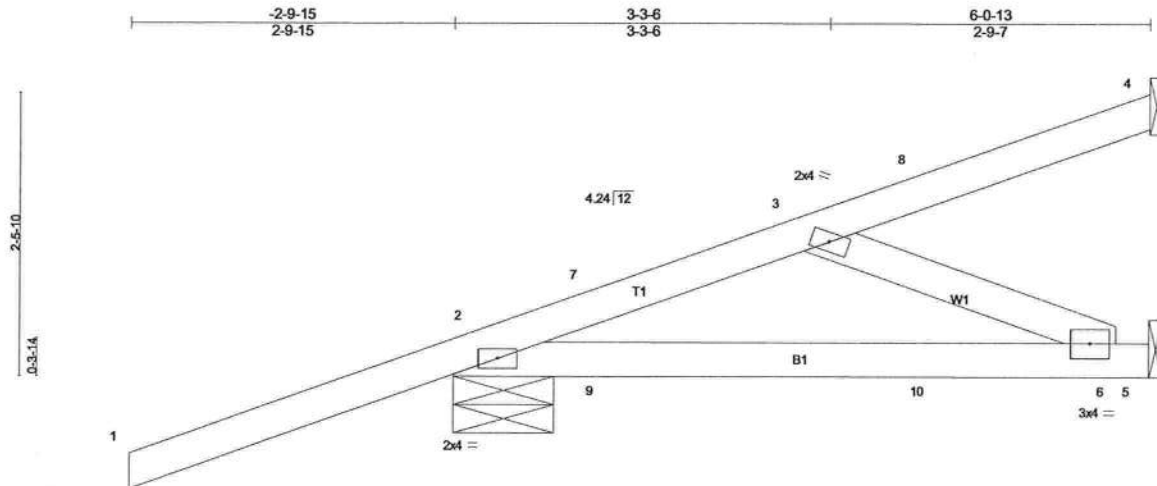
NOTES (7-8)

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6-0 tall by 2'-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 3=108, 2=196.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 8) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



Job 300029	Truss HJ6	Truss Type JACK	Qty 1	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029009 Job Reference (optional)
Builders FirstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:03 2009 Page 1		



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES MT20	GRIP 244/190
TCLL 20.0	Plates Increase 1.25	TC 0.64	Vert(LL) -0.05 2-6 >999 360		
TCDL 7.0	Lumber Increase 1.25	BC 0.25	Vert(TL) -0.09 2-6 >745 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.03	Horz(TL) -0.00 5 n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.00 2-6 >999 240		Weight: 27 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-13 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 4=68/Mechanical, 2=303/0-1-8 (input: 0-10-10), 5=3/Mechanical
Max Horz 2=199(LC 3)
Max Uplift 4=80(LC 3), 2=421(LC 3), 5=24(LC 8)
Max Grav 4=68(LC 1), 2=303(LC 1), 5=68(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (9-10)
- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5 except (if=lb) 2=421.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-3-15, 40 lb up at 1-3-15, and 19 lb down and 38 lb up at 4-1-14, and 19 lb down and 38 lb up at 4-1-14 on top chord, and 16 lb up at 1-3-15, 16 lb up at 1-3-15, and 9 lb down at 4-1-14, and 9 lb down at 4-1-14 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=54, 2-5=10
Concentrated Loads (lb)
Vert: 7=79(F=40, B=40) 8=76(F=38, B=38) 9=11(F=5, B=5) 10=6(F=3, B=3)

Job 300029	Truss HJ9	Truss Type MONO TRUSS	Qty 4	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029010 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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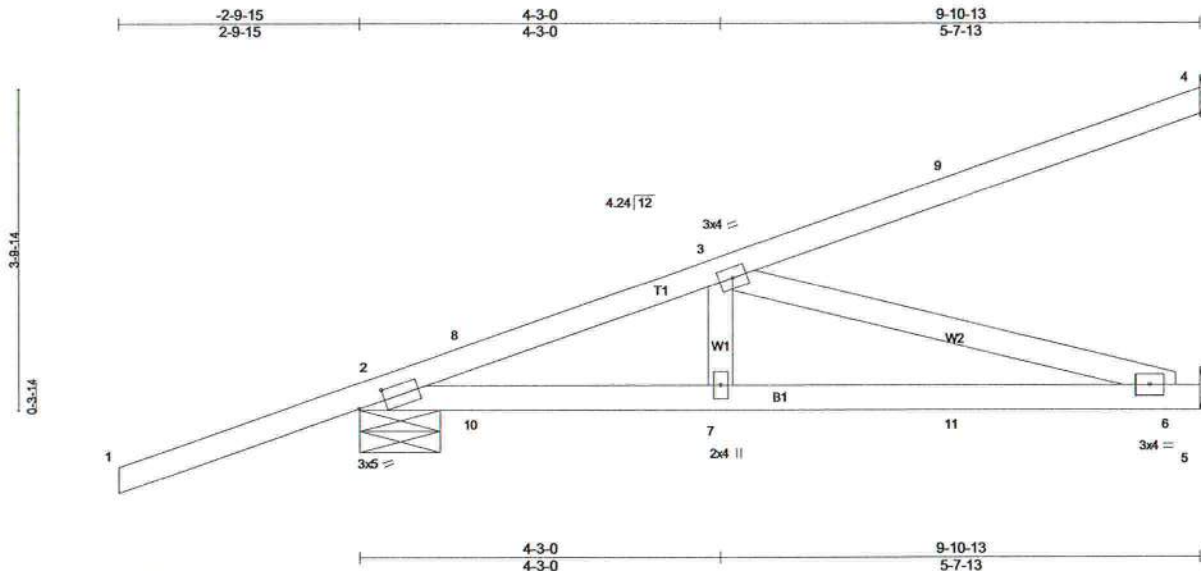


Plate Offsets (X,Y): [2:0-3-12,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.64	Vert(LL)	-0.07	6-7	>999	360	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.45	Vert(TL)	-0.13	6-7	>870	240		
BCLL 0.0	Lumber Increase 1.25	WB 0.24	Horz(TL)	0.01	5	n/a	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Wind(LL)	0.04	6-7	>999	240		
	Code FBC2007/TPI2002							Weight: 45 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 4=163/Mechanical, 2=413/0-1-8 (input: 0-11-6), 5=148/Mechanical
Max Horz 2=276(LC 3)
Max Uplift 4=174(LC 3), 2=479(LC 3), 5=83(LC 6)
Max Grav 4=163(LC 1), 2=413(LC 1), 5=213(LC 2)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=449/281, 3-8=457/275
BOT CHORD 2-10=368/405, 7-10=368/405, 7-11=368/405, 6-11=368/405
WEBS 3-6=421/383

NOTES

- 1) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (it=lb) 4=174, 2=479.
- 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-4-11, 40 lb up at 1-4-11, 19 lb down and 38 lb up at 4-2-10, 19 lb down and 38 lb up at 4-2-10, and 40 lb down and 85 lb up at 7-0-9, and 40 lb down and 85 lb up at 7-0-9 on top chord, and 16 lb up at 1-4-11, 16 lb up at 1-4-11, 9 lb down at 4-2-10, 9 lb down at 4-2-10, and 39 lb down at 7-0-9, and 39 lb down at 7-0-9 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S)

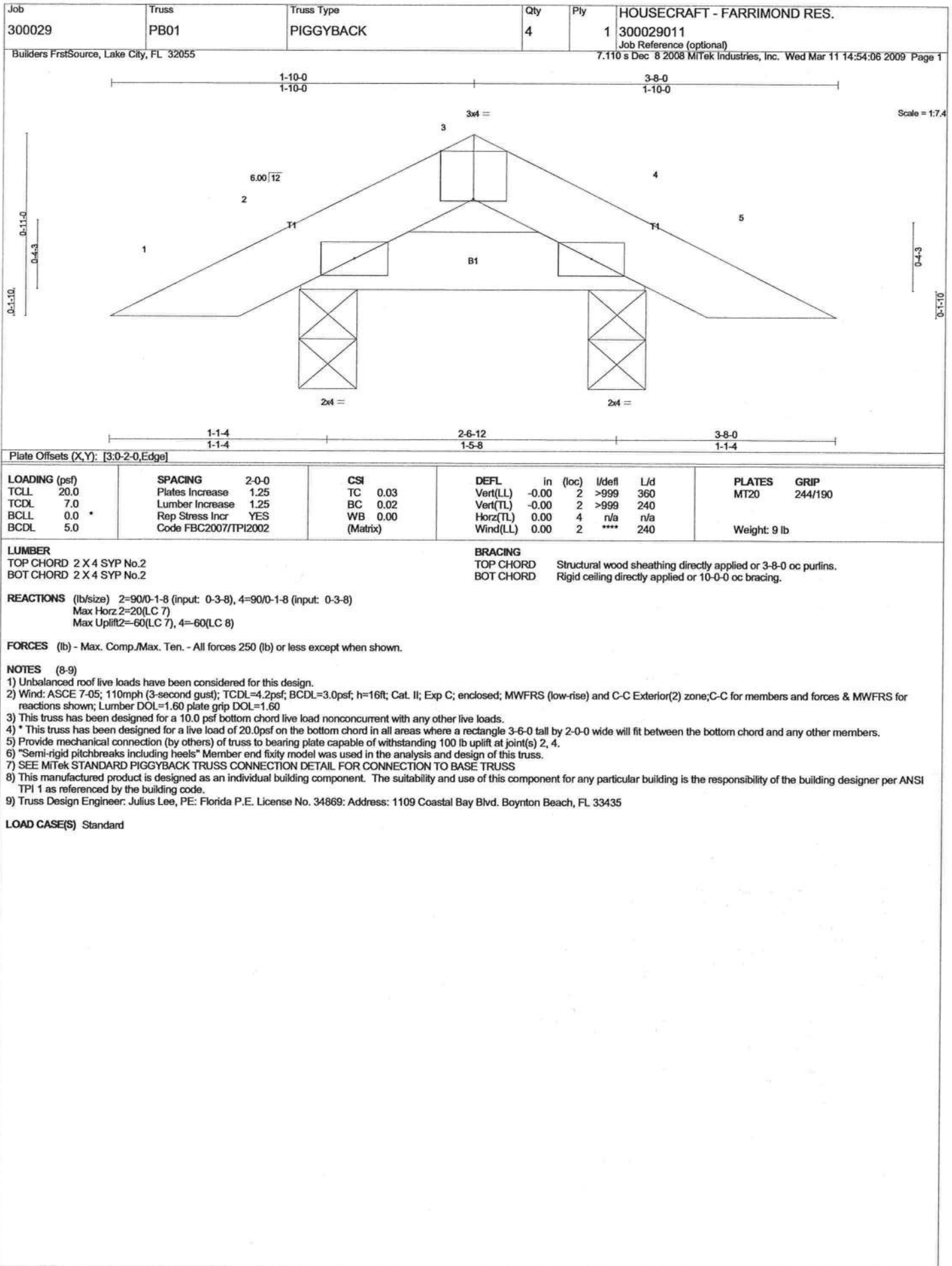
Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 2-5=-10

Concentrated Loads (lb)

Vert: 3=76(F=38, B=38) 7=-6(F=-3, B=3) 8=79(F=40, B=40) 9=-79(F=-40, B=40) 10=11(F=5, B=5) 11=-26(F=-13, B=13)



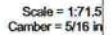


Plate Offsets (X,Y): [2:0-8-0,0-0-6], [3:0-3-0,0-2-0], [6:0-3-0,0-3-0], [7:0-3-0,0-2-0], [8:0-8-0,0-0-6]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.46	Vert(LL) -0.33 11-13 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.73	Vert(TL) -0.67 11-13 >687 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.52	Horz(TL) 0.18 8 n/a n/a		
BCDL 5.0	Code FBC2007/TP12002	(Matrix)	Wind(LL) 0.37 11-13 >999 240	Weight: 365 lb	

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 5-0-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 7-7-3 oc bracing.

Max Horiz 2=95(LC 6)
Max Uplift 2=1209(LC 5), 8=1197(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=4687/2204, 3-15=4127/2011, 15-16=4126/2011, 16-17=4126/2011, 17-18=4126/2011, 18-19=6594/3073, 19-20=6594/3073, 20-21=6594/3073, 21-22=6594/3066, 22-23=6594/3066, 23-24=6594/3066, 24-25=4126/2019, 25-26=4126/2019, 26-27=4126/2019, 27-28=4127/2019, 28-29=4687/2214

BOT CHORD 2-15=1908/4064, 15-27=2932/6266, 27-28=2932/6266, 28-29=2932/6266, 29-30=2932/6266, 30-31=2932/6266, 31-32=3233/6955, 32-33=3233/6955, 33-34=3233/6955, 34-35=3233/6955, 35-36=2905/6266, 36-37=2905/6266, 37-38=2905/6266, 38-39=2905/6266, 39-40=1875/4064

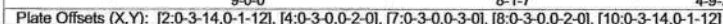
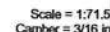
WEBS 3-14=596/1546, 4-14=2525/1215, 4-13=148/738, 5-13=513/352, 5-11=502/342, 6-11=141/738, 6-10=2511/1202, 7-10=589/1539

NOTES (11-12)

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 2=1209, 8=1197.
- 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 206 lb down and 249 lb up at 7-0-0, 97 lb down and 92 lb up at 9-0-12, 97 lb down and 92 lb up at 11-0-12, 97 lb down and 92 lb up at 13-0-12, 97 lb down and 92 lb up at 15-0-12, 97 lb down and 92 lb up at 17-0-12, 97 lb down and 92 lb up at 19-0-12, 97 lb down and 92 lb up at 19-11-4, 97 lb down and 92 lb up at 21-11-4, 97 lb down and 92 lb up at 23-11-4, 97 lb down and 92 lb up at 25-11-4, 97 lb down and 92 lb up at 27-11-4, and 97 lb down and 92 lb up at 29-11-4, and 246 lb down and 249 lb up at 32-0-0 on top chord, and 246 lb down and 76 lb up at 7-0-0, 63 lb down at 9-0-12, 63 lb down at 11-0-12, 63 lb down at 13-0-12, 63 lb down at 15-0-12, 63 lb down at 17-0-12, 63 lb down at 19-0-12, 63 lb down at 19-11-4, 63 lb down at 21-11-4, 63 lb down at 23-11-4, 63 lb down at 25-11-4, 63 lb down at 27-11-4, and 63 lb down at 29-11-4, and 246 lb down and 76 lb up at 31-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code.
- 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-3=54, 3-7=54, 7-9=54, 2-8=10
 Concentrated Loads (lb)
 Vert: 3=206(B) 7=206(B) 14=168(B) 10=168(B) 15=97(B) 16=97(B) 17=97(B) 18=97(B) 19=97(B) 20=97(B) 21=97(B) 22=97(B) 23=97(B)
 24=97(B) 25=97(B) 26=97(B) 27=29(B) 28=29(B) 29=29(B) 30=29(B) 31=29(B) 32=29(B) 33=29(B) 34=29(B) 35=29(B) 36=29(B)
 37=29(B) 38=29(B)

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LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-10-7 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 5-0-14 oc bracing.
WEBS 2 X 4 SYP No.3	

NOTES (8-9)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (ft-lb) 2=438, 10=438.
- 7) *Semi-rigid pitchbreaks including heels* Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.52	Vert(LL) -0.20 10-11 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.49	Vert(TL) -0.34 10-11 >999 240		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.41	Horz(TL) -0.15 8 n/a n/a		
BCDL 5.0	Code FBC2007/TP12002	(Matrix)	Wind(LL) 0.23 11 >999 240	Weight: 208 lb	

TOP CHORD	2 X 4 SYP No.2	BRACING	Structural wood sheathing directly applied or 3-7-2 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 5-4-0 oc bracing.
WEBS	2 X 4 SYP No.3		

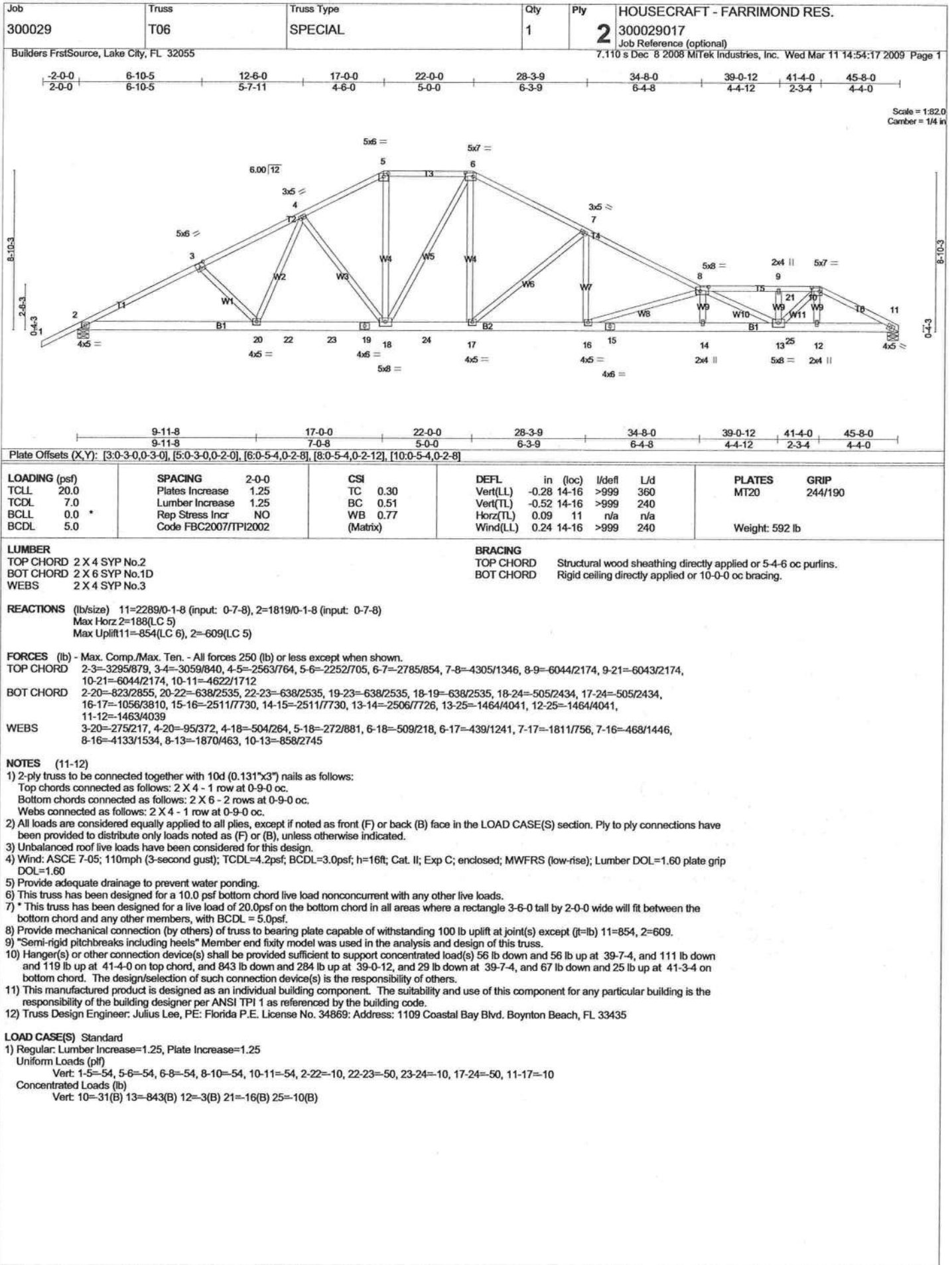
REACTIONS (lb/size) 8=1329/0-1-9 (input: 0-7-8), 2=1461/0-1-12 (input: 0-7-8)
Max Horz 2=155(LC 7)
Max Uplift 8=347(LC 8), 2=476(LC 7)

