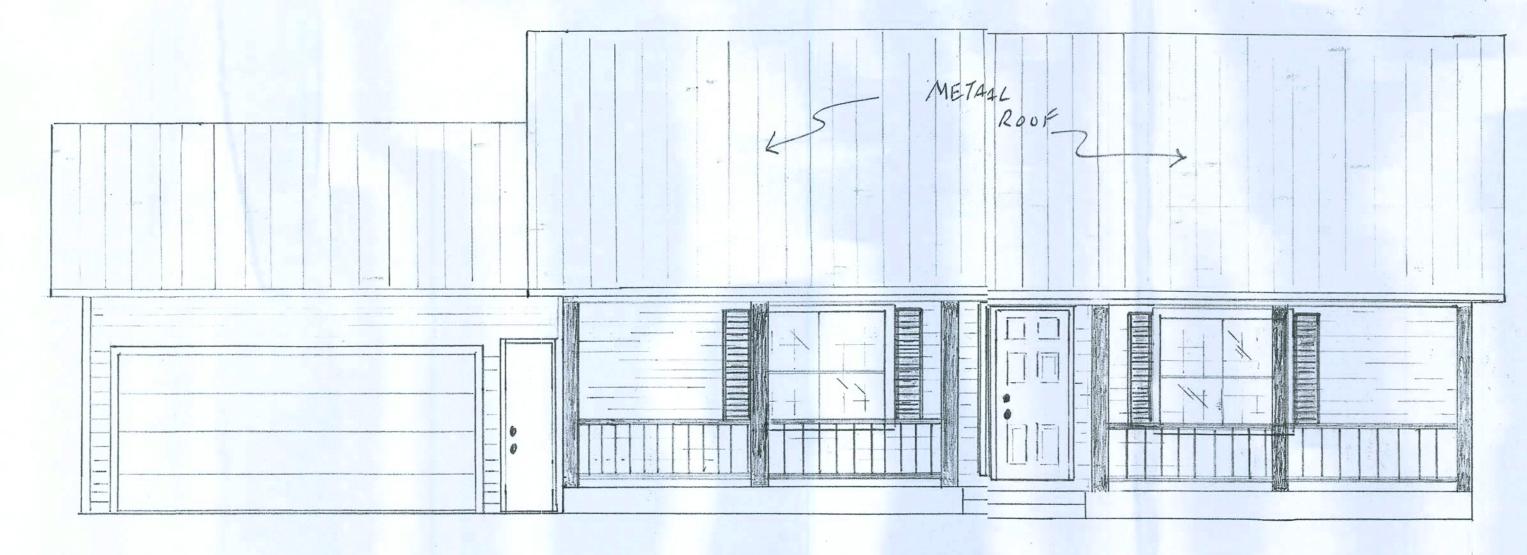




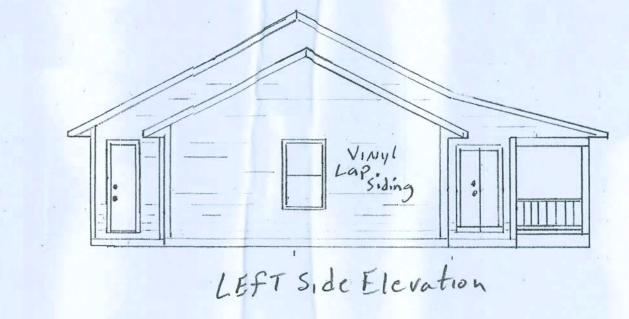
REAR ELEVATION



RIGHT SIDE ELEVATION

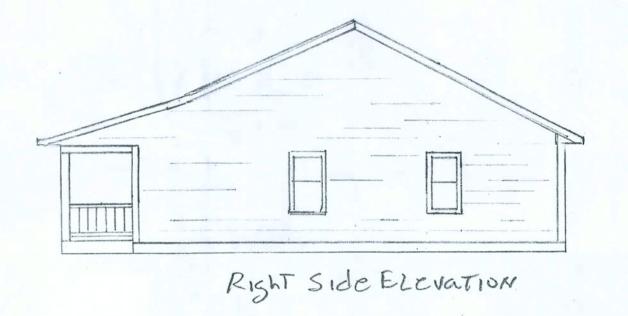


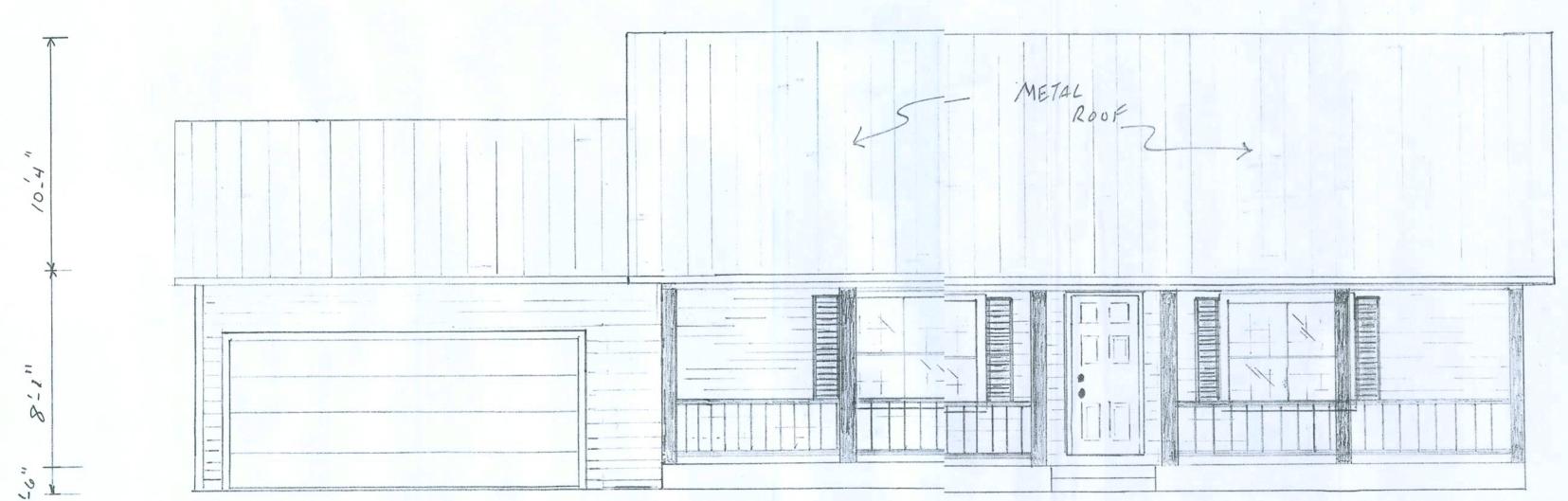
FRONT ELEVATION



Saulsby House Jay Milton Contractor James 5-5-09





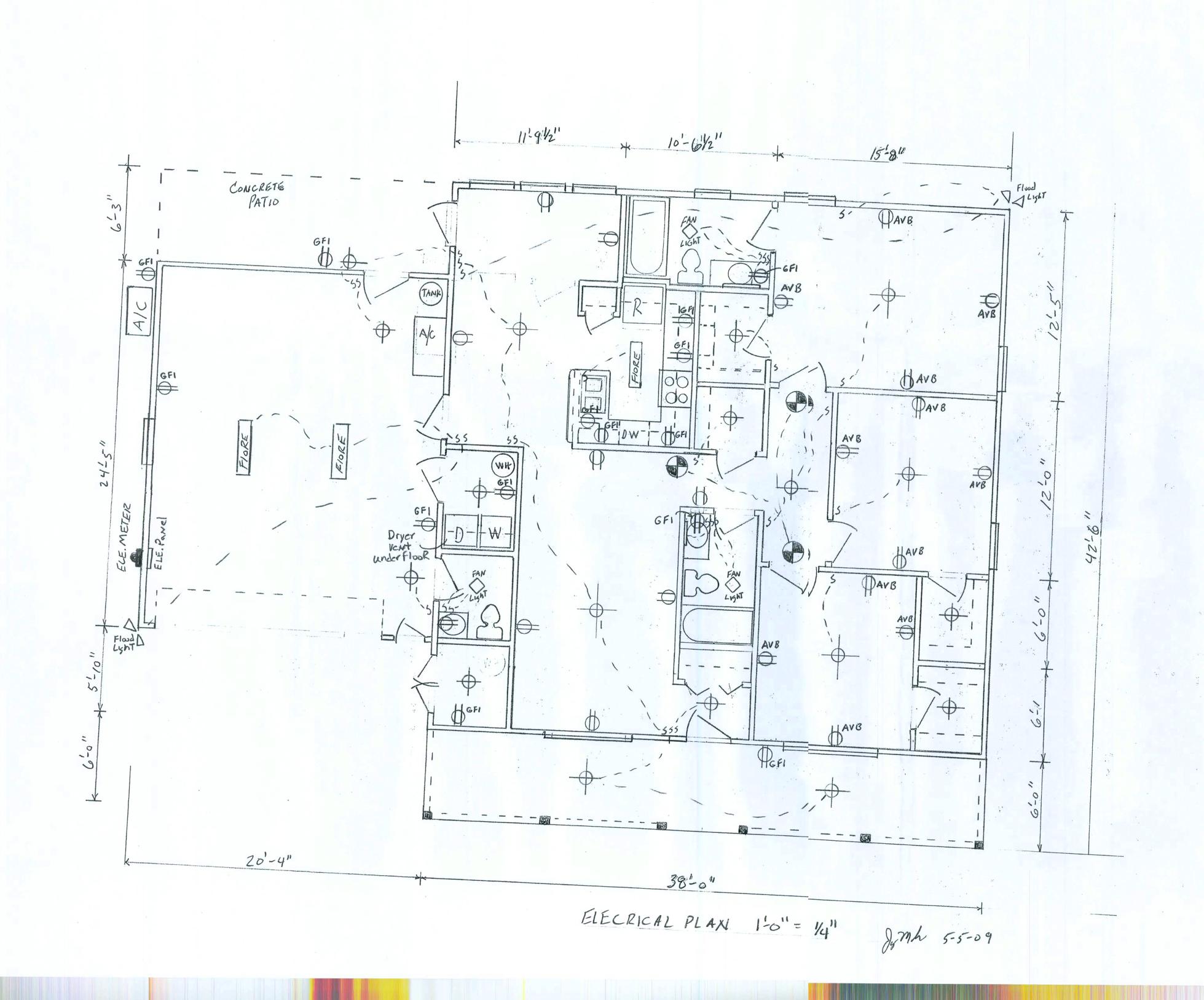


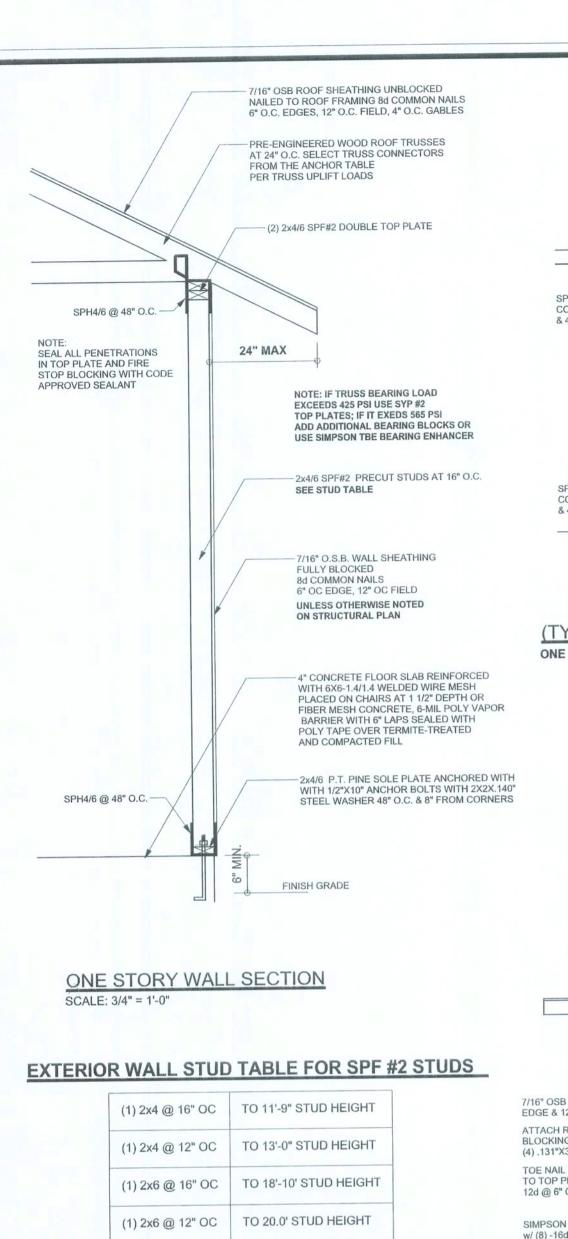
