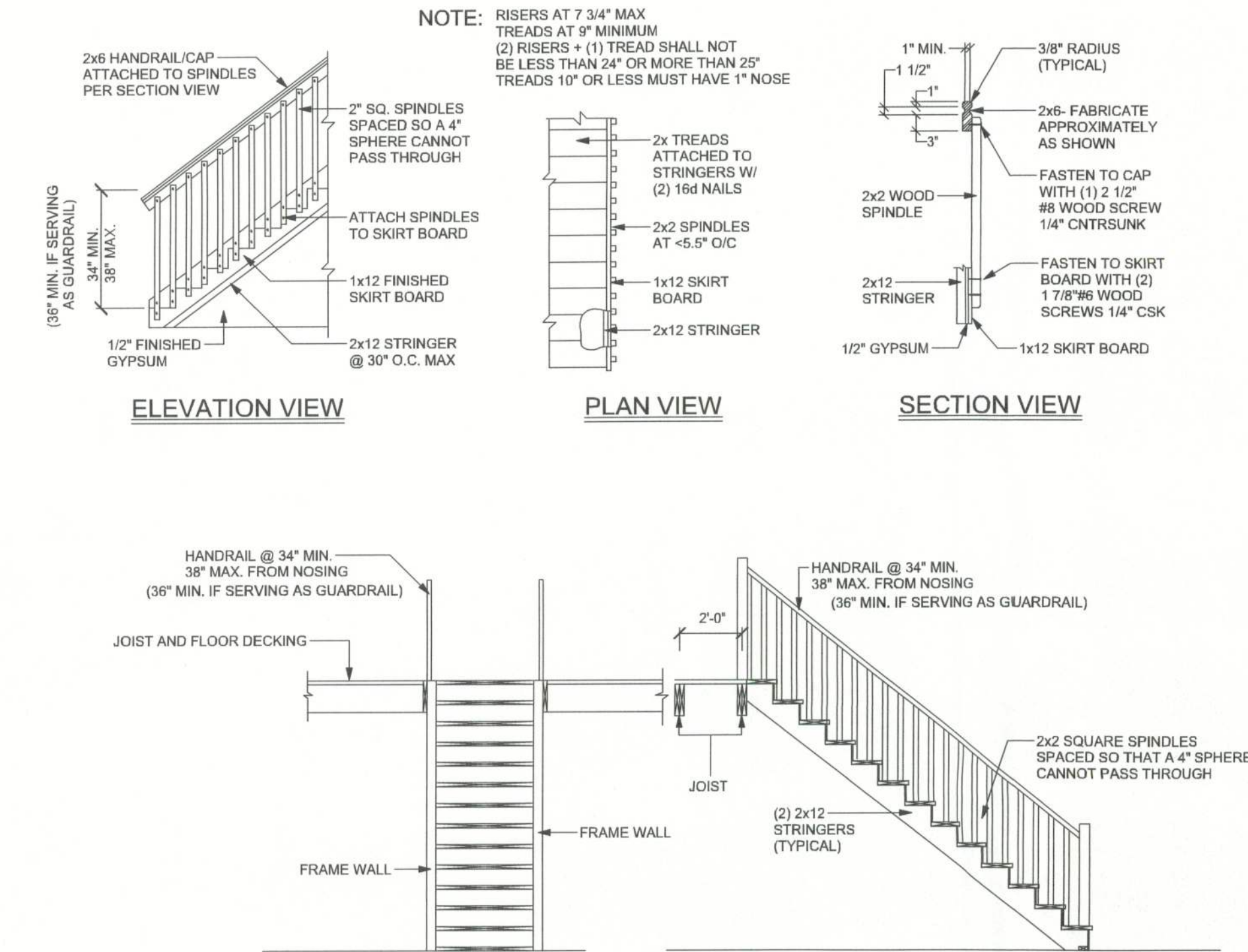


REVISIONS

SOFTPLAN
ARCHITECTURAL DEDICATED TEAM



N19 STAIR DETAIL
SCALE: NTS

UPLIFT LBS.	TRUSS CONNECTOR	TO PLATES	TO RAFTER
455	H3	4-8d	4-8d
535	H2.5A		5-8d
990	H10	8-8d, 1 1/2"	8-8d, 1 1/2"
1470	H16	10-10d, 1 1/2"	2-10d, 1 1/2"
3965	MGT	5/8" THD. ROD	22-10d
UPLIFT LBS.	STRAP CONNECTOR	TO ONE MEMBER	TO OTHER MEMBER
885	SP4	6-10d, 1 1/2"	N/A
1030	CS20	9-8d OR 7-10d	9-8d OR 7-10d
1235	LSTA21	8-10d	8-10d
1240	SPH4	10-10d, 1 1/2"	N/A
1705	CS16	13-8d OR 11-10d	13-8d OR 11-10d
UPLIFT LBS.	COLUMN ANCHOR	TO COLUMN	TO FOUNDATION
1350	LTT19	8-16 sinkers	5/8" x 16" AB
2310	LTT131	18-10d, 1 1/2"	5/8" x 16" AB
2775	HD2A	2-5/8" bolts	5/8" x 16" AB
4175	HTT16	18-16d	5/8" x 16" AB
720	ABA66	8-16d	5/8" x 16" AB
2300	ABU66	12-16d	5/8" x 16" AB

NOTE: ALL CONNECTORS ARE SIMPSON, UNO USE FASTENERS SPECIFIED IN THIS TABLE, UNO

MANUFACTURER AND PRODUCT NUMBERS FOR CONNECTORS, ANCHOR, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLE AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ALL CONNECTIONS EXPOSED DIRECTLY TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION AND HAVE BEEN INCREASED FOR WIND DURATION, UNO, AND MUST BE ADJUSTED FOR OTHER SPECIES OR DURATION, STRAP CONNECTOR CAPACITY MAY BE REDUCED PROPORTIONALLY TO NUMBER OF FASTENERS.

N23 CONNECTOR TABLE
SCALE: NTS

STRUCTURAL AND ROOF DESIGN NOTES

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLE AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. ALL CONNECTIONS EXPOSED DIRECTLY TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