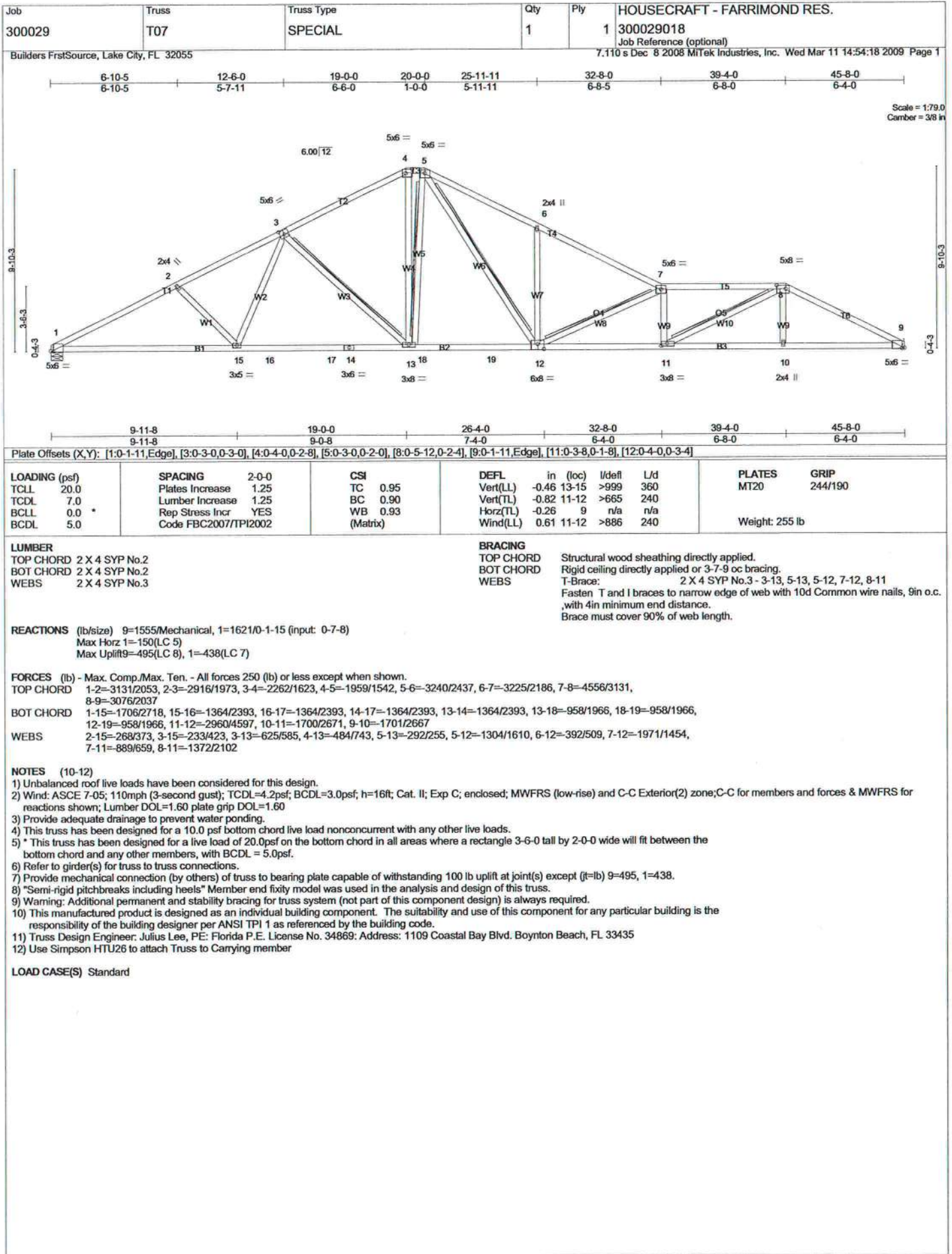
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2511/1624, 3-4=2098/1412, 4-5=2058/1500, 5-6=2058/1500, 6-7=2107/1421, 7-8=2530/1659
BOT CHORD 2-13=1323/2150, 12-13=1323/2150, 12-14=934/1809, 11-14=934/1809, 11-15=941/1816, 10-15=941/1816, 9-10=1363/2190, 8-9=1363/2190
WEBS 3-12=400/443, 4-12=171/342, 4-11=225/478, 5-11=396/348, 6-11=212/471, 6-10=187/357, 7-10=438/482

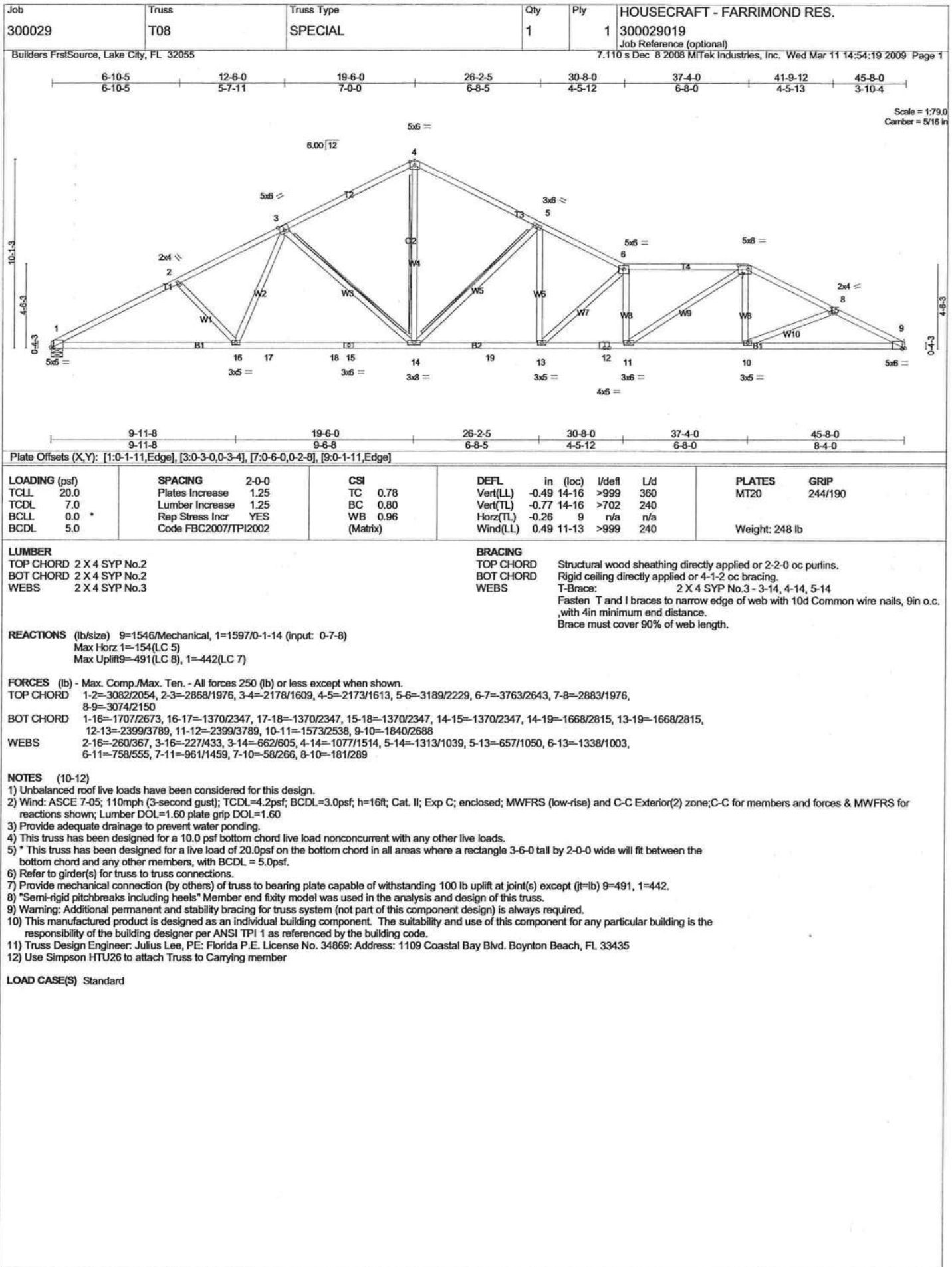
NOTES (8-9)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=347, 2=476.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 9) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard







Job 300029	Truss T09	Truss Type SPECIAL	Qty 1	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029020 Job Reference (optional)
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Builders FrstSource, Lake City, FL 32055

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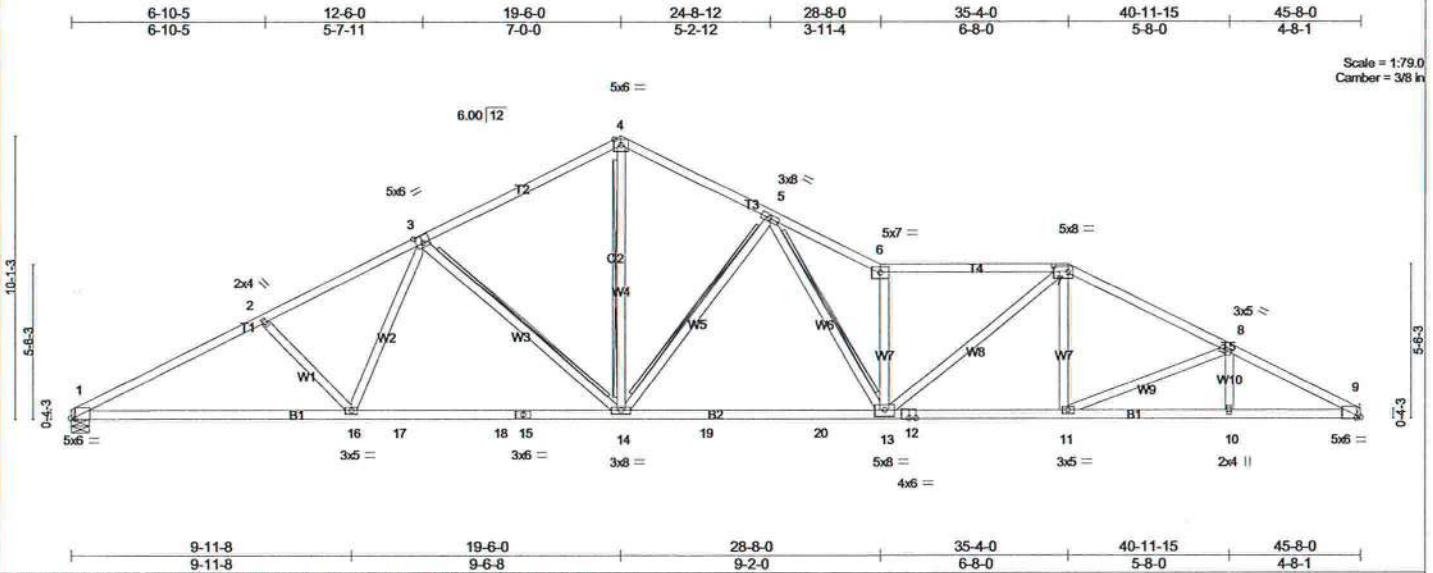


Plate Offsets (X,Y): [1:0-1-11,Edge], [3:0-3-0,0-3-4], [7:0-6-0,0-2-8], [9:0-1-11,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.68	TC -0.54 13-14 >999 360	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.79	Vert(TL) -0.89 13-14 >611 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.97	Horz(TL) -0.24 9 n/a n/a		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL) 0.47 13-14 >999 240		
				Weight: 250 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-8-5 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 3-14, 4-14, 5-14, 5-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 9=1574/Mechanical, 1=1623/0-1-15 (input: 0-7-8)
Max Horz 1=154(LC 5)
Max Uplift 9=491(LC 8), 1=442(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

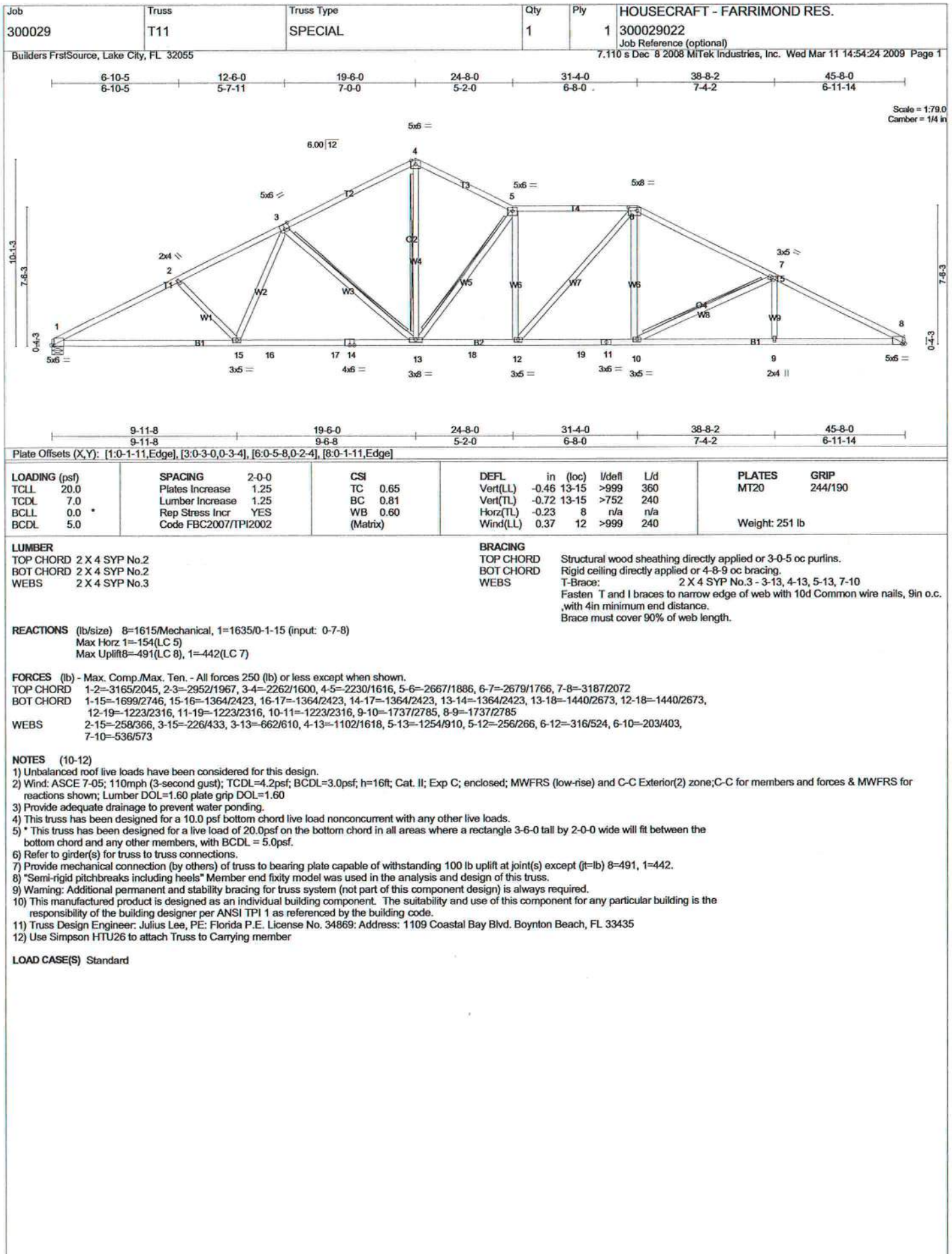
TOP CHORD 1-2=3132/2051, 2-3=2919/1973, 3-4=2245/1605, 4-5=2217/1624, 5-6=3866/2694, 6-7=3367/2311, 7-8=2818/1914, 8-9=3161/2117
BOT CHORD 1-16=1704/2718, 16-17=1369/2400, 17-18=1369/2400, 15-18=1369/2400, 14-15=1369/2400, 14-19=1469/2611, 19-20=1469/2611, 13-20=1469/2611, 12-13=1465/2464, 11-12=1465/2464, 10-11=1806/2767, 9-10=1806/2767
WEBS 2-16=260/365, 3-16=227/417, 3-14=650/611, 4-14=1116/1607, 5-14=1161/943, 5-13=1124/1639, 6-13=1974/1448, 7-13=668/1156, 7-11=117/263, 8-11=346/372

NOTES (10-12)

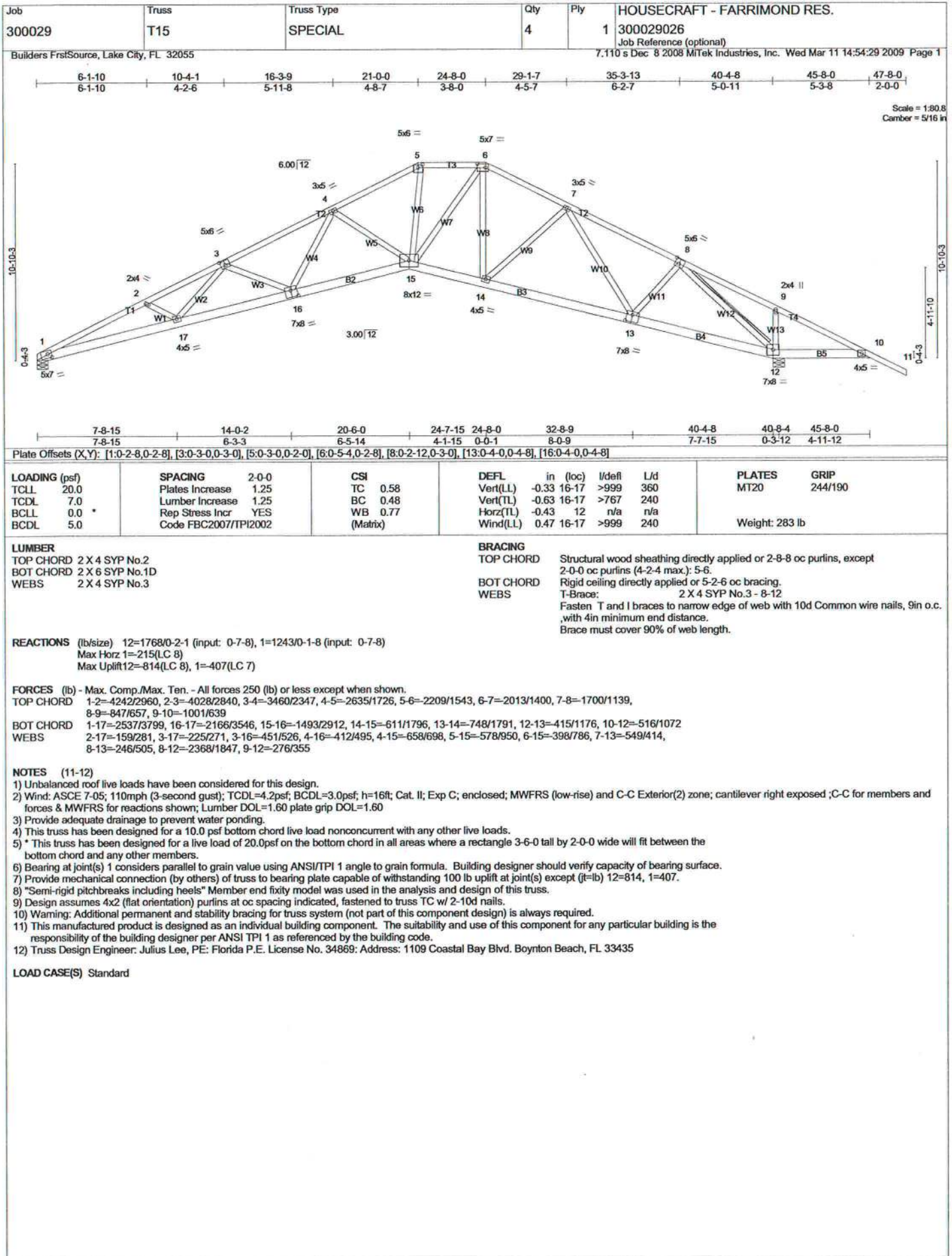
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=491, 1=442.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- Warning: Additional permanent and stability bracing for building system (not part of this component design) is always required.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
- Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

LOAD CASE(S) Standard



LOAD CASE(S) Standard



Job

300029

Truss

T16

Truss Type

HIP

Qty

1

Ply

2

300029027

Job Reference (optional)

FARRIMOND RES.