FRONT ELEVATITION
1-0"= 1/4"

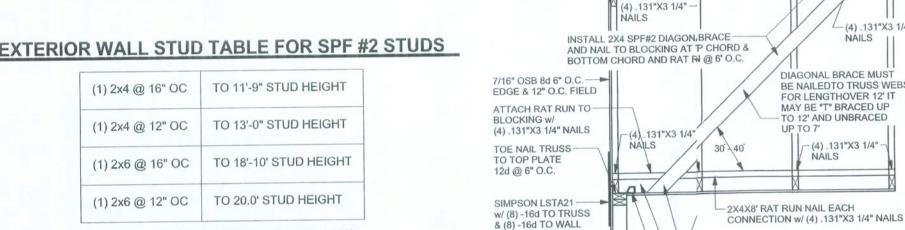




#27832 Revised Control Commoderate Bour



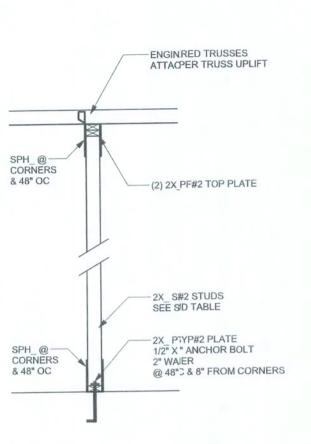




— 8d 6" OC @ PANEL EDGES 8d 12" OC NOT @ PANEL EDGES

@ 48" O.C. U.N.O.

THIS STUD HEIGHT TABLE IS PER WFCM 2001, TABLE 3.20B, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS RESISTING INTERIOR ZONE WINDLOADS 110 MPH EXPOSURE B STUD SPACINGS SHALL BE MULTIPLIED BY 0.85 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. EXAMPLE 16" O.C. x 0.85 = 13.6" O.C.



(TYP.) INTERIOR BERING WALL ONE STORY WOOD FRAME w/ STAPS & ANCHORS

—(4 OUTRIGGER @ 24" O.C.

-(4) .131"X3 1/4" NAILS

(8) .131"X3 1/4" NAILS

----2X4 SPF#2 BLOCKING

SPACE RAT RUN & DIAGONAL RACE 6'-0" O.C.