LOG WALLS: ALL LOG WALLS ARE MILLED LOGS WITH FLAT STACKING SURFACES. EACH COURSE IS ATTACHED TO THE COURSE BELOW WITH LOG FASTENERS. FASTENER SPACING IS BASED ON REQUIRED PULLOUT STRENGTH FOR WIND UPLIFT AND REQUIRED SHEAR STRENGTH FOR LATERAL WIND LOADS.

INTERIOR STUD WALLS: ALL INTERIOR STUD WALLS ARE NON-LOAD BEARING. UNO, ROOF LOADS TO BE SUPPORTED BY INTERIOR BEAMS WITH INTERIOR SUPPORT COLUMNS. UNO, BEARING WALL STUDS TO BE SPF#2, UNO, NON-LOAD BEARING WALL STUDS MAY BE SPF STUD GRADE.

ALL PLATES NOT PROTECTED FROM MOISTURE TO BE SPF#2 PT.

EXTERIOR STUD WALLS: ALL EXTERIOR STUD WALLS ARE LOAD BEARING SHEAR WALLS WITH SPF#2 STUDS, SYP#2 PT BOTTOM PLATE, SPF#2 DOUBLE TOP PLATE WITH 10-16d NAILS PER LAP SPLICE. SP4, 6-10d 1/4" STRAP TOP AND BOTTOM AT 48" OC UNO; 7/16" QSB OR 5/8" CDX SHEATHING, WITH PANEL EDGES FULLY BLOCKED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12" OC INTERMEDIATE FRAMING MEMBERS, UNO.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi, UNO, SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS, 7/16" QSB OR 5/8" CDX SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES, STAGGERED, FASTENED WITH 8d COMMON NAILS (131), 6"OC PANEL EDGES, 12" OC INTERMEDIATE MEMBERS, 4" OC GABLE ENDS AND DIAPHRAGM BOUNDARY, UNO.

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBC 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION ON THE BUILDING STRUCTURE.

ROOF VENTILATION: ROOF VENTILATION IS TO MEET OR EXCEED FLORIDA BUILDING CODE SEC. 209.7 FLASHING. BUILDER IS TO PROVIDE FLASHING TO MEET LOCAL CODE REQUIREMENTS AND INSTALLED IN A WORKMANLIKE MANNER TO PREVENT ANY POSSIBILITY OF MOISTURE DAMAGE, TOXIC MOULD, OR ANY OTHER DETRIMENTAL EFFECT. ALSO, FOLLOW FLASHING MANUFACTURER'S DATA SHEET AND SIMILAR LITERATURE AND STANDARDS.