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-2-0-0

7-0-0

13-5-12

19-8-5

25-11-11

32-2-5

38-8-0

45-8-0

47-8-0

2-0-0

7-0-0

6-5-12

6-2-10

6-3-6

6-2-10

6-5-12

7-0-0

2-0-0

Scale = 1/82.6

Camber = 7/16 in

5x6 =

6.00/12

3

16

17

4 18

19

T2

20

21 5

22

23

24

6 25

26

27

7 28

29

30

8 5x6 =

2

15

31

32

33

34 14

35

36

37

13

38

39

40

12 41

42

43

44

11

9

4x6 =

4x5 =

7x8 =

4x5 =

7x8 =

4x5 =

4x6 =

3-10-3

0-4-3

0-4-3

3-10-3

7-0-0

15-10-8

22-10-0

29-9-8

38-8-0

45-8-0

7-0-0

8-10-8

6-11-8

6-11-8

8-10-8

7-0-0

Plate Offsets (X,Y): [2:0-3-10,0-2-0], [3:0-3-0,0-2-0], [6:0-3-0,0-3-0], [8:0-3-0,0-2-0], [9:0-3-10,0-2-0], [12:0-4-0,0-4-8], [14:0-4-0,0-4-8]

LOADING (psf)

TCLL 20.0

TCDL 7.0

BCLL 0.0

BCDL 5.0

SPACING

2-0-0

Plates Increase 1.25

Lumber Increase 1.25

Rep Stress Incr NO

Code FBC2007/TP12002

CSI

TC 0.59

BC 0.54

WB 0.70

(Matrix)

DEFL

in (loc)

l/defl

L/d

Vert(LL) -0.46 13 >999 360

Vert(TL) -0.90 13 >599 240

Horz(TL) 0.16 9 n/a n/a

Wind(LL) 0.52 13 >999 240

PLATES

GRIP

MT20

244/190

Weight: 507 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.1D

WEBS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-10-14 oc purlins.

BOT CHORD Rigid ceiling directly applied or 8-9-15 oc bracing.

REACTIONS

(lb/size) 2=2886/0-1-11 (input: 0-7-8), 9=2886/0-1-11 (input: 0-7-8)

Max Horz 2=98(LC 6)

Max Uplift 2=1363(LC 5), 9=1350(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=5628/2624, 3-16=5002/2399, 16-17=5001/2399, 17-18=5001/2399, 4-18=5001/2399, 4-19=8566/3956, 19-20=8566/3956, 20-21=8566/3956, 5-21=8566/3956, 5-22=9433/4341, 22-23=9433/4341, 23-24=9433/4341, 6-24=9433/4341, 6-25=8567/3952, 25-26=8567/3952, 26-27=8567/3952, 7-27=8567/3952, 7-28=5001/2409, 28-29=5001/2409, 29-30=5001/2410, 8-30=5002/2410, 8-9=5628/2636

BOT CHORD 2-15=2287/4921, 15-31=3681/7929, 31-32=3681/7929, 32-33=3681/7929, 33-34=3681/7929, 14-34=3681/7929, 14-35=4304/9361, 35-36=4304/9361, 36-37=4304/9361, 13-37=4304/9361, 13-38=4291/9361, 38-39=4291/9361, 39-40=4291/9361, 12-40=4291/9361, 12-41=3645/7929, 41-42=3645/7929, 42-43=3645/7929, 43-44=3645/7929, 11-44=3645/7929, 9-11=2258/4921

WEBS 3-15=755/1929, 4-15=3396/1613, 4-14=366/1142, 5-14=1096/605, 5-13=0/265, 6-13=0/265, 6-12=1087/597, 7-12=360/1135, 7-11=3384/1601, 8-11=749/1923

NOTES (11-12)

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-7-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-7-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 2=1363, 9=1350.

9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 206 lb down and 249 lb up at 7-0-0, 97 lb down and 92 lb up at 9-0-12, 97 lb down and 92 lb up at 11-0-12, 97 lb down and 92 lb up at 13-0-12, 97 lb down and 92 lb up at 15-0-12, 97 lb down and 92 lb up at 17-0-12, 97 lb down and 92 lb up at 19-0-12, 97 lb down and 92 lb up at 21-0-12, 97 lb down and 92 lb up at 22-10-0, 97 lb down and 92 lb up at 24-7-4, 97 lb down and 92 lb up at 26-7-4, 97 lb down and 92 lb up at 28-7-4, 97 lb down and 92 lb up at 30-7-4, 97 lb down and 92 lb up at 32-7-4, 97 lb down and 92 lb up at 34-7-4, and 97 lb down and 92 lb up at 36-7-4, and 246 lb down and 249 lb up at 38-8-0 on top chord, and 246 lb down and 76 lb up at 7-0-0, 63 lb down at 9-0-12, 63 lb down at 11-0-12, 63 lb down at 13-0-12, 63 lb down at 15-0-12, 63 lb down at 17-0-12, 63 lb down at 19-0-12, 63 lb down at 21-0-12, 63 lb down at 22-10-0, 63 lb down at 24-7-4, 63 lb down at 26-7-4, 63 lb down at 28-7-4, 63 lb down at 30-7-4, 63 lb down at 32-7-4, 63 lb down at 34-7-4, and 63 lb down at 36-7-4, and 246 lb down and 76 lb up at 38-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=54, 3-8=54, 8-10=54, 2-9=10

Continued on page 2

Job 300029	Truss T16	Truss Type HIP	Qty 1	Ply 2	HOUSECRAFT - FARRIMOND RES. 300029027 Job Reference (optional)
Builders FrstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:33 2009 Page 2		
LOAD CASE(S) Standard Concentrated Loads (lb) Vert 3=206(F) 8=206(F) 15=168(F) 13=29(F) 11=168(F) 16=97(F) 17=97(F) 18=97(F) 19=97(F) 20=97(F) 21=97(F) 22=97(F) 23=97(F) 24=97(F) 25=97(F) 26=97(F) 27=97(F) 28=97(F) 29=97(F) 30=97(F) 31=29(F) 32=29(F) 33=29(F) 34=29(F) 35=29(F) 36=29(F) 37=29(F) 38=29(F) 39=29(F) 40=29(F) 41=29(F) 42=29(F) 43=29(F) 44=29(F)					

Job: 300029 Truss: T17 Truss Type: HIP Qty: 1 Ply: 1 HOUSECRAFT - FARRIMOND RES. 300029028 Job Reference (optional) 7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:34 2009 Page 1

Builders FrstSource, Lake City, FL 32055

Scale = 1/8" = 1'-0" Camber = 3/8" in

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-2-0], [6:0-3-0,0-3-0], [8:0-3-0,0-2-0], [10:0-1-11,Edge]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.54	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.68	Vert(LL) -0.38 14-15 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.51	Vert(TL) -0.76 14-15 >709 240		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.27 10 n/a n/a		
	Code FBC2007/TPI2002		Wind(LL) 0.54 14-15 >999 240		
				Weight: 230 lb	

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-2-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-2-8 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-17, 7-12
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1566/0-1-14 (input: 0-7-8), 10=1566/0-1-14 (input: 0-7-8)
Max Horz 2=110(LC 8)
Max Uplift 2=478(LC 6), 10=478(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2712/1910, 3-4=2537/1787, 4-5=2250/1678, 5-6=3389/2443, 6-7=3389/2443, 7-8=2250/1678, 8-9=2537/1787, 9-10=2712/1910
BOT CHORD 2-17=1493/2326, 16-17=2038/3245, 15-16=2038/3245, 14-15=2262/3562, 13-14=2048/3245, 12-13=2048/3245, 10-12=1537/2326
WEBS 4-17=516/811, 5-17=1259/862, 5-15=131/357, 6-15=291/233, 6-14=291/233, 7-14=131/357, 7-12=1259/863, 8-12=516/811

NOTES (9-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 2=478, 10=478.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job 300029 Truss T18 Truss Type HIP Qty 1 Ply 1 HOUSECRAFT - FARRIMOND RES. 300029029 Job Reference (optional) Builders FirstSource, Lake City, FL 32055 7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:36 2009 Page 1

Scale = 1/8" = 1'-0" Camber = 5/16" in

Plate Offsets (X,Y): [2:0-1-11,Edge], [4:0-3-0,0-2-0], [6:0-3-0,0-3-0], [8:0-3-0,0-2-0], [10:0-1-11,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.50	Vert(LL)	-0.42	13-15	>999	360	
TCDL 7.0	Lumber Increase	1.25	BC 0.73	Vert(TL)	-0.73	13-15	>740	240	
BCLL 0.0	Rep Stress Incr	YES	WB 0.40	Horz(TL)	-0.23	10	n/a	n/a	
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)	Wind(LL)	0.42	15-16	>999	240	

Weight: 247 lb

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-8-12 oc bracing.
WEBS T-Brace: 2 X 4 SYP No.3 - 5-18, 7-13
Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.
Brace must cover 90% of web length.

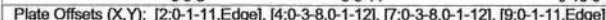
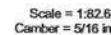
REACTIONS (lb/size) 2=1666/0-1-15 (input: 0-7-8), 10=1666/0-1-15 (input: 0-7-8)
Max Horz 2=124(LC 8)
Max Uplift 2=496(LC 7), 10=496(LC 8)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=2943/1909, 3-4=2663/1753, 4-5=2343/1652, 5-6=3147/2122, 6-7=3147/2121, 7-8=2343/1652, 8-9=2663/1753, 9-10=2943/1909
BOT CHORD 2-19=1491/2533, 18-19=1491/2533, 17-18=1675/3013, 17-20=1675/3013, 20-21=1675/3013, 16-21=1675/3013, 15-16=1789/3221, 15-22=1681/3013, 22-23=1681/3013, 14-23=1681/3013, 13-14=1681/3013, 12-13=1534/2533, 10-12=1534/2533
WEBS 3-18=243/330, 4-18=499/861, 5-18=991/590, 5-16=99/358, 7-15=99/358, 7-13=991/590, 8-13=498/861, 9-13=243/329

NOTES (9-10)
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 2=496, 10=496.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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PLATES	GRIP
MT20	244/190
Weight: 243 lb	

BRACING	
TOP CHORD	Structural wood sheathing directly applied or 3-4-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-1-6 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 5-16, 6-12 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3053/1919, 3-4=-2671/1709, 4-5=-2333/1617, 5-6=-2925/1894, 6-7=-2333/1617, 7-8=-2671/1709, 8-9=-3053/1918

BOT CHORD 2-17=-1490/2629, 16-17=-1490/2629, 15-16=-1446/2848, 15-18=-1446/2848, 18-19=-1446/2848, 14-19=-1446/2848, 14-20=-1449/2848, 20-21=-1449/2848, 13-21=-1449/2848, 12-13=-1449/2848, 11-12=-1531/2629, 9-11=-1531/2629

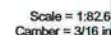
WEBS 3-16=-360/436, 4-16=-453/834, 5-16=-840/425, 5-14=-5/250, 6-14=-5/250, 6-12=-840/425, 7-12=-453/834, 8-12=-360/436

NOTES (9-10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=515, 9=515.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE; Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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Weight: 246 lb

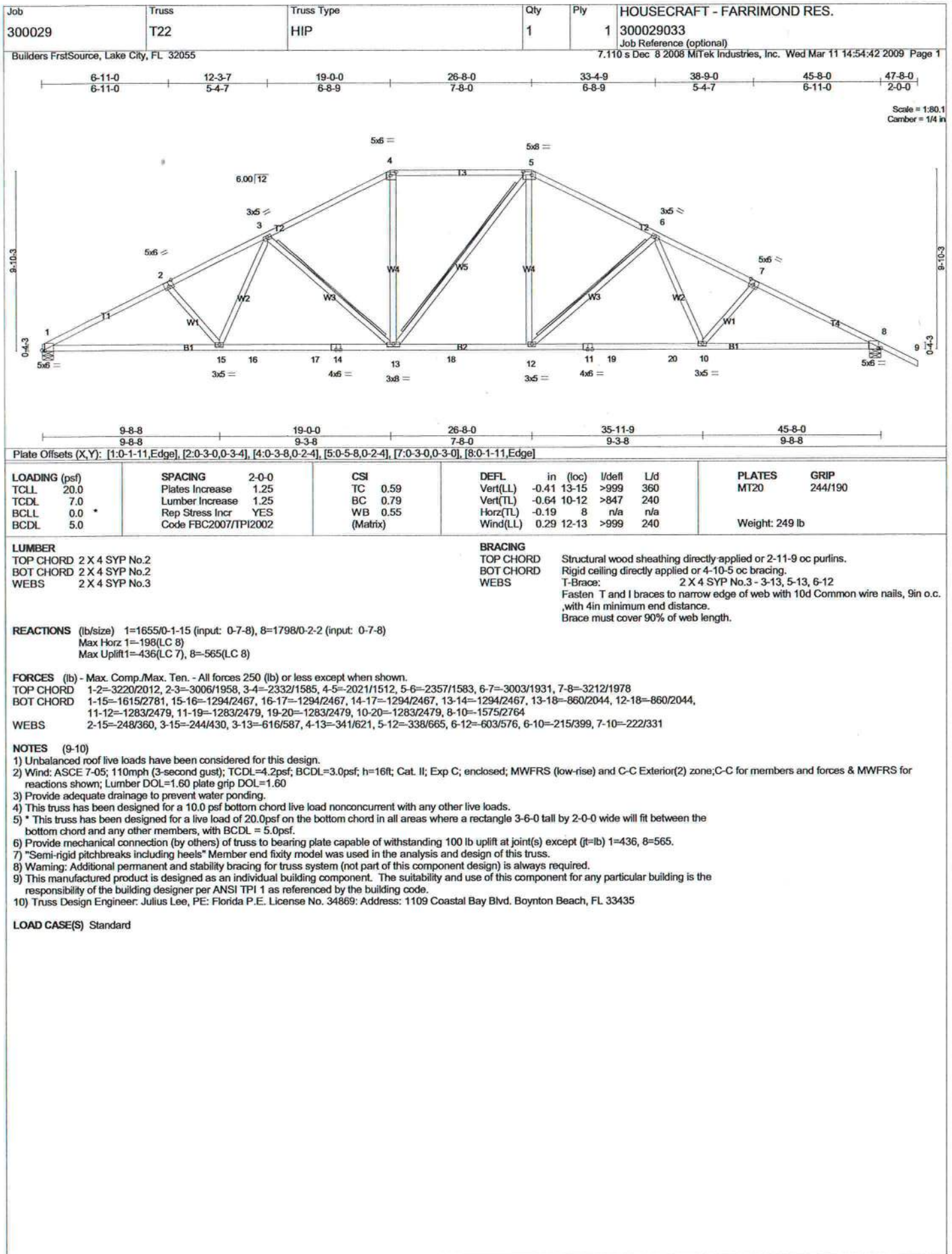
TOP CHORD	Structural wood sheathing directly applied or 3-1-7 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 5-0-8 oc bracing.
WEBS	T-Brace: 2 X 4 SYP No.3 - 3-15, 7-11 Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance. Brace must cover 90% of web length.

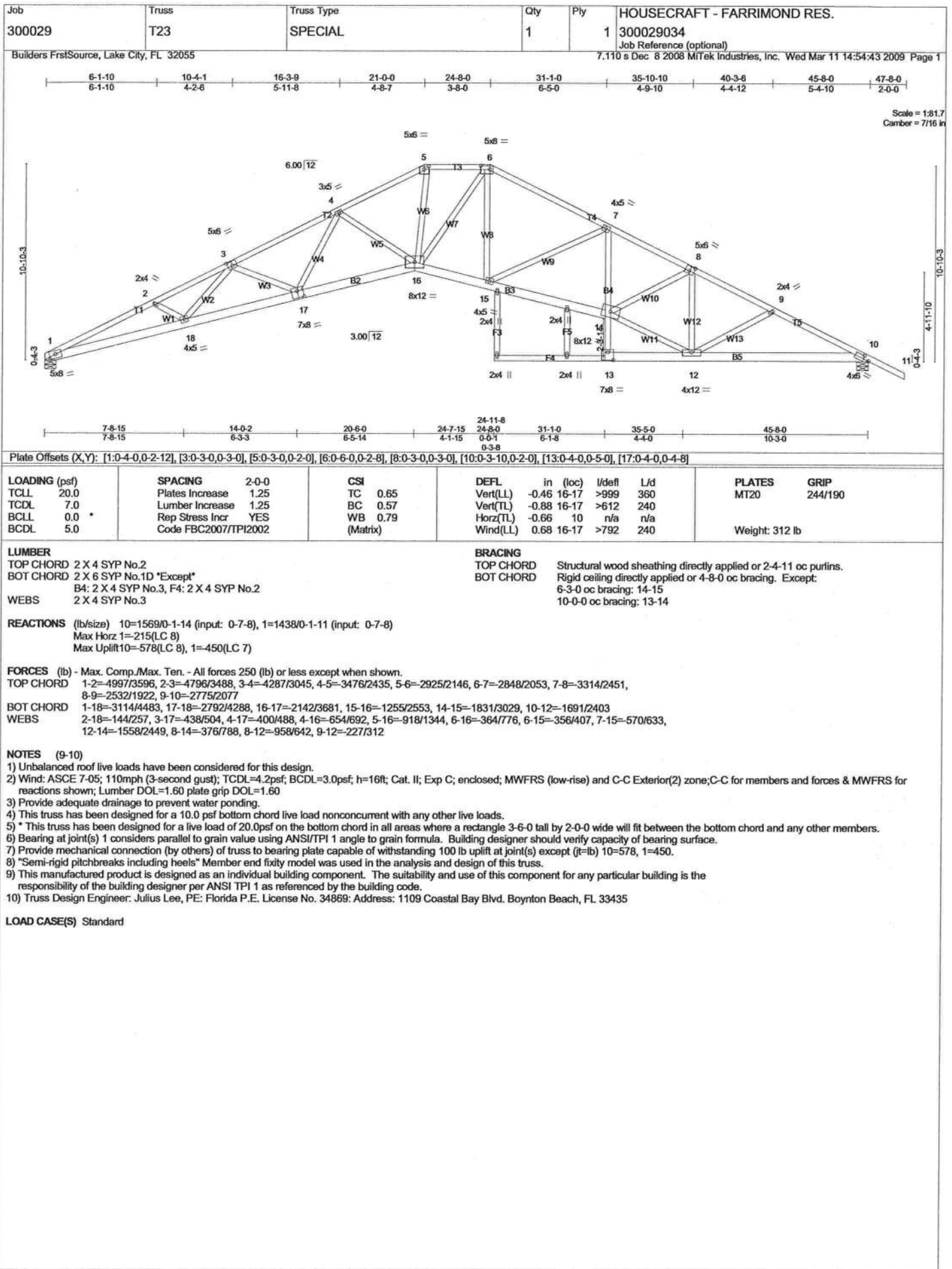
Max Horiz 2=153(LC 8)
Max Uplift 2=533(LC 7), 8=533(LC 8)

