

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: 3576025 - TERRANCE JONES | COOLEY WAY CUSTOM 220 N.E Deb Gln.

MiTek USA, Inc.

16023 Swingley Ridge Rd

Chesterfield, MO 63017

Site Information:

Customer Info: Terrance Jones Project Name: 3576025 Model: Custom

Lot/Block:

Subdivision: .

Address: 000 Cooley Way, .

City: Columbia County

State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building,

Name:

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Wind Code: ASCE 7-16

Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.6

Wind Speed: 130 mph Floor Load: N/A psf

Truss Name

T18

Date

6/30/23

This package includes 27 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

> T30949312 T30949313

Seal# T30949309 T30949310 T30949311

No.

No.	Seal#	Truss Name	Date
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 22 22 22 22 22 22 22 22 22 22 22	T30949287 T30949288 T30949290 T30949291 T30949292 T30949293 T30949295 T30949296 T30949297 T30949299 T30949300 T30949300 T30949301 T30949301 T30949303 T30949304 T30949305 T30949305 T30949307 T30949308	T01 T01G T02G T03 T03G T05 T05 T05 T05 T06 T07 T07G T07G T08 T09 T10 T11 T11A T11G T12A T13 T14 T15	6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23 6/30/23



The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



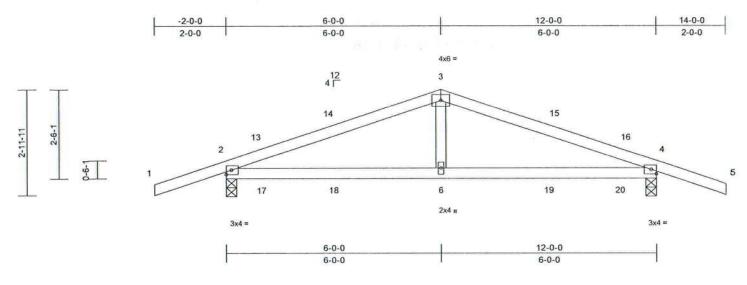
Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 30,2023

Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T01	Common	4	1	T30949287 Job Reference (optional)

Run: 8.63 S. Apr. 6.2023 Print: 8.630 S. Apr. 6.2023 MiTek Industries, Inc. Thu Jun 29.16:07:18 ID:XE9L4ivS5MmXEbfqs5MuzLz1QC3-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1

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Sca		

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.55	Vert(LL)	0.11	6-9	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.39	Vert(CT)	0.10	6-9	>999	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.09	Horz(CT)	-0.01	4	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS		2 2					Weight: 46 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No 2 BOT CHORD 2x4 SP No 2 2x4 SP No.3

WEBS BRACING

TOP CHORD Structural wood sheathing directly applied or

BOT CHORD Rigid ceiling directly applied or 5-1-8 oc

bracing.

2=0-3-8, 4=0-3-8 REACTIONS (size)

Max Horiz 2=54 (LC 8)

Max Uplift 2=-417 (LC 8), 4=-417 (LC 9) Max Grav 2=600 (LC 1), 4=600 (LC 1) (lb) - Maximum Compression/Maximum

FORCES Tension

TOP CHORD 1-2=0/38, 2-3=-761/1282, 3-4=-761/1282,

4-5=0/38

BOT CHORD

2-6=-1096/667, 4-6=-1096/667

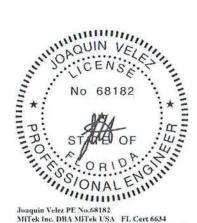
WEBS 3-6=-492/247

NOTES

- Unbalanced roof live loads have been considered for 1) this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior (1) 1-0-0 to 6-0-0, Exterior(2R) 6-0-0 to 9-0-0, Interior (1) 9-0-0 to 14-0-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 417 lb uplift at joint 2 and 417 lb uplift at joint 4.

LOAD CASE(S) Standard



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 30,2023

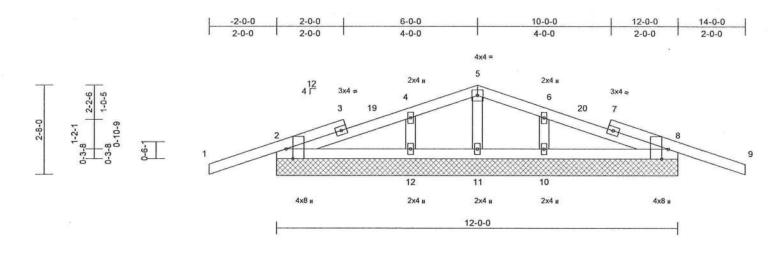
MARNING - Verify design perameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T01G	Common Supported Gable	1	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:20 ID:IJiDu3QoCG3ZH2tuKv6lgCz1QBP-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.35	Vert(LL)	n/a		n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.08	Vert(CT)	n/a		n/a	999			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.00	8	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS						51/25/TE	Weight; 54 lb	FT = 20%	

LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No 2 2x4 SP No.3 **OTHERS**

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size)

2=12-0-0, 8=12-0-0, 10=12-0-0, 11=12-0-0, 12=12-0-0, 13=12-0-0,

16=12-0-0

Plate Offsets (X V): 12:0-3-8 Edgel 18:0-3-8 Edgel

Max Horiz 2=50 (LC 8), 13=50 (LC 8) Max Uplift 2=-189 (LC 8), 8=-196 (LC 9), 10=-119 (LC 13), 11=-19 (LC 8),

12=-118 (LC 12), 13=-189 (LC 8),

16=-196 (LC 9)

2=289 (LC 23), 8=289 (LC 24), 10=267 (LC 24), 11=94 (LC 1), 12=267 (LC 23), 13=289 (LC 23),

16=289 (LC 24) (lb) - Maximum Compression/Maximum

FORCES Tension

TOP CHORD 1-2=0/38, 2-4=-219/107, 4-5=-39/88,

5-6=-37/90, 6-8=-218/107, 8-9=0/38 2-12=-101/257, 11-12=-19/106,

BOT CHORD

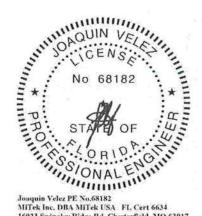
10-11=-19/106, 8-10=-101/260 5-11=-83/69, 4-12=-189/268, 6-10=-189/268

WEBS NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16: Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -2-0-0 to 1-2-8, Exterior(2N) 1-2-8 to 6-0-0, Corner(3R) 6-0-0 to 9-0-0, Exterior(2N) 9-0-0 to 14-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom 7) chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 189 lb uplift at joint 2, 196 lb uplift at joint 8, 19 lb uplift at joint 11, 118 Ib uplift at joint 12, 119 lb uplift at joint 10, 189 lb uplift at joint 2 and 196 lb uplift at joint 8.

LOAD CASE(S) Standard



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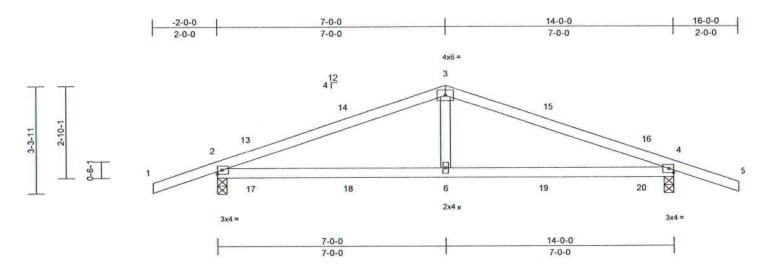
June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MT ek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design, Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T02	Common	3	1	Job Reference (optional)

Run: 8.63 S. Apr. 6 2023 Print: 8.630 S. Apr. 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:21 ID:u?YWqsbavaqayCxa8rM1E9z1QBB-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:34.1

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.63	Vert(LL)	0.17	6-12	>977	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.53	Vert(CT)	0.15	6-12	>999	180	Manager and American	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.11	Horz(CT)	-0.02	4	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS		0.15					Weight: 53 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 2x4 SP No 2 BOT CHORD

2x4 SP No.3 WEBS

BRACING

Structural wood sheathing directly applied or TOP CHORD

5-4-14 oc purlins

BOT CHORD Rigid ceiling directly applied or 4-7-13 oc

bracing.

REACTIONS (size) 2=0-3-8, 4=0-3-8

Max Horiz 2=-61 (LC 13)

Max Uplift 2=-467 (LC 8), 4=-467 (LC 9) Max Grav 2=680 (LC 1), 4=680 (LC 1)

FORCES Tension

(lb) - Maximum Compression/Maximum

1-2=0/38, 2-3=-929/1434, 3-4=-929/1434,

TOP CHORD 4-5=0/38

2-6=-1239/818, 4-6=-1239/818 **BOT CHORD**

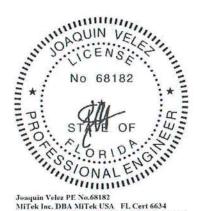
WEBS 3-6=-566/297

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior (1) 1-0-0 to 7-0-0, Exterior(2R) 7-0-0 to 10-0-0, Interior (1) 10-0-0 to 16-0-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 467 lb uplift at joint 2 and 467 lb uplift at joint 4.

LOAD CASE(S) Standard



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June 30,2023

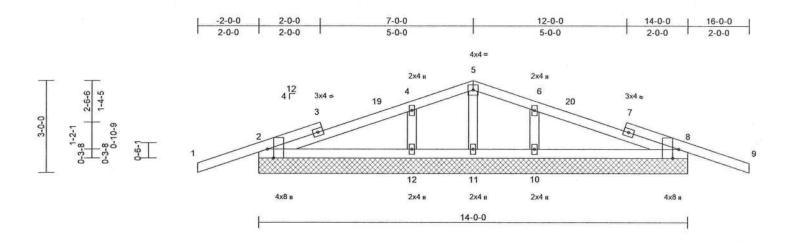
🚵 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property anage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T02G	Common Supported Gable	1	1	Job Reference (optional)

Run: 8.63 S. Apr. 6.2023 Print: 8.630 S. Apr. 6.2023 MiTek Industries, Inc. Thu Jun 29.16:07:21 ID:QM1_I8_H7i_vQGzwaHSm46z1QAg-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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Scale = 1:36.2

Loading	(psf)	Spacing	2-0-0	CSI	1	DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.32	Vert(LL)	n/a		n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.15	Vert(CT)	n/a		n/a	999			
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.09	Horz(CT)	0.00	8	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS	1000000						Weight; 61 lb	FT = 20%	

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

OTHERS 2x4 SP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or

10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc

bracing.

Plate Offsets (X V): 12:0-3-8 Ednel 18:0-3-8 Ednel

REACTIONS (size)

2=14-0-0, 8=14-0-0, 10=14-0-0, 11=14-0-0, 12=14-0-0, 13=14-0-0,

16=14-0-0

Max Horiz 2=-55 (LC 9), 13=-55 (LC 9) Max Uplift 2=-190 (LC 8), 8=-198 (LC 9), 10=-164 (LC 13), 11=-17 (LC 3),

12=-164 (LC 12), 13=-190 (LC 8),

16=-198 (LC 9)

Max Grav 2=305 (LC 23), 8=305 (LC 24), 10=368 (LC 1), 11=31 (LC 13), 12=368 (LC 1), 13=305 (LC 23),

16=305 (LC 24)

FORCES (lb) - Maximum Compression/Maximum

TOP CHORD 1-2=0/38

1-2=0/38, 2-4=-221/116, 4-5=-15/71,

5-6=-14/73 6-

5-6=-14/73, 6-8=-220/116, 8-9=0/38 2-12=-106/248, 11-12=-49/138,

BOT CHORD 2-12=-106/248, 11-12=-49/138 10-11=-49/138, 8-10=-106/252

WEBS

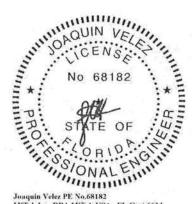
5-11=-53/18, 4-12=-250/311, 6-10=-250/310

NOTES

 Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -2-0-0 to 1-2-8, Exterior(2N) 1-2-8 to 7-0-0, Corner(3R) 7-0-0 to 10-0-0, Exterior(2N) 1-0-0 to 16-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 2, 198 lb uplift at joint 8, 17 lb uplift at joint 11, 164 lb uplift at joint 12, 164 lb uplift at joint 10, 190 lb uplift at joint 2 and 198 lb uplift at joint 8.

LOAD CASE(S) Standard



Juaquin Velez PE No.68182 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

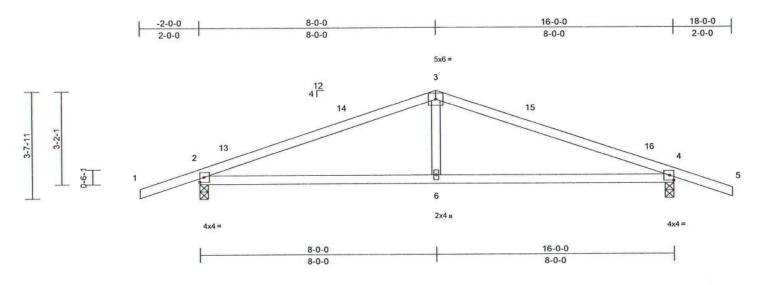
June 30,2023



	Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
Ì	3576025	T03	Common	6	1	T30949291 Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:21 ID:nJrtor2OyFcBW1ruNq2xnAz1QAb-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.82	Vert(LL)	0.09	6-9	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.65	Vert(CT)	-0.18	6-9	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.02	2	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS							Weight: 59 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

WEBS BRACING

TOP CHORD Structural wood sheathing directly applied or

2-2-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 9-7-12 oc

bracing.

REACTIONS (size) 2=0-3-8, 4=0-3-8

Max Horiz 2=67 (LC 12)

Max Uplift 2=-339 (LC 8), 4=-339 (LC 9)

Max Grav 2=760 (LC 1), 4=760 (LC 1) (lb) - Maximum Compression/Maximum

FORCES (lb) - Ma Tension

TOP CHORD 1-2=0/38, 2-3=-1096/490, 3-4=-1096/490,

4-5=0/38

BOT CHORD 2-6=-340/967, 4-6=-340/967

WEBS 3-6=0/347

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior (1) 1-0-0 to 8-0-0, Exterior(2R) 8-0-0 to 11-0-0, Interior (1) 11-0-0 to 18-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 339 lb uplift at joint 2 and 339 lb uplift at joint 4.

LOAD CASE(S) Standard

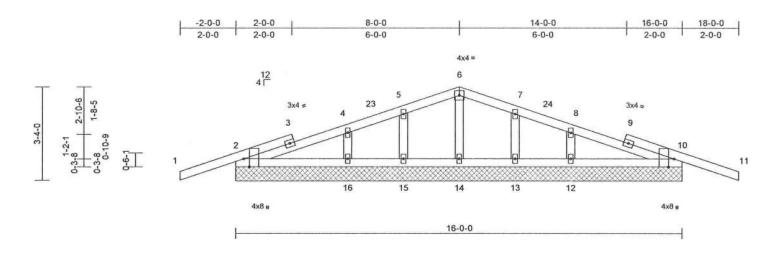
Josquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 30,2023



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T03G	Common Supported Gable	1	1	Job Reference (optional)

Run: 8.63 S. Apr. 6 2023 Print: 8.630 S. Apr. 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:22 ID:4CpNPsY8lrnNxLUmGxHZxpz1Q9y-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:39 6

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.29	Vert(LL)	n/a		n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.08	Vert(CT)	n/a	~	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	20	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS	04000000	The second second				7709700019	Weight: 72 lb	FT = 20%

LUMBER TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc

Dioto Officato (V. V): 12:0.3.9 Edgel [10:0.3.9 Edgel

bracing.

REACTIONS (size) 2=16-0-0, 10=16-0-0, 12=16-0-0, 13=16-0-0, 14=16-0-0, 15=16-0-0, 16=16-0-0, 17=16-0-0, 20=16-0-0

Max Horiz 2=61 (LC 12), 17=61 (LC 12) Max Uplift 2=-187 (LC 8), 10=-195 (LC 9), 12=-113 (LC 13), 13=-76 (LC 9), 14=-17 (LC 12), 15=-77 (LC 8),

16=-112 (LC 12), 17=-187 (LC 8), 20=-195 (LC 9)

Max Grav 2=291 (LC 23), 10=291 (LC 24), 12=256 (LC 1), 13=132 (LC 24), 14=170 (LC 1), 15=132 (LC 23),

16=256 (LC 1), 17=291 (LC 23), 20=291 (LC 24)

FORCES (lb) - Maximum Compression/Maximum

Tension

1-2=0/38, 2-4=-183/103, 4-5=-32/66, TOP CHORD 5-6=-39/113, 6-7=-39/116, 7-8=-30/70,

8-10=-183/103, 10-11=0/38

BOT CHORD 2-16=-97/214, 15-16=-16/92, 14-15=-16/92,

13-14=-16/92, 12-13=-16/92, 10-12=-97/217 WEBS 6-14=-122/57, 5-15=-108/148,

4-16=-180/188, 7-13=-108/148,

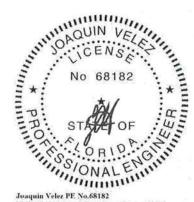
8-12=-180/188

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -2-0-0 to 1-2-8, Exterior(2N) 1-2-8 to 8-0-0, Corner(3R) 8-0-0 to 11-0-0, Exterior(2N) 11-0-0 to 18-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 2, 195 lb uplift at joint 10, 17 lb uplift at joint 14, 77 lb uplift at joint 15, 112 lb uplift at joint 16, 76 lb uplift at joint 13, 113 lb uplift at joint 12, 187 lb uplift at joint 2 and 195 lb uplift at joint 10.