LOGHOG FASTENERS ARE SELF DRILLING, HIGH STRENGTH, F1 = 11KSI STEEL WOOD SCREWS WITH CORROSION COATING MANUFACTURED BY OLYMPIC MANUFACTURING GROUP. SHANK DIAMETER = 0.22", THREAD DIAMETER = 0.32", THREAD LENGTH = 3.0". OLYGOGS ARE AVAILABLE IN LENGTHS OF 8", 11", 12", 15" & 16". SCREWS IN LOG WALL ARE TO HAVE A MIN. PENETRATION OF 3" INTO LOWER LOG. SELECT A THE FASTENER LENGTH ACCORDINGLY (COUNTER SINK IF NECESSARY).

LOGHOGS FASTENERS ARE LISTED FOR EXAMPLE NOT ENDORSEMENT, AN EQUIVALENT FASTENER OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS. LISTED LOADS HAVE BEEN INCREASED FOR WIND DURATION, AND MUST BE ADJUSTED FOR OTHER DURATIONS OR SPECIES.

LOGHOG FASTENERS DESIGN VALUE VALUE ARE IN POUNDS		
WOOD SPECIES (.005G = SPECIFIC GRAVITY)	SHINGLE SHEAR DESIGN VALUE	WITHDRAWAL DESIGN VALUE:
SOUTHER YELLOW PINE .55 SG	436	1473
SPRUCE - PINE - FIR 42 SG	320	968

* ALLOWABLE WITHDRAWAL STRENGTH IS BASED ON NDS1997 TABLE 8.5 5/16" LAG SCREW VALUE x 3" THREAD x 1.6 WIND LOAD DURATION FACTOR. EXAMPLE FOR SYP: 555G (307LB x 3" x 1.6 = 1473 WITHDRAWAL DESIGN VALUE)

* ALLOWABLE SINGLE SHEAR RESISTANCE IS PER BO PICKETT, NABH, WOOD COUNCIL, BASED ON NDS1997 WORST CASE OF FOUR SHEAR MODES. ALLOWABLE SHEAR VALUES ARE INCREASED 1.6 FOR WIND DURATION.

N12 LOGHOG FASTENER DATA
SCALE: NTS

SITE / FOUNDATION NOTES

SITE PREPARATION: SITE ANALYSIS AND PREPARATION INFORMATION IS NOT PART OF THIS PLAN AND IS THE RESPONSIBILITY OF THE OWNER. ALL FOUNDATIONS AND FOOTINGS ARE DESIGNED FOR STABLE SOIL CONDITIONS WITH 1000 PSF BEARING CAPACITY. SITE INSPECTION OF SOIL CONDITIONS SHALL DETERMINE IF THERE IS ANY EVIDENCE OF UNSUITABLE BEARING MATERIALS. QUESTIONABLE MATERIALS PRESENT SHOULD CALL FOR SOILS TEST AND ANALYSIS BY GEOTECHNICAL ENGINEER TO ASSURE THAT EXPANDING CLAYS AND OTHER PROBLEMATIC SOILS CONDITIONS DO NOT EXIST, OR TO ALLOW MITIGATION SHOULD THEY EXIST. ALL FILL UNDER STRUCTURAL ELEMENTS SHALL BE CLEAN SAND/SOIL FILL, FREE FROM DEBRIS AND ORGANIC MATERIALS COMPACTED IN LIFTS OF NOT MORE THAN 6 IN. LOOSE MEASURE. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL. NO FOUNDATION CONCREAT SHALL BE INSTALLED UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. FOOTING SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. TO MINIMIZE WEATHERING, THE LAST 6" OF EXCAVATION FOR ALL FOOTINGS SHALL BE MADE IMMEDIATELY PRIOR TO PLACEMENT OF FOOTINGS.