2-3=3031/1916, 3-4=2545/1663, 4-5=2526/1770, 5-6=2526/1770, 6-7=2545/1663, 7-8=3031/1916
2-16=1481/2605, 15-16=1481/2605, 14-15=1047/2199, 14-17=1047/2199, 13-17=1047/2199, 13-18=1053/2199, 12-18=1053/2199,
11-12=1053/2199, 10-11=1514/2605, 8-10=1514/2605
3-15=478/530, 4-13=211/426, 4-13=275/599, 5-13=481/426, 6-13=275/599, 6-11=211/426, 7-11=478/530

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (if=lb) 2=533, 8=533.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.
- 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	HOUSECRAFT - FARRIMOND RES.
300029	T24	SPECIAL	1	1	300029035 Job Reference (optional)

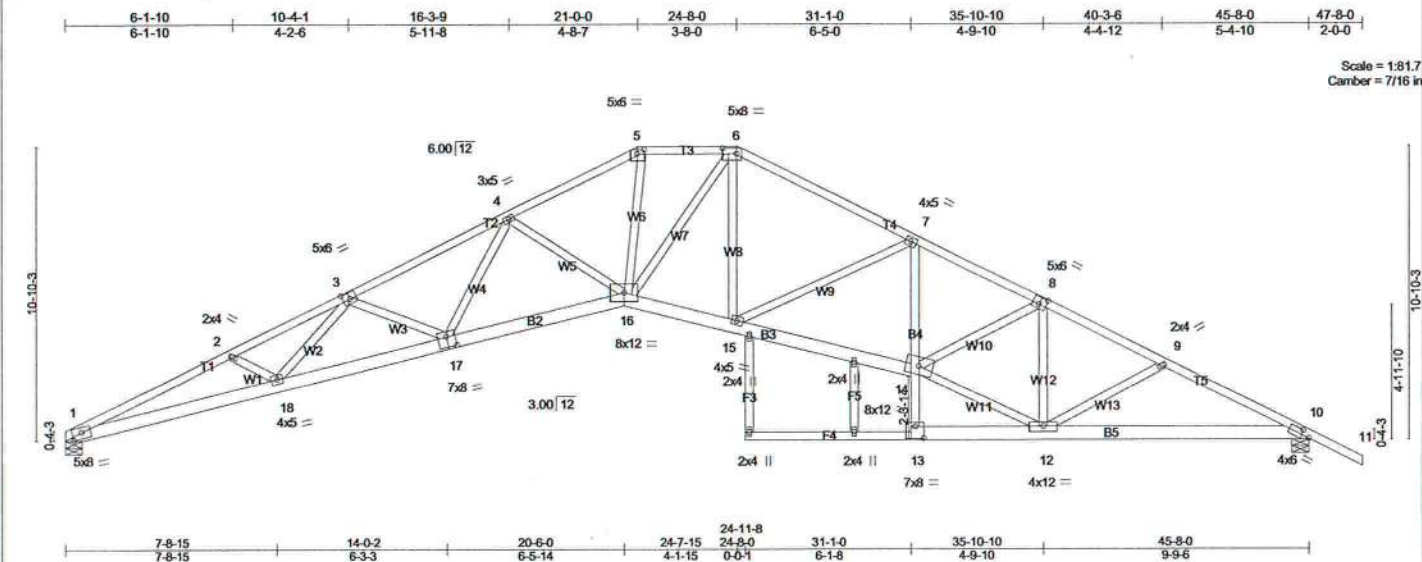
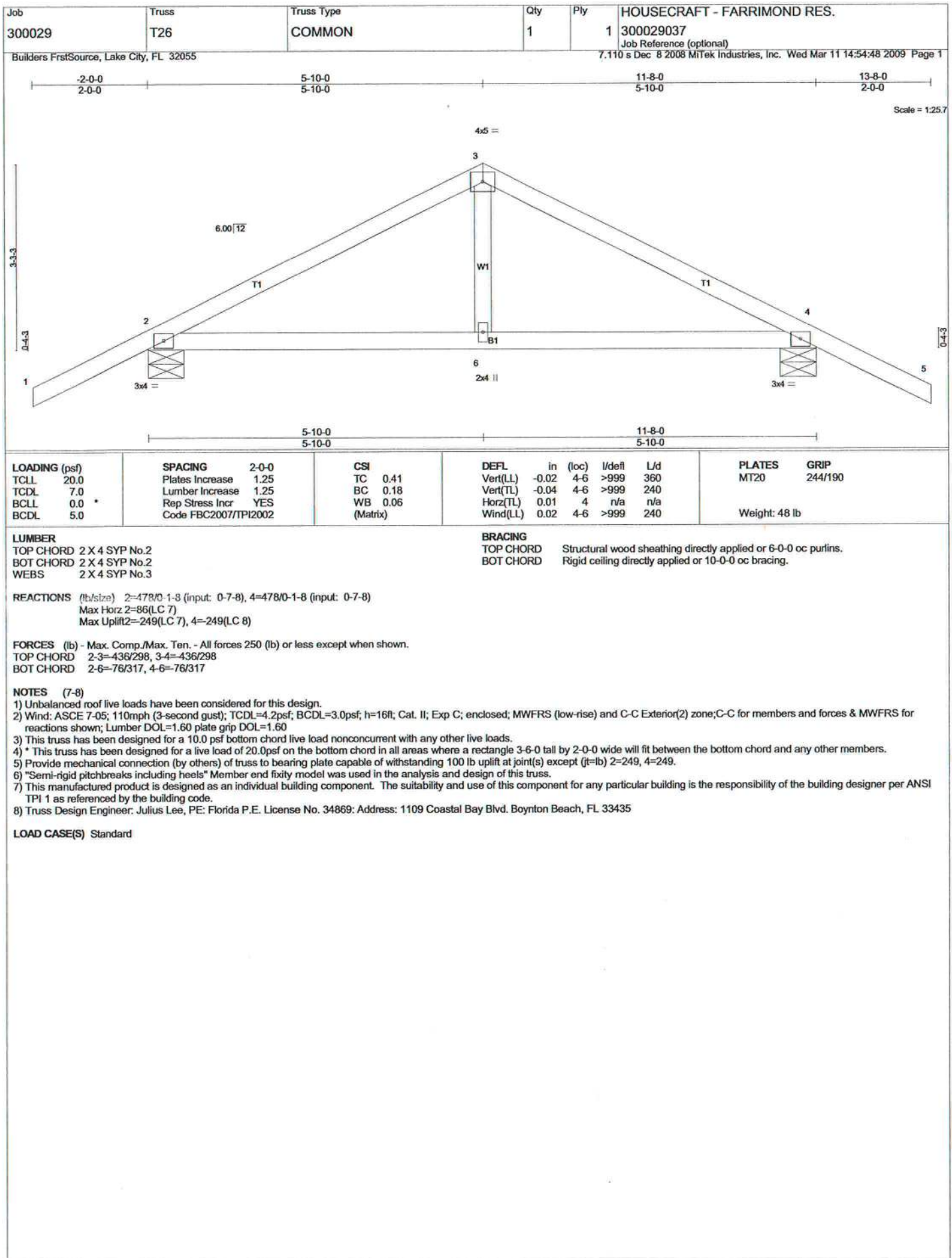


Plate Offsets (X,Y): [1:0-4,0-2-12], [3:0-3,0-3-0], [5:0-3,0-2-0], [6:0-6,0-2-8], [8:0-3,0-3-0], [10:0-3,10,0-2-0], [13:0-4,0-5-0], [17:0-4,0-4-8]									
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 20.0	Plates Increase 1.25	TC 0.65	Vert(LL)	-0.46 16-17	>999	360	MT20	244/190	
TCDL 7.0	Lumber Increase 1.25	BC 0.57	Vert(TL)	-0.88 16-17	>612	240			
BCLL 0.0	Rep Stress Incr YES	WB 0.79	Horz(TL)	-0.66 10	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Wind(LL)	0.68 16-17	>792	240			
							Weight: 312 lb		

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 6 SYP No.1D *Except*
 B4: 2 X 4 SYP No.3, F4: 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3



Job 300029	Truss T26G	Truss Type GABLE	Qty 1	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029038 Job Reference (optional)
Builders FrstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:50 2009 Page 1		

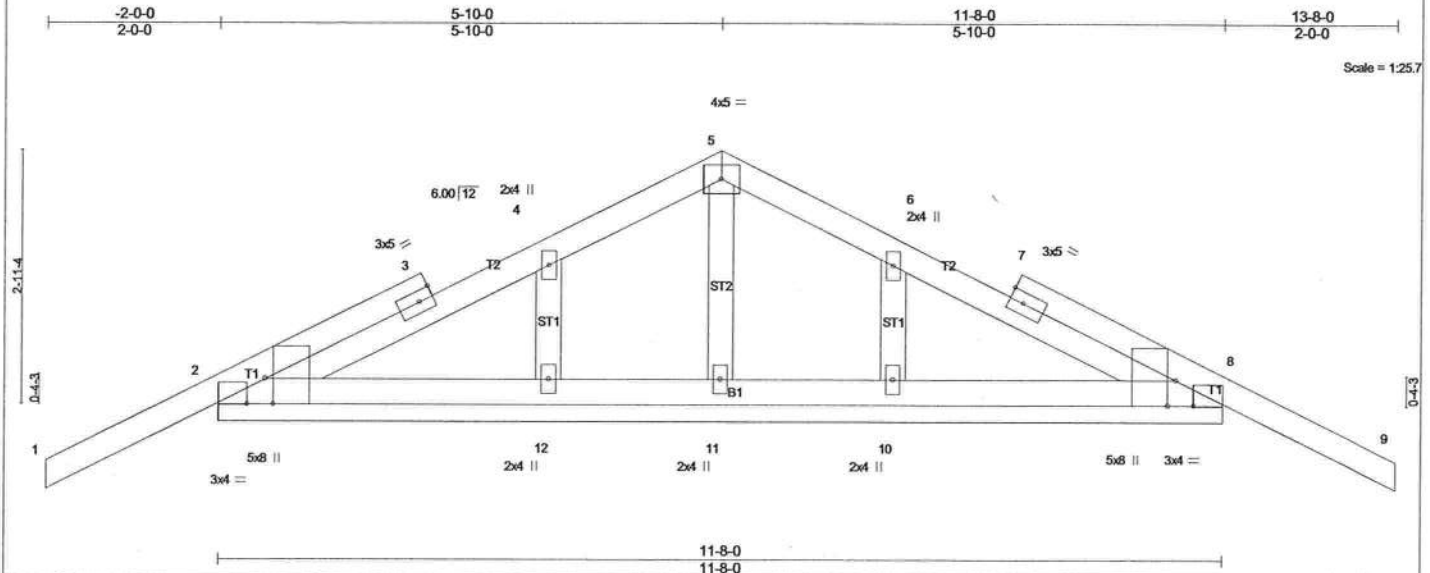


Plate Offsets (X,Y): [2-0-3-8,Edge], [2-0-2-8,Edge], [8-0-3-8,Edge], [8-0-2-8,Edge]					
LOADING (psf)		SPACING	CSI	DEFL	PLATES GRIP
TCLL	20.0	Plates Increase	TC 0.54	in (loc) l/defl L/d	MT20 244/190
TCDL	7.0	Lumber Increase	BC 0.07	Vert(LL) -0.04 9 n/r 120	
BCLL	0.0	Rep Stress Incr	WB 0.09	Vert(TL) -0.06 9 n/r 90	
BCDL	5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.01 8 n/a n/a	
					Weight: 57 lb

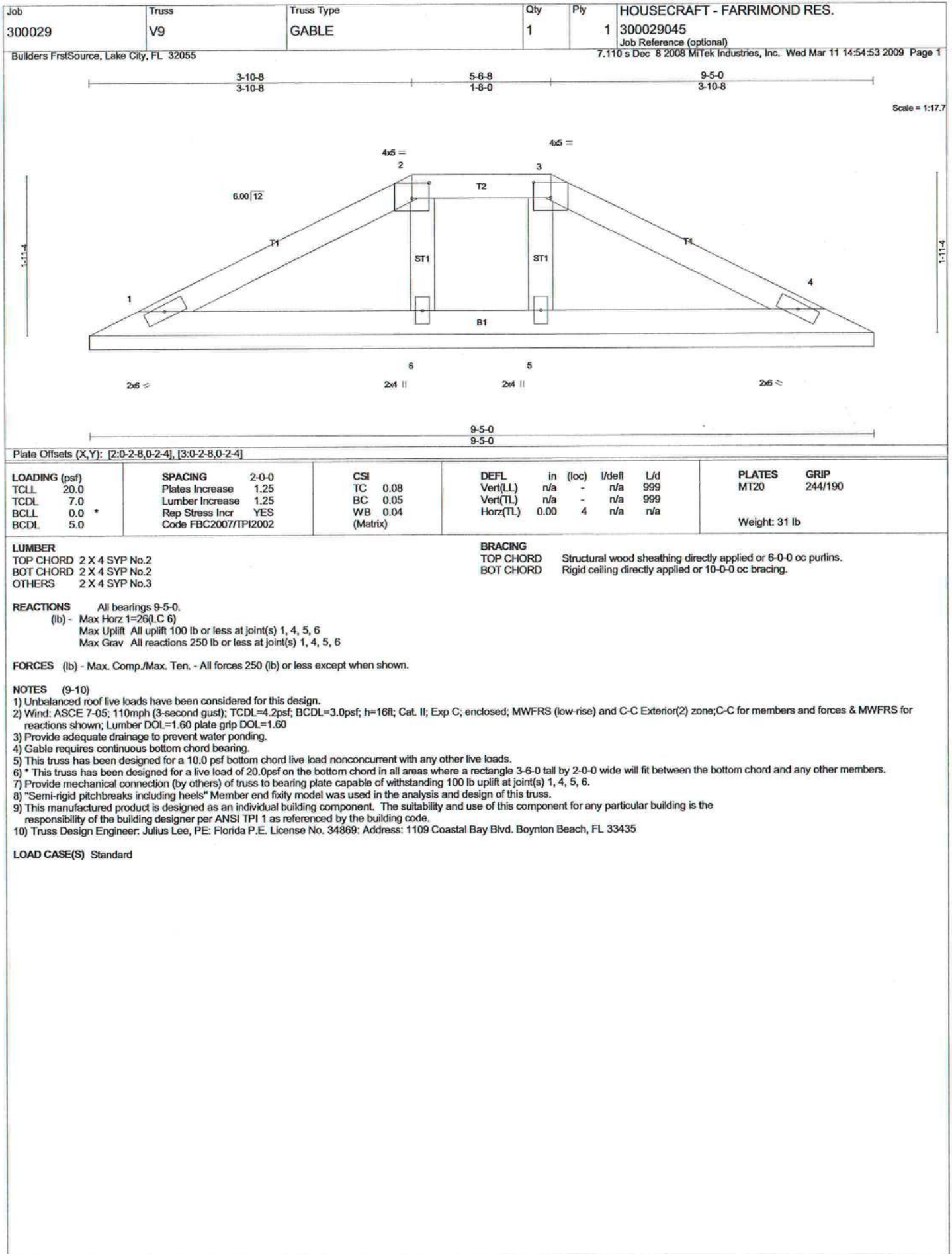
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
OTHERS 2 X 4 SYP No.3	

REACTIONS All bearings 11-8-0.
 (lb) - Max Horz 2=91(LC 7)
 Max Uplift All uplift 100 lb or less at joint(s) 11 except 2=422(LC 7), 8=438(LC 8), 12=230(LC 7), 10=234(LC 8)
 Max Grav All reactions 250 lb or less at joint(s) 11 except 2=496(LC 11), 8=496(LC 12), 12=358(LC 11), 10=358(LC 12)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 4-12=323/369, 6-10=323/368

- NOTES** (11-12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (it=lb) 2=422, 8=438, 12=230, 10=234.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
 Uniform Loads (plf)
 Vert: 1-5=114(F=60), 5-9=114(F=60), 2-8=10



Job 300029	Truss V17	Truss Type VALLEY	Qty 1	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029041 Job Reference (optional)
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Builders FirstSource, Lake City, FL 32055

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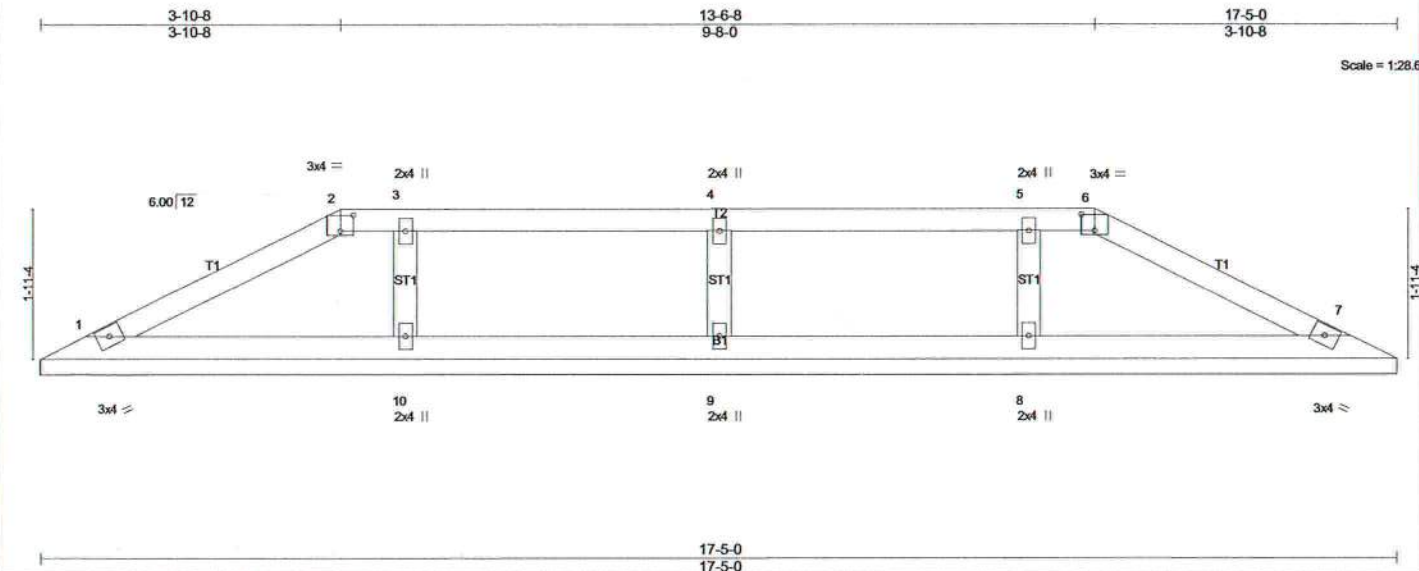


Plate Offsets (X,Y): [2-0-2-0,0-2-8], [6-0-2-0,0-2-8]

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.07	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0	Lumber Increase 1.25	WB 0.05	Horz(TL)	-0.00	7	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2007/TPI2002								
								Weight: 57 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

All bearings 17-5-0.
(lb) - Max Horz 1=26(LC 6)
Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10, 8 except 9=108(LC 5)
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 9=258(LC 11), 10=273(LC 11), 8=273(LC 12)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (9-10)

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10, 8 except (it=lb) 9=108.
- "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job 300029	Truss V21	Truss Type VALLEY	Qty 1	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029042 Job Reference (optional)
Builders FrstSource, Lake City, FL 32055			7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:56 2009 Page 1		

3-10-8
3-10-8
17-6-8
13-8-0
21-5-0
3-10-8

Scale = 1:35.2

21-5-0
21-5-0

Plate Offsets (X,Y): [3:0-2-0,0-2-8], [7:0-2-0,0-2-8]						
LOADING (psf)	SPACING 2-0-0	CSI	DEFL	in (loc)	L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.13	Vert(LL)	n/a	n/a	MT20 244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.06	Vert(TL)	n/a	n/a	
BCLL 0.0	Rep Stress Incr YES	WB 0.05	Horz(TL)	0.00	9	
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)				Weight: 72 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2 X 4 SYP No.3	