WOOD FRAME

FOR GABLE HEIGHT UP TO 25" 110 MPH, EXP. C, ENCLOSED

(TYP.) GABLE RACING DETAIL

I @ 6" OC EDGES, 12" OC FIELD, 4" OC GABLES

-OCKING REQUIRED BETWEEN OUT RIGGERS

-(4) .131"X3 1/4" -

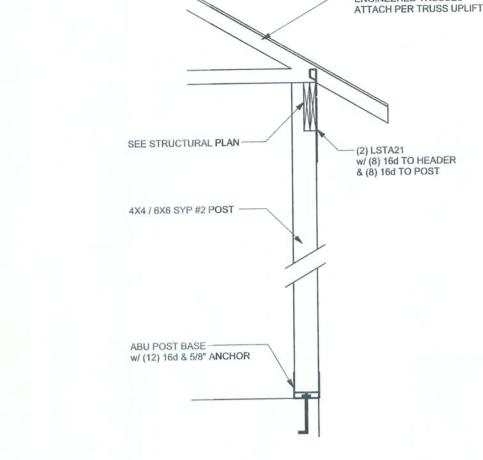
BE NAILEDTO TRUSS WEBS

FOR LENGTHOVER 12' IT

TO 12' AND UNBRACED

NAILS

1/1/

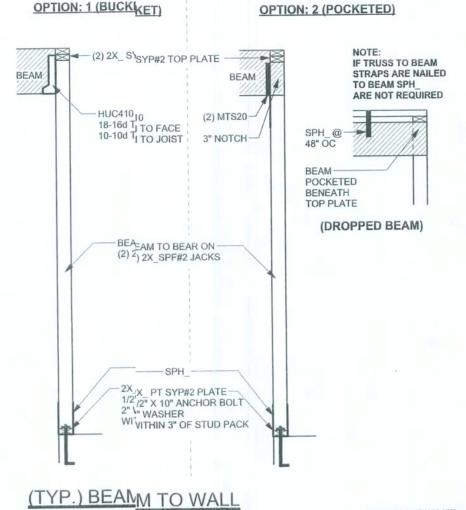


(TYP.) PORCH POST ONE STORY WOOD

2X4 OUTLOOKERS @ 24" OC -

(4) .131"X3 1/4" TOE NAILS

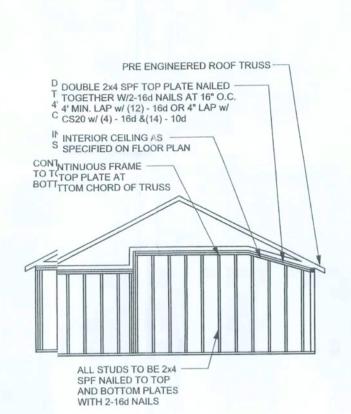
WOOD FRAME



GRADE & SPECIES TABLE

WOOD FRAME w/ S STRAPS & ANCHORS

		Fb (psi)	E (10 ⁶ psi)
2x8	SYP #2	1200	1.6
2x10	SYP #2	1050	1.6
2x12	SYP #2	975	1.6
GLB	24F-V3 SP	2400	1.8
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	1600	1.9
PSL	PARALAM	2900	2.0



CONTINUOUS FRAME TO CEILING DIAPHRAGM DETAIL

SCALE: N.T.S. IF TRUSS TO WALL STRAZAPS ARE NAILED TO THE HEADER THE SPIPH4/6 @ 48" O.C. ARE NOT REQUIRED

(TYP.) GABLE WALL w/ VAULTED CEILING

H3 EACH OUTLOOKER

- PLATE NAILED TO TRUSS

w/ .131X3 1/4" @ 6" OC

- EXTERIOR SHEATHING

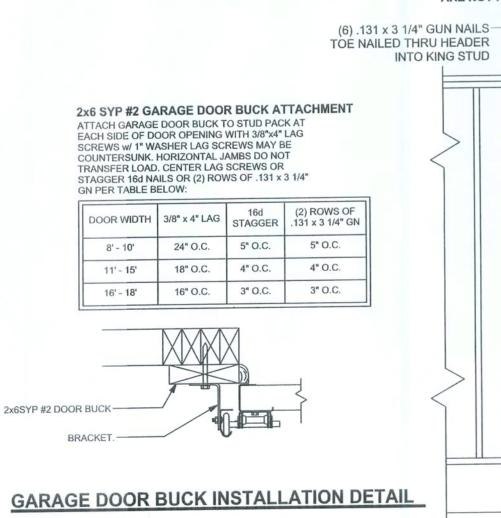
BETWEEN POINTS OF

LATERAL SUPPORT

SEE STUD TABLE

STUDS MUST BE CONTINUOUS

ROOF SHEATHING



-(6) .131 x 3 1/4" GUN NAILS TOE NAILED THRU HEADER SPH4/6 ALL OPENINGS (U.N.O.) INTO KING STUD INTO KING STUD SPH4/6 @ 48" O.C. (U.N.O.) CRIPPLES IF REQUIRED (4) .131 x 3 1/4" GUN NAILS TOE NAILED THRU SILL -INTO JACK STUD U.N.O. TYPICAL STRAPPING (U.N.O.) (SEE STRUCTURAL PLAN) -SPH4/6 ALL OPENINGS (U.N.O.)-(1) 2X6 SPF #2 SILL UP TO 11'-0" U.N.O.) 2X4 SPF #2 SILL UP TO 7'-3" U.N.O. (FOR: 110 MPH, 10'-0" WALL HIGHT U.N.O.)