LOAD CASE(S) Standard



Joaquin Velez PF. No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

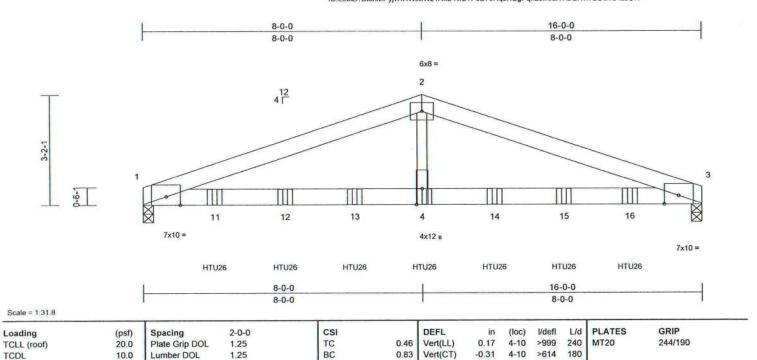
June 30,2023

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T04	Common Girder	1	2	T30949293 Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:22 ID:L5olzi?Bt6hMPvivRHWtMWz1NxL-RfC?PsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



LUMBER

BCLL

BCDL

TOP CHORD 2x6 SP M 26 **BOT CHORD** 2x6 SP M 26 WEBS 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or

0.0

10.0

Rep Stress Incr

Code

NO

FBC2020/TPI2014

5-11-14 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing. REACTIONS (size)

1=0-3-8, 3=0-3-8 Max Horiz 1=54 (LC 12)

Max Uplift 1=-1762 (LC 4), 3=-1775 (LC 5)

Max Grav 1=4854 (LC 1), 3=4890 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-10069/3618, 2-3=-10067/3617

TOP CHORD 1-4=-3411/9595, 3-4=-3411/9595 **BOT CHORD**

WEBS 2-4=-1847/5305

NOTES

2-ply truss to be connected together with 10d 1) (0.131"x3") nails as follows Top chords connected as follows: 2x6 - 2 rows

staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-8-0 oc.

Web connected as follows: 2x4 - 1 row at 0-9-0 oc. All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B),

unless otherwise indicated.

Unbalanced roof live loads have been considered for

this design.

Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone; Lumber DOL=1.60 plate grip DOL=1.60

Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

0.60

Horz(CT)

0.05

3

n/a n/a

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

WB

Matrix-MS

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP M 26 crushing capacity of 805 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1762 lb uplift at joint 1 and 1775 lb uplift at joint 3.
- 10) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 13-11-4 to connect truss(es) to front face of bottom chord.
- 11) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

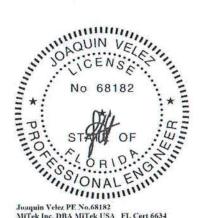
Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (lb/ft)

Vert: 1-2=-60, 2-3=-60, 5-8=-20

Concentrated Loads (lb)

Vert: 4=-1207 (F), 11=-1212 (F), 12=-1212 (F), 13=-1207 (F), 14=-1207 (F), 15=-1207 (F), 16=-1212



Weight: 162 lb FT = 20%

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June 30,2023

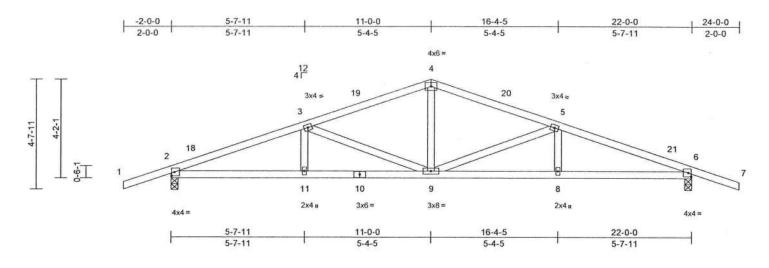
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify dasign parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MINT-A FeV. 3.19.200 BEFORE 05S.

Design valid for use only with MITERS connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM				
3576025	T05	Common	1	1	T30949294 Job Reference (optional)				

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Scale = 1:46.8

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.48	Vert(LL)	0.10	9-11	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.68	Vert(CT)	-0.21	9-11	>999	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.06	6	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS						2000	Weight: 101 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or

3-10-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-3-12 oc

bracing

REACTIONS (size) 2=0-3-8, 6=0-3-8 Max Horiz 2=-88 (LC 13)

Max Uplift 2=-422 (LC 8), 6=-422 (LC 9)

Max Grav 2=1000 (LC 1), 6=1000 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

TOP CHORD 1-2=0/38, 2-3=-1901/758, 3-4=-1384/599,

4-5=-1384/599, 5-6=-1901/758, 6-7=0/38

BOT CHORD 2-11=-626/1747, 9-11=-626/1747,

8-9=-642/1747, 6-8=-642/1747 3-11=0/182, 3-9=-562/287, 4-9=-154/528,

WEBS 5-9=-562/288, 5-8=0/182

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior (1) 1-0-0 to 11-0-0, Exterior(2R) 11-0-0 to 14-0-0, Interior (1) 14-0-0 to 24-0-0 zone: C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 422 lb uplift at joint 2 and 422 lb uplift at joint 6.

LOAD CASE(S) Standard

No 681

No 681

No 681

No 681

No 681

No 681

No 68182

Mirek Inc. DBA MFT-1603 SOAQUIN VEL 68182

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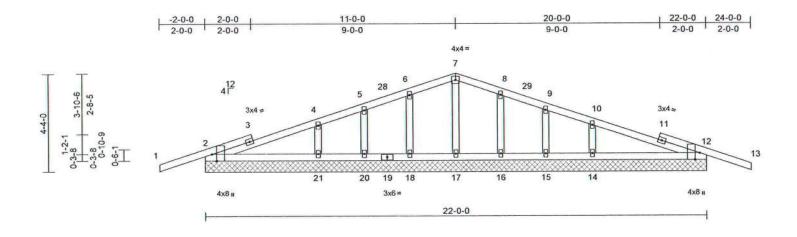
June 30,2023

MARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and tracing of frusses and truss systems, see

ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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Scale = 1:48.9

Plate Offsets (X, Y):	[2:0-3-8,Edge],	[12:0-3-8,Edge]										
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.14	Vert(CT)	n/a		n/a	999		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	12	n/a	n/a		
DOOL	10.0	Codo	EBC2020/TDI2014	Matriy MS		0.100					Weight: 102 lb	FT = 20%

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_	ı.	,	n	п	D	_	п	

TOP CHORD 2x4 SP No.2 2x4 SP No.2 **BOT CHORD** OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size)

2=22-0-0, 12=22-0-0, 14=22-0-0, 15=22-0-0, 16=22-0-0, 17=22-0-0, 18=22-0-0, 20=22-0-0, 21=22-0-0, 22=22-0-0, 25=22-0-0

Max Horiz 2=81 (LC 16), 22=81 (LC 16) 2=-185 (LC 8), 12=-195 (LC 9), Max Uplift

14=-160 (LC 13), 15=-48 (LC 9), 16=-89 (LC 13), 17=-2 (LC 12), 18=-90 (LC 12), 20=-48 (LC 8), 21=-159 (LC 12), 22=-185 (LC 8),

25=-195 (LC 9)

Max Grav 2=310 (LC 23), 12=310 (LC 24), 14=356 (LC 24), 15=68 (LC 1), 16=193 (LC 24), 17=155 (LC 1), 18=193 (LC 23), 20=68 (LC 1), 21=356 (LC 23), 22=310 (LC 23), 25=310 (LC 24)

FORCES TOP CHORD (lb) - Maximum Compression/Maximum

1-2=0/38, 2-4=-153/104, 4-5=-20/67, 5-6=-16/87, 6-7=-35/128, 7-8=-35/126,

8-9=-17/84, 9-10=-19/56, 10-12=-154/104, 12-13=0/38

BOT CHORD

2-21=-95/178, 20-21=-36/98, 18-20=-36/98, 17-18=-36/98, 16-17=-36/98, 15-16=-36/98,

14-15=-36/98, 12-14=-95/176

7-17=-120/25, 6-18=-143/161, 5-20=-64/73, WEBS 4-21=-245/190, 8-16=-143/161, 9-15=-64/73,

10-14=-245/190

NOTES

- Unbalanced roof live loads have been considered for 1) this design
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -2-0-0 to 1-2-8, Exterior(2N) 1-2-8 to 11-0-0, Corner(3R) 11-0-0 to 14-0-0, Exterior (2N) 14-0-0 to 24-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc. 7)
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf 9) on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 2, 195 lb uplift at joint 12, 2 lb uplift at joint 17, 90 lb uplift at joint 18, 48 lb uplift at joint 20, 159 lb uplift at joint 21, 89 lb uplift at joint 16, 48 lb uplift at joint 15, 160 Ib uplift at joint 14, 185 lb uplift at joint 2 and 195 lb uplift at joint 12.

LOAD CASE(S) Standard



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20801



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T06	Common Girder	1	3	Job Reference (optional)

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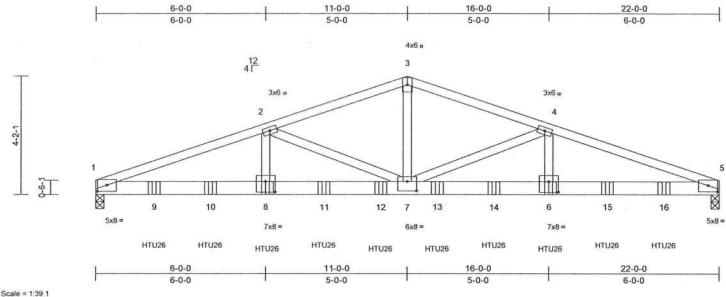


Plate Offsets (X, Y): [1:0-4-0,0-2-10], [5:0-4-0,0-2-10], [6:0-4-0,0-4-8], [7:0-4-0,0-4-0], [8:0-4-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	1.00	Vert(LL)	0.20	6-7	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.57	Vert(CT)	-0.37	6-7	>710	180			
BCLL	0.0*	Rep Stress Incr	NO	WB	0.89	Horz(CT)	0.08	5	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S		10752 107					Weight: 335 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP M 26 2x4 SP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or

4-6-8 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 1=0-3-8, 5=0-3-8 Max Horiz 1=-77 (LC 26)

Max Uplift 1=-2496 (LC 4), 5=-2512 (LC 5)

Max Grav 1=6880 (LC 1), 5=6958 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

TOP CHORD

1-2=-16370/5910, 2-3=-11604/4195,

BOT CHORD

3-4=-11604/4195, 4-5=-16411/5912 1-7=-5560/15351, 5-7=-5505/15390

WEBS 3-7=-2492/6985, 4-7=-4826/1830,

4-6=-1180/3469, 2-7=-4784/1828,

2-8=-1179/3439

NOTES

- 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 - Top chords connected as follows: 2x4 1 row at 0-9-0 OC.
 - Bottom chords connected as follows: 2x6 2 rows staggered at 0-7-0 oc.
 - Web connected as follows: 2x4 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated
- Unbalanced roof live loads have been considered for this design.

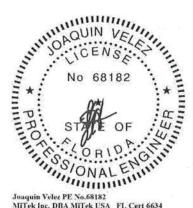
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP M 26 crushing capacity of 805 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2496 lb uplift at joint 1 and 2512 lb uplift at joint 5.
- 10) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 20-0-12 to connect truss(es) to front face of bottom chord.
- 11) Fill all nail holes where hanger is in contact with lumber. LOAD CASE(S) Standard
- Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (lb/ft)

Concentrated Loads (lb)

Vert: 1-3=-60, 3-5=-60, 1-5=-20

Vert: 6=-1207 (F), 8=-1207 (F), 9=-1207 (F), 10=-1207 (F), 11=-1207 (F), 12=-1207 (F), 13=-1207 (F), 14=-1207 (F), 15=-1220 (F), 16=-1224 (F)



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

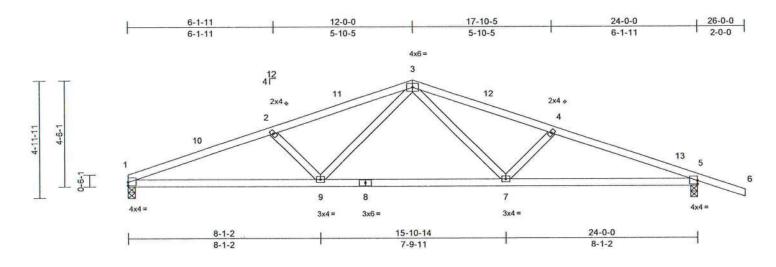
June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify to specify of the overall design. Building of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent occliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	Т07	Common	3	1	T30949297 Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:24 ID:IULcAvyzR s8JwB0RELDWnz1Q6r-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:46.8

Plate Ulisets (A, T).	[1.Edge,0-1-11], [3.Euge,0-1-11]										
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.53	Vert(LL)	-0.13	1-9	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.80	Vert(CT)	-0.30	1-9	>936	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.23	Horz(CT)	0.07	5	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S						All arguests	Weight: 102 lb	FT = 20%

LUMBER

2x4 SP No.2 TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.3 WEBS

BRACING TOP CHORD

Structural wood sheathing directly applied or

3-3-11 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-10-5 oc

Dieta Officia (V. V): [1:Edgs 0 1 11] [5:Edgs 0 1 11]

bracing.

REACTIONS (size) 1=0-3-8, 5=0-3-8

Max Horiz 1=-104 (LC 17)

Max Uplift 1=-333 (LC 8), 5=-456 (LC 9)

Max Grav 1=943 (LC 1), 5=1083 (LC 1)

FORCES (lb) - Maximum Compression/Maximum

Tension 1-2=-2126/874, 2-3=-1879/789,

TOP CHORD 3-4=-1851/773, 4-5=-2101/857, 5-6=0/27

BOT CHORD 1-9=-739/1955, 7-9=-438/1332,

5-7=-730/1913

WEBS

2-9=-363/284, 3-9=-220/599, 3-7=-200/563, 4-7=-338/268

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior (1) 3-1-12 to 12-0-0, Exterior(2R) 12-0-0 to 15-0-0, Interior (1) 15-0-0 to 26-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate orin DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 333 lb uplift at joint 1 and 456 lb uplift at joint 5.

LOAD CASE(S) Standard

ENGIA

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd, Chesterfield, MO 63017

June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T07G	Common Supported Gable	1	1	Job Reference (optional)

Run: 8.63 S. Apr. 6 2023 Print: 8.630 S. Apr. 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:24 ID:2wNyvZs?m1lVpRQQKUBp2bz1Q5g-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1

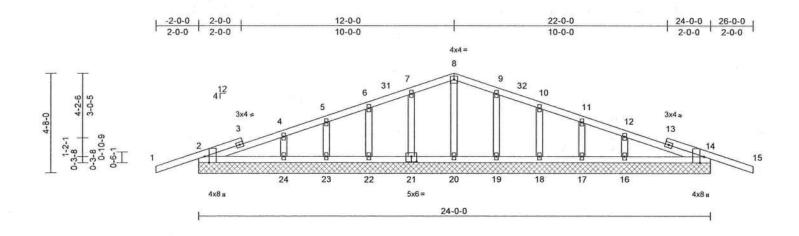


Plate Offsets (X, Y): [2:0-3-8,Edge], [14:0-3-8,Edge], [21:0-3-0,0-3-0]													
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.26	Vert(LL)	n/a	2	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.08	Vert(CT)	n/a	12	n/a	999	100000000000000000000000000000000000000		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	28	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS		Perescular de Santa de La Companya d				1100000	Weight: 114 lb	FT = 20%	

LUMBER TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3 **OTHERS**

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 2=24-0-0, 14=24-0-0, 16=24-0-0, 17=24-0-0, 18=24-0-0, 19=24-0-0, 20=24-0-0, 21=24-0-0, 22=24-0-0, 23=24-0-0, 24=24-0-0, 25=24-0-0, 28=24-0-0

> Max Horiz 2=-88 (LC 13), 25=-88 (LC 13) Max Uplift 2=-179 (LC 8), 14=-190 (LC 9), 16=-113 (LC 13), 17=-72 (LC 9), 18=-78 (LC 13), 19=-79 (LC 9), 21=-79 (LC 8), 22=-78 (LC 12), 23=-72 (LC 8), 24=-112 (LC 12), 25=-179 (LC 8), 28=-190 (LC 9)

2=291 (LC 23), 14=291 (LC 24), Max Grav 16=258 (LC 24), 17=124 (LC 24), 18=168 (LC 1), 19=167 (LC 24), 20=151 (LC 1), 21=167 (LC 23), 22=168 (LC 1), 23=124 (LC 23), 24=258 (LC 23), 25=291 (LC 23),

28=291 (LC 24) **FORCES** (lb) - Maximum Compression/Maximum

Tension

TOP CHORD 1-2=0/38, 2-4=-144/101, 4-5=-42/68,

5-6=-29/85, 6-7=-43/115, 7-8=-57/151, 8-9=-57/146, 9-10=-43/111, 10-11=-29/75, 11-12=-24/42, 12-14=-137/101, 14-15=0/38

2-24=-95/163, 23-24=-21/85, 22-23=-21/85, **BOT CHORD** 20-22=-21/85, 19-20=-21/85, 18-19=-21/85 17-18=-21/85, 16-17=-21/85, 14-16=-95/160 WEBS

8-20=-111/13, 7-21=-128/150, 6-22=-124/101, 5-23=-99/88, 4-24=-182/129, 9-19=-128/150, 10-18=-124/101, 11-17=-99/88, 12-16=-182/130

NOTES

Unbalanced roof live loads have been considered for 1) this design

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -2-0-0 to 1-2-8, Exterior(2N) 1-2-8 to 12-0-0, Corner(3R) 12-0-0 to 15-0-0, Exterior (2N) 15-0-0 to 26-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component. All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 179 lb uplift at joint 2, 190 lb uplift at joint 14, 79 lb uplift at joint 21, 78 lb uplift at joint 22, 72 lb uplift at joint 23, 112 lb uplift at joint 24, 79 lb uplift at joint 19, 78 lb uplift at joint 18, 72 Ib uplift at joint 17, 113 lb uplift at joint 16, 179 lb uplift at joint 2 and 190 lb uplift at joint 14.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

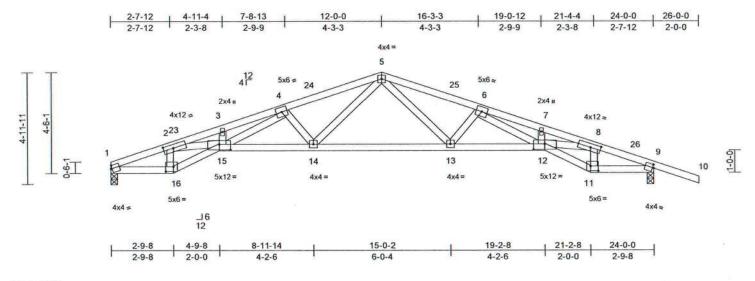
June 30,2023

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Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T08	Roof Special	6	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:24 ID:n?B9YUdbOuinKtbUt9p2zOz1Q3O-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:49.2

Plate Offsets (X, Y):	[1:0-0-11,0-1-1	[2], [2:0-5-4,0-2-0], [8:0-5-4,0-2-0], [9:0-0-11,	,0-1-12], [11:0-4-0	0,0-2-8], [1	2:0-6-0,0-3-0], [15:0-6	5-0,0-3-0)], [16:0-	4-0,0-2	2-8]	
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defI	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	0.29	14-15	>990	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.81	Vert(CT)	-0.60	13-14	>479	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.26	9	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS		1 00 1052					Weight: 115 lb	FT = 20%

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TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

2-3-1 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-9-8 oc

bracing.

