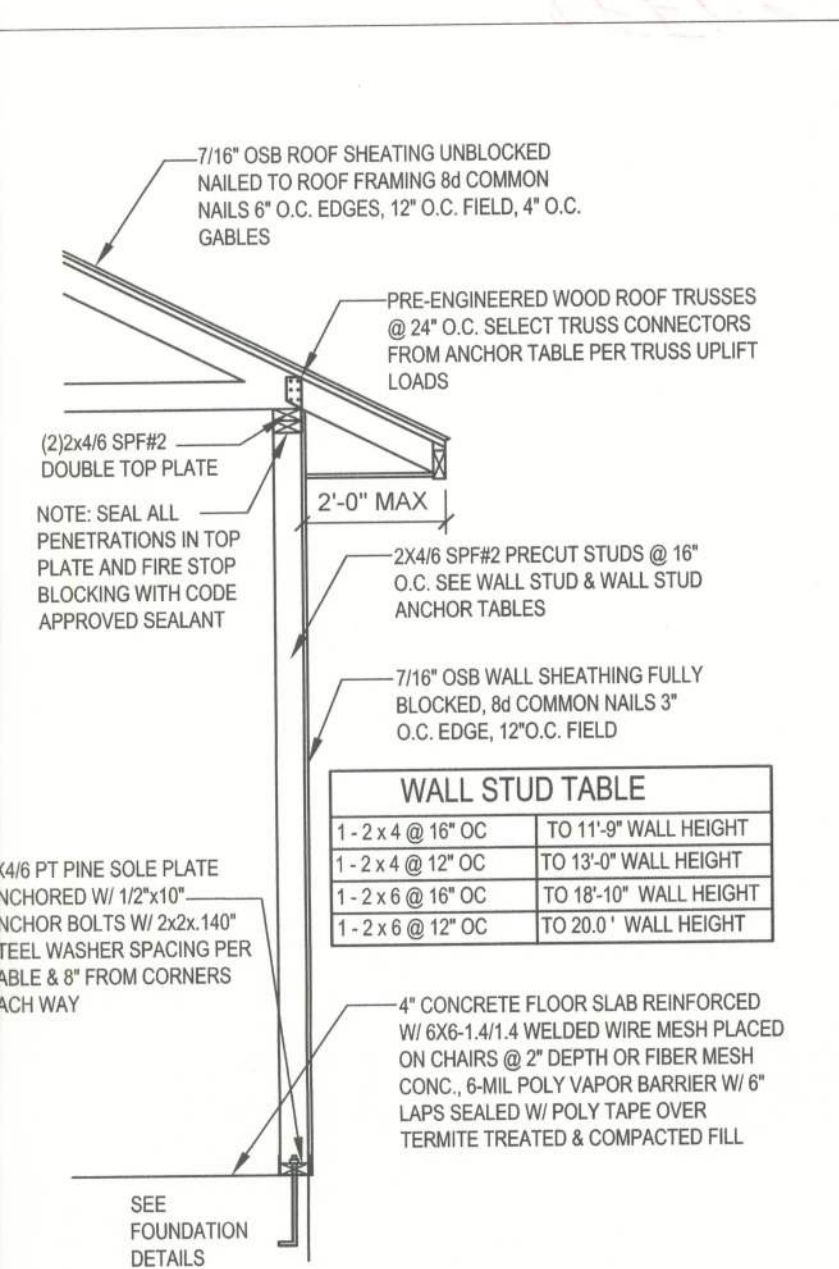




F1 - STEM WALL FOUNDATION
SCALE: 1/2"=1'-0" REV-27-MAY-03

F2 - PORCH SLAB
SCALE: 1/2"=1'-0" REV-22-AUG-03

F4 - INTERIOR BEARING FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03

W13-TYPICAL HEADER SIZING & STRAPING DETAIL
SCALE: N.T.S. REV-22-AUG-03

W13-TYPICAL HEADER SIZING & STRAPING DETAIL
SCALE: N.T.S. REV-22-AUG-03

W13-TYPICAL HEADER SIZING & STRAPING DETAIL
SCALE: N.T.S. REV-22-AUG-03

SPECIAL STUD ANCHOR TABLE
(USE ONLY WITH 7/16" OSB SHEATHING FULLY BLOCKED NAILED WITH 8d @ 3" O.C. ON EDGES)

TYPICAL TRUSS UPLIFT & MAX 12'-0" WALL HEIGHT	ANCHOR BOLT SPACING	SP4 / SP6 SPACING	ALTERNATE SPACING
800 LB	48" O.C.	48" O.C.	N/A
1070 LB	48" O.C.	48" O.C.	N/A
1250 LB	48" O.C.	32" O.C.	N/A
1570 LB	32" O.C.	16" O.C.	32" O.C.
1800 LB	24" O.C.	16" O.C.	16" O.C.
2200 LB	LTT01 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

NOTE: SP2 TOP & SP1 BOTTOM ALTERNATE FOR SP4/6

NOTE: MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.