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2007, TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" X 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'. FIBER CONCRETE SLABS: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD

PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER

TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL. CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO

DWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS. ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"0C INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

SPECIFICALLY	NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CO BACKFILL HEIGHT	IDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERI REQUIREMENTS I	LS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2007 OR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
BELIEVE THE PLA	IUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL IGINEER IMMEDIATELY.
DESIGN PLACEM	MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS NT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL INS.

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2007, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBC 2007 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

MASONRY NOTES:

ACI530.1-02 Section

1.4A Compressive strength

CMU standard

Clay brick standard

Movement joints

Reinforcing bars, #3 - #11

Coating for corrosion protection

Mortar

Grout

IN WRITING.

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL

CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY

MUST IMMEDIATELY, BEFORE PROCEDING, NOTIFY THE ENGINEER OF

ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS.

ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER

Specific Requirements

5.5"x2.75"x11.5"

or 304SS

Pipes, conduits, and accessories | Any not shown on the project drawings

2.4F Coating for corrosion protection | Joint reinforcement in walls exposed to

ASTM C 270, Type N, UNO

8" block bearing walls F'm = 1500 psi

ASTM C 476, admixtures require approval

ASTM C 90-02, Normal weight, Hollow,

medium surface finish, 8"x8"x16" running

ASTM C 216-02, Grade SW, Type FBS,

ASTM 615, Grade 60, Fy = 60 ksi, Lap

splices min 48 bar dia. (30" for #5)

Anchors, sheet metal ties completely

embedded in mortar or grout, ASTM

A525, Class G60, 0.60 oz/ft2 or 304SS

moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or

Contractor assumes responsibility for type

and location of movement joints if not

grout, ASTM A153, Class B2, 1.50 oz/ft2

require engineering approval.

detailed on project drawings.

bond and 12"x12" or 16"x16" column

STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON

DESIGN DATA

ANCHOR TABLE

< 455

< 455

< 415

< 600

< 950

< 745

< 1465

< 1465

< 760

< 1470

< 1470

< 1000

< 1450

< 2900

< 2050

< 3965

< 10980

< 10530

< 9250

< 455

< 825

< 825

< 885

< 1240

< 1240

< 1235

< 1235

< 1030

< 1705

< 2310

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

MANUFACTURER'S ENGINEERING

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

< 265

< 235

< 365

< 535

< 820

< 565

< 1050

1050

< 850

< 655

< 1265

< 1265

< 860

< 1245

1785

< 6485

< 9035

< 420

< 825

< 760

< 1065

< 1165

< 1235

< 1030

< 1705

< 1305

< 2310

< 2570

< 3695

< 1400

< 3335

< 2300

< 2320

H2.5

H2.5A

H14-1

H10-1

H10-2

H16-2

HTS24

2 - HTS24

HEAVY GIRDER TIEDOWNS

MGT

HGT-2

HGT-3

HGT-4

SSP DOUBLE TOP PLATE

SSP SINGLE SILL PLATE

DSP SINGLE SILL PLATE

SP4

SPH6

LSTA18

LSTA21

CS20

CS16

STUD ANCHORS

LTTI31

HD2A

HTT16

HPAHD22

ABU66

ABU88

DSP DOUBLE TOP PLATE

TO PLATES TO RAFTER/TRUSS

4-8d

4-8d

4-8d

5-8d

8-8d

5-10d, 1 1/2

12-8d, 1 1/2"

12-8d, 1 1/2'

8-8d, 1 1/2"

6-10d

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

22 -10d

16 -10d

16 -10d

16 -10d

4-8d

4-8d

4-8d

5-8d

8-8d

5-10d, 1 1/2"

13-8d

15-8d

8-8d, 1 1/2"

6-10d

7-10d 1 1/2"

12-10d 1 1/2"

14 -16d

14-10d

16-10d

18-8d

28-8d

TO STUDS

8-16d

18-10d, 1 1/2"

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

12-16d

18 - 16d

10-10d, 1 1/2" 2-10d, 1 1/2"

TO STUDS

TO FOUNDATION

12" EMBEDMENT

5/8" THREADED ROL

12" EMBEDMENT

/8" THREADED ROI

12" EMBEDMENT

TO STUDS

4 -10d

4-10d

8-10d

8 -10d

6-10d, 1 1/2"

10-10d, 1 1/2"

6-10d, 1 1/2"

10-10d, 1 1/2"