REACTIONS (size) 1=0-3-8, 9=0-3-8

Max Horiz 1=-108 (LC 13)

Max Uplift 1=-338 (LC 8), 9=-452 (LC 9) Max Grav 1=955 (LC 1), 9=1085 (LC 1)

(lb) - Maximum Compression/Maximum FORCES

Tension

TOP CHORD

1-2=-2168/874, 2-3=-4525/1739,

3-4=-4487/1766, 4-5=-2617/1060,

5-6=-2602/1060, 6-7=-4407/1721, 7-8=-4438/1695, 8-9=-2066/810, 9-10=0/38

BOT CHORD 1-16=-759/2008, 15-16=-789/2101,

14-15=-1024/2870, 13-14=-586/1782,

12-13=-1037/2845, 11-12=-730/1990,

9-11=-704/1903

7-12=-30/64, 8-12=-866/2403,

8-11=-927/374, 3-15=-16/54,

2-15=-841/2382, 2-16=-954/387,

5-14=-354/961, 5-13=-348/940, 4-14=-711/357, 4-15=-623/1631,

6-13=-693/352, 6-12=-574/1571

NOTES

WEBS

Unbalanced roof live loads have been considered for 1) this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 12-0-0, Exterior(2R) 12-0-0 to 15-0-0, Interior (1) 15-0-0 to 26-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 1 and 452 lb uplift at joint 9.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

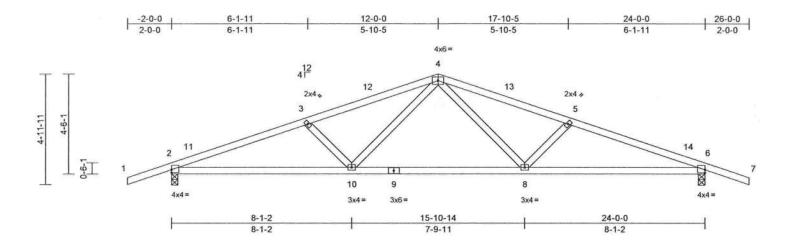
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Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T09	Common	7	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:25 ID:gJUWXT5QRZUOtjUo67UyVOz1Q2o-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:49.9

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.41	Vert(LL)	-0.13	6-8	>999	1070000	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.78	Vert(CT)	-0.29	2-10	>996	180	N. C.		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.07	6	n/a	n/a	W		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 105 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 2x4 SP No.3 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or

3-9-4 oc purlins.

BOT CHORD Rigid ceiling directly applied or 7-0-4 oc

Plate Offsets (X, Y): [2:Edge 0-1-11] [6:Edge 0-1-11]

bracing.

REACTIONS (size) 2=0-3-8, 6=0-3-8

Max Horiz 2=-94 (LC 13)

Max Uplift 2=-454 (LC 8), 6=-454 (LC 9)

Max Grav 2=1077 (LC 1), 6=1077 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

Tension TOP CHORD

1-2=0/27, 2-3=-2085/835, 3-4=-1835/751,

4-5=-1835/751, 5-6=-2085/834, 6-7=0/27 BOT CHORD

2-10=-701/1898, 8-10=-416/1315, 6-8=-709/1898

3-10=-338/268, 4-10=-200/564, WEBS

4-8=-200/564, 5-8=-338/268

NOTES

- 1) Unbalanced roof live loads have been considered for
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior (1) 1-0-0 to 12-0-0, Exterior(2R) 12-0-0 to 15-0-0, Interior (1) 15-0-0 to 26-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing

capacity of 565 psi.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 454 lb uplift at joint 2 and 454 lb uplift at joint 6.

LOAD CASE(S) Standard

No 6811 68182 Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634

16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 30,2023

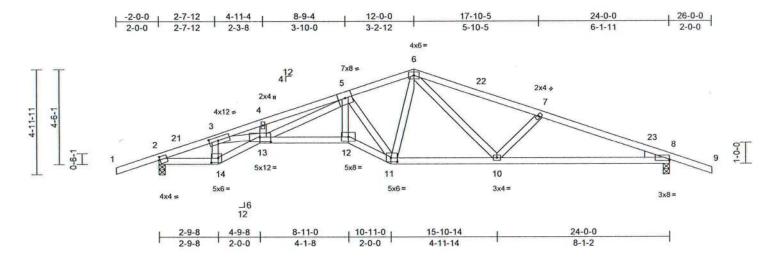
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Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T10	Roof Special	1	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:25 ID:lbWk6MveJ2tkN?tyyo4Shdz1Pui-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:52.2

Plate Offsets (X, Y):	[2:0-0-11,0-1-1	2], [3:0-5-4,0-2-0], [8	8:Edge,0-0-11], [11:0-3-	3,0-2-4], [13:0-6-0),0-2-12], [14:0-4-0,0-2-	8]					
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.62	Vert(LL)	0.27	12-13	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.80	Vert(CT)	-0.54	12-13	>538	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.19	8	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS		No. of the last of				70000	Weight: 120 lb	FT = 20%

LIMOED

LUMBER	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
WEDGE	Right: 2x4 SP No.3

BRACING

min to iii to					
TOP CHORD	Structural wood	sheathing	directly	applied	or

2-2-1 oc purlins.

BOT CHORD Rigid ceiling directly applied or 6-0-10 oc bracing.

REACTIONS

(size) 2=0-3-8, 8=0-3-8

Max Horiz 2=-94 (LC 13)

Max Uplift 2=-450 (LC 8), 8=-450 (LC 9) Max Grav 2=1080 (LC 1), 8=1080 (LC 1)

FORCES (lb) - Maximum Compression/Maximum

Tension

1-2=0/38, 2-3=-2050/793, 3-4=-4423/1653, TOP CHORD

4-5=-4429/1714, 5-6=-1577/700, 6-7=-1852/761, 7-8=-2111/854, 8-9=0/38

BOT CHORD 2-14=-671/1887, 13-14=-695/1972,

12-13=-845/2486, 11-12=-970/2867,

10-11=-431/1357, 8-10=-729/1945

3-14=-911/351, 3-13=-860/2407,

4-13=-121/123, 5-13=-739/1932,

5-12=-417/1353, 6-11=-180/452, 6-10=-199/529, 7-10=-372/274,

5-11=-1956/729

NOTES

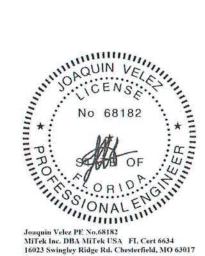
WEBS

- Unbalanced roof live loads have been considered for 1) this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior (1) 1-0-0 to 12-0-0, Exterior(2R) 12-0-0 to 15-0-0, Interior (1) 15-0-0 to 26-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads.

 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 450 lb uplift at joint 2 and 450 lb uplift at joint 8.

LOAD CASE(S) Standard



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information

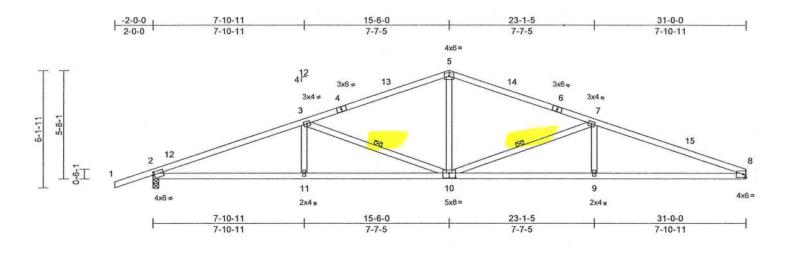
available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T11	Common	8	1 -	T30949302 Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:25
ID:6Rzk7PJGOBAYW8OWWOuW4z1PYD-RfC?PsB70Ho3NSqPanL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:58

Plate Offsets (X, Y):	[2:0-0-11,0-1-1	0], [10:0-4-0,0-3-0]											
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.92	Vert(LL)	0.18	10-11	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.92	Vert(CT)	-0.40	10-11	>928	180	2		
BCLL	0.0	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.13	8	n/a	n/a	-		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S	0.3332.57					101/690	Weight: 136 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 6-8:2x4 SP 2850F

2.0E or 2x4 SP M 31

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc

bracing.

bracin

WEBS 1 Row at midpt 3-10, 7-10
REACTIONS (size) 2=0-3-8, 8= Mechanical

Max Horiz 2=127 (LC 12)

Max Uplift 2=-555 (LC 8), 8=-434 (LC 9)

Max Grav 2=1365 (LC 1), 8=1227 (LC 1)

FORCES (lb) - Ma Tension

(lb) - Maximum Compression/Maximum

Tension

TOP CHORD 1-2=0/27, 2-3=-2875/1116, 3-5=-1974/829,

5-7=-1976/840, 7-8=-2913/1142 BOT CHORD 2-11=-987/2632, 9-11=-1006/2692,

8-9=-1006/2692

WEBS 3-11=0/332, 3-10=-930/461, 5-10=-256/796,

7-10=-994/496, 7-9=0/337

NOTES

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-1-3, Interior (1) 1-1-3 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 555 lb uplift at joint 2 and 434 lb uplift at joint 8.

LOAD CASE(S) Standard



Jusquin Velez PE No.68182 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 30,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

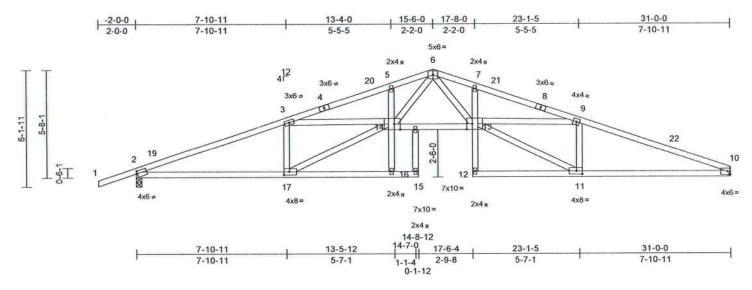
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see MSVIPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM T30949303
3576025	T11A	Roof Special	1	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:26 ID:NPINptpkafhlrzCBFQpDRJz1PAL-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:58

Plate Offsets (X, Y):	[2:0-0-11,0-1-10], [11:0-3-8,0-2-0]	, [13:0-3-8,Edge], [14:0-3-12,Edge], [17:0-3-8,0-2-0]
-----------------------	-------------------------------------	-------------------------------------------------------

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L∕d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	-0.41	15	>895	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.93	Vert(CT)	-0.84	15	>438	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.67	Horz(CT)	0.36	10	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S	20,000	100000000000000000000000000000000000000					Weight: 166 lb	FT = 20%	

LUMBER TOP CHORD

2x4 SP No.2 *Except* 1-4,8-10:2x4 SP

2850F 2.0E or 2x4 SP M 31

2x4 SP No.2 *Except* 7-12,5-16:2x4 SP No.3 **BOT CHORD** WEBS 2x4 SP No.3 *Except* 17-14,13-11:2x4 SP

BRACING

TOP CHORD Structural wood sheathing directly applied or

2-4-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc

bracing. Except: 10-0-0 oc bracing: 14-16

REACTIONS (size)

2=0-3-8, 10= Mechanical

Max Horiz 2=127 (LC 16)

Max Uplift 2=-550 (LC 8), 10=-430 (LC 9) Max Grav 2=1380 (LC 1), 10=1240 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

Tension

2-17=-959/2654, 16-17=-23/63, 15-16=0/0,

1-2=0/27, 2-3=-2900/1088, 3-5=-4633/1716,

TOP CHORD 5-6=-4611/1773, 6-7=-4596/1764,

7-9=-4620/1716, 9-10=-2931/1120

13-14=-1070/3336, 12-13=0/78,

7-13=-158/140, 11-12=-20/60,

10-11=-983/2708, 14-16=0/144,

5-14=-171/147

WEBS

BOT CHORD

3-17=-1041/473, 6-14=-645/1712, 6-13=-658/1691, 9-11=-1053/484, 3-14=-495/1680, 14-17=-1030/2850,

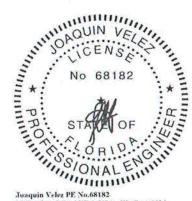
9-13=-459/1611, 11-13=-1060/2913

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-1-3, Interior (1) 1-1-3 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 550 lb uplift at joint 2 and 430 lb uplift at joint 10.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 30,2023

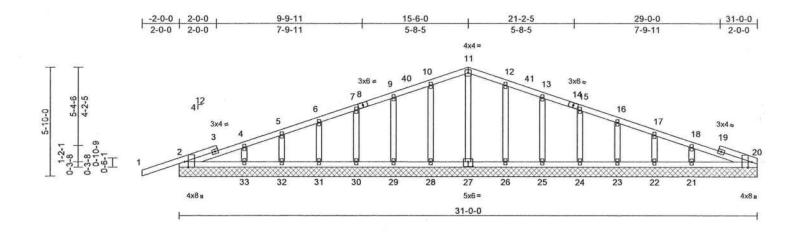
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI1-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MITEMS connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building designs. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T11G	Common Supported Gable	1	1	Job Reference (optional)

Run: 8.63 S. Apr. 6 2023 Print: 8.630 S. Apr. 6 2023 MiTek Industries. Inc. Thu Jun 29 16:07:26 ID:irFjU1CFRITOwmhgc3xZhlz1PDj-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:59.4

FORCES

Plate Offsets (X, Y):	[2:0-3-8,Edge],	[20:0-3-8,Edge], [2	7:0-3-0,0-3-0]										
Loading	(psf)	Spacing	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.26	Vert(LL)	n/a	000 g	n/a	999	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.09	Vert(CT)	n/a	14	n/a	999	The State of the S		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.05	Horz(CT)	0.00	20	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-MS	Cornello					1-04V K 331	Weight: 155 lb	FT = 20%	

BCDL		10.0	Code	FE
LUMBER				
TOP CHORD	2x4 SP N	0.2		
BOT CHORD	2x4 SP N	0.2		
OTHERS	2x4 SP N	0.3		
BRACING				
TOP CHORD	Structura 6-0-0 oc		eathing directly ap	plied or
BOT CHORD	Rigid ceil bracing.	ing directly	y applied or 10-0-0	Оос
REACTIONS	(size)	2=31-0-0	, 20=31-0-0, 21=3	31-0-0,
		22=31-0-	0, 23=31-0-0, 24=	31-0-0,
		25=31-0-	0, 26=31-0-0, 27=	31-0-0,
			0, 29=31-0-0, 30=	
			0, 32=31-0-0, 33=	31-0-0,
			0, 37=31-0-0	
	Max Horiz		C 16), 34=125 (LO	C. 17
	Max Uplift		LC 8), 20=-29 (LC	
			(LC 9), 22=-57 (L	
			LC 9), 24=-74 (LC	
			LC 9), 26=-78 (LC	
			LC 12), 29=-75 (L	
			LC 8), 31=-76 (LC	
			LC 8), 33=-89 (LC	
			(LC 8), 37=29 (LC	
	Max Grav		C 1), 20=120 (LC	
			LC 1), 22=119 (LC LC 1), 24=158 (LC	
			LC 1), 24=158 (LC LC 1), 26=169 (LC	
			LC 1), 28=169 (LC	
			LC 1), 30=159 (LC	
			LC 1), 32=144 (LC	

33=213 (LC 1), 34=283 (LC 1), 37=120 (LC 1) (lb) - Maximum Compression/Maximum

- 1-2=0/38, 2-4=-153/102, 4-5=-77/60, TOP CHORD 5-6=-55/78, 6-7=-38/97, 7-9=-48/116, 9-10=-61/143, 10-11=-74/173, 11-12=-74/165, 12-13=-61/130, 13-15=-48/98, 15-16=-36/67, 16-17=-25/36, 17-18=-39/14, 18-20=-61/27 **BOT CHORD** 2-33=-97/137, 32-33=-19/76, 31-32=-19/76, 30-31=-19/76, 29-30=-19/76, 28-29=-19/76,
- 26-28=-19/76, 25-26=-19/76, 24-25=-19/76, 23-24=-19/76, 22-23=-19/76, 21-22=-19/76, 20-21=-19/76 WEBS 11-27=-104/0, 10-28=-129/151, 9-29=-119/89, 7-30=-120/87, 6-31=-122/88, 5-32=-111/89, 4-33=-154/97, 12-26=-129/150, 13-25=-119/89,
 - 15-24=-119/87, 16-23=-125/90, 17-22=-99/78, 18-21=-184/116