FOUNDATION: THE OWNER HAS NOT YET PROVIDED A GEOTECHNICAL REPORT TO THE ENGINEER. ASSUMED SAFE BEARING CAPACITY OF 1000 PSF SHALL BE CONFIRMED IN THE FIELD BY REGISTERED GEOTECHNICAL ENGINEER OR SHALL BE APPROVED BY THE OWNER. FOOTING AND SLABS ARE TO BEAR ON FIRM UNDISTURBED EARTH OR CLEAN SAND / SOIL FILL, FREE FROM DEBRIS AND ORGANIC MATERIALS COMPACTED IN LIFTS OF NOT MORE THAN 6 IN. LOOSE MEASURE. WHERE UNACCEPTABLE MATERIAL OCCURS, EXCAVATE AND REPLACE WITH ENGINEERED FILL. NO FOUNDATION CONCREAT SHALL BE INSTALLED UNTIL ALL FOUNDATION WORK HAS BEEN COORDINATED WITH UNDERGROUND UTILITIES. FOOTING SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. TO MINIMIZE WEATHERING, THE LAST 6" OF EXCAVATION FOR ALL FOOTINGS SHALL BE MADE IMMEDIATELY PRIOR TO PLACEMENT OF FOOTINGS.

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 3000 PSI, WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116, SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, Fy = 60 KSI, ALL LAP SPLICES 40" DB (25" FOR #5 BARS), UNO, ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 2'.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WMM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

CONCREAT BLOCK: ASTM C-80 WITH MEDIUM SURFACE FINISH, Fm = 1500 PSI.

MORTAR: TYPE M OR N FOR ALL MASONRY UNITS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64", WITH 5/8" BOLTS TO BE 3" x 3" x 9/64", WITH 3/4" BOLTS TO BE 3" x 3" x 9/64", WITH 7/8" BOLTS TO BE 3" x 3" x 5/16", UNO.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK. CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBC 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

GRADE & SPECIES TABLE

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS

(1) 2x4 @ 16" OC	TO 11'-1" WALL HEIGHT
(1) 2x4 @ 12" OC	TO 12'-3" WALL HEIGHT
(1) 2x6 @ 16" OC	TO 17'-9" WALL HEIGHT
(1) 2x6 @ 12" OC	TO 19'-7" WALL HEIGHT

DESIGN DATA

WIND LOADS ARE PER FLORIDA BUILDING CODE 2004, SECTION 1609
(FOR ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED AND GABLE-SHAPED ROOFS HAVING A MEAN ROOF HEIGHT NOT EXCEEDING THE LEAST HORIZONTAL DIMENSION OF THE BUILDING OR 60 FT. NOT SITED ON THE UPPER HALF OF A HILL OR ESCARPMENT 60FT IN EXPOSURE B, 30FT IN EXPOSURE C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.)

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE		
BUILDING IS IN THE WIND-BORNE DEBRIS REGION		
1.)	BASIC WIND SPEED = 150 MPH	
2.)	WIND EXPOSURE = B	
3.)	WIND IMPORTANCE FACTOR = 1.0	
4.)	BUILDING CATEGORY = II	
5.)	ROOF ANGLE = 10-45 DEGREES	
6.)	MEAN ROOF HEIGHT = <30 FT	
7.)	INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	
8.)	COMPONENTS AND CLADDING DESIGN WIND PRESSURES: INTERIOR IF OPENING IS >5.00 FROM CORNER ZONE 4	
AREA	+ PRESSURE	- PRESSURE
10	40.5	-43.9
20	38.7	-42.1
50	36.2	-39.7
100	34.4	-37.8
END ZONE, IF OPENING IS <=5.00 FROM CORNER ZONE 5		
AREA	+ PRESSURE	- PRESSURE
10	40.5	-54.2
20	38.7	-50.5
50	36.2	-45.7
100	34.4	-42.1

DESIGN LOADS	
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)
	30 PSF (SLEEPING ROOMS)
	30 PSF (ATTICS WITH STORAGE)
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)
ROOF	20 PSF (FLAT OR <4:12)
	16 PSF (4:12 TO <12:12)
	12 PSF (12:12 AND GREATER)
STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS)	
NO SNOW LOAD	
OWNER / BUILDER TO VERIFY FLOOD ZONE	

N24 DESIGN DATA
SCALE: NTS

WINDLOAD ENGINEER: Mark Disoway,
FE No.53915, PCB 868, Lake City, FL
32056, 386-754-5419

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with section 1609, Florida building code 2004, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY
P.E. 53915

Mark Disoway
09 nov 05
SEAL

ANTHONY & CARMAN MIKULIC RESIDENCE

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PRINTED DATE:
November 09, 2005

DRAWN BY: Evan Beamley CHECKED BY:

FINALS DATE:
09 / Nov / 05

JOB NUMBER:
511014

DRAWING NUMBER

S1

OF 4 SHEETS