TO FOUNDATION

1/2" AB

1/2" AB

5/8" AB

5/8" AB

1/2" AB

1/2" AB

2-5/8" AB

1 19.9 -21.8 18.1 -18.1

2 | 19.9 | -25.5 | 18.1 | -21.8

3 | 19.9 | -25.5 | 18.1 | -21.8

4 21.8 -23.6 18.5 -20.4

5 21.8 -29.1 18.5 -22.6

Doors & Windows 21.8 -29.1

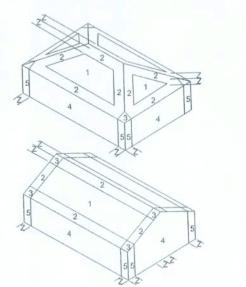
-40.6

2 O'hg -40.6

3 O'hg -68.3

12" EMBEDMENT

	IND LOADS PER FLORIDA BUILDING CODE 2007 RESIDENTIAL, SECTION R301.	
ME	NCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE R EAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 F N UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND LOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER	T; NO >10%
BUI	JILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE	
BUI	JILDING IS NOT IN THE WIND-BORNE DEBRIS REGION	
1.)) BASIC WIND SPEED = 110 MPH	
2.)) WIND EXPOSURE = B	
3.)) WIND IMPORTANCE FACTOR = 1.0	
4.)) BUILDING CATEGORY = II	
5.)) ROOF ANGLE = 10-45 DEGREES	
6.)) MEAN ROOF HEIGHT = <30 FT	
7.)) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)	
8.)) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2	(2))
	Zone Effective Wind Area	(ft2)
	75	



3/2	2	Worst Case (Zone 5, 10 ft2)		
5	2 3	8x7 Garage Door	19.5	-22.9
2	4 3 5	16x7 Garage Door	18.5	-21.0
	3 4 12			
	55 22		-	
	2×2			
DESIGN	LOADS			
FLOOR	40 PSF (ALL OTHER DWELLING ROOMS)			
	30 PSF (SLEEPING ROOMS)			
	30 PSF (ATTICS WITH STORAGE)			
	10 PSF (ATTICS WITHOUT STORAGE, <3:12)			
ROOF	20 PSF (FLAT OR <4:12)			
	16 PSF (4:12 TO <12:12)			
	12 PSF (12:12 AND GREATER)			
STAIRS	40 PSF (ONE & TWO FAMILY DWELLINGS)			
SOIL BE	ARING CAPACITY 1000PSF			

REVSIONS

SOFTPLAN

E No.5396, POB 868, Lake City, FL 2056, 386/54-5419 ensions Refer all questions to not proced without clarification. PYRIGHTS AND PROPERTY RIGHTS:

ark Disosvay, P.E. hereby expressly rese monlaw copyrights and property right i se instruments of service. This document ot to be reroduced, altered or copied in any m or mainer without first the express written ission and consent of Mark Disosway. RTIFICATION: I hereby certify that I hav

amined this plan, and that the applicable rtions of he plan, relating to wind engin omply with section R301.2.1, florida building ode residential 2007, to the best of my

IMITATIOI: This design is valid for one ouilding, atspecified location.

MARK DISOSWAY P.E. 53915

Milton Builders

Sailsby Residence

ADDRESS: Lake Jeffrey Rd. Clumbia County, Florida

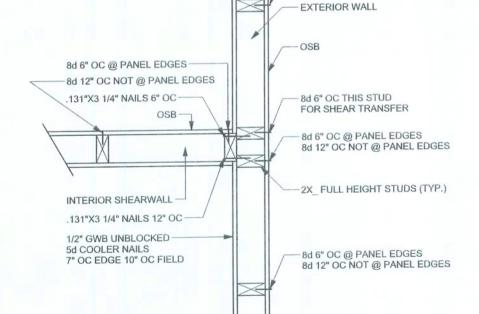
Mark Disosway P.E. P.O. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

PRINTED DATE: May 06, 2009 STRUCTURAL BY DRAVN BY: David Disosway

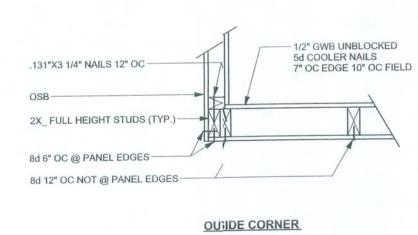
FINALS DATE: 6May99 JOB NUMBER: 905043

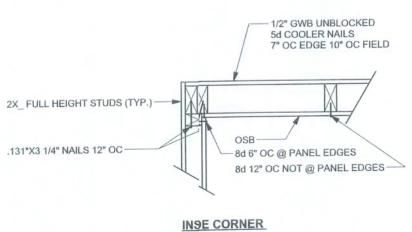
DRAWING NUMBER

OF 3 SHEETS

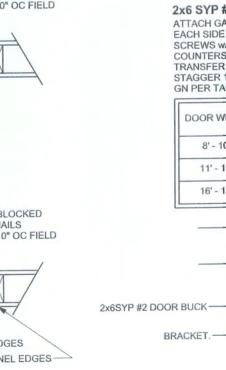


(TYP.) INTERSECTING WALL FRAMING WOOD FRAME



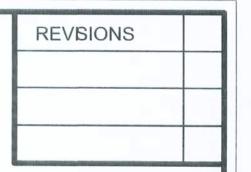


(TYP.) CORIER FRAMING WOOD FRAME

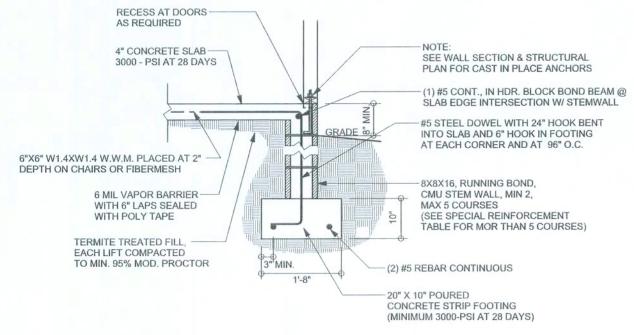


SCALE: N.T.S.