NOTES

- Unbalanced roof live loads have been considered for 1) this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -2-0-0 to 1-2-8, Exterior(2N) 1-2-8 to 15-6-0, Corner(3R) 15-6-0 to 18-7-3, Exterior (2N) 18-7-3 to 31-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 2, 29 lb uplift at joint 20, 79 lb uplift at joint 28, 75 lb uplift at joint 29, 76 lb uplift at joint 30, 76 lb uplift at joint 31, 80 lb uplift at joint 32, 89 lb uplift at joint 33, 78 lb uplift at joint 26, 76 lb uplift at joint 25, 74 lb uplift at joint 24, 80 lb uplift at joint 23, 57 lb uplift at joint 22, 126 lb uplift at joint 21, 167 lb uplift at joint 2 and 29 lb uplift at joint 20

LOAD CASE(S) Standard



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

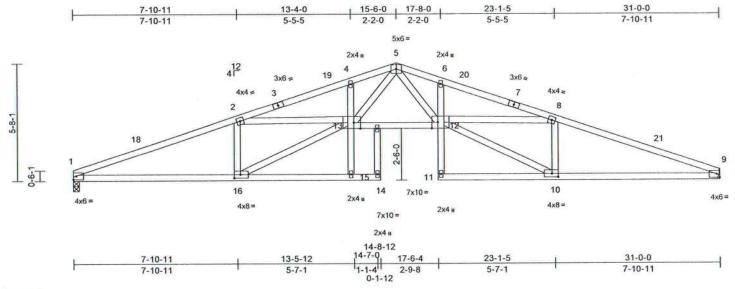
June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITE&S connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must venify the applicability of design parameters and properly incorporate this design into the overall building designer must venify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/PH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM T30949305
3576025	T12A	Roof Special	1	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:27 ID:zb4EDkAgiGy6xoPH4uFZIEz1P27-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:53.3

Plate Offsets (X, Y):	[10:0-3-8,0-2-0]	[12:0-3-8,Edge]	, [13:0-3-12,Edge]	[16:0-3-8,0-2-0]
-----------------------	------------------	-----------------	--------------------	------------------

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.56	Vert(LL)	-0.42	14	>885	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.93	Vert(CT)	-0.85	14	>434	180		
BCLL.	0.0*	Rep Stress Incr	YES	WB	0.67	Horz(CT)	0.37	9	n/a	n/a	and the continue	
BCDL.	10.0	Code	FBC2020/TPI2014	Matrix-S	5-7783	No. of the second second					Weight: 163 lb	FT = 20%

LUMBER

2x4 SP No.2 *Except* 1-3,7-9:2x4 SP 2850F TOP CHORD

2 0F or 2x4 SP M 31

2x4 SP No.2 *Except* 6-11,4-15:2x4 SP No.3 BOT CHORD 2x4 SP No.3 *Except* 12-10,13-16:2x4 SP **WEBS**

BRACING

TOP CHORD Structural wood sheathing directly applied or

2-4-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc

bracing. Except:

10-0-0 oc bracing: 13-15

REACTIONS (size) 1=0-3-8, 9= Mechanical Max Horiz 1=109 (LC 16)

Max Uplift 1=-430 (LC 8), 9=-432 (LC 9)

Max Grav 1=1247 (LC 1), 9=1244 (LC 1)

FORCES (lb) - Maximum Compression/Maximum

Tension

1-2=-2932/1118, 2-4=-4668/1731, TOP CHORD

4-5=-4644/1789, 5-6=-4625/1791,

6-8=-4649/1734, 8-9=-2945/1127

BOT CHORD 1-16=-985/2705, 15-16=-23/63, 14-15=0/0, 12-13=-1083/3361, 11-12=0/78,

6-12=-158/138, 10-11=-20/61,

9-10=-990/2720, 13-15=0/143,

4-13=-159/140

2-16=-1058/483, 5-13=-652/1726, 5-12=-664/1695, 8-10=-1059/487,

2-13=-484/1659, 8-12=-475/1626,

10-12=-1067/2926, 13-16=-1058/2907

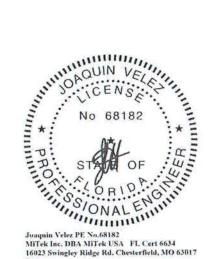
NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-2-15, Interior (1) 3-2-15 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-11-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 1 SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 430 lb uplift at joint 1 and 432 lb uplift at joint 9.

LOAD CASE(S) Standard



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

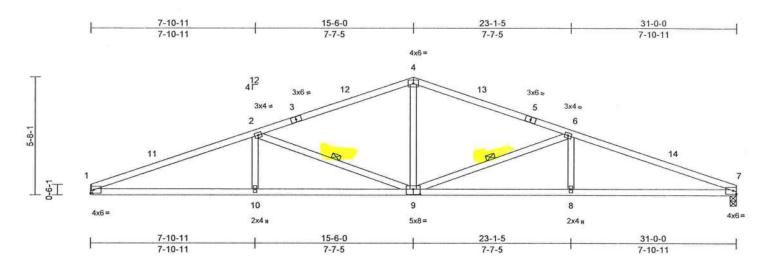
June 30,2023

₩ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20501



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T13	Common	1	1	T30949306 Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:27 ID:WUdZHf_iwsOcU519VTyHzMz1P14-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:53.3

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.72	Vert(LL)	0.18	9-10	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.92	Vert(CT)	-0.38	9-10	>975	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.14	7	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S	100000	STORY OF THE STORY				50000000	Weight: 133 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 3-1,5-7:2x4 SP 2850F

2.0E or 2x4 SP M 31

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

BRACING TOP CHORD

Structural wood sheathing directly applied or

2-2-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc

bracing

WEBS 1 Row at midpt 2-9.6-9 1= Mechanical, 7=0-3-8

REACTIONS (size) Max Horiz 1=109 (LC 12)

Max Uplift 1=-435 (LC 8), 7=-435 (LC 9)

Max Grav 1=1232 (LC 1), 7=1232 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2926/1149, 2-4=-1988/846,

4-6=-1988/846, 6-7=-2908/1142

BOT CHORD 1-10=-1024/2705, 8-10=-1024/2705,

7-8=-1009/2685

2-10=0/337, 2-9=-994/495, 4-9=-265/811,

6-9=-975/489, 6-8=0/335

WEBS NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-12 to 3-1-15, Interior (1) 3-1-15 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 7 SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at joint 1 and 435 lb uplift at joint 7.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

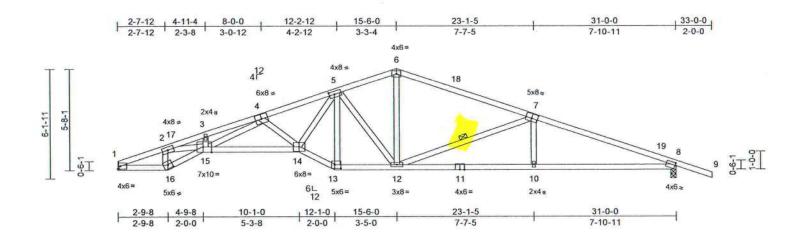
June 30,2023

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Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T14	Roof Special	1	1	T30949307 Job Reference (optional)

Run: 8 63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:27 $ID: 2CqFMhVTXWWA_f41YAHFBtz1Ny_-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?ffCPsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJCPsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJCPsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJCPsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJCPsB70Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4zJAWqAyUlTXbGKWrCDoi7J4ZAyUlTXbGKWrCDoi7J4ZAyUlTXbGKWrCDoi7J4ZAyUlTXbGKWrCDoi7J4WqAyUlTXbGKWrCDoi7J4WqAyUlTXbGKWrCDoi7J4WqAyUlTXbGKWrCDoi7J4WqAyUlTXbGKWrCDoi7J4WqAyUl$



Scale = 1:61.7

Plate Offsets (X, Y): [5:0-3-6,0-2-0], [7:0-4-0,0-3-0], [8:0-0-11,0-1-10], [13:0-4-0,0-2-8], [15:0-5-12,Edge], [16:0-3-0,0-2-7]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.94	Vert(LL)	0.36	14-15	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.90	Vert(CT)	-0.71	14-15	>517	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.79	Horz(CT)	0.27	8	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S		0.00					Weight: 155 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2

2x4 SP No.2 *Except* 15-14:2x4 SP No.1 **BOT CHORD** 2x4 SP No.3 *Except* 15-2:2x4 SP No.2

WEBS BRACING

TOP CHORD Structural wood sheathing directly applied. BOT CHORD Rigid ceiling directly applied or 5-3-7 oc

bracing.

WEBS 1 Row at midpt 7-12

1= Mechanical, 8=0-3-8 REACTIONS (size)

Max Horiz 1=-127 (LC 13) Max Uplift 1=-434 (LC 8), 8=-555 (LC 9)

Max Grav 1=1227 (LC 1), 8=1365 (LC 1)

(lb) - Maximum Compression/Maximum

FORCES Tension

TOP CHORD 1-2=-2953/1165, 2-3=-5930/2254,

3-5=-5872/2270, 5-6=-1906/852,

6-8=-2877/1114, 8-9=0/27

BOT CHORD 1-16=-1021/2702, 15-16=-1054/2808,

14-15=-1418/3894, 13-14=-792/2340, 12-13=-721/2119, 10-12=-962/2630,

8-10=-961/2634

2-16=-1099/440, 2-15=-1069/3022, WEBS

3-15=0/93, 5-14=-724/2065, 5-13=-1001/353,

5-12=-583/297, 6-12=-316/872,

7-12=-928/460, 7-10=0/341, 4-14=-896/422,

NOTES

Unbalanced roof live loads have been considered for 1) this design.

Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-12 to 3-1-15, Interior (1) 3-1-15 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 33-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 8 SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 434 lb uplift at joint 1 and 555 lb uplift at joint 8.

LOAD CASE(S) Standard



Page: 1

MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 30,2023

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ANSITPH Quality Criteria, DSB-89 and BCSI Building Co Safety Information

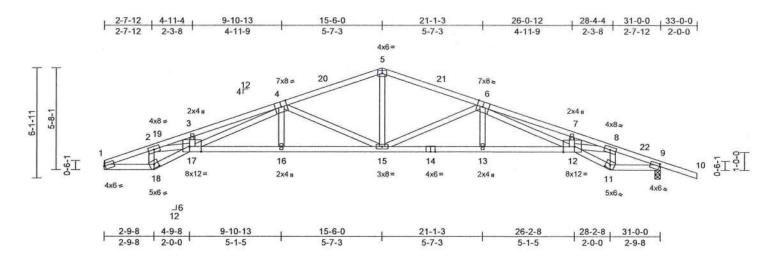
available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T15	Roof Special	3	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:28 ID:nvwhB066ZfpYHuiyQb2cVlz1PVv-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



			[6:0-3-12,Edge], [9:0-0-	T	0 0,0 2 7,		-						
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/a	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.87	Vert(LL)	0.45	16-17	>825	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.99	Vert(CT)	-0.88	15-16	>420	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.95	Horz(CT)	0.45	9	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S		37.74					Weight: 155 lb	FT = 20%	

LUMBER	
TOP CHORD	2x4 SP No.2 *Except* 1-4,6-10:2x4 SP No.1
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* 12-8,17-2:2x4 SP No.2
BRACING	

TOP CHORD Structural wood sheathing directly applied or 1-10-6 oc purlins.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc

bracing.

REACTIONS (size) 1= Mechanical, 9=0-3-8 Max Horiz 1=-127 (LC 13) Max Uplift 1=-434 (LC 8), 9=-555 (LC 9)

Max Grav 1=1227 (LC 1), 9=1365 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-2950/1161, 2-3=-5920/2270, 3-5=-5888/2310, 5-7=-5707/2216,

7-8=-5732/2175, 8-9=-2763/1055, 9-10=0/27 BOT CHORD 1-18=-1017/2698, 17-18=-1049/2800,

16-17=-1195/3393, 15-16=-1195/3396, 13-15=-1196/3358, 12-13=-1197/3356, 11-12=-942/2584, 9-11=-915/2491

6-12=-870/2293, 7-12=-90/116, 8-12=-1112/3048, 8-11=-1010/399, 4-17=-966/2445, 3-17=-72/97, 2-17=-1091/3023, 2-18=-1079/433, 4-16=0/250, 4-15=-1328/581, 5-15=-407/1160, 6-15=-1287/551,

6-13=0/249

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-12 to 3-1-15, Interior (1) 3-1-15 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 33-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: , Joint 9 SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 434 lb uplift at joint 1 and 555 lb uplift at joint 9.

LOAD CASE(S) Standard



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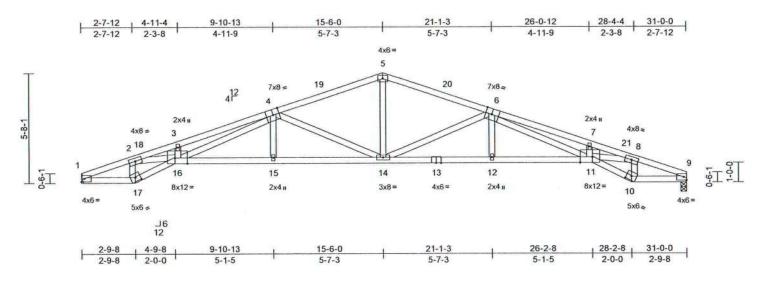
June 30,2023

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Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T16	Roof Special	2	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:28 ID:UUuKRMC3EcNBnMTYWLC0zTz1PX4-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:56.9

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.88	Vert(LL)	0.45	15-16	>814	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	1.00	Vert(CT)	-0.89	14-15	>415	180	and the same of th	
BCLL	0.0	Rep Stress Incr	YES	WB	0.95	Horz(CT)	0.46	9	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 152 lb	FT = 20%

L	U	M	B	E	R	

TOP CHORD 2x4 SP No.2 *Except* 1-4,6-9:2x4 SP No.1 2x4 SP No.2 **BOT CHORD**

2x4 SP No.3 *Except* 11-8,16-2:2x4 SP No.2 WEBS

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-10-2 oc purlins.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc

bracing.

REACTIONS (size) 1= Mechanical, 9=0-3-8

Max Horiz 1=-109 (LC 17)

Max Uplift 1=-435 (LC 8), 9=-435 (LC 9)

Max Grav 1=1232 (LC 1), 9=1232 (LC 1)

FORCES (lb) - Maximum Compression/Maximum

Tension TOP CHORD

1-2=-2961/1167, 2-3=-5945/2302,

3-5=-5913/2342, 5-7=-5834/2319, 7-8=-5865/2279, 8-9=-2892/1140

1-17=-1042/2709, 16-17=-1074/2811,

15-16=-1257/3411, 14-15=-1256/3413,

12-14=-1241/3398, 11-12=-1241/3395,

10-11=-1048/2731, 9-10=-1017/2632

6-11=-953/2385, 7-11=-74/101,

8-11=-1124/3034, 8-10=-1051/432,

4-16=-977/2452, 3-16=-72/96,

2-16=-1132/3037, 2-17=-1084/444, 4-15=0/250, 4-14=-1330/584,

5-14=-417/1171, 6-14=-1313/567,

6-12=0/250

NOTES

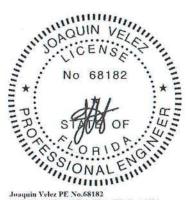
WEBS

BOT CHORD

 Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-12 to 3-1-15, Interior (1) 3-1-15 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: , Joint 9 SP No.2 crushing capacity of 565 psi.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at joint 1 and 435 lb uplift at joint 9.

LOAD CASE(S) Standard



Josquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

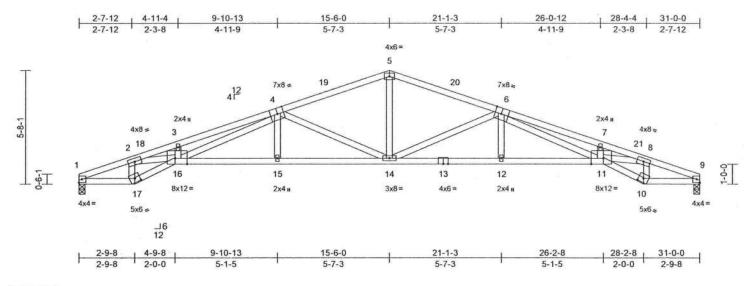
June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T17	Roof Special	1	1	Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:29 ID:QGmWOgToJT6gNG4O6khKd0z1PdB-RtC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:55.3

Plate Offsets (X, Y):	[4:0-3-12,Edge], [6:0-3-12,Edge], [10:0-3-0,0-2-7], [11:0-7-	4,Edge], [16:0-7-	4,Edge], [1	7:0-3-0,0-2-7]						
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	- 6
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.85	Vert(LL)	0.44	15-16	>834	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.25	BC	0.99	Vert(CT)	-0.87	12-14	>422	180			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.94	Horz(CT)	0.45	9	n/a	n/a			
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S	1989						Weight: 152 lb	FT = 20%	

LU	BA	D	_	0
_0	IVI	D	_	n

TOP CHORD 2x4 SP No.2 *Except* 1-4,6-9:2x4 SP No.1 2x4 SP No.2 **BOT CHORD**

WEBS 2x4 SP No.3 *Except* 11-8,16-2:2x4 SP No.2 BRACING

Structural wood sheathing directly applied or TOP CHORD

1-10-15 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 2-2-0 oc

bracing.