REACTIONS All bearings 21-5-0.
 (lb) - Max Horz 1=26(LC 5)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 15, 11, 10 except 13=103(LC 5)
 Max Grav All reactions 250 lb or less at joint(s) 1, 9, 15, 10 except 13=258(LC 11), 14=251(LC 11), 11=251(LC 12)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

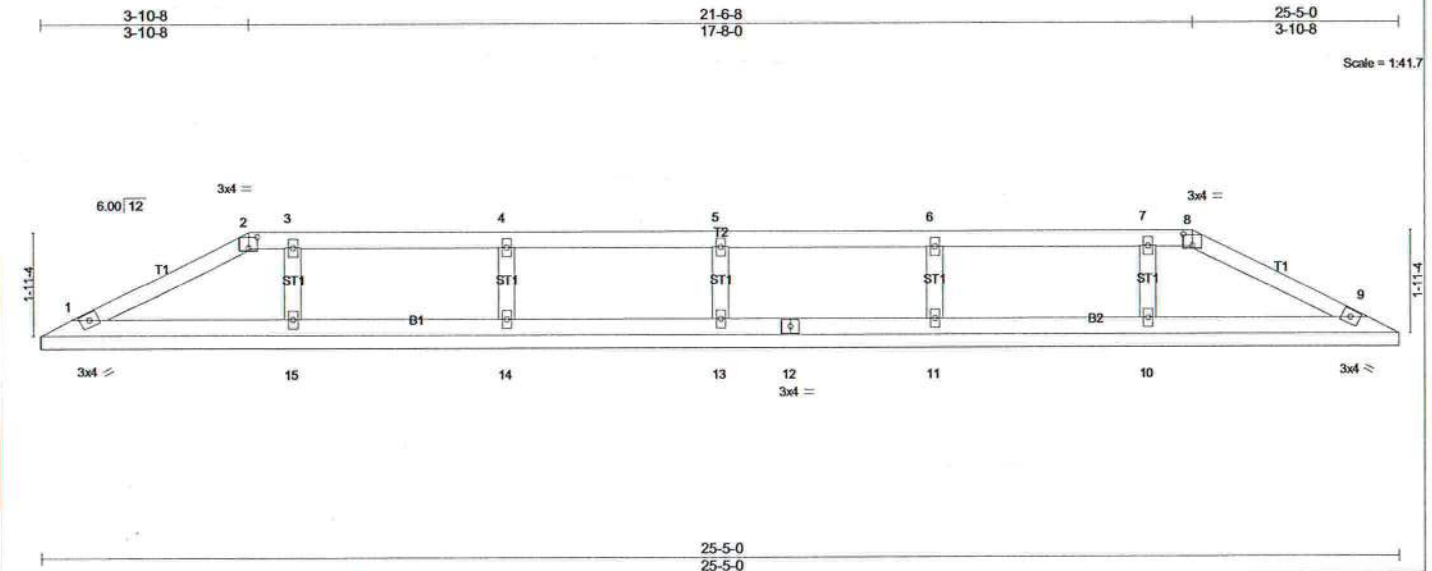
NOTES (10-11)
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Provide adequate drainage to prevent water ponding.
 4) All plates are 2x4 MT20 unless otherwise indicated.
 5) Gable requires continuous bottom chord bearing.
 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 15, 11, 10 except (it=lb) 13=103.
 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869; Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Job 300029	Truss V25	Truss Type VALLEY	Qty 1	Ply 1	HOUSECRAFT - FARRIMOND RES. 300029043 Job Reference (optional)
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Builders FrstSource, Lake City, FL 32055

7.110 s Dec 8 2008 MiTek Industries, Inc. Wed Mar 11 14:54:58 2009 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.15	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.07	Vert(TL)	n/a	-	n/a	999		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.05	Horz(TL)	0.00	9	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2007/TPI2002								
								Weight: 85 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2 X 4 SYP No.3	

REACTIONS All bearings 25-5-0.
 (lb) - Max Horz 1=26(LC 6)
 Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 13, 15, 10 except 14=108(LC 5), 11=108(LC 6)
 Max Grav All reactions 250 lb or less at joint(s) 1, 9 except 13=259(LC 1), 14=265(LC 12), 15=279(LC 11), 11=265(LC 11), 10=279(LC 12)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (10-11)
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 110mph (3-second gust); TCDF=4.2psf; BCDL=3.0psf; h=16ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) All plates are 2x4 MT20 unless otherwise indicated.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 13, 15, 10 except (it=lb) 14=108, 11=108.
 - 9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

#2 HIP OR COMMON TRUSS

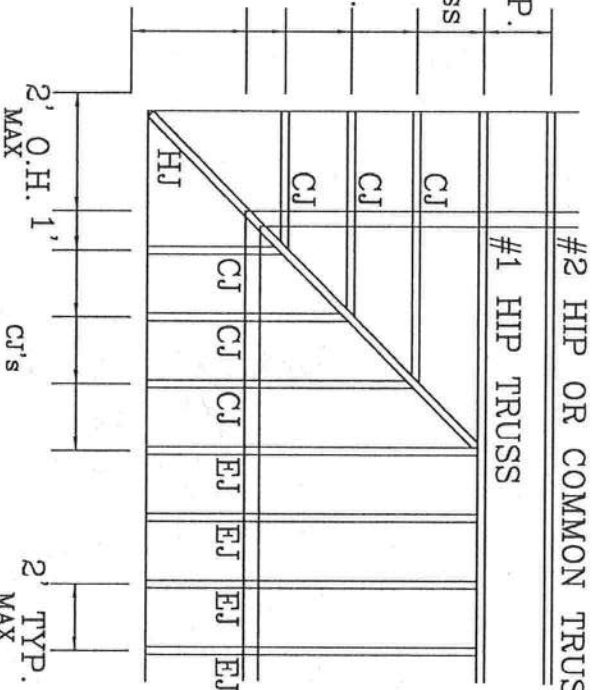
MAX 2' TYP. or Less
Setback 7' or Less

2' TYP.
MAX
or Less
CJ's

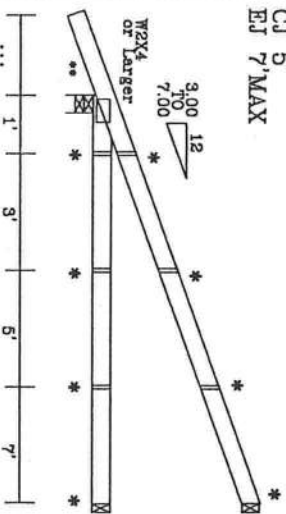
2' CJ's
TYP.
MAX

1.

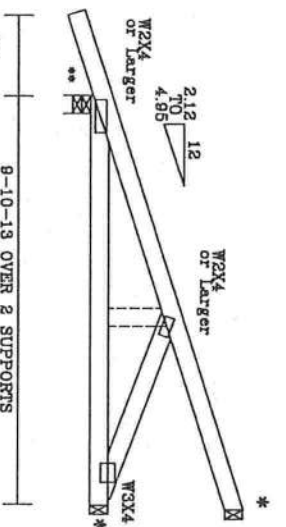
1



ALL HEELS TO BE STANDEAR WITH NO CANTILEVER



END AND CORNER JACKS



HIP JACK

UPLIFT VALUES DO TAKE INTO ACCOUNT PORCHES EXPOSED
BC LIVE LOAD IS NON CONCURRENT 10*

CORNER SET
SETBACK

7'0" MAX

BRACING: ALL TRUSSES REQUIRING EXTREME CARE FABRICATING, HANDLING, SHIPPING, INSTALLING AND UNBRACING. REFER TO BCST 1-04E BUILDING COMPONENT SAFETY INFORMATION, PUBLISHED BY TPI TRUSS PLATE INSTITUTE, 588 DUNDORF DR., SUITE 200, MADISON, WI 53719, AND VITA CYCLOD TRUSS CONSULT OF AMERICA, 6500 ENTERPRISE LVL, MADISON, WI 53719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

PRODUCTS: A FLUSH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ALPINE ENGINEERED PRODUCTS, INC., SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR APPLICABLE HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMANCE WITH APPLICABLE PROVISIONS OF NDS NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE USER.

PLATES: ALL PLATE ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA. V.H.S/AS A578 A653 GRADE 40/60 (V.H.S/AS) GALV. STEEL. PLATES TO EACH FACE OF TRUSS AND UNLESS OTHERWISE LOCALIZED LOCAL BRACING POSITION PER DRAWINGS 1604-2. ANY INSPECTION OF PLATES FOLLOWED BY A CD SHALL BE THE DESIGNER'S RESPONSIBILITY. ANY INSPECTION OF PLATES FOLLOWED BY A CD SHALL BE THE DESIGNER'S RESPONSIBILITY.

PROFESSIONAL ENGINEERING RESPONSIBILITY: THE DESIGNING ENGINEER ASSUMES DESIGN AND PROFESSIONAL ENGINEERING RESPONSIBILITY FOR THE TRUSS. THE USER ASSUMES THE TRUSS'S SUITABILITY AND FOR THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1, SEC. 2.

SEE FOR THE DOWN

SEE FOR THE DOWN
 看看

CONC. ENGINEERS, P.A.
1065 ST. ALB. AVENUE
DETROIT, MICH. 48211-2703
NO. 47869

[illegible]

REVIEWED
By Julius Iee at 10:52 am, Jun 27, 2008

MAX GABLE VERTICAL LENGTH																			
CABLE VERTICAL SPACING	2X4 SPECIES	BRACE GRADE	NO. BRACES	(1) 2X4 "L" BRACE *						(2) 2X4 "L" BRACE **			(1) 2X6 "L" BRACE *		(2) 2X6 "L" BRACE *				
				GROUP A		GROUP B		GROUP A		GROUP B		GROUP A		GROUP B		GROUP A		GROUP B	
				(1) 1X4 "L" BRACE *															
24" O.C.	SPF	#1 / #2	3' 4"	6' 10"	6' 0"	6' 11"	7' 1"	6' 3"	6' 6"	10' 10"	11' 2"	12' 11"	13' 3"						
			3' 3"	4' 11"	4' 11"	6' 6"	6' 6"	6' 3"	6' 3"	10' 1"	12' 11"	12' 11"	12' 11"						
		STUD	3' 3"	4' 11"	4' 11"	6' 5"	6' 6"	6' 3"	6' 3"	10' 0"	10' 0"	12' 11"	12' 11"						
			STANDARD	3' 3"	4' 2"	4' 2"	5' 6"	5' 6"	7' 5"	7' 5"	8' 8"	8' 8"	11' 8"	11' 8"					
		HF	4' 1	3' 8"	5' 10"	6' 3"	6' 11"	7' 5"	8' 3"	6' 11"	10' 10"	11' 8"	12' 11"	13' 11"					
			STANDARD	3' 7"	6' 10"	6' 3"	6' 11"	7' 6"	8' 3"	8' 11"	10' 10"	11' 6"	12' 11"	13' 11"					
	SP DFL	#3	3' 6"	5' 0"	6' 0"	6' 8"	6' 8"	8' 3"	8' 6"	10' 4"	10' 4"	12' 11"	13' 7"						
			STUD	3' 6"	5' 0"	5' 0"	6' 7"	6' 7"	8' 3"	8' 6"	10' 3"	10' 3"	12' 11"	13' 7"					
		STANDARD	3' 4"	4' 3"	4' 3"	5' 8"	5' 8"	7' 8"	7' 8"	8' 10"	8' 10"	12' 0"	12' 0"						
			#1 / #2	3' 10"	6' 8"	6' 10"	7' 11"	8' 1"	9' 6"	9' 6"	12' 6"	12' 9"	14' 0"	14' 0"					
		SPF	#3	3' 8"	6' 0"	6' 0"	6' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"					
				STUD	3' 6"	6' 0"	6' 0"	7' 11"	7' 11"	9' 5"	9' 5"	12' 4"	12' 4"	14' 0"	14' 0"				
HF	STANDARD	3' 6"	5' 8"	6' 2"	6' 10"	6' 10"	8' 2"	8' 2"	10' 7"	10' 7"	14' 0"	14' 0"							
		#1	4' 3"	6' 8"	7' 2"	7' 11"	8' 6"	9' 5"	10' 2"	12' 5"	13' 5"	14' 0"							
	SP	#2	4' 0"	6' 8"	7' 2"	7' 11"	8' 8"	9' 6"	10' 2"	12' 6"	13' 5"	14' 0"	14' 0"						
			#3	4' 0"	6' 8"	6' 2"	7' 11"	8' 2"	9' 11"	9' 11"	12' 6"	13' 8"	14' 0"	14' 0"					
	DFL	STUD	4' 0"	6' 1"	6' 1"	7' 11"	8' 1"	9' 5"	9' 11"	12' 5"	12' 5"	14' 0"	14' 0"						
			STANDARD	3' 10"	5' 3"	6' 11"	6' 11"	8' 4"	9' 4"	10' 10"	10' 10"	14' 0"	14' 0"						
16" O.C.	SPF	#1 / #2	4' 8"	7' 4"	7' 7"	8' 9"	8' 11"	10' 6"	10' 6"	13' 8"	14' 0"	14' 0"							
			4' 2"	6' 11"	6' 11"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"							
		STUD	4' 2"	6' 11"	6' 11"	7' 10"	7' 10"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"							
			STANDARD	4' 8"	7' 4"	7' 11"	8' 9"	8' 9"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"						
		HF	#1	4' 8"	7' 4"	7' 11"	8' 9"	8' 9"	10' 6"	11' 2"	13' 8"	14' 0"	14' 0"						
				#2	4' 4"	7' 4"	7' 11"	8' 9"	8' 9"	10' 5"	11' 2"	13' 8"	14' 0"	14' 0"					
	SP DFL	#3	4' 4"	7' 1"	7' 1"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"							
			STUD	4' 4"	7' 1"	7' 1"	8' 9"	8' 9"	10' 5"	10' 11"	13' 8"	14' 0"	14' 0"						
		STANDARD	4' 4"	7' 1"	7' 1"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"							
			#1	4' 4"	7' 1"	7' 1"	8' 9"	8' 9"	10' 5"	10' 5"	13' 8"	14' 0"	14' 0"						

GROUP A:		HDL - FTR	
SPRICE - PINE - YR		#1	#2
STANDARD		STUD	
#3		STUD	
DOUGLAS FIR - LARCH			
#1		#3	
STUD		STUD	
STANDARD		STANDARD	
SOUTHERN PINE			
#1		#3	
STUD		STUD	
STANDARD		STANDARD	

GROUP B:

RED-FIR

#1 & BTR

#1

SOUTHERN PINE

#1

#2

DOUGLAS FIR-LARGE

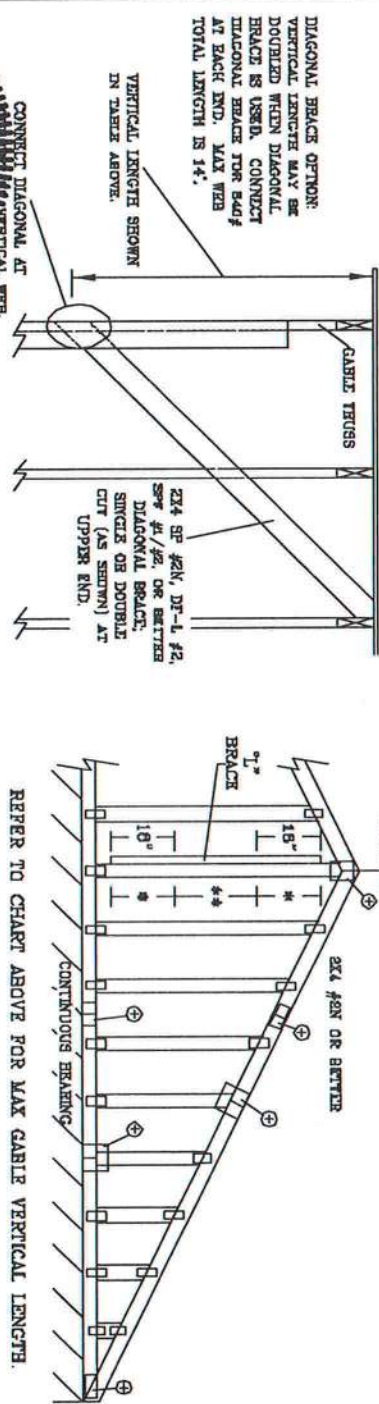
#1

#2

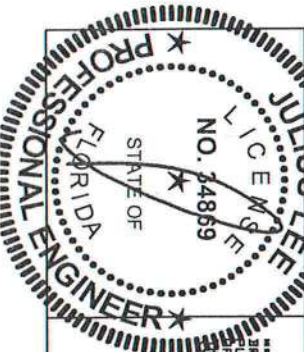
LIVE LOAD DEFLECTION CRITERIA IS $L/240$.
 PROVIDE UPLIFT CONNECTIONS FOR 136 PLF OVER
 CONTINUOUS BEARING (6 PSF TC DEAD LOAD).
 CABLE END SUPPORTS LOAD FROM 4" O"
 OUTDOORS WITH 2" O" OVERHANG, OR 12"
 PLYWOOD OVERHANG.

CABLE VERTICAL PLATE SIZES	
VERTICAL LENGTH	NO SPLICE
LESS THAN 4' 0"	1X4 OR 2X3
GREATER THAN 4' 0", BUT LESS THAN 11' 8"	2X4
GREATER THAN 11' 8"	2, 3X4

+ REFER TO COLUMN THROUS DESIGN FOR
PEAK, SPLICE, AND BEEL PLATES.



WARNING THESE REQUIRE EXTENSIVE CARE FABRICATING, HANDING, SHIPPING, INSTALLING AND MAINTAINING. REFER TO B251-142 (BUILDING COMPONENT SAFETY AND PROTECTION) BY IP (THIS IS A REPEATED REFERENCE TO THE SAME DOCUMENT). SEE ALSO B251-142, STATE 20, 53715 AND APCA (AFTER TRUCKS COLLAPSED IN AMERICA, 6500 ENTERPRISE DR., WASHINGTON, VI 53715) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP COATED SHALL HAVE PROTECTIVE ATTACHED STRUCTURAL PATENTS AND DOTTEN CHECK SHALL HAVE A PROTECTIVE ATTACHED PROTECTIVE CEILING.



JULIUS LEE'S
CONS. ENGINEERS P.A.