TYPICALL HEADER STRAPING DETAIL







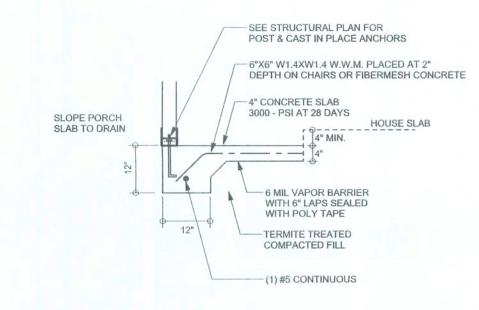
STEM WALL FOOTING

SCALE: 1/2" = 1'-0"

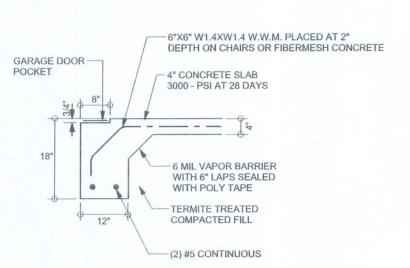
TALL STEM WALL TABLE

T table assumes 60 ksi reinforcing bars with 6" hook in the footing and bent 24" into the rforced slab at the top. The vertical steel is to be placed toward the tension side of the CJ wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall iser 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond bm with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used w reinforcement as shown in the table below.

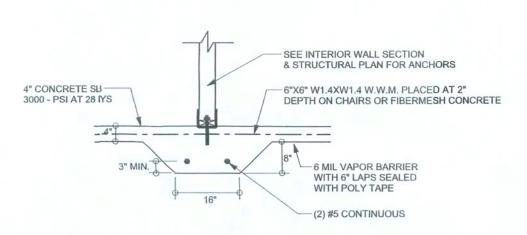
EMWALL IEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)		VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)			
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48