REACTIONS (size) 1=0-3-8, 9=0-3-8

Max Horiz 1=-109 (LC 13)

Max Uplift 1=-434 (LC 8), 9=-434 (LC 9) Max Grav 1=1228 (LC 1), 9=1228 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

Tension TOP CHORD

1-2=-2883/1137, 2-3=-5846/2264, 3-5=-5816/2305, 5-7=-5816/2312,

7-8=-5846/2272, 8-9=-2883/1137 1-17=-1009/2624, 16-17=-1040/2723,

BOT CHORD 15-16=-1246/3382, 14-15=-1245/3385,

12-14=-1236/3385, 11-12=-1236/3382,

10-11=-1045/2723, 9-10=-1014/2624

6-11=-951/2380, 7-11=-74/101,

8-11=-1120/3024, 8-10=-1048/431, 4-16=-951/2380, 3-16=-74/97,

2-16=-1127/3024, 2-17=-1048/430,

4-15=0/250, 4-14=-1311/577,

5-14=-413/1163, 6-14=-1311/567, 6-12=0/250

NOTES

WEBS

Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-1-12 to 3-2-15, Interior (1) 3-2-15 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 434 lb uplift at joint 1 and 434 lb uplift at joint 9.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

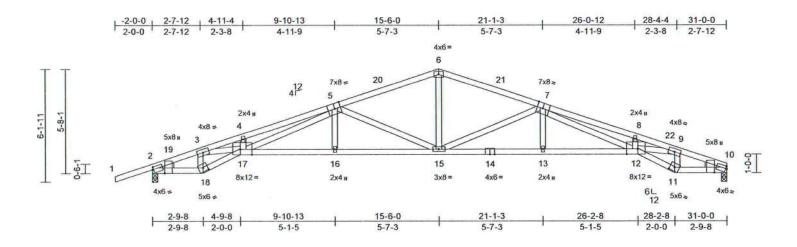
June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T18	Roof Special	2	1	T30949311 Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:29 ID:i2iJoxgP8bND2rF8aAQEKhz1PZ3-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:60

[2:0-0-11,0-1-10], [2:0-2-6,Edge], [5:0-3-12,Edge], [7:0-3-12,Edge], [10:0-0-11,0-1-10], [10:0-2-6,Edge], [11:0-3-0,0-2-7], [12:0-7-4,Edge], [17:0-7-4,Edge],

Plate Offsets (X, Y): [18:0-3-0.0-2-7]

1 1000 00110000 (14 17)	A MONTH POST OF											Color Contractor
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.85	Vert(LL)	-0.43	15	>865	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.98	Vert(CT)	-0,86	13-15	>427	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.94	Horz(CT)	0.45	10	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S		ALC: ALC: ALC: ALC: ALC: ALC: ALC: ALC:					Weight: 157 lb	FT = 20%

LUMBER

2x4 SP No.2 *Except* 1-5,7-10:2x4 SP No.1 TOP CHORD

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 *Except* 12-9,17-3:2x4 SP No.2

WEDGE Left: 2x4 SP No.3

Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or

1-11-3 oc purlins.

BOT CHORD Rigid ceiling directly applied or 2-2-0 oc

bracing.

REACTIONS (size) 2=0-3-8, 10=0-3-8

Max Horiz 2=127 (LC 12)

Max Uplift 2=-554 (LC 8), 10=-432 (LC 9)

Max Grav 2=1362 (LC 1), 10=1224 (LC 1)

FORCES

(lb) - Maximum Compression/Maximum

TOP CHORD

Tension 1-2=0/27, 2-3=-2755/1052, 3-4=-5714/2195,

4-6=-5689/2236, 6-8=-5791/2300, 8-9=-5821/2259, 9-10=-2872/1131

BOT CHORD

2-18=-937/2483. 17-18=-965/2576.

16-17=-1225/3343, 15-16=-1225/3345, 13-15=-1218/3367, 12-13=-1218/3364,

11-12=-1040/2712, 10-11=-1008/2614

7-12=-947/2373, 8-12=-74/101, 9-12=-1113/3010, 9-11=-1044/429,

5-17=-886/2288, 4-17=-91/112,

3-17=-1135/3038, 3-18=-1007/408,

5-16=0/249, 5-15=-1285/560,

6-15=-408/1152, 7-15=-1309/564,

7-13=0/250

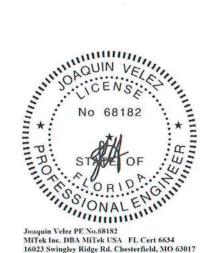
NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-1-3, Interior (1) 1-1-3 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 554 lb uplift at joint 2 and 432 lb uplift at joint 10.

LOAD CASE(S) Standard



MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 30,2023

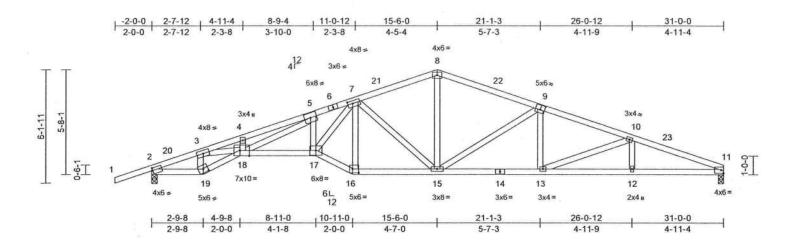
MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MTER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ucliapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TH1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T19	Roof Special	1	1	T30949312 Job Reference (optional)

Run: 8.63 S. Apr. 6 2023 Print: 8.630 S. Apr. 6 2023 MiTak Industries, Inc. Thu Jun 29 16:07:30 ID:uTOJGDcSyyhmv28SCkkrakz1PQ5-RfC?Ps870Hq3NSqPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:60

Plate Offsets (X, Y):	[2:0-0-11,0-1-1	0], [7:0-3-2,0-1-12],	[9:0-2-12,0-3-0], [16:0-4	-0,0-2-8], [18:0-6	6-0,Edge], [19:0-3-0,0-2-	7]					
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.84	Vert(LL)	0.35	17-18	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.81	Vert(CT)	-0.68	17-18	>538	180		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.87	Horz(CT)	0.27	11	n/a	n/a		
BCDL	10.0	Code	FBC2020/TPI2014	Matrix-S							Weight: 163 lb	FT = 20%

LUMBER

WEBS

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

2x4 SP No.2 *Except* 18-17:2x4 SP No.1

2x4 SP No.3 *Except* 18-3:2x4 SP No.2

BRACING TOP CHORD

OP CHORD Structural wood sheathing directly applied or 1-9-2 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-5-6 oc

bracing.

REACTIONS (size) 2=0-3-8, 11=0-3-8

Max Horiz 2=127 (LC 12)

Max Uplift 2=-554 (LC 8), 11=-432 (LC 9) Max Grav 2=1362 (LC 1), 11=1224 (LC 1)

FORCES (lb) - Max

(lb) - Maximum Compression/Maximum

TOP CHORD 1-2=0/27

1-2=0/27, 2-3=-2757/1053, 3-4=-5697/2193,

4-5=-5667/2230, 5-7=-3757/1522, 7-8=-1903/834, 8-10=-2548/1040,

10-11=-2952/1154

BOT CHORD 2-19=-937/2486, 18-19=-965/2576,

17-18=-1315/3585, 16-17=-883/2446,

15-16=-804/2221, 13-15=-860/2381, 12-13=-1029/2719, 11-12=-1029/2719

3-19=-1002/406, 3-18=-1133/3018,

4-18=-75/102, 7-17=-847/2274,

7-16=-1021/407, 7-15=-627/312,

8-15=-328/883, 9-15=-742/364, 9-13=-43/314, 10-13=-400/211, 10-12=0/197,

5-17=-641/299, 5-18=-801/2040

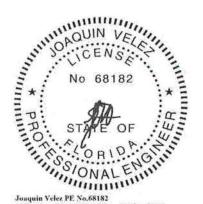
NOTES

WEBS

 Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-1-3, Interior (1) 1-1-3 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 30-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 554 lb uplift at joint 2 and 432 lb uplift at joint 11.

LOAD CASE(S) Standard



Josaquin Velez PE. No.68182 MITek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

June 30,2023

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design in the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

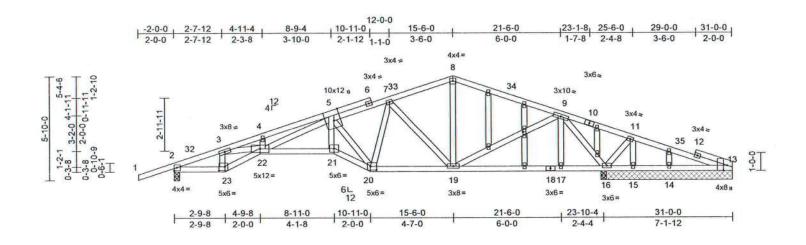
**AMSI/TPIT Quality Criteria, DSB-89 and BCSI Building Component available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qty	Ply	TERRANCE JONES COOLEY WAY CUSTOM
3576025	T19G	Roof Special	1	1	T30949313 Job Reference (optional)

Run: 8.63 S Apr 6 2023 Print: 8.630 S Apr 6 2023 MiTek Industries, Inc. Thu Jun 29 16:07:30 ID:HkYHNDovM7xl9ot9?INYvmz1Oig-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:61.8

Plate Offsets (X, Y):	[5:0-7-0,Edge],	[13:0-3-8,Edge], [2	0:0-4-0,0-2-8], [23:0-4-0	0-2-8], [28:0-1-1	5,0-1-0]							
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.25	TC	0.62	Vert(LL)	0.20	21-22	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.60	Vert(CT)	-0.36	21-22	>792	180	300100000000	
BCLL	0.0*	Rep Stress Incr	YES	WB	0.67	Horz(CT)	0.13	16	n/a	n/a		
BCDI	10.0	Code	EBC2020/TPI2014	Matrix-S							Weight: 181 lb	FT = 20%

LUMBER	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
BRACING	
TOP CHORD	Structural wood sheathing directly

applied or 2-9-13 oc purlins.

BOT CHORD Rigid ceiling directly applied or 5-1-6 oc bracing.

REACTIONS (size)

2=0-3-8, 13=7-3-8, 14=7-3-8, 15=7-3-8, 16=7-3-8

Max Horiz 2=121 (LC 16) Max Uplift 2=-427 (LC 8), 13=-279 (LC 23),

14=-8 (LC 13), 15=-62 (LC 1), 16=-618 (LC 9)

2=949 (LC 1), 13=111 (LC 12),

14=143 (LC 3), 15=43 (LC 8), 16=1834 (LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

Max Grav

1-2=0/27, 2-3=-1720/640, 3-4=-3421/1284, 4-5=-3369/1308, 5-7=-1198/533,

7-8=-690/361, 8-9=-721/347, 9-11=-556/1531, 11-13=-503/1360

BOT CHORD 2-23=-607/1535, 22-23=-625/1598, 21-22=-728/2043, 20-21=-817/2297, 19-20=-333/1029, 17-19=-274/179, 16-17=-274/179, 15-16=-1241/509,

14-15=-1241/509, 13-14=-1241/509 4-22=0/106, 5-22=-566/1321, 5-21=-338/1062, 8-19=-38/237,

9-19=-340/945, 9-17=0/173, 3-23=-616/273, 3-22=-669/1768, 9-16=-1910/741, 7-20=-156/400, 7-19=-621/306, 5-20=-1615/634, 11-16=-283/206

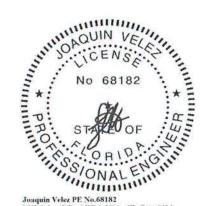
NOTES

WEBS

TOP CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -2-0-0 to 1-1-3, Interior (1) 1-1-3 to 15-6-0, Exterior(2R) 15-6-0 to 18-7-3, Interior (1) 18-7-3 to 31-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 279 lb uplift at joint 13, 427 lb uplift at joint 2, 62 lb uplift at joint 15, 8 lb uplift at joint 14 and 618 lb uplift at joint 16.

LOAD CASE(S) Standard



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017

June 30,2023

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see _____ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



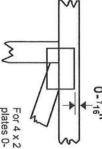
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- "16" from outside edge of truss.

8

0

S

1

This symbol indicates the required direction of slots in connector plates.

Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 × 4

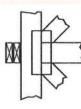
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only

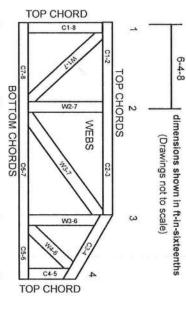
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction. DSB-89: Design Standard for Bracing.

DSB-89 BCSI:

Design Standard for Bracing.
Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports

ESR-1311, ESR-1352, ESR1988 ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet MII-7473 rev. 5/19/2020

2

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- . Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.



Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

RE: Details -

MiTek USA, Inc.

6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info:

Lot/Block:

Address:

City:

Project Name:

Subdivision:

State:

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

License #:

Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014

Wind Code: ASCE 7-16

Roof Load: Varies

Design Program: MiTek 20/20 8.4

Wind Speed: Varies Floor Load: Varies

This package includes 19 individual, MiTek General Details and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date
The second	T23949105	MII-T-BRACE 2	5/17/21
2	T23949106	MII-WEBBRACE-2	5/17/21
3	T23949107	MII-SCAB-BRACE	5/17/21
4	T23949108 T23949109	MII=REP05 MII-GE-130-D-SP	5/17/21 5/17/21
1 2 3 4 5 6 7 8 9	T23949110	MII-GE-130-D-3F	5/17/21
7	T23949111	MII-GE-130-SP LETTER MII-GE170-D-SP MII-GE-180-D-SP	5/17/21
8	T23949112	MII-GE170-D-SP	5/17/21
9	T23949113		5/17/21
10	T23949114	MII-PIGGY-7-16	5/17/21
11	T23949115	MII-PIGGY-7-16 MII-PIGGY-ALT-7-16 MII-REP01A1	5/17/21 5/17/21
12	T23949116 T23949117	MII-REPUTAT	5/17/21
12 13 14	T23949118	MII-VALLEY RIGH WIND1	5/17/21
15	T23949119	MII-VALLEY HIGH WIND2	5/17/21
16	T23949120	MII-VALLEY-SP	5/17/21
17	T23949121	MII-VALLEY	5/17/21
18	T23949122	MII-REP13B MII-STRGBCK	5/17/21 5/17/21
19	T23949123	MIII-9 I KOBOK	3/1//21

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: Magid, Michael

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



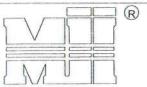
Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

May 17,2021

AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2



MiTek USA, Inc.

T-Brace size

2x4 or 2x6 or 2x8

Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

Nail Spacing

6" o.c.

MiTek USA, Inc.

T23949105

Page 1 of 1

CHELLE	ace Size ne-Ply Truss
	d Continuous Lateral Bracing
1	2

	THOMAS OF EC	ateral braoning
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace Size for Two-Ply Truss					
		Continuous ateral Bracing				
Web Size	1 .	2				
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace				
2x6	2x6 T-Brace	2x6 I-Brace				
2x8	2x8 T-Brace	2x8 I-Brace				

T-Brace / I-Brace must be same species and grade (or better) as web member.

	111 11+	Nails	
		SF	PACING
WEB		1+ 1-1	
			T-BRACE
	A. Company of the com		
Nails	T-Brace Web		

Nailing Pattern Nail Size

10d (0.131" X 3")

Note: Nail along entire length of T-Brace / I-Brace (On Two-Ply's Nail to Both Plies)

Nails Web I-Brace Nails

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May 17,2021

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see
ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



WEB BRACING RECOMMENDATIONS

MII-WEBBRACE-2

T230/0106



							12394	9100
			MAXIMU	I TRUSS	WEB FOR	CE (lbs.)		59 1
BB 405	24"O.C. TRUSS SPACING			48"O.C.TRUSS SPACING			72" O.C. TRUSS SPACING	
BRACE BAY SIZE B	BRACII	BRACING MATERIAL TYPE		BRACING MATERIAL TYPE		BRACING MATERIAL TYPE		
	A	В	С	Α	В	С	В	С
10'-0"	1886	1886	2829	******			-	
12'-0"	1572	1572	2358	3143	3143	4715	4715	7074
14'-0"	1347	1347	2021					
16'-0"	1179	1179	1768	2358	2358	3536	-	
18'-0"	1048	1048	1572				3143	4715
20'-0"	943	943	1414	1886	1886	2829		

TYPE	BRACING MATERIALS			
А	2 X 3 #3, STD, CONST (SPF, DF, HF, OR SP)			
В	2 X 4 #3, STD, CONST (SPF, DF, HF, OR SP)			
С	2 X 6 #3 OR BETTER (SPF, DF, HF, OR SP)			

FOR STABILIZERS:

FOR A SPACING OF 24" O.C. ONLY, MITEK "STABILIZER" TRUSS BRACING SYSTEMS CAN BE SUBSTITUTED FOR TYPE A, B AND C BRACING MATERIAL. DIAGONAL BRACING FOR STABILIZERS ARE TO BE PROVIDED AT BAY SIZE INDICATED ABOVE. WHERE DIAPHRAGM BRACING IS REQUIRED AT PITCH BREAKS, STABILIZERS MAY BE REPLACED WITH WOOD BLOCKING. SEE "STABILIZER" TRUSS BRACING INSTALLATION GUIDE AND PRODUCT SPECIFICATION.

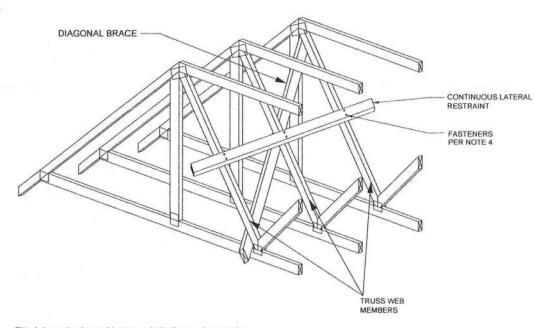
GENERAL NOTES

- DIAGONAL BRACING AND BLOCKING IS REQUIRED TO TRANSFER THE CUMULATIVE LATERAL BRACE FORCE INTO THE ROOF AND/OR CEILING DIAPHRAGM. THE DIAPHRAGM IS AND ANY BLOCKING TO BE DESIGNED BY A QUALIFIED PROFESSIONAL.
 TABULATED VALUES ARE BASED ON LATERAL BRACE CARRYING 2% OF THE WEB FORCE.
- WITH A DOL = 1.15.
 3. DIAGONAL BRACING MATERIAL MUST BE SAME SIZE AND GRADE OR BETTER, AS THE LATERAL
- BRACE MATERIAL, AND SHALL BE INSTALLED IN SUCH A MANNER THAT IT INTERSECTS WEB MEMBERS AT APPROX. 45 DEGREES AND SHALL BE NAILED AT EACH END AND EACH INTERMEDIATE TRUSS WITH 2 - (0.131"x 3") FOR 2x3 and 2x4 BRACES, AND 3- (0.131"x3") FOR
- 2.4 CONNECT LATERAL BRACE TO EACH TRUSS WITH 2 (0.131"x3") NAILS FOR 2x3 AND 2x4 LATERAL BRACES AND 3- (0.131"x3") FOR 2x6 LATERAL BRACES.