W3 - SINGLE STORY EXT. WALL SECTION
SCALE: 1/2"=1'-0" REV-22-AUG-03

F3 - GARAGE DOOR POCKET
SCALE: 1/2"=1'-0" REV-22-AUG-03

F12 - NON - BEARING STEP FOOTING
SCALE: 1/2"=1'-0" REV-10-FEB-03

F5 - INTERIOR BEARING STEP FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03

W25 - INTERIOR SHEAR WALL DETAIL
SCALE: 1/2"=1'-0" REV-11-JUN-03

W71 - HEADER SPANS FOR ROOF/CEILING LOAD

N2-GENERAL NOTES:

FOUNDATION: FOR POINT LOADS GRATER THAN 5000 lb OR REPETITIVE TRUSS LOADS GRATER THAN 2000 lb PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0" X 1 sq ft. FOR EVERY 1000 lb OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE $F_c = 3000$ PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED FABRIC: 6" x 6" W1.4 X W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT W/M OR REINFORCING STEEL. RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTORS APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

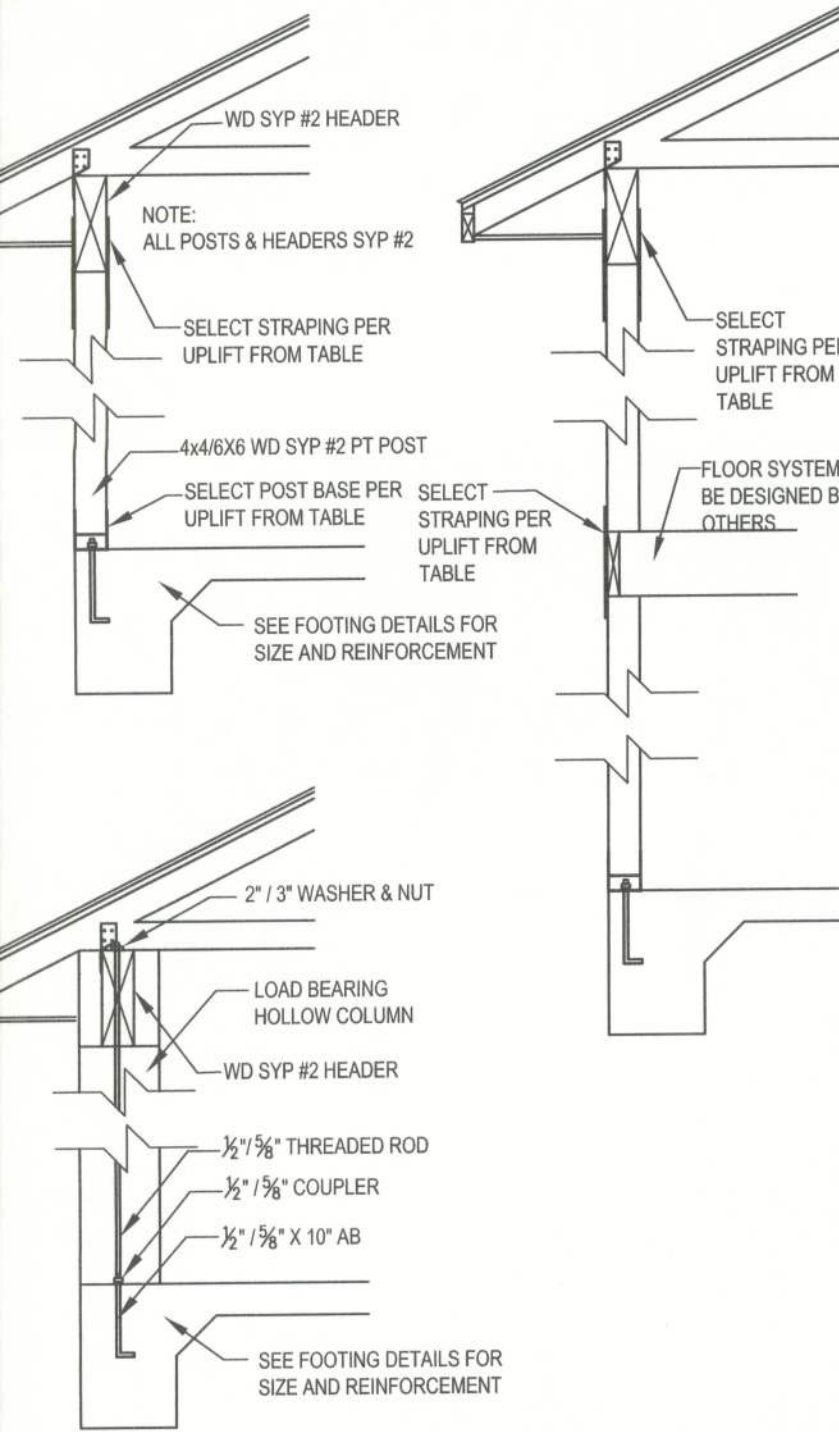
REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI. ALL LAPS SPLICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-85 WITH ACI 315-86 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 23 INCHES.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES, MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED CONCRETE BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 3" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16";