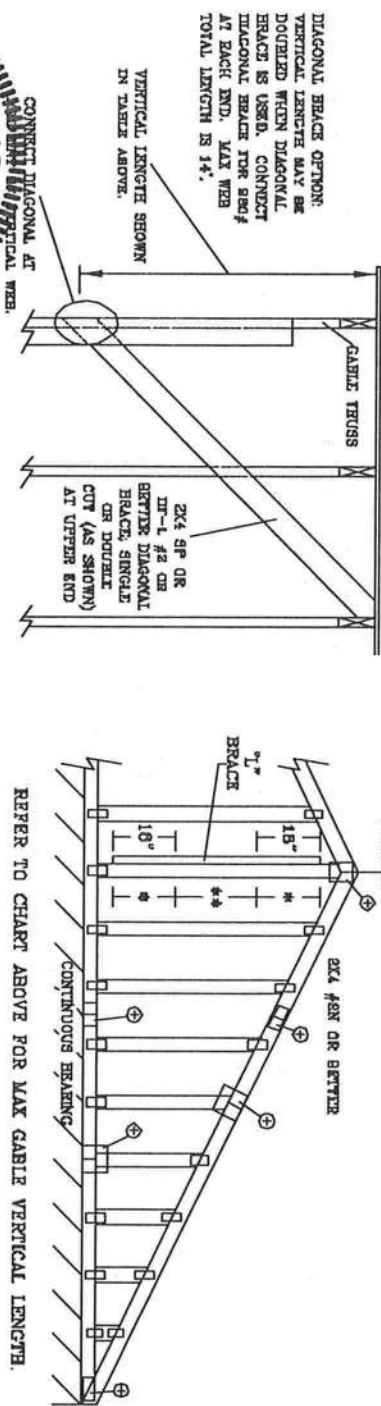
No: 34869
STATE OF FLORIDA

REVIEWED
By Julius lee at 12:00 pm, Jun 11, 2008

REF	ASCET-02-CAB13015
DATE	11/26/03
DRWG	MTRX STD CASE 15 E HD
-ENG	

ASCE 7-02: 130 MPH WIND SPEED, 30' MEAN HEIGHT, ENCLOSED, I = 1.00, EXPOSURE C

MAX GABLE VERTICAL LENGTH																		
CABLE VERTICAL SPACING	2x4 SPECIES	BRACE GRADE	NO BRACES	(1) 1x4 "L" BRACE * (1) 2x4 "L" BRACE * (2) 2x4 "L" BRACE ** (1) 2x6 "L" BRACE * (2) 2x8 "L" BRACE **														
				GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B	GROUP A	GROUP B			
24" O.C.	SPF	#1 / #2	3' 2"	5' 6"	6' 8"	6' 8"	6' 9"	7' 10"	8' 0"	10' 3"	10' 7"	12' 3"	12' 7"					
		#3	3' 1"	4' 5"	4' 5"	5' 10"	5' 10"	7' 10"	7' 10"	9' 1"	9' 1"	12' 3"	12' 3"					
	HF	STANDARD	2' 11"	3' 6"	3' 9"	6' 0"	5' 0"	6' 9"	6' 9"	7' 10"	7' 10"	10' 7"	10' 7"					
		#1	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"					
	SP	#2	3' 6"	5' 6"	5' 11"	6' 8"	7' 0"	7' 10"	8' 5"	10' 3"	11' 1"	12' 3"	13' 2"					
		#3	3' 3"	4' 6"	4' 6"	6' 0"	6' 0"	7' 10"	8' 1"	9' 4"	9' 4"	12' 3"	12' 8"					
	DFL	STANDARD	3' 3"	4' 8"	4' 8"	5' 11"	5' 11"	7' 10"	8' 0"	9' 3"	9' 3"	12' 3"	12' 8"					
		STUD	3' 0"	3' 10"	3' 10"	6' 11"	5' 11"	8' 11"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"					
	16" O.C.	SPF	#1 / #2	3' 8"	5' 4"	6' 8"	7' 6"	7' 6"	8' 11"	9' 2"	11' 9"	12' 1"	14' 0"					
			#3	3' 7"	5' 5"	5' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 2"	11' 2"	14' 0"					
HF		STANDARD	3' 7"	5' 6"	6' 5"	7' 2"	7' 2"	8' 11"	8' 11"	11' 1"	11' 1"	14' 0"						
		STUD	3' 4"	4' 6"	4' 6"	6' 2"	6' 2"	8' 3"	8' 3"	9' 7"	9' 7"	12' 11"						
SP		#1	4' 0"	6' 4"	6' 10"	7' 8"	8' 11"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"						
		#2	3' 11"	5' 4"	6' 10"	7' 8"	8' 11"	8' 11"	9' 7"	11' 9"	12' 8"	14' 0"						
DFL		#3	3' 8"	5' 7"	6' 7"	7' 4"	7' 4"	8' 11"	8' 6"	11' 5"	11' 5"	14' 0"						
		STUD	3' 8"	5' 6"	5' 6"	7' 3"	7' 3"	8' 11"	8' 5"	11' 4"	11' 4"	14' 0"						
12" O.C.		SPF	#1 / #2	3' 8"	4' 9"	4' 9"	6' 3"	6' 3"	8' 5"	8' 5"	9' 9"	9' 9"	13' 3"					
			#3	4' 0"	6' 11"	7' 2"	6' 3"	8' 6"	9' 10"	10' 1"	12' 11"	13' 4"						
	HF	STANDARD	3' 11"	5' 4"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"						
		STUD	3' 11"	5' 4"	6' 3"	8' 3"	8' 3"	9' 10"	9' 10"	12' 11"	12' 11"	14' 0"						
	SP	#1	4' 5"	6' 11"	7' 8"	7' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"						
		#2	4' 4"	6' 11"	7' 8"	8' 3"	8' 11"	9' 10"	10' 7"	12' 11"	13' 11"	14' 0"						
	DFL	#3	4' 2"	6' 6"	6' 5"	8' 3"	8' 3"	9' 10"	10' 4"	12' 11"	13' 3"	14' 0"						
		STUD	4' 2"	6' 4"	6' 4"	8' 3"	8' 6"	9' 10"	10' 4"	12' 11"	13' 1"	14' 0"						
	STANDARD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	8' 9"	8' 9"	11' 4"	11' 4"	14' 0"							
		STUD	4' 0"	5' 6"	5' 6"	7' 3"	7' 3"	8' 9"	8' 9"	11' 4"	11' 4"	14' 0"						



CABLE TRUSS DETAIL NOTES:	
LIVE LOAD DEFLECTION CRITERIA IS L/240.	
PROVIDE UPLIFT CONNECTIONS PER 150 PSF OVER CONTINUOUS BEARING (6 PSF TC DEAD LOAD).	
CABLE END SUPPORTS LOAD FROM 4' 0" OUTLINE WITH 8' 0" OVERHANG, OR 12" PLYWOOD OVERHANG.	
ATTACH EACH "L" BRACE WITH 10d NAILS.	
* FOR (1) "L" BRACE, SPACE NAILS AT 8" O.C. IN 18" END ZONES AND 4" O.C. BETWEEN ZONES.	
** FOR (2) "L" BRACES, SPACE NAILS AT 3" O.C. IN 18" END ZONES AND 6" O.C. BETWEEN ZONES.	
"L" BRACING MUST BE A MINIMUM OF 80% OF WEB MEMBER LENGTH.	



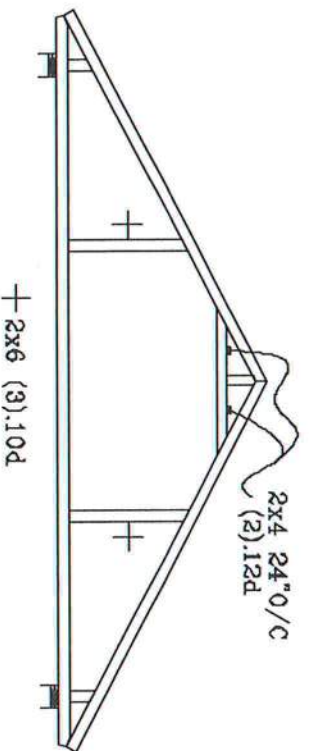
REVIEWED
By Julius Lee at 12:00 pm, Jun 11, 2008

ADVANCING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO SECS 1-03 CALLING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURING INSTITUTE, 588 DUNSTON RD., SUITE 200, HANOVER, VA 22919 AND VITA (404) TRUSS CONSULTING, 15000 BUCKINGHAM DR., SUITE 100, ATLANTA, GA 30341. ALL TRUSSES SHALL HAVE PROPERLY ATTACHED BRACING, PLYWOOD AND BRITISH CROWN SHALL HAVE A PROPERLY ATTACHED BRIDGING.

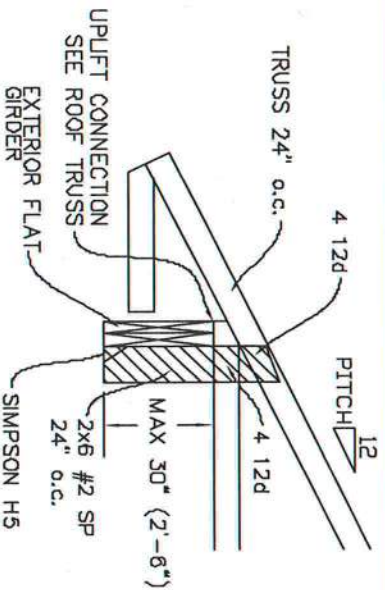
JULIUS LEE'S
CONS. ENGINEERS P.A.
1456 SW 4th AVENUE
DELMAR BEACH, FL 33444-2161
No. 34869
STATE OF FLORIDA

MAX. TOT. LD. 60 PSF	REF ASCE 7-02-CAB13030
MAX. SPACING 24.0"	DATE 11/26/03
	DWG. BY: DWG. DATE: 5/6/03
	ENG

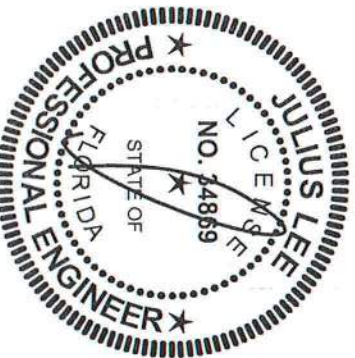
TYPICAL ATTIC TRUSS BRACING



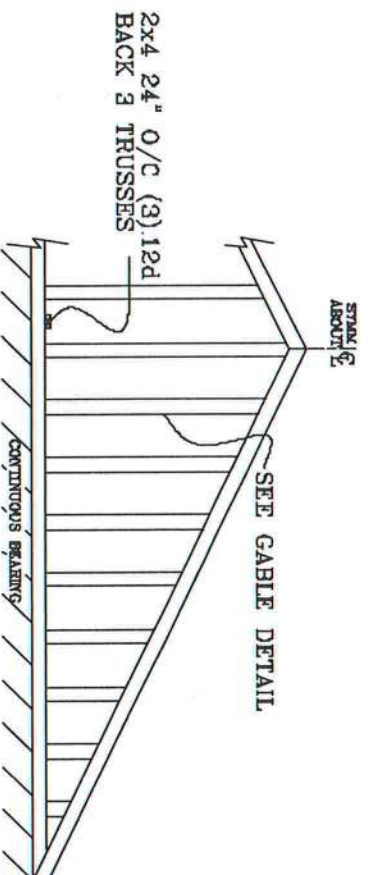
TYPICAL ALTERNATE BRACING DETAIL FOR EXTERIOR FLAT GIRDER TRUSS



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

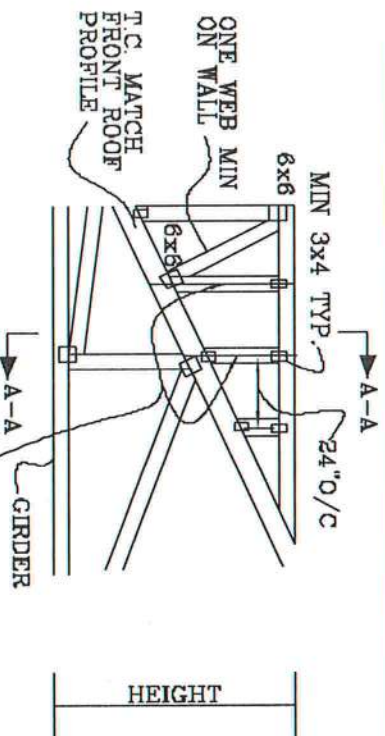


GABLE END TRUSS DETAIL



MINIMUM BC BRACING ON GABLE TRUSS. OTHER PERMANENT BRACING DESIGNS BY ARCHITECT OR EOR

TYPICAL WALL GIRDER VERTICAL WEB BRACING DETAIL



SEE ROOF TRUSSES FOR UPLIFT

SEE GABLE END DETAIL FOR T-BRACE BEHIND EACH VERTICAL

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No. 34869
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TOP CHORD 2X4 #2 OR BETTER
BOT CHORD 2X4 #2 OR BETTER
WEBS 2X4 #3 OR BETTER

PICGYBACK DETAIL

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PICGYBACK VERTICALS AT 4' OC MAX.

TOP AND BOTTOM CHORD SPICES MUST BE STAGGERED SO THAT ONE SPICE IS NOT DIRECTLY OVER ANOTHER.

PICGYBACK BOTTOM CHORD MAY BE OMITTED. ATTACH VERTICAL WEBS TO TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FLAT TOP CHORD. IF PICGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

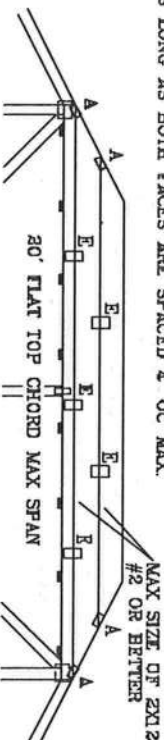
REFER TO ENGINEER'S SEALED DESIGN FOR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE FOLLOWING WIND CONDITIONS:

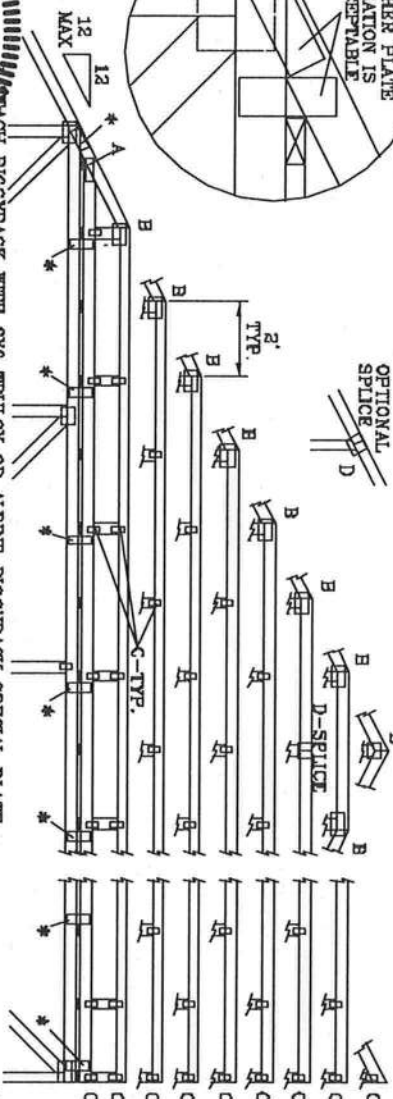
110 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST, CAT I, EXP C, WIND TC DL=5 PSF, WIND BC DL=5 PSF
110 MPH WIND, 30' MEAN HGT, EBC ENCLOSED BLDG, LOCATED ANYWHERE IN ROOF, WIND TC DL=5 PSF, WIND BC DL=5 PSF

130 MPH WIND, 30' MEAN HGT, ASCE 7-02, CLOSED BLDG, LOCATED ANYWHERE IN ROOF, CAT II, EXP C, WIND TC DL=6 PSF, WIND BC DL=6 PSF

FRONT FACE (E*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.



OPTIONAL SPICE



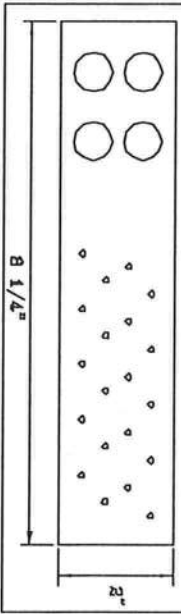
THIS DRAWING REPLACES DRAWINGS 634.016 634.017 & 647.045

JOINT TYPE	SPANS UP TO		
	30'	36'	62'
A	2X4	2.5X4	2.5X4
B	4X6	6X6	6X6
C	1.5X3	1.5X4	1.5X4
D	5X4	6X6	6X6
E	4X6 OR 3X6 TRUSS AT 4' OC, ROTATED VERTICALLY		

ATTACH TRUSS PLATES WITH (E) 0.120" X 1.375" NAILS OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRUSS INFORMATION.

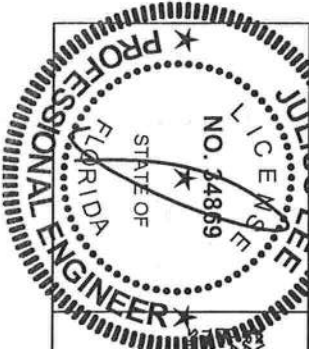
WEB LENGTH	WEB BRACING CHART
0' TO 7'9"	NO BRACING
7'9" TO 10'	1X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 6d NAILS AT 4' OC.
10' TO 14'	2X4 "T" BRACE, SAME GRADE, SPECIES AS WEB MEMBER OR BETTER, AND 80% LENGTH OF WEB MEMBER. ATTACH WITH 16d NAILS AT 4' OC.

* PICGYBACK SPECIAL PLATE
ATTACH TEETH TO THE PICGYBACK AT THE TRUE OR FABRICATION ATTACH TO SUPPORTING TRUSS WITH (4) 0.120" X 1.375" NAILS PER FACE PER PLY. APPLY PICGYBACK SPECIAL PLATE TO EACH TRUSS FACE AND SPACE 4' OC OR LESS.



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MAX LOADING	REF
55 PSF AT	DATE 09/12/07
1.33 DUR. FAC.	DRWG/ITEK STD PICGY
50 PSF AT	-ENG JL
1.25 DUR. FAC.	
47 PSF AT	
1.15 DUR. FAC.	
SPACING 24.0"	



REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

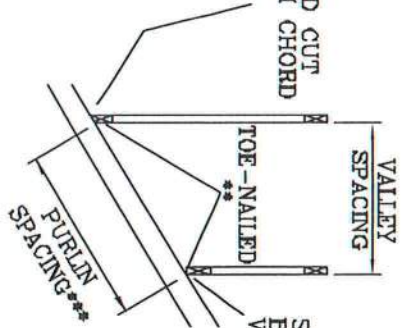
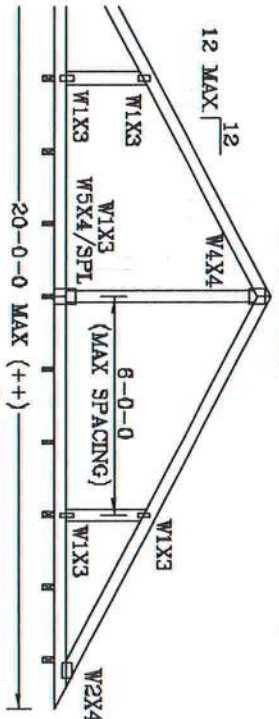
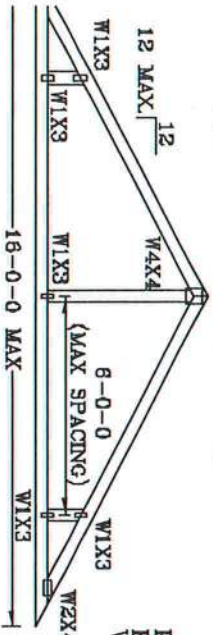
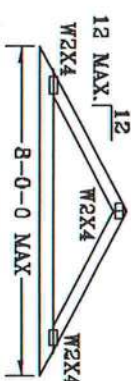
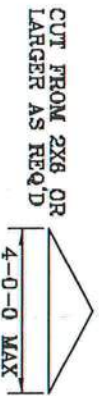
No. 34869
STATE OF FLORIDA

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

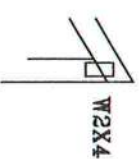
** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH:
(2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR
FBC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR
ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED
BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=6 PSF.



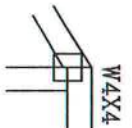
SQUARE CUT BOTTOM CHORD VALLEY

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.

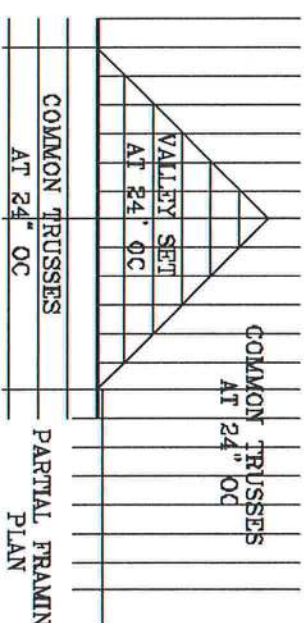
UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 90% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.6") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".
TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN OR BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.