 5. LATERAL BRACE SHOULD BE CONTINUOUS AND SHOULD OVERLAP AT LEAST ONE TRUSS
- SPACE FOR CONTINUITY.

 6. FOR ADDITIONAL GUIDANCE REGARDING DESIGN AND INSTALLATION OF BRACING, CONSULT DSB-89 TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES AND BCSI 1 GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES, PRODUCED BY STRUCTURAL BUILDING COMPONENT ASSOCIATION, www.sbcindustry.com

 7. REFER TO SPECIFIC MITENTRENCO TRUSS DESIGN DRAWING FOR WEB MEMBER FORCE.
- 8. BAY SIZE SHALL BE MEASURED IN BETWEEN THE CENTERS OF PAIRS OF DIAGONALS.



This information is provided to assist in the requirement for permanent bracing of the individual truss web members. Additional bracing may still be required for the stability of the overall roof system. The method shown here is just one method that can be used to provide stability against web buckling. Engineering seal, if any, is supporting the web force chart only.



Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

May 17,2021

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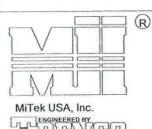
SCAB-BRACE DETAIL

MII-SCAB-BRACE

T23949107

Page 1 of 1

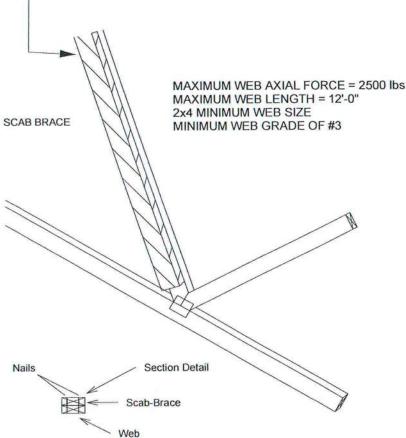
MiTek USA, Inc.



Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

SCAB TO ONE FACE OF WEB WITH APPLY 2x 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. SCAB MUST BE THE SAME GRADE, SIZE AND SPECIES (OR BETTER) AS THE WEB.



Scab-Brace must be same species grade (or better) as web member.



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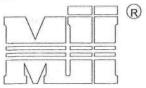


AUGUST 1, 2016

STANDARD REPAIR TO REMOVE END VERTICAL (RIBBON NOTCH VERTICAL)

MII-REP05

MiTek USA, Inc. Page 1 of 1 T23949108



MiTek USA, Inc.

ENGINEERED BY 1. THIS IS A SPECIFIC REPAIR DETAIL TO BE USED ONLY FOR ITS ORIGINAL INTENTION. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLYING REPAIR AND HELD IN PLACE DURING APPLICATION OF REPAIR.

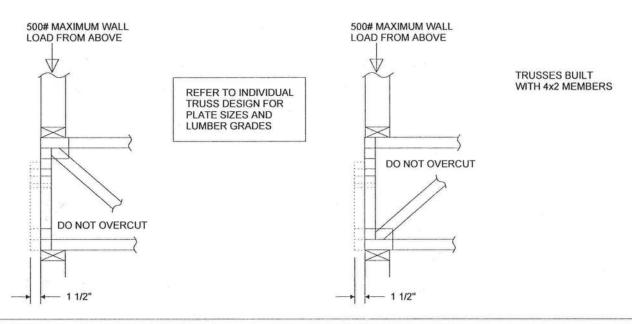
THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE

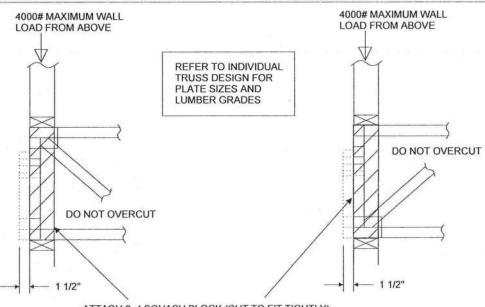
SUCH AS TO AVOID SPLITTING OF THE WOOD.

4. LUMBER MUST BE CUT CLEANLY AND ACCURATELY AND THE REMAINING WOOD MUST BE UNDAMAGED.

5. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 4X_ ORIENTATION ONLY.

6. CONNECTOR PLATES MUST BE FULLY IMBEDDED AND UNDISTURBED.





ATTACH 2x4 SQUASH BLOCK (CUT TO FIT TIGHTLY) TO BOTH SIDES OF THE TRUSS AS SHOWN WITH 10d (0.131" X 3") NAILS SPACED 3" O.C.

TRUSSES BUILT WITH 4x2 MEMBERS



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May 17,2021

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ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MO 20601

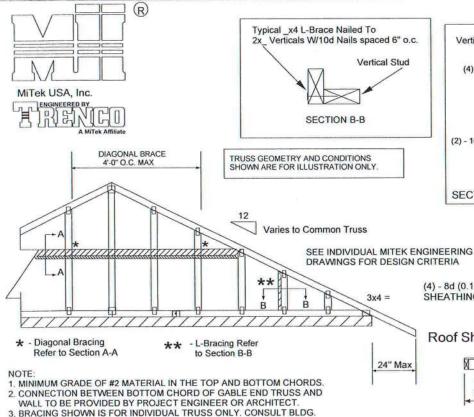


6904 Parke East Blvd. Tampa, FL 36610

APRIL 12, 2019

Standard Gable End Detail

MII-GE130-D-SP



T23949109 MiTek USA, Inc. Page 1 of 2 Vertical Stud DIAGONAL (4) - 16d Nails 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x Verticals w/(4)-10d Nails SECTION A-A

> PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

Roof Sheathing

1'-3"

Max.

(2) - 10dNAILS (2) - 10d NAILS

BRACING OF ROOF SYSTEM. 4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C. 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

DIAPHRAM AT 4'-0" O.C

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.
DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

TYPE TRUSSES

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Diag. Brace at 1/3 points if needed

End Wall

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED

Trusses @ 24" o.c.

TO BLOCKING WITH (5) - 10d NAILS.

HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size Species and Grade	Stud Spacing	Without Brace	1x4 L-Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS
		Maximum Stud Length				
2x4 SP No. 3 / Stud	12" O.C.	3-9-13	4-1-1	5-9-6	7-1-3	11-5-7
2x4 SP No. 3 / Stud	16" O.C.	3-5-4	3-6-8	5-0-2	6-10-8	10-3-13
2x4 SP No. 3 / Stud	24" O.C.	2-9-11	2-10-11	4-1-1	5-7-6	8-5-1

* Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE D

ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH

ASCE 7-10, ASCE 7-16 160 MPH DURATION OF LOAD INCREASE

No 53681

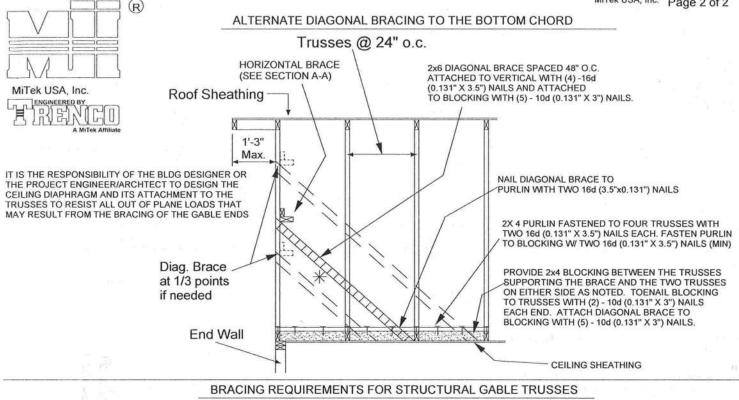
**
PD STATE OF STATE Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

May 17,2021

E 7-10, ASCE 7-16 160 MPH STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
ATION OF LOAD INCREASE: 1:00 CONNECTION OF BRACING IS BASED ON MWFRS.

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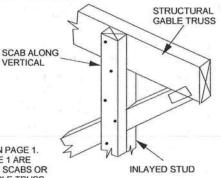
STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED:

METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE
 FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE
 FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL
 MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING
 NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE
 AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:
 FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL
 MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.
 FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL
 MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1.
 ALL BRACING METHODS SHOWN ON PAGE 1 ARE
 VALID AND ARE TO BE FASTENED TO THE SCABS OR

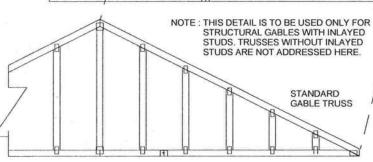


VALID AND ARE TO BE FASTENED TO THE SCABS OF VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE.

AN ADEQUATE DIAPHRAGM OF THE STRUCTURE OF THE STRUCTURE OF THE STRUCTURE.

AN ADEQUATE DIAPHRAGM OF THE VERTICAL OF THE VERTIC

AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.



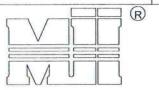
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, cerection and bracing of trusses and truss systems, see ANSITP1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Standard Gable End Detail

MII-GE130-SP

Page 1 of 2

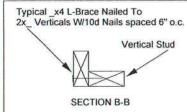


DIAGONAL BRACE

4'-0" O.C. MAX

MiTek USA, Inc.

0



TRUSS GEOMETRY AND CONDITIONS SHOWN ARE FOR ILLUSTRATION ONLY.

24" 1

End Wall

Vertical Stud DIAGONAL (4) - 16d Nails Spaced 6" o.c. (2) - 10d Nails into 2x6 2x6 Stud or 2x4 No.2 of better Typical Horizontal Brace Nailed To 2x Verticals w/(4)-10d Nails SECTION A-A

T23949110 MiTek USA, Inc.

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH (5) - 10d NAILS.

SHEATHING TO 2x4 STD SPF BLOCK

(4) - 8d (0.131" X2.5") NAILS MINIMUM, PLYWOOD

Varies to Common Truss SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA ** 3x4 = - Diagonal Bracing ** - L-Bracing Refer

Refer to Section A-A

to Section B-B

NOTE

- 1. MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT
- 3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT BRACING OF ROOF SYSTEM.
- "L" BRACES SPECIFIED ARE TO BE FULL LENGTH. GRADES: 1x4 SRB OR 2x4 STUD OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.
- 5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF DIAPHRAM AT 4'-0" O.C
- 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 STUD AND A 2x4 STUD AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST STUD. ATTACH TO VERTICAL STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)
 GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.
- THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES
- DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR
- TYPE TRUSSES
- SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC
- NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

Roof Sheathi	ng —
1'-3" Max.	(2) - 10d NAILS (2) - 10d NAILS
Diag. Brace at 1/3 points if needed	Zx6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS AND ATTACHED TO BLOCKING WITH (5) - 10d NAILS.

2 DIAGONAL DIAGONAL Minimum Without **BRACES AT** Stud BRACE Stud Size Brace L-Brace L-Brace 1/3 POINTS Spacing Species Maximum Stud Length and Grade 2x4 SP No. 3 / Stud 12" O.C. 8-0-15 12-1-6 4-0-7 6-3-8 7-4-1 11-0-1 2x4 SP No. 3 / Stud 16" O.C. 3-8-0 3-10-4 5-5-6 2x4 SP No. 3 / Stud 24" O.C. 3-0-10 6-1-5 9-1-15 3-1-12 4-5-6

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 8" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length.

MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING EXPOSURE B or C

ASCE 7-98, ASCE 7-02, ASCE 7-05 130 MPH ASCE 7-10, ASCE 7-16 160 MPH PURATION OF LOAD INCREASE: 1.60

7-98, ASCE 7-02, ASCE 7-05 130 MPH
7-10 ASCE 7-16 160 MPH
STUD DESIGN IS BASED ON COMPONENTS AND CLADDING.
CONNECTION OF BRACING IS BASED ON MW-RS.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PASE MIT 7473 rev. \$119:2020 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, crection and bracing of trusses and truss systems, see ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



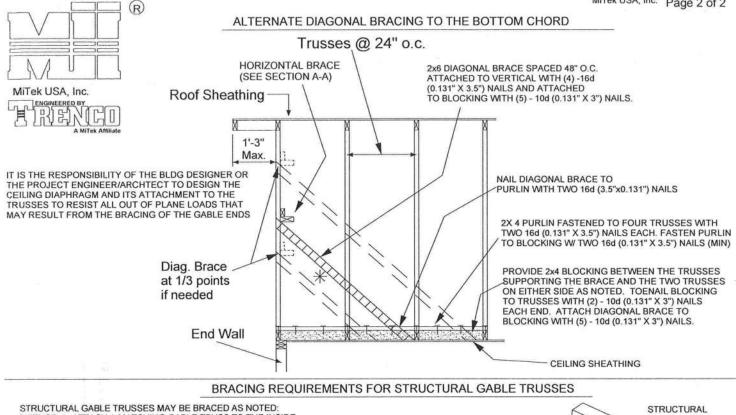
HORIZONTAL BRACE

(SEE SECTION A-A)

Michael S. Magld PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

May 17,2021





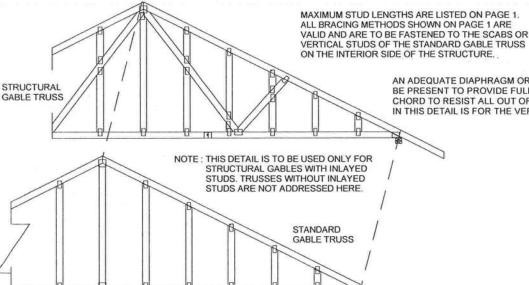
METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

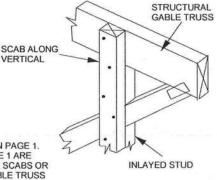
METHOD 2 : ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.

FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

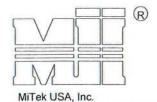




AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115 T23949111

May 17, 2021

TO WHOM IT MAY CONCERN:

RE: MiTek 20/20 drawings showing continuous lateral bracing or "T" bracing on interior webs and chords.

Truss design drawings designed using MiTek 20/20 software show the bracing to be located on a side of the member needing to be braced. The actual side of the member where the brace is to be located does not change the design. If the brace cannot physically be placed on the side of the member as the drawings show, then place the brace on the member at the same location except attach it to the opposite edge.

If we can be of any further assistance in this matter, please feel free to contact our office.

Sincerely,

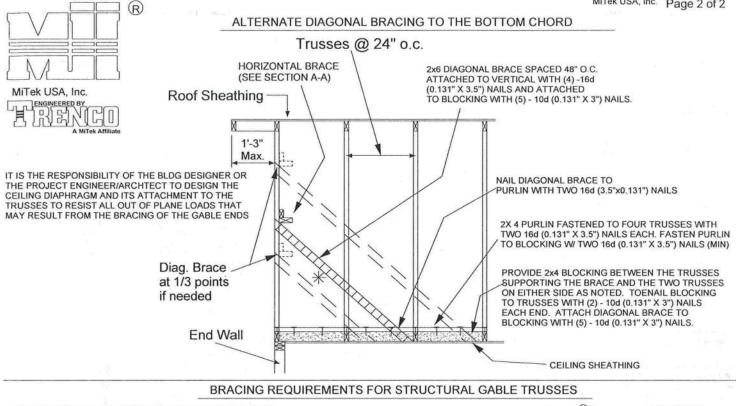
Michael Magid, PE

Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

May 17,2021







STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED: METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

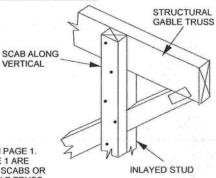
METHOD 2: ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C. FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)

MAXIMUM STUD LENGTHS ARE LISTED ON PAGE 1. ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS ON THE INTERIOR SIDE OF THE STRUCTURE. STRUCTURAL **GABLE TRUSS** NOTE: THIS DETAIL IS TO BE USED ONLY FOR STRUCTURAL GABLES WITH INLAYED STUDS, TRUSSES WITHOUT INLAYED STUDS ARE NOT ADDRESSED HERE. STANDARD GABLE TRUSS



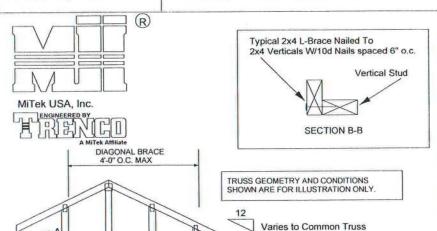
AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

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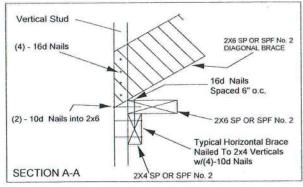


Standard Gable End Detail

MII-GE170-D-SP



T23949112 MiTek USA, Inc. Page 1 of 2



PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK

- Diagonal Bracing Refer to Section A-A

- L-Bracing Refer to Section B-B

**

NOTE

MINIMUM GRADE OF #2 MATERIAL IN THE TOP AND BOTTOM CHORDS. CONNECTION BETWEEN BOTTOM CHORD OF GABLE END TRUSS AND

WALL TO BE PROVIDED BY PROJECT ENGINEER OR ARCHITECT.

3. BRACING SHOWN IS FOR INDIVIDUAL TRUSS ONLY, CONSULT BLDG. ARCHITECT OR ENGINEER FOR TEMPORARY AND PERMANENT

BRACING OF ROOF SYSTEM.

4. "L" BRACES SPECIFIED ARE TO BE FULL LENGTH, SPF or SP No.3 OR BETTER WITH ONE ROW OF 10d NAILS SPACED 6" O.C.

DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

DIAPHRAM AT 4'-0" O.C. 6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A

2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD. ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4. (REFER TO SECTION A-A)

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES.

DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR TYPE TRUSSES.

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE 06-01-13 BY SPIB/ALSC.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

M	
1'-0" Max.	(2) - 1 NAILS
ce	
nts	
Wall	₹
	Max.