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.



SYP #2 FT WOOD POSTS

TYPICAL POST UPLIFT	POST BASE ANCHOR	BETWEEN FLOOR STRAPPING	HEADER STRAPPING
550 LB	AB44 W/ (8)-10d A 3/4" AB	(2) LST421 W/ (8)-10d EA	(2) LST421 W/ (8)-10d EA
720 LB	AB46 W/ (8)-16d 5/8" AB	(2) LST421 W/ (8)-10d EA	(2) LST421 W/ (8)-10d EA
2200 LB	AB44 W/ (12)-16d (2) 3/4" x 10" AB	(2) LST421 W/ (8)-10d EA	(2) LST421 W/ (8)-10d EA
2300 LB	AB46 W/ (12)-16d (2) 3/4" x 10" AB	(2) LST421 W/ (8)-10d EA	(2) LST421 W/ (8)-10d EA

HOLLOW COLUMN

1/2" x 10" AB ATTACHED TO 2" THREADED ROD WITH 2" COUPLER THROUGH COLUMN & HEADER WITH 2" WASHERS & NUT TOP

3/4" x 10" AB ATTACHED TO 2" THREADED ROD WITH 2" COUPLER THROUGH COLUMN & HEADER WITH 2" WASHERS & NUT TOP

W12 - PORCH HEADER ANCHORS
SCALE: N.T.S. REV-18-JUL-03

N5 - TRUSS UPLIFT CONNECTOR TABLE REV-18-NOV-04

All connectors are Simpson Strongtie, uno. Seld top and bottom connections from this table or SST catalog to meet truss uplift. Use fasteners as specified.

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter
320	455	H3	4-8d	4-8d
245	350	H5A	3-8d	3-8d
535	600	H2-5A	5-8d	5-8d
620	720	H10	6-10dx 1/2"	6-10dx 1/2"
850	990	LTS12	8-8dx 1/2"	8-8dx 1/2"
1245	1450	HTS20	10-10dx 12-10dx 1/2"	10-10d or 12-10dx 1/2"
1265	1470	H16, H16-2	10-10dx 1/2"	2-10dx 1/2"
1785	2050	LGT2	14-10dx Sinker	16-10d Sinker
3655	4200	MG1	3/4" Th Rod	22-10d
SPF	SYP	Strap Connector	To Member	To Other Member
760	885	SP4	6-10dx 1/2"	N/A
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d
1085	1265	LSTA18-24	7-10d	7-10d
1170	1360	SPH4	12-10dx 1/2"	N/A
420	455	SSP	4-10d	3-10d to double plate or 1-10d to single
600	825	DSP	6-10d	6-10d to double plate or 2-10d to single
1420	1650	CS16	14-8d or 11-10d	14-8d or 11-10d
SPF	SYP	Column Anchor	To Foundation	To Column / Truss
1160	1350	LTT19	3/4" x 10" AB	8-16d Sinker
1985	2310	LTT01	3/4" x 10" AB	16-10dx 1/2"
2385	2775	HC2A	3/4" x 10" AB	2-3/4" Bolts
3590	4175	HTT16	3/4" x 10" AB	16-16d
1975	2300	ABU66	3/4" x 10" AB	12-16d