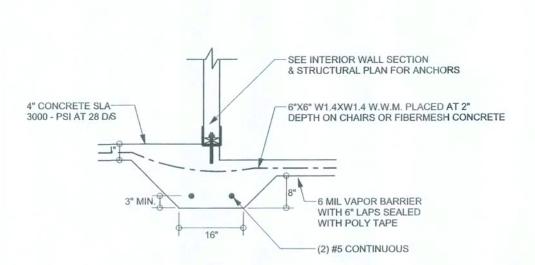




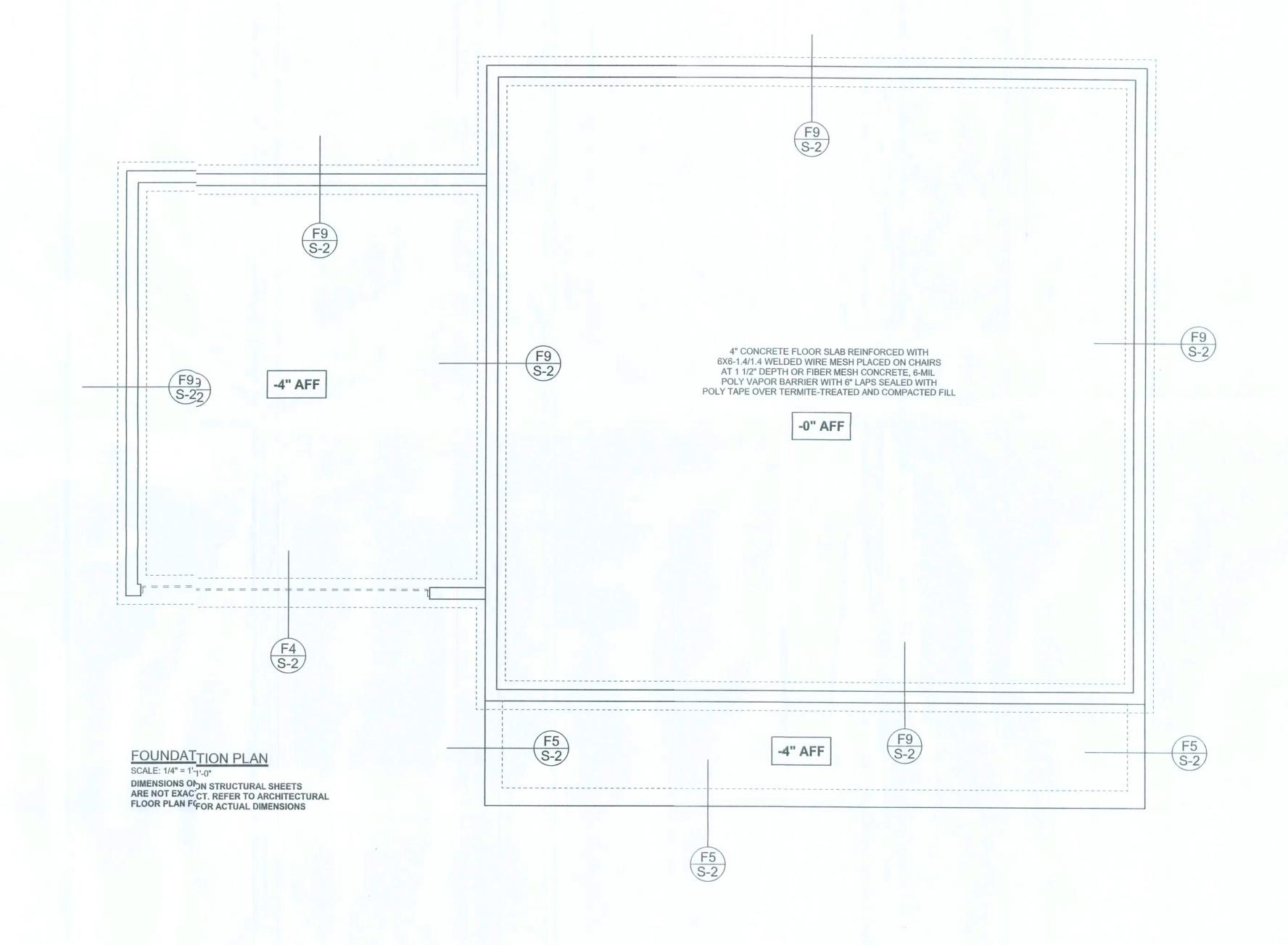




F2 INTERIOR BEARING FOOTING S-2 3CALE: 1/2" = 1'-0"



ITERIOR BEARING STEP FOOTING SALE: 1/2" = 1'-0"

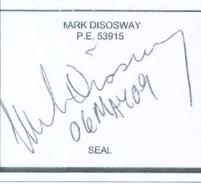


WINDLOAD EIGINEER: Mark Disosway, PE No.53915, IOB 868, Lake City, FL 32056, 386-75-5419 DIMENSIONS:

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CERTIFICATION: I hereby certify that I have examined this lan, and that the applicable portions of the lan, relating to wind engineerin comply with setion R301.2.1, florida building code residentia 2007, to the best of my

LIMITATION: This design is valid for one building, at speiffed location.



Milton Builders

Sauliby Residence

Iake Jeffrey Rd. Colunbia County, Florida

ADDRESS:

MarkDispsway P.E. PO. Box 868 Lake City, Florida 32056 Phone: (386) 754 - 5419 Fax: (386) 269 - 4871

'RINTED DATE: May 06, 2009 DRAWN B':

STRUCTURAL BY David Disosway

FINALS DATE: 6May09

JOB NUMBER:

905043 DR\WING NUMBER

S-2 OF 3 SHEETS

USE H2.5A (4880Ib) FOR ALL TRUSS TO FRAME WALL AND PORCH BEAM CONNECTION IS UNLESS NOTED OTHERWISE A3 B1 - (2) SPH_ TOP & BOTTOM-SEE BEAM TO WALL DETAIL W ANCHOR BOLT WITHIN 3" (2) 1.75"X12.00"X16'LVL,3J 2K UPLIFT (2) 2X12X6',2J 2K SEE BEAM TO WALL DETAIL SEE PORCH POST DETAIL (TYP.)

STRUCTURAL PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL PLAN NOTES

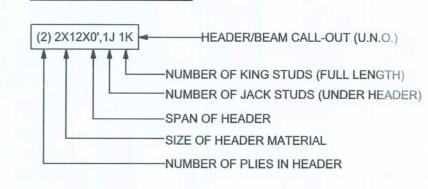
- SN-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SYP #2 (U.N.O.)
- SN-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- SN-3
 DIMENSIONS ON STRUCTURAL SHEETS
 ARE NOT EXACT. REFER TO ARCHITECTURAL
 FLOOR PLAN FOR ACTUAL DIMENSIONS
- PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS.

 LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

WALL LEGEND

EXTERIOR V WALL
INTERIOR N NON-LOAD BEARING WALL
INTERIOR L(LOAD BEARING WALL W/ NO UPLIFT
INTERIOR L'(LOAD BEARING WALL W/ UPLIFT

HEADER LEGEND



TOTAL SHEAR WALL SEGMENTS INDICATES SHEAR WALL SEGMENTS

	REQUIRED	ACTUAL
TRANSVERSE	36.5'	56.0'
LONGITUDINAL	29.5'	59.5'

REVISIONS

SOFTPLAN

WINDLOAD :NGINEER: Mark Disosway, PE No.5391t POB 868, Lake City, FL 32056, 386-74-5419

DIMENSION:
Stated dimersions supercede scaled dimensions, tefer all questions to Mark Disoswy, P.E. for resolution.
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CERTIFICATON: I hereby certify that I have examined thi plan, and that the applicable portions of the plan, relating to wind engineerin comply with ection R301.2.1, florida building code residenal 2007, to the best of my

code residenal 2007, to the best knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY
P.E. 53915

SEAL

Mlton Builders

Sausby Residence

ADDRESS: Lake Jeffrey Rd. Columbia County, Florida

Mark Disosway P.E. P.O. Box 868 Lake ¢ity, Florida 32056 Phon∈ (386) 754 - 5419 Fax:(386) 269 - 4871

> PRINTED DATE: August 24, 2009

IY: STRUCTURAL BY:
David Disosway

FINALS DATE:

JCB NUMBER: 905043

S-3
OF 3 SHEETS

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB # 9-104 ANDERSON TRUSS