Roof Sheathing

SEE INDIVIDUAL MITEK ENGINEERING DRAWINGS FOR DESIGN CRITERIA

24" Max

3x4 =

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

(2) - 10d NAILS

Trusses @ 24" o.c.

HORIZONTAL BRACE (SEE SECTION A-A)

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-9-7	5-8-8	6-11-1	11-4-4			
2x4 SP No. 3 / Stud	16" O.C.	3-4-12	4-11-15	6-9-8	10-2-3			
2x4 SP No. 3 / Stud	24" O.C.	2-9-4	4-0-7	5-6-8	8-3-13			
2x4 SP No. 2	12" O.C.	3-11-13	5-8-8	6-11-1	11-11-7			
2x4 SP No. 2	16" O.C.	3-7-7	4-11-5	6-11-1	10-10-5			
2x4 SP No. 2	24" O.C.	3-1-15	4-0-7	6-3-14	9-5-14			

Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6" o.c., with 3" minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

MAX MEAN ROOF HEIGHT = 30 FEET EXPOSURE D ASCE 7-10, ASCE 7-16 170 MPH DURATION OF LOAD INCREASE: 1.60

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWERS

NO 53681

NO 53681

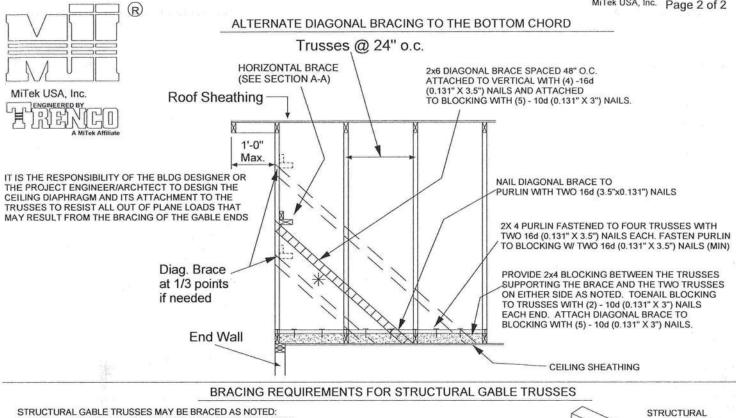
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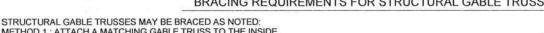
Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

May 17,2021

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly into properly into properly into properly into properly into properly designer. Properly designer in the design parameters and properly designer into properly designer into properly designer. Properly designer into the design parameters and properly designer into the overall building component in the properly designer. Properly designer into the design parameters and properly designer into properly designer. Properly designer into the overall building component into the properly designer into the design parameters and properly designer. Properly designer into the design parameters and properly designer into the design parameters and properly designer. Properly designer into the design parameters and properly designer into the design parameters and properly designer. Properly designer into the design parameters and properly designer into th





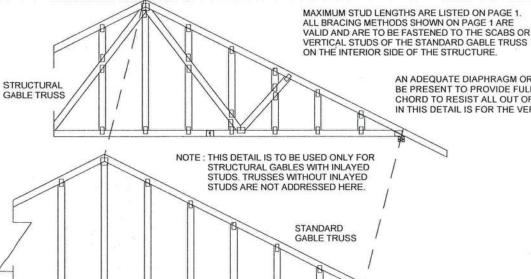


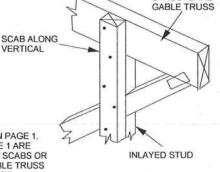
METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2 : ATTACH 2X _ SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C. FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL

MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)





AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY





Standard Gable End Detail

MII-GE180-D-SP

Page 1 of 2

2X6 SP OR SPF No. 2 DIAGONAL BRACE

2X6 SP OR SPF No. 2

(2) - 10d NAILS

Typical Horizontal Brace

Nailed To 2x4 Verticals w/(4)-10d Nails

16d Nails Spaced 6" o.c.

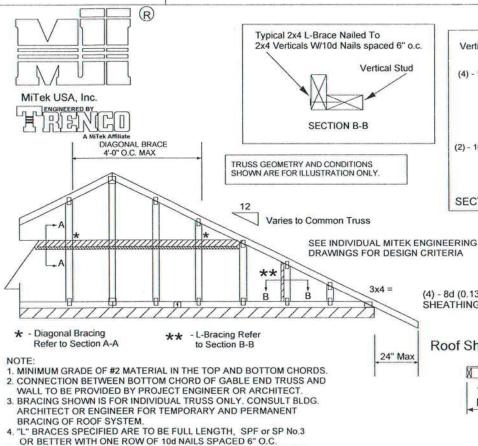
MiTek USA, Inc.

2X4 SP OR SPF No. 2

PROVIDE 2x4 BLOCKING BETWEEN THE FIRST TWO TRUSSES AS NOTED. TOENAIL BLOCKING

TO TRUSSES WITH (2) - 10d NAILS AT EACH END. ATTACH DIAGONAL BRACE TO BLOCKING WITH

T23949113



(4) - 8d (0.131" X 2.5") NAILS MINIMUM, PLYWOOD SHEATHING TO 2x4 STD SPF BLOCK Roof Sheathing

1'-0"

End Wall

(5) - 10d NAILS.

Vertical Stud

(4) - 16d Nails

(2) - 10d Nails into 2x6

SECTION A-A

- 10d Max. NAILS Trusses @ 24" o.c. Diag. Brace at 1/3 points if needed

2x6 DIAGONAL BRACE SPACED 48" O.C. ATTACHED TO VERTICAL WITH (4) -16d NAILS, AND ATTACHED TO BLOCKING WITH (5) -10d NAILS.

May 17,2021

Minimum Stud Size	Stud Spacing	Without Brace	2x4 L-Brace	DIAGONAL BRACE	2 DIAGONAL BRACES AT 1/3 POINTS			
Species and Grade		Maximum Stud Length						
2x4 SP No. 3 / Stud	12" O.C.	3-7-12	5-4-11	6-2-1	10-11-3			
2x4 SP No. 3 / Stud	16" O.C.	3-2-8	4-8-1	6-2-1	9-7-7			
2x4 SP No. 3 / Stud	24" O.C.	2-7-7	3-9-12	5-2-13	7-10-4			
2x4 SP No. 2	12" O.C.	3-10-0	5-4-11	6-2-1	11-6-1			

4-8-1

3-9-12

6-1-1 24" O.C. 2x4 SP No. 2 Diagonal braces over 6'-3" require a 2x4 T-Brace attached to one edge. Diagonal braces over 12'-6" require 2x4 I-braces attached to both edges. Fasten T and I braces to narrow edge of diagonal brace with 10d nails 6in o.c., with 3in minimum end distance. Brace must cover 90% of diagonal length. T or I braces must be 2x4 SPF No. 2 or SP No. 2.

3-5-13

3-0-8

5. DIAGONAL BRACE TO BE APPROXIMATELY 45 DEGREES TO ROOF

THIS DETAIL DOES NOT APPLY TO STRUCTURAL GABLES. DO NOT USE FLAT BOTTOM CHORD GABLES NEXT TO SCISSOR

10. SOUTHERN PINE LUMBER DESIGN VALUES ARE THOSE EFFECTIVE

BRACE TO BE LOCATED AT THE MIDSPAN OF THE LONGEST GABLE STUD.

ATTACH TO VERTICAL GABLE STUDS WITH (4) 10d NAILS THROUGH 2x4.

6. CONSTRUCT HORIZONTAL BRACE CONNECTING A 2x6 AND A 2x4 AS SHOWN WITH 16d NAILS SPACED 6" O.C. HORIZONTAL

GABLE STUD DEFLECTION MEETS OR EXCEEDS L/240.

NAILS DESIGNATED 10d ARE (0.131" X 3") AND NAILS DESIGNATED 16d ARE (0.131" X 3.5")

MAX MEAN ROOF HEIGHT = 30 FEET **EXPOSURE D**

16" O.C.

ASCE 7-10, ASCE 7-16 180 MPH

DIAPHRAM AT 4'-0" O.C.

(REFER TO SECTION A-A)

06-01-13 BY SPIB/ALSC.

TYPE TRUSSES.

2x4 SP No. 2

STUD DESIGN IS BASED ON COMPONENTS AND CLADDING. CONNECTION OF BRACING IS BASED ON MWFRS.

10-5-7

9-1-9

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6-2-1

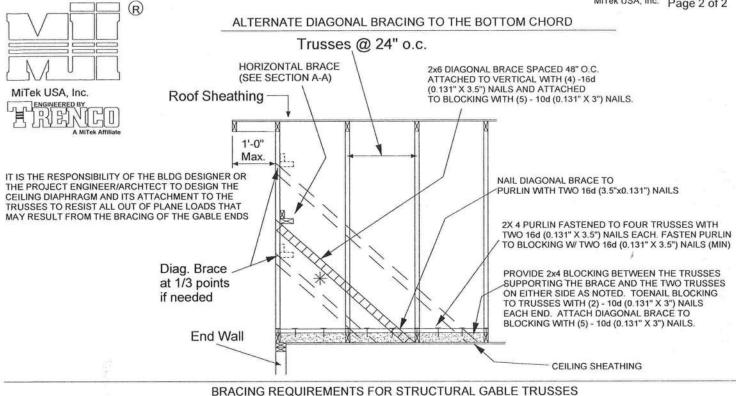


6904 Parke East Blvd.

STRUCTURAL

GABLE TRUSS

INLAYED STUD

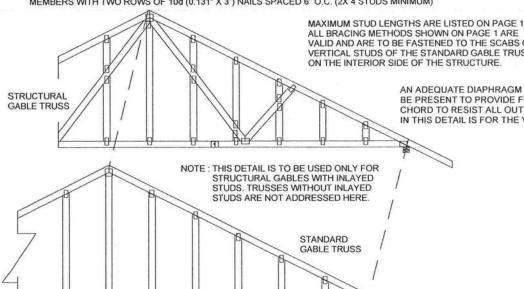


STRUCTURAL GABLE TRUSSES MAY BE BRACED AS NOTED METHOD 1: ATTACH A MATCHING GABLE TRUSS TO THE INSIDE FACE OF THE STRUCTURAL GABLE AND FASTEN PER THE FOLLOWING NAILING SCHEDULE.

METHOD 2: ATTACH 2X SCABS TO THE FACE OF EACH VERTICAL MEMBER ON THE STRUCTURAL GABLE PER THE FOLLOWING NAILING SCHEDULE. SCABS ARE TO BE OF THE SAME SIZE, GRADE AND SPECIES AS THE TRUSS VERTICALS NAILING SCHEDULE:

FOR WIND SPEEDS 120 MPH (ASCE 7-98, 02, 05), 150 MPH (ASCE 7-10, 16) OR LESS, NAIL ALL MEMBERS WITH ONE ROW OF 10d (0.131" X 3") NAILS SPACED 6" O.C.

FOR WIND SPEEDS 120-150 MPH (ASCE 7-98, 02, 05), 150-190 MPH (ASCE 7-10, 16) NAIL ALL MEMBERS WITH TWO ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. (2X 4 STUDS MINIMUM)



ALL BRACING METHODS SHOWN ON PAGE 1 ARE VALID AND ARE TO BE FASTENED TO THE SCABS OR VERTICAL STUDS OF THE STANDARD GABLE TRUSS

> AN ADEQUATE DIAPHRAGM OR OTHER METHOD OF BRACING MUST BE PRESENT TO PROVIDE FULL LATERAL SUPPORT OF THE BOTTOM CHORD TO RESIST ALL OUT OF PLANE LOADS. THE BRACING SHOWN IN THIS DETAIL IS FOR THE VERTICAL/STUDS ONLY.

SCAB ALONG VERTICAL

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters and properly design parameters and properly design parameters and properly design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer parameters and properly design. Pracing indicated is to prevent buckling of individual truss web and/for chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/THI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



January 8, 2019

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-7-16

T23949114

MiTek USA, Inc. Page 1 of 1

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C.

CATEGORY II BUILDING EXPOSURE B or C **ENCLOSED BUILDING** LOADING = 5 PSF TCDL

ASCE 7-10, ASCE 7-16 DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

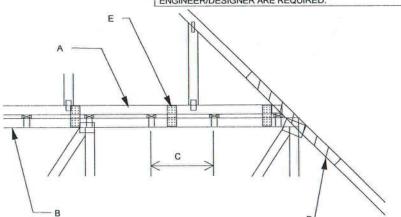


- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING. SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED. B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING
- ONLECS SPECIFIED CLOSER ON MITER TRUSS DESIGN DRAWING.
 CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH.
 -2 X _ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF
 PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED
 ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C.
 SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
 IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
- DIRECTIONS AND:

 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

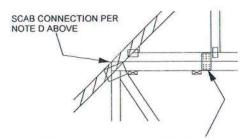
 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
 PIGGYBACK SPAN OF 12 ft.

 E FOR WIND SPEEDS BETWEEN 116 AND 180 MPH, ATTACH
 MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT
 72" O.C. WI (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS
 FROM OPPOSING FACES. ENSURE 0.5" NAIL EDGE DISTANCE.
 (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

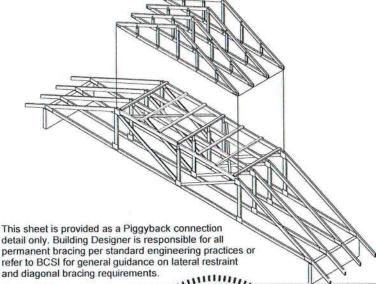


WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

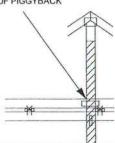
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" NAIL EDGE DISTANCE.



VERTICAL WEB TO EXTEND THROUGH **BOTTOM CHORD** OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP
- MOST MATCH IN SIZE, GRADE, AND MOST LINE OF AS SHOWN IN DETAIL. ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH ATTACH 2 x VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.)
- (MINIMUM 2X4)
 THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS
- GREATER THAN 4000 LBS. FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
- 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS DESIGN.

TO STATE OF Michael S. Magid PE No.53681 MAG

Michael S. Magid PE No.53681

May 17,2021

MiTek USA, Inc.,FL Cert 6634, 6904 Parke East Blvd, Tampa FL 33610

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Date:

Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date: Date:

MiTek

6904 Parke East Blvd

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT 7-16

T23949115

Page 1 of 1 MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING EXPOSURE B or C

ENCLOSED BUILDING LOADING = 5 PSF TCDL MINIMUM ASCE 7-10 ASCE 7-16

DURATION OF LOAD INCREASE: 1.60

DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED



ENGINEERED BY

- PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

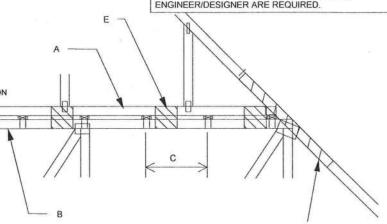
- PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
SHALL BE CONNECTED TO EACH PURLIN
WITH (2) 0(0, 131" X 3.5") TOE-NAILED.

- BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
CONNECT TO BASE TRUSS WITH (2) (0,131" X 3.5") NAILS EACH.

- 2 X _ X 4"-0" SCAB, SIZE TO MATCH TOP CHORD OF
PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON
INTERSECTION, WITH (2) ROWS OF (0,131" X 3") NAILS @ 4" O.C.
SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING
IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND: 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR

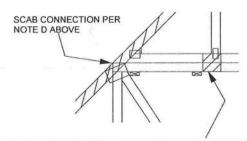
WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM

PIGGYBACK SPAN OF 12 ft. E - FOR WIND SPEED IN THE RANGE 116 MPH - 180 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)



WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint No 53681

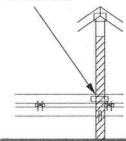
No 53681

No FIATE OF

ORIO

Michael S. Magid PE No.53681 and diagonal bracing requirements.





FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL

ATTACH 2 x ___ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS 2) ATTACH 2 x SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)

THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS

4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.

Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL,33610

Date:

May 17,2021

5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH
THE PIGGYBACK AND THE BASE TRUSS DESIGN.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Date

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MO 20601



AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1

MiTek USA, Inc.

Page 1 of 1

T23949116

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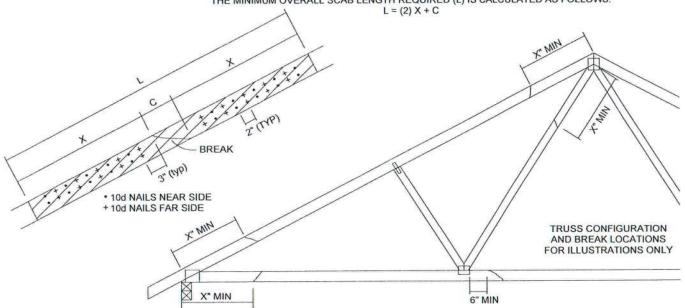
MiTek USA, Inc.

TOTAL NUMBER OF NAILS EACH SIDE OF BREAK *			MAXIMUM FORCE (lbs) 15% LOAD DURATION							
		X	SP		DF		SPF		HF	
2x4	2x6		2x4	2x6	2x4	2x6	2x4	2x6	2x4	2x6
20	30	24"	1706	2559	1561	2342	1320	1980	1352	2028
26	39	30"	2194	3291	2007	3011	1697	2546	1738	2608
32	48	36"	2681	4022	2454	3681	2074	311 <mark>1</mark>	2125	3187
38	57	42"	3169	4754	2900	4350	2451	3677	2511	3767
44	66	48"	3657	5485	3346	5019	2829	4243	2898	4347

* DIVIDE EQUALLY FRONT AND BACK

ATTACH 2x_ SCAB OF THE SAME SIZE AND GRADE AS THE BROKEN MEMBER TO EACH FACE OF THE TRUSS (CENTER ON BREAK OR SPLICE) WITH 10d (0.131" X 3") NAILS (TWO ROWS FOR 2x4, THREE ROWS FOR 2x6) SPACED 4" O.C. AS SHOWN. STAGGER NAIL SPACING FROM FRONT FACE AND BACK FACE FOR A NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

THE LENGTH OF THE BREAK (C) SHALL NOT EXCEED 12". (C=PLATE LENGTH FOR SPLICE REPAIRS) THE MINIMUM OVERALL SCAB LENGTH REQUIRED (L) IS CALCULATED AS FOLLOWS:

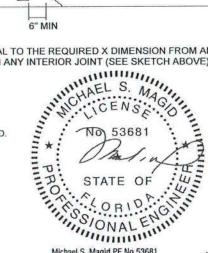


THE LOCATION OF THE BREAK MUST BE GREATER THAN OR EQUAL TO THE REQUIRED X DIMENSION FROM ANY PERIMETER BREAK OR HEEL JOINT AND A MINIMUM OF 6" FROM ANY INTERIOR JOINT (SEE SKETCH ABOVE)

DO NOT USE REPAIR FOR JOINT SPLICES

- THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.