OPTIONAL STUB END DETAIL



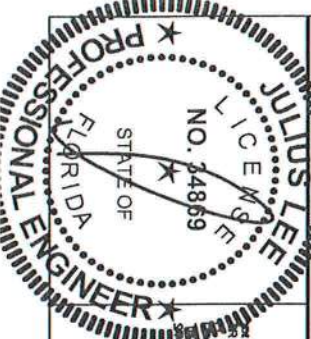
OPTIONAL HIP JOINT DETAIL



COMMON TRUSSES AT 24" OC

PARTIAL FRAMING PLAN

WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND ERECTION. REFER TO DESIGNER'S BUILDING DEPARTMENT SPECIFICATIONS AND LOCAL BUILDING DEPARTMENT REGULATIONS. IN AMERICA, 6000 DIFFERENCE IN WINDSPEED, 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=6 PSF. STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.



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REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

No. 34869
STATE OF FLORIDA

TC LL	20	20	PSF	REF	VALLEY DETAIL
TC DL	7	15	PSF	DATE	11/26/03
BC DL	5	5	PSF	DRWG	VALTRUSS1103
BC LL	0	0	PSF	-ENG	JL
TOT. LD.	32	40	PSF		
DUR.FAC.	1.25	1.25			
SPACING	24"				

THIS DRAWING REPLACES DRAWING A105

TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AP&PA NDS-2001 SECTION 12.4.1 - EDGE DISTANCE, END DISTANCE, SPACING, EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

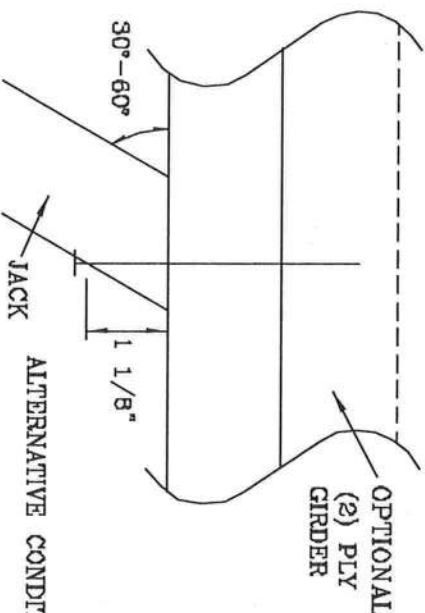
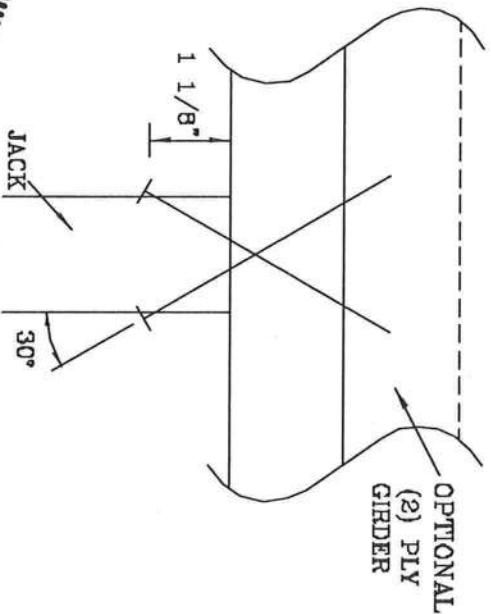
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES AND NAIL TYPE. PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

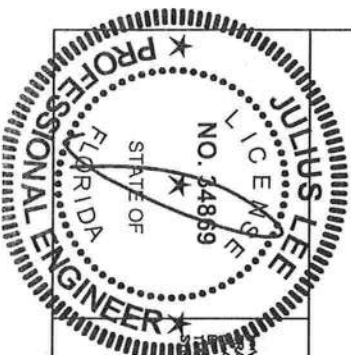
MAXIMUM VERTICAL RESISTANCE OF 16d (0.162"x3.5") COMMON TOE-NAILS

NUMBER OF TOE-NAILS	SOUTHERN PINE		DOUGLAS FIR-LARCH		HEM-FIR		SPRUCE PINE FIR	
	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES	1 PLY	2 PILES
2	197#	256#	181#	234#	156#	203#	154#	199#
3	296#	383#	271#	351#	234#	304#	230#	298#
4	394#	511#	361#	468#	312#	406#	307#	397#
5	493#	638#	452#	585#	390#	507#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



THIS DRAWING REPLACES DRAWING 784040



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SAPPING, INSTALLING AND SPACING. REFER TO BCS 1-03 CHAIRING COMPONENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURERS ASSOCIATION, 388 JONES RD., SUITE 200, NATION, VA 22079 AND VICA (AIA) TRUSS COUNCIL, 6800 ENTERPRISE LN, NATION, VA 22079 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, THE OWNER SHALL HAVE PREPARED AND ATTACHED TO THE DRAWING PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED ROOF CEILING.

REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

JULIUS LEE'S
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1450 ST 4TH AVENUE
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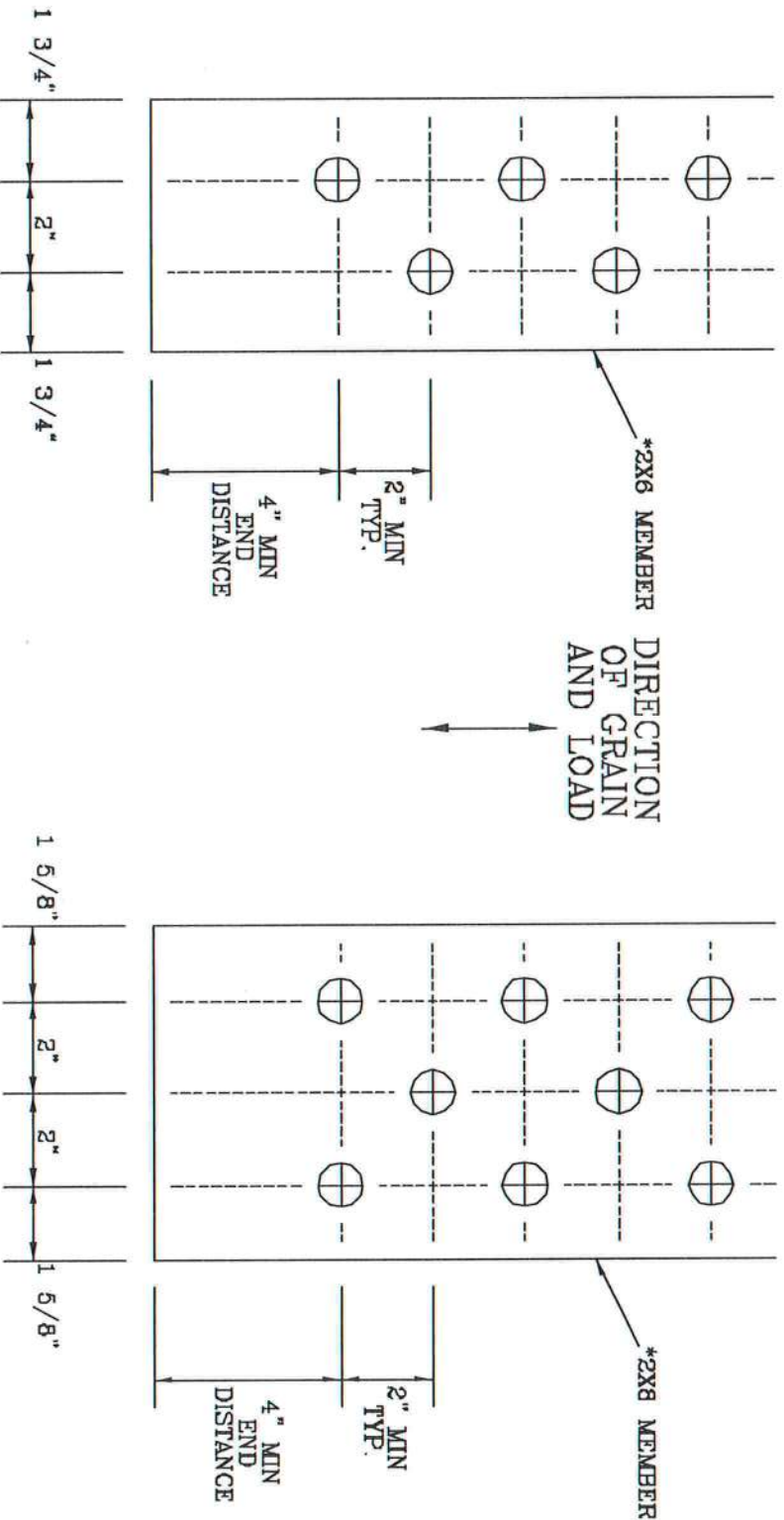
No. 34869
STATE OF FLORIDA

TC LL	PSF	REF	TOE-NAIL
TC DL	PSF	DATE	09/12/07
BC DL	PSF	DRWG	CNTONAIL103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.	1.00		
SPACING			

1/2" DIAMETER BOLT SPACING FOR LOAD APPLIED PARALLEL TO GRAIN.

* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.
BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

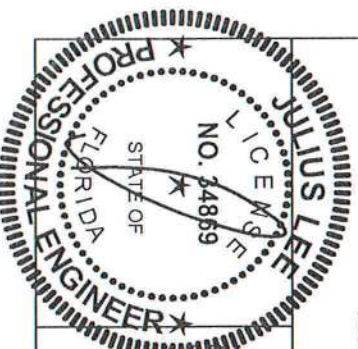
TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. BOLT QUANTITIES AS NOTED ON SEALED DESIGN MUST BE APPLIED IN ONE OF THE PATTERNS SHOWN BELOW.
WASHERS REQUIRED UNDER BOLT HEAD AND NUT



2X6 DETAIL

2X8 DETAIL

THIS DRAWING REPLACES DRAWING A828.016



VARIOUS TRUSSES REQUIRE EXTENSIVE CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND MAINTAINING. TRUSSES SHOULD BE CONSIDERED SAFETY INFORMATION, PROVIDED BY THE TRUSS MANUFACTURER. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS DESIGN AND CONSTRUCTION. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS DESIGN AND CONSTRUCTION. THE TRUSS MANUFACTURER SHALL BE RESPONSIBLE FOR THE TRUSS DESIGN AND CONSTRUCTION.

REVIEWED
By Julius Lee at 11:59 am, Jun 11, 2008

JULIUS LEE'S
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No: 34869
STATE OF FLORIDA

TC LL	PSF	REF	BOLT SPACING
TC DL	PSF	DATE	11/26/03
BC DL	PSF	DRWG	CNBOLTP1103
BC LL	PSF	-ENG	JL
TOT. LD.	PSF		
DUR. FAC.			
SPACING			

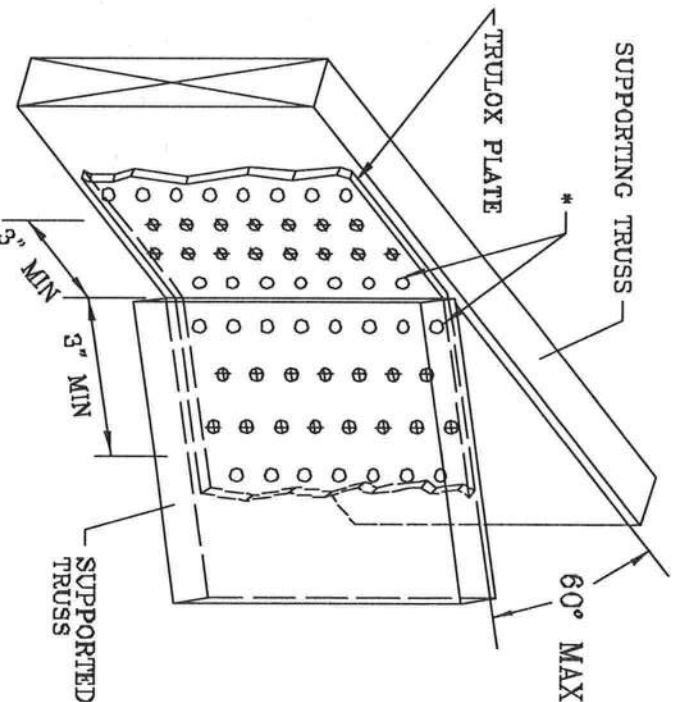
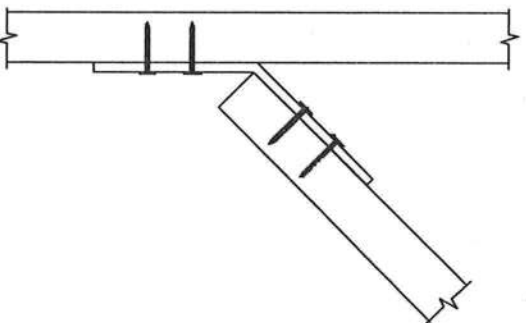
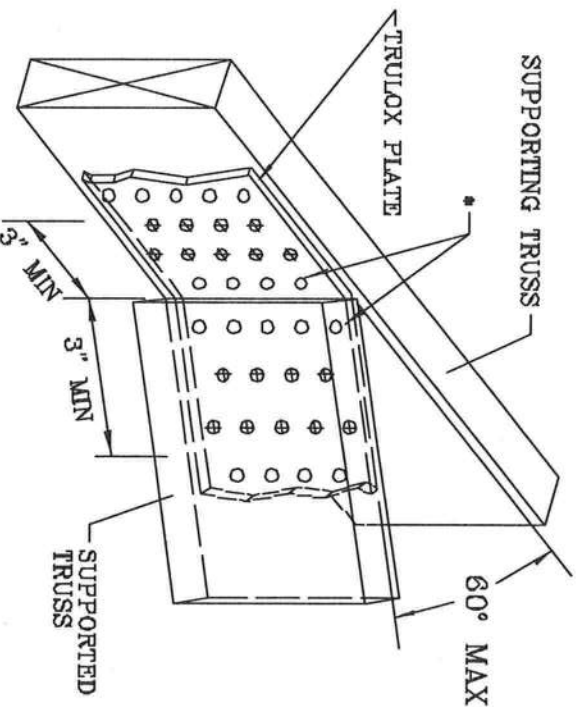
TRULOX CONNECTION DETAIL

11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (Φ).

* NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.
REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.

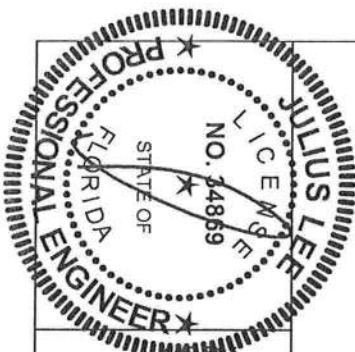


MINIMUM 3X6 TRULOX PLATE

MINIMUM 5X6 TRULOX PLATE

TRULOX PLATE SIZE	REQUIRED NAILS PER TRUSS	MAXIMUM LOAD UP OR DOWN
3X6	9	350#
6X6	15	930#

THIS DRAWING REPLACES DRAWINGS 1,168,985 1,158,989/R
1,154,844 1,152,217 1,152,017 1,159,154 & 1,151,524



WARNING: TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO 2031-1-03 (BUILDING DEPARTMENT SAFETY INFORMATION, PUBLISHED BY THE TRUSS MANUFACTURING INSTITUTE, 383 DUNSTON DR., STATE RD., WATSON, VT 55719) AND VITA CYCLO TRUSS COUNCIL, 6300 ENTERPRISE LN, WATSON, VT 55719 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED ROCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

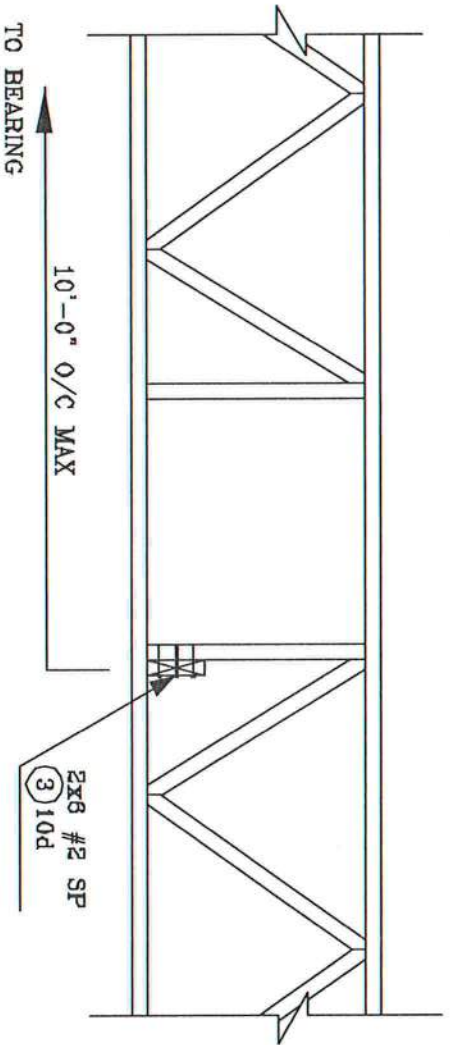
JULIUS LEE'S
CONS. ENGINEERS P.A.

1455 SW 4th AVENUE
DELRAY BEACH, FL 33444-2101

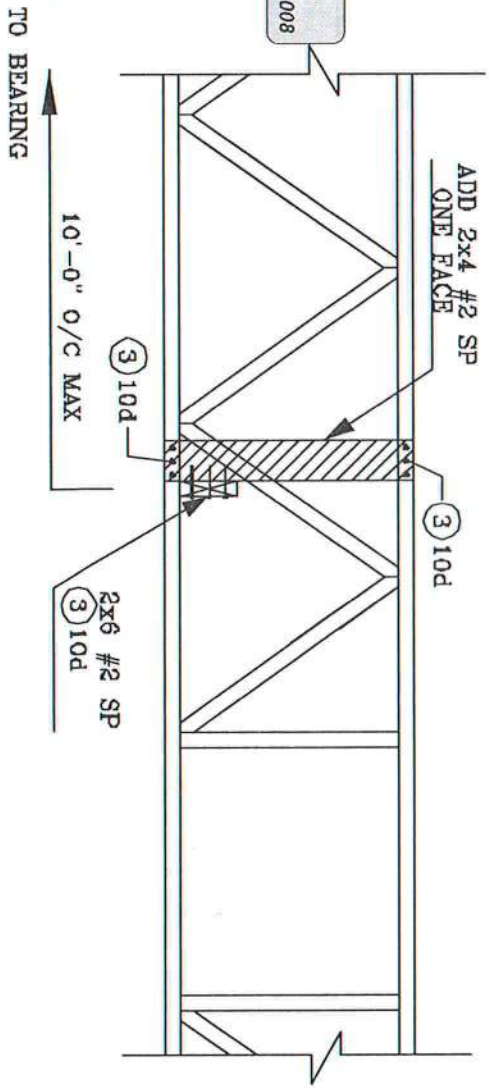
No: 34869
STATE OF FLORIDA

REF	TRULOX
DATE	11/26/03
DRWG	CNTRULOX1103
-ENG	JL

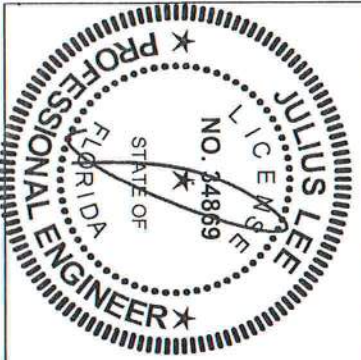
STRONG BACK DETAIL SYSTEM-42 OR FLAT TRUSS



ALTERNATE DETAIL FOR STRONG BACK WITH VERTICAL NOT LINING UP



REVIEWED
By Julius Lee at 11:58 am, Jun 11, 2008

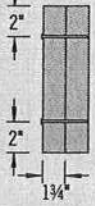
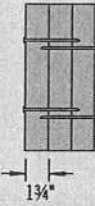
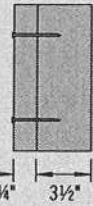


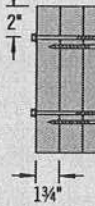


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No. 34869
STATE OF FLORIDA

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Maximum Uniform Load Applied to Either Outside Member (PLF)

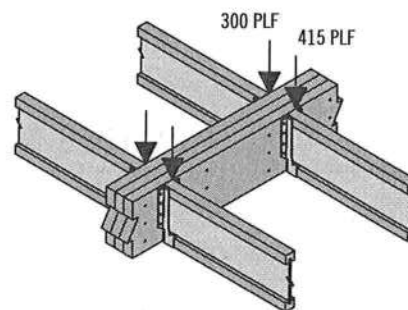
Connector Type	Number of Rows	Connector On-Center Spacing	Connector Pattern					
			Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
								
			3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail ⁽¹⁾	2	12"	370	280	280	245		
	3	12"	555	415	415	370		
1/2" A307 Through Bolts ⁽²⁾⁽⁴⁾	2	24"	505	380	520	465	860	340
		19.2"	635	475	655	580	1,075	425
		16"	760	570	785	695	1,290	505
SDS 1/4" x 3 1/2" ⁽⁴⁾	2	24"	680	510	510	455		
		19.2"	850	640	640	565		
		16"	1,020	765	765	680		
SDS 1/4" x 6" ⁽³⁾⁽⁴⁾	2	24"				455	465	455
		19.2"				565	580	565
		16"				680	695	680
USP WS35 ⁽⁴⁾	2	24"	480	360	360	320		
		19.2"	600	450	450	400		
		16"	715	540	540	480		
USP WS6 ⁽³⁾⁽⁴⁾	2	24"				350	525	350
		19.2"				440	660	440
		16"				525	790	525
3 3/4" TrussLok ⁽⁴⁾	2	24"	635	475	475	425		
		19.2"	795	595	595	530		
		16"	955	715	715	635		
5" TrussLok ⁽⁴⁾	2	24"		500	500	445	480	445
		19.2"		625	625	555	600	555
		16"		750	750	665	725	665
6 3/4" TrussLok ⁽⁴⁾	2	24"				445	620	445
		19.2"				555	770	555
		16"				665	925	665

- (1) Nailed connection values may be doubled for 6" on-center or tripled for 4" on-center nail spacing.
- (2) Washers required. Bolt holes to be 3/16" maximum.
- (3) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.
- (4) 24" on-center bolted and screwed connection values may be doubled for 12" on-center spacing.

General Notes

- Connections are based on NDS® 2005 or manufacturer's code report.
- Use specific gravity of 0.5 when designing lateral connections.
- Values listed are for 100% stress level. Increase 15% for snow-loaded roof conditions or 25% for non-snow roof conditions, where code allows.
- **Bold Italic** cells indicate **Connector Pattern** must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 the required **Connector Spacing**.
- Verify adequacy of beam in allowable load tables on pages 16–33.
- 7" wide beams should be side-loaded only when loads are applied to both sides of the members (to minimize rotation).
- Minimum end distance for bolts and screws is 6".
- Beams wider than 7" require special consideration by the design professional.

Uniform Load Design Example



First, check the allowable load tables on pages 16–33 to verify that three pieces can carry the total load of 715 plf with proper live load deflection criteria. Maximum load applied to either outside member is 415 plf. For a 3-ply 13/4" assembly, two rows of 10d (0.128" x 3") nails at 12" on-center is good for only 280 plf. Therefore, use three rows of 10d (0.128" x 3") nails at 12" on-center (good for 415 plf).

Alternates:

Two rows of 1/2" bolts or SDS 1/4" x 3 1/2" screws at 19.2" on-center.

MULTIPLE-MEMBER CONNECTIONS FOR SIDE-LOADED BEAMS

Point Load—Maximum Point Load Applied to Either Outside Member (lbs)

Connector Type	Number of Connectors	Connector Pattern					
		Assembly A	Assembly B	Assembly C	Assembly D	Assembly E	Assembly F
		3 1/2" 2-ply	5 1/4" 3-ply	5 1/4" 2-ply	7" 3-ply	7" 2-ply	7" 4-ply
10d (0.128" x 3") Nail	6	1,110	835	835	740		
	12	2,225	1,670	1,670	1,485		
	18	3,335	2,505	2,505	2,225		
	24	4,450	3,335	3,335	2,965		
SDS Screws 1/4" x 3 1/2" or WS35 1/4" x 6" or WS6 ⁽¹⁾	4	1,915	1,435 ⁽⁴⁾	1,435	1,275	1,860 ⁽²⁾	1,405 ⁽²⁾
	6	2,870	2,150 ⁽⁴⁾	2,150	1,915	2,785 ⁽²⁾	2,110 ⁽²⁾
	8	3,825	2,870 ⁽⁴⁾	2,870	2,550	3,715 ⁽²⁾	2,810 ⁽²⁾
3 3/8" or 5" TrussLok™	4	2,545	1,910 ⁽⁴⁾	1,910	1,695	1,925 ⁽³⁾	1,775 ⁽³⁾
	6	3,815	2,860 ⁽⁴⁾	2,860	2,545	2,890 ⁽³⁾	2,665 ⁽³⁾
	8	5,090	3,815 ⁽⁴⁾	3,815	3,390	3,855 ⁽³⁾	3,550 ⁽³⁾

(1) 6" SDS or WS screws can be used with Parallam® PSL and Microllam® LVL, but are not recommended for TimberStrand® LSL.

See General Notes on page 38

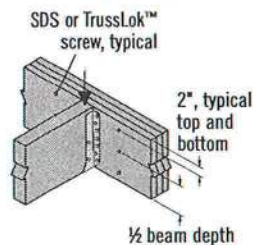
(2) 6" long screws required.

(3) 5" long screws required.

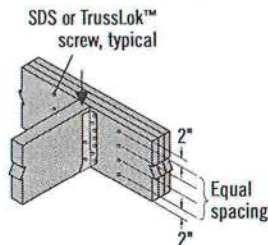
(4) 3 1/2" and 3 3/8" long screws must be installed on both sides.

Connections

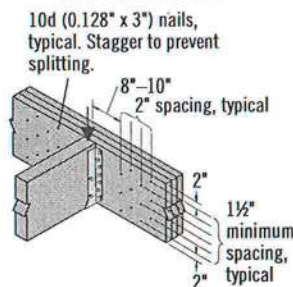
4 or 6 or Screw Connection



8 Screw Connection

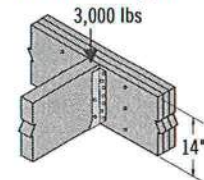


Nail Connection



There must be an equal number of nails on each side of the connection

Point Load Design Example



First, verify that a 3-ply 1 3/4" x 14" beam is capable of supporting the 3,000 lb point load as well as all other loads applied. The 3,000 lb point load is being transferred to the beam with a face mount hanger. For a 3-ply 1 3/4" assembly, eight 3 3/8" TrussLok™ screws are good for 3,815 lbs with a face mount hanger.

MULTIPLE-MEMBER CONNECTIONS FOR TOP-LOADED BEAMS

1 3/4" Wide Pieces

- Minimum of three rows of 10d (0.128" x 3") nails at 12" on-center.
- Minimum of four rows of 10d (0.128" x 3") nails at 12" on-center for 14" or deeper.
- If using 12d–16d (0.148"–0.162" diameter) nails, the number of nailing rows may be reduced by one.
- Minimum of two rows of SDS, WS, or TrussLok™ screws at 16" on-center. Use 3 3/8" minimum length with two or three plies; 5" minimum for 4-ply members. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. For 3- or 4-ply members, connectors must be installed

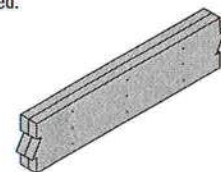
on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.

3 1/2" Wide Pieces

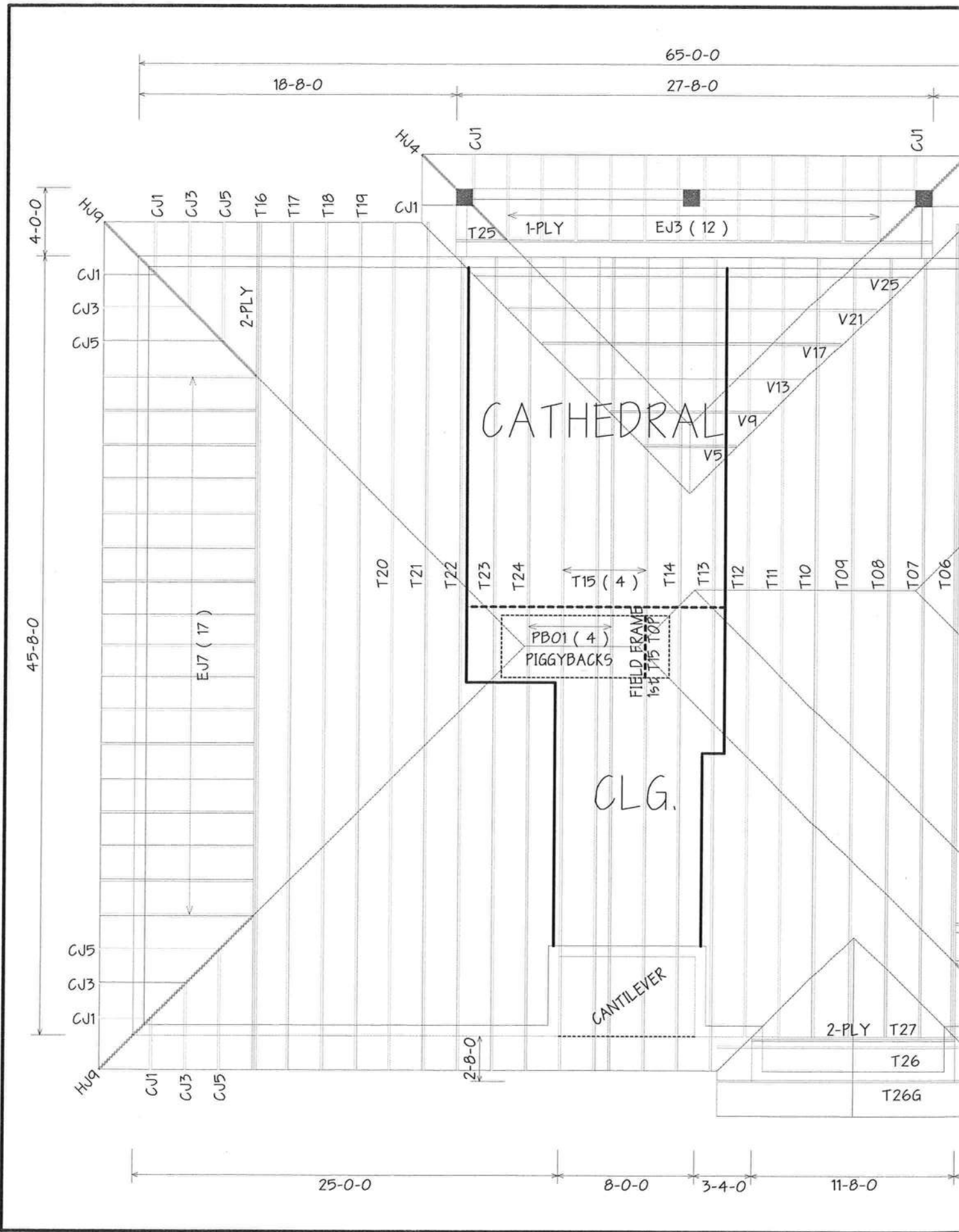
- Minimum of two rows of SDS, WS, or TrussLok™ screws, 5" minimum length, at 16" on-center. 6" SDS and WS screws are not recommended for use with TimberStrand® LSL. Connectors must be installed on both sides. Stagger fasteners on opposite side of beam by 1/2 of the required connector spacing.

- Load must be applied evenly across entire beam width. Otherwise, use connections for side-loaded beams.
- Minimum of two rows of 1/2" bolts at 24" on-center staggered.



Multiple pieces can be nailed or bolted together to form a header or beam of the required size, up to a maximum width of 7"





6/12 PITCH
2'0" O/H

BEARING HEIGHT SCHEDULE

8'-0"

HANGER SCHEDULE 7 - HTU26

NOTES:

- 1) REFER TO HIB #1 (RECOMMENDATIONS FOR HANDLING INSTALLATION AND TEMPORARY BRACING) REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V105 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 2' o.c. MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) SY42 TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL ROOF TRUSS HANGERS TO BE SIMPSON HTU26 UNLESS OTHERWISE NOTED. ALL FLOOR TRUSS HANGERS TO BE SIMPSON THA422 UNLESS OTHERWISE NOTED.
- 8) BEAM/HEADER/INTEL (HDR) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VOIDS ALL PREVIOUS ARCHITECTURAL OR OTHER TRUSS LAYOUTS. REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE AGAINST CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Requested Delivery Date: _____

Approved by: _____ Date: _____



Bunnell

PHONE: 904-437-3349 FAX: 904-437-3994

Jacksonville

PHONE: 904-772-6100 FAX: 904-772-1473

Lake City

PHONE: 386-755-6894 FAX: 386-755-7973

Sanford

PHONE: 407-322-0059 FAX: 407-322-5553

BUILDER:

HOUSECRAFT

LEGAL ADDRESS

FARRIMOND RES.

MODEL:

CUSTOM

REVISION:

SCALE: NTS

DATE:

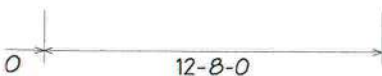
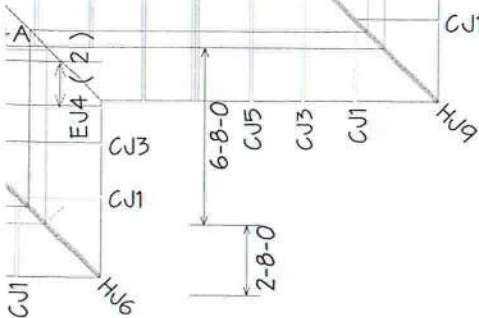
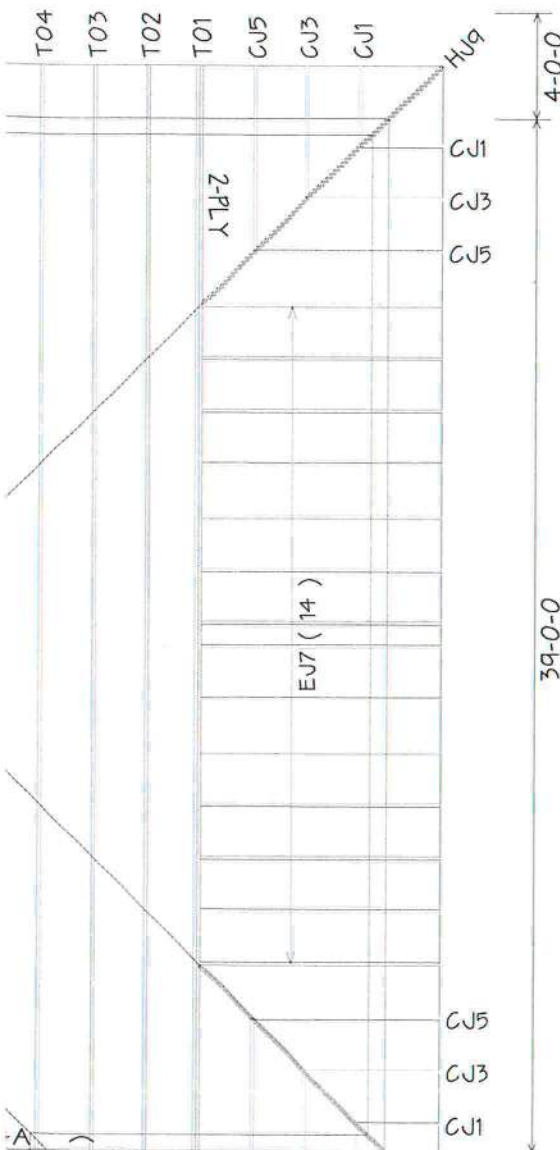
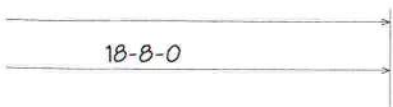
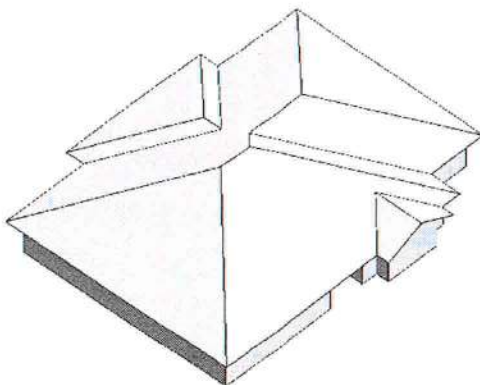
3-11-09

DESIGNED BY:

K.L.H.

JOB #:

300029



BEARING HEIGHT SCHEDULE
8'-0"

6/12 PITCH
2'0" O/H

HANGER SCHEDULE
7 - HTU26

NOTES:

- 1) REFER TO HIR 91 RECOMMENDATIONS FOR HANGING INSTALLATION AND TEMPORARY BRACING. REFER TO ENGINEERED DRAWINGS FOR PERMANENT BRACING REQUIRED.
- 2) ALL TRUSSES (INCLUDING TRUSSES UNDER VALLEY FRAMING) MUST BE COMPLETELY DECKED OR REFER TO DETAIL V09 FOR ALTERNATE BRACING REQUIREMENTS.
- 3) ALL VALLEYS ARE TO BE CONVENTIONALLY FRAMED BY BUILDER.
- 4) ALL TRUSSES ARE DESIGNED FOR 7' OC MAXIMUM SPACING, UNLESS OTHERWISE NOTED.
- 5) ALL WALLS SHOWN ON PLACEMENT PLAN ARE CONSIDERED TO BE LOAD BEARING, UNLESS OTHERWISE NOTED.
- 6) 5"x2" TRUSSES MUST BE INSTALLED WITH THE TOP BEING UP.
- 7) ALL 820T TRUSS HANGERS TO BE SHIPSON 1020 UNLESS OTHERWISE NOTED. ALL FIELD TRUSSES MUST BE SHIPSON 1044Z UNLESS OTHERWISE NOTED.
- 8) BEAM/JOIST/INTEL (JOI) TO BE FURNISHED BY BUILDER.

SHOP DRAWING APPROVAL

THIS LAYOUT IS THE SOLE SOURCE FOR FABRICATION OF TRUSSES AND VIDS. ALL PREVIOUS REVISIONS, OR OTHER TRUSS LAYOUTS, REVIEW AND APPROVAL OF THIS LAYOUT MUST BE RECEIVED BEFORE ANY TRUSSES WILL BE BUILT. VERIFY ALL CONDITIONS TO INSURE ADAPTIVE CHANGES THAT WILL RESULT IN EXTRA CHARGES TO YOU.

Expedited Survey Date: _____

Approved By: _____



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SANFORD
PHONE: 407-322-0094 FAX: 407-322-5553

HOUSECRAFT

FARRIMOND RES.

DATE: 3-11-04
DRAWN BY: K.L.H.
REVISION: NT5
300029