Shots Supporting Trusses: The builder is responsible for gravity loads, but you should not use a shot and under truss bearing location for each 2000 lb of truss. Check the minimum bearing requirements of the truss top plate (SPF, Fc=1200-12300/800). Manufacture and product number are listed for example not endorsement. If equivalent device of the same or other manufacturer can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads. All connections exposed directly to the weather shall be hot dipped galvanized after fabrication. Loads are increased for wind duration. Strap uplift may be reduced proportionally to number of nails. Specifier sheet for alternate nail sizes (10d=8d*16d, 10d=12d*16d, 10d=16d*16d) SPF=38F99

N4-WIND LOAD DESIGN DATA

(Wind loads are per FBC 2001, Section 1606.2 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with >10% slope.)

Basic Wind Speed	110 MPH
Wind Exposure	B
Wind Importance Factor	1.0
Building Category	II
Internal pressure Coefficient	N/A (Enclosed)
Building not in the high velocity hurricane zone	
Building not in the wind-borne debris region	
Mean Roof Height	< 30 ft
Roof Angle	10-45 degrees
Components And Cladding Wind Pressures (FBC Table 1606.2 B&C)	
Zone	Effective Wind Area (ft ²)
4	21.8 -23.6 18.5 -20.4
5	21.8 -29.1 18.5 -22.6
Total Shear Wall Segments	
2'-4" min for 8'-0" wall	2'-10" min for 12'-0" wall
Transverse	Longitudinal
Required 41.5	35.4
Actual 18.9	35.2

All exterior walls are type I shear walls. ACTUAL SHEAR WALL length is the total of all wall segments with full height sheathing and width to height ratio greater than 1:3.5 plus special shear wall segments (if noted). REQUIRED SHEAR WALL length is from WFCM-2001, table 3.7A & 3.7B with table 3.7E adjustment for type II shear wall (or equivalent calculation) REV-27-Jun-03

N11-WINDLOAD ENGINEER'S SCOPE OF WORK: The wind load engineer is engineer of record for compliance of the structure to wind load requirements of FBC 2001, Section 1606. If trusses are used, the wind load engineer is not engineer of record for the trusses and not design the trusses or delegate to the truss designer.

BUILDER'S RESPONSIBILITY: The builder and owner are responsible for the following, which are specifically not part of the wind load engineer's scope of work.

- * Confirm that the foundation design & site conditions meet gravity load requirements (assume 2000 PSF bearing capacity unless visual observation or soils test proves otherwise)
- * Provide materials and construction techniques, which comply with FBC 2001 requirements for the stated wind velocity and design pressures.
- * Provide a continuous load path from roof to foundation. If you believe the plan omits a continuous load path connection, call the wind load engineer immediately.
- * Verify the truss engineering includes truss design, placement plans, temporary and permanent bracing details, truss-to-truss connections, and load reactions for all bearing locations.
- * Select uplift connections, walls, columns, and footings based on truss engineering bearing locations and reactions, including interior bearing walls.
- * Size headers for gravity loads, headers sized by the builder for gravity loads will also satisfy wind loads.

DOCUMENT CONTROL AND PRIORITY: Structural requirements on S-1 control unless the building code or architectural sheets have more stringent requirements. Non-structural requirements on architectural sheets control. Specific requirements take precedence over general requirements. Revision control is by the latest signature date and is the responsibility of the builder.

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WINDLOAD ENGINEER: Mark Disosway, PE No.53915

CERTIFICATION: The attached plans and "Windload Engineering", sheet S-1, comply with FBC 2001, Section 1606 wind loads, to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location. This drawing is not valid for construction unless raised seal is affixed.

REV-27-Jul-04

WINDLOAD ENGINEERING

"EVERYTHING YOU NEED FOR YOUR BUILDING PERMIT"

Mark Disosway P.E.

POB 868, Lake City, FL 32056 Phone: (386) 754-5419
Fax: (386) 754-6749 Email: windloadengineer@bellsouth.net

Location: Lot 12a Country Lane Estate S/D Columbia County, Florida

Timothy Morgan Residence

Builder:

Designer:

Approved: FLPE#53915

Revisions:

Sheet S-1 of 5 Shee

Windload Engineering

Job # 506014

22Jun05

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