 2. ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
- AND HELD IN PLACE DURING APPLICATION OF REPAIR.
- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID
- LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.
 THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2x ORIENTATION ONLY.
- THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.



Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd-Tampa FL-33610

May 17,2021

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ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP

SIDE VIEW

NEAR SIDE

NEAR SIDE

NEAR SIDE NEAR SIDE

(2x6) 4 NAILS

THIS DETAIL APPLICABLE TO THE THREE END DETAILS SHOWN BELOW

> VIEWS SHOWN ARE FOR ILLUSTRATION PURPOSES ONLY

> > SIDE VIEW (2x3)2 NAILS

SIDE VIEW

NEAR SIDE

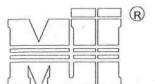
NEAR SIDE

NEAR SIDE

3 NAILS

NEAR SIDE NEAR SIDE MiTek USA, Inc. Page 1 of 1

T23949117





NOTES:

- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

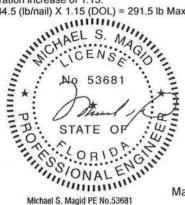
	TOE-NAIL SINGLE SHEAR VALUES PER NDS 2018 (lb/nail)								
	DIAM.	SP	DF	HF	SPF	SPF-S			
m	.131	88.0	80.6	69.9	68.4	59.7			
LONG	.135	93.5	85.6	74.2	72.6	63.4			
3.5" L	.162	108.8	99.6	86.4	84.5	73.8			
LONG	.128	74.2	67.9	58.9	57.6	50.3			
	.131	75.9	69.5	60.3	59.0	51.1			
3.25"	.148	81.4	74.5	64.6	63.2	52.5			

VALUES SHOWN ARE CAPACITY PER TOE-NAIL APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

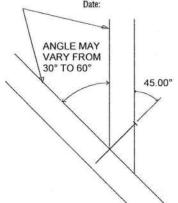
For load duration increase of 1.15:

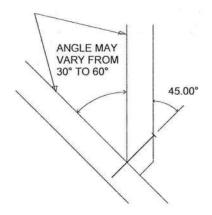
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

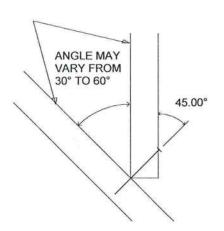


May 17,2021

Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610







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MiTek USA, Inc.

ENGINEERED BY

R

TRUSSED VALLEY SET DETAIL

GABLE END, COMMON TRUSS

OR GIRDER TRUSS

MII-VALLEY HIGH WIND1

MiTek USA, Inc.

Page 1 of 1 T23949118

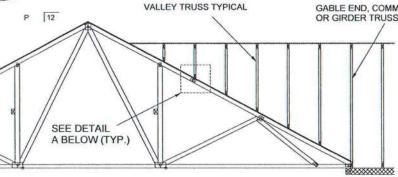
GENERAL SPECIFICATIONS

- 1. NAIL SIZE 10d (0.131" X 3")
- 2. WOOD SCREW = 3" WS3 USP OR EQUIVALENT DO NOT USE DRYWALL OR DECKING TYPE SCREW
- 3. INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A
- 4. BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.
- 5. BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.

EQUILIVANT TO THE RAKE DIMENSION OF A STATE OF WILLIAM STATE OF MAJOR STATE OF MA

Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610

GABLE END, COMMON TRUSS OR GIRDER TRUSS



SECURE VALLEY TRUSS W/ ONE ROW OF 10d NAILS 6" O.C.

VALLEY TRUSS TYPICAL

ATTACH 2x4 CONTINUOUS NO.2 SP TO THE ROOF W/ TWO USP WS3 (1/4" X 3") WOOD SCREWS INTO EACH BASE TRUSS.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16, 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING **EXPOSURE C** WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF

ON THE TRUSSES

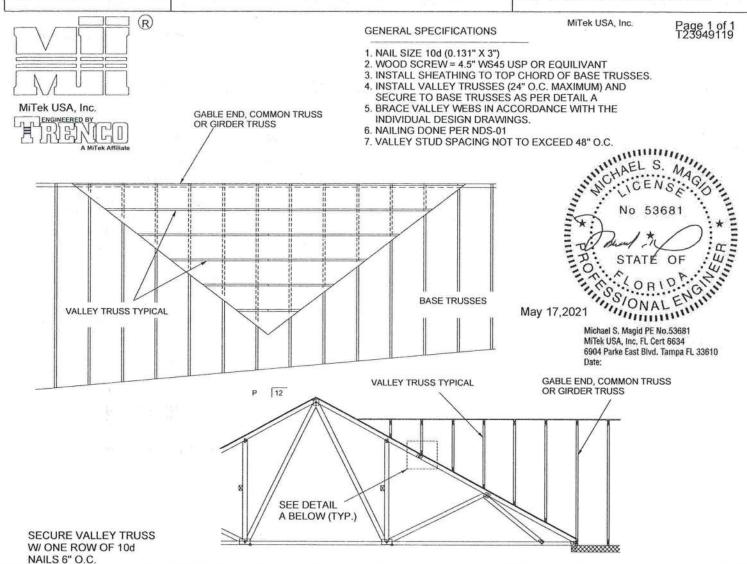
DETAIL A (NO SHEATHING)

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSITPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



TRUSSED VALLEY SET DETAIL

MII-VALLEY HIGH WIND2



ATTACH 2x4 CONTINUOUS NO.2 SP
TO THE ROOF W/TWO USP WS45 (1/4" X 4.5")
WOOD SCREWS INTO EACH BASE TRUSS.

WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 6/12 CATEGORY II BUILDING EXPOSURE C
WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 6 PSF ON THE TRUSSES

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see

ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



MiTek USA, Inc.

ENGINEERED BY

R

TRUSSED VALLEY SET DETAIL

GABLE END, COMMON TRUSS

OR GIRDER TRUSS

MII-VALLEY SP

MiTek USA, Inc.

Page 1 of 1 T23949120

GENERAL SPECIFICATIONS

1. NAIL SIZE 16d (0.131" X 3.5")

 INSTALL VALLEY TRUSSES (24" O.C. MAXIMUM) AND SECURE PER DETAIL A

BRACE VALLEY WEBS IN ACCORDANCE WITH THE INDIVIDUAL DESIGN DRAWINGS.

 BASE TRUSS SHALL BE DESIGNED WITH A PURLIN SPACING EQUILIVANT TO THE RAKE DIMENSION OF THE VALLEY TRUSS SPACING.

5. NAILING DONE PER NDS - 01

6. VALLEY STUD SPACING NOT TO EXCEED 48" O.C.

7. ALL LUMBER SPECIES TO BE SP.

BASE TRUSSES

TO STATE OF

Michael S. Maguer May 17,2021

Michael S. Magid PE No.53681

Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

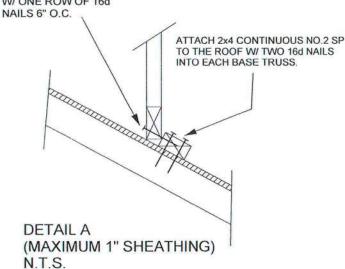
VALLEY TRUSS TYPICAL

GABLE END, COMMON TRUSS OR GIRDER TRUSS

SEE DETAIL
A BELOW (TYP.)

SECURE VALLEY TRUSS W/ ONE ROW OF 16d

VALLEY TRUSS TYPICAL



WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 120 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 150 MPH MAX MEAN ROOF HEIGHT = 30 FEET ROOF PITCH = MINIMUM 3/12 MAXIMUM 10/12 CATEGORY II BUILDING EXPOSURE C OR B WIND DURATION OF LOAD INCREASE: 1.60 MAX TOP CHORD TOTAL LOAD = 60 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) MINIMUM REDUCED DEAD LOAD OF 4.2 PSF ON THE TRUSSES



TRUSSED VALLEY SET DETAIL MII-VALLEY APRIL 12, 2019 (HIGH WIND VELOCITY) NOTE: VALLEY STUD SPACING NOT R MiTek USA, Inc. Page 1 of 1 TO EXCEED 48" O.C. SPACING T23949121 MiTek USA, Inc. ENGINEERED BY FOR BEVELED BOTTOM CHORD, CLIP MAY BE APPLIED TO EITHER FACE CLIP MAY BE APPLIED TO THIS FACE UP TO A MAXIMUM 6/12 PITCH ATTACH VALLEY TRUSSES TO LOWER TRUSSES WITH USP RT7 OR EQUIVALENT WIND DESIGN PER ASCE 7-98, ASCE 7-02, ASCE 7-05 146 MPH WIND DESIGN PER ASCE 7-10, ASCE 7-16 160 MPH MAX MEAN ROOF HEIGHT = 30 FEET CATEGORY II BUILDING NON-BEVELED EXPOSURE B or C **BOTTOM CHORD** WIND DURATION OF LOAD INCREASE: 1.6 MAX TOP CHORD TOTAL LOAD = 50 PSF MAX SPACING = 24" O.C. (BASE AND VALLEY) SUPPORTING TRUSSES DIRECTLY UNDER

VALLEY TRUSSES MUST BE DESIGNED WITH A MAXIMUM UNBRACED LENGTH OF 2'-10" ON AFFECTED TOP CHORDS.

NOTES:

- SHEATHING APPLIED AFTER INSTALLATION OF VALLEY TRUSSES
- THIS DETAIL IS NOT APPLICABLE FOR SPF-S SPECIES LUMBER.

Michael S. Magid PE No.53681 CLIP MUST BE APPLIED MiTek USA, Inc. FL Cert 6634 TO THIS FACE WHEN 6904 Parke East Blvd. Tampa FL 33610 PITCH EXCEEDS 6/12. (MAXIMUM 12/12 PITCH) Date:

NON-BEVELED

BOTTOM CHORD

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and portange. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



May 17,2021

OCTOBER 5, 2016

REPLACE BROKEN OVERHANG

MII-REP13B

MiTek USA, Inc.

Page 1 of 1

T23949122

R

MiTek USA, Inc.

ENGINEERED BY

A MITCK Affiliate

TRUSS CRITERIA:

LOADING: 40-10-0-10 DURATION FACTOR: 1.15 SPACING: 24" O.C. TOP CHORD: 2x4 OR 2x6 PITCH: 4/12 - 12/12 HEEL HEIGHT: STANDARI

HEEL HEIGHT: STANDARD HEEL UP TO 12" ENERGY HEEL

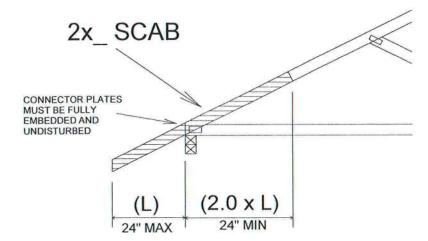
END BEARING CONDITION

NOTES:

1. ATTACH 2x_SCAB (MINIMUM NO.2 GRADE SPF, HF, SP, DF) TO ONE FACE OF TRUSS WITH TWO ROWS OF 10d (0.131" X 3") SPACED 6" O.C.

2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.

 WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf.

Trusses not fitting these criteria should be examined individually.

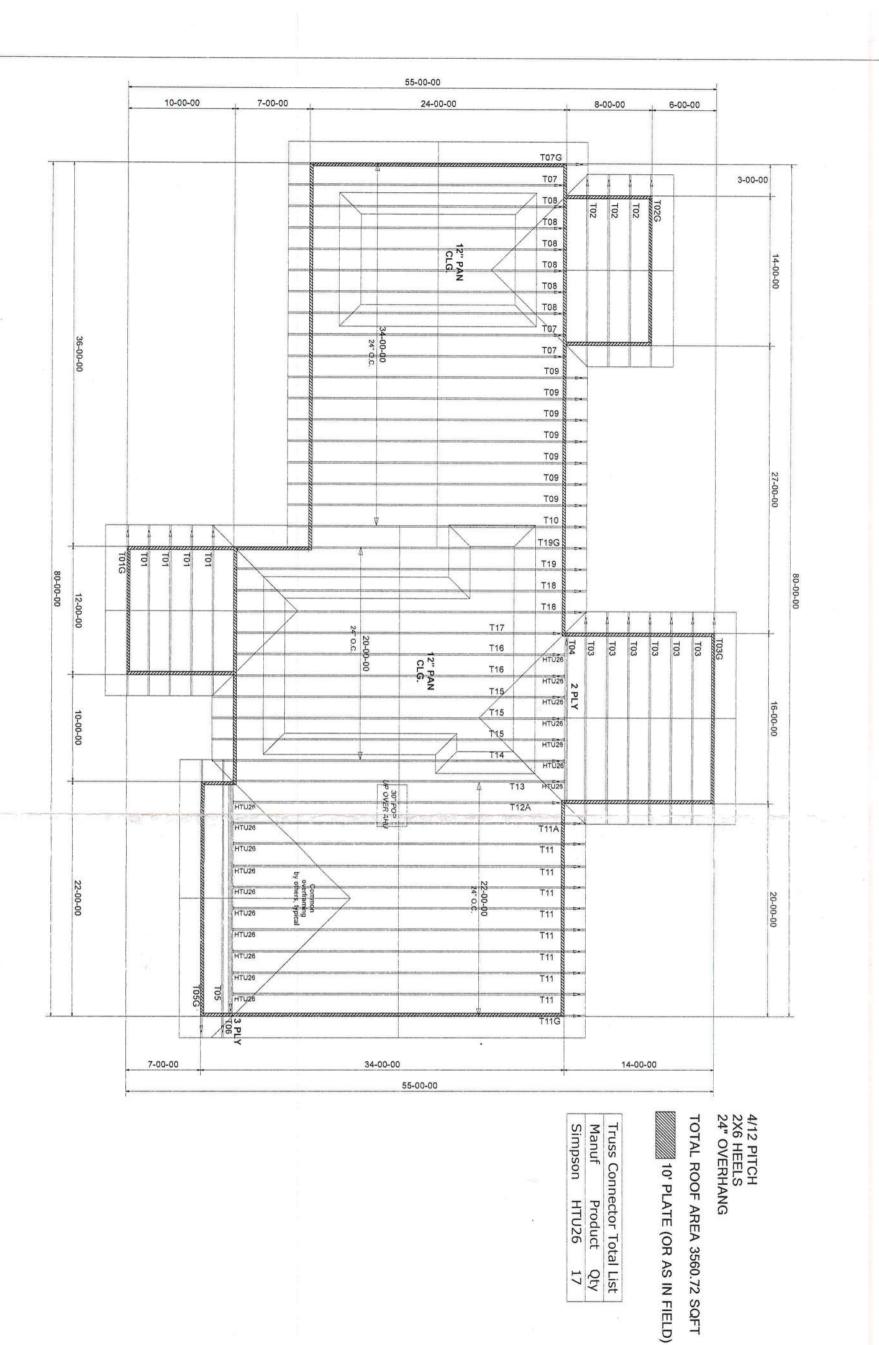
REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES



Michael S. Magid PE No.53681 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

May 17,2021





×× HATCH LEGEND SEE PLAN

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General Notes:

Per ANSI/IYI 1 2000 2 all "Truss to Wall"
connections are the responsibility of the Building
Designer, not the Truss Manufacturer.

Use Manufacturer's specifications for all hanger
connections unless noted otherwise.

Trusses are to be 24" o.c. U.N.O.

All hangers are to be Simpson or equivalent U.N.O.

Use Unit x 1.12" Nails in hanger connections to single
ply grider trusses.

Trusses are not designed to support brick U.N.O.

Dimensions are Feet luches Stateenths

No back charges will be accepted by Builders FirstSource unless approved in writing first.

ACQ lumber is corrosive to truss plates. Any ACQ lumber that comes in contact with truss plates (i.e. scabbed on tails) must have an approved barrier applied form

is the responsibility of the Contractor to ensure of the roper orientation of the truss placement plans as to the mstruction documents and field conditions of the ructure orientation. If a reversed or flipped layout is quired, it will be supplied at no extra cost by Puilders for to BCSI-B1 Summary Sheet-Guide for handling stalling and Bracing of Metal Plate Connected Wood uss prior to and during truss installation.

It is the responsibility of the Contractor to make sure the placement of trusses are adjusted for plumbing drops, can lights, cet..., so the trusses do not interfere with these type of items.

All common framed roof or thor systems must be designed as to NOT impose any hords on the floor trusses below. The floor trusses laive not been designed to carry any additional loads from above. This truss planement plan was not created by an ingineer, but rather by the Builders FirstSource staff and so solely to be used as an astallation guide and does not require a seal. Complete truss engineering and analysis an be found on the truss design drawings which may be easied by the truss design engineer.

able end trusses require continuous bottom chord suring. Refer to local codes for wall framing

Although all attempts have been made to do so, trusses may not be designed symmetrically. Please refer to the mily dual trues drawings and truss placement plans for

FirstSource Builders

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Lake City

WWW.BLDR.COM

Cooley Way, Lake City FL

Terrance Jones

6 - 29 - 23BPC Original Ref #:

M. Roberts Custom Ficor 2 Job #: 3576025

MITEK PRODUCT APPROVAL #'S FL2197